

BORING NOTES

All samples and vanes are made ahead of casing

✚ Water elevation

↘ Number of blows required to drive extra heavy casing one foot with 400 ft. lbs. of energy per blow

▮ Location of sample or sample attempt

Number and type of dry sample

ID 5 & H Sampler #1290's

IC 2" O.D. 16 ga. seamless tubing

IU 3 1/2" O.D 16 ga. seamless tubing

IW Wash sample and number

MD Unsuccessful sample attempt and type of sampler

⊥ Number of blows required to drive spoon or tubing one foot with 350 ft. lbs. of energy per blow

H Sampling spoon or seamless tubing driven by static weight of drill rods and hammer

P Piston sampler

▮ Field vane test

▲ Bottom of boring (may not be bottom of soil strata)

⬆ Refusal of drill rods or casing (may not be ledge)

▨ 75% Locations cored by diamond bit and per cent recovery of rock

SHEAR NOTES

● Field vane shear strengths

X Laboratory vane shear strengths

→ Shear strengths in excess of capacity of equipment

○ One half unconfined compressive strengths

WATER CONTENT NOTES

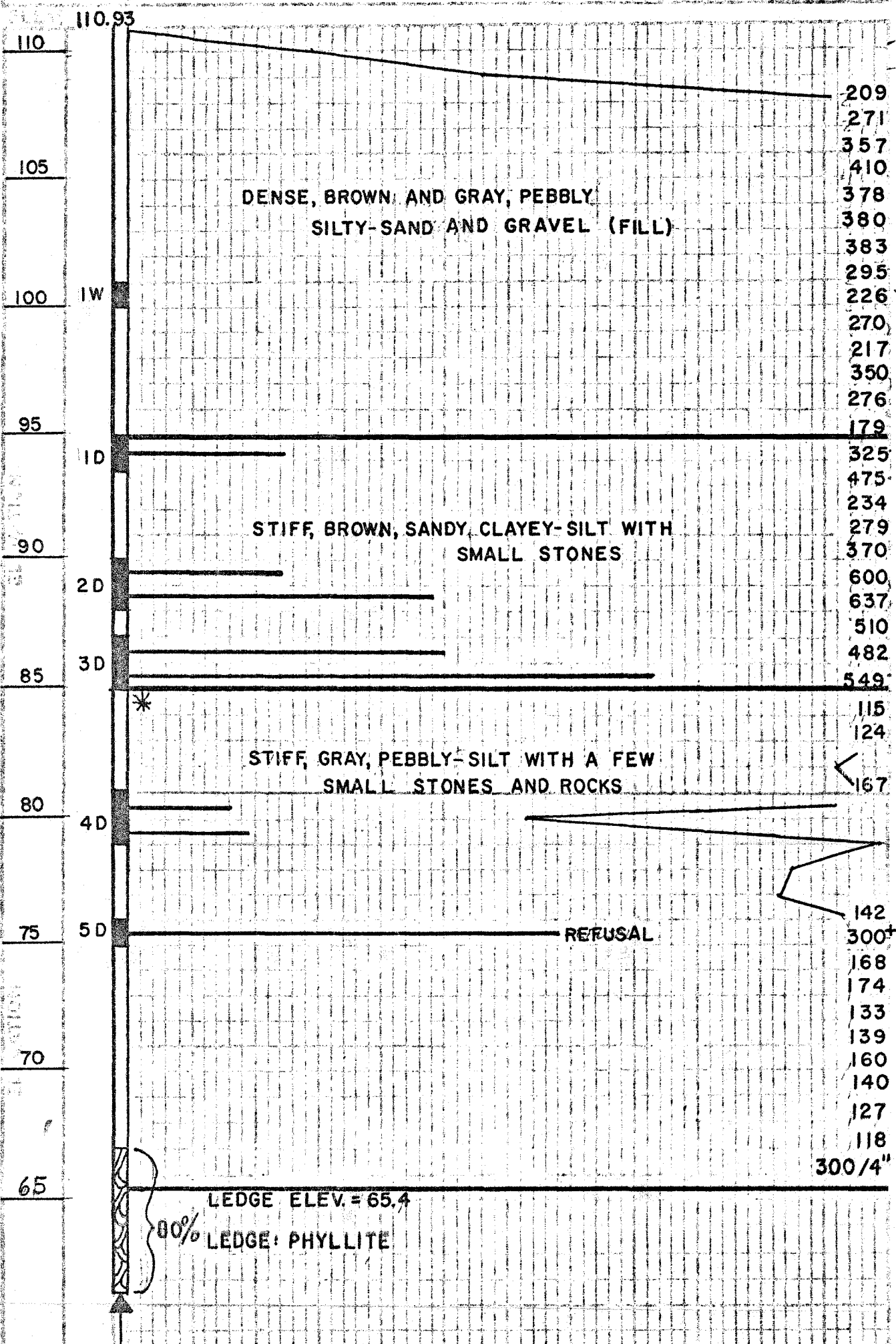
○ Natural water contents, given as per cent of dry weight

●---X Plastic and liquid limits

Ignition losses are given as per cent of dry weight

BORING CB-12-82 STATION 66+70, 30' LT. (RT. IAC)

CASING SIZE	DRIVING RESISTANCE	Blows/Ft.
2 1/2", 4"	20 40 60 80 100	



EXAMINATION OF BORING LOGS
MADE BY THE DIVISION
DETAILED SOIL STRATIFICATION

CONSISTENCY DATA
BORING CB-12-82

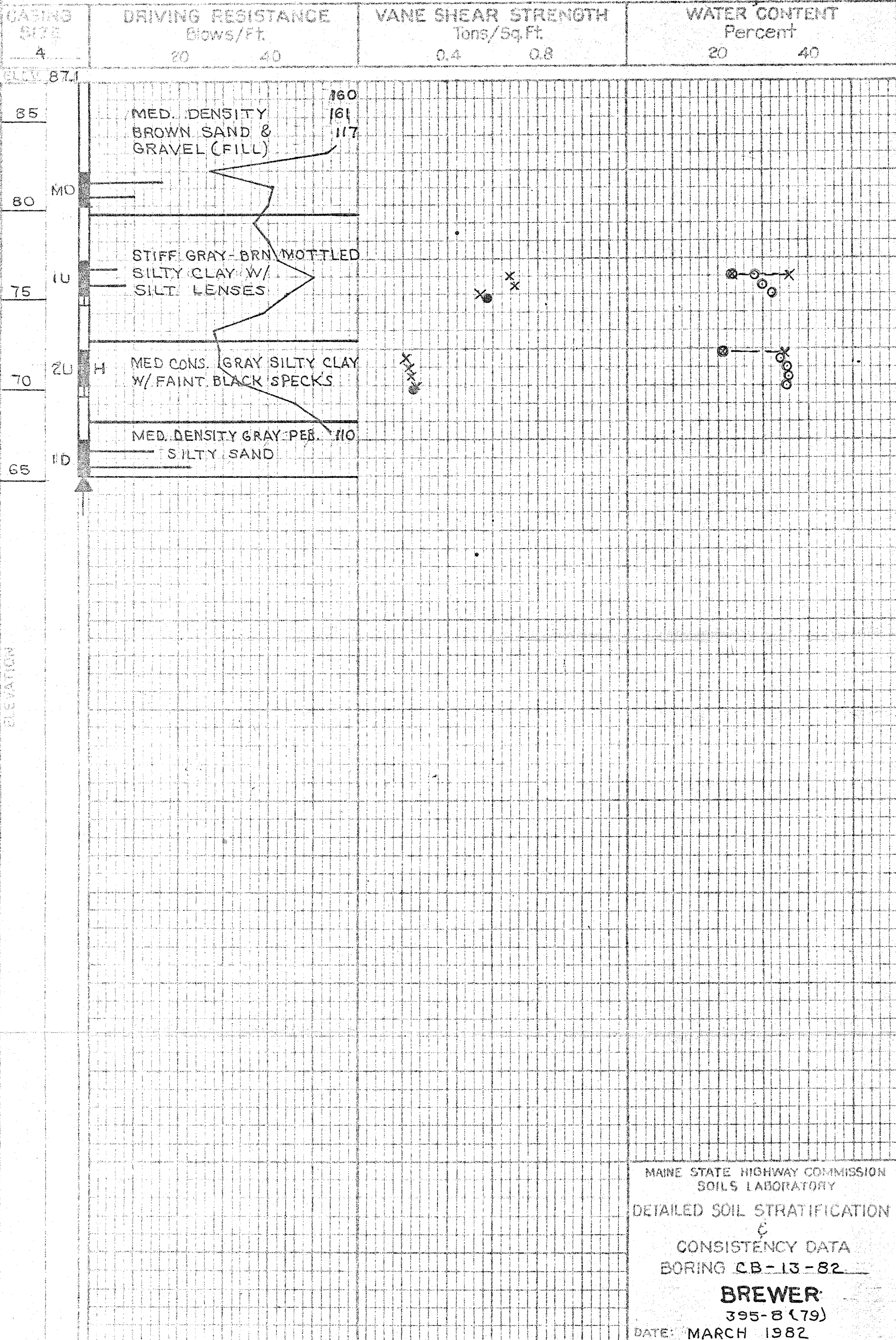
BREWER
395-8(79)

DATE: MARCH 1982

* CHANGED TO 2 1/2" CASING

BORING CB-13-82

STATION 57+50 40' RT.

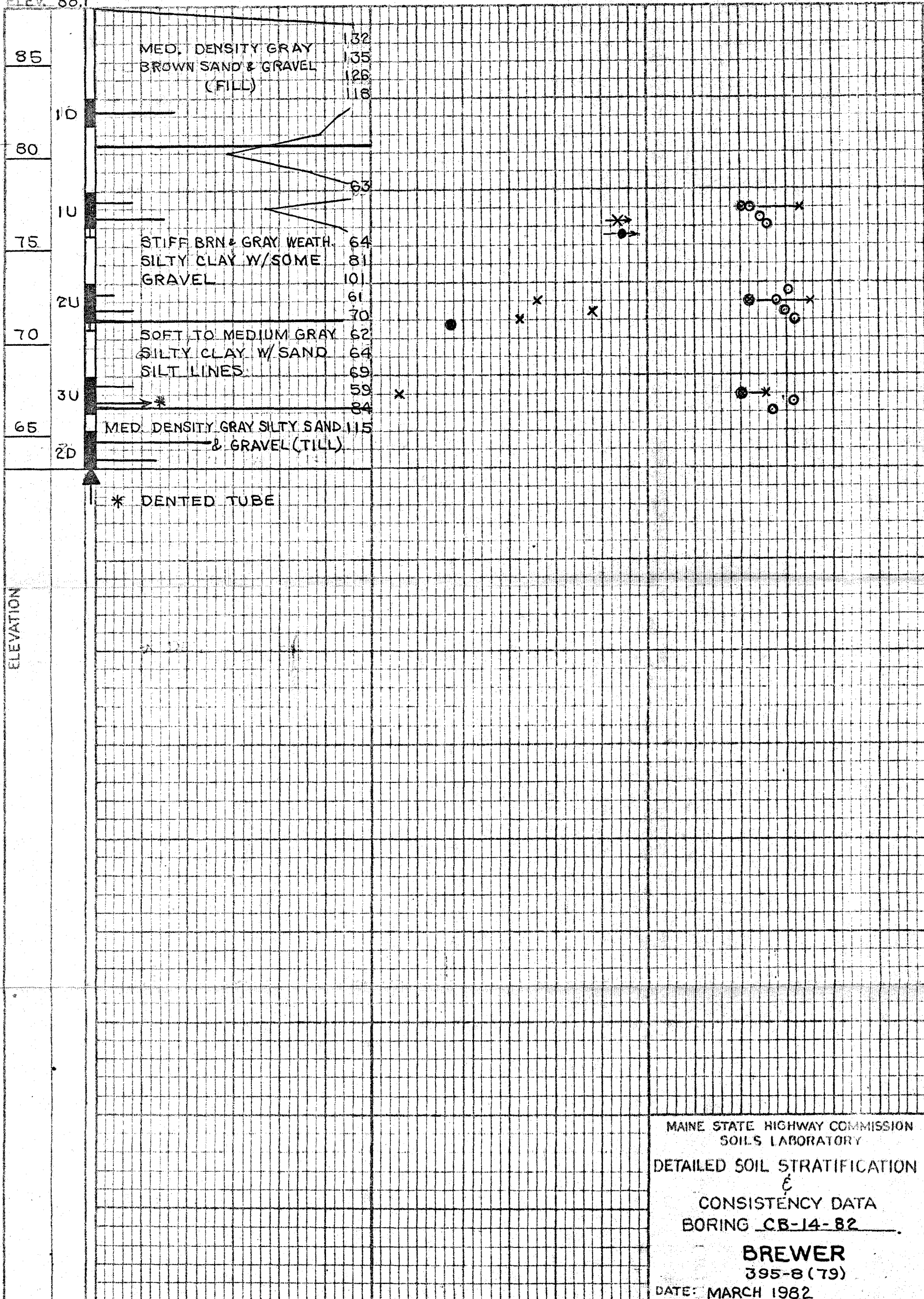


BORING CB-14-82

STATION 57+50, 33' LT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq.Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

ELEV. 88.1

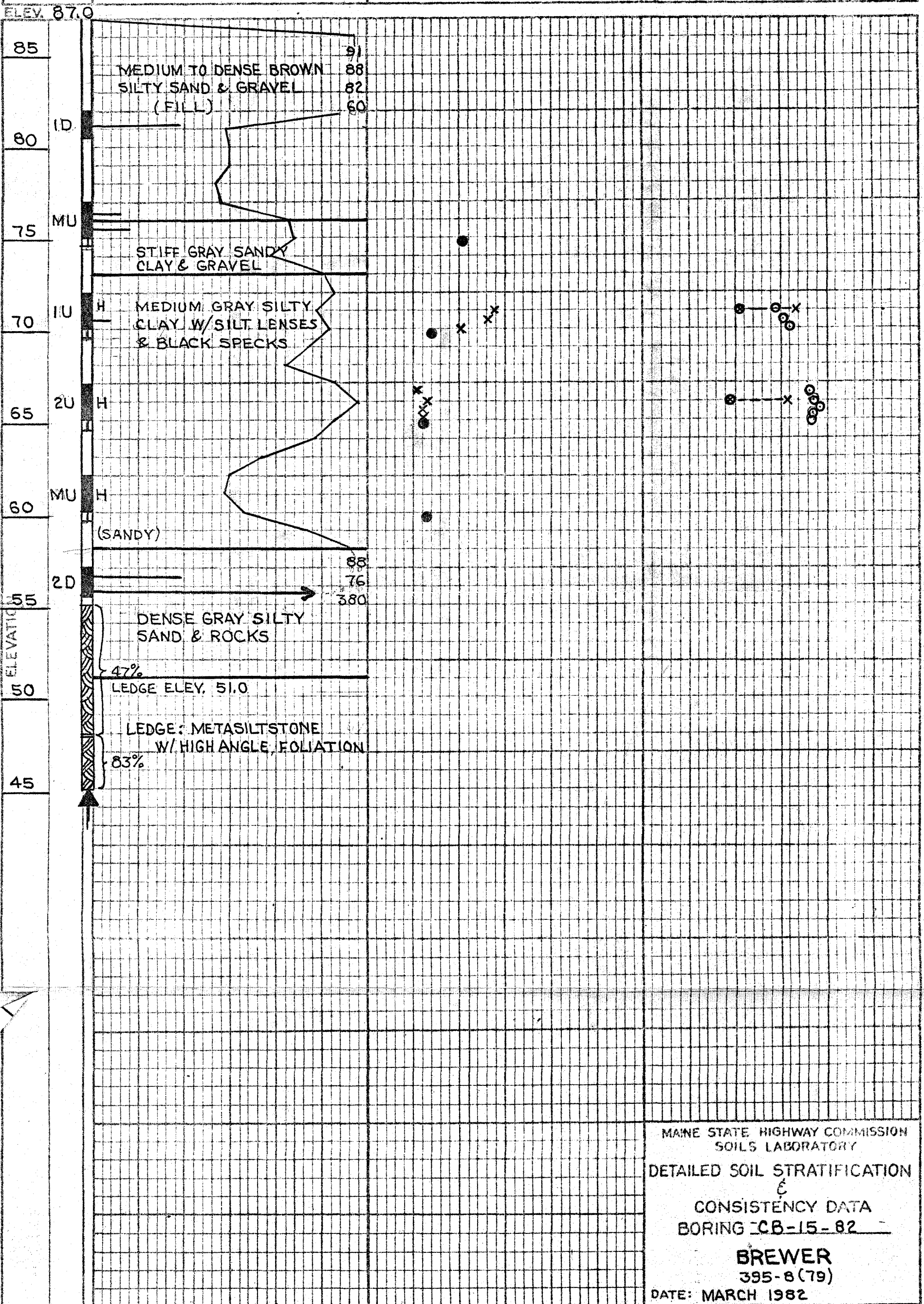


MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING CB-14-82
BREWER
395-8 (79)
DATE: MARCH 1982

BORING CB-15-82

STATION 55+15 30' RT.

CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

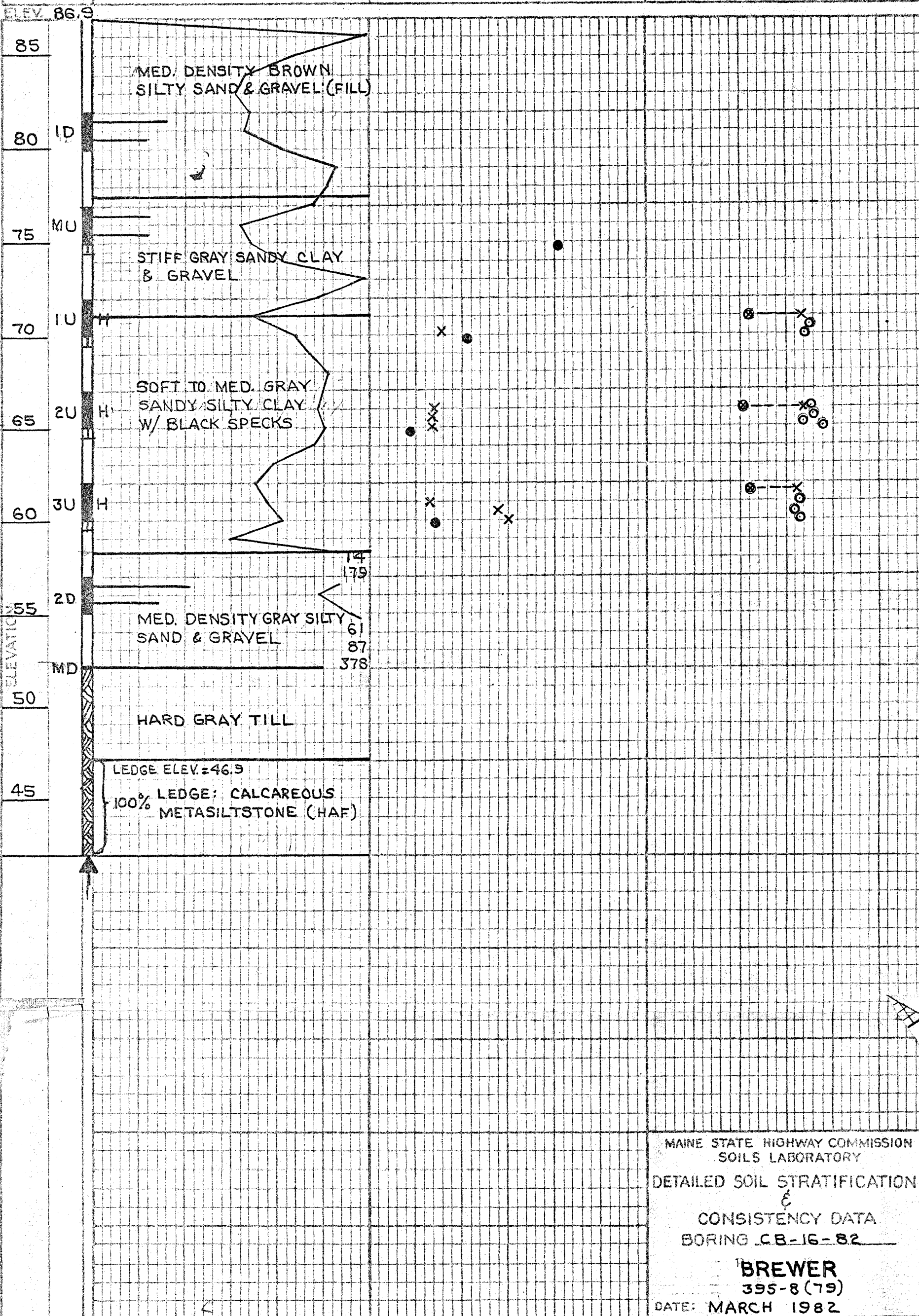


BORING CB-16-82

STATION 54+65

33' LT.

CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq.Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

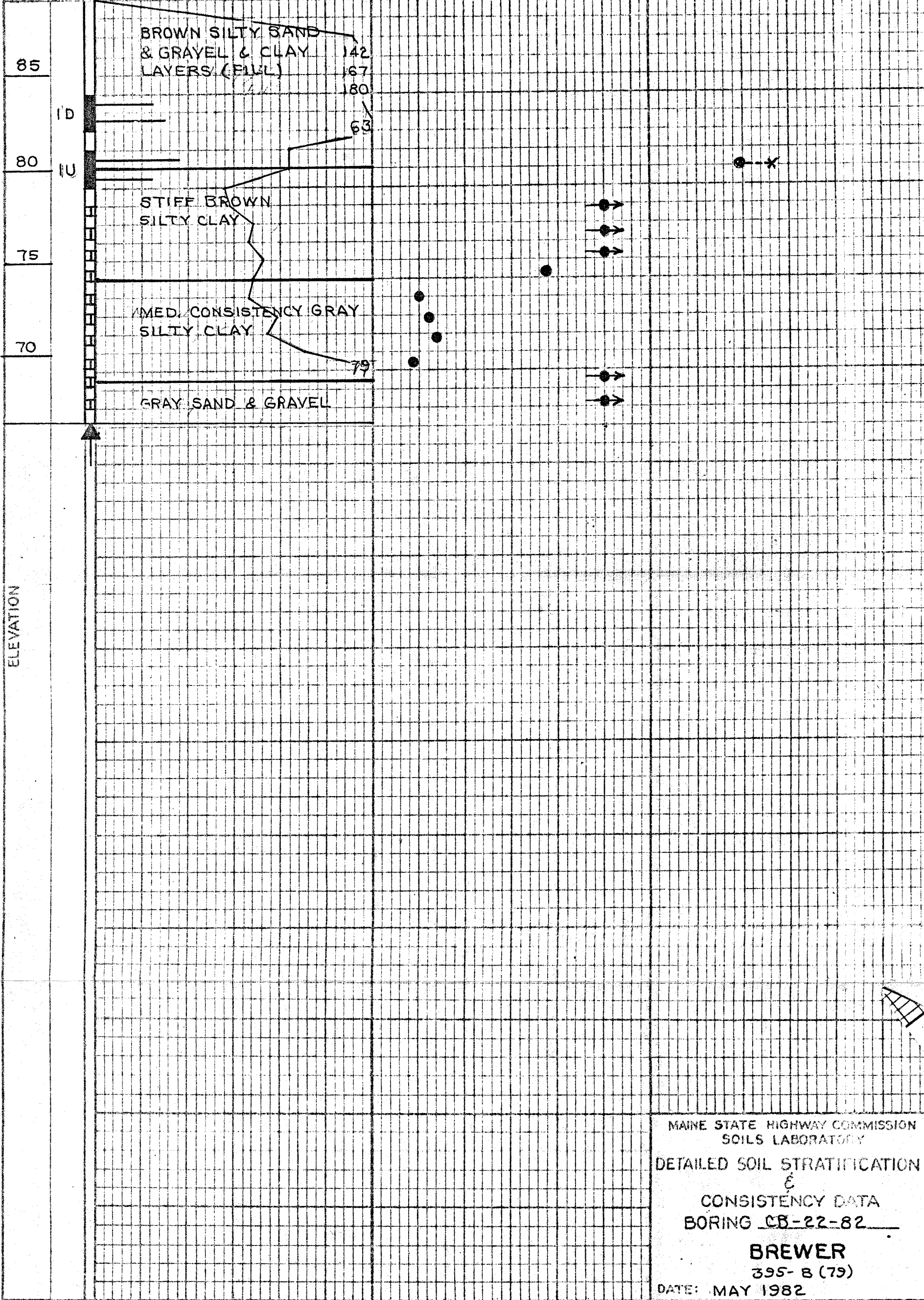


BORING CB-22-82

STATION 58+48.34 RT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq.Ft.	WATER CONTENT Percent
4"	20 40	0.4 0.8	20 40

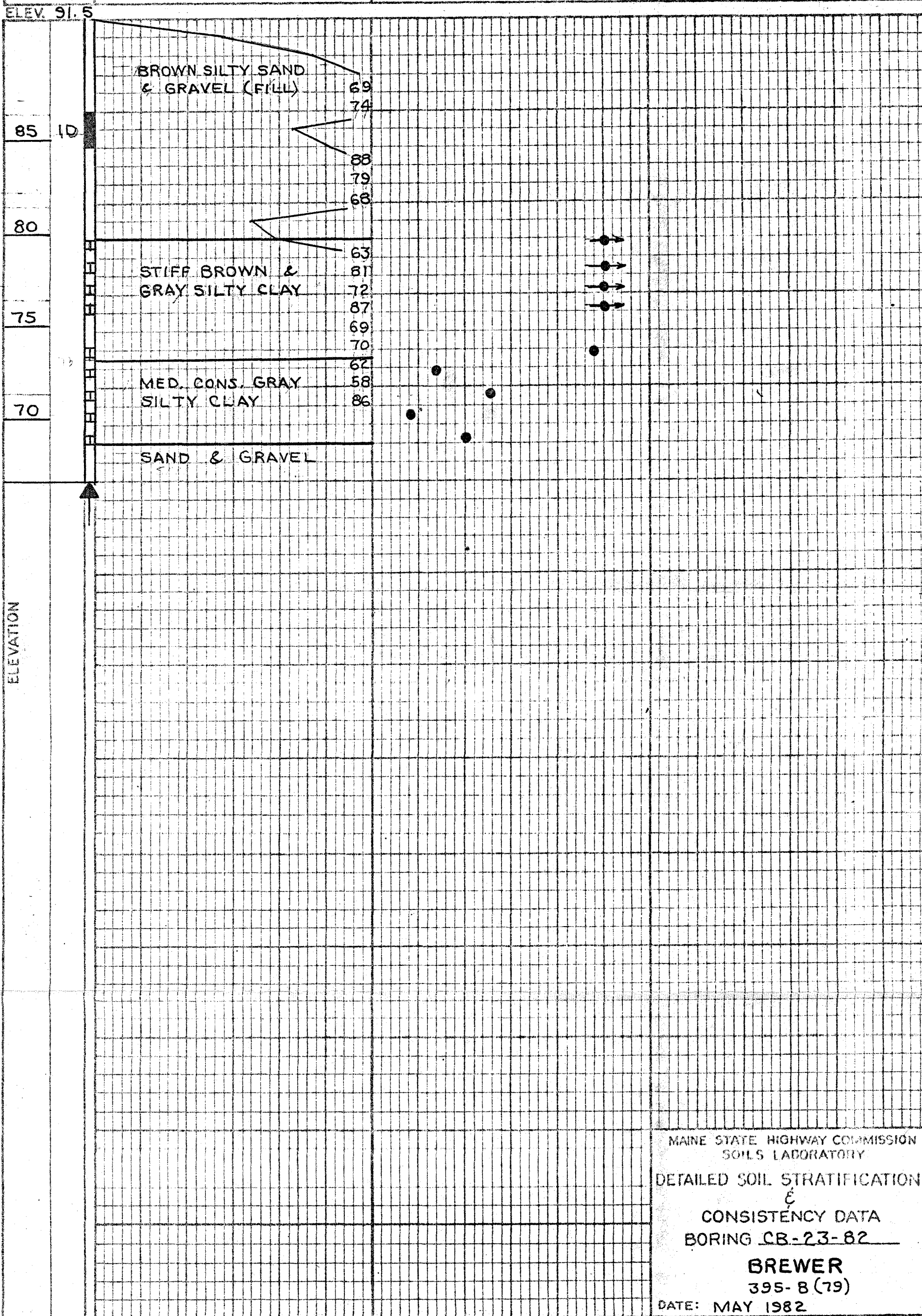
ELEV. 88.6



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING CB-22-82
BREWER
395- B (79)
DATE: MAY 1982

BORING CB-23-82 STATION 61+23.37 RT.

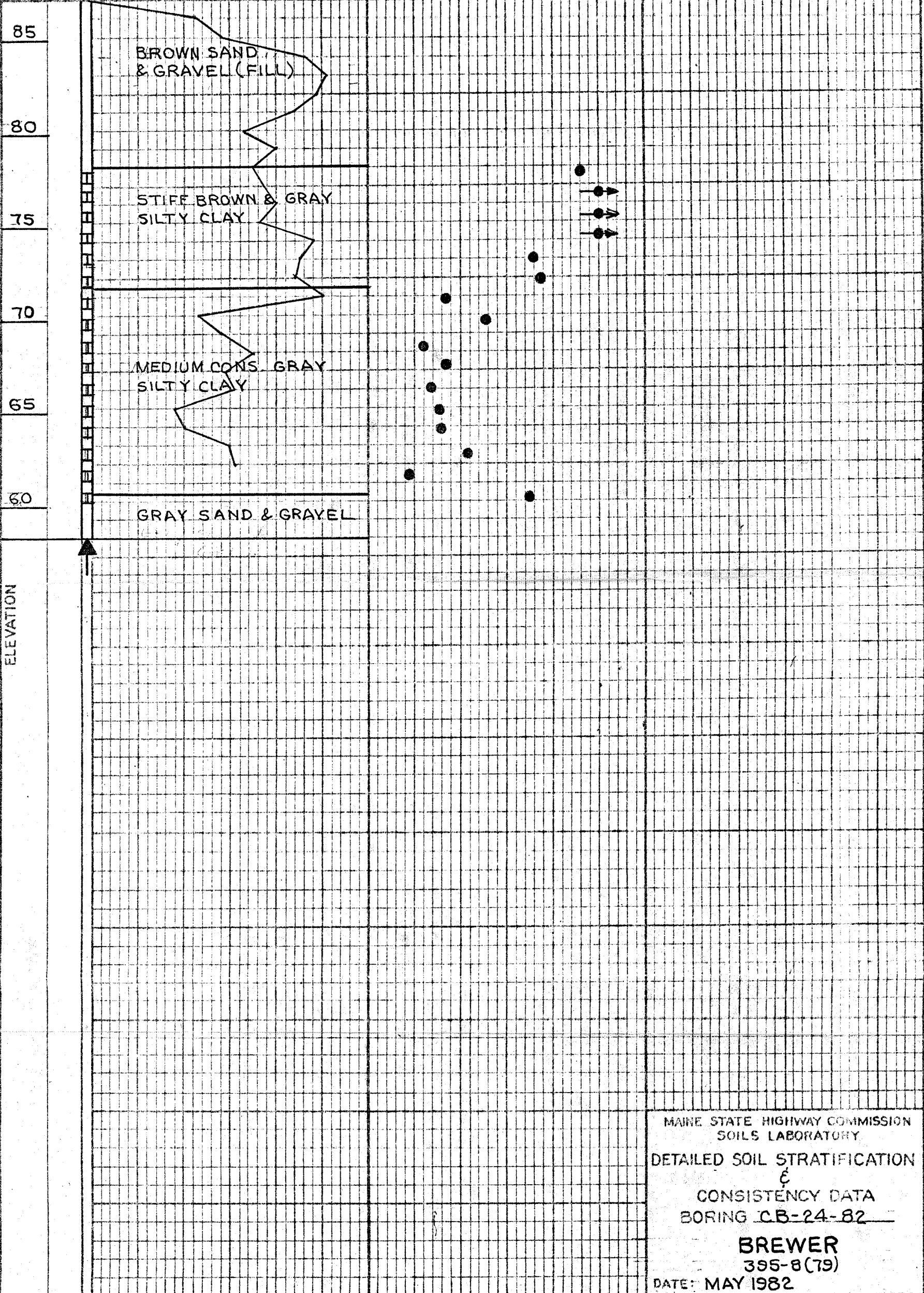
CASING SIZE - 4" -	DRIVING RESISTANCE Blows/Ft. 20 40	VANE SHEAR STRENGTH Tons/Sq.Ft. 0.4 0.8	WATER CONTENT Percent 20 40	

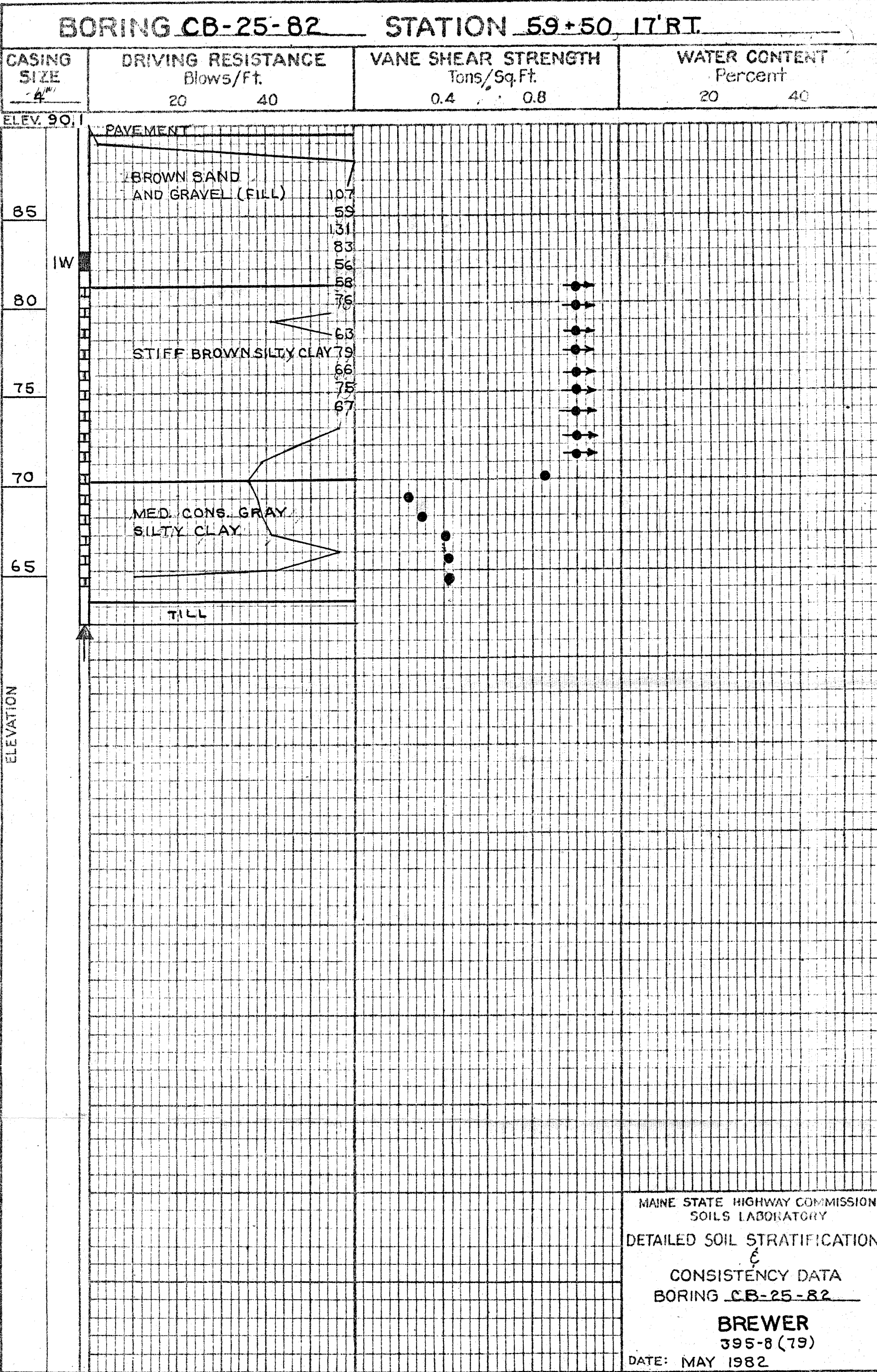


BORING CB-24-82 STATION 56+75.37' LT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq.Ft.	WATER CONTENT Percent
4"-	20 40	0.4 0.8	20 40

ELEV. 87.1

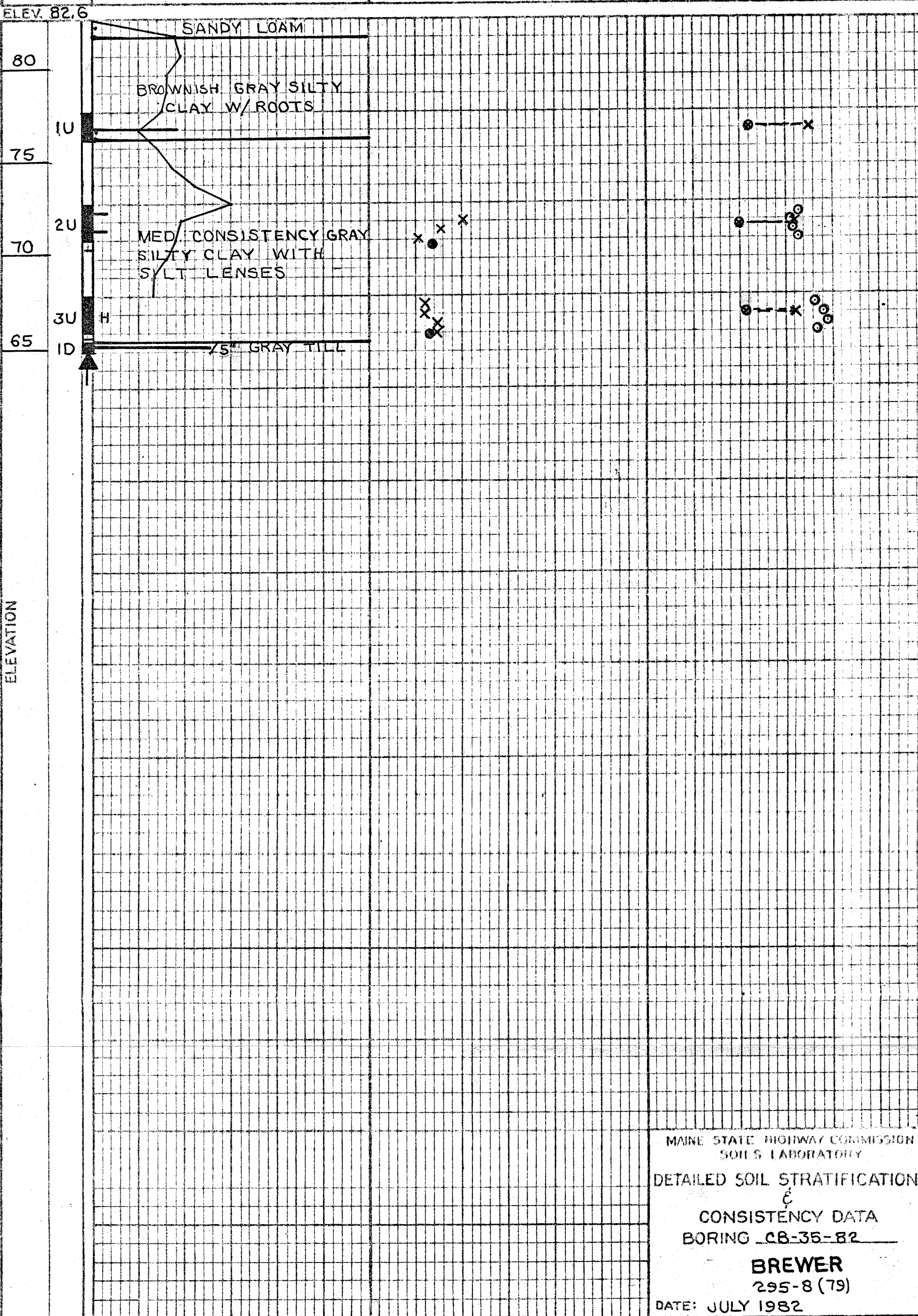




MAINE STATE HIGHWAY COMMISSION
 SOILS LABORATORY
 DETAILED SOIL STRATIFICATION
 CONSISTENCY DATA
 BORING CB-25-82
BREWER
 395-8 (79)
 DATE: MAY 1982

BORING CB-35-82 STATION 61+50 120' RT.

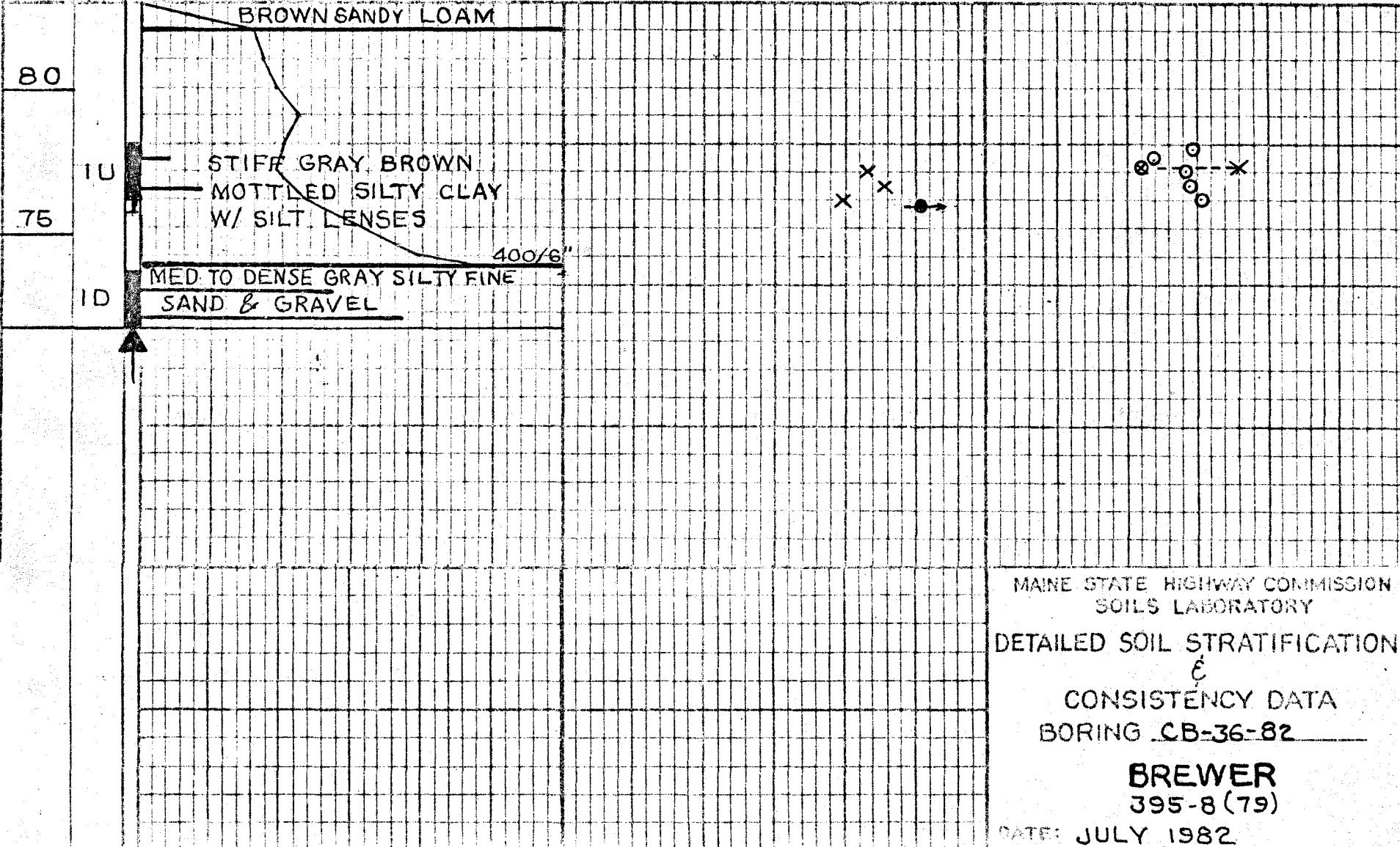
CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4"	20 40	0.4 0.8	20 40



BORING CB-36-82 STATION 63+00 120' RT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4"	20 40	0.4 0.6	20 40

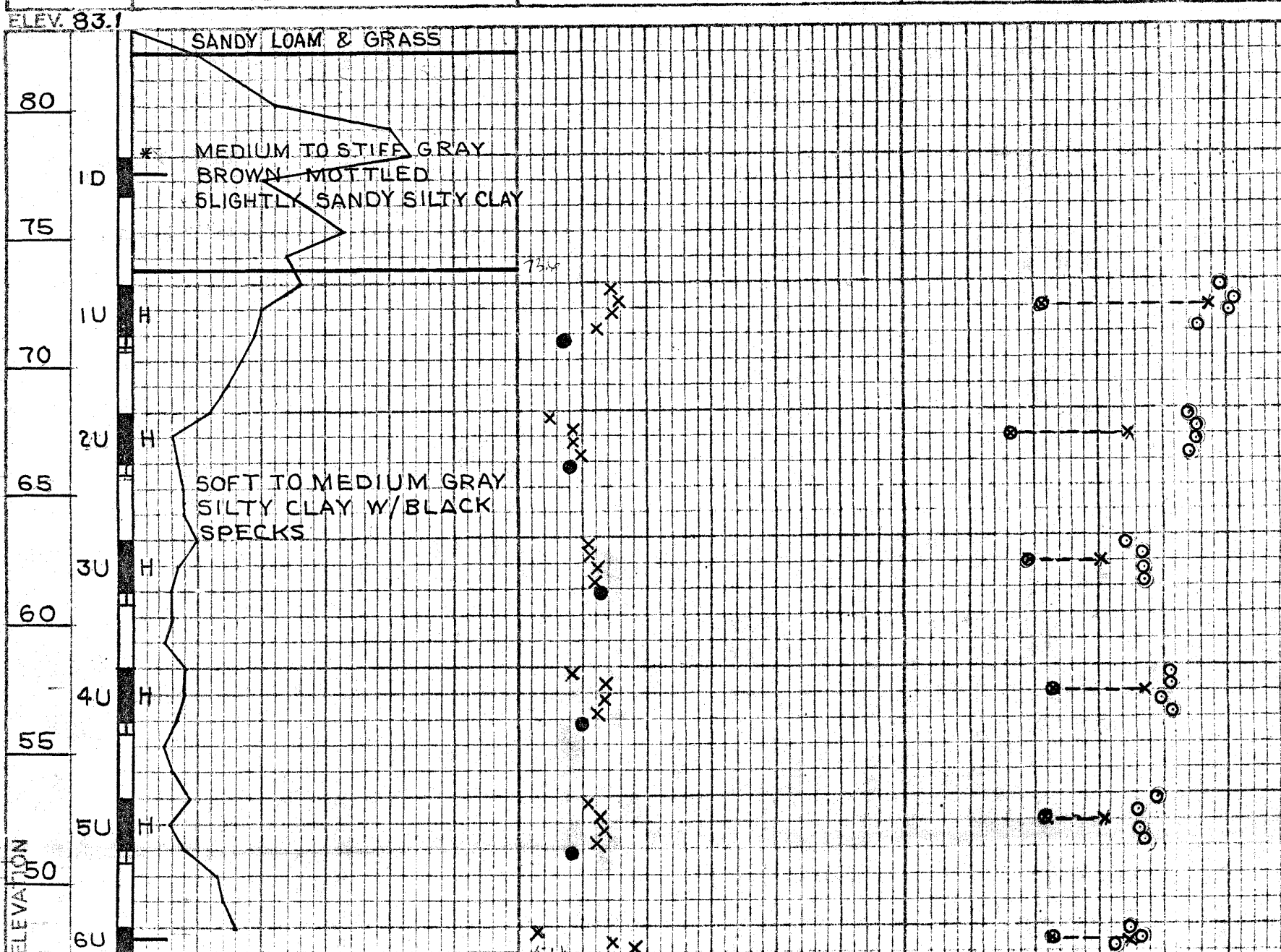
ELEV. 82.9



BORING CB-37-82

STATION 64+60 115' RT.

CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq.Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40



(R)

* FAILED TUBE SAMPLE

MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING CB-37-82
BREWER
395-8 (79)
DATE: JULY 1982

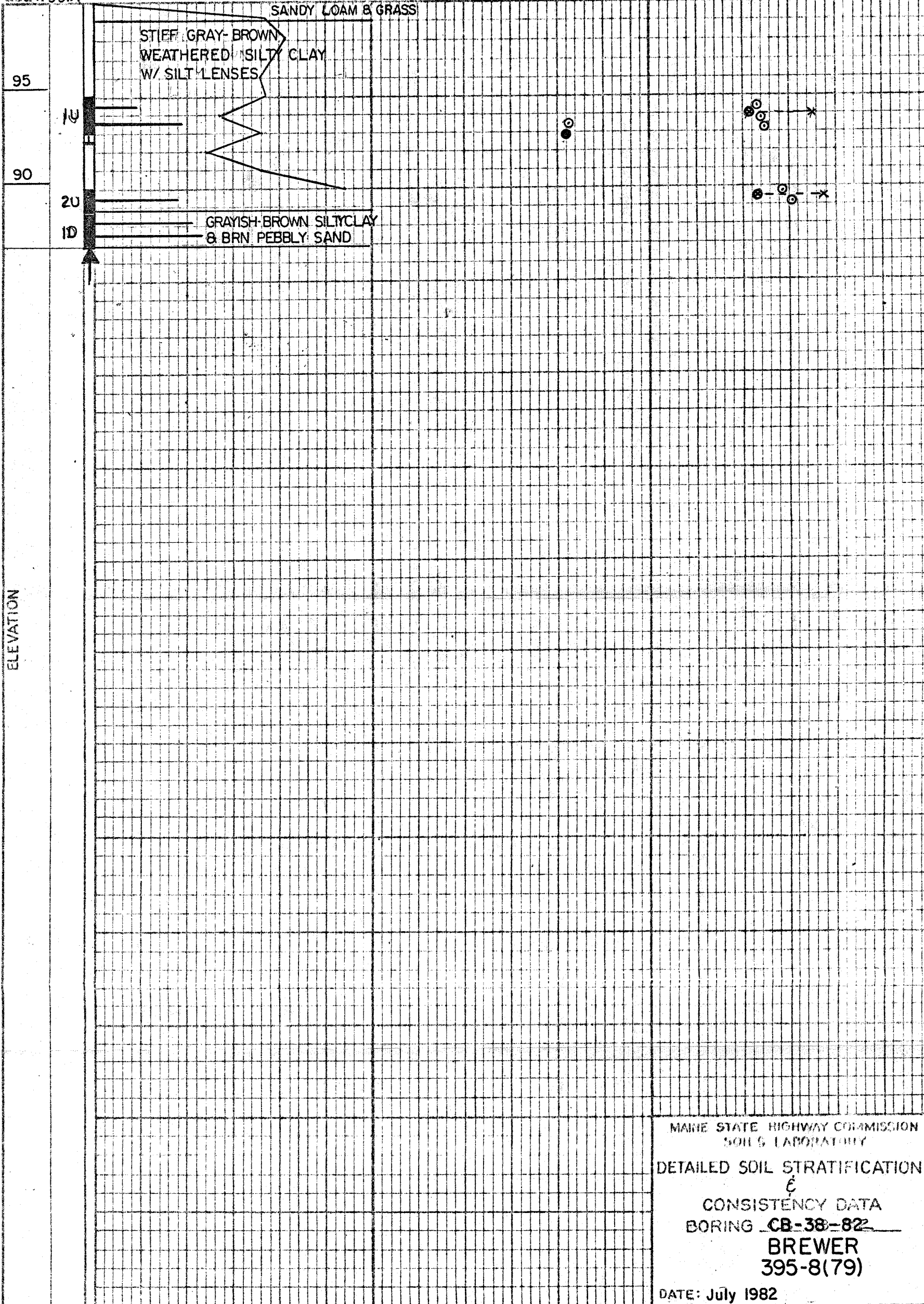
SHEET NO. 43

BORING CB-38-82

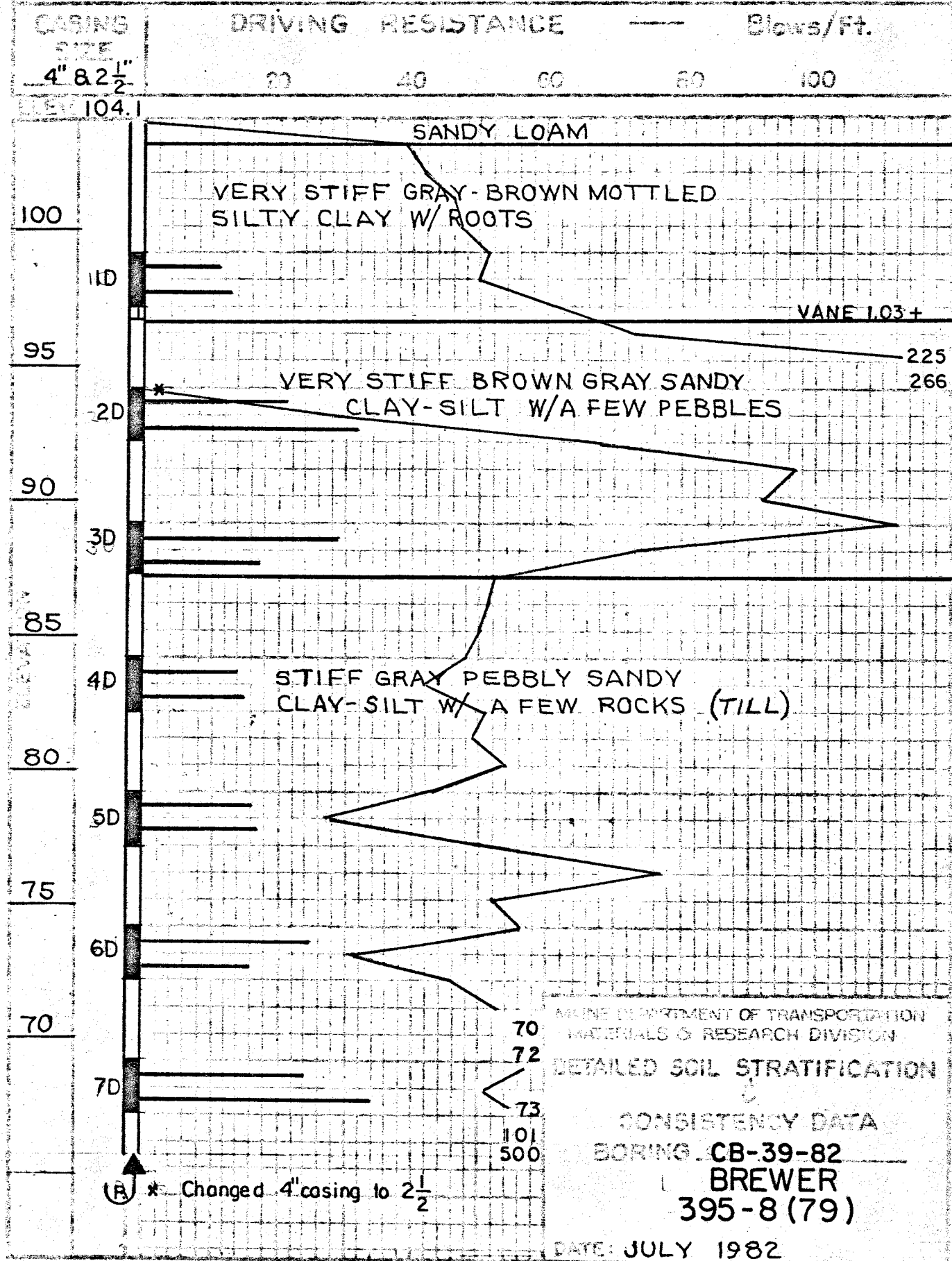
STATION 337+00, 50' Lt.

CASING
SIZE
4"DRIVING RESISTANCE
Blows/Ft.
20 40VANE SHEAR STRENGTH
Tons/Sq.Ft.
0.4 0.8WATER CONTENT
Percent
20 40

ELEV. 99.4



BORING CB-39-82 STATION 335+50 200' Lt



BORING CB-40-82

STATION 334+50

50 Lt.

CASING
SIZE
4"

DRIVING RESISTANCE

Blows/Ft.

20

40

VANE SHEAR STRENGTH

Tons/Sq. Ft.

0.4

0.8

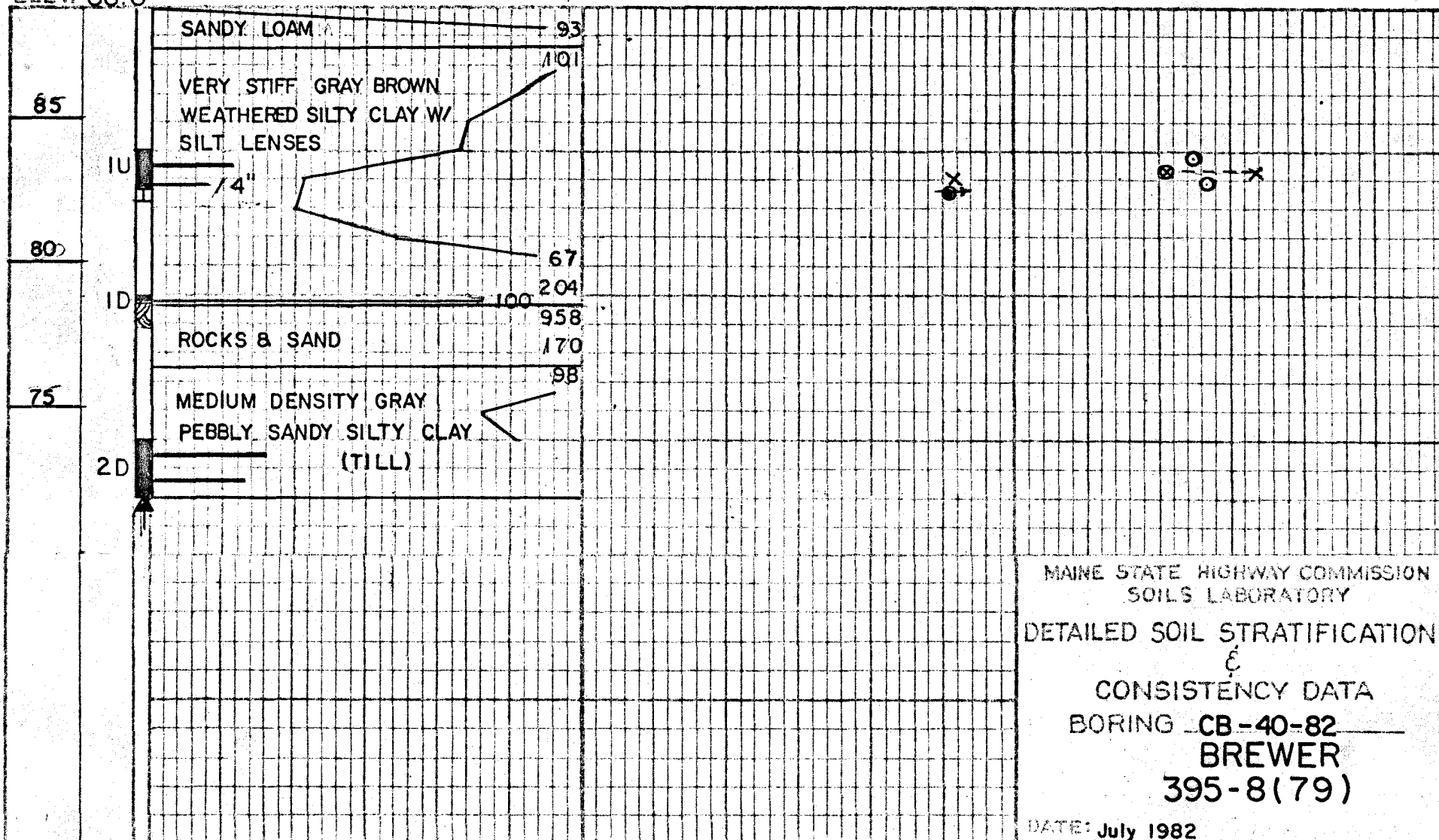
WATER CONTENT

Percent

20

40

ELEV. 88.8



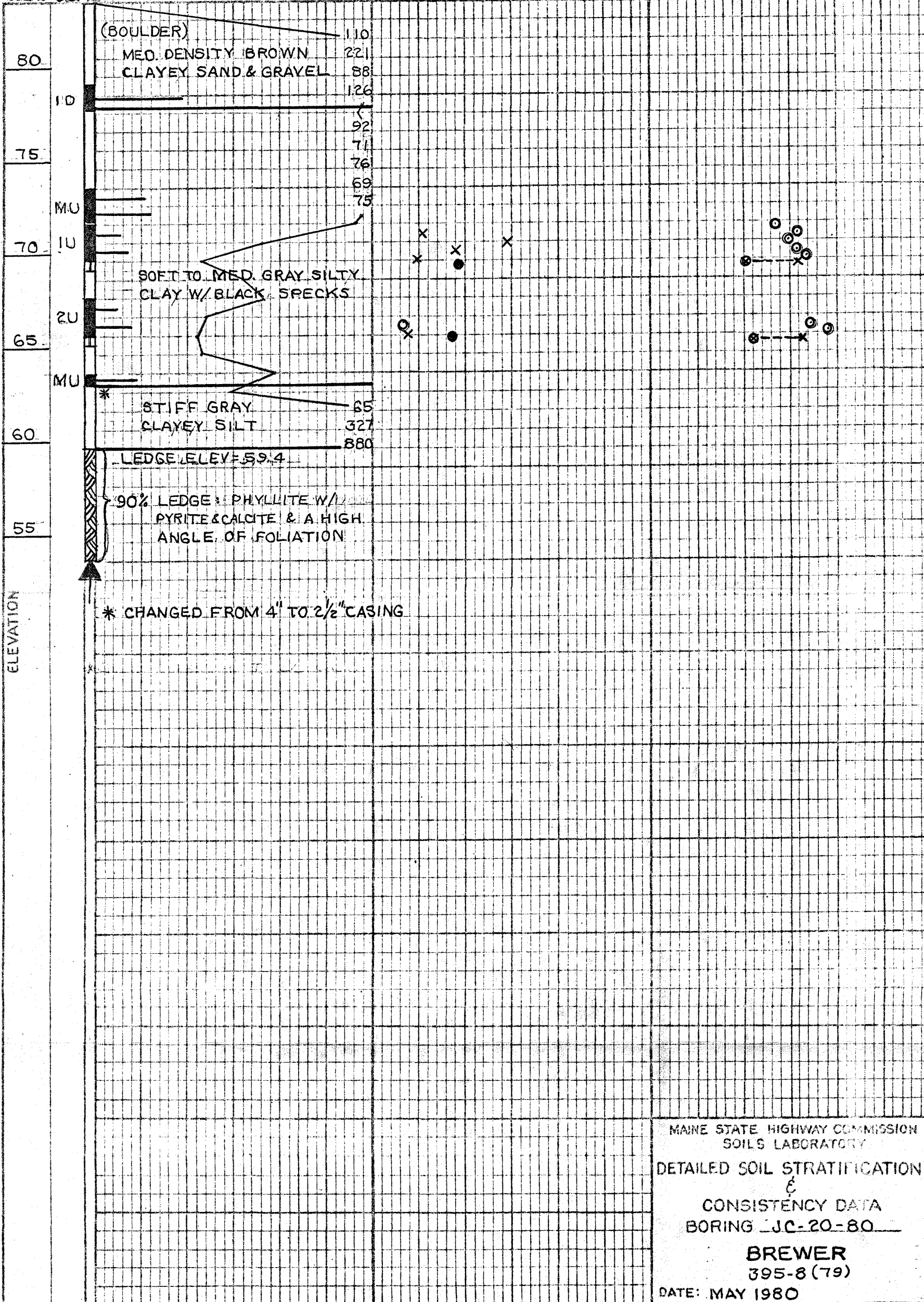
59+59 64RT. (RT. 1A)

BORING JC-20-80

STATION 41+15 10' LT. (FELTS BK. LINE)

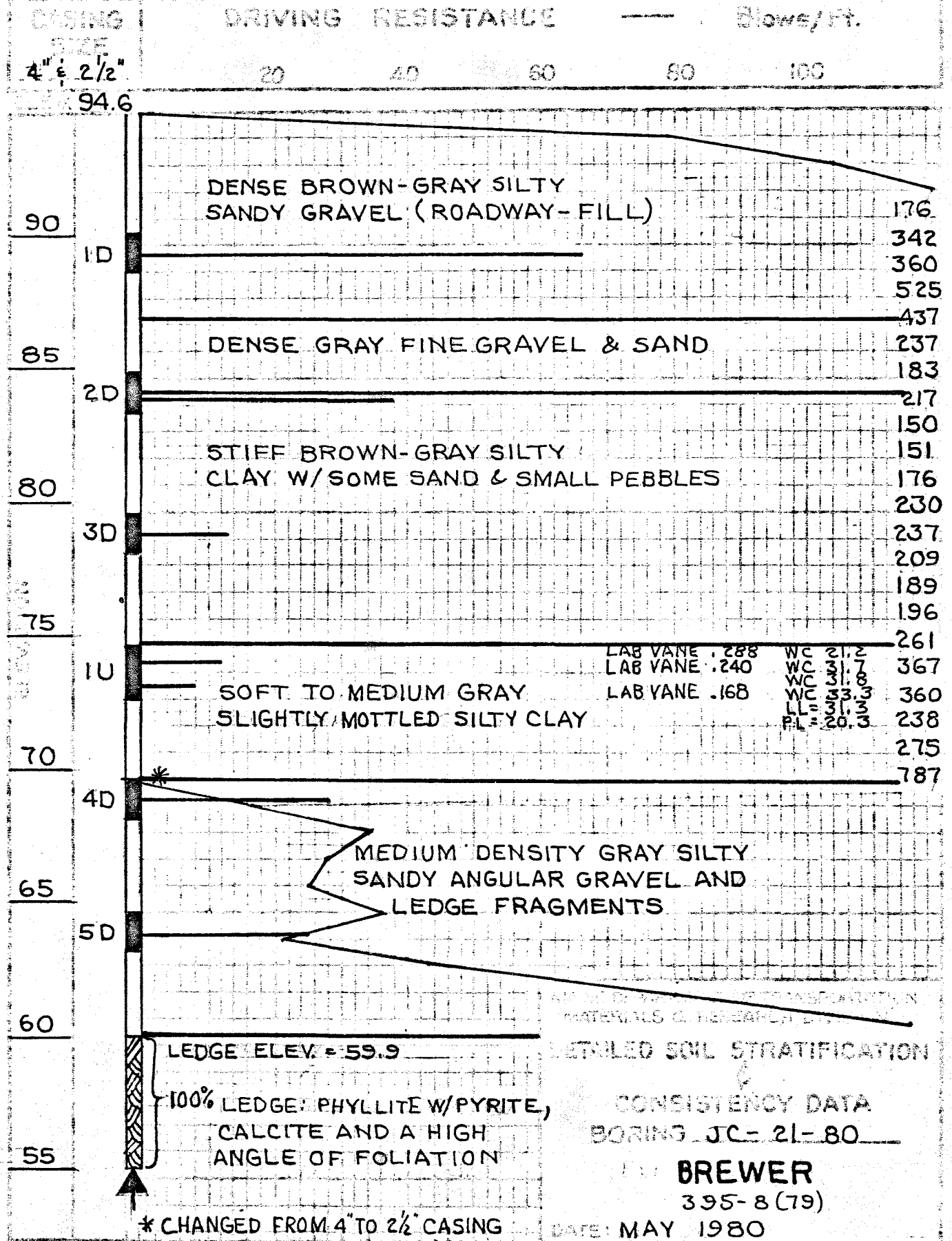
CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4" & 2 1/2"	20 40	0.4 0.8	20 40

ELEV. 83.4



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING JC-20-80
BREWER
395-8 (79)
DATE: MAY 1980

BORING JC-21-80 STATION 63+02 37' RT.

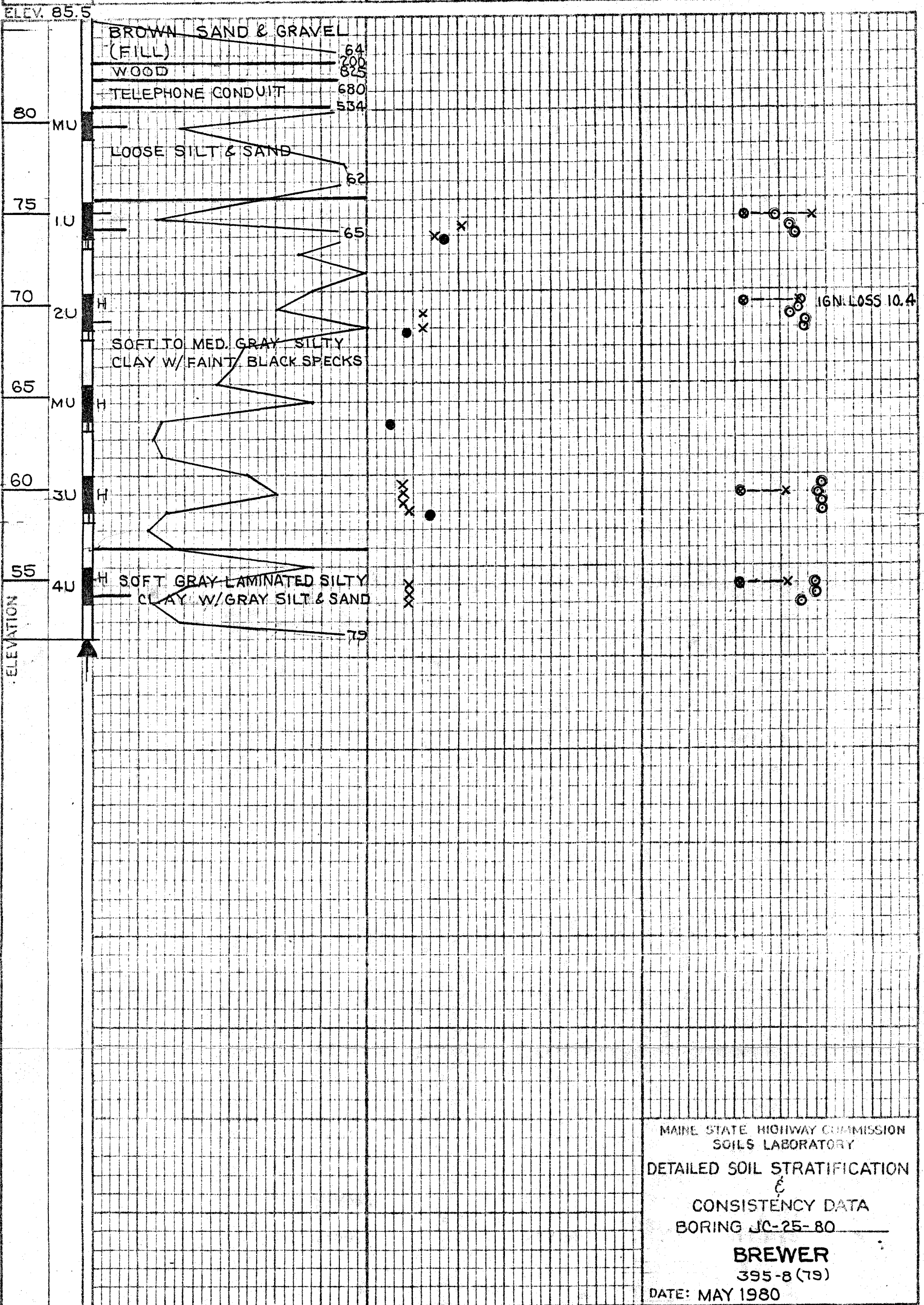


55+47.4 LT. (RT. 1A)

BORING JC-25-80

STATION 9+02.18 LT. (RR TRAVERSE)

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4" -	20 40	0.4 0.8	20 40



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
CONSISTENCY DATA
BORING JC-25-80
BREWER
395-8 (79)
DATE: MAY 1980

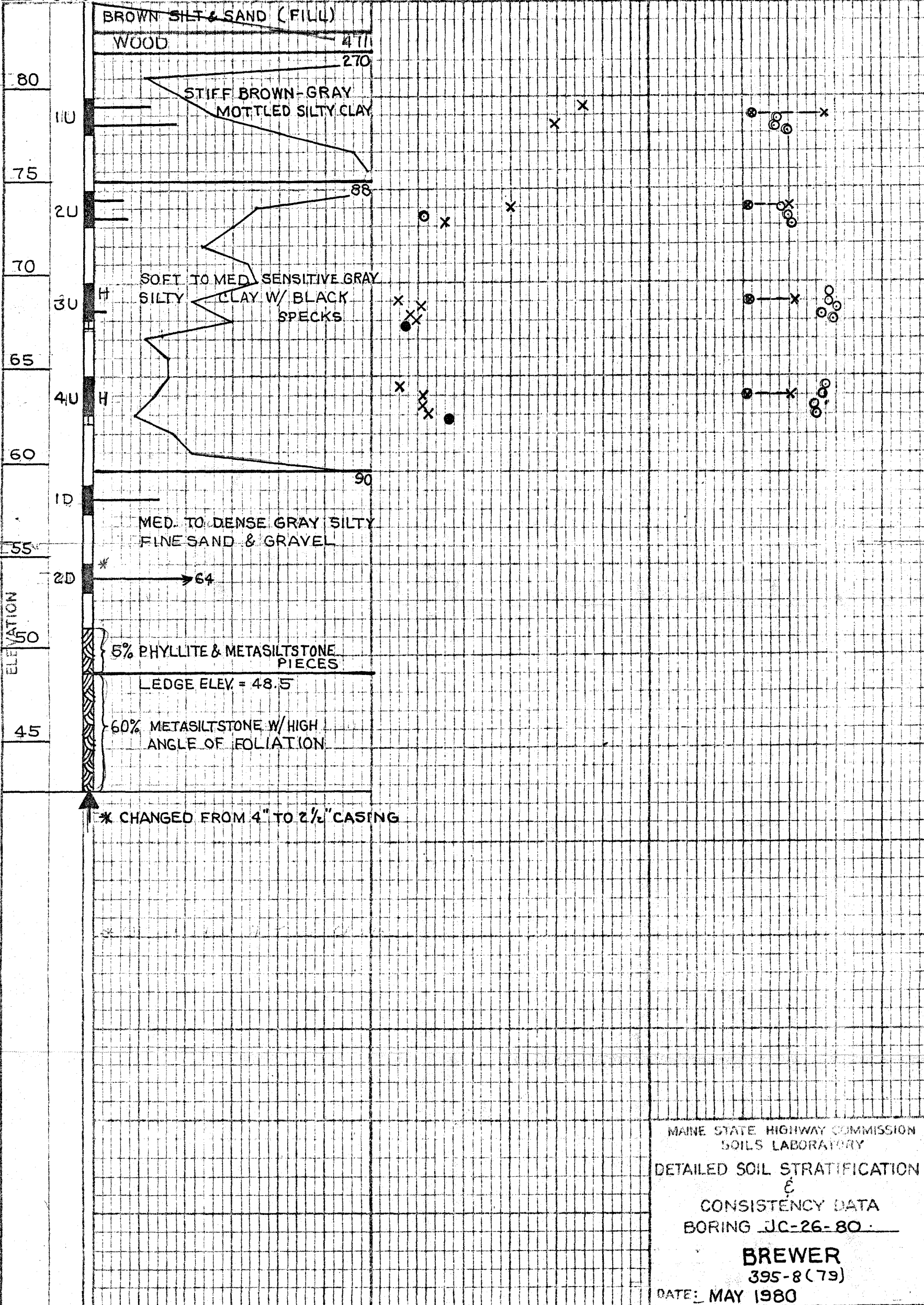
55+65 61 LT. (RT. 1A)

BORING JC-26-80

STATION 8+77 20.5 LT. (RR TRAVERSE)

CASING SIZE 4" & 2 1/2"	DRIVING RESISTANCE Blows/Ft. 20 40	VANE SHEAR STRENGTH Tons/Sq. Ft. 0.4 0.8	WATER CONTENT Percent 20 40

ELEV. 84.5



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING JC-26-80
BREWER
395-8(79)
DATE: MAY 1980

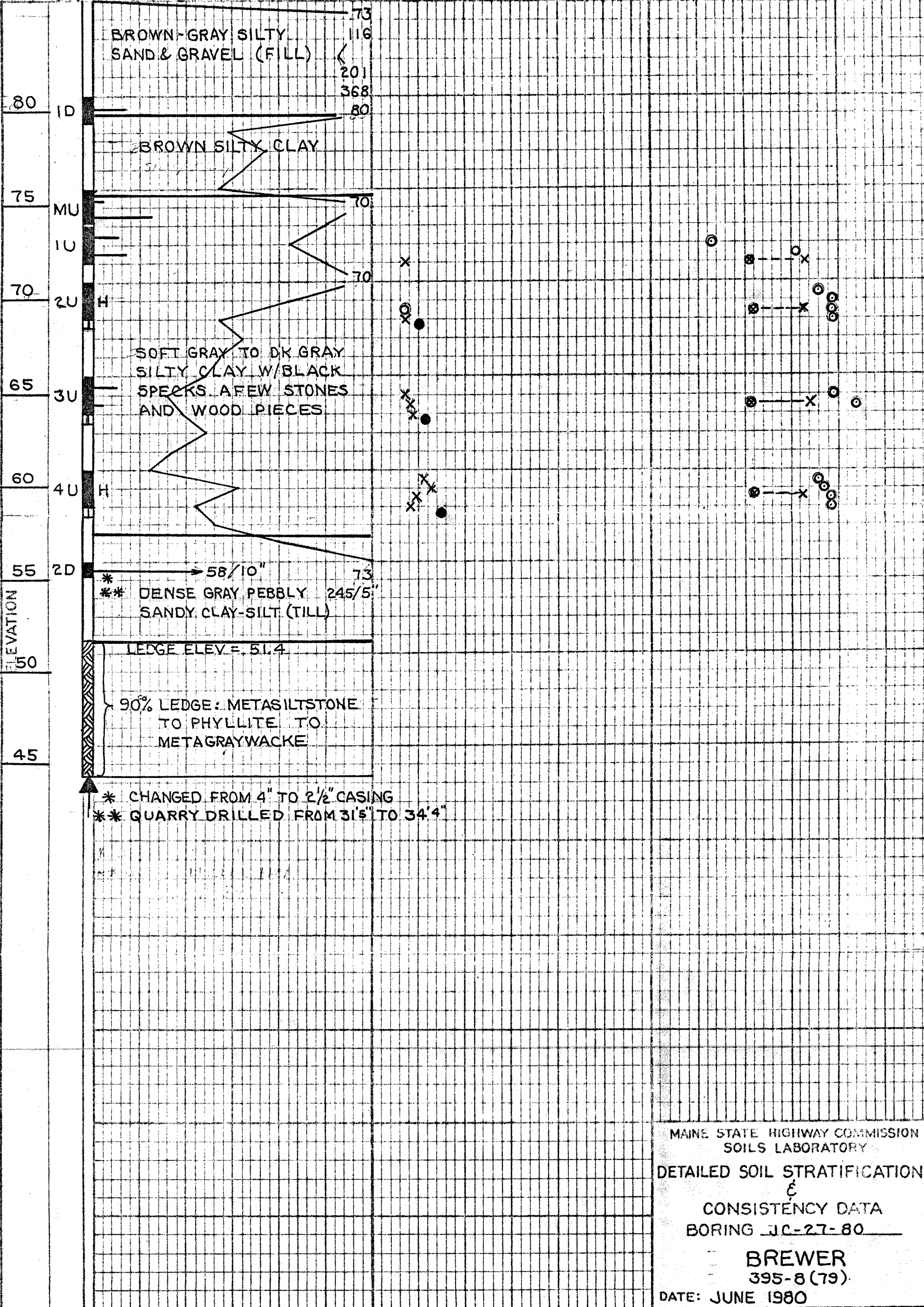
54+41 37' RT. (RT. 1A)

BORING JC-27-80

STATION 10+30 17.5' RT. (RR LINE)

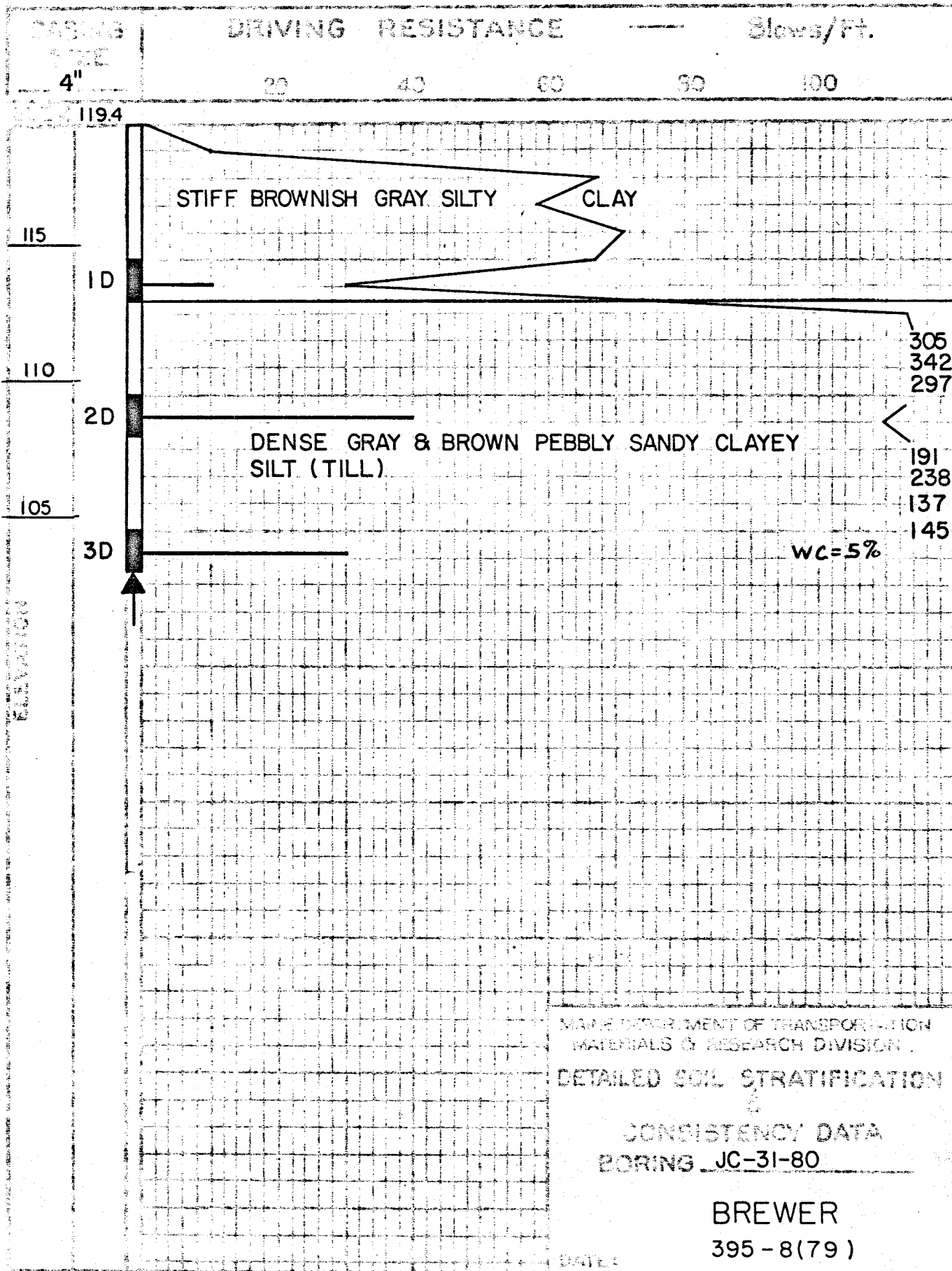
CASING SIZE	DRIVING RESISTANCE		VANE SHEAR STRENGTH		WATER CONTENT	
	Blows/Ft.		Tons/Sq.Ft.		Percent	
4" & 2 1/2"	20	40	0.4	0.8	20	40

ELEV. 85.7

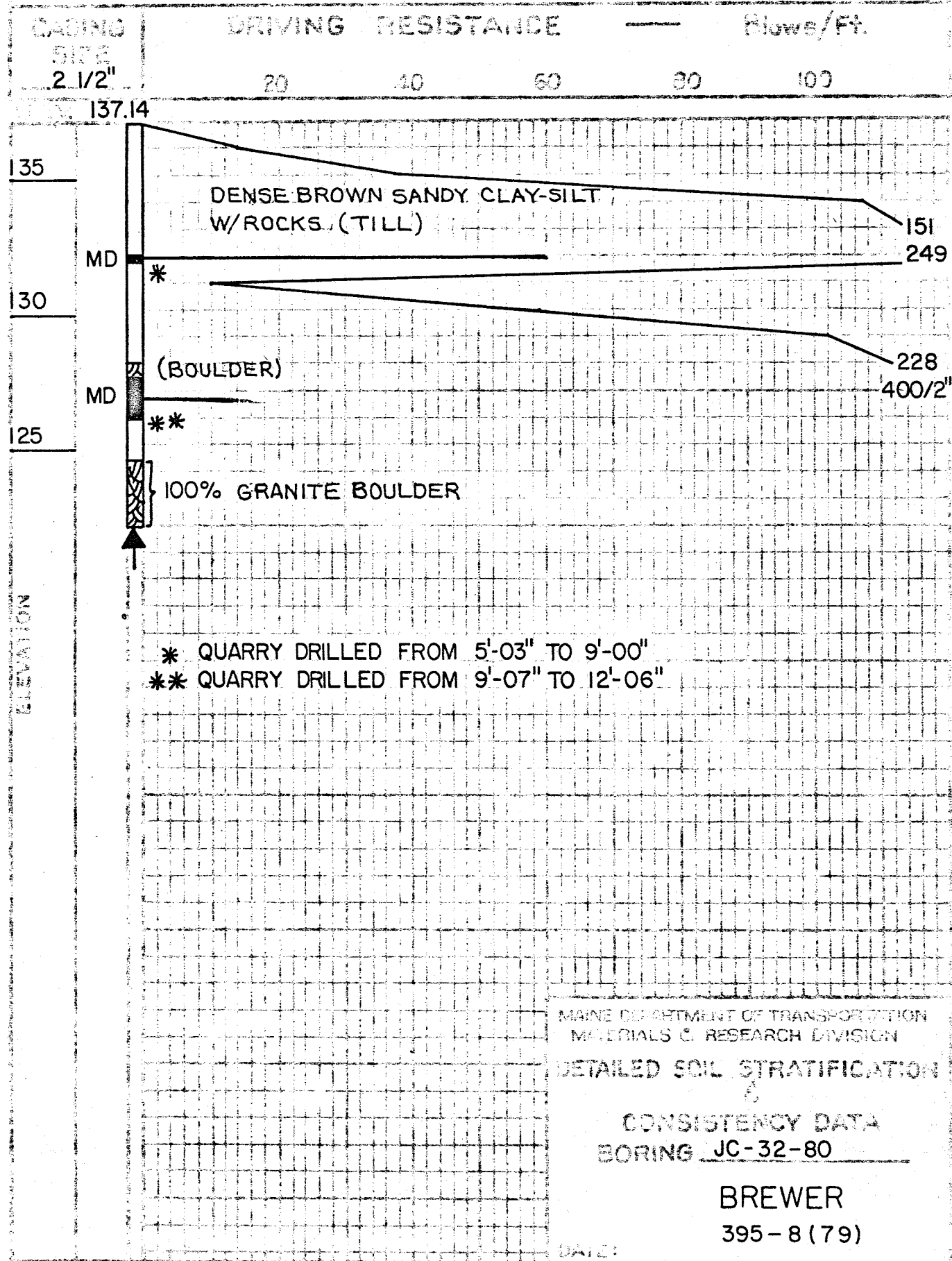


MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING JC-27-80
BREWER
395-8(79)
DATE: JUNE 1980

BORING JC-31-80 STATION 327 + 20, C



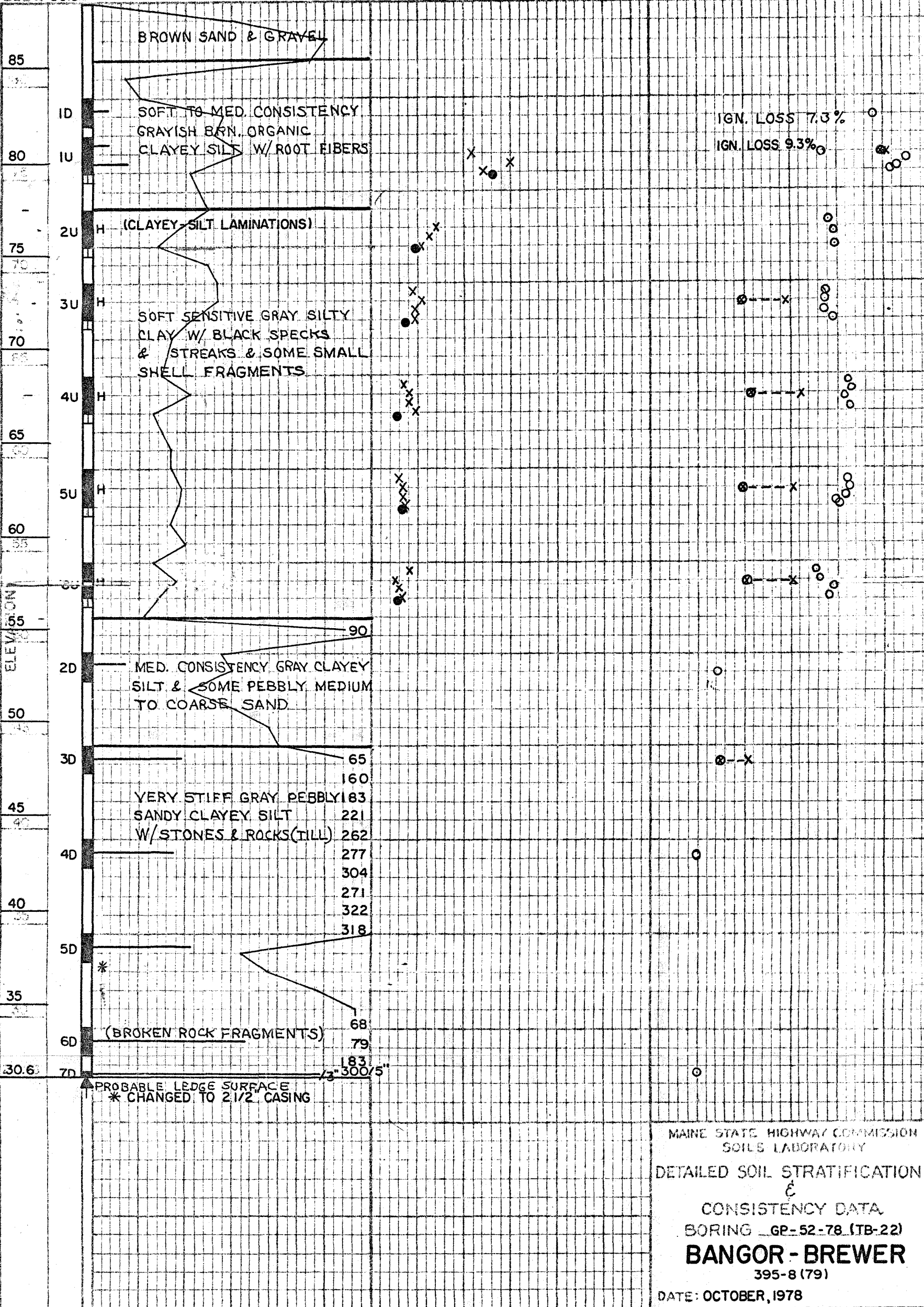
BORING JC-32-80 STATION 318+97, 36' LT.



BORING GP-52-78(TB-22) STATION 49+90 45' Lt

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4" & 2 1/2"	20 40	0.4 0.8	20 40

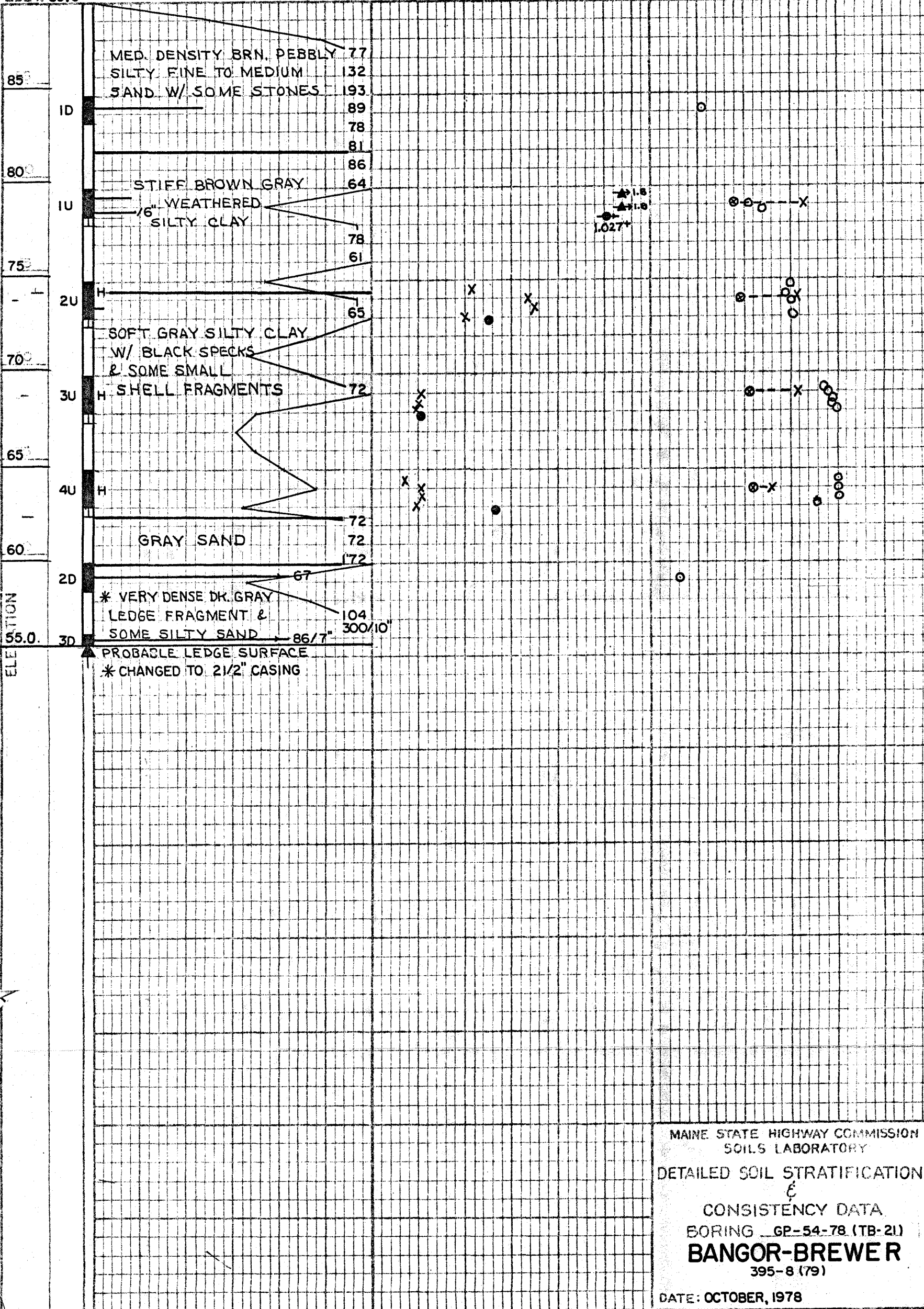
ELEV. 88.3



BORING GP-54-78 (TB-21) STATION 59+50 39' Lt

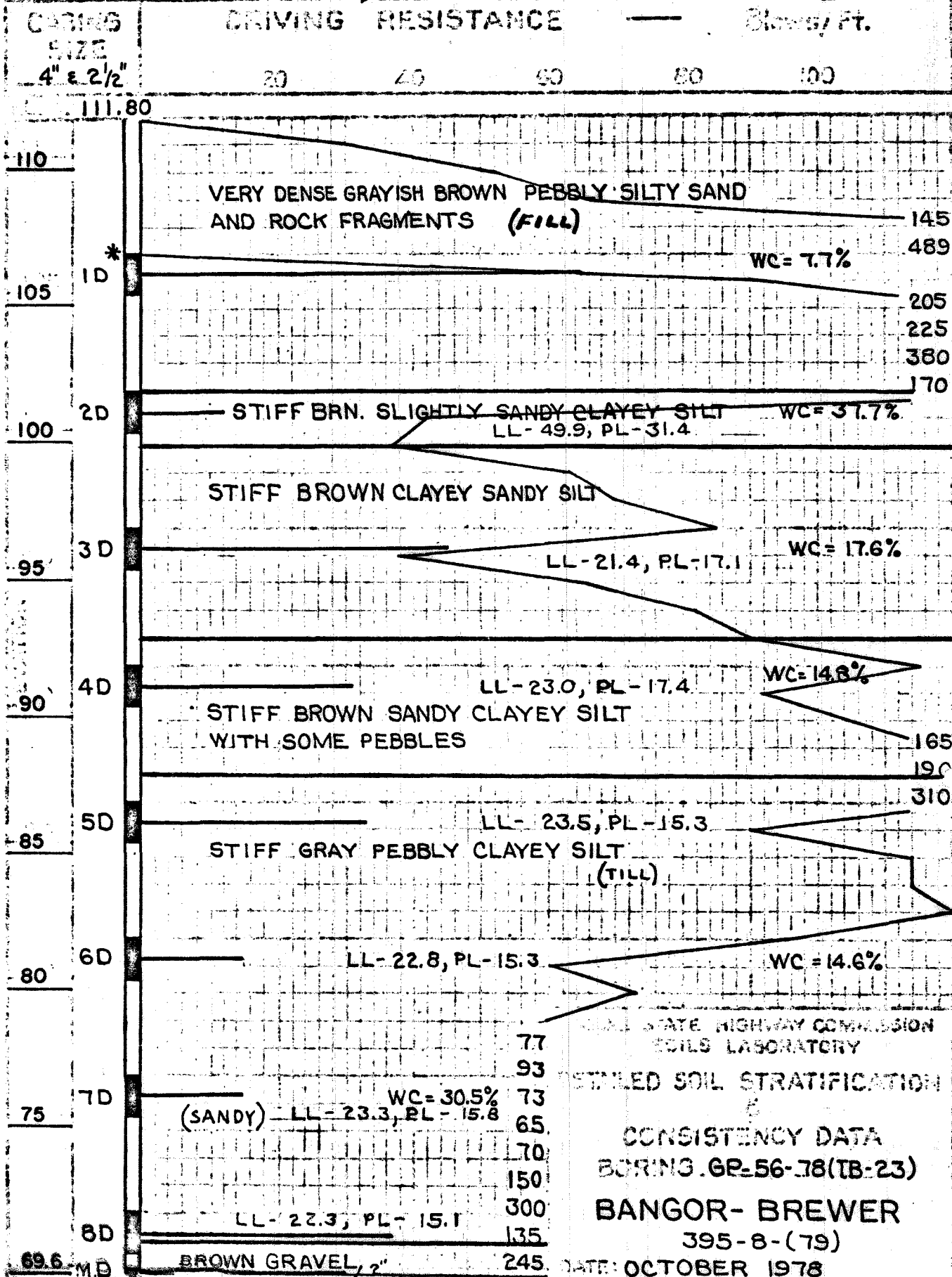
CASING SIZE	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

ELEV. 89.5



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-54-78 (TB-21)
BANGOR-BREWER
395-8 (79)
DATE: OCTOBER, 1978

BORING GP-56-78 STATION 66+83, 31' Lt.
(TB-23) RT. 1A E



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY

STANDARD SOIL STRATIFICATION

CONSISTENCY DATA
BORING GP-56-78(TB-23)

BANGOR-BREWER
395-8-(79)

DATE: OCTOBER 1978

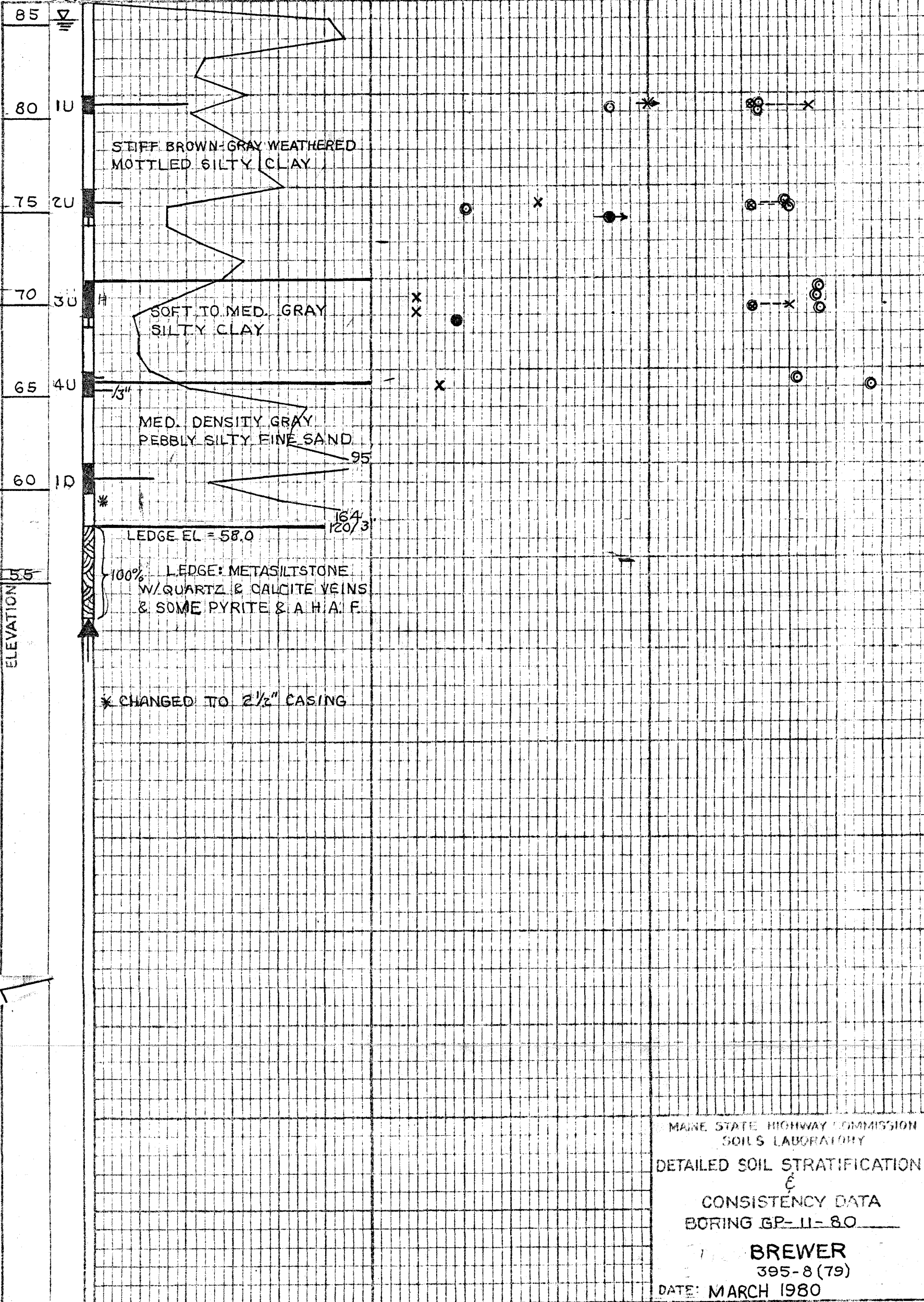
225 * CHANGED 4" CASING TO 6"

BORING GP-II-80

STATION 62+00 135' LT.

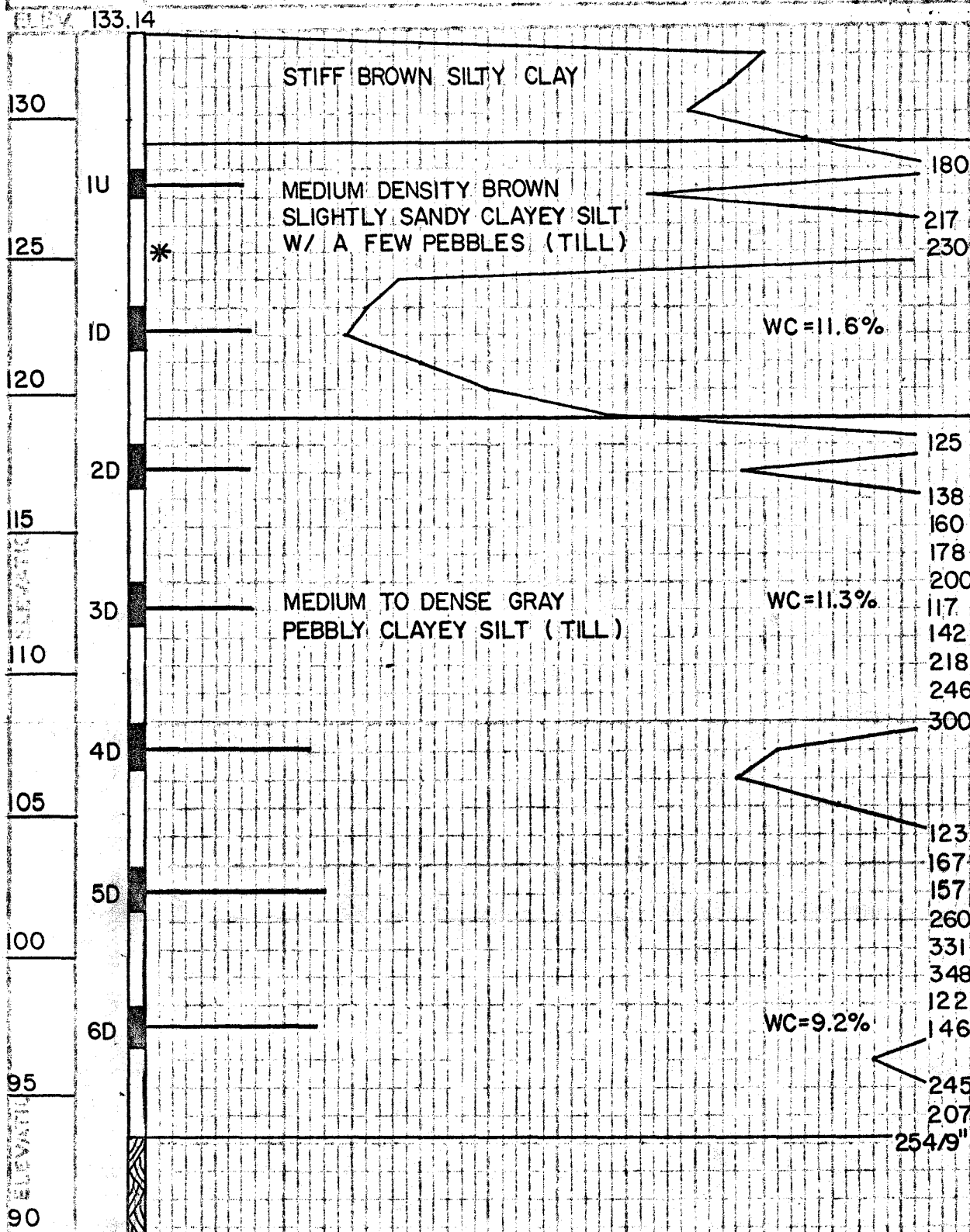
CASING SIZE	DRIVING RESISTANCE Blows/Ft.			VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
4" & 2 1/2"	20	40	160	0.4	0.8	20	40

ELEV. 86.2



BORING GP-12-80 STATION 73+00, 125' LT. (RTE. I-AL)

CASING SIZE	DRIVING RESISTANCE				
	10	40	60	80	100



* CHANGED TO 2-1/2" CASING

MAINT. DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION
DETAILED SOIL STRATIFICATION

CONSISTENCY DATA
BORING GP-12-80

BREWER
395-8(79)
MARCH 1980

395-8(79) (S-72)

BORING GP-13-80

STATION 335+71.83' Rt.
51+50.5 Rt. (Felts Brk. Line)CASING
SIZE
4" & 2 1/2"DRIVING RESISTANCE
Blows/Ft.

20 40

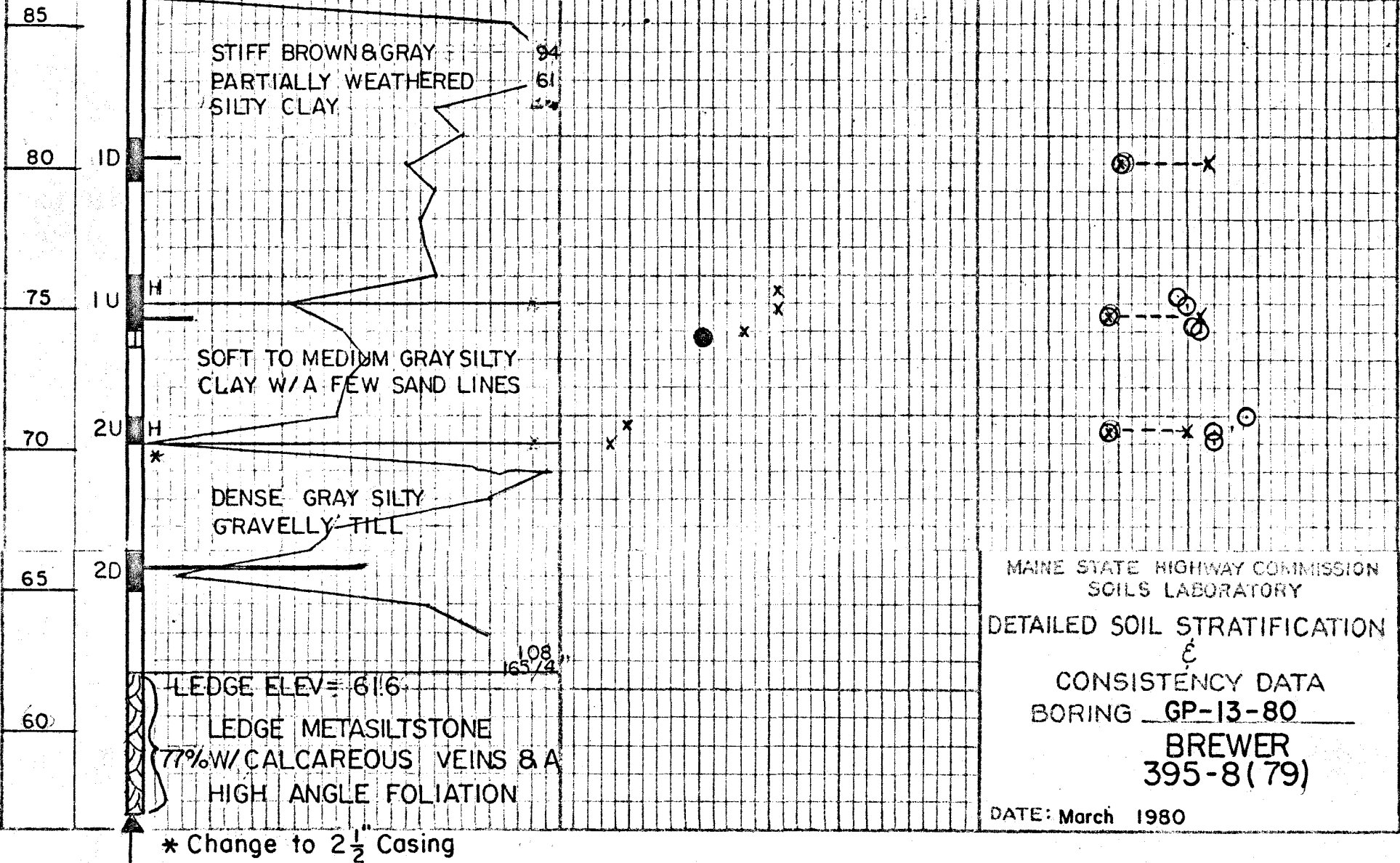
VANE SHEAR STRENGTH
Tons/Sq. Ft.

0.4 0.8

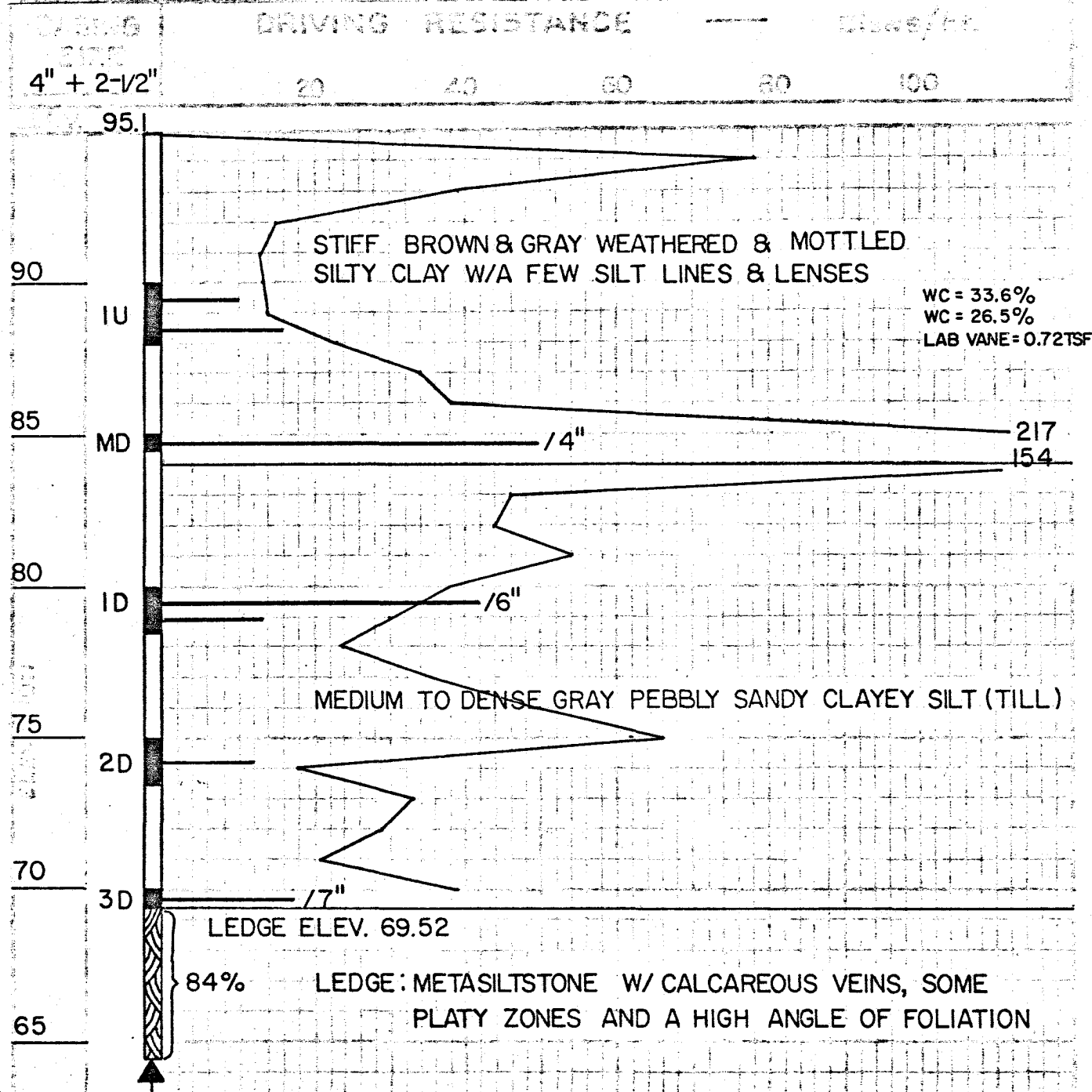
WATER CONTENT
Percent

20 40

ELEV. 86.1



BORING GP-14-80 STATION 67+50 75' RT



ILLINOIS DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA

BORING GP-14-80

BREWER

395-8(79)

DATE: APRIL 1980

BORING GP-63-80

STATION 55+85 43' RT.

CASING
SIZE

4" ± 2 1/2"

DRIVING RESISTANCE

Blows/Ft.

20

40

VANE SHEAR STRENGTH

Tons/Sq. Ft.

0.4

0.8

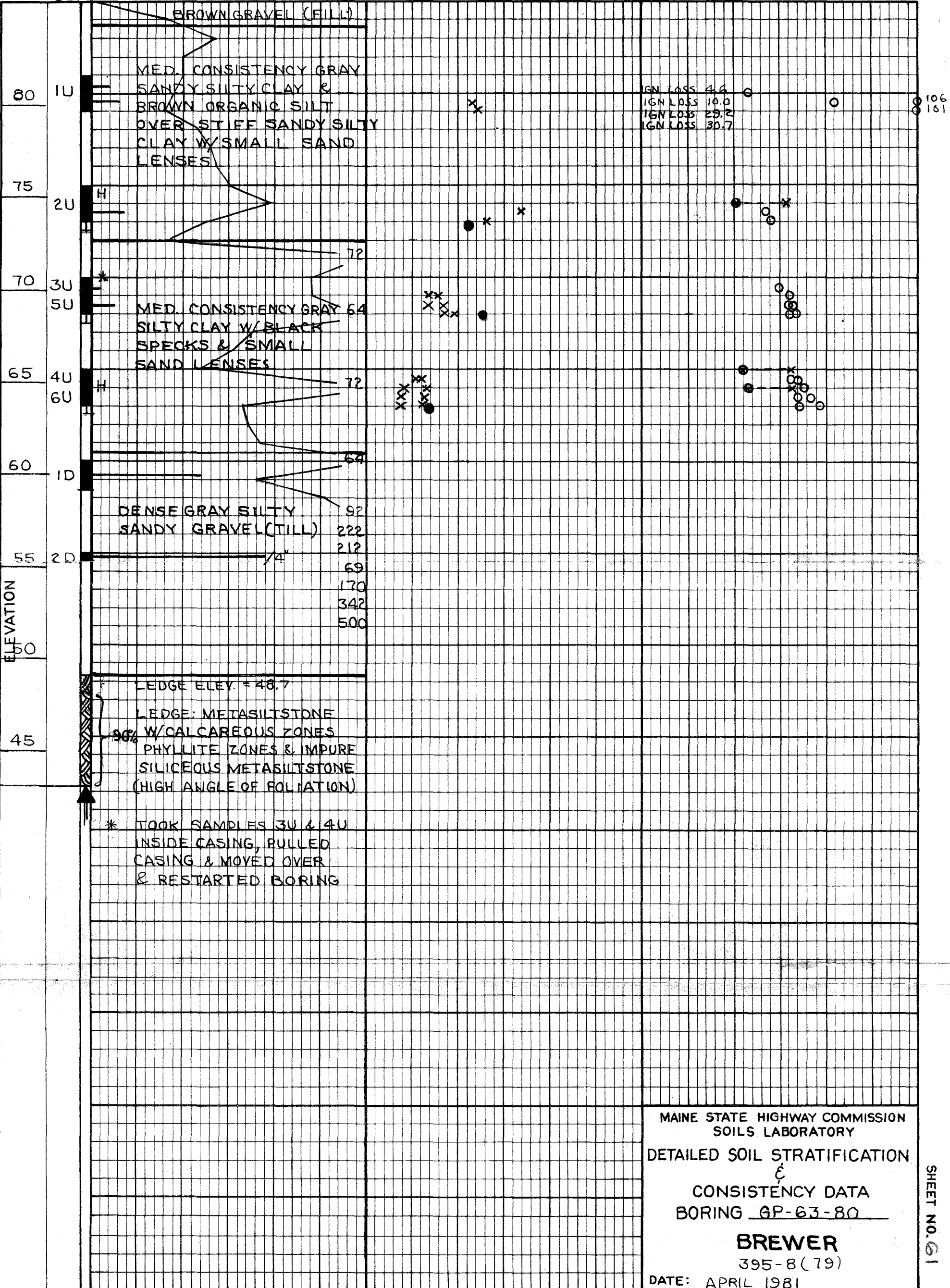
WATER CONTENT

Percent

20

40

ELEV. 85.6



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-63-80
BREWER
395-8(79)
DATE: APRIL 1981

BORING GP-4-81 STATION 316+94, C

CASING

DRIVING RESISTANCE

Blows/Ft.



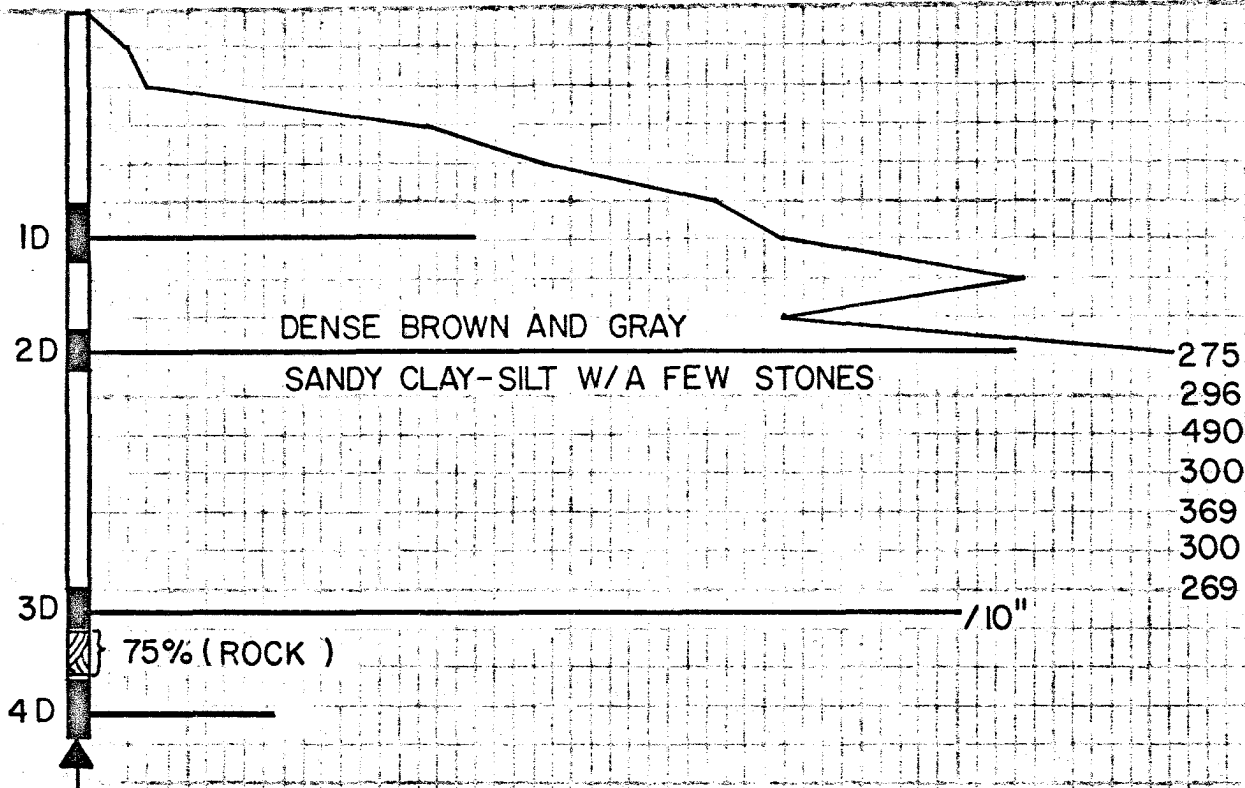
20

40

60

80

100



STATION

FLORIDA DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA
BORING GP-4-81

BREWER
395-8(79)

ONE

BORING GP-11-81 STATION 312+02, C

CASING

DRIVING RESISTANCE

Blows/Ft.

2-1/2"

20

40

60

80

100

1D

2D

3D

MEDIUM TO DENSE BROWN
AND GRAY PEBBLY SANDY
CLAY-SILT W/A FEW STONES

GRAY SILTY PEBBLY SAND

MAINE DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA

BORING GP-11-81

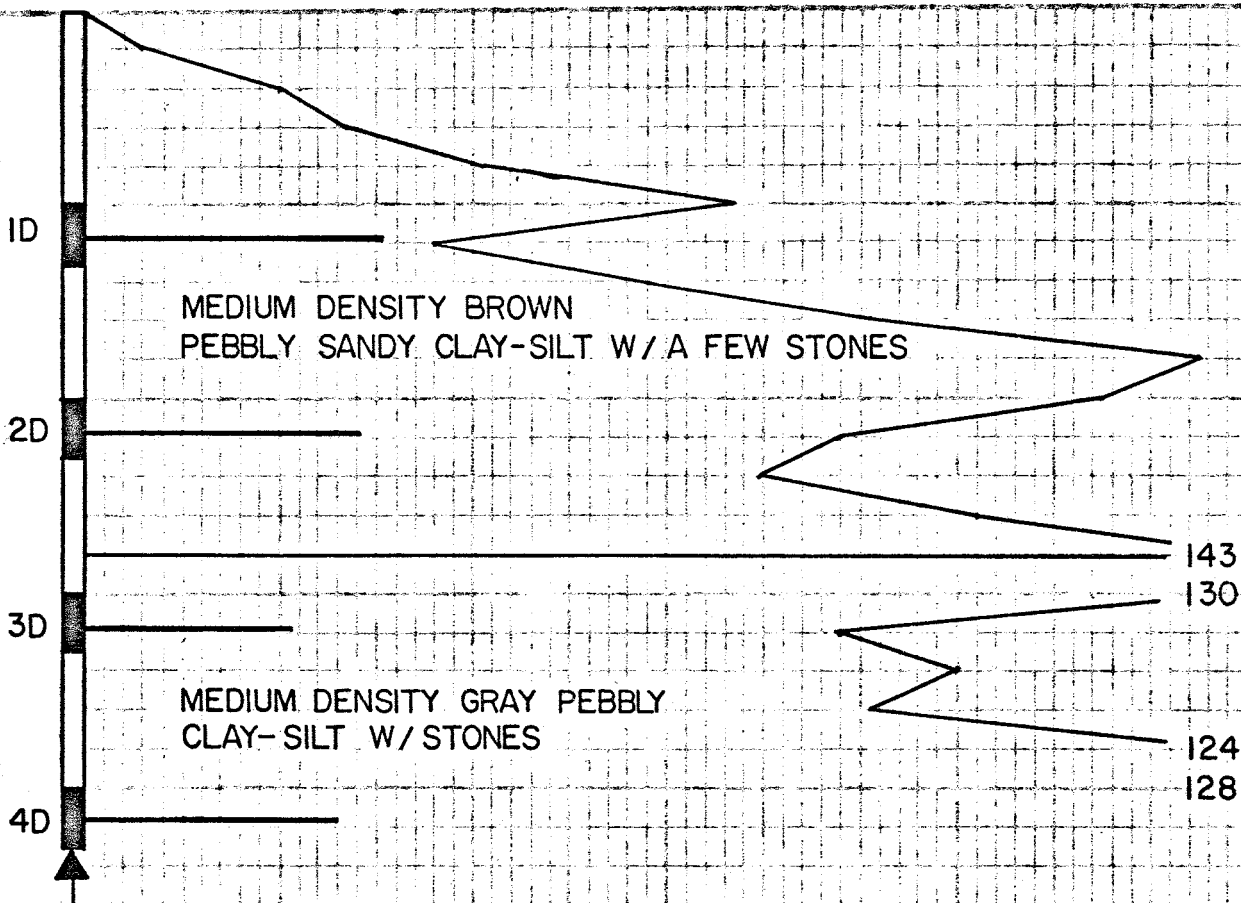
BREWER

395-8(79)

DATE:

BORING GP-12-81 STATION 308+00, 5' RT.

CASING SIZE	DRIVING RESISTANCE	Slows/Ft.
2-1/2"	20 40 60 80 100	



MAINE DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA
BORING GP-12-81
BREWER
395-8(79)

DATE:

BORING GP-13-81

STATION 335+71 8' Rt
51+50, 70' Lt (Felts Brook Line)

CASING
SIZE 1"
4" + 2 1/2"

DRIVING RESISTANCE
Blows/Ft.

VANE SHEAR STRENGTH
Tons/Sq. Ft.

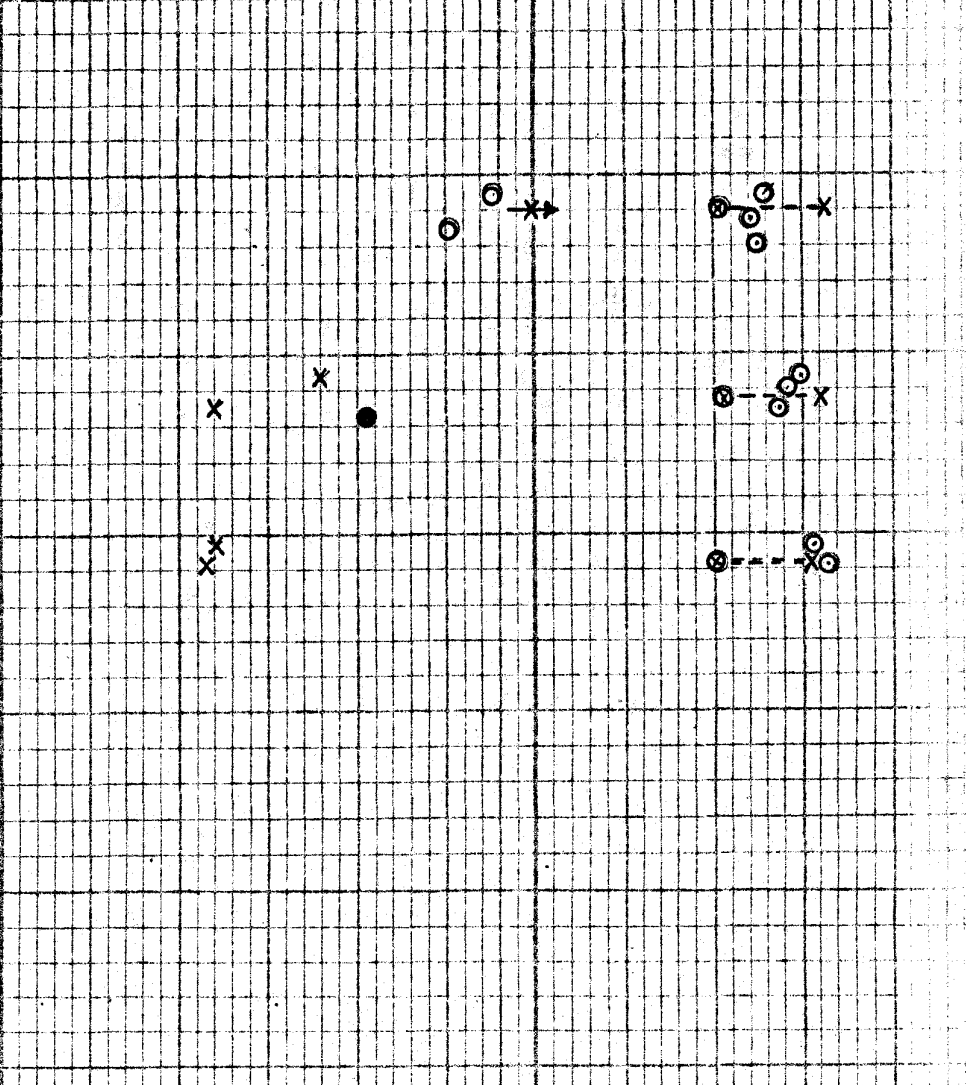
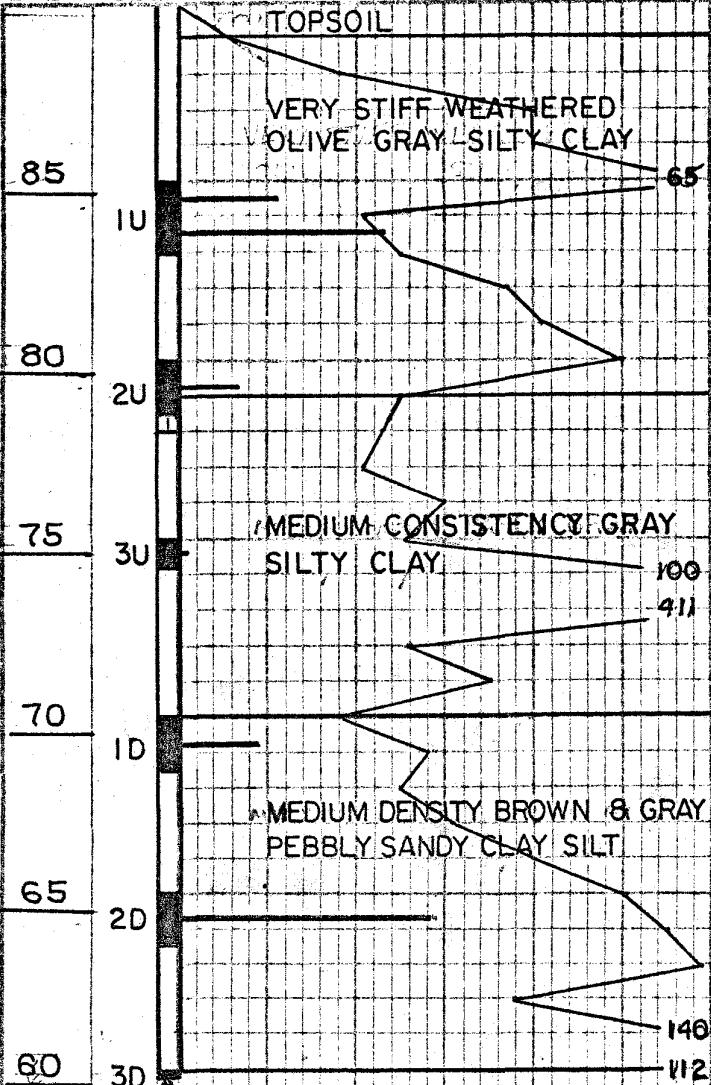
WATER CONTENT
Percent

20 40

0.4 0.8

20 40

ELEV. 90.3



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA

BORING GP-13-81

BREWER
395-8(79)

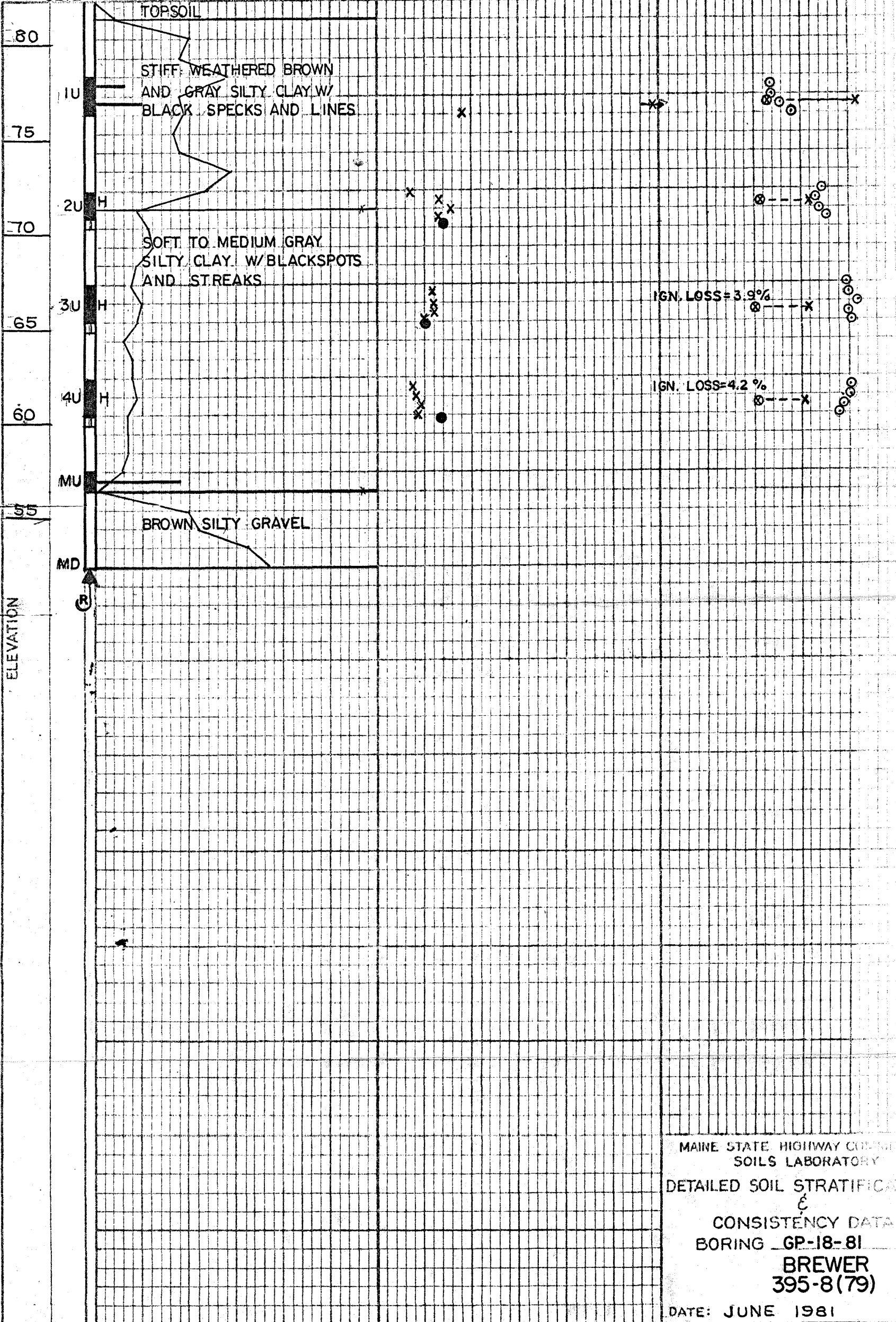
DATE: MAY 1981

BORING GP-18-81

STATION 335+04, 150Rt
50+50, 60Rt (Felts Brook Line)

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4" 2 1/2"	20 40	0.4 0.8	20 40

ELEV. 82.2



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-18-81
BREWER
395-8(79)
DATE: JUNE 1981

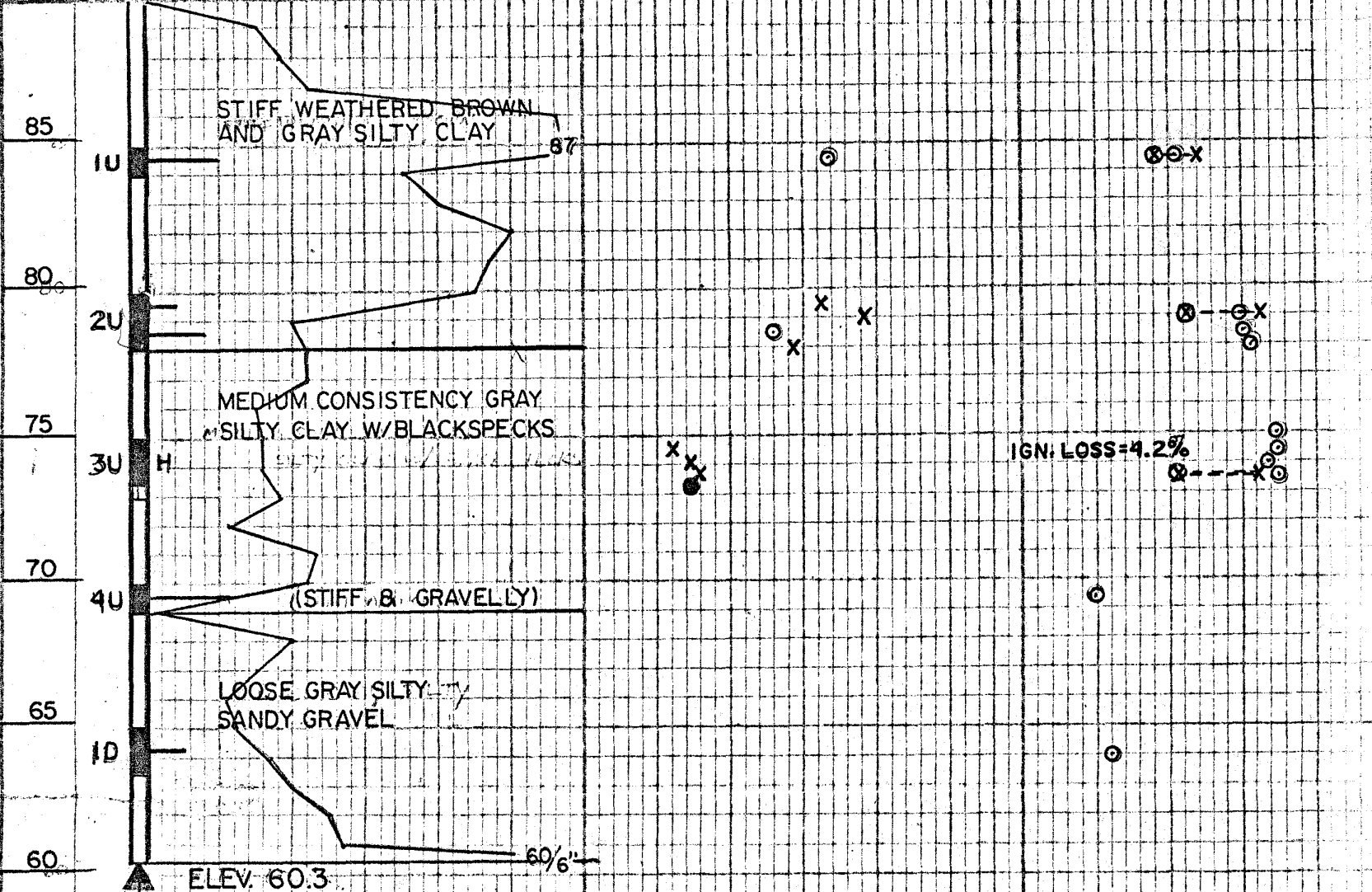
BORING GP-19-81

STATION

333+56 2' Lt
49+50, 150' Lt (FELTS BROOK)

CASING SIZE	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

ELEV. 89/82



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-19-81
BREWER
395-8 (79)

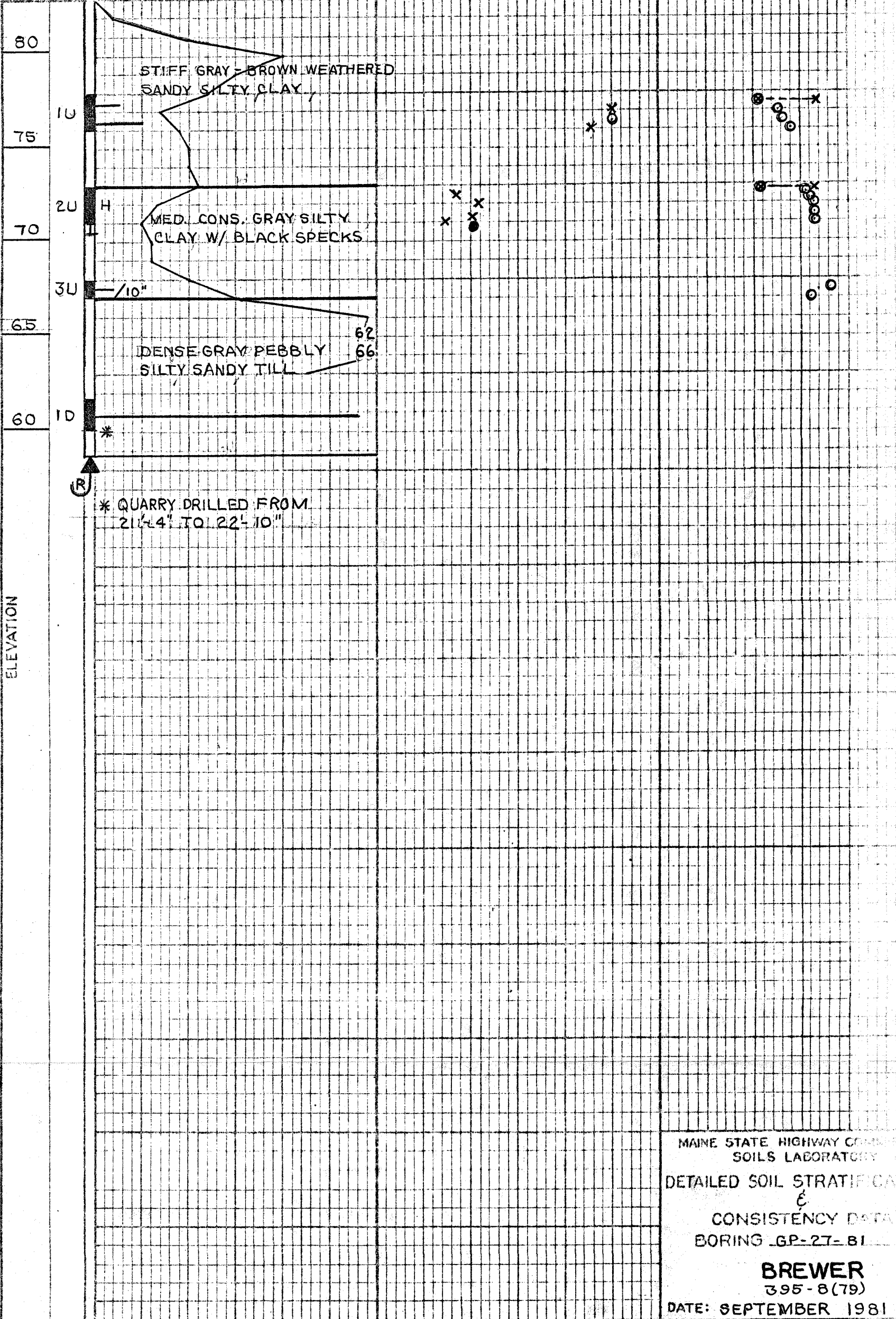
DATE: June 1981

BORING GP-27-81

STATION 60+50, 150' LT.

CASING SIZE	DRIVING RESISTANCE		VANE SHEAR STRENGTH		WATER CONTENT	
	Blows/Ft.		Tons/Sq. Ft.		Percent	
4 1/4"	20	40	0.4	0.8	20	40

ELEV. 82.7



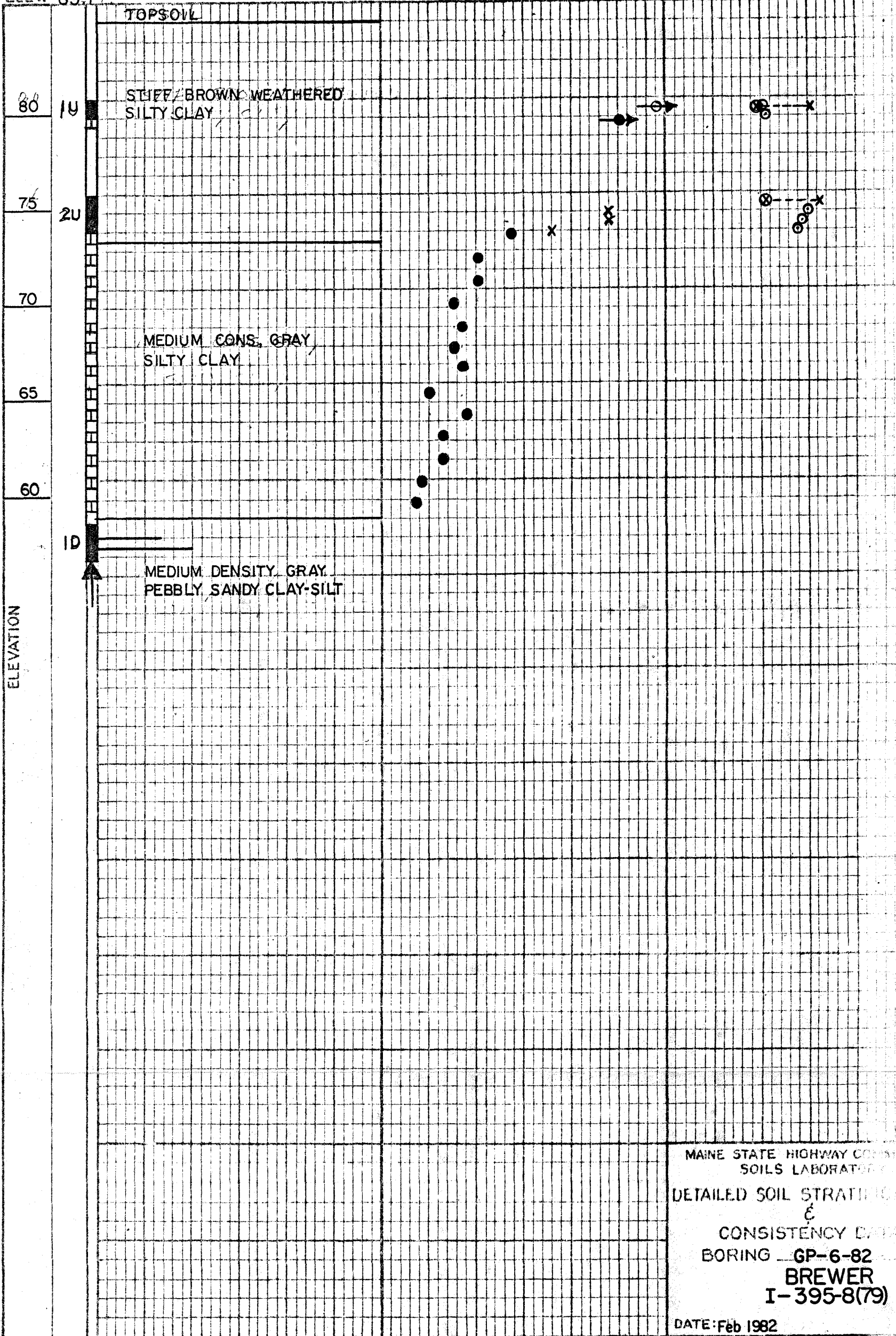
MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-27-81
BREWER
395-8(79)
DATE: SEPTEMBER 1981

BORING GP-6-82

STATION 334+45, 65' Rt.

CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

ELEV. 85.77

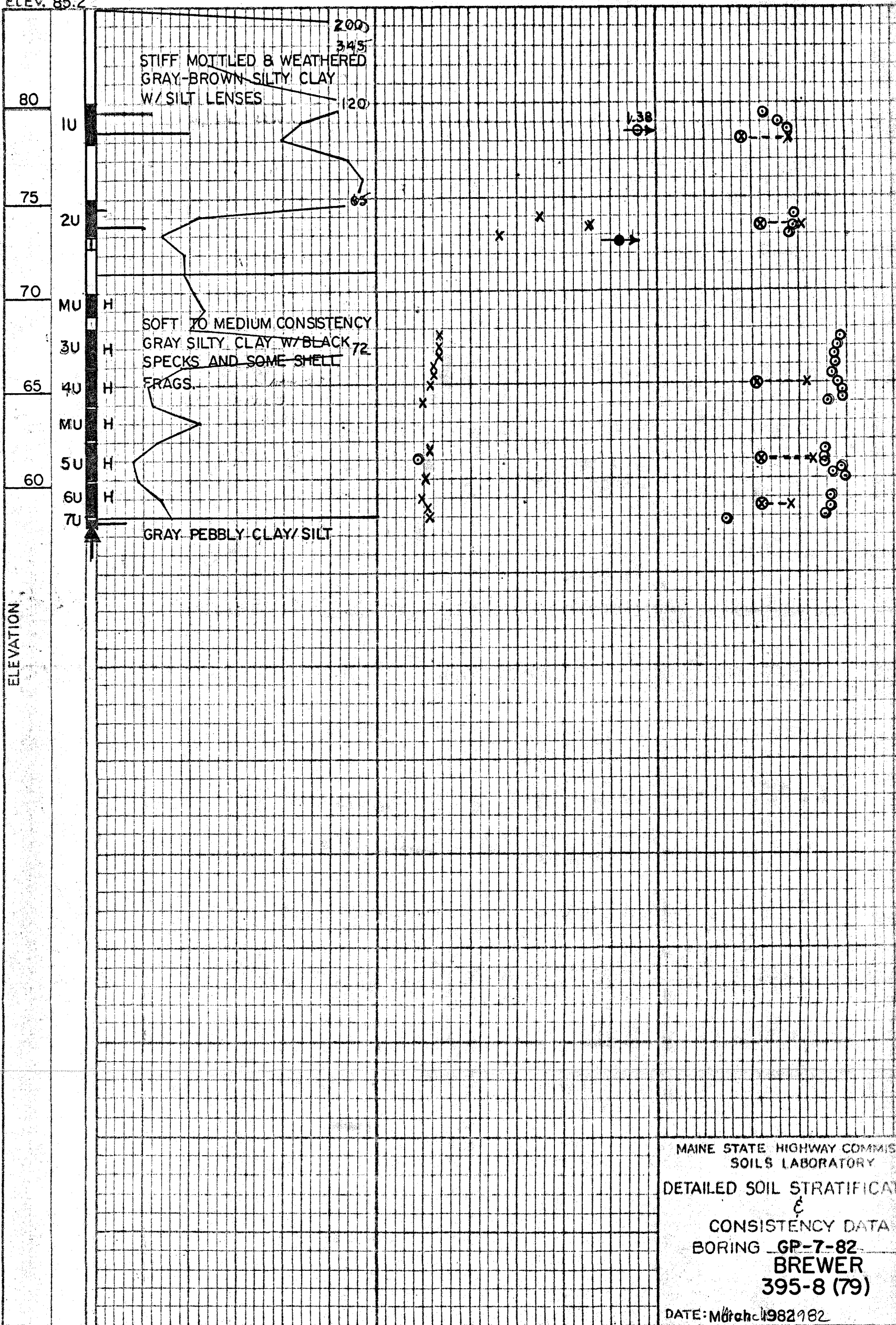


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SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-6-82
BREWER
I-395-8(79)
DATE: Feb 1982

STATION 334 +45, 60' Rt' Rt.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
4"	20	40	0.4	0.8	20	40

ELEV. 85.2



MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-7-82
BREWER
395-8 (79)

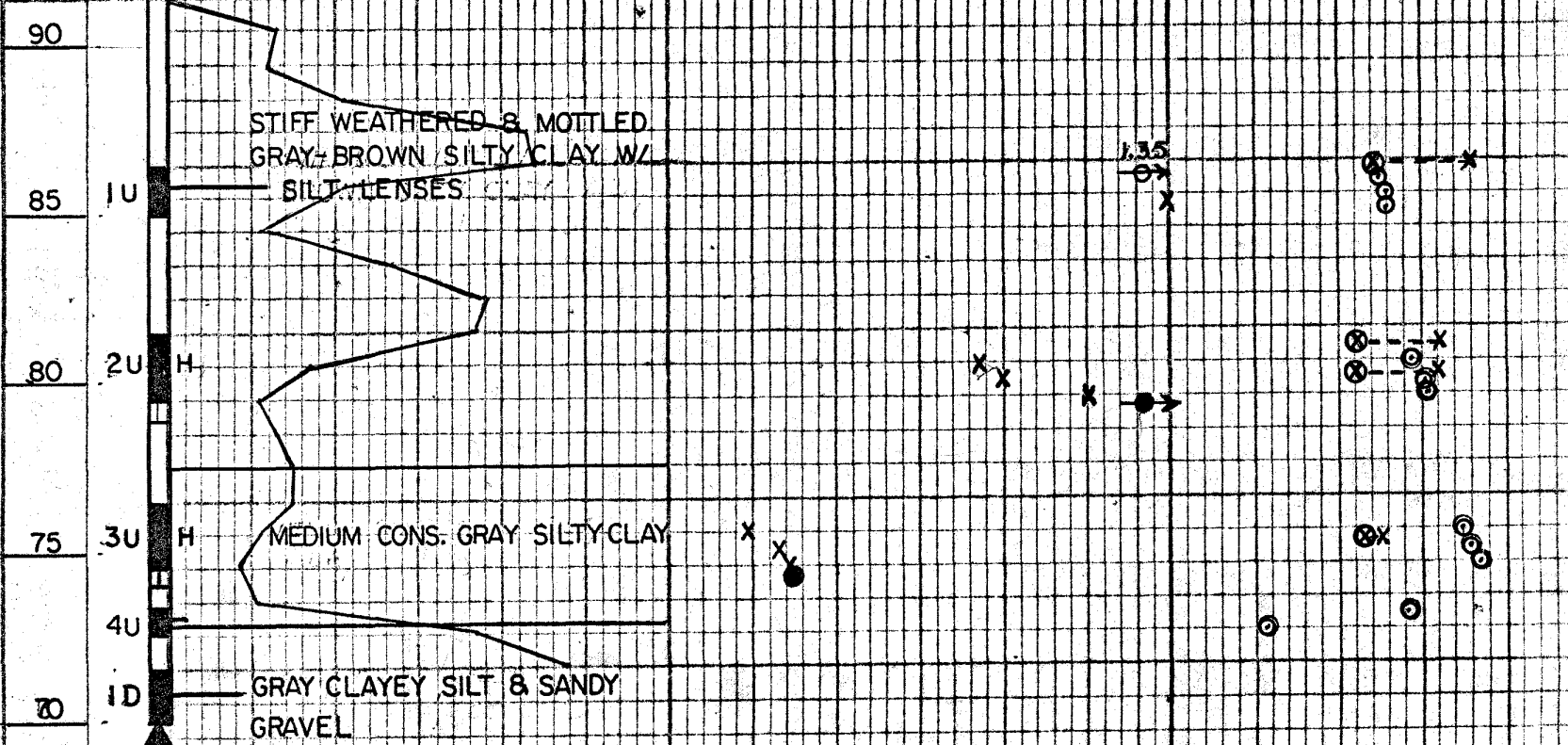
DATE: March 1982

BORING GP-8-82

STATION 332+50, 20' Lt

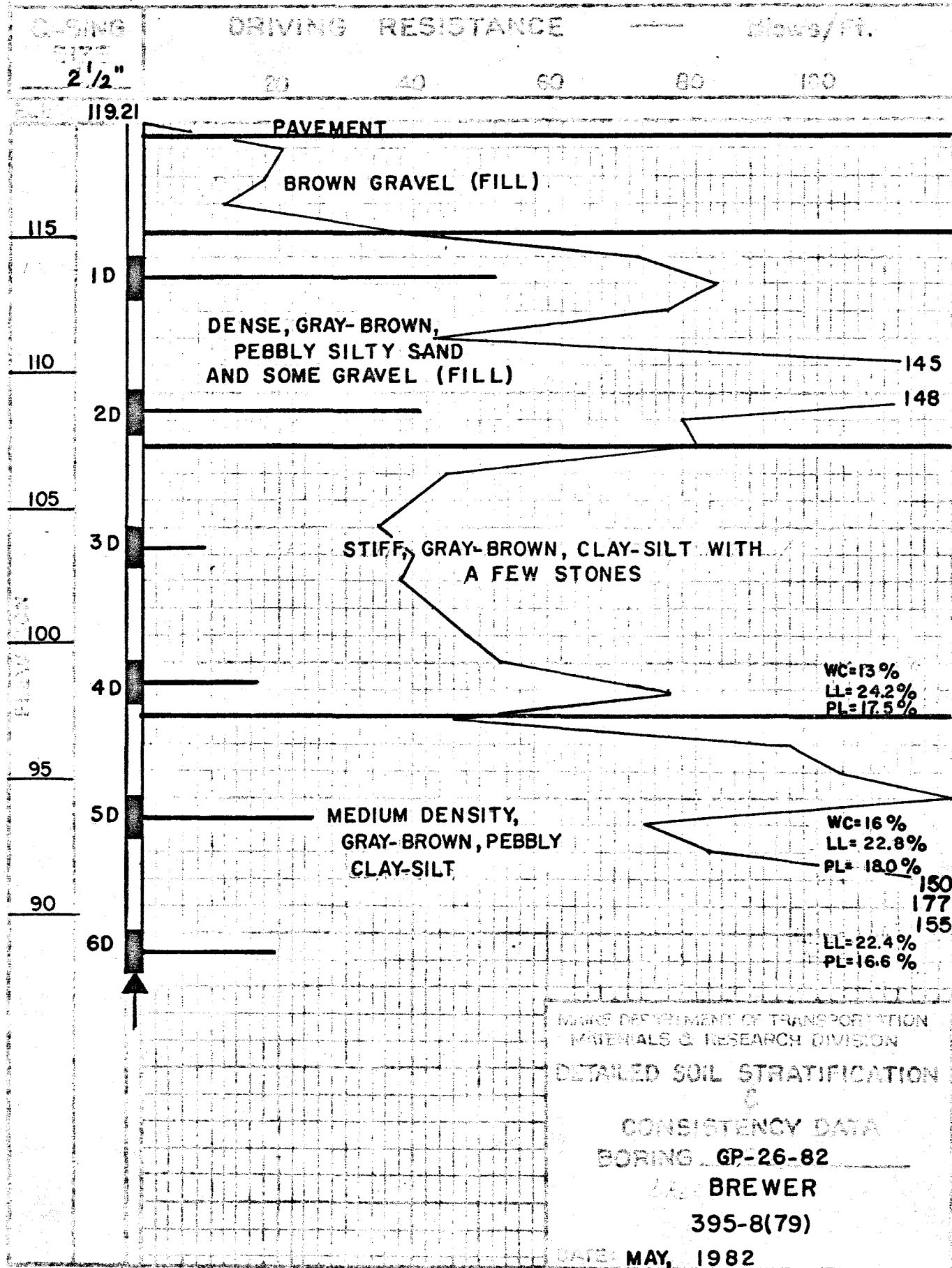
CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40

ELEV. 91.41

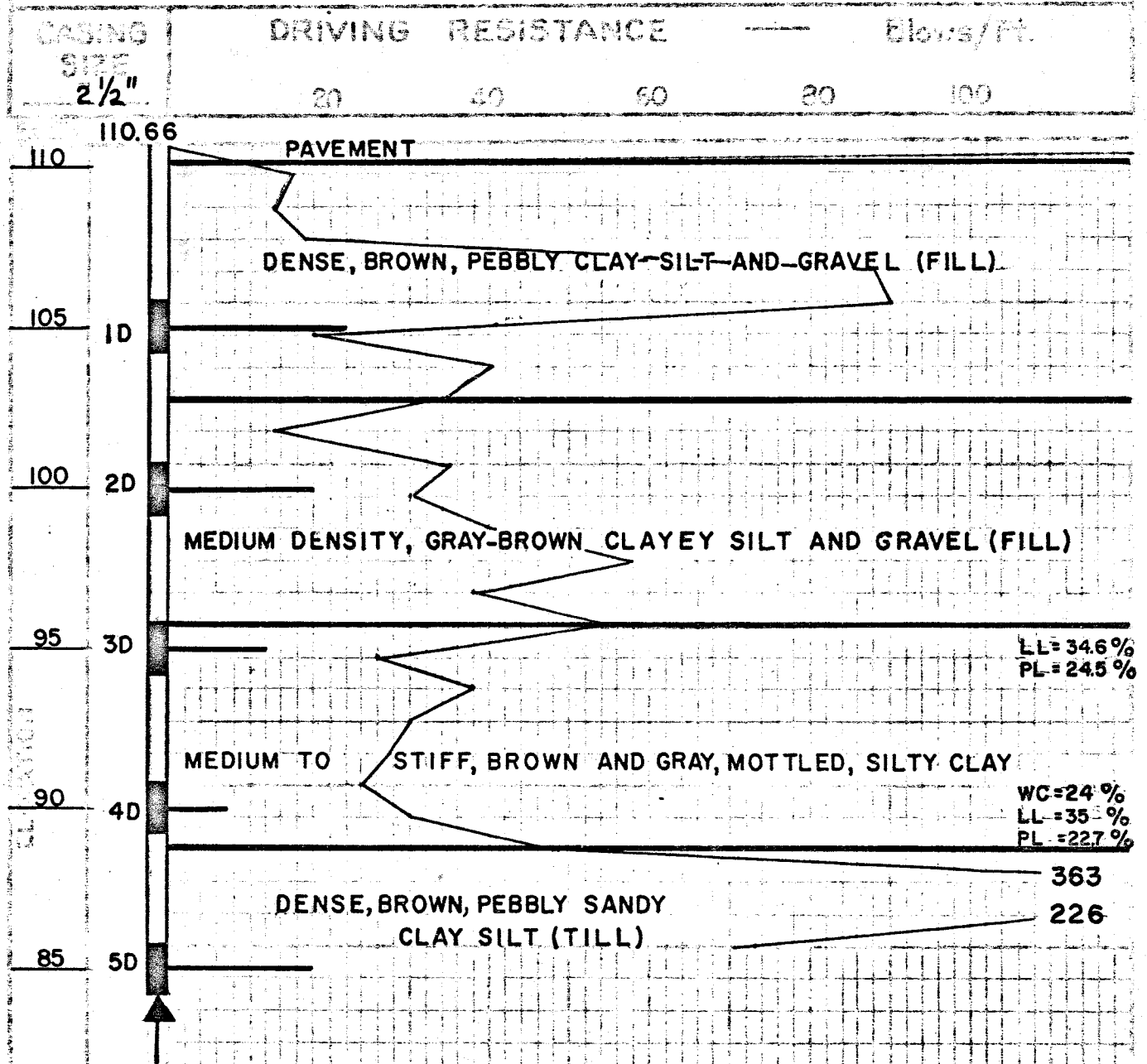


MAINE STATE HIGHWAY COMMISS
SOILS LABORATORY
DETAILED SOIL STRATIFICATI
&
CONSISTENCY DATA
BORING GP-8-82
BREWER
395-8(78)
DATE: MARCH 1982

BORING GP-26-82 STATION 68+ 07, 24' LT.(RT.1A)



BORING GP-27-82 STATION 66+69.25' RT. (RT. 1A4)



MAINE DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

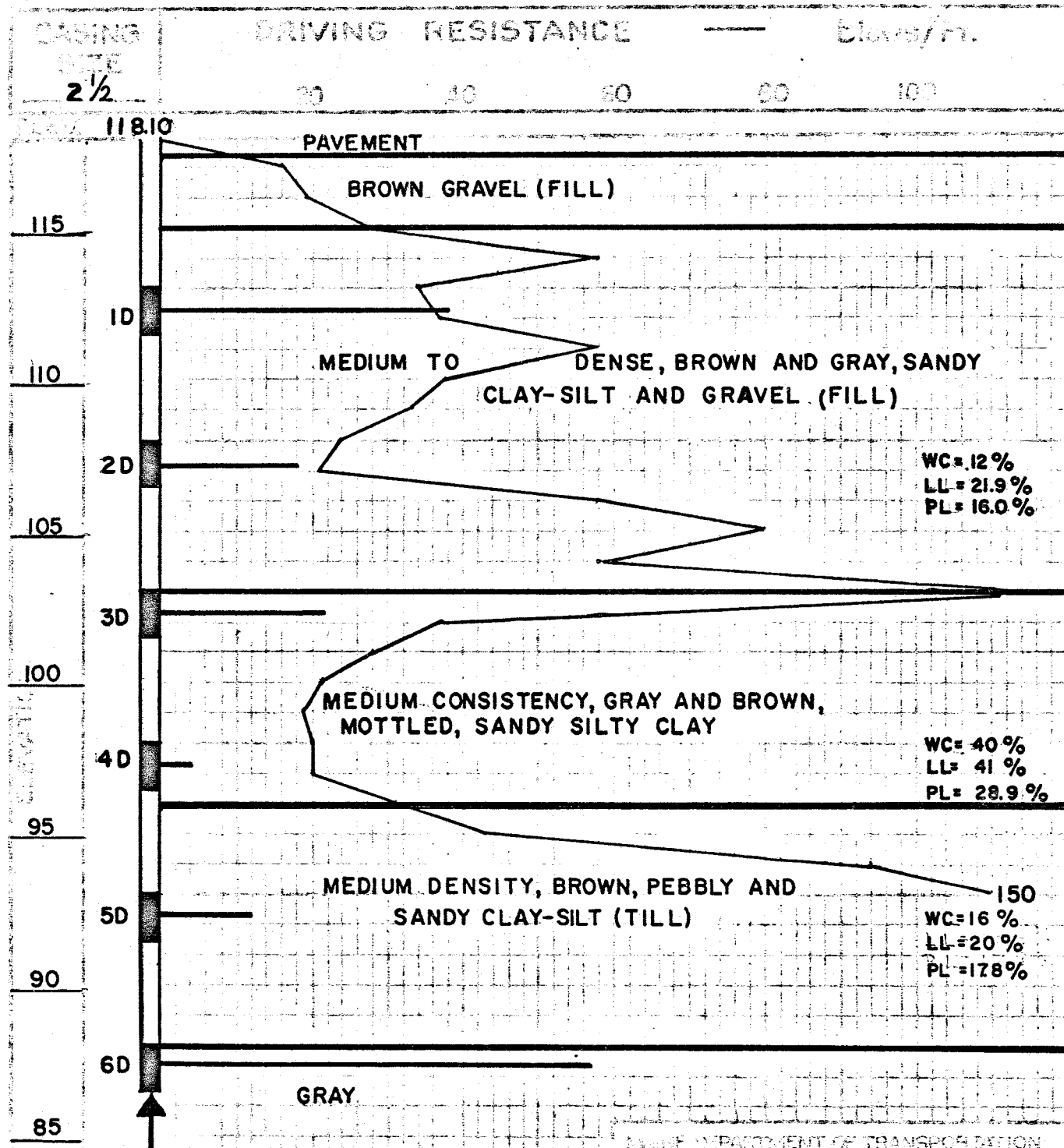
DETAILED SOIL STRATIFICATION

CONSISTENCY DATA
BORING GP-27-82

BREWER
395-8(79)

DATE: JUNE, 1982

BORING GP-28-82 STATION 67 + 95, 25' RT. (RT. 1A)



STATE DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

CONSISTENCY DATA

BORING GP-28-82

BREWER

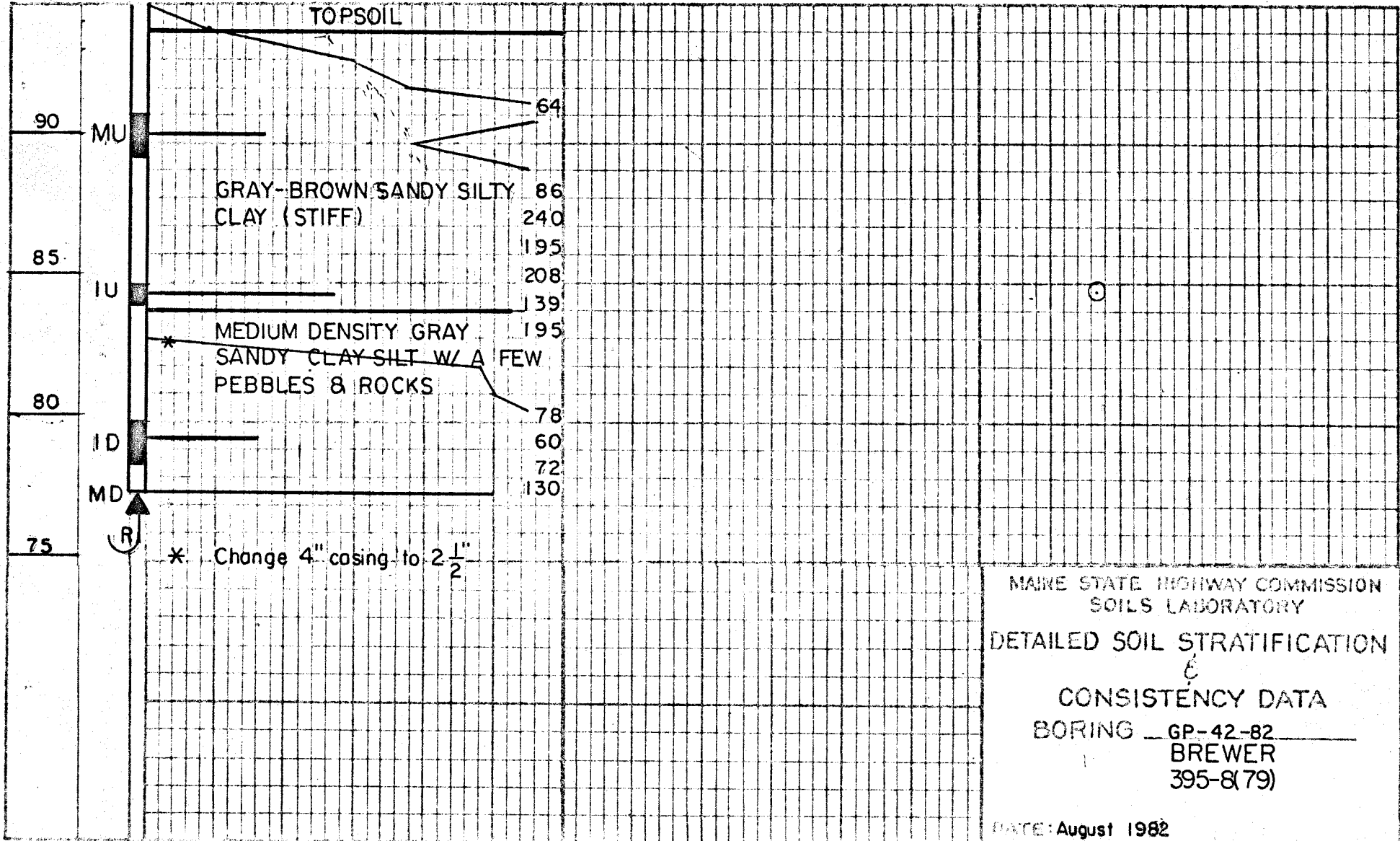
395-8(79)

DATE: JUNE, 1982

BORING GP-42-82 STATION 65+00 82' Lt.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4" 8 2 1/2"	20 40	0.4 0.8	20 40

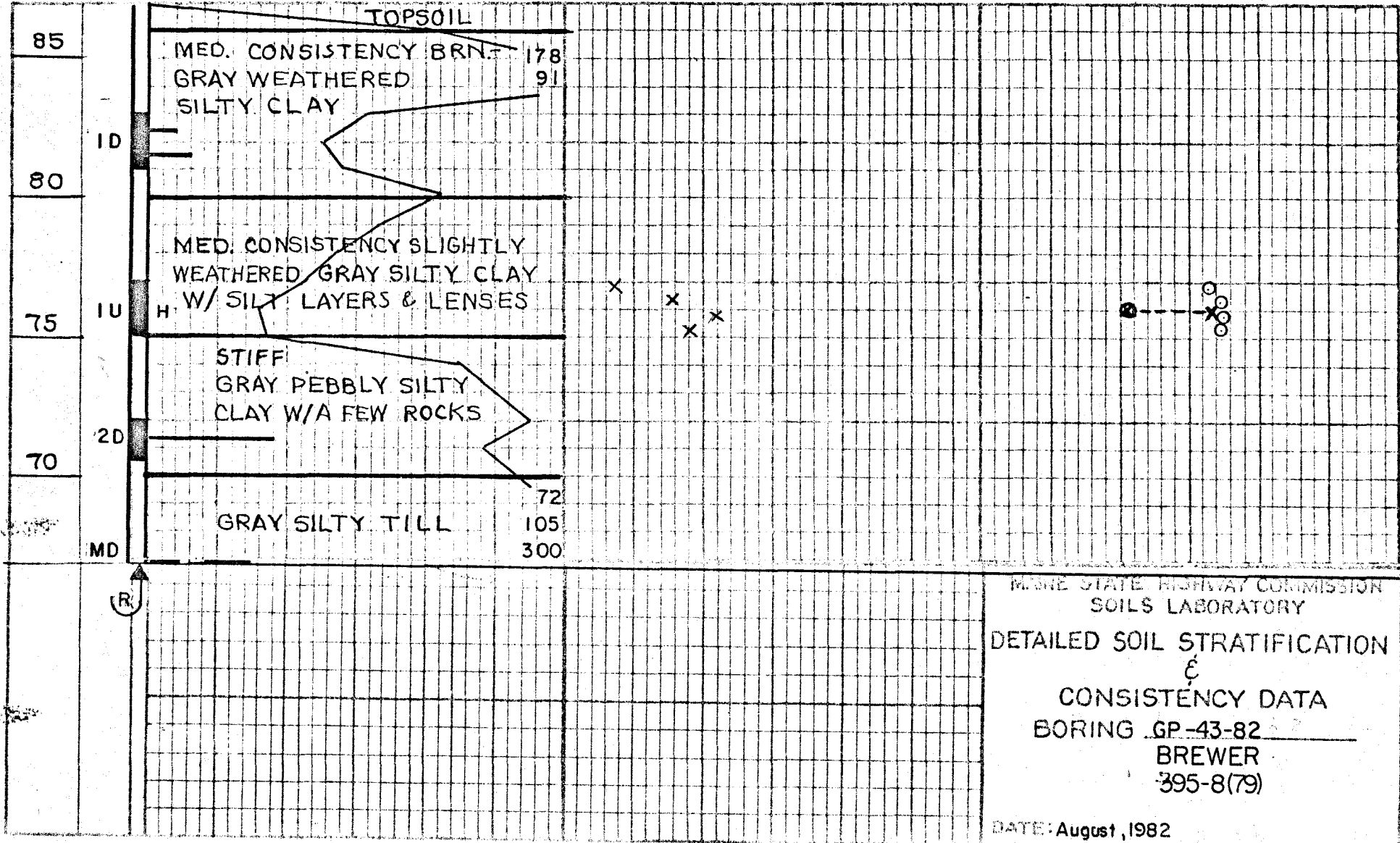
ELEV. 94.6



BORING GP-43-82 STATION 52+50 40' Rt. (RT. 1A)

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent
4"	20 40	0.4 0.8	20 40

ELEV. 86.9

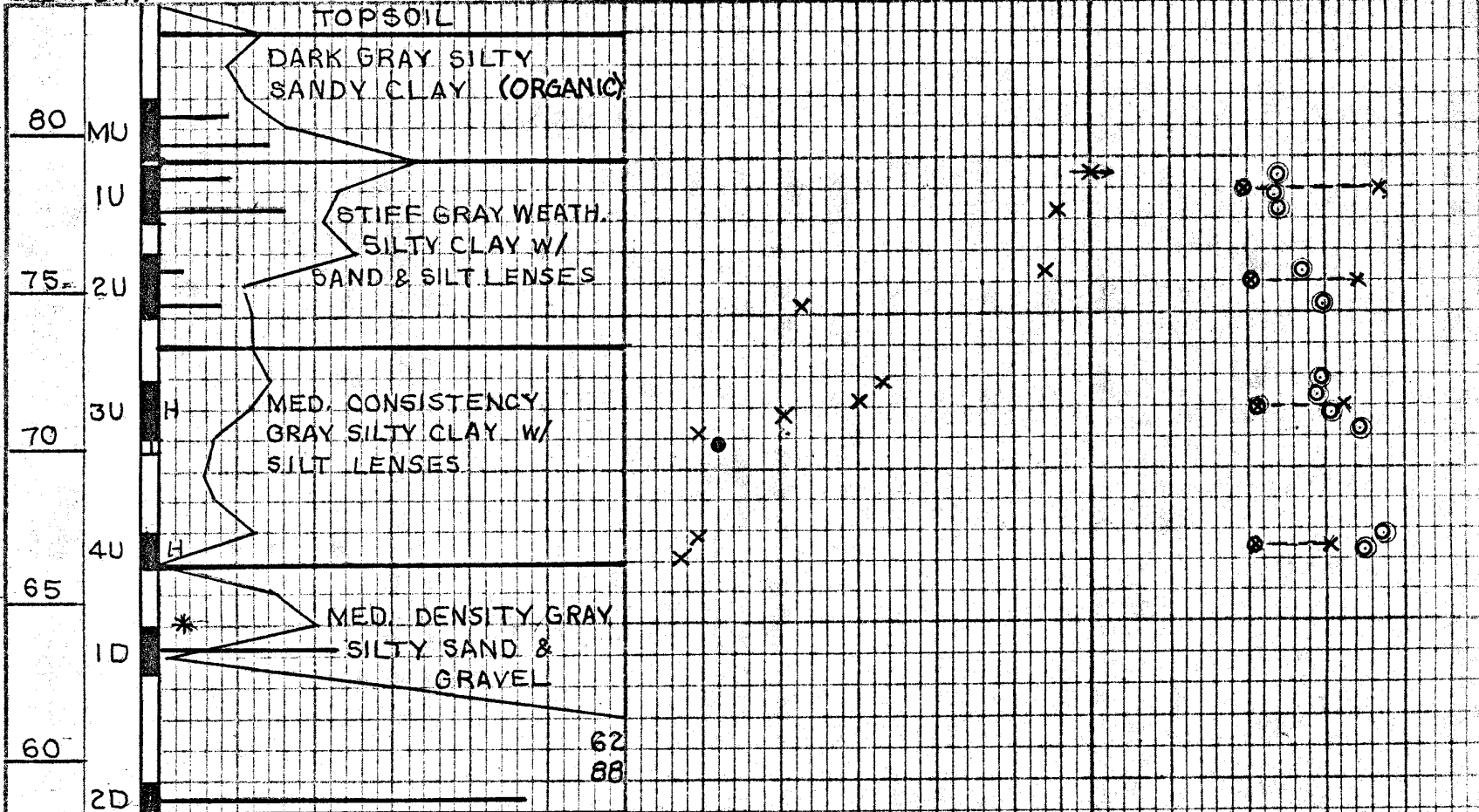


BORING GP-44-82

STATION 53 + 50 55' LT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq Ft.		WATER CONTENT Percent	
4" & 2 1/2"	20	40	0.4	0.8	20	40

ELEV. 84.1



* CHANGED 4" CASING TO 2 1/2"

ELEVATION

MAINE STATE HIGHWAY COMMISS
SOILS LABORATORY

DETAILED SOIL STRATIFICATI

CONSISTENCY DATA
BORING GP-44-82

BREWER

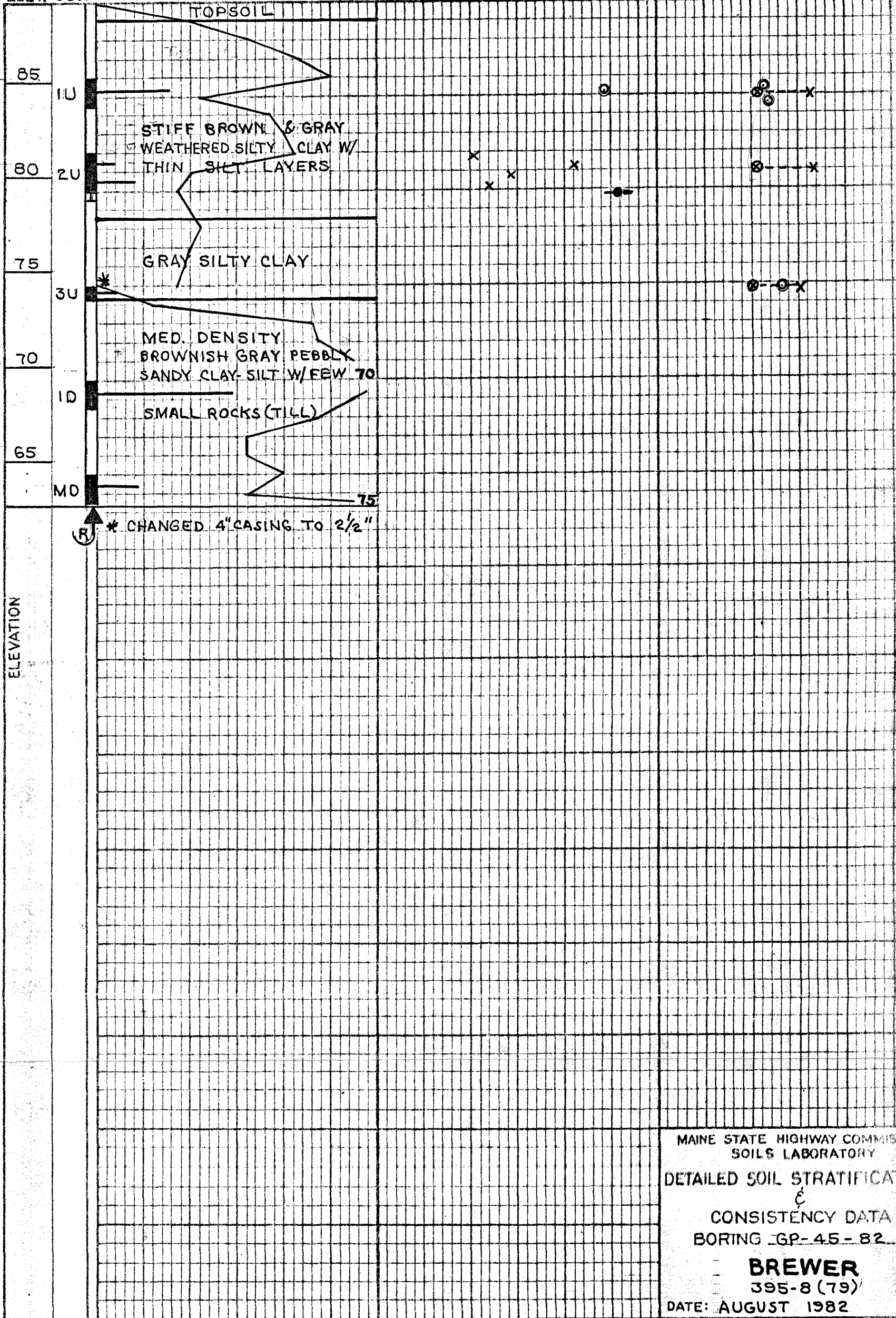
395-8 (79)

DATE: AUGUST 1982

BORING GP-45-82 STATION 66+00 79' RT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq.Ft.	WATER CONTENT Percent
4" & 2 1/2"	20 40	0.4 0.8	20 40

ELEV. 89.7

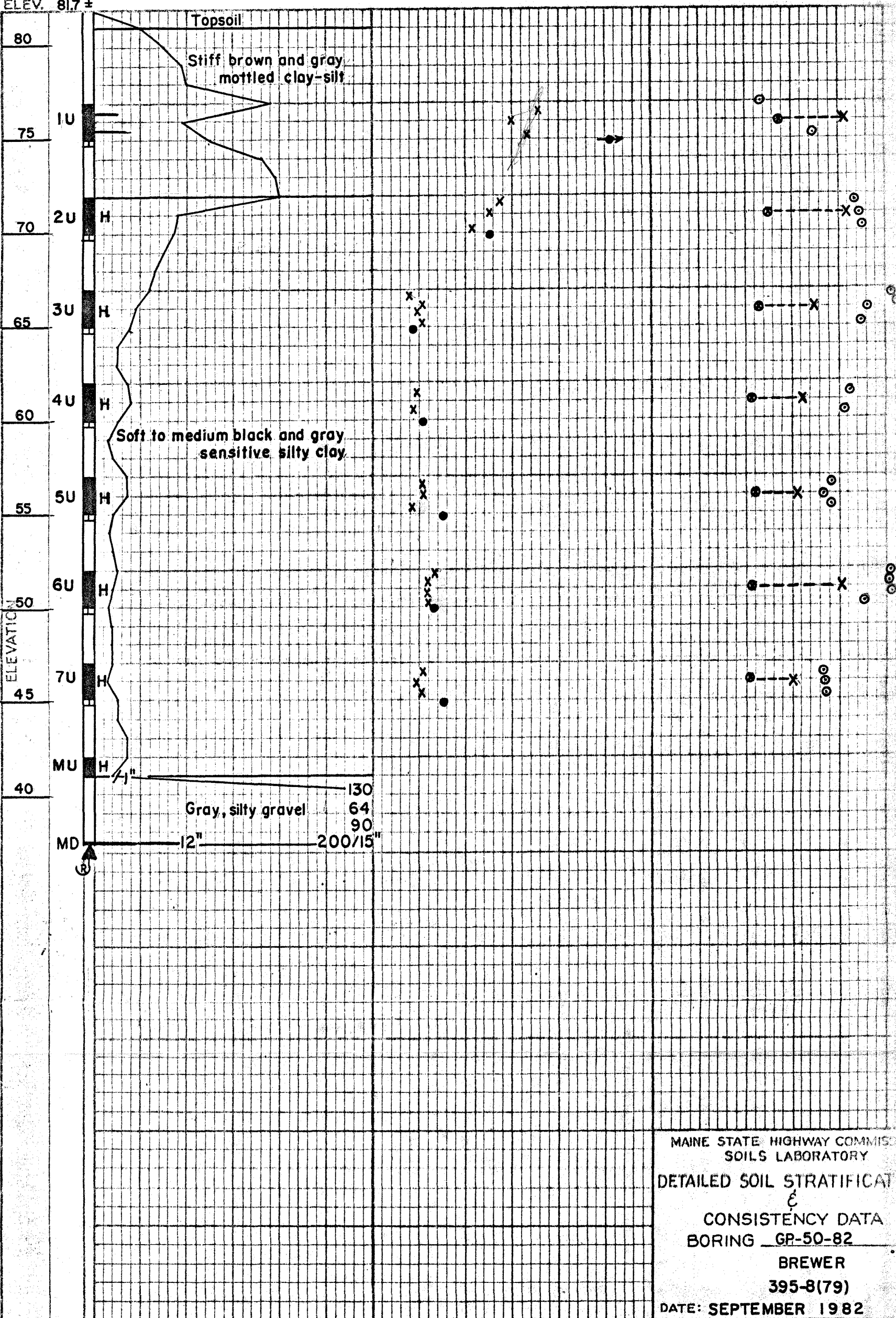


MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-45-82
BREWER
395-8 (79)
DATE: AUGUST 1982

BORING GP-50-82 **STATION** 64+50, 180' RT.

CASING SIZE	DRIVING RESISTANCE Blows/Ft.	VANE SHEAR STRENGTH Tons/Sq. Ft.	WATER CONTENT Percent	
			20	40

ELEV. 81.7 ±

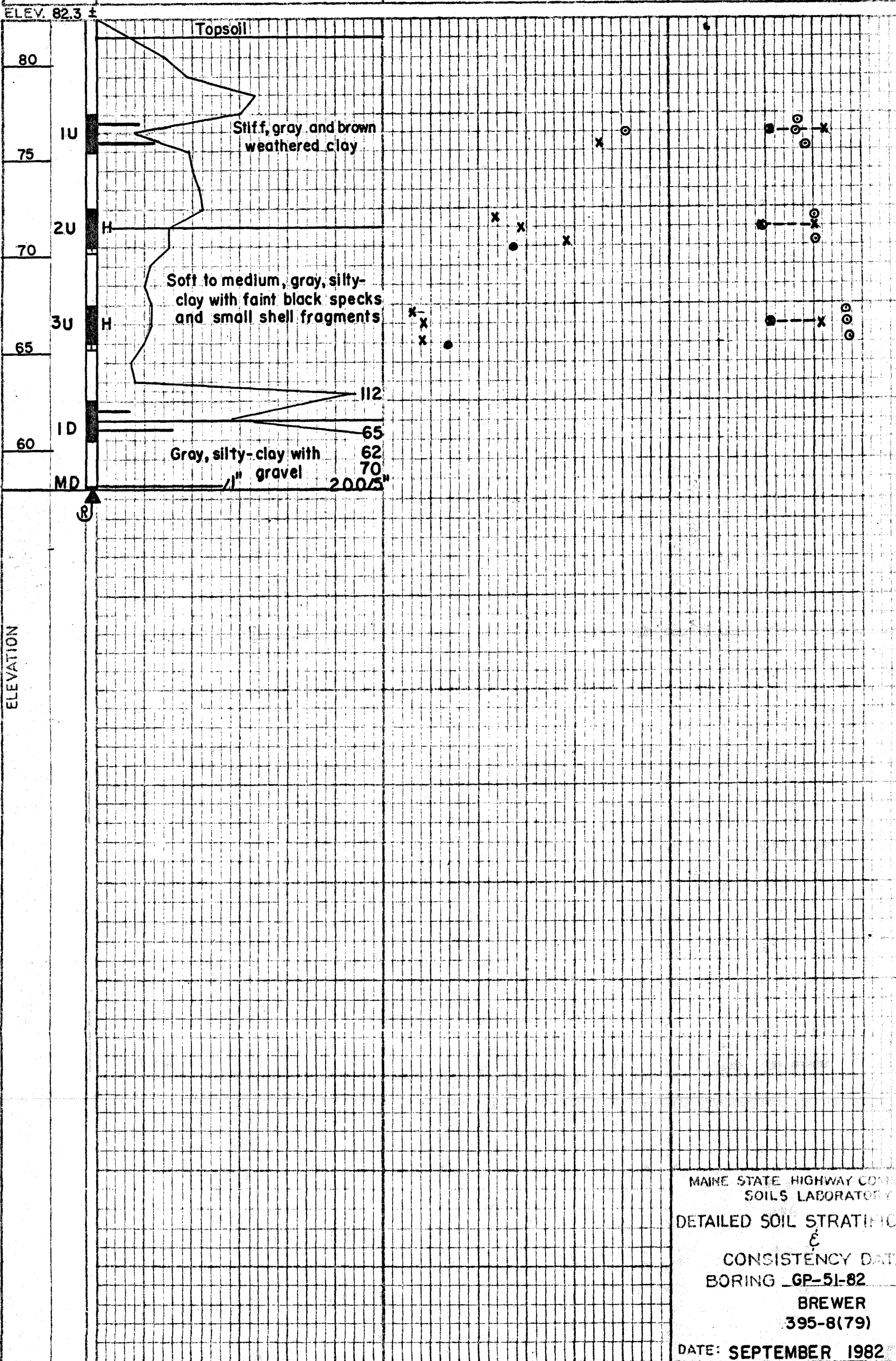


MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-50-82
BREWER
395-8(79)
DATE: SEPTEMBER 1982

BORING GP-51-82

STATION 63+80.120' RT

CASING SIZE 4"	DRIVING RESISTANCE Blows/Ft.		VANE SHEAR STRENGTH Tons/Sq. Ft.		WATER CONTENT Percent	
	20	40	0.4	0.8	20	40



BORING GP-52-82

STATION 65+30 120^{RT}

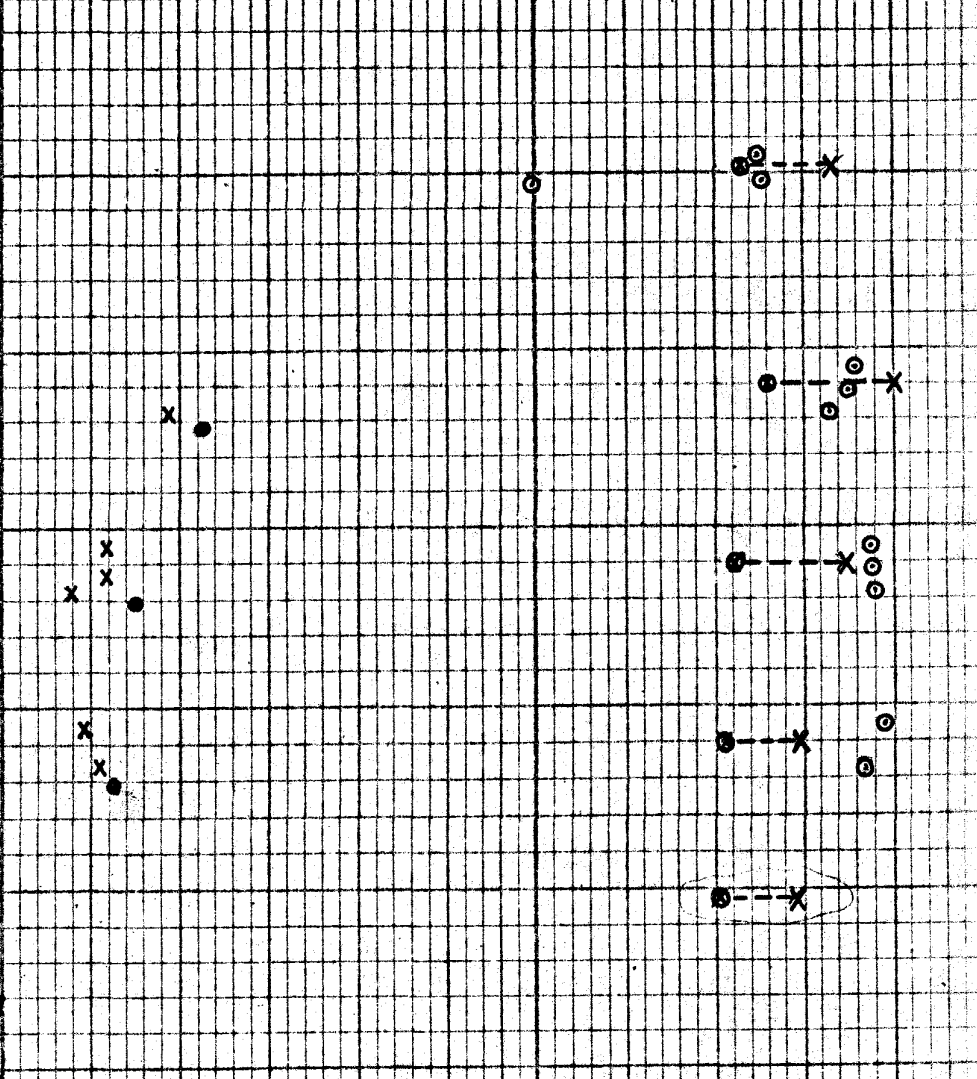
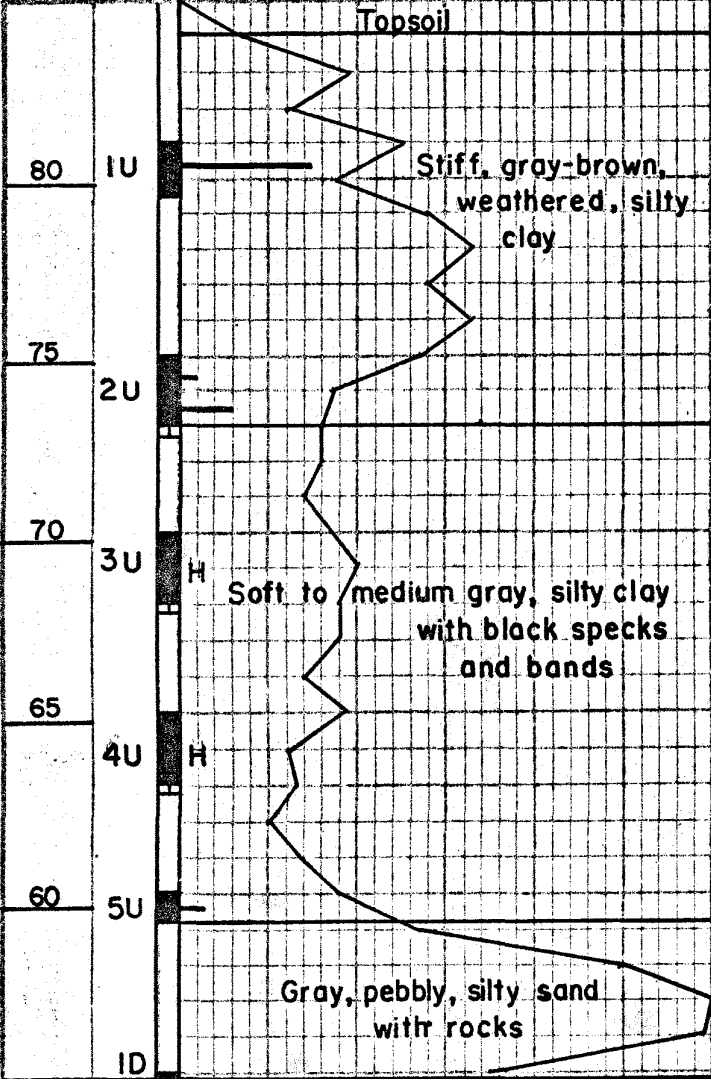
CASING
SIZE
4"

DRIVING RESISTANCE
Blows/Ft.
20 40

VANE SHEAR STRENGTH
Tons/Sq.Ft.
0.4 0.8

WATER CONTENT
Percent
20 40

ELEV. 85.11



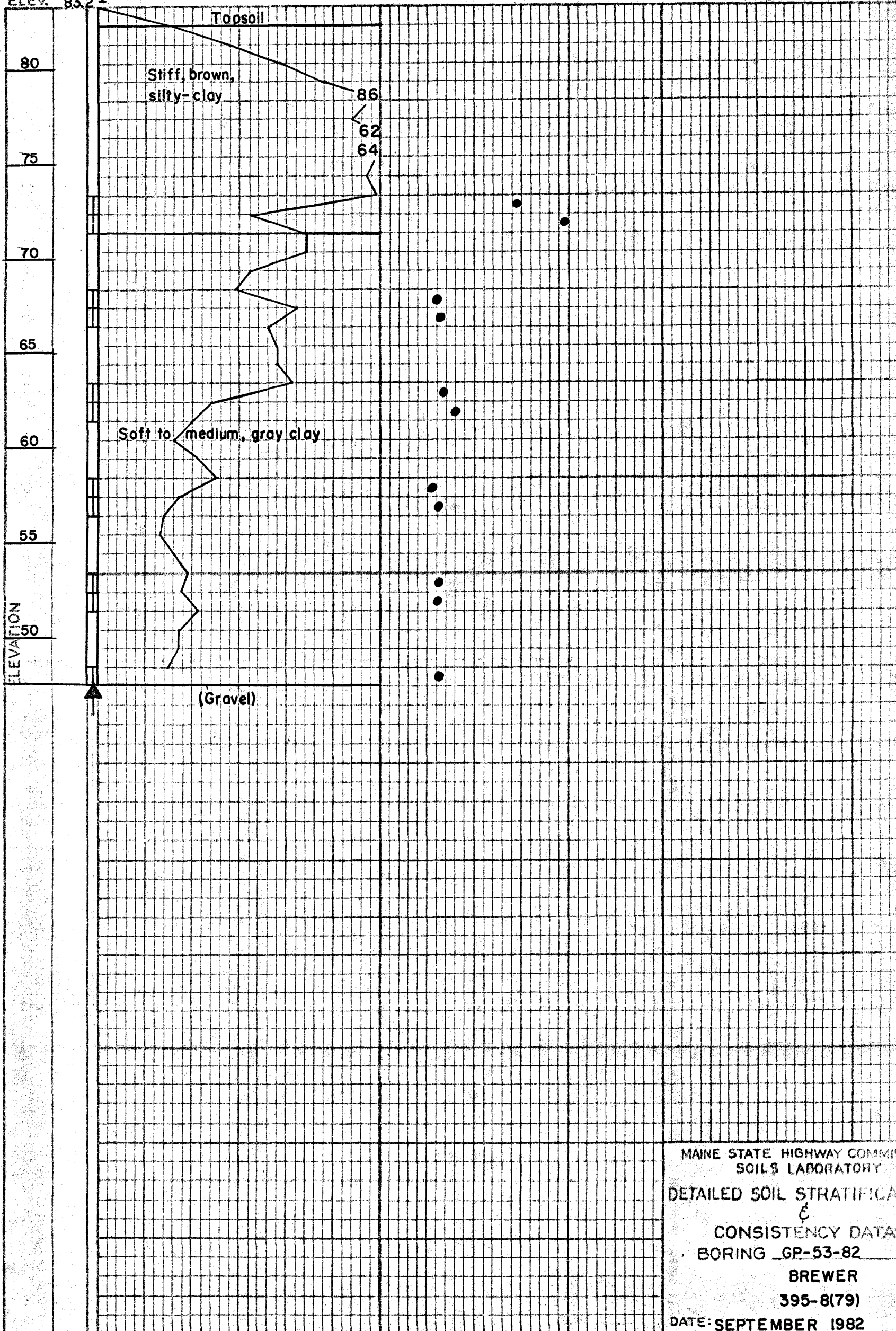
MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
CONSISTENCY DATA
BORING GP-52-82
BREWER
395-8(79)
DATE: SEPTEMBER, 1982

BORING GP-53-82

STATION 65+30 180' RT.

CASING
SIZE
4"DRIVING RESISTANCE
Blows/Ft.
20 40VANE SHEAR STRENGTH
Tons/Sq.Ft.
0.4 0.8WATER CONTENT
Percent
20 40

ELEV. 83.2 ±

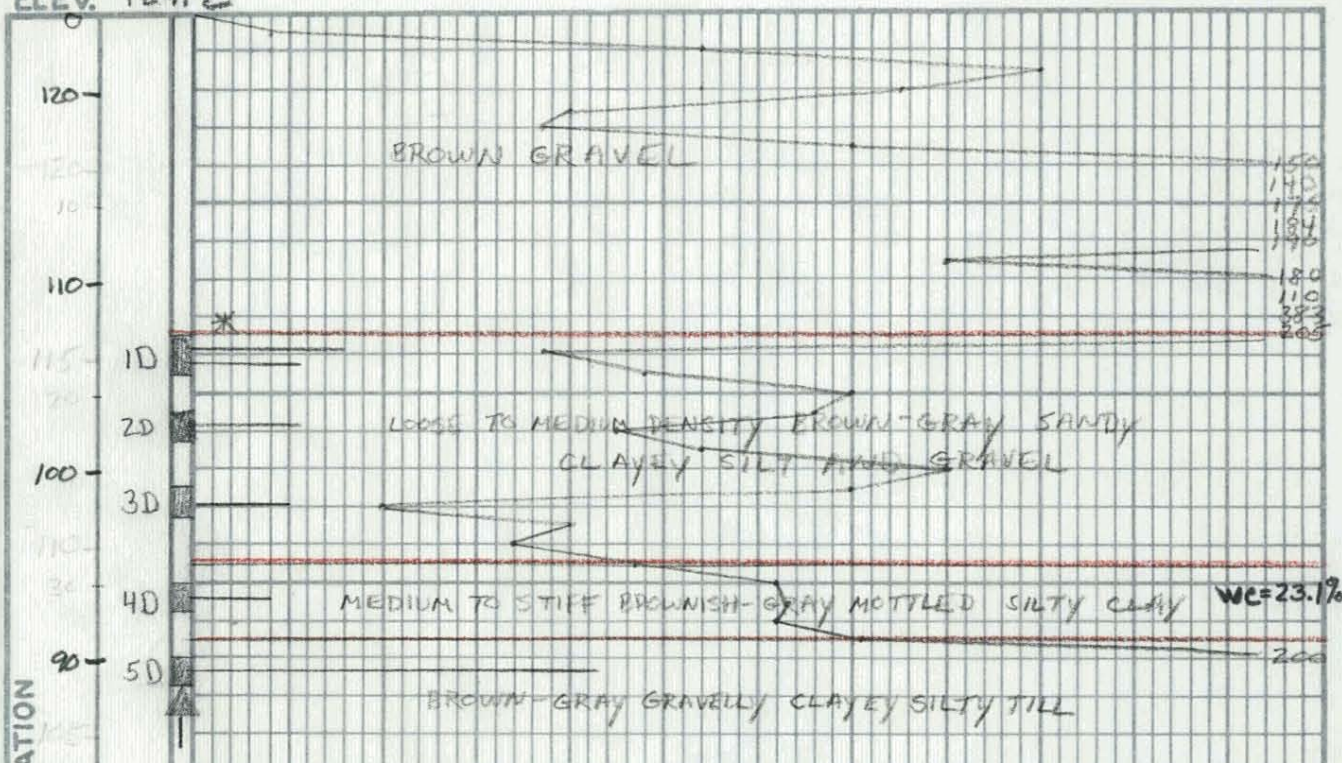


MAINE STATE HIGHWAY COMMISSION
SOILS LABORATORY
DETAILED SOIL STRATIFICATION
&
CONSISTENCY DATA
BORING GP-53-82
BREWER
395-8(79)
DATE: SEPTEMBER 1982

BORING GP-43-84 STATION

CASING SIZE	DRIVING RESISTANCE	Blows/Ft.
4" + 2 1/2"	20 40 60 80 100	

ELEV. 124.2



MAINE DEPARTMENT OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

DETAILED SOIL STRATIFICATION

&
CONSISTENCY DATA

BORING GP-43-84

DATE:

BORING GP-44-84 STATION _____

CASING
SIZE

4" \pm 2 1/2"

DRIVING RESISTANCE

Blows/Ft.

20

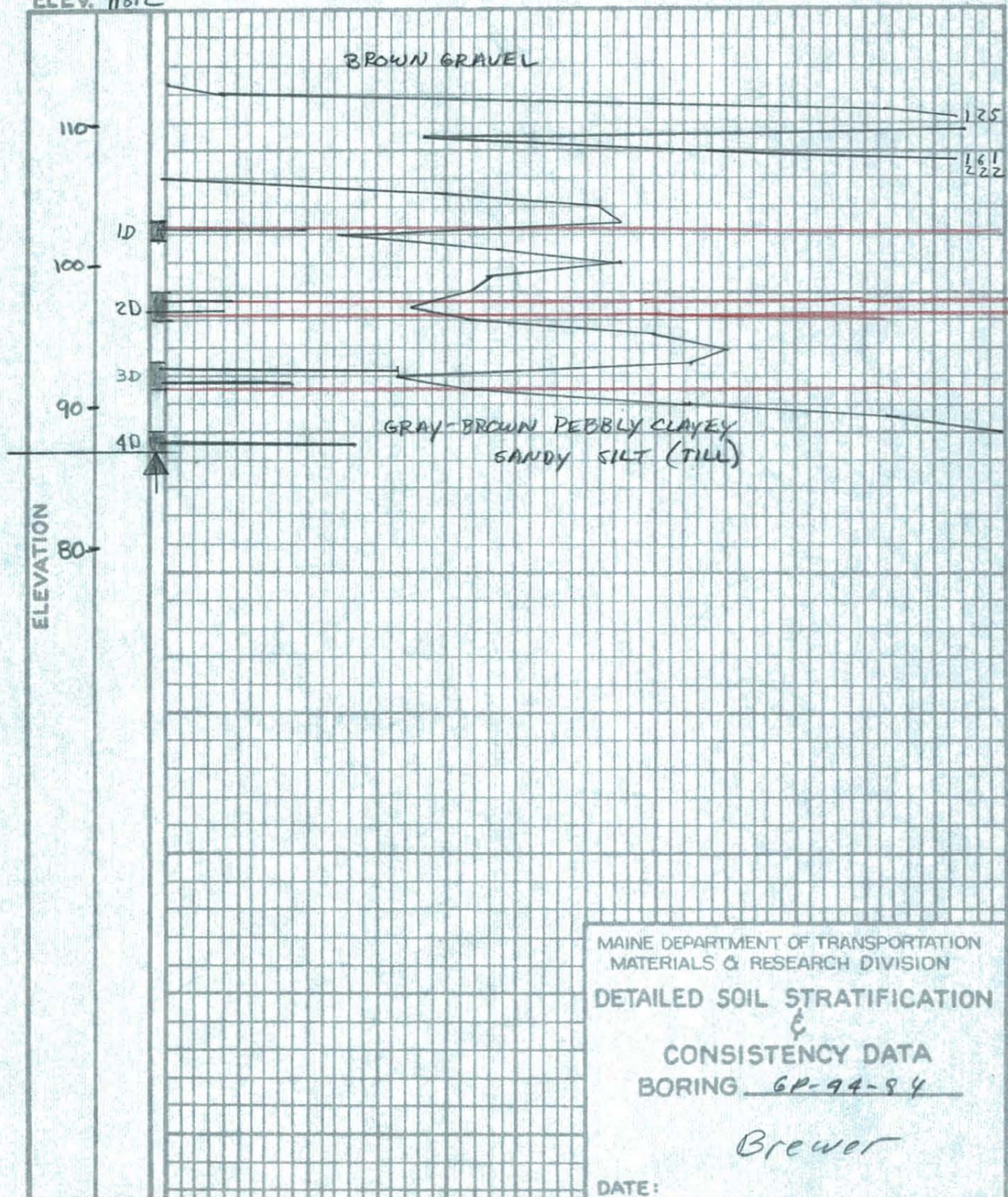
40

60

80

100

ELEV. 118.2



APPENDIX C

Observation Well Installation and Groundwater Monitoring Reports

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-BEA-102(OW)															
			Boring No. BB-BEA-102															
PROJECT LOCATION CLIENT CONTRACTOR DRILLER	Route 9/I-395 Connector Brewer, Maine/Eddington, Maine Maine Department of Transportation Northern Test Borings Inc. M. Nadeau		H&A FILE NO. PROJECT MGR. FIELD REP. DATE INSTALLED WATER LEVEL															
		132076-002 B. Steinert N. Klausmeyer 8/2/2018 12.5 ft (depth below gs)																
Ground El. <u>139.3</u> ft El. Datum <u>NAVD 88</u>		Location <u>SEE PLAN</u>																
		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box																
SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL	Type of protective cover <u>Locking Cap</u>																
Granular -FILL-	-FILTER SAND- 2.0 ft	Height of top of guard pipe above ground surface <u>2.0</u> ft																
	-BENTONITE- 4.0 ft	Height of top of riser pipe above ground surface <u>1.9</u> ft																
6.5 ft	-GLACIAL TILL-	Type of protective casing: <u>Steel Guard Pipe</u>																
		Length <u>7.0</u> ft Inside Diameter <u>4.0</u> in																
		Depth of bottom of guard pipe <u>3.0</u> ft																
		<table style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="text-align: left; border-bottom: 1px solid black;">Type of Seals</th> <th style="text-align: left; border-bottom: 1px solid black;">Top of Seal (ft)</th> <th style="text-align: left; border-bottom: 1px solid black;">Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Concrete</td> <td>--</td> <td>--</td> </tr> <tr> <td>Bentonite Seal</td> <td><u>3.0</u></td> <td><u>1.0</u></td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>		Type of Seals	Top of Seal (ft)	Thickness (ft)	Concrete	--	--	Bentonite Seal	<u>3.0</u>	<u>1.0</u>						
Type of Seals	Top of Seal (ft)	Thickness (ft)																
Concrete	--	--																
Bentonite Seal	<u>3.0</u>	<u>1.0</u>																
		Type of riser pipe: <u>Sch. 40 PVC</u>																
		Inside diameter of riser pipe <u>2.0</u> in																
		Type of backfill around riser <u>Bentonite/Sand</u>																
		Diameter of borehole <u>4.0</u> in																
		Depth to top of well screen <u>5.0</u> ft																
		Type of screen <u>Sch. 40 PVC</u>																
		Screen gauge or size of openings <u>0.010</u> in																
		Diameter of screen <u>2.0</u> in																
		Type of backfill around screen <u>No. 2 Filter Sand</u>																
		Depth of bottom of well screen <u>15.0</u> ft																
		Bottom of Silt trap <u>15.2</u> ft																
		Depth of bottom of borehole <u>15.2</u> ft																
15.0 15.0 (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)																
<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;"> <u>6.9</u> ft </td> <td style="text-align: center;">+ <u>10.0</u> ft</td> <td style="text-align: center;">+ <u>0.2</u> ft</td> <td style="text-align: center;">= <u>17.1</u> ft</td> </tr> <tr> <td style="text-align: center;">Riser Pay Length (L1)</td> <td style="text-align: center;">Length of screen (L2)</td> <td style="text-align: center;">Length of silt trap (L3)</td> <td style="text-align: center;">Pay length</td> </tr> </table>				<u>6.9</u> ft	+ <u>10.0</u> ft	+ <u>0.2</u> ft	= <u>17.1</u> ft	Riser Pay Length (L1)	Length of screen (L2)	Length of silt trap (L3)	Pay length							
<u>6.9</u> ft	+ <u>10.0</u> ft	+ <u>0.2</u> ft	= <u>17.1</u> ft															
Riser Pay Length (L1)	Length of screen (L2)	Length of silt trap (L3)	Pay length															
COMMENTS:																		



BB-BEA-102

Page 1 of 1

ELEVATION OF REFERENCE POINT (ft)	139.3	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-BEB-102(OW)															
			Boring No. BB-BEB-102															
PROJECT LOCATION CLIENT CONTRACTOR DRILLER	Route 9/I-395 Connector Brewer, Maine/Eddington, Maine Maine Department of Transportation New England Boring Contractors B. Enos		H&A FILE NO. PROJECT MGR. FIELD REP. DATE INSTALLED WATER LEVEL															
		132076-002 B. Steinert N. Klausmeyer 11/7/2018 --																
Ground El. <u>75.3</u> ft El. Datum <u>NAVD 88</u>		Location <u>SEE PLAN</u>	<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box															
SOIL/ROCK CONDITIONS <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> -MARINE DEPOSIT- 14.8 ft </div> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> -GLACIAL TILL- 27.0 </div>	BOREHOLE BACKFILL -FILTER SAND- 3.0 ft -BENTONITE- 8.0 ft -FILTER SAND- 27.0	<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> </div> <div style="width: 50%;"> <p>Type of protective cover <u>Locking Cap</u></p> <p>Height of top of guard pipe above ground surface <u>3.0</u> ft</p> <p>Height of top of riser pipe above ground surface <u>2.9</u> ft</p> <p>Type of protective casing: <u>Steel Guard Pipe</u></p> <p>Length <u>5.0</u> ft</p> <p>Inside Diameter <u>3.0</u> in</p> <p>Depth of bottom of guard pipe <u>2.0</u> ft</p> <table border="1" style="width: 100%; border-collapse: collapse; margin-top: 10px;"> <thead> <tr> <th>Type of Seals</th> <th>Top of Seal (ft)</th> <th>Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Bentonite Seal</td> <td>3.0</td> <td>5.0</td> </tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table> <p>Type of riser pipe: <u>Sch. 40 PVC</u></p> <p>Inside diameter of riser pipe <u>2.0</u> in</p> <p>Type of backfill around riser <u>Hollister Filter Sand</u></p> <p>Diameter of borehole <u>4.0</u> in</p> <p>Depth to top of well screen <u>10.0</u> ft</p> <p>Type of screen <u>Sch. 40 PVC</u></p> <p>Screen gauge or size of openings <u>0.010</u> in</p> <p>Diameter of screen <u>2.0</u> in</p> <p>Type of backfill around screen <u>Hollister Filter Sand</u></p> <p>Depth of bottom of well screen <u>19.8</u> ft</p> <p>Bottom of Silt trap <u>20.0</u> ft</p> <p>Depth of bottom of borehole <u>27.0</u> ft</p> </div> </div>		Type of Seals	Top of Seal (ft)	Thickness (ft)	Bentonite Seal	3.0	5.0									
Type of Seals	Top of Seal (ft)	Thickness (ft)																
Bentonite Seal	3.0	5.0																
(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)																
<div style="display: flex; justify-content: space-around; align-items: center;"> <div> <u>12.9</u> ft Riser Pay Length (L1) </div> <div>+</div> <div> <u>9.8</u> ft Length of screen (L2) </div> <div>+</div> <div> <u>0.2</u> ft Length of silt trap (L3) </div> <div>=</div> <div> <u>22.9</u> ft Pay length </div> </div>																		
COMMENTS:																		

GROUNDWATER MONITORING REPORT

OW/PZ NUMBER

BB-BEB-102

Page 1 of 1

PROJECT	Route 9/I-395 Connector
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LOCATION	Brewer, Maine / Eddington, Maine
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CLIENT	Maine Department of Transportation
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CONTRACTOR	New England Boring Contractors
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H&A FILE NO. 132076-002

PROJECT MGR.	B. Steinert
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FIELD REP. H. Hollauer/N. Klausmeyer

DATE 12/19/2018

ELEVATION OF REFERENCE POINT (ft)	75.3
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REFERENCE POINT: Ground Surface ☒ PVC ☐ Other ☐

[illegible]

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-BWS-102(OW)															
			Boring No. BB-BWS-102															
PROJECT	Route 9/I-395 Connector		H&A FILE NO.															
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.															
CLIENT	Maine Department of Transportation		FIELD REP.															
CONTRACTOR	Northern Test Borings Inc.		DATE INSTALLED															
DRILLER	M. Nadeau		WATER LEVEL															
			132076-002															
			B. Steinert															
			N. Klausmeyer															
			7/12/2018															
			2.9 ft. (depth below gs)															
Ground El.	112.2	ft																
El. Datum	NAVD 88																	
Location		SEE PLAN																
		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box																
SOIL/ROCK CONDITIONS Clayey SILT -FILL- 7.0 ft -GLACIAL TILL- 18.0 (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)	BOREHOLE BACKFILL -BENTONITE- 2.0 ft -FILTER SAND- 18.0	<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p>Type of protective cover</p> <p>Height of top of guard pipe above ground surface</p> <p>Height of top of riser pipe above ground surface</p> <p>Type of protective casing:</p> <p>Length</p> <p>Inside Diameter</p> <p>Depth of bottom of guard pipe</p> <p>Type of seals</p> <p>Concrete</p> <p>Bentonite Seal</p> <p>Type of riser pipe:</p> <p>Inside diameter of riser pipe</p> <p>Type of backfill around riser</p> <p>Diameter of borehole</p> <p>Depth to top of well screen</p> <p>Type of screen</p> <p>Screen gauge or size of openings</p> <p>Diameter of screen</p> <p>Type of backfill around screen</p> <p>Depth of bottom of well screen</p> <p>Bottom of Silt trap</p> <p>Depth of bottom of borehole</p> </div> <div style="width: 50%;"> <p>Locking Cap</p> <p>2.6 ft</p> <p>2.3 ft</p> <p>Steel Guard Pipe</p> <p>5.0 ft</p> <p>4.0 in</p> <p>2.4 ft</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Type of Seals</th> <th>Top of Seal (ft)</th> <th>Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Concrete</td> <td>--</td> <td>--</td> </tr> <tr> <td>Bentonite Seal</td> <td>0.0</td> <td>2.0</td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table> <p>Sch. 40 PVC</p> <p>2.0 in</p> <p>Bentonite/Sand</p> <p>4.0 in</p> <p>2.7 ft</p> <p>Sch. 40 PVC</p> <p>0.010 in</p> <p>2.0 in</p> <p>No. 2 Filter Sand</p> <p>17.7 ft</p> <p>18.0 ft</p> <p>18.0 ft</p> </div> </div> <div style="display: flex; justify-content: space-between; margin-top: 10px;"> <div style="width: 30%;"> <p>L1</p> <p>L2</p> <p>L3</p> </div> <div style="width: 65%; text-align: right;"> <p>(Not to Scale)</p> </div> </div>		Type of Seals	Top of Seal (ft)	Thickness (ft)	Concrete	--	--	Bentonite Seal	0.0	2.0						
Type of Seals	Top of Seal (ft)	Thickness (ft)																
Concrete	--	--																
Bentonite Seal	0.0	2.0																
<div style="display: flex; justify-content: space-around; align-items: center;"> <div> <p>5.0 ft</p> <p>Riser Pay Length (L1)</p> </div> <div> <p>+</p> </div> <div> <p>15.0 ft</p> <p>Length of screen (L2)</p> </div> <div> <p>+</p> </div> <div> <p>0.3 ft</p> <p>Length of silt trap (L3)</p> </div> <div> <p>=</p> </div> <div> <p>20.3 ft</p> <p>Pay length</p> </div> </div>																		
COMMENTS:																		



BB-BWS-102(OW)

Page 1 of 1

H&A FILE NO. 132076-005

PROJECT MGR.	B. Steinert
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FIELD REP. Klausmeyer/Hollauer/Fletcher

DATE 4/9/2020

ELEVATION OF REFERENCE POINT (ft)	112.2	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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\\haleyaldrich.com\share\por_common\PROJECTS\132076 - brewer eddington\002 - Exploration + Laboratory Testing Programs\Field\Ground Water Monitoring Reports\Phase1A\2020-0409-BB-BWS-102 - Well Monitoring Report.xlsx
Form 2021

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-BWS-104(OW)
			Boring No. BB-BWS-104
PROJECT	Route 9/I-395 Connector		H&A FILE NO.
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.
CLIENT	Maine Department of Transportation		FIELD REP.
CONTRACTOR	Northern Test Borings Inc.		DATE INSTALLED
DRILLER	M. Nadeau		WATER LEVEL
			132076-002
			B. Steinert
			N. Klausmeyer
			7/11/2018
			0.0 ft. (at ground surface)

Ground El.	100.9	ft	Location	SEE PLAN	<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box
El. Datum	NAVD 88				

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL																			
Clayey SILT -FILL- 4.0 ft	-BENTONITE- 2.0 ft	Type of protective cover																		
		Locking Cap																		
		Height of top of guard pipe above ground surface																		
		2.6 ft																		
-GLACIAL TILL- 36.8 ft	-FILTER SAND- 48.0	Height of top of riser pipe above ground surface																		
		2.3 ft																		
		Type of protective casing:																		
		Steel Guard Pipe																		
		Length																		
		5.0 ft																		
		Inside Diameter																		
		4.0 in																		
		Depth of bottom of guard pipe																		
		2.3 ft																		
		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th style="width: 30%;">Type of Seals</th> <th style="width: 20%;">Top of Seal (ft)</th> <th style="width: 20%;">Thickness (ft)</th> </tr> <tr> <td>Concrete</td> <td style="text-align: center;">--</td> <td></td> </tr> <tr> <td>Bentonite Seal</td> <td style="text-align: center;">0.0</td> <td style="text-align: center;">2.0</td> </tr> <tr> <td> </td> <td></td> <td></td> </tr> <tr> <td> </td> <td></td> <td></td> </tr> </table>				Type of Seals	Top of Seal (ft)	Thickness (ft)	Concrete	--		Bentonite Seal	0.0	2.0						
Type of Seals	Top of Seal (ft)	Thickness (ft)																		
Concrete	--																			
Bentonite Seal	0.0	2.0																		
		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td colspan="2" style="text-align: right;">Type of riser pipe:</td> <td style="text-align: right;">Sch. 40 PVC</td> </tr> <tr> <td colspan="2" style="text-align: right;">Inside diameter of riser pipe</td> <td style="text-align: right;">2.0 in</td> </tr> <tr> <td colspan="2" style="text-align: right;">Type of backfill around riser</td> <td style="text-align: right;">Bentonite/Sand</td> </tr> </table>				Type of riser pipe:		Sch. 40 PVC	Inside diameter of riser pipe		2.0 in	Type of backfill around riser		Bentonite/Sand						
Type of riser pipe:		Sch. 40 PVC																		
Inside diameter of riser pipe		2.0 in																		
Type of backfill around riser		Bentonite/Sand																		
		Diameter of borehole																		
		4.0 in																		
		Depth to top of well screen																		
		2.7 ft																		
		Type of screen																		
		Sch. 40 PVC																		
		Screen gauge or size of openings																		
		0.010 in																		
		Diameter of screen																		
		2.0 in																		
		Type of backfill around screen																		
		No. 2 Filter Sand																		
		Depth of bottom of well screen																		
		17.7 ft																		
		Bottom of Silt trap																		
		18.0 ft																		
		Depth of bottom of borehole																		
		18.0 ft																		

(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)	
5.0 ft	+	15.0 ft	+
Riser Pay Length (L1)		Length of screen (L2)	Length of silt trap (L3)
0.3 ft		=	
20.3 ft		Pay length	

COMMENTS:

GROUNDWATER MONITORING REPORT

OW/PZ NUMBER
BB-BWS-104(OW)
Page 1 of 1

PROJECT	Route 9/I-395 Connector	H&A FILE NO.	132076-005
LOCATION	Brewer, Maine / Eddington, Maine	PROJECT MGR.	B. Steinert
CLIENT	Maine Department of Transportation	FIELD REP.	Klausmeyer/Hollauer/Fletcher
CONTRACTOR	Northern Test Borings Inc.	DATE	4/9/2020
ELEVATION OF REFERENCE POINT (ft)	100.9	REFERENCE POINT:	Ground Surface
			<input checked="" type="checkbox"/> PVC <input type="checkbox"/> Other

[illegible]

COMMENTS: _____



BB-ECR-102

Page 1 of 1

H&A FILE NO. 132076-002

PROJECT MGR.	B. Steinert
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FIELD REP. N. Klausmeyer

DATE 4/20/2020

REFERENCE POINT: Ground Surface ☒ PVC ☐ Other ☐

[illegible]

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-EEBT2-102(OW)															
			Boring No. BB-EEBT2-102															
PROJECT LOCATION CLIENT CONTRACTOR DRILLER	Route 9/I-395 Connector Brewer, Maine/Eddington, Maine Maine Department of Transportation New England Boring Contractors B. Enos		H&A FILE NO. 132076-002 PROJECT MGR. B. Steinert FIELD REP. N. Klausmeyer DATE INSTALLED 12/3/2018 WATER LEVEL 1.4 ft (depth below gs)															
Ground El. 181.3 ft El. Datum NAVD 88	Location <u>SEE PLAN</u>	<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box																
SOIL/ROCK CONDITIONS -TOPSOIL- 0.1 ft -MARINE DEPOSIT- 12.3 ft -GLACIAL TILL- 17.2 ft -BEDROCK- 17.5 ft	BOREHOLE BACKFILL -FILTER SAND- 3.0 ft -BENTONITE- 4.0 ft -FILTER SAND- 17.5 ft	<div style="display: flex; align-items: center;"> <div style="margin-left: 10px;"> <p>Type of protective cover <u>Locking Cap</u></p> <p>Height of top of guard pipe above ground surface <u>3.0</u> ft</p> <p>Height of top of riser pipe above ground surface <u>3.0</u> ft</p> <p>Type of protective casing: <u>Steel Guard Pipe</u></p> <p>Length <u>5.3</u> ft</p> <p>Inside Diameter <u>3.0</u> in</p> <p>Depth of bottom of guard pipe <u>2.3</u> ft</p> <table border="1" style="width: 100%; border-collapse: collapse; margin-top: 10px;"> <thead> <tr> <th>Type of Seals</th> <th>Top of Seal (ft)</th> <th>Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Bentonite Seal</td> <td>3.0</td> <td>1.0</td> </tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table> <p>Type of riser pipe: <u>Sch. 40 PVC</u></p> <p>Inside diameter of riser pipe <u>2.0</u> in</p> <p>Type of backfill around riser <u>Hollister Filter Sand</u></p> <p>Diameter of borehole <u>4.0</u> in</p> <p>Depth to top of well screen <u>4.8</u> ft</p> <p>Type of screen <u>Sch. 40 PVC</u></p> <p>Screen gauge or size of openings <u>0.010</u> in</p> <p>Diameter of screen <u>2.0</u> in</p> <p>Type of backfill around screen <u>Hollister Filter Sand</u></p> <p>Depth of bottom of well screen <u>14.8</u> ft</p> <p>Bottom of Silt trap <u>15.0</u> ft</p> <p>Depth of bottom of borehole <u>17.5</u> ft</p> </div> </div>		Type of Seals	Top of Seal (ft)	Thickness (ft)	Bentonite Seal	3.0	1.0									
Type of Seals	Top of Seal (ft)	Thickness (ft)																
Bentonite Seal	3.0	1.0																
(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)																
<div style="display: flex; justify-content: space-between; align-items: center;"> <div> <u>7.8</u> ft + <u>10.0</u> ft + <u>0.2</u> ft = <u>18.0</u> ft Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length </div> </div>																		
COMMENTS:																		



BB-EEBT2-102

Page 1 of 1

H&A FILE NO.	132076-002
PROJECT MGR.	B. Steinert
FIELD REP.	H. Hollauer/N. Klausmeyer
DATE	4/20/2020

ELEVATION OF REFERENCE POINT (ft)	181.3	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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G:\PROJECTS\132076 - brewer eddington\002 - Exploration + Laboratory Testing Programs\Field\Ground Water Monitoring Reports\Phase I\BB-EEBT2-102 - Well Monitoring Report.xlsx
Form 2021

COMMENTS: _____



BB-ELER-101

Page 1 of 1

H&A FILE NO. 132076-002

PROJECT MGR.	B. Steinert
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FIELD REP. N. Klausmeyer

DATE 4/20/2020

ELEVATION OF REFERENCE POINT (ft)	225.0	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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Form 2021

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. BB-EWC-101(OW)	
			Boring No. BB-EWC-101	
PROJECT	Route 9/I-395 Connector		H&A FILE NO.	132076-002
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.	B. Steinert
CLIENT	Maine Department of Transportation		FIELD REP.	H. Hollauer
CONTRACTOR	New England Boring Contractors		DATE INSTALLED	11/28/2018
DRILLER	M. Porter		WATER LEVEL	0.3 ft. (depth below gs)
Ground El.	191.2 ft	Location	SEE PLAN	
El. Datum	NAVD 88			<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL															
-TOPSOIL-		Type of protective cover														
0.1 ft	-FILTER SAND-	Locking Cap														
	2.3 ft	Height of top of guard pipe above ground surface														
-MARINE DEPOSIT-	-BENTONITE-	3.3 ft														
	3.4 ft	Height of top of riser pipe above ground surface														
3.8 ft		3.2 ft														
	-FILTER SAND-	Type of protective casing:														
-GLACIAL TILL-		Steel Guard Pipe														
	9.5 ft	Length														
		5.3 ft														
	10.0 ft	Inside Diameter														
		3.0 in														
	-BENTONITE-	Depth of bottom of guard pipe														
9.6 ft		2.0 ft														
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Type of Seals</th> <th>Top of Seal (ft)</th> <th>Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Bentonite Seal</td> <td>2.3</td> <td>1.1</td> </tr> <tr> <td>Bentonite Seal</td> <td>9.5</td> <td>0.5</td> </tr> <tr> <td>Bentonite Seal</td> <td>17.5</td> <td>6.7</td> </tr> </tbody> </table>			Type of Seals	Top of Seal (ft)	Thickness (ft)	Bentonite Seal	2.3	1.1	Bentonite Seal	9.5	0.5	Bentonite Seal	17.5	6.7
Type of Seals	Top of Seal (ft)	Thickness (ft)														
Bentonite Seal	2.3	1.1														
Bentonite Seal	9.5	0.5														
Bentonite Seal	17.5	6.7														
	-BEDROCK-	Type of riser pipe:														
		Sch. 40 PVC														
		Inside diameter of riser pipe														
		2.0 in														
		Type of backfill around riser														
		Holliston Filter Sand														
		Diameter of borehole														
		4.0 in														
		Depth to top of well screen														
		4.3 ft														
		Type of screen														
		Sch. 40 PVC														
		Screen gauge or size of openings														
		0.010 in														
		Diameter of screen														
		2.0 in														
		Type of backfill around screen														
		Holliston Filter Sand														
		Depth of bottom of well screen														
		9.3 ft														
		Bottom of Silt trap														
		9.5 ft														
		Depth of bottom of borehole														
		24.2 ft														
24.2 (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)														

7.5 ft Riser Pay Length (L1)	+ 5.0 ft Length of screen (L2)	+ 0.2 ft Length of silt trap (L3)	= 12.7 ft Pay length
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COMMENTS:



BB-EWC-101

Page 1 of 1

ELEVATION OF REFERENCE POINT (ft)	191.2	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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[illegible]

		<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. HB-BE-101(OW) <hr/> Boring No. HB-BE-101																
PROJECT	Route 9/I-395 Connector	H&A FILE NO.	132076-002																	
LOCATION	Brewer, Maine/Eddington, Maine	PROJECT MGR.	B. Steinert																	
CLIENT	Maine Department of Transportation	FIELD REP.	N. Klausmeyer																	
CONTRACTOR	Northern Test Borings Inc.	DATE INSTALLED	7/7/2018																	
DRILLER	M. Nadeau	WATER LEVEL	--																	
Ground El. <u>82.0</u> ft El. Datum <u>NAVD 88</u>		Location <u>SEE PLAN</u>		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box																
SOIL/ROCK CONDITIONS Clayey SILT -FILL- 2.0 ft -MARINE DEPOSIT- 37.0 ft -GLACIAL TILL- 42.6 ft -BEDROCK- 12.2 (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)	BOREHOLE BACKFILL -BENTONITE- 1.8 ft -FILTER SAND- 12.2	<div style="display: flex; align-items: center;"> <div style="margin-left: 20px;"> <p>Type of protective cover <u>Locking Cap</u></p> <p>Height of top of guard pipe above ground surface <u>2.1</u> ft</p> <p>Height of top of riser pipe above ground surface <u>1.5</u> ft</p> <p>Type of protective casing: <u>Steel Guard Pipe</u></p> <p>Length <u>5.0</u> ft</p> <p>Inside Diameter <u>4.0</u> in</p> <p>Depth of bottom of guard pipe <u>2.9</u> ft</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Type of Seals</th> <th>Top of Seal (ft)</th> <th>Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Concrete</td> <td>--</td> <td>--</td> </tr> <tr> <td>Bentonite Seal</td> <td>0.0</td> <td>1.8</td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table> <p>Type of riser pipe: <u>Sch. 40 PVC</u></p> <p>Inside diameter of riser pipe <u>2.0</u> in</p> <p>Type of backfill around riser <u>Filter Sand</u></p> <p>Diameter of borehole <u>4.0</u> in</p> <p>Depth to top of well screen <u>2.0</u> ft</p> <p>Type of screen <u>Sch. 40 PVC</u></p> <p>Screen gauge or size of openings <u>0.010</u> in</p> <p>Diameter of screen <u>2.0</u> in</p> <p>Type of backfill around screen <u>Filter Sand</u></p> <p>Depth of bottom of well screen <u>12.0</u> ft</p> <p>Bottom of Silt trap <u>12.2</u> ft</p> <p>Depth of bottom of borehole <u>12.2</u> ft</p> </div> </div>				Type of Seals	Top of Seal (ft)	Thickness (ft)	Concrete	--	--	Bentonite Seal	0.0	1.8						
Type of Seals	Top of Seal (ft)	Thickness (ft)																		
Concrete	--	--																		
Bentonite Seal	0.0	1.8																		
<div style="display: flex; justify-content: space-between; align-items: center;"> <div> <u>3.5</u> ft + <u>10.0</u> ft + <u>0.2</u> ft = <u>13.7</u> ft Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length </div> <div>(Not to Scale)</div> </div>																				
COMMENTS: _____																				



HB-BF-101

Page 1 of 1

DATE 8/10/2018

REFERENCE POINT: ☐ Ground Surface ☒ PVC ☐ Other ☐

	<h2 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h2>		Well No. HB-BE-103(OW)
			Boring No. HB-BE-103
PROJECT LOCATION CLIENT CONTRACTOR DRILLER	Route 9/I-395 Connector Brewer, Maine/Eddington, Maine Maine Department of Transportation New England Boring Contractors M. Porter		H&A FILE NO. PROJECT MGR. FIELD REP. DATE INSTALLED WATER LEVEL
		132076-002 B. Steinert H. Hollauer 10/25/2018 1.4 ft. (depth below gs)	
Ground El. El. Datum	81.2 ft NAVD 88	Location	SEE PLAN
		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box	

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL			
-TOPSOIL- 0.5 ft	-FILTER SAND- 2.7 ft	-MARINE DEPOSIT- 6.3 ft	-GLACIAL TILL- 28.7 ft	-BEDROCK- 35.0 ft
36.5 (Bottom of Exploration) <small>(Numbers refer to depth from ground surface in feet)</small>	36.5	(Not to Scale)		

Type of protective cover Locking Cap	Height of top of guard pipe above ground surface 2.6 ft	Height of top of riser pipe above ground surface 2.4 ft															
Type of protective casing: Length Inside Diameter	Steel Guard Pipe 5.3 ft 3.0 in																
Depth of bottom of guard pipe	2.7 ft																
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="text-align: left;">Type of Seals</th> <th style="text-align: left;">Top of Seal (ft)</th> <th style="text-align: left;">Thickness (ft)</th> </tr> </thead> <tbody> <tr> <td>Bentonite Seal</td> <td>2.7</td> <td>3.6</td> </tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table>	Type of Seals	Top of Seal (ft)	Thickness (ft)	Bentonite Seal	2.7	3.6											
Type of Seals	Top of Seal (ft)	Thickness (ft)															
Bentonite Seal	2.7	3.6															
Type of riser pipe: Inside diameter of riser pipe Type of backfill around riser	Sch. 40 PVC 2.0 in Filter Sand																
Diameter of borehole Depth to top of well screen	4.0 in 10.0 ft																
Type of screen Screen gauge or size of openings Diameter of screen	Sch. 40 PVC 0.010 in 2.0 in																
Type of backfill around screen Depth of bottom of well screen	Filter Sand #25 35.0 ft																
Bottom of Silt trap Depth of bottom of borehole	35.0 ft 36.5 ft																

12.4 ft + 25.0 ft + 0.0 ft = 37.4 ft Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length
--

COMMENTS:



HB-BE-103

PROJECT	Route 9/I-395 Connector
LOCATION	Brewer, Maine / Eddington, Maine
CLIENT	Maine Department of Transportation
CONTRACTOR	New England Boring Contractors

H&A FILE NO.	<u>132076-002</u>
PROJECT MGR.	<u>B. Steinert</u>
FIELD REP.	<u>H. Hollauer/N. Klausmeyer</u>
DATE	<u>4/20/2020</u>

ELEVATION OF REFERENCE POINT (ft)	81.2	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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[illegible]

		<h1 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h1>		Well No. HB-BE-108(OW)	
				Boring No. HB-BE-108	
PROJECT	Route 9/I-395 Connector		H&A FILE NO.	132076-002	
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.	B. Steinert	
CLIENT	Maine Department of Transportation		FIELD REP.	H. Hollauer	
CONTRACTOR	New England Boring Contractors		DATE INSTALLED	10/19/2018	
DRILLER	M. Porter		WATER LEVEL	0.9 ft. (depth below gs)	
Ground El.	81.8 ft	Location	SEE PLAN		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box
El. Datum	NAVD 88				

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL				
-TOPSOIL-	-FILTER SAND-	Type of protective cover <u>Locking Cap</u>			
0.5 ft	2.5 ft	Height of top of guard pipe above ground surface		2.8 ft	
	-BENTONITE-	Height of top of riser pipe above ground surface		2.6 ft	
	3.5 ft	Type of protective casing:		Steel Guardpipe	
	-FILTER SAND-	Length		5.0 ft	
		Inside Diameter		4.0 in	
-MARINE DEPOSIT-	6.0 ft	Depth of bottom of guard pipe		2.2 ft	
	-BENTONITE-	Type of Seals		Top of Seal (ft)	Thickness (ft)
	8.0 ft	Bentonite Seal		2.5	1.0
		Bentonite Seal		6.0	2.0
17.2 ft		Type of riser pipe:		Sch. 40 PVC	
		Inside diameter of riser pipe		2.0 in	
		Type of backfill around riser		Filter Sand #25	
	-FILTER SAND-	Diameter of borehole		6.0 in	
		Depth to top of well screen		10.0 ft	
		Type of screen		Sch. 40 PVC	
		Screen gauge or size of openings		0.010 in	
		Diameter of screen		2.0 in	
		Type of backfill around screen		Filter Sand #25	
		Depth of bottom of well screen		20.0 ft	
		Bottom of Silt trap		20.0 ft	
		Depth of bottom of borehole		23.5 ft	
23.5	23.5	(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)			

12.8 ft + 10.0 ft + 0.0 ft = 22.8 ft Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length	
--	--

COMMENTS: Sand below wellscreen 20-23.5 ft. 4.5 ft sand. ~1/4 bag bentonite.



HB-BF-108

Page 1 of 1

H&A FILE NO. 132076-002

PROJECT MGR. B. Steinert

FIELD REP. H. Hollauer/N. Klausmeyer

DATE 4/20/2020

ELEVATION OF REFERENCE POINT (ft)	81.8
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REFERENCE POINT: Ground Surface ☒ PVC ☐ Other ☐

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Form 2021

		<h1 style="margin: 0;">OBSERVATION WELL INSTALLATION REPORT</h1>		Well No. HB-BE-111(OW)	
				Boring No. HB-BE-111	
PROJECT	Route 9/I-395 Connector	H&A FILE NO.	132076-002		
LOCATION	Brewer, Maine/Eddington, Maine	PROJECT MGR.	B. Steinert		
CLIENT	Maine Department of Transportation	FIELD REP.	H. Hollauer		
CONTRACTOR	New England Boring Contractors	DATE INSTALLED	11/2/2018		
DRILLER	M. Porter	WATER LEVEL	5.1 ft. (depth below gs)		
Ground El.	91.4 ft	Location	SEE PLAN		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box
El. Datum	NAVD 88				

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL		Type of protective cover	Locking Cap		
-TOPSOIL- 0.2 ft					2.8 ft	
	-FILTER SAND-					
					2.7 ft	
	5.0 ft					
-MARINE DEPOSIT-			Type of protective casing:	Steel Guard Pipe		
			Length	5.0	ft	
	-BENTONITE-		Inside Diameter	3.0	in	
	8.0 ft		Depth of bottom of guard pipe	2.3	ft	
12.2 ft						
			Type of Seals	Top of Seal (ft)	Thickness (ft)	
			Bentonite Seal	5.0	3.0	
-GLACIAL TILL-						
15.0 ft			Type of riser pipe:	Sch. 40 PVC		
			Inside diameter of riser pipe	2.0	in	
-BOULDER/COBBLE-			Type of backfill around riser	Filter Sand		
16.2 ft						
	-FILTER SAND-		Diameter of borehole	4.0	in	
			Depth to top of well screen	9.5	ft	
			Type of screen	Sch. 40 PVC		
			Screen gauge or size of openings	0.010	in	
			Diameter of screen	2.0	in	
			Type of backfill around screen	Filter Sand #25		
			Depth of bottom of well screen	19.5	ft	
			Bottom of Silt trap	20.0	ft	
			Depth of bottom of borehole	20.0	ft	
20.0	20.0					
(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)		(Not to Scale)				

12.2 ft + 10.0 ft + 0.5 ft = 22.7 ft Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length
--

COMMENTS:



HB-BF-111

Page 1 of 1

H&A FILE NO.	132076-002
PROJECT MGR.	B. Steinert
FIELD REP.	H. Hollauer/N. Klausmeyer
DATE	4/20/2020

ELEVATION OF REFERENCE POINT (ft)	91.4	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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\\haleyaldrich.com\share\por_common\PROJECTS\132076 - brewer eddington\002 - Exploration + Laboratory Testing Programs\Field\Ground Water Monitoring Reports\Phase I\B\HB-BE-111 - Well Monitoring Report.xlsx
Form 2021

	OBSERVATION WELL INSTALLATION REPORT		Well No. HB-BE-115(OW)
			Boring No. HB-BE-115
PROJECT	Route 9/I-395 Connector		H&A FILE NO.
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.
CLIENT	Maine Department of Transportation		FIELD REP.
CONTRACTOR	Northern Test Borings Inc.		DATE INSTALLED
DRILLER	M. Nadeau		WATER LEVEL
Ground El.	126.2 ft	Location	SEE PLAN
El. Datum	NAVD 88		
		<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box	
SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL		
-TOPSOIL- 0.3 ft	-FILTER SAND- 3.0 ft		
	-BENTONITE- 4.0 ft		
-GLACIOFLUVIAL DEPOSIT-			
9.5 ft	-FILTER SAND-		
-GLACIAL TILL-			
22.0	22.0		
(Bottom of Exploration) (Numbers refer to depth from ground surface in feet)			
		<div style="display: flex; justify-content: space-between;"> 7.3 ft 15.0 ft 0.2 ft 22.5 ft </div> <div style="display: flex; justify-content: space-between;"> Riser Pay Length (L1) Length of screen (L2) Length of silt trap (L3) Pay length </div>	
COMMENTS:			

GROUNDWATER MONITORING REPORT

OW/PZ NUMBER

HB-BE-115

Page 1 of 1

PROJECT	Route 9/I-395 Connector
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LOCATION	Brewer, Maine / Eddington, Maine
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CLIENT	Maine Department of Transportation
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CONTRACTOR	Northern Test Borings Inc.
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H&A FILE NO. 132076-002

PROJECT MGR.	B. Steinert
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FIELD REP. N. Klausmeyer

DATE 4/20/2020

ELEVATION OF REFERENCE POINT (ft)	126.2
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REFERENCE POINT: Ground Surface ☒ PVC ☐ Other ☐

[illegible]



HB-BE-120

Page 1 of 1

PROJECT	Route 9/I-395 Connector
LOCATION	Brewer, Maine / Eddington, Maine
CLIENT	Maine Department of Transportation
CONTRACTOR	New England Boring Contractors

H&A FILE NO.	<u>132076-002</u>
PROJECT MGR.	<u>B. Steinert</u>
FIELD REP.	<u>H. Hollauer/N. Klausmeyer</u>
DATE	<u>4/20/2020</u>

ELEVATION OF REFERENCE POINT (ft)	132.8	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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[illegible]

HALEY ALDRICH	OBSERVATION WELL INSTALLATION REPORT		Well No. HB-BE-132(OW)
			Boring No. HB-BE-132
PROJECT	Route 9/I-395 Connector		H&A FILE NO.
LOCATION	Brewer, Maine/Eddington, Maine		PROJECT MGR.
CLIENT	Maine Department of Transportation		FIELD REP.
CONTRACTOR	New England Boring Contractors		DATE INSTALLED
DRILLER	M. Porter		WATER LEVEL
Ground El.		172.7 ft	<input checked="" type="checkbox"/> Guard Pipe <input type="checkbox"/> Roadway Box
El. Datum		NAVD 88	
Location		SEE PLAN	

SOIL/ROCK CONDITIONS	BOREHOLE BACKFILL		Type of protective cover		Locking Cap										
-TOPSOIL- 0.2 ft	-FILTER SAND- 3.0 ft -BENTONITE- 5.0 ft	L1	Height of top of guard pipe above ground surface	2.4	ft										
-GLACIAL TILL-	-FILTER SAND- 23.0 ft -BENTONITE- 26.0 ft		Height of top of riser pipe above ground surface	2.3	ft										
			Type of protective casing: Length Inside Diameter	Steel Guard Pipe	5.0	ft									
			Depth of bottom of guard pipe	2.6	ft										
			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th style="text-align: left;">Type of Seals</th> <th style="text-align: center;">Top of Seal (ft)</th> <th style="text-align: center;">Thickness (ft)</th> </tr> <tr> <td>Concrete</td> <td style="text-align: center;">--</td> <td style="text-align: center;">--</td> </tr> <tr> <td>Bentonite Seal</td> <td style="text-align: center;">3.0</td> <td style="text-align: center;">2.0</td> </tr> <tr> <td>Bentonite Seal</td> <td style="text-align: center;">23.0</td> <td style="text-align: center;">3.0</td> </tr> </table>	Type of Seals	Top of Seal (ft)	Thickness (ft)	Concrete	--	--	Bentonite Seal	3.0	2.0	Bentonite Seal	23.0	3.0
Type of Seals	Top of Seal (ft)	Thickness (ft)													
Concrete	--	--													
Bentonite Seal	3.0	2.0													
Bentonite Seal	23.0	3.0													
		Type of riser pipe: Inside diameter of riser pipe Type of backfill around riser	Sch. 40 PVC	2.0	in										
		Diameter of borehole	4.0	in											
		Depth to top of well screen	28.0	ft											
		Type of screen Screen gauge or size of openings Diameter of screen	Sch. 40 PVC	0.010	in										
		Type of backfill around screen	Filter Sand #25												
		Depth of bottom of well screen	48.0	ft											
		Bottom of Silt trap	0.5	ft											
		Depth of bottom of borehole	48.5	ft											
48.5 (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)			(Not to Scale)												

30.3 ft	+	20.0 ft	+	0.5 ft	=	50.8 ft
Riser Pay Length (L1)		Length of screen (L2)		Length of silt trap (L3)		Pay length

COMMENTS:

GROUNDWATER MONITORING REPORT

OW/PZ NUMBER

HB-BE-135

Page 1 of 1

PROJECT	Route 9/I-395 Connector
LOCATION	Brewer, Maine / Eddington, Maine
CLIENT	Maine Department of Transportation
CONTRACTOR	New England Boring Contractors

H&A FILE NO.	132076-002
PROJECT MGR.	B. Steinert
FIELD REP.	H. Hollauer/N. Klausmeyer
DATE	4/20/2020

ELEVATION OF REFERENCE POINT (ft)	93.0	REFERENCE POINT:	Ground Surface	<input checked="" type="checkbox"/>	PVC	<input type="checkbox"/>	Other	<input type="checkbox"/>
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[illegible]



HB-BE-164

PROJECT	Route 9/I-395 Connector
LOCATION	Brewer, Maine / Eddington, Maine
CLIENT	Maine Department of Transportation
CONTRACTOR	New England Boring Contractors

H&A FILE NO.	<u>132076-002</u>
PROJECT MGR.	<u>B. Steinert</u>
FIELD REP.	<u>H. Hollauer/N. Klausmeyer</u>
DATE	<u>4/20/2020</u>

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Form 2021

APPENDIX D

CPT Report

PRESENTATION OF SITE INVESTIGATION RESULTS

I-395 & Route 9 Connector Brewer & Eddington, Maine

Prepared for:

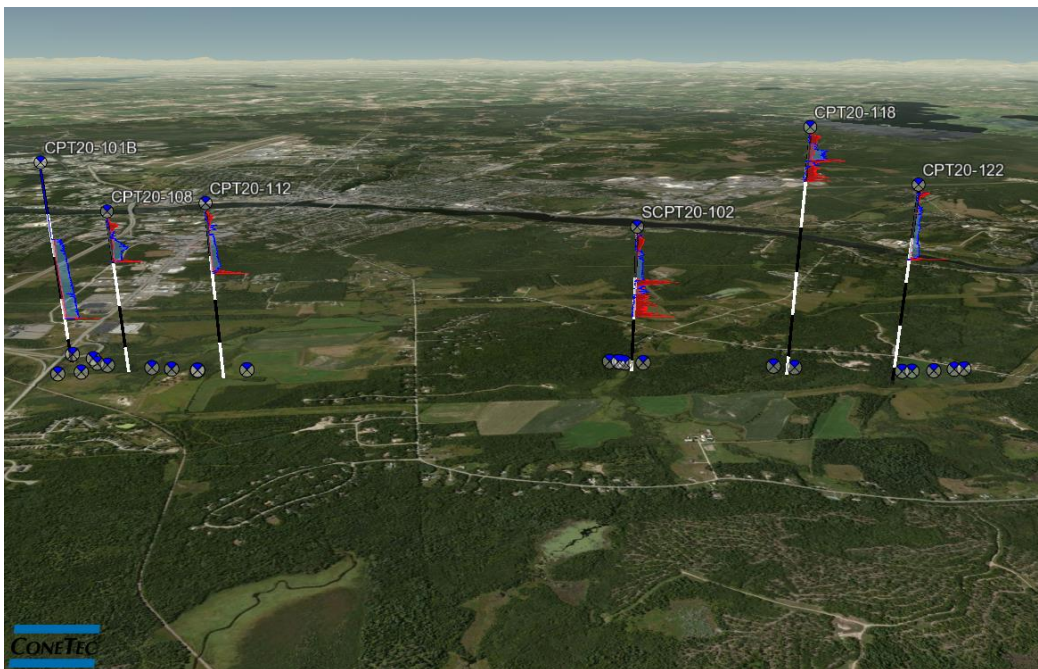
Haley & Aldrich

ConeTec Job No: 20-53-21525

Project Start Date: 26-Oct-2020

Project End Date: 2-Nov-2020

Report Date: 20-Nov-2020



Prepared by:

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www.conetec.com
www.conetecdataservices.com



Introduction

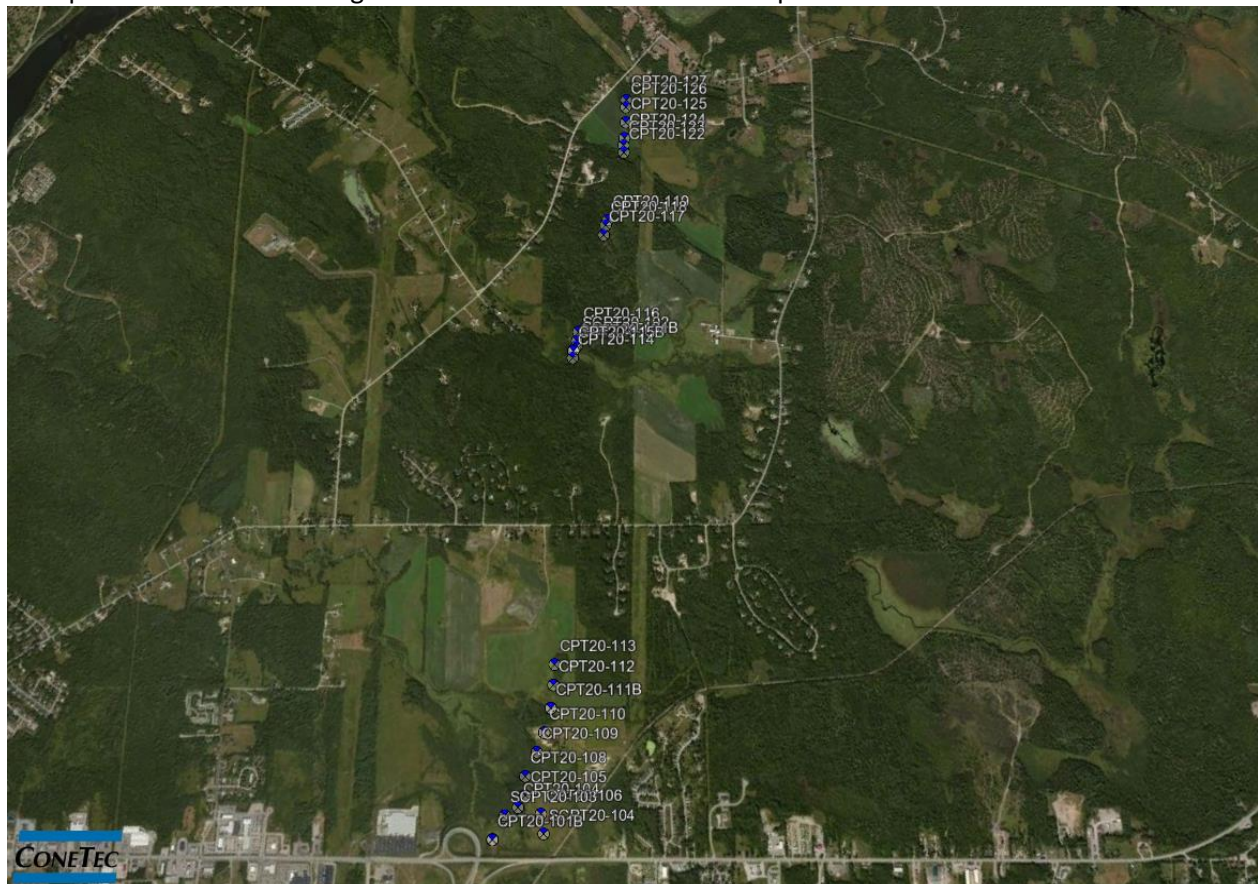
The enclosed report presents the results of a piezocone penetration testing (CPTu or CPT) and seismic piezocone penetration testing (SCPTu or SCPT) program carried out for the I-395 and Route 9 Connector located in Brewer and Eddington, Maine. The site investigation program was conducted by ConeTec Inc. (ConeTec), under contract to Haley & Aldrich of Portland, Maine.

A total of 25 cone penetration tests and 5 seismic cone penetration tests were completed at 26 locations (several locations were offset or drilled out and attempted again due to shallow refusal). The CPT and SCPT program was performed to evaluate the subsurface soil conditions. CPT and SCPT sounding locations were selected and numbered under supervision of Haley & Aldrich personnel (Dave Dearden).

Project Information

Project	
Client	Haley & Aldrich
Project	I-395 & Route 9 Connector, Brewer & Eddington, ME
ConeTec project number	20-53-21525

A map from CESIUM including the CPT and SCPT test locations is presented below.



Rig Description	Deployment System	Test Type
Mobile B51	Track mounted drill rig (single cylinder)	CPT and SCPT

Coordinates		
Test Type	Collection Method	EPSG Number
CPT and SCPT	GPS (Handheld)	WGS 84 / Lat and Long (Decimal Degrees)

Cone Penetration Test (CPT)	
Depth reference	Ground surface at the time of the investigation.
Tip and sleeve data offset	0.1 meter. This has been accounted for in the CPT data files.
Pore pressure dissipation (PPD) tests	Eight pore pressure dissipation tests were completed primarily to determine the phreatic surface and consolidation characteristics.
Additional plots	Advanced, Seismic and Soil Behavior Type (SBT) scatter plots are included in the data release package.

Cone Penetrometers Used for this Project						
Cone Description	Cone Number	Cross Sectional Area (cm ²)	Sleeve Area (cm ²)	Tip Capacity (bar)	Sleeve Capacity (bar)	Pore Pressure Capacity (psi)
524:T375F10U500	524	15	225	375	10	500
Cone 524 was used for each sounding.						

Calculated Geotechnical Parameters Tables	
Additional information	<p>The Normalized Soil Behavior Type Chart based on Q_{tn} (SBT Q_{tn}) (Robertson, 2009) was used to classify the soil for this project. A detailed set of calculated CPT parameters have been generated and are provided in Excel format files in the release folder. The CPT parameter calculations are based on values of corrected tip resistance (q_t) sleeve friction (f_s) and pore pressure (u_2).</p> <p>Effective stresses are calculated based on unit weights that have been assigned to the individual soil behavior type zones and the assumed equilibrium pore pressure profile.</p> <p>Soils were classified as either drained or undrained based on the Q_{tn} Normalized Soil Behavior Type Chart (Robertson, 2009). Calculations for both drained and undrained parameters were included for materials that classified as silt mixtures (zone 4).</p>

Limitations

This report has been prepared for the exclusive use of Haley & Aldrich (Client) for the project titled "I-395 & Route 9 Connector, Brewer & Eddington, ME". The report's contents may not be relied upon by any other party without the express written permission of ConeTec. ConeTec has provided site investigation services, prepared the factual data reporting and provided geotechnical parameter calculations consistent with current best practices. No other warranty, expressed or implied, is made.

The information presented in the report document and the accompanying data set pertain to the specific project, site conditions and objectives described to ConeTec by the Client. In order to properly understand the factual data, assumptions and calculations, reference must be made to the documents provided and their accompanying data sets, in their entirety.

Cone penetration tests (CPTu) are conducted using an integrated electronic piezocone penetrometer and data acquisition system manufactured by Adara Systems Ltd., a subsidiary of ConeTec.

ConeTec's piezocone penetrometers are compression type designs in which the tip and friction sleeve load cells are independent and have separate load capacities. The piezocones use strain gauged load cells for tip and sleeve friction and a strain gauged diaphragm type transducer for recording pore pressure. The piezocones also have a platinum resistive temperature device (RTD) for monitoring the temperature of the sensors, an accelerometer type dual axis inclinometer and a geophone sensor for recording seismic signals. All signals are amplified down hole within the cone body and the analog signals are sent to the surface through a shielded cable.

ConeTec penetrometers are manufactured with various tip, friction and pore pressure capacities in both 10 cm² and 15 cm² tip base area configurations in order to maximize signal resolution for various soil conditions. The specific piezocone used for each test is described in the CPT summary table presented in the first appendix. The 15 cm² penetrometers do not require friction reducers as they have a diameter larger than the deployment rods. The 10 cm² piezocones use a friction reducer consisting of a rod adapter extension behind the main cone body with an enlarged cross sectional area (typically 44 mm diameter over a length of 32 mm with tapered leading and trailing edges) located at a distance of 585 mm above the cone tip.

The penetrometers are designed with equal end area friction sleeves, a net end area ratio of 0.8 and cone tips with a 60 degree apex angle.

All ConeTec piezocones can record pore pressure at various locations. Unless otherwise noted, the pore pressure filter is located directly behind the cone tip in the "u₂" position (ASTM Type 2). The filter is 6 mm thick, made of porous plastic (polyethylene) having an average pore size of 125 microns (90-160 microns). The function of the filter is to allow rapid movements of extremely small volumes of water needed to activate the pressure transducer while preventing soil ingress or blockage.

The piezocone penetrometers are manufactured with dimensions, tolerances and sensor characteristics that are in general accordance with the current ASTM D5778 standard. ConeTec's calibration criteria also meet or exceed those of the current ASTM D5778 standard. An illustration of the piezocone penetrometer is presented in Figure CPTu.

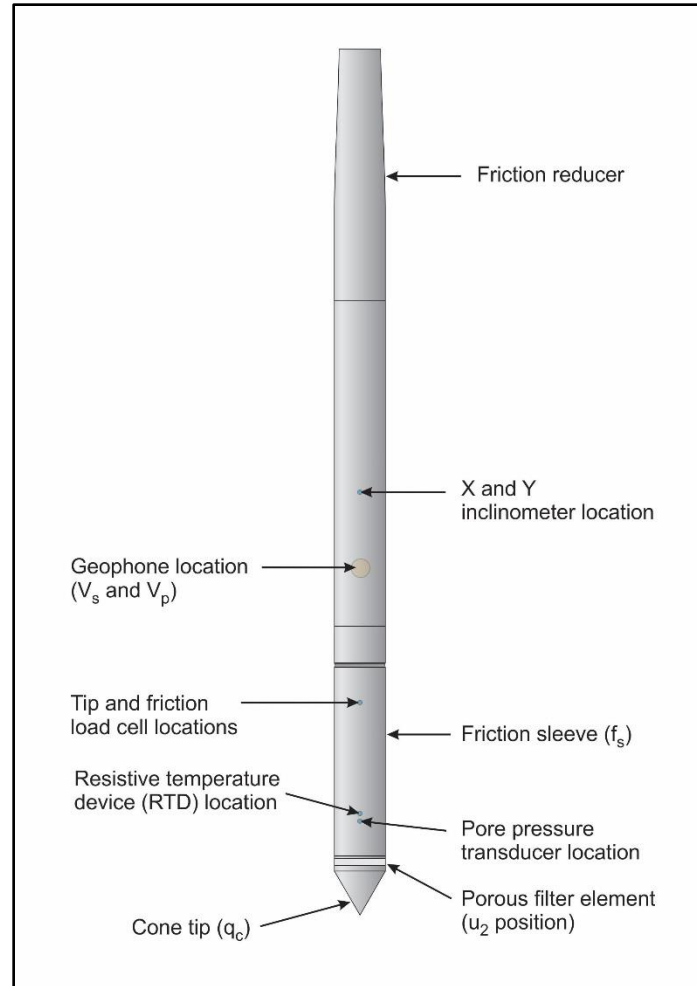


Figure CPTu. Piezocone Penetrometer (15 cm²)

The ConeTec data acquisition systems consist of a Windows based computer and a signal conditioner and power supply interface box with a 16 bit (or greater) analog to digital (A/D) converter. The data is recorded at fixed depth increments using a depth wheel attached to the push cylinders or by using a spring loaded rubber depth wheel that is held against the cone rods. The typical recording interval is 2.5 cm; custom recording intervals are possible. The system displays the CPTu data in real time and records the following parameters to a storage media during penetration:

- Depth
- Uncorrected tip resistance (q_c)
- Sleeve friction (f_s)
- Dynamic pore pressure (u)
- Additional sensors such as resistivity, passive gamma, ultra violet induced fluorescence, if applicable

All testing is performed in accordance to ConeTec's CPT operating procedures which are in general accordance with the current ASTM D5778 standard.

Prior to the start of a CPTu sounding a suitable cone is selected, the cone and data acquisition system are powered on, the pore pressure system is saturated with either glycerin or silicone oil and the baseline readings are recorded with the cone hanging freely in a vertical position.

The CPTu is conducted at a steady rate of 2 cm/s, within acceptable tolerances. Typically one meter length rods with an outer diameter of 1.5 inches are added to advance the cone to the sounding termination depth. After cone retraction final baselines are recorded.

Additional information pertaining to ConeTec's cone penetration testing procedures:

- Each filter is saturated in silicone oil under vacuum pressure prior to use
- Recorded baselines are checked with an independent multi-meter
- Baseline readings are compared to previous readings
- Soundings are terminated at the client's target depth or at a depth where an obstruction is encountered, excessive rod flex occurs, excessive inclination occurs, equipment damage is likely to take place, or a dangerous working environment arises
- Differences between initial and final baselines are calculated to ensure zero load offsets have not occurred and to ensure compliance with ASTM standards

The interpretation of piezocone data for this report is based on the corrected tip resistance (q_t), sleeve friction (f_s) and pore water pressure (u). The interpretation of soil type is based on the correlations developed by Robertson et al. (1986) and Robertson (1990, 2009). It should be noted that it is not always possible to accurately identify a soil behavior based on these parameters. In these situations, experience, judgment and an assessment of other parameters may be used to infer soil behavior type.

The recorded tip resistance (q_c) is the total force acting on the piezocone tip divided by its base area. The tip resistance is corrected for pore pressure effects and termed corrected tip resistance (q_t) according to the following expression presented in Robertson et al. (1986):

$$q_t = q_c + (1-a) \cdot u_2$$

where: q_t is the corrected tip resistance

q_c is the recorded tip resistance

u_2 is the recorded dynamic pore pressure behind the tip (u_2 position)

a is the Net Area Ratio for the piezocone (0.8 for ConeTec probes)

The sleeve friction (f_s) is the frictional force on the sleeve divided by its surface area. As all ConeTec piezocones have equal end area friction sleeves, pore pressure corrections to the sleeve data are not required.

The dynamic pore pressure (u) is a measure of the pore pressures generated during cone penetration. To record equilibrium pore pressure, the penetration must be stopped to allow the dynamic pore pressures to stabilize. The rate at which this occurs is predominantly a function of the permeability of the soil and the diameter of the cone.

The friction ratio (R_f) is a calculated parameter. It is defined as the ratio of sleeve friction to the tip resistance expressed as a percentage. Generally, saturated cohesive soils have low tip resistance, high friction ratios and generate large excess pore water pressures. Cohesionless soils have higher tip resistances, lower friction ratios and do not generate significant excess pore water pressure.

A summary of the CPTu soundings along with test details and individual plots are provided in the appendices. A set of files with calculated geotechnical parameters were generated for each sounding based on published correlations and are provided in Excel format in the data release folder. Information regarding the methods used is also included in the data release folder.

For additional information on CPTu interpretations and calculated geotechnical parameters, refer to Robertson et al. (1986), Lunne et al. (1997), Robertson (2009), Mayne (2013, 2014) and Mayne and Peuchen (2012).

References

ASTM D5778-12, 2012, "Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils", ASTM, West Conshohocken, US.

Lunne, T., Robertson, P.K. and Powell, J. J. M., 1997, "Cone Penetration Testing in Geotechnical Practice", Blackie Academic and Professional.

Mayne, P.W., 2013, "Evaluating yield stress of soils from laboratory consolidation and in-situ cone penetration tests", Sound Geotechnical Research to Practice (Holtz Volume) GSP 230, ASCE, Reston/VA: 406-420.

Mayne, P.W. and Peuchen, J., 2012, "Unit weight trends with cone resistance in soft to firm clays", Geotechnical and Geophysical Site Characterization 4, Vol. 1 (Proc. ISC-4, Pernambuco), CRC Press, London: 903-910.

Mayne, P.W., 2014, "Interpretation of geotechnical parameters from seismic piezocone tests", CPT'14 Keynote Address, Las Vegas, NV, May 2014.

Robertson, P.K., Campanella, R.G., Gillespie, D. and Greig, J., 1986, "Use of Piezometer Cone Data", Proceedings of InSitu 86, ASCE Specialty Conference, Blacksburg, Virginia.

Robertson, P.K., 1990, "Soil Classification Using the Cone Penetration Test", Canadian Geotechnical Journal, Volume 27: 151-158.

Robertson, P.K., 2009, "Interpretation of cone penetration tests – a unified approach", Canadian Geotechnical Journal, Volume 46: 1337-1355.

Shear wave velocity (V_s) testing is performed in conjunction with the piezocone penetration test (SCPTu) in order to collect interval velocities. For some projects seismic compression wave velocity (V_p) testing is also performed.

ConeTec's piezocone penetrometers are manufactured with a horizontally active geophone (28 hertz) that is rigidly mounted in the body of the cone penetrometer, 0.2 meters behind the cone tip.

Shear waves are typically generated by using an impact hammer horizontally striking a beam that is held in place by a normal load. In some instances an auger source or an imbedded impulsive source maybe used for both shear waves and compression waves. The hammer and beam act as a contact trigger that initiates the recording of the seismic wave traces. For impulsive devices an accelerometer trigger may be used. The traces are recorded using an up-hole integrated digital oscilloscope which is part of the SCPTu data acquisition system. An illustration of the shear wave testing configuration is presented in Figure SCPTu-1.

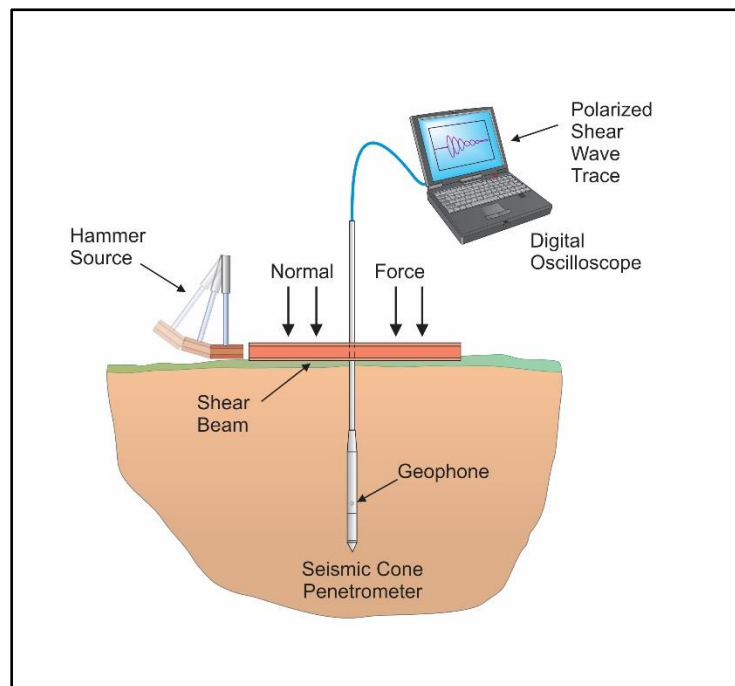


Figure SCPTu-1. Illustration of the SCPTu system

All testing is performed in accordance to ConeTec's SCPTu operating procedures which are in general accordance with the current ASTM 5778 and ASTM D7400 standards.

Prior to the start of a SCPTu sounding, the procedures described in the Cone Penetration Test section are followed. In addition, the active axis of the geophone is aligned parallel to the beam (or source) and the horizontal offset between the cone and the source is measured and recorded.

Prior to recording seismic waves at each test depth, cone penetration is stopped and the rods are decoupled from the rig to avoid transmission of rig energy down the rods. Typically, five wave traces for each orientation are recorded for quality control purposes and uncertainty analysis. After reviewing wave traces for consistency the cone is pushed to the next test depth (typically one meter intervals or as requested by the client). Figure SCPTu-2 presents an illustration of a SCPTu test.

For additional information on seismic cone penetration testing refer to Robertson et. al. (1986).

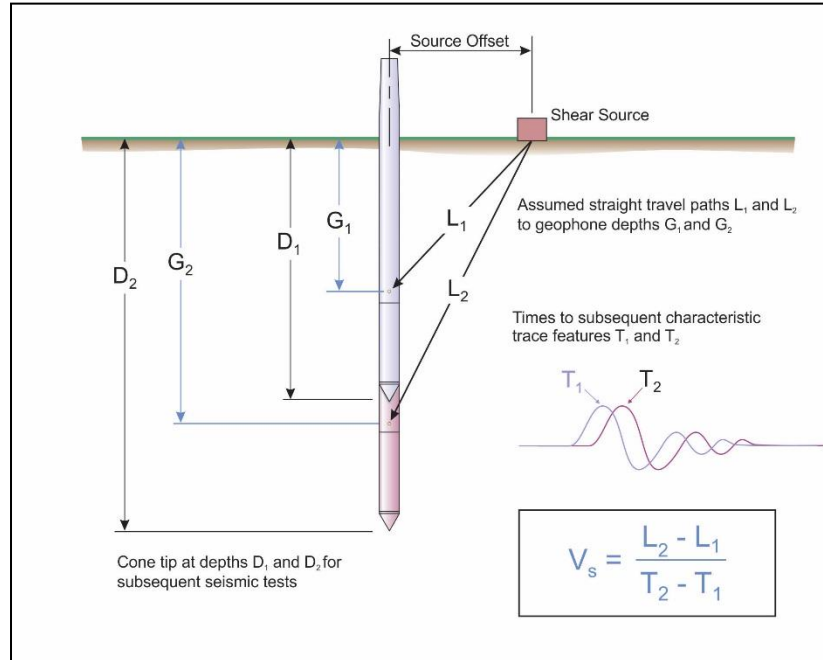


Figure SCPTu-2. Illustration of a seismic cone penetration test

Calculation of the interval velocities are performed by visually picking a common feature (e.g. the first characteristic peak, trough, or crossover) on all of the recorded wave sets and taking the difference in ray path divided by the time difference between subsequent features. Ray path is defined as the straight line distance from the seismic source to the geophone, accounting for beam offset, source depth and geophone offset from the cone tip.

For all SCPTu soundings that have achieved a depth of at least 100 feet (30 meters), the average shear wave velocity to a depth of 100 feet (\bar{v}_s) has been calculated using the following equation presented in ASCE (2010).

$$\bar{v}_s = \frac{\sum_{i=1}^n d_i}{\sum_{i=1}^n \frac{d_i}{v_{si}}}$$

where: \bar{v}_s = average shear wave velocity ft/s (m/s)
 d_i = the thickness of any layer between 0 and 100 ft (30 m)
 v_{si} = the shear wave velocity in ft/s (m/s)
 $\sum_{i=1}^n d_i = 100 \text{ ft (30 m)}$

Average shear wave velocity, \bar{v}_s is also referenced to V_{s100} or V_{s30} .

The layer travel times refers to the travel times propagating in the vertical direction, not the measured travel times from an offset source.

Tabular results and SCPTu plots are presented in the relevant appendix.

References

American Society of Civil Engineers (ASCE), 2010, "Minimum Design Loads for Buildings and Other Structures", Standard ASCE/SEI 7-10, American Society of Civil Engineers, ISBN 978-0-7844-1085-1, Reston, Virginia.

ASTM D5778-12, 2012, "Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils", ASTM, West Conshohocken, US.

ASTM D7400-14, 2014, "Standard Test Methods for Downhole Seismic Testing", ASTM, West Conshohocken, US.

Robertson, P.K., Campanella, R.G., Gillespie D and Rice, A., 1986, "Seismic CPT to Measure In-Situ Shear Wave Velocity", Journal of Geotechnical Engineering ASCE, Vol. 112, No. 8: 791-803.

The cone penetration test is halted at specific depths to carry out pore pressure dissipation (PPD) tests, shown in Figure PPD-1. For each dissipation test the cone and rods are decoupled from the rig and the data acquisition system measures and records the variation of the pore pressure (u) with time (t).

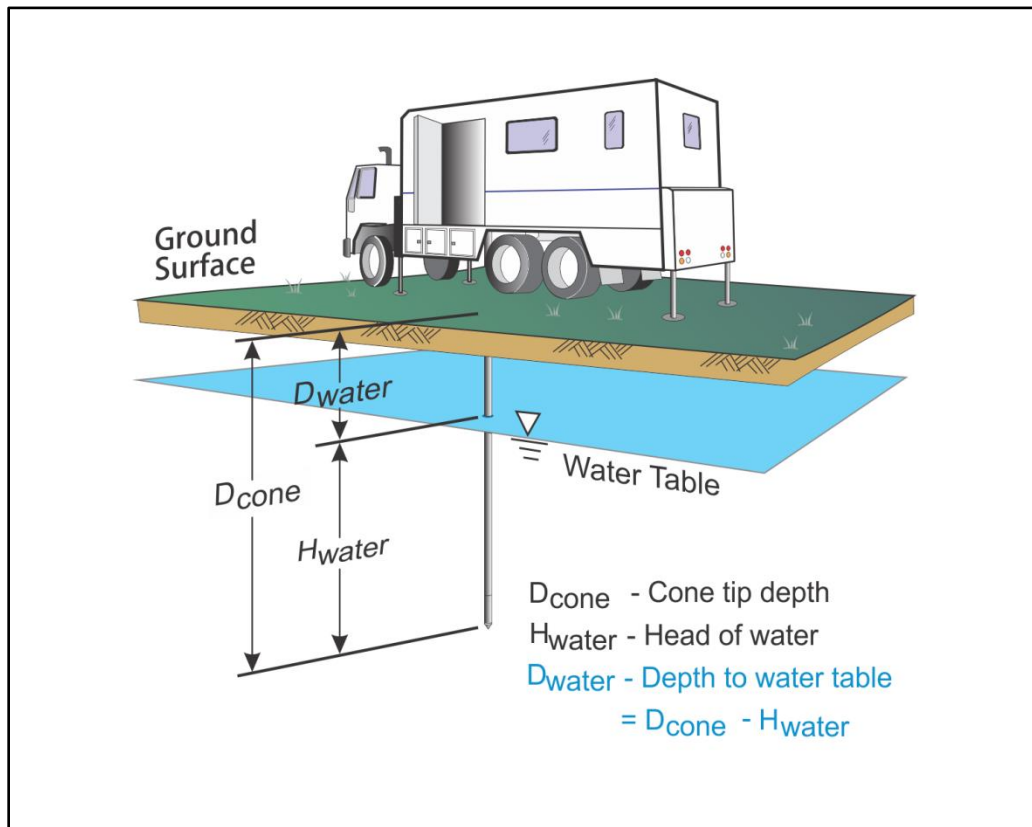


Figure PPD-1. Pore pressure dissipation test setup

Pore pressure dissipation data can be interpreted to provide estimates of ground water conditions, permeability, consolidation characteristics and soil behavior.

The typical shapes of dissipation curves shown in Figure PPD-2 are very useful in assessing soil type, drainage, in situ pore pressure and soil properties. A flat curve that stabilizes quickly is typical of a freely draining sand. Undrained soils such as clays will typically show positive excess pore pressure and have long dissipation times. Dilative soils will often exhibit dynamic pore pressures below equilibrium that then rise over time. Overconsolidated fine-grained soils will often exhibit an initial dilatory response where there is an initial rise in pore pressure before reaching a peak and dissipating.

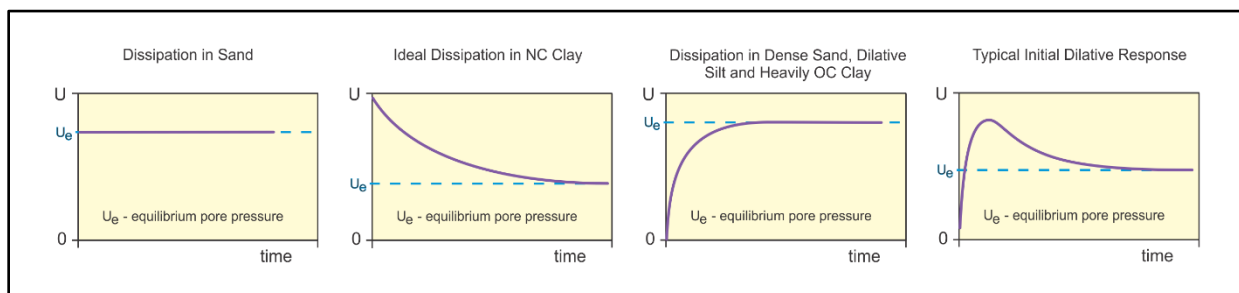


Figure PPD-2. Pore pressure dissipation curve examples

In order to interpret the equilibrium pore pressure (u_{eq}) and the apparent phreatic surface, the pore pressure should be monitored until such time as there is no variation in pore pressure with time as shown for each curve in Figure PPD-2.

In fine grained deposits the point at which 100% of the excess pore pressure has dissipated is known as t_{100} . In some cases this can take an excessive amount of time and it may be impractical to take the dissipation to t_{100} . A theoretical analysis of pore pressure dissipations by Teh and Houlsby (1991) showed that a single curve relating degree of dissipation versus theoretical time factor (T^*) may be used to calculate the coefficient of consolidation (c_h) at various degrees of dissipation resulting in the expression for c_h shown below.

$$c_h = \frac{T^* \cdot a^2 \cdot \sqrt{I_r}}{t}$$

Where:

- T^* is the dimensionless time factor (Table Time Factor)
 a is the radius of the cone
 I_r is the rigidity index
 t is the time at the degree of consolidation

Table Time Factor. T^* versus degree of dissipation (Teh and Houlsby (1991))

Degree of Dissipation (%)	20	30	40	50	60	70	80
$T^* (u_2)$	0.038	0.078	0.142	0.245	0.439	0.804	1.60

The coefficient of consolidation is typically analyzed using the time (t_{50}) corresponding to a degree of dissipation of 50% (u_{50}). In order to determine t_{50} , dissipation tests must be taken to a pressure less than u_{50} . The u_{50} value is half way between the initial maximum pore pressure and the equilibrium pore pressure value, known as u_{100} . To estimate u_{50} , both the initial maximum pore pressure and u_{100} must be known or estimated. Other degrees of dissipations may be considered, particularly for extremely long dissipations.

At any specific degree of dissipation the equilibrium pore pressure (u at t_{100}) must be estimated at the depth of interest. The equilibrium value may be determined from one or more sources such as measuring the value directly (u_{100}), estimating it from other dissipations in the same profile, estimating the phreatic surface and assuming hydrostatic conditions, from nearby soundings, from client provided information, from site observations and/or past experience, or from other site instrumentation.

For calculations of c_h (Teh and Houlsby (1991)), t_{50} values are estimated from the corresponding pore pressure dissipation curve and a rigidity index (I_r) is assumed. For curves having an initial dilatatory response in which an initial rise in pore pressure occurs before reaching a peak, the relative time from the peak value is used in determining t_{50} . In cases where the time to peak is excessive, t_{50} values are not calculated.

Due to possible inherent uncertainties in estimating I_r , the equilibrium pore pressure and the effect of an initial dilatatory response on calculating t_{50} , other methods should be applied to confirm the results for c_h .

Additional published methods for estimating the coefficient of consolidation from a piezocone test are described in Burns and Mayne (1998, 2002), Jones and Van Zyl (1981), Robertson et al. (1992) and Sully et al. (1999).

A summary of the pore pressure dissipation tests and dissipation plots are presented in the relevant appendix.

References

Burns, S.E. and Mayne, P.W., 1998, "Monotonic and dilatory pore pressure decay during piezocone tests", Canadian Geotechnical Journal 26 (4): 1063-1073.

Burns, S.E. and Mayne, P.W., 2002, "Analytical cavity expansion-critical state model cone dissipation in fine-grained soils", Soils & Foundations, Vol. 42(2): 131-137.

Jones, G.A. and Van Zyl, D.J.A., 1981, "The piezometer probe: a useful investigation tool", Proceedings, 10th International Conference on Soil Mechanics and Foundation Engineering, Vol. 3, Stockholm: 489-495.

Robertson, P.K., Sully, J.P., Woeller, D.J., Lunne, T., Powell, J.J.M. and Gillespie, D.G., 1992, "Estimating coefficient of consolidation from piezocone tests", Canadian Geotechnical Journal, 29(4): 551-557.

Sully, J.P., Robertson, P.K., Campanella, R.G. and Woeller, D.J., 1999, "An approach to evaluation of field CPTU dissipation data in overconsolidated fine-grained soils", Canadian Geotechnical Journal, 36(2): 369-381.

Teh, C.I., and Houlsby, G.T., 1991, "An analytical study of the cone penetration test in clay", Geotechnique, 41(1): 17-34.

The appendices listed below are included in the report:

- Cone Penetration Test Summary and Standard Cone Penetration Test Plots
- Advanced Cone Penetration Test Plots with I_c , $S_u(Nkt)$, Φ and $N1(60)I_c$
- Seismic Cone Penetration Test Plots
- Seismic Cone Penetration Test Shear Wave (V_s) Traces
- Seismic Cone Penetration Test Tabular Results
- Soil Behavior Type (SBT) Scatter Plots
- Pore Pressure Dissipation Summary and Pore Pressure Dissipation Plots

Cone Penetration Test Summary and Standard Cone Penetration Test Plots



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Start Date: 26-Oct-2020
End Date: 02-Nov-2020

CONE PENETRATION TEST SUMMARY

Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface ¹ (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Latitude ² (°)	Longitude ² (°)	Refer to Notation Number
CPT20-101	20-53-21525_CP101	01-Nov-2020	524:T375F10U500		1.97		44.77144	-68.72026	5
CPT20-101B	20-53-21525_CP101B	02-Nov-2020	524:T375F10U500	17.0	57.17		44.77142	-68.72029	4
SCPT20-101	20-53-21525_SP101	27-Oct-2020	524:T375F10U500	3.5	8.45	3	44.78975	-68.69922	3
SCPT20-101B	20-53-21525_SP101B	28-Oct-2020	524:T375F10U500	3.5	21.33	9	44.78983	-68.69899	3
SCPT20-102	20-53-21525_SP102	29-Oct-2020	524:T375F10U500	6.0	31.58	15	44.79006	-68.69877	3
SCPT20-103	20-53-21525_SP103	01-Nov-2020	524:T375F10U500	4.0	34.20	13	44.77205	-68.71887	3
CPT20-104	20-53-21525_CP104	01-Nov-2020	524:T375F10U500	2.5	16.73		44.77203	-68.71788	3
SCPT20-104	20-53-21525_SP104	01-Nov-2020	524:T375F10U500	5.0	18.04	8	44.77043	-68.71722	4
CPT20-105	20-53-21525_CP105	01-Nov-2020	524:T375F10U500	2.5	13.45		44.77231	-68.71711	
CPT20-106	20-53-21525_CP106	01-Nov-2020	524:T375F10U500	2.5	11.73		44.77124	-68.71678	3
CPT20-108	20-53-21525_CP108	01-Nov-2020	524:T375F10U500	3.0	15.91		44.77305	-68.71653	3
CPT20-109	20-53-21525_CP109	26-Oct-2020	524:T375F10U500	2.5	14.11		44.77371	-68.71513	3
CPT20-110	20-53-21525_CP110	26-Oct-2020	524:T375F10U500	2.0	14.44		44.77429	-68.71409	3
CPT20-111	20-53-21525_CP111	26-Oct-2020	524:T375F10U500	2.0	4.02		44.77508	-68.71296	3
CPT20-111B	20-53-21525_CP111B	26-Oct-2020	524:T375F10U500	2.0	6.73		44.77509	-68.71294	3
CPT20-112	20-53-21525_CP112	26-Oct-2020	524:T375F10U500	3.0	22.88		44.77594	-68.71206	3
CPT20-113	20-53-21525_CP113	26-Oct-2020	524:T375F10U500	3.5	19.77		44.77673	-68.71133	3
CPT20-114	20-53-21525_CP114	27-Oct-2020	524:T375F10U500	3.0	15.34		44.78939	-68.69967	3
CPT20-115	20-53-21525_CP115	27-Oct-2020	524:T375F10U500	5.0	11.16		44.78969	-68.69938	3
CPT20-115B	20-53-21525_CP115B	27-Oct-2020	524:T375F10U500	5.0	6.56		44.78971	-68.69936	3
CPT20-116	20-53-21525_CP116	29-Oct-2020	524:T375F10U500	5.0	16.73		44.79045	-68.69834	3



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Start Date: 26-Oct-2020
End Date: 02-Nov-2020

CONE PENETRATION TEST SUMMARY

Sounding ID	File Name	Date	Cone	Assumed Phreatic Surface ¹ (ft)	Final Depth (ft)	Shear Wave Velocity Tests	Latitude ² (°)	Longitude ² (°)	Refer to Notation Number
CPT20-117	20-53-21525_CP117	29-Oct-2020	524:T375F10U500	2.0	5.66		44.79429	-68.69303	3
CPT20-118	20-53-21525_CP118	29-Oct-2020	524:T375F10U500	2.5	16.16		44.79470	-68.69252	3
CPT20-119	20-53-21525_CP119	29-Oct-2020	524:T375F10U500	4.0	14.35		44.79490	-68.69217	3
CPT20-122	20-53-21525_CP122	30-Oct-2020	524:T375F10U500	3.0	24.28		44.79780	-68.68837	3
CPT20-123	20-53-21525_CP123	30-Oct-2020	524:T375F10U500	3.0	14.68		44.79816	-68.68809	3
CPT20-124	20-53-21525_CP124	30-Oct-2020	524:T375F10U500	3.0	11.40		44.79850	-68.68774	3
CPT20-125	20-53-21525_CP125	30-Oct-2020	524:T375F10U500	3.0	10.25		44.79925	-68.68700	3
CPT20-126	20-53-21525_CP126	30-Oct-2020	524:T375F10U500	2.0	8.20		44.80002	-68.68636	3
CPT20-127	20-53-21525_CP127	30-Oct-2020	524:T375F10U500	2.0	4.51		44.80037	-68.68603	3
Totals	30 soundings				471.78	48			

1. The assumed phreatic surface was based on pore pressure dissipation tests. Hydrostatic data were used for the calculated parameters.
2. Coordinates were acquired using a handheld GPS Receiver in datum: WGS 84 Lat / Long Decimal Degrees.
3. The assumed phreatic surface was estimated from the dynamic pore pressure data.
4. The assumed phreatic surface was estimated from elevation datum provided by Google Earth Pro.
5. No phreatic surface detected.



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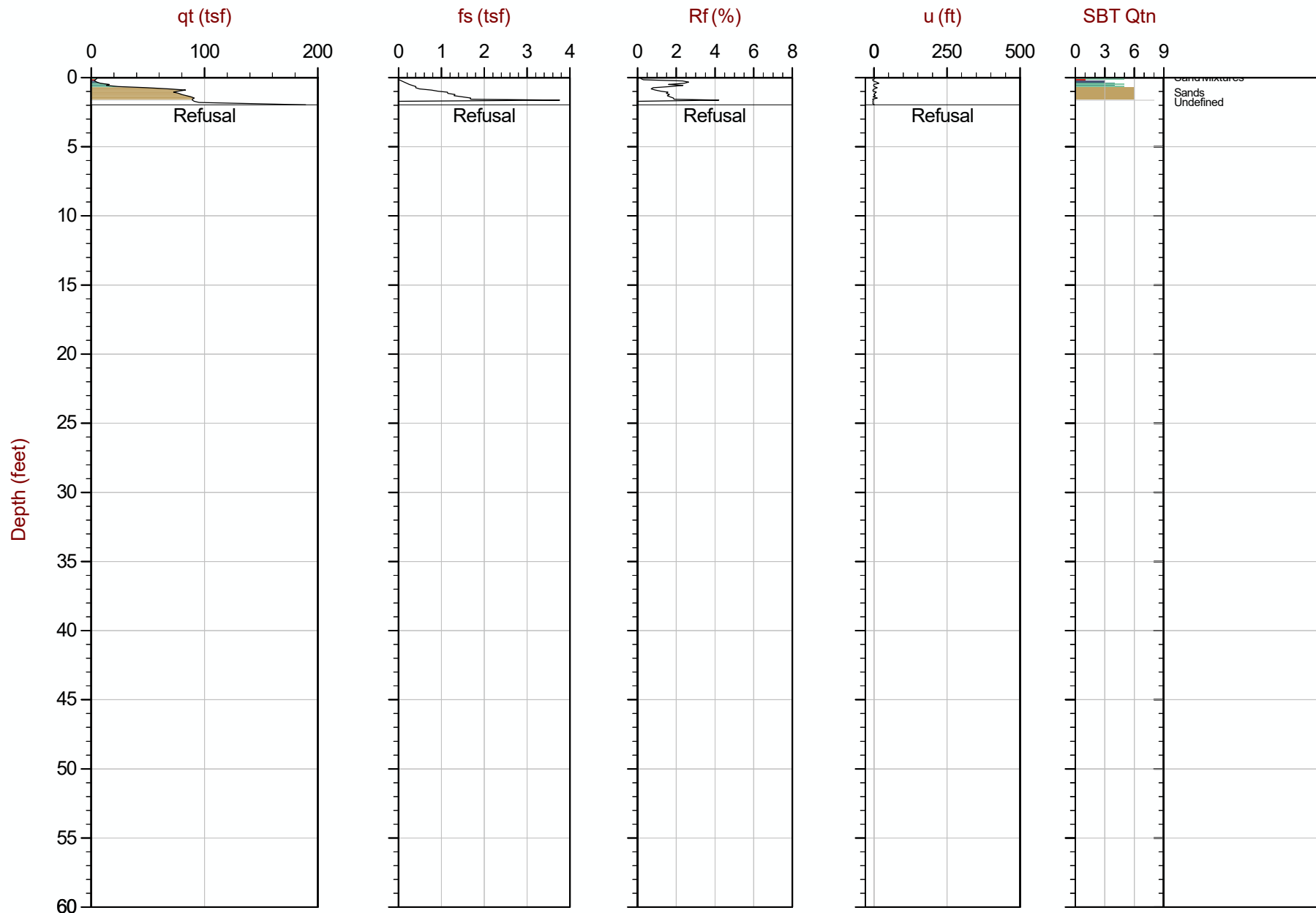
Job No: 20-53-21525

Date: 2020-11-01 07:33

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101

Cone: 524:T375F10U500



Max Depth: 0.600 m / 1.97 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP101.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77144 ° Long: -68.72026 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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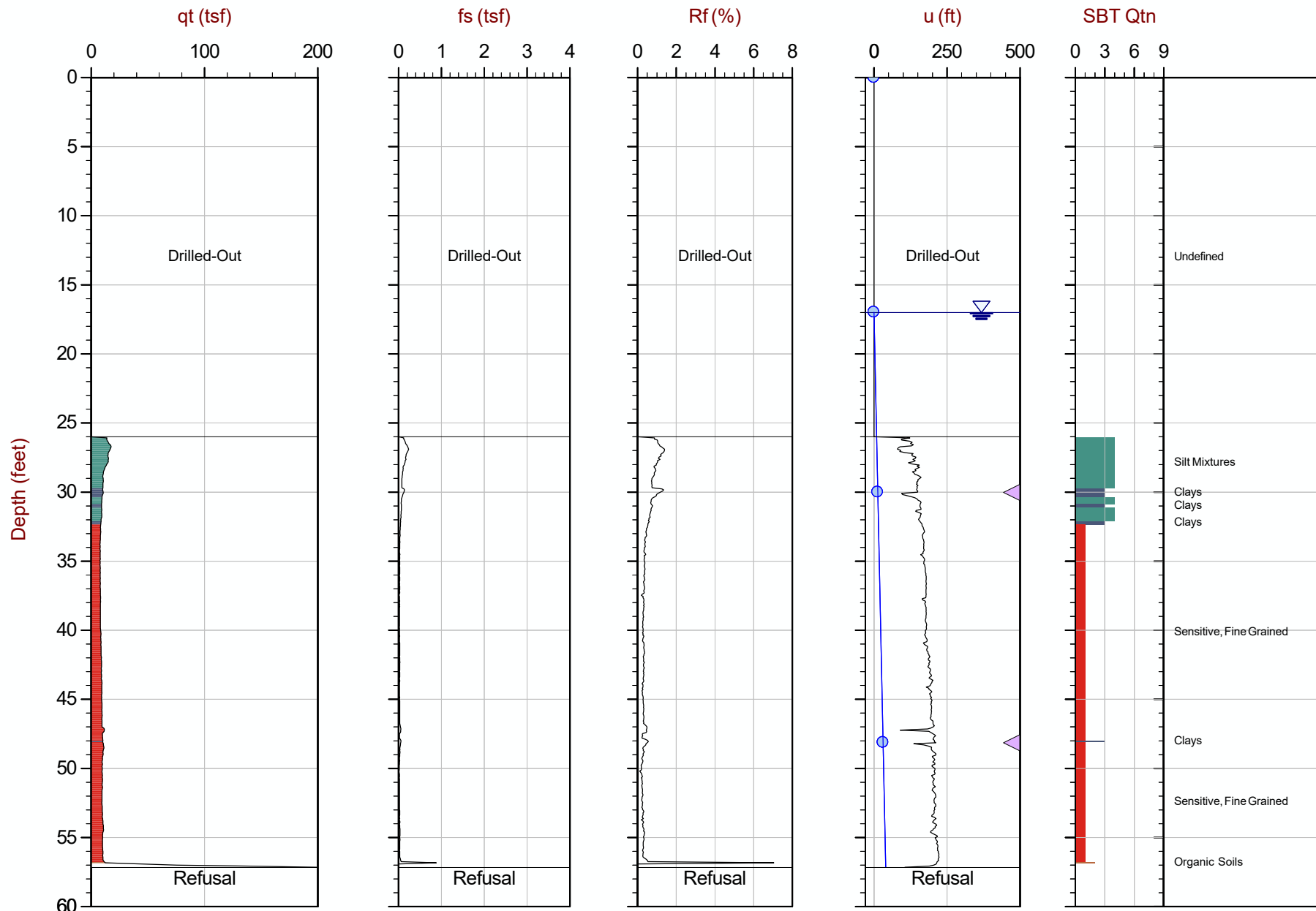
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Date: 2020-11-02 10:10

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101B

Cone: 524:T375F10U500



Max Depth: 17.425 m / 57.17 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP101B.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77142 ° Long: -68.72029 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▲ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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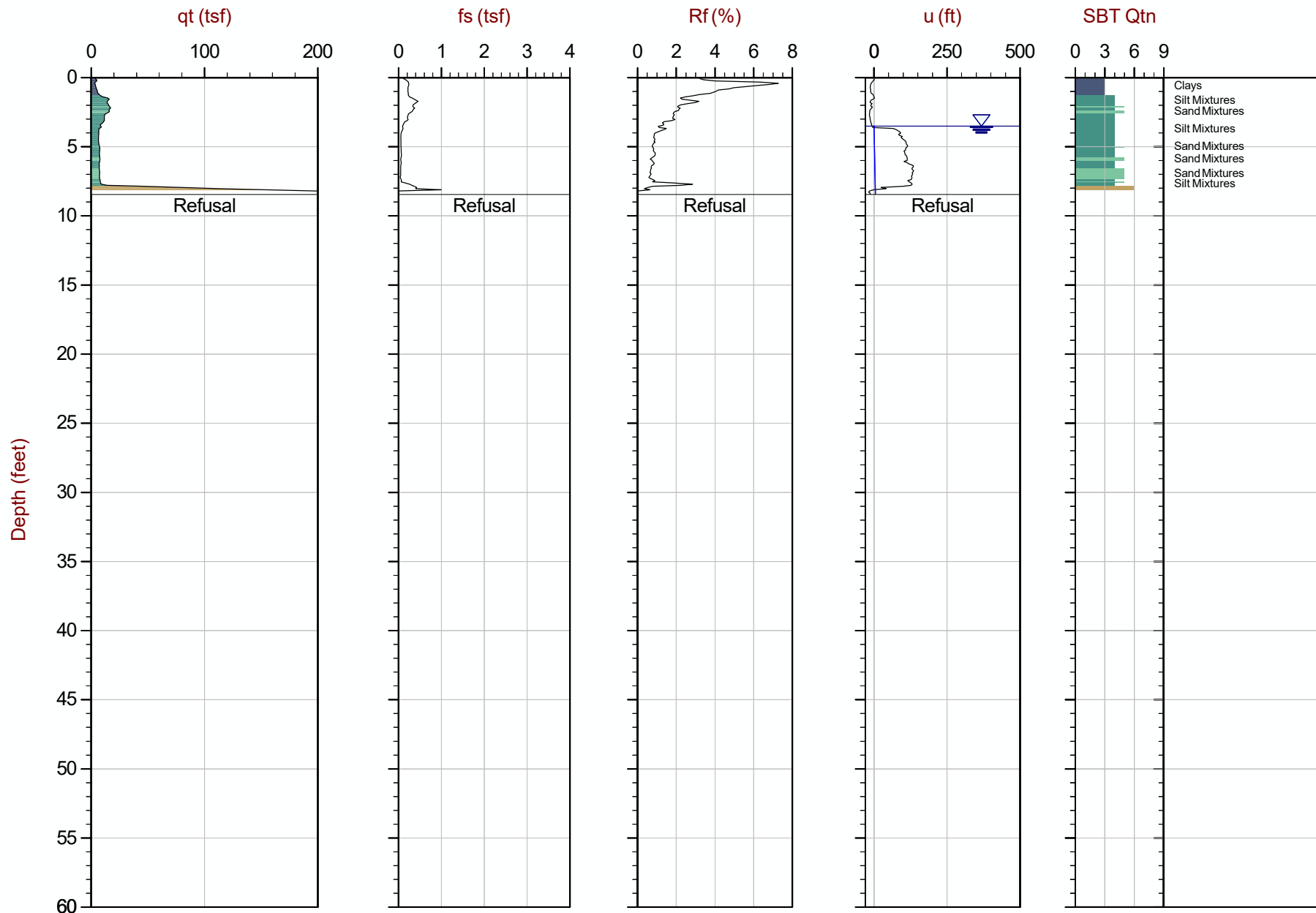
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Date: 2020-10-27 14:05

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101

Cone: 524:T375F10U500



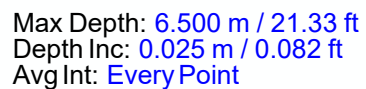
Max Depth: 2.575 m / 8.45 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP101.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78975 ° Long: -68.69922 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



File: 20-53-21525_SP101B.COR
Unit Wt: SBTQtn (PKR2009)

SBT: Robertson, 2009 and 2010
 Coords: Lat: 44.78983 ° Long: -68.69899 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ◀ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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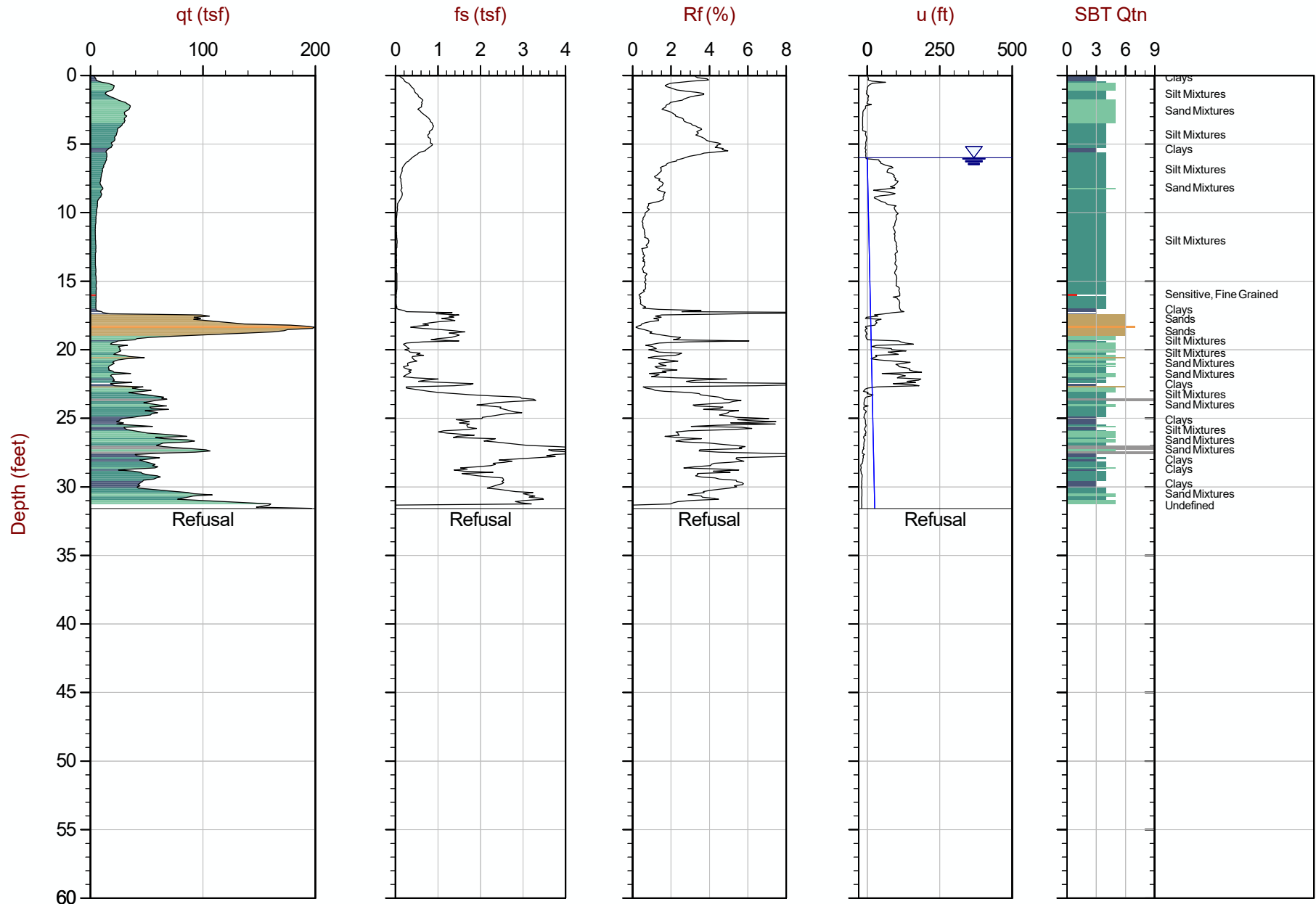
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Date: 2020-10-29 08:44

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-102

Cone: 524:T375F10U500



Max Depth: 9.625 m / 31.58 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP102.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79006 ° Long: -68.69877 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

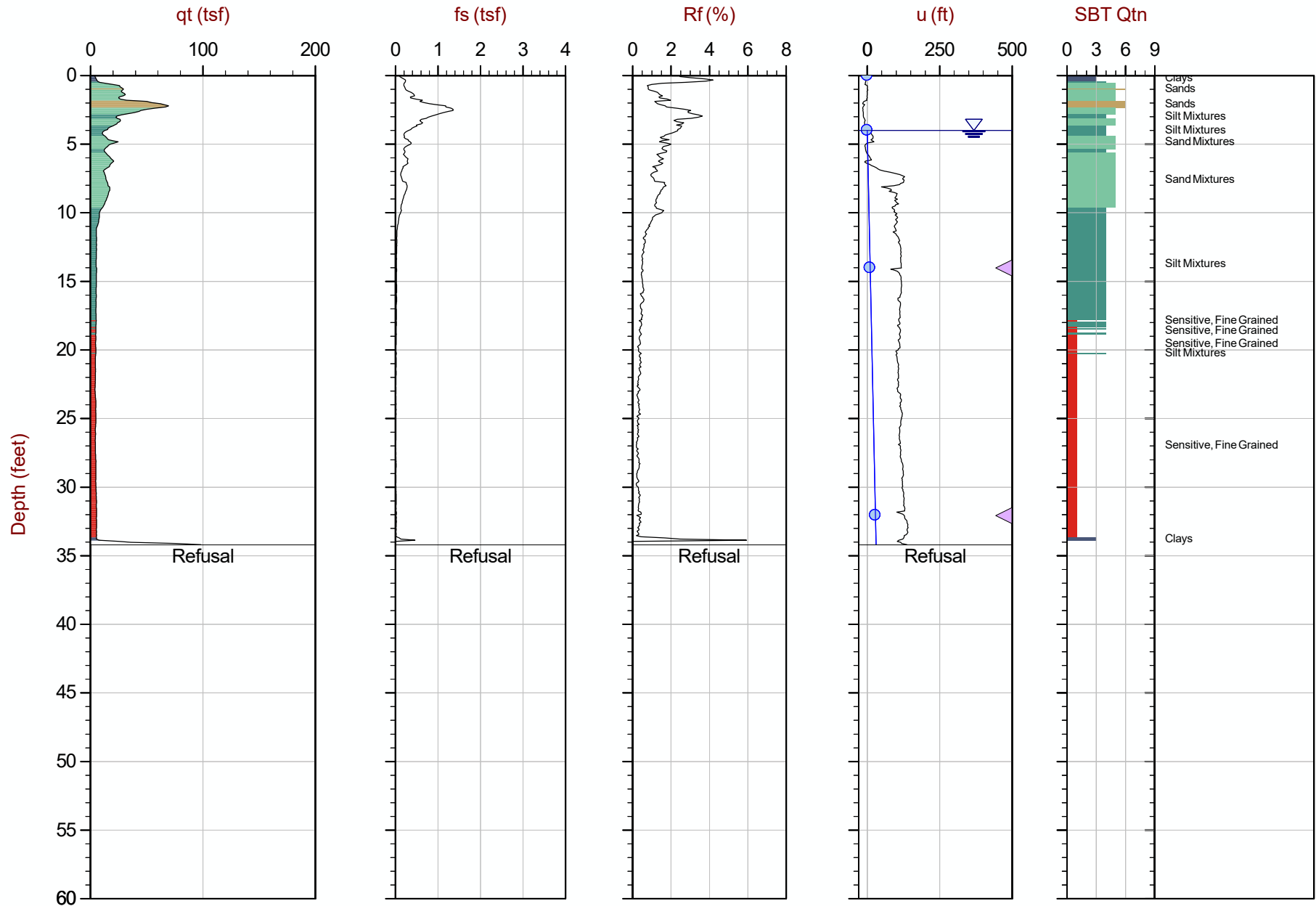
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Date: 2020-11-01 12:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103

Cone: 524:T375F10U500



Max Depth: 10.425 m / 34.20 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP103.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77204 ° Long: -68.71887 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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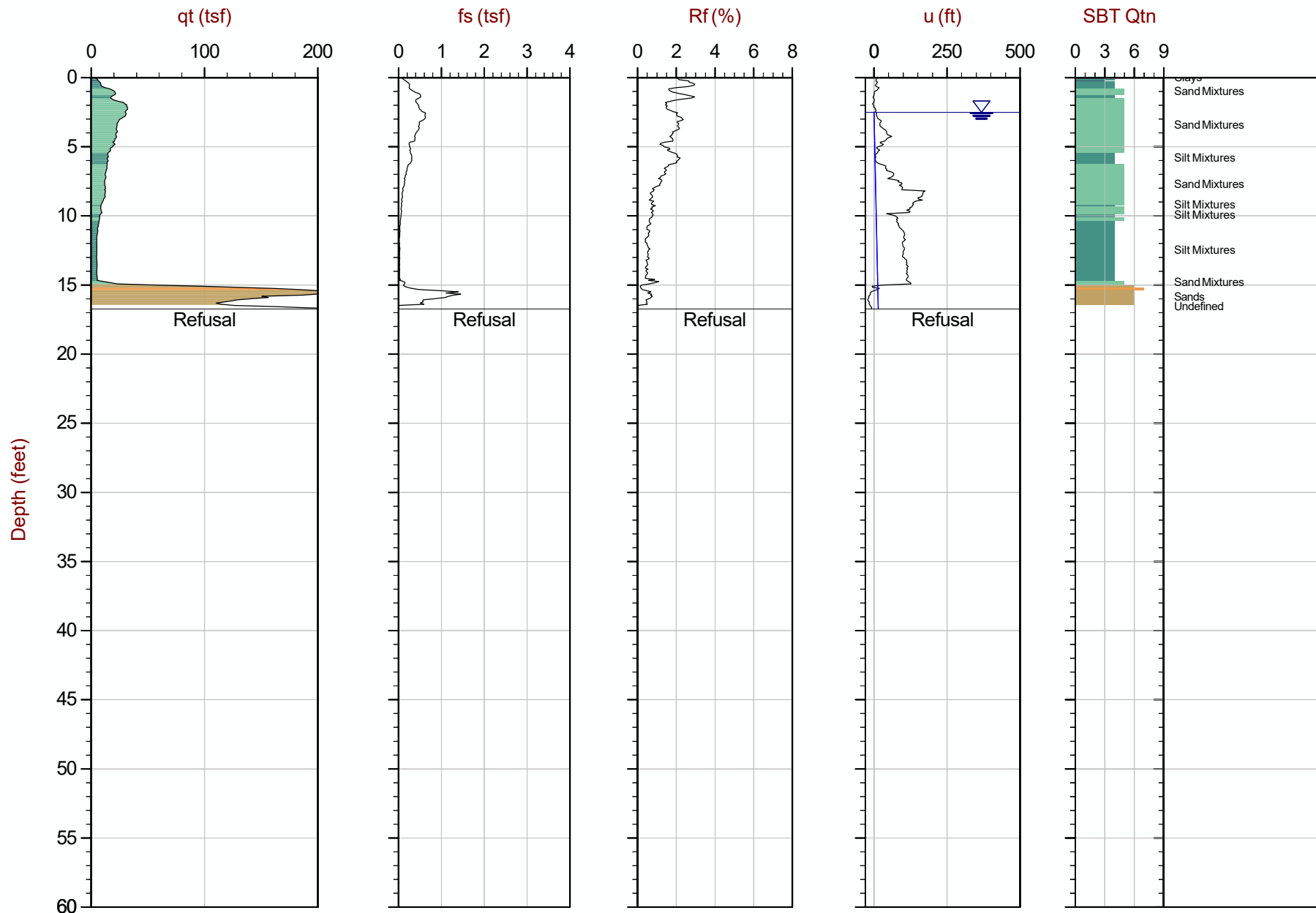
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Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-104

Cone: 524:T375F10U500



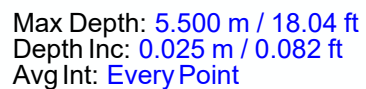
Max Depth: 5.100 m / 16.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP104.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77203 ° Long: -68.71788 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



File: 20-53-21525_SP104.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
 Coords: Lat: 44.77043 ° Long: -68.71722 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ◀ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

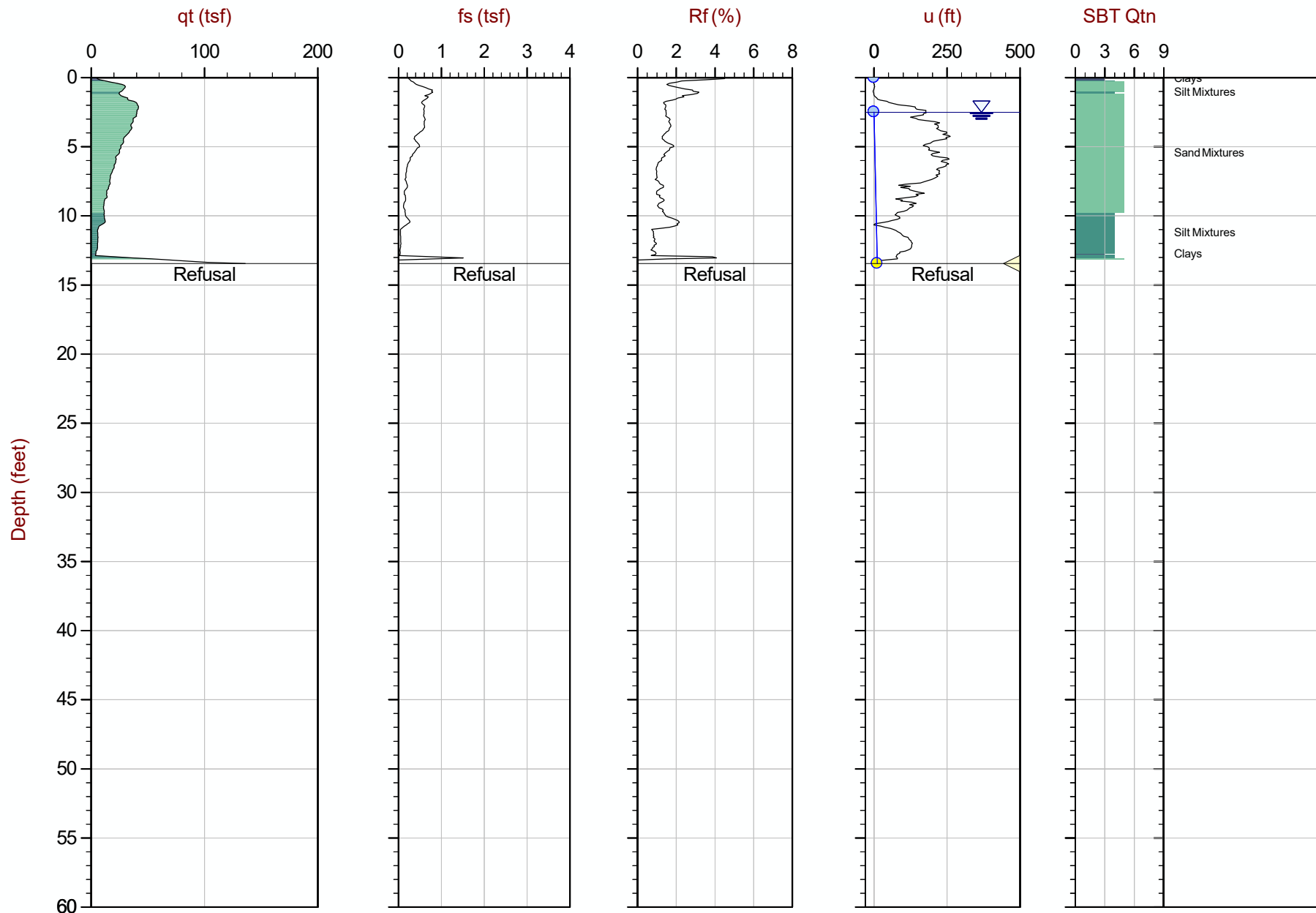
Job No: 20-53-21525

Date: 2020-11-01 10:13

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-105

Cone: 524:T375F10U500



Max Depth: 4.100 m / 13.45 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP105.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77231 ° Long: -68.71711 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

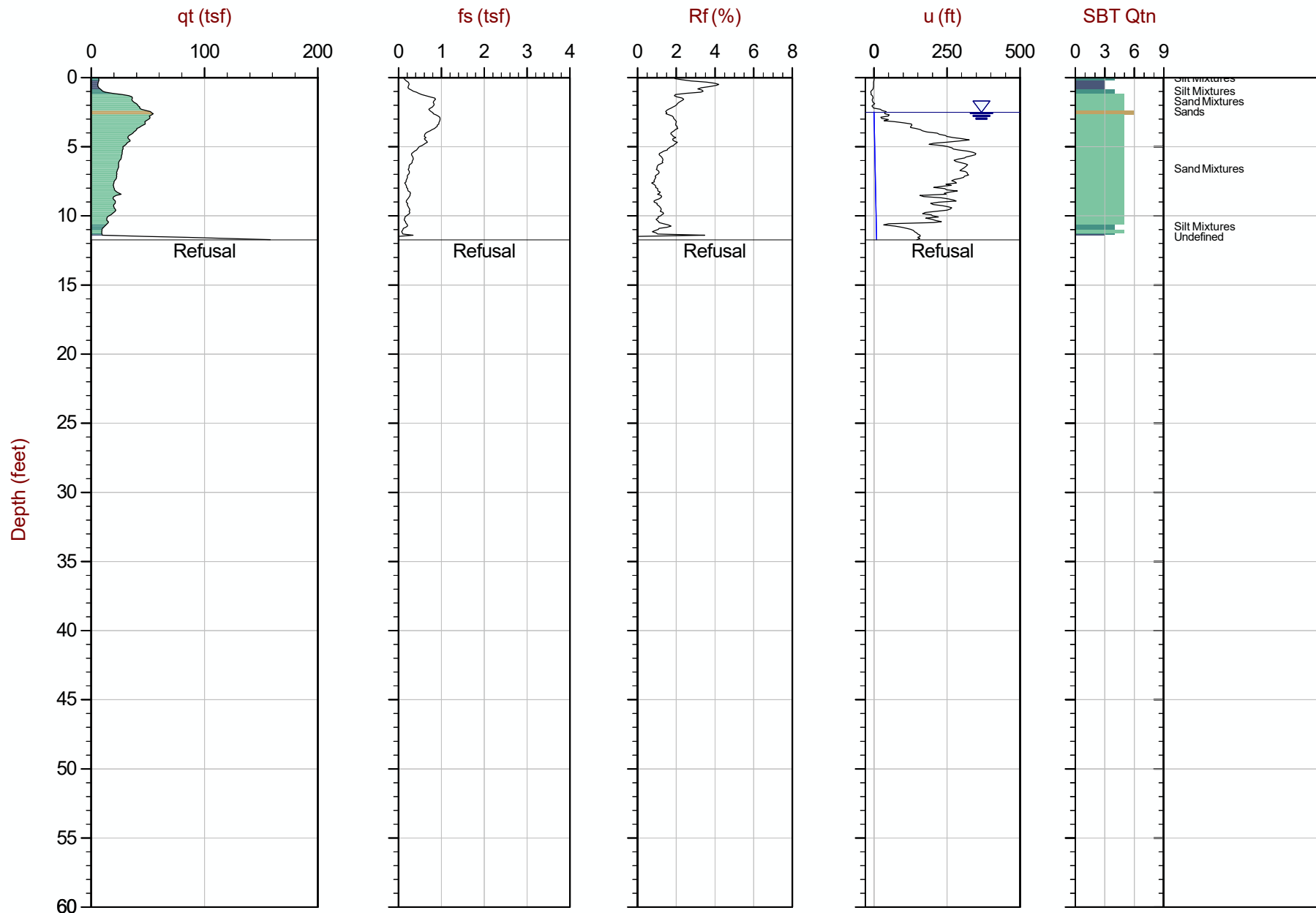
Job No: 20-53-21525

Date: 2020-11-01 09:22

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-106

Cone: 524:T375F10U500



Max Depth: 3.575 m / 11.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP106.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77124 ° Long: -68.71678 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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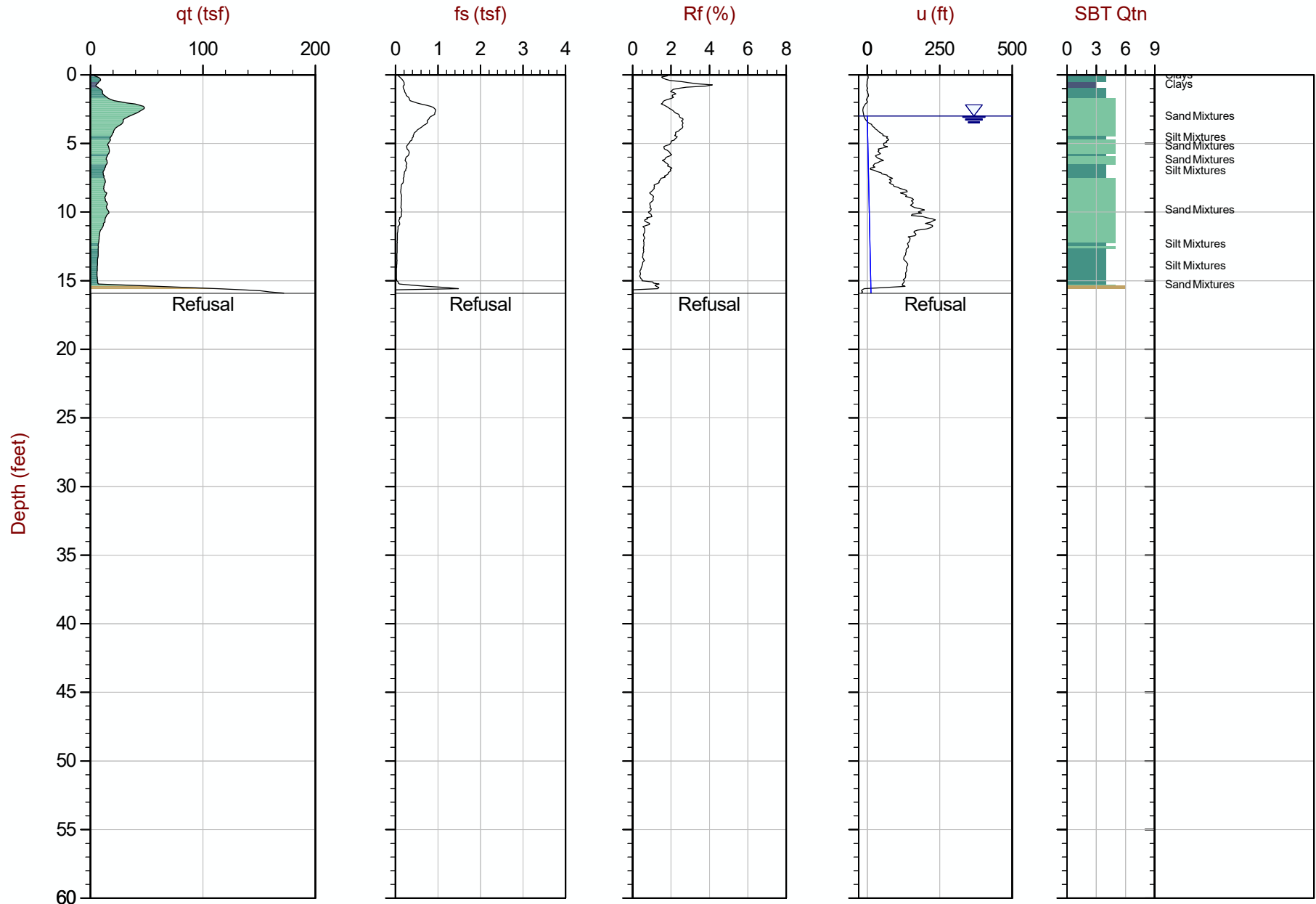
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Date: 2020-11-01 11:13

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-108

Cone: 524:T375F10U500



Max Depth: 4.850 m / 15.91 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP108.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77305 ° Long: -68.71653 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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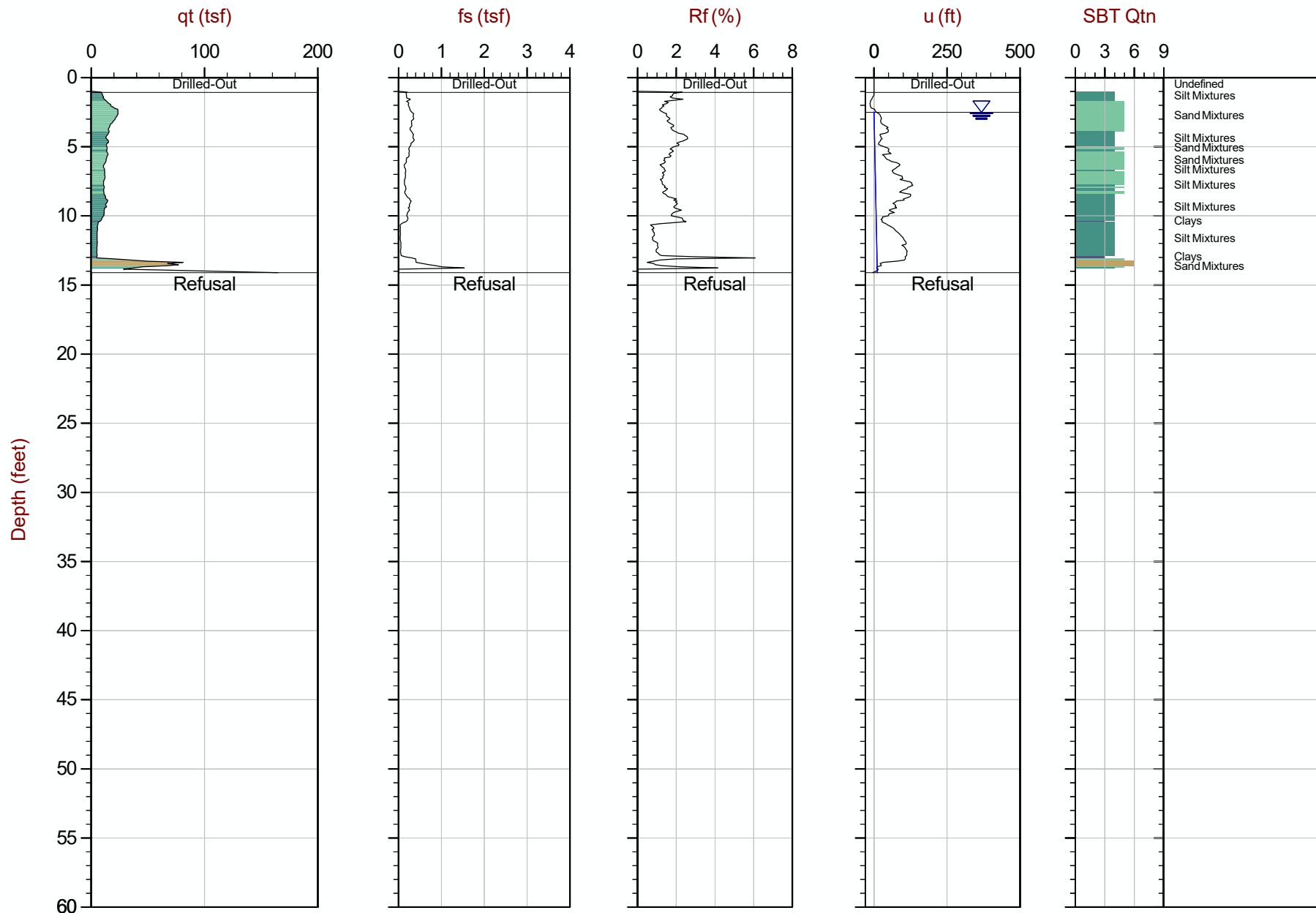
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Date: 2020-10-26 09:58

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-109

Cone: 524:T375F10U500



Max Depth: 4.300 m / 14.11 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP109.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77371 ° Long: -68.71513 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



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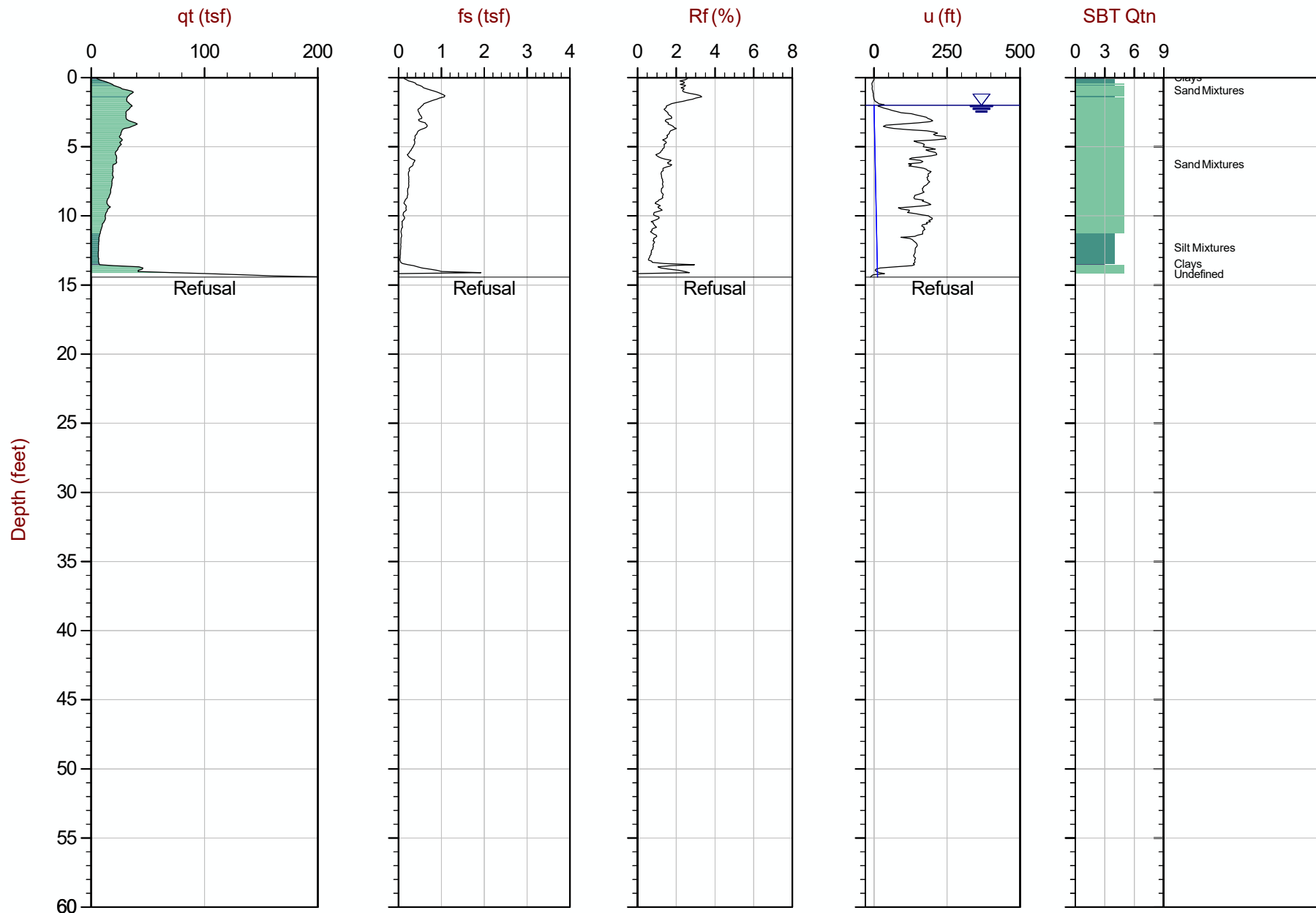
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Date: 2020-10-26 11:12

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-110

Cone: 524:T375F10U500



Max Depth: 4.400 m / 14.44 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP110.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77429 ° Long: -68.71409 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ◀ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

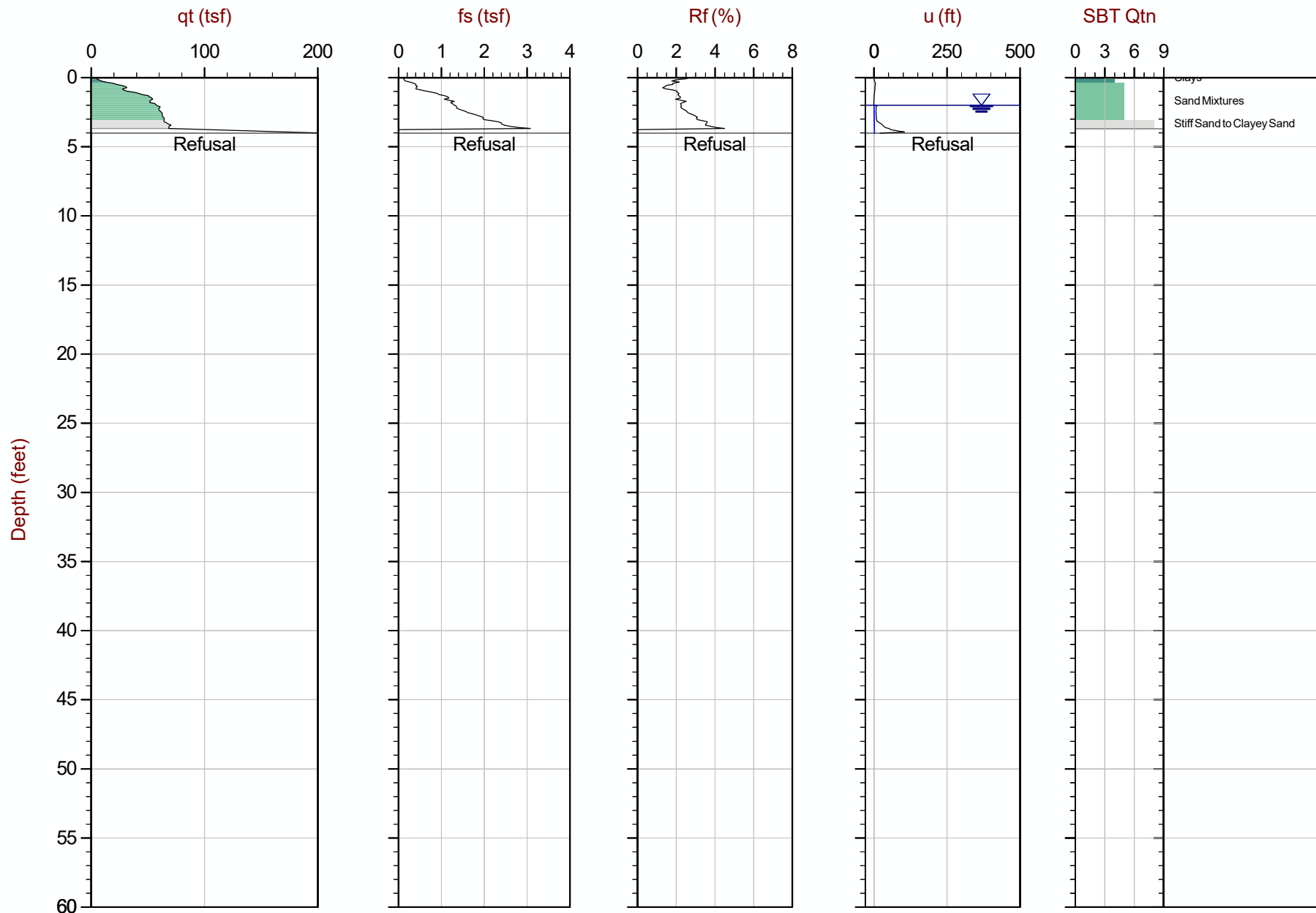
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Date: 2020-10-26 12:14

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-111

Cone: 524:T375F10U500



Max Depth: 1.225 m / 4.02 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP111.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77508 ° Long: -68.71296 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

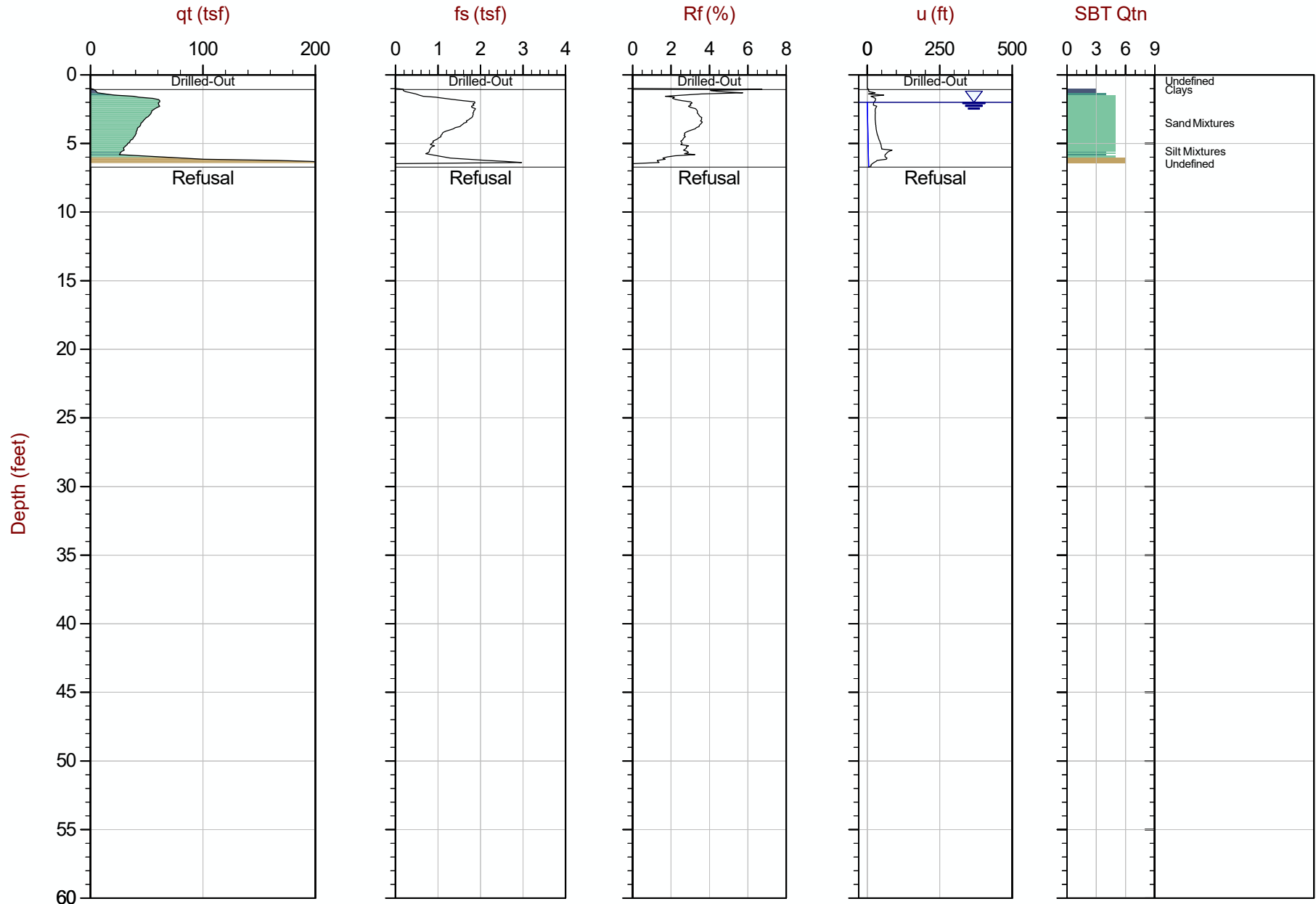
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Date: 2020-10-26 12:35

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-111B

Cone: 524:T375F10U500



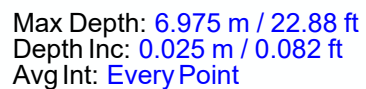
Max Depth: 2.050 m / 6.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP111B.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77509 ° Long: -68.71294 °

Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ▶ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



File: 20-53-21525_CP112.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
 Coords: Lat: 44.77594 ° Long: -68.71206 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ◀ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

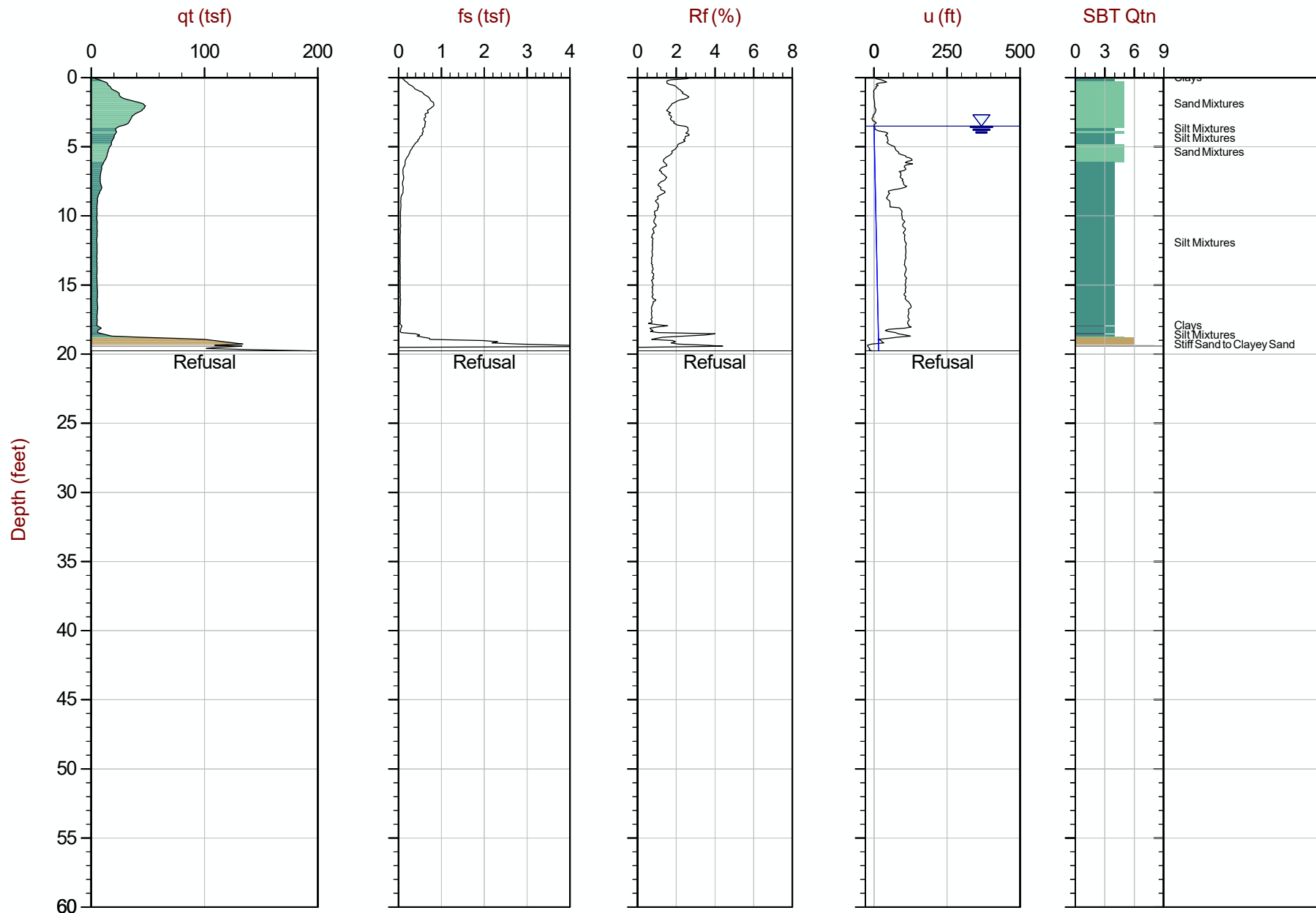
Job No: 20-53-21525

Date: 2020-10-26 14:50

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-113

Cone: 524:T375F10U500



Max Depth: 6.025 m / 19.77 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP113.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77673 ° Long: -68.71133 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

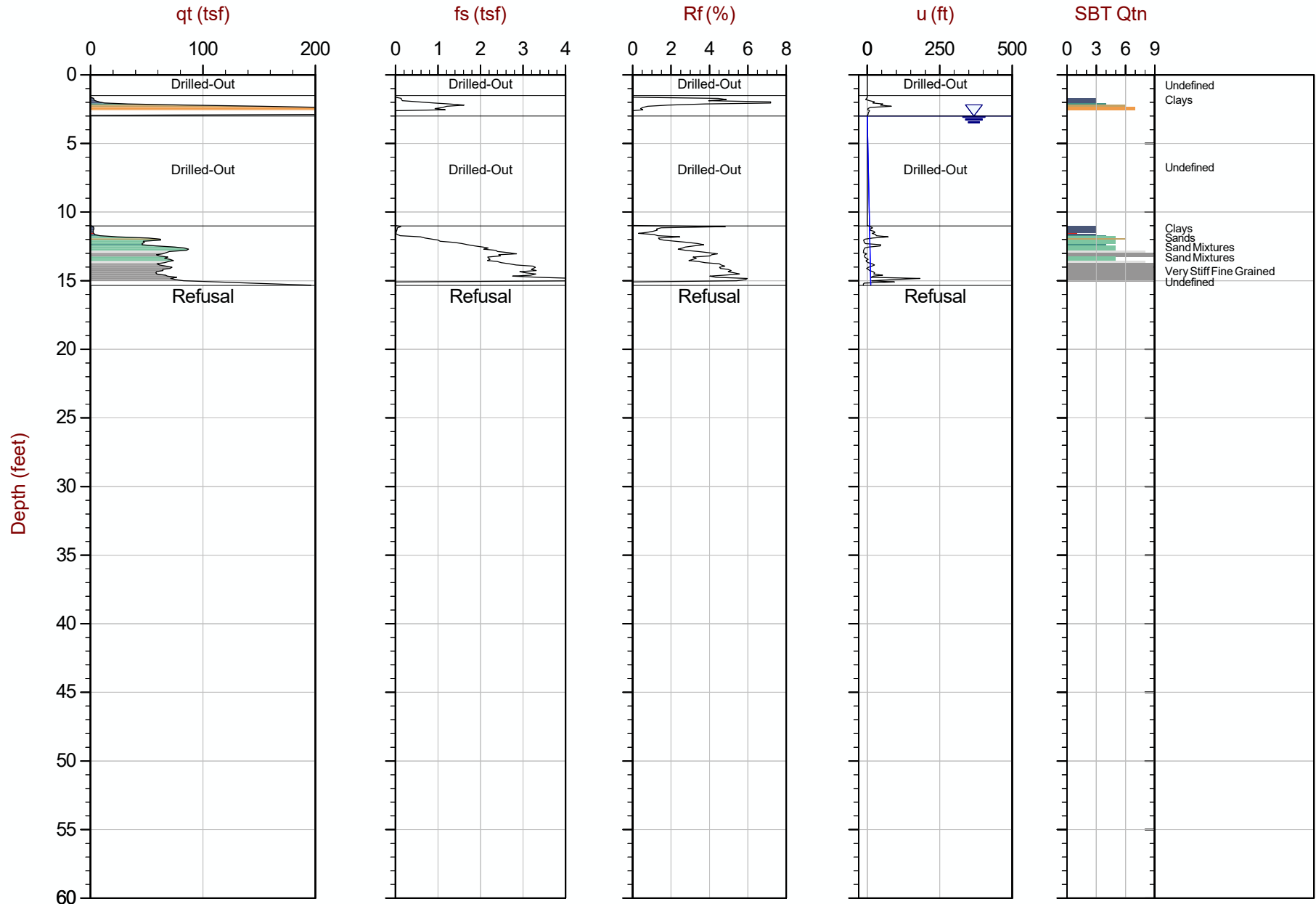
Job No: 20-53-21525

Date: 2020-10-27 10:25

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-114

Cone: 524:T375F10U500



Max Depth: 4.675 m / 15.34 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP114.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78939 ° Long: -68.69967 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

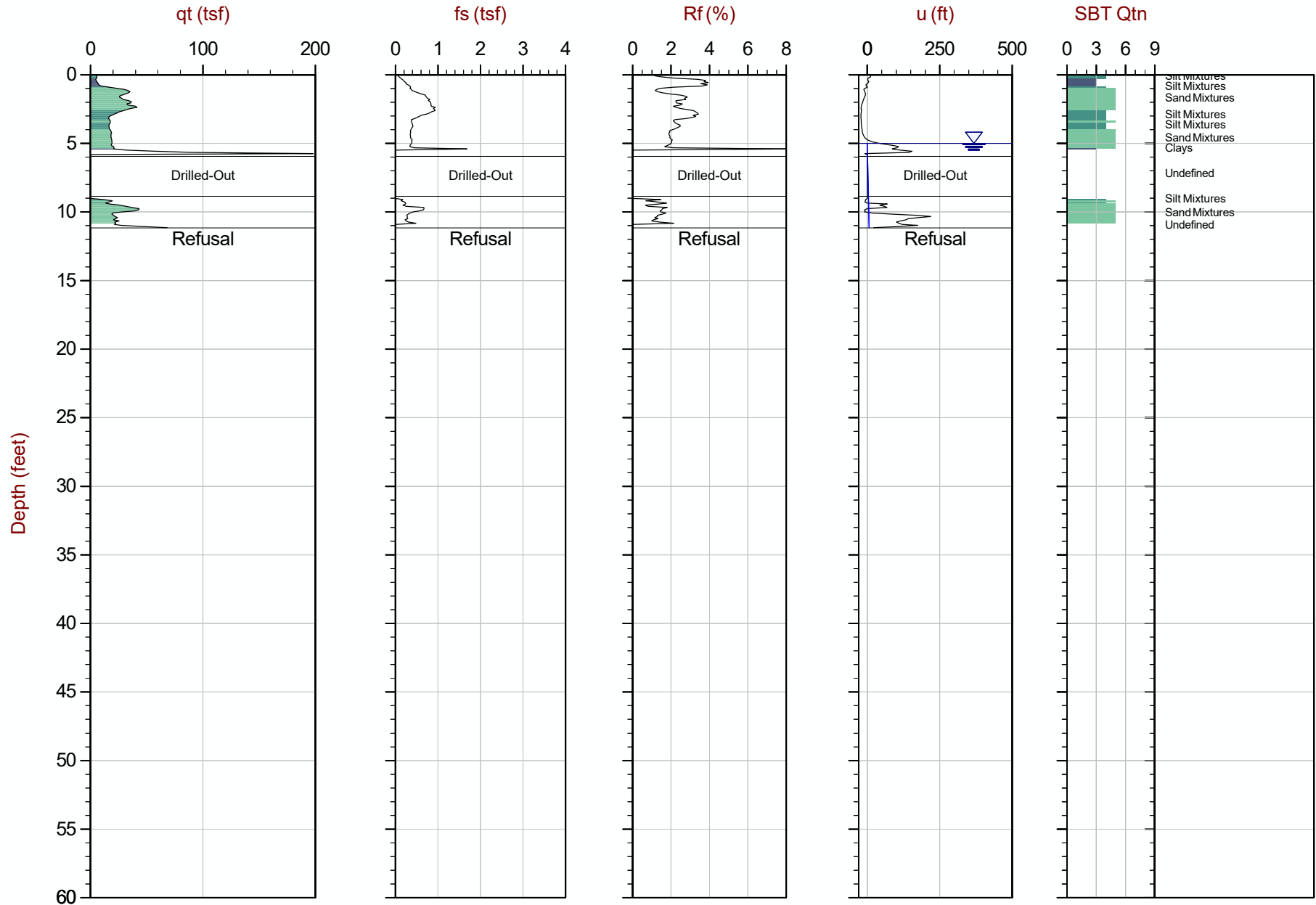
Job No: 20-53-21525

Date: 2020-10-27 12:04

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-115

Cone: 524:T375F10U500



Max Depth: 3.400 m / 11.15 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP115.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78969 ° Long: -68.69938 °

Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ▶ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

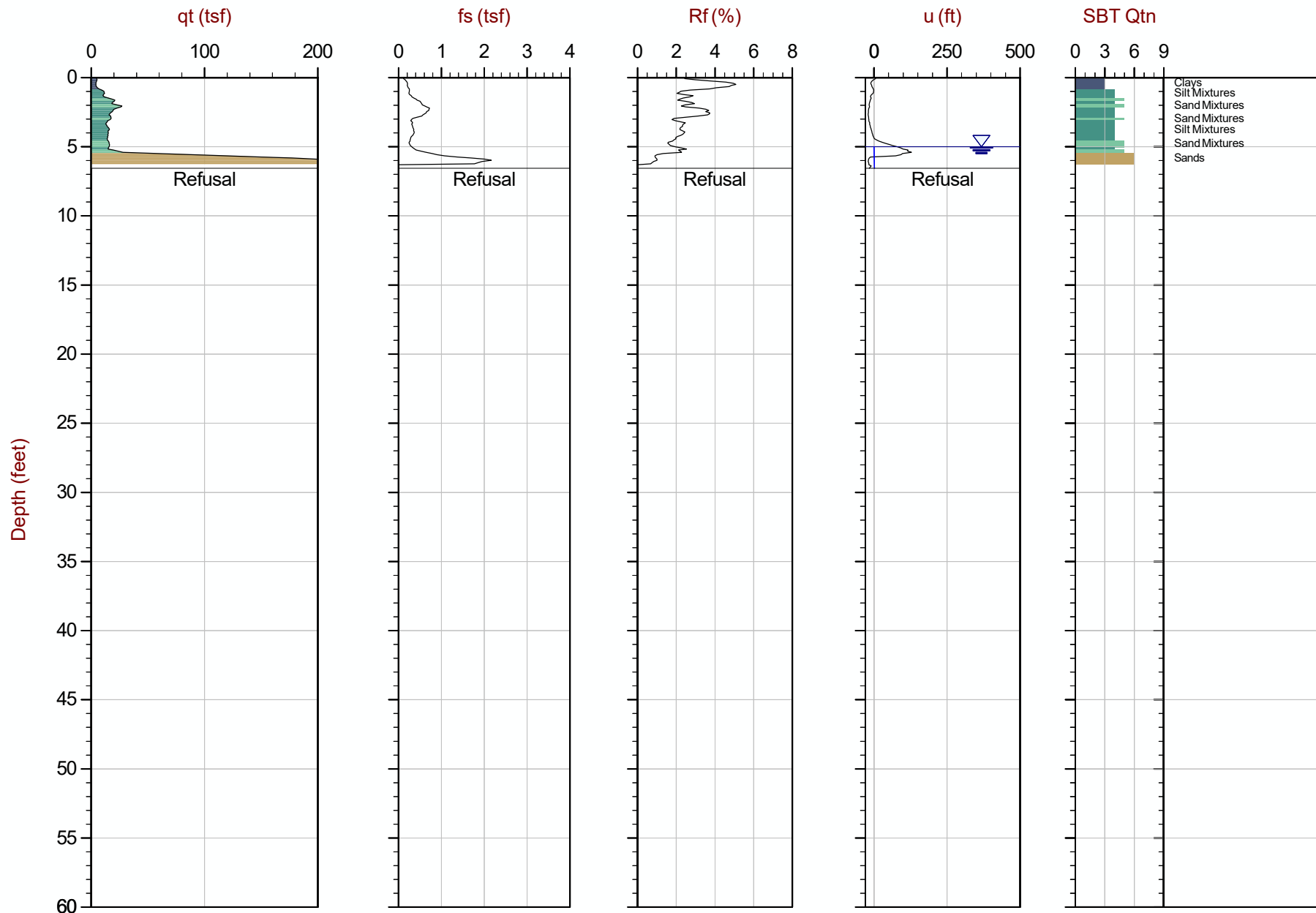
Job No: 20-53-21525

Date: 2020-10-27 13:16

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-115B

Cone: 524:T375F10U500



Max Depth: 2.000 m / 6.56 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP115B.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78971 ° Long: -68.69936 °

Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▲ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

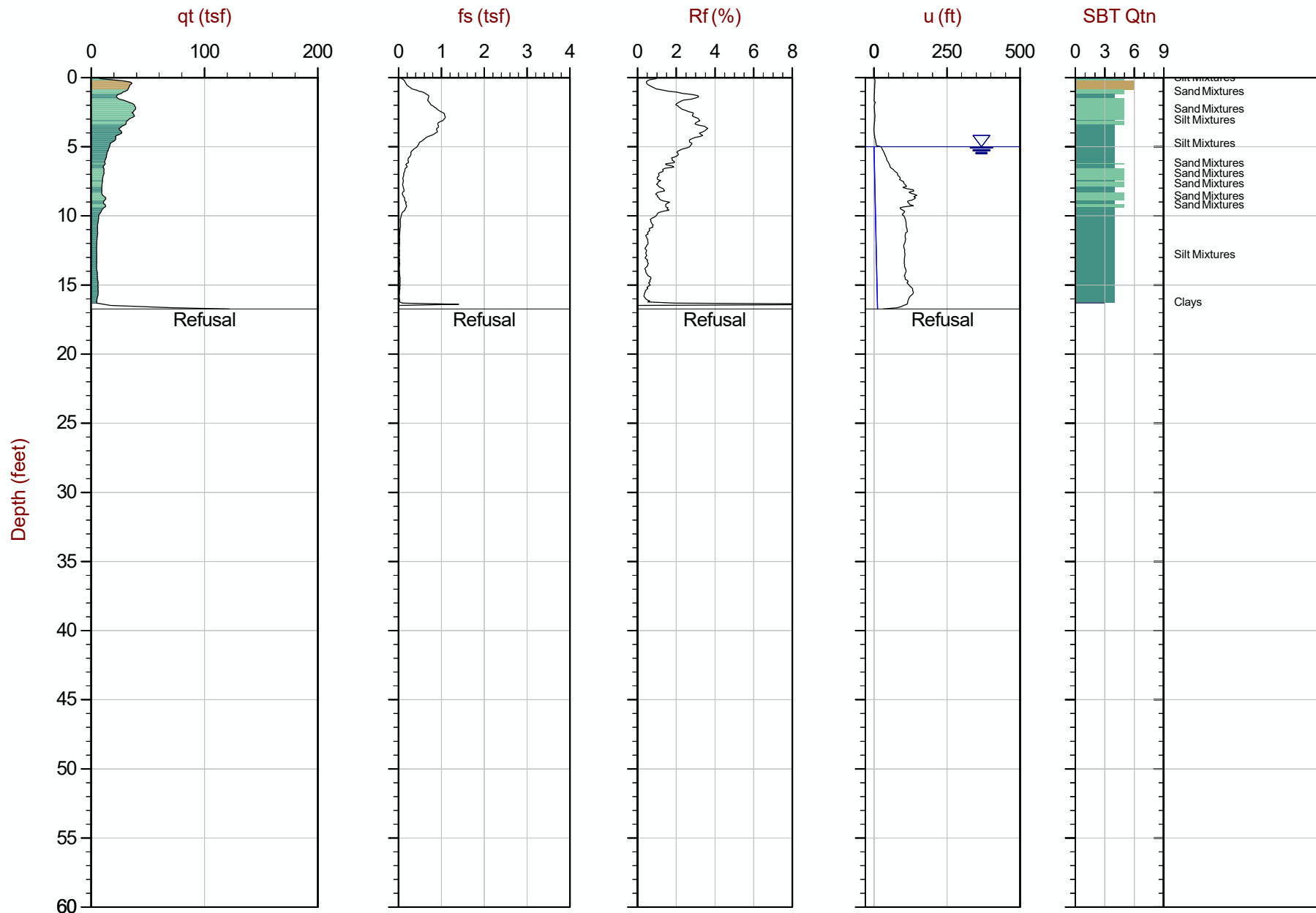
Job No: 20-53-21525

Date: 2020-10-29 10:24

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-116

Cone: 524:T375F10U500



Max Depth: 5.100 m / 16.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP116.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79045 ° Long: -68.69834 °

Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ▶ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

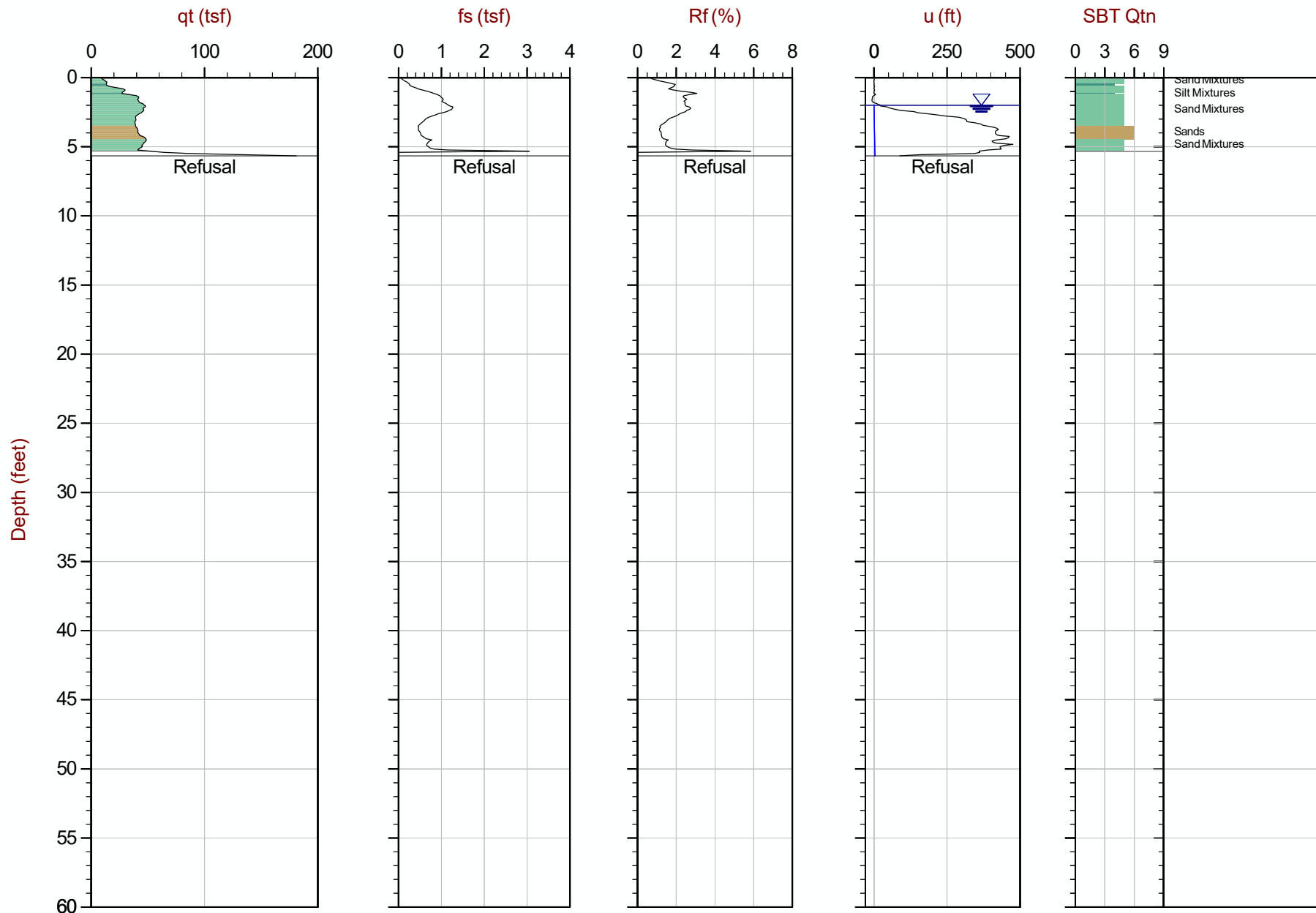
Job No: 20-53-21525

Date: 2020-10-29 11:34

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-117

Cone: 524:T375F10U500



Max Depth: 1.725 m / 5.66 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP117.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79429 ° Long: -68.69303 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

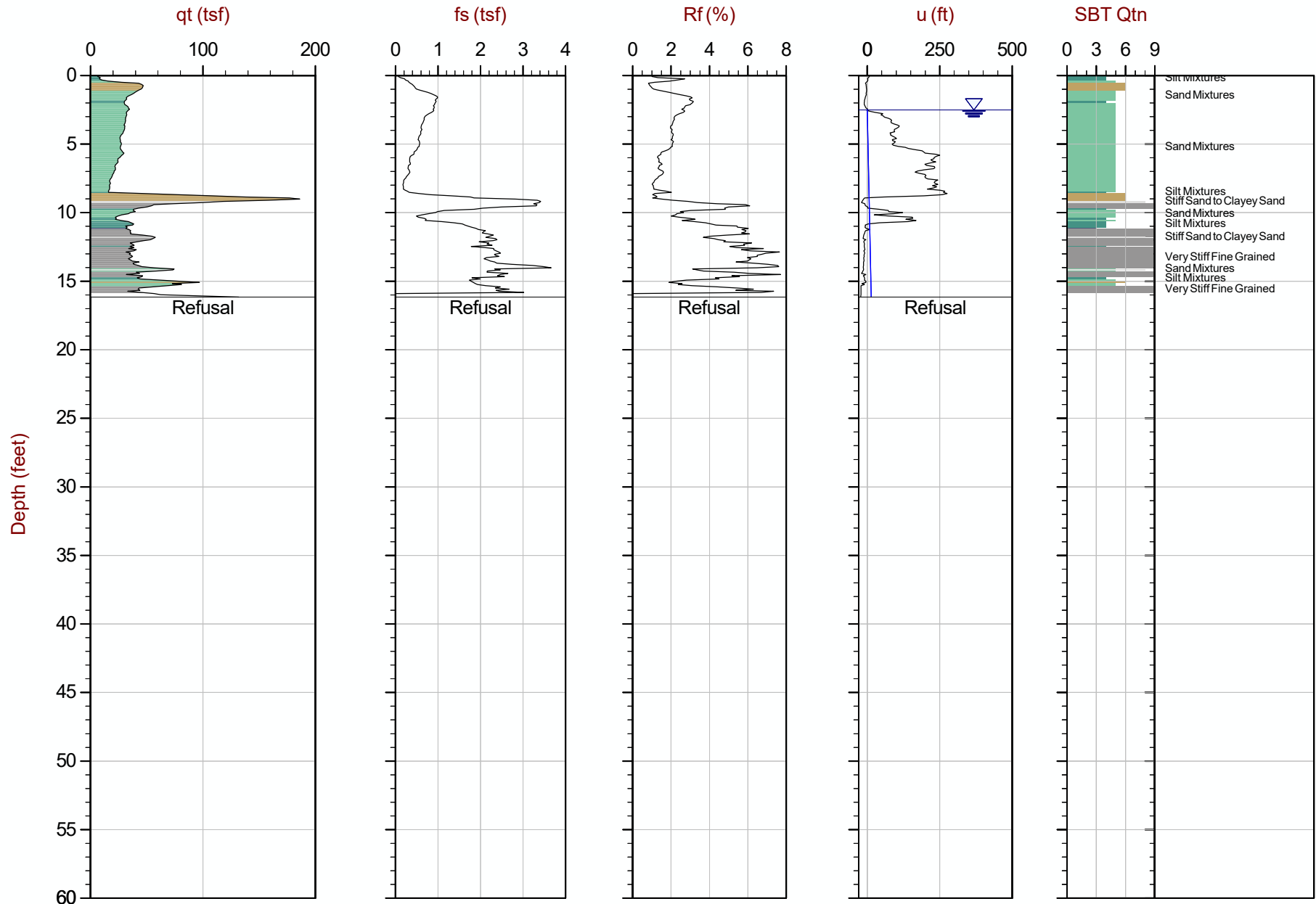
Job No: 20-53-21525

Date: 2020-10-29 12:05

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-118

Cone: 524:T375F10U500



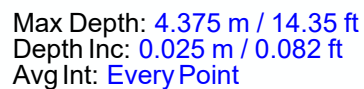
Max Depth: 4.925 m / 16.16 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP118.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79470 ° Long: -68.69252 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



File: 20-53-21525_CP119.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79490 ° Long: -68.69217 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ◀ PPD, Ueq achieved ◀ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

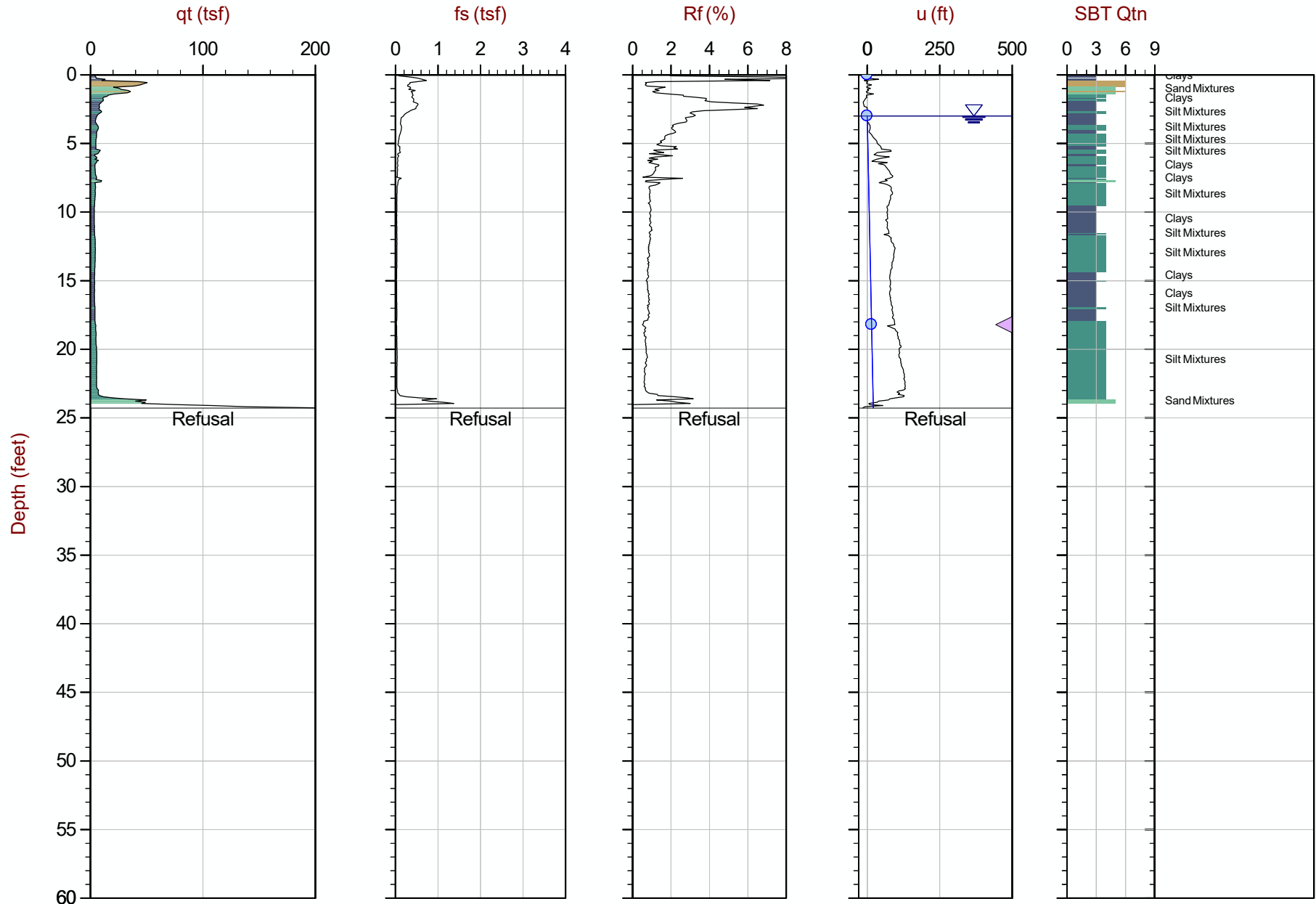
Job No: 20-53-21525

Date: 2020-10-30 12:58

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-122

Cone: 524:T375F10U500



Max Depth: 7.400 m / 24.28 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP122.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79780 ° Long: -68.68837 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▲ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

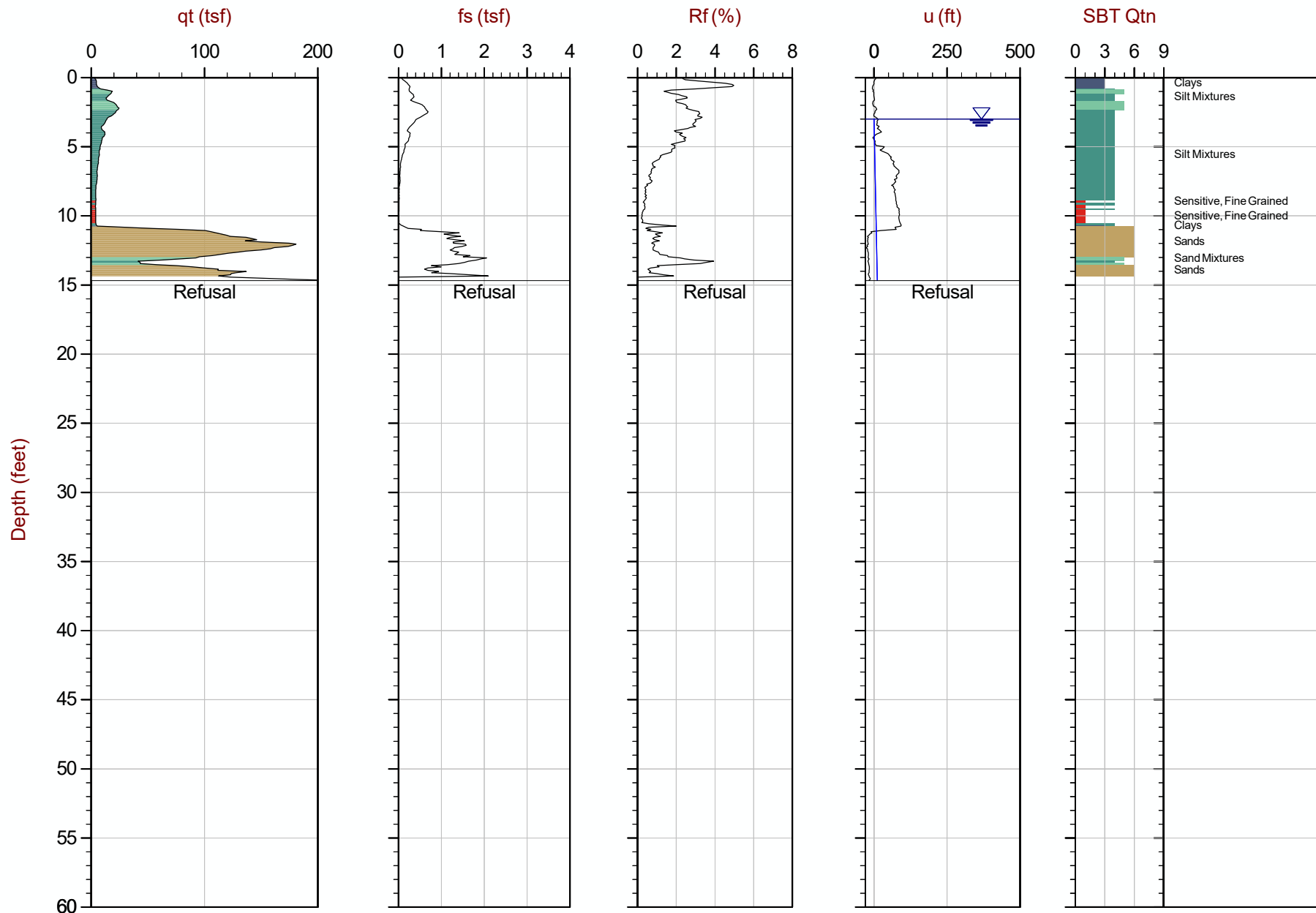
Job No: 20-53-21525

Date: 2020-10-30 11:30

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-123

Cone: 524:T375F10U500



Max Depth: 4.475 m / 14.68 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP123.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79816 ° Long: -68.68809 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

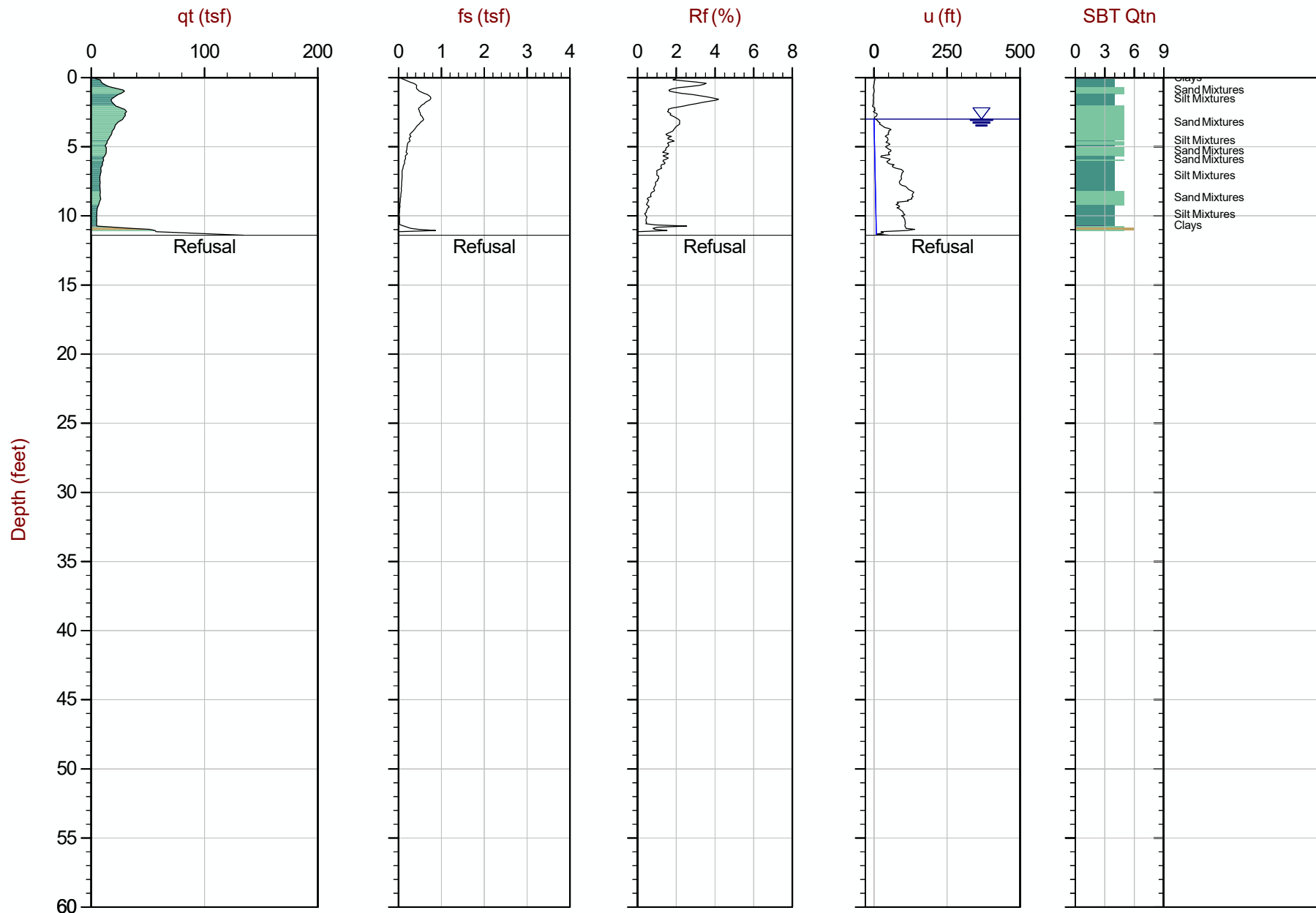
Job No: 20-53-21525

Date: 2020-10-30 10:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-124

Cone: 524:T375F10U500



Max Depth: 3.475 m / 11.40 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP124.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79850 ° Long: -68.68774 °

Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▲ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

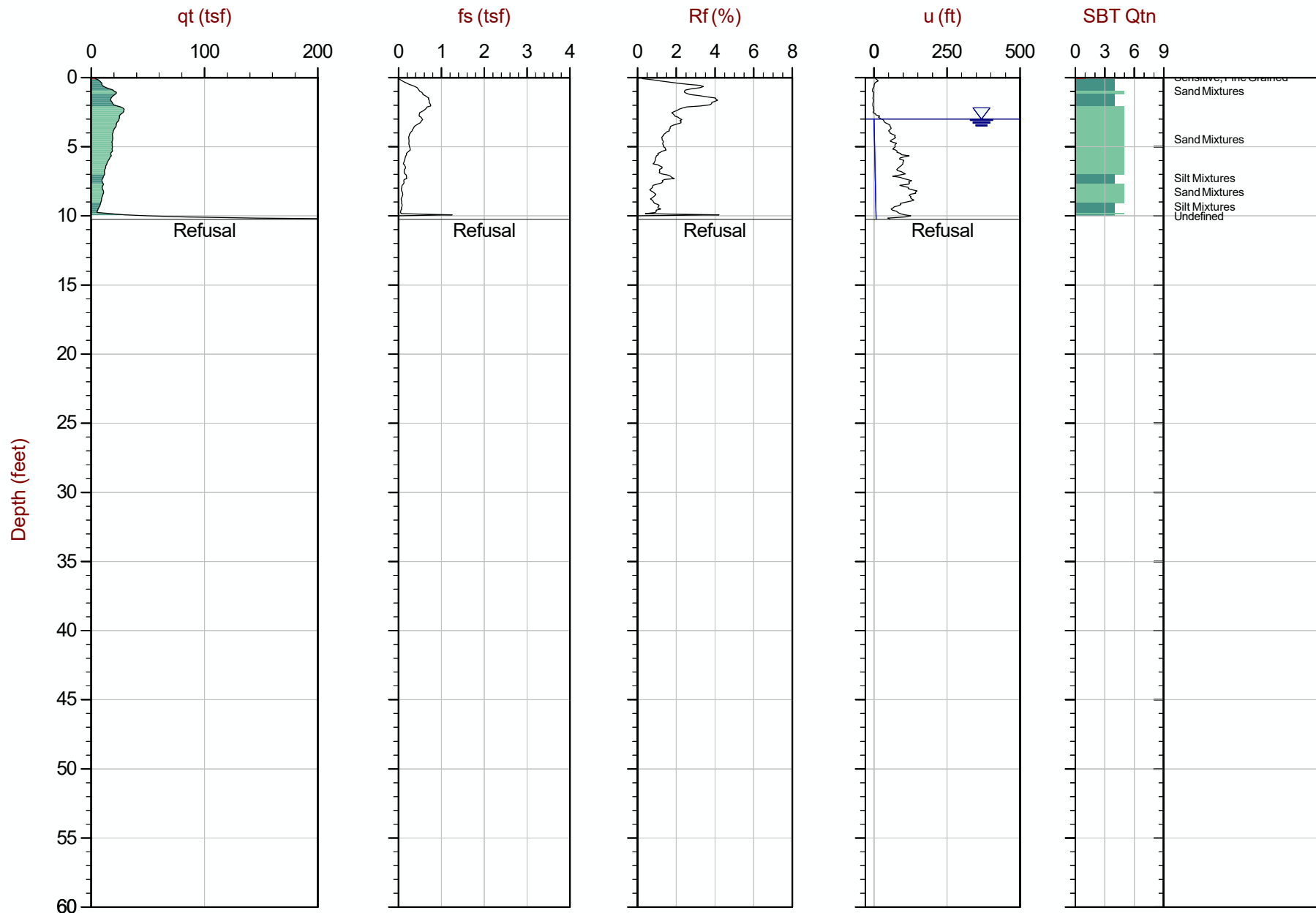
Job No: 20-53-21525

Date: 2020-10-30 10:15

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-125

Cone: 524:T375F10U500



Max Depth: 3.125 m / 10.25 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP125.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79925 ° Long: -68.68700 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

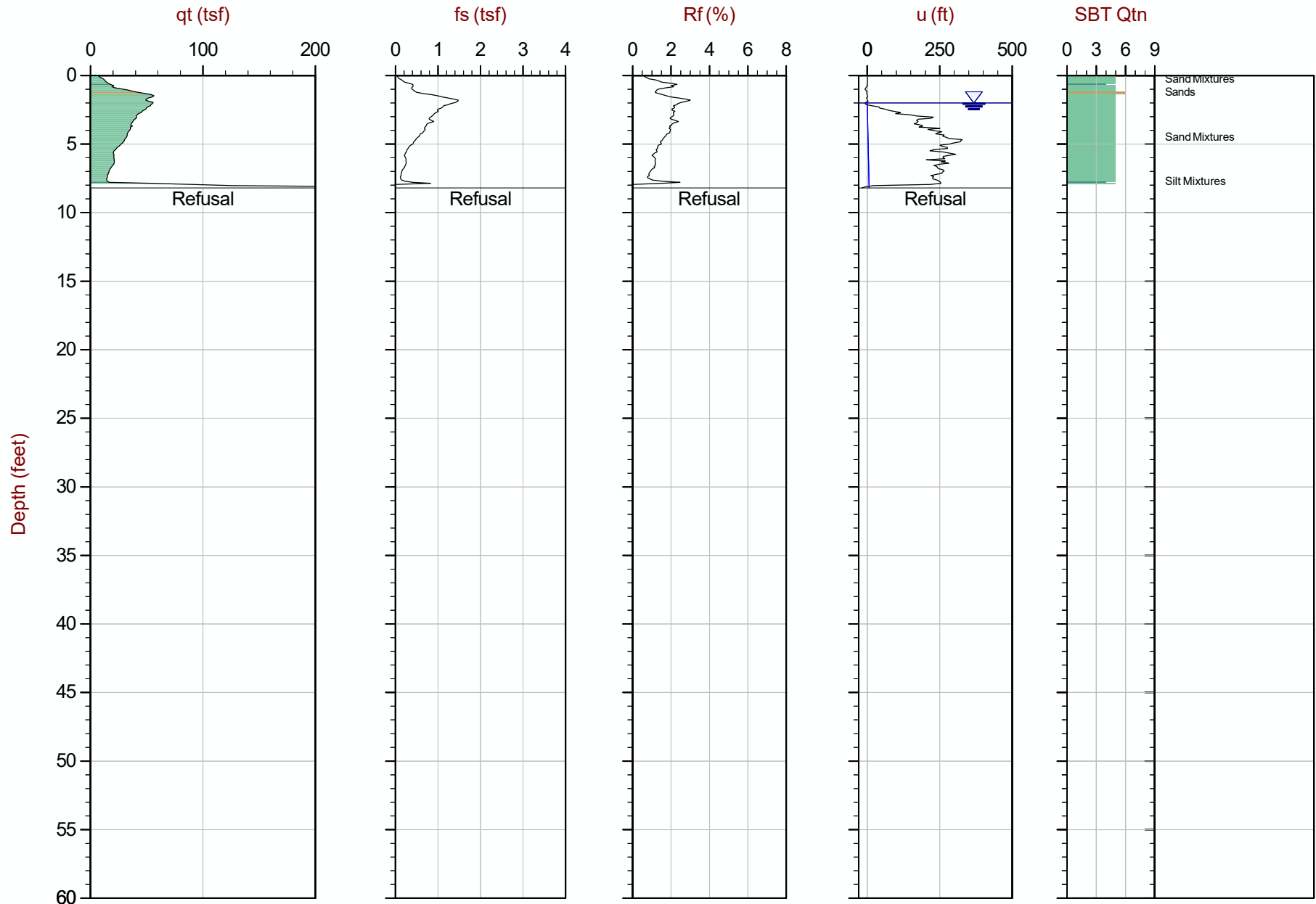
Job No: 20-53-21525

Date: 2020-10-30 09:44

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-126

Cone: 524:T375F10U500



Max Depth: 2.500 m / 8.20 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP126.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.80002 ° Long: -68.68636 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

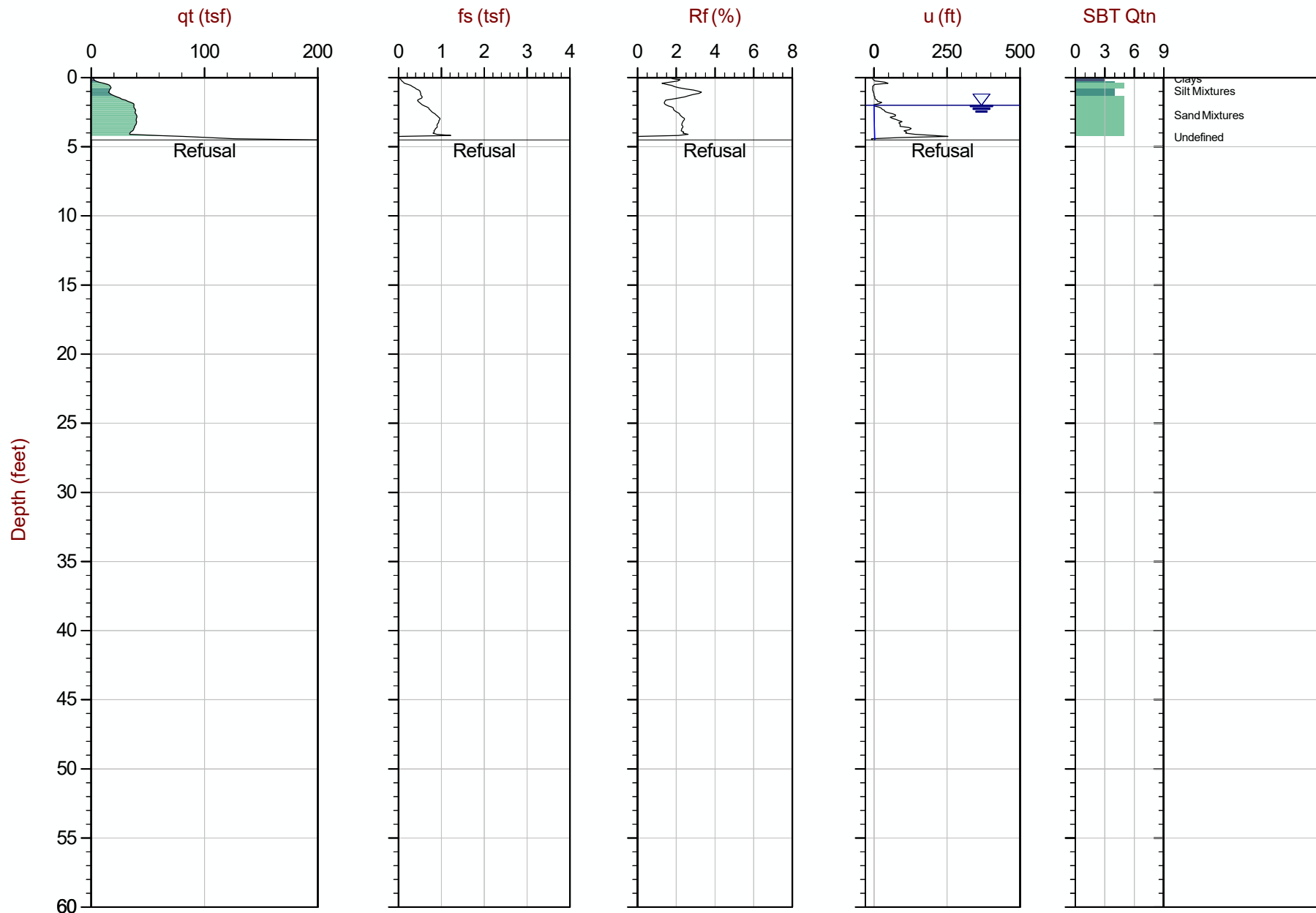
Job No: 20-53-21525

Date: 2020-10-30 09:11

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-127

Cone: 524:T375F10U500



Max Depth: 1.375 m / 4.51 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP127.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.80037 ° Long: -68.68603 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.

Advanced Cone Penetration Plots with I_c , $S_u(N_{kt})$, Φ and $N1(60)I_c$



Haley & Aldrich

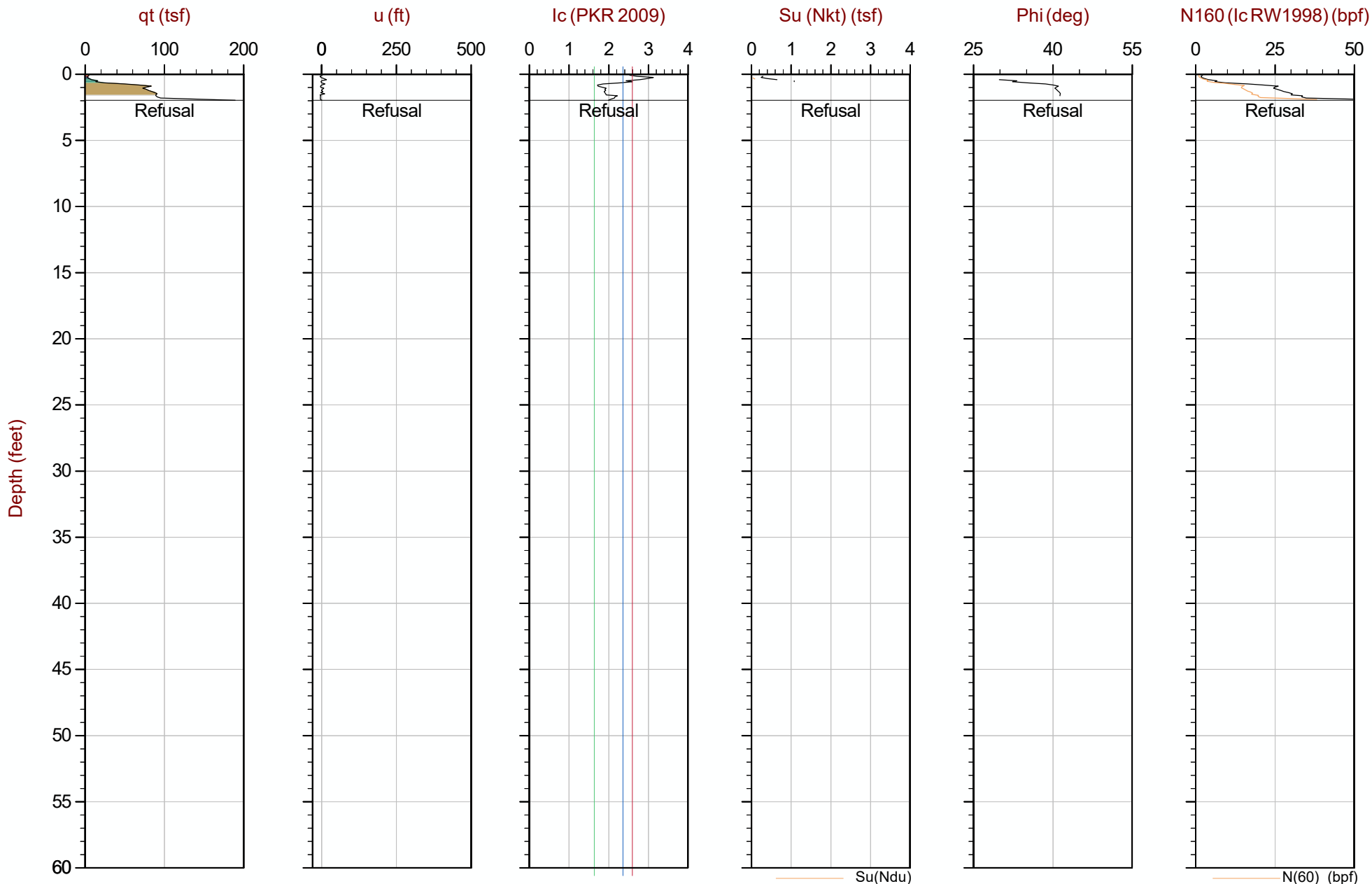
Job No: 20-53-21525

Date: 2020-11-01 07:33

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101

Cone: 524:T375F10U500



Max Depth: 0.600 m / 1.97 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP101.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77144 ° Long: -68.72026 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

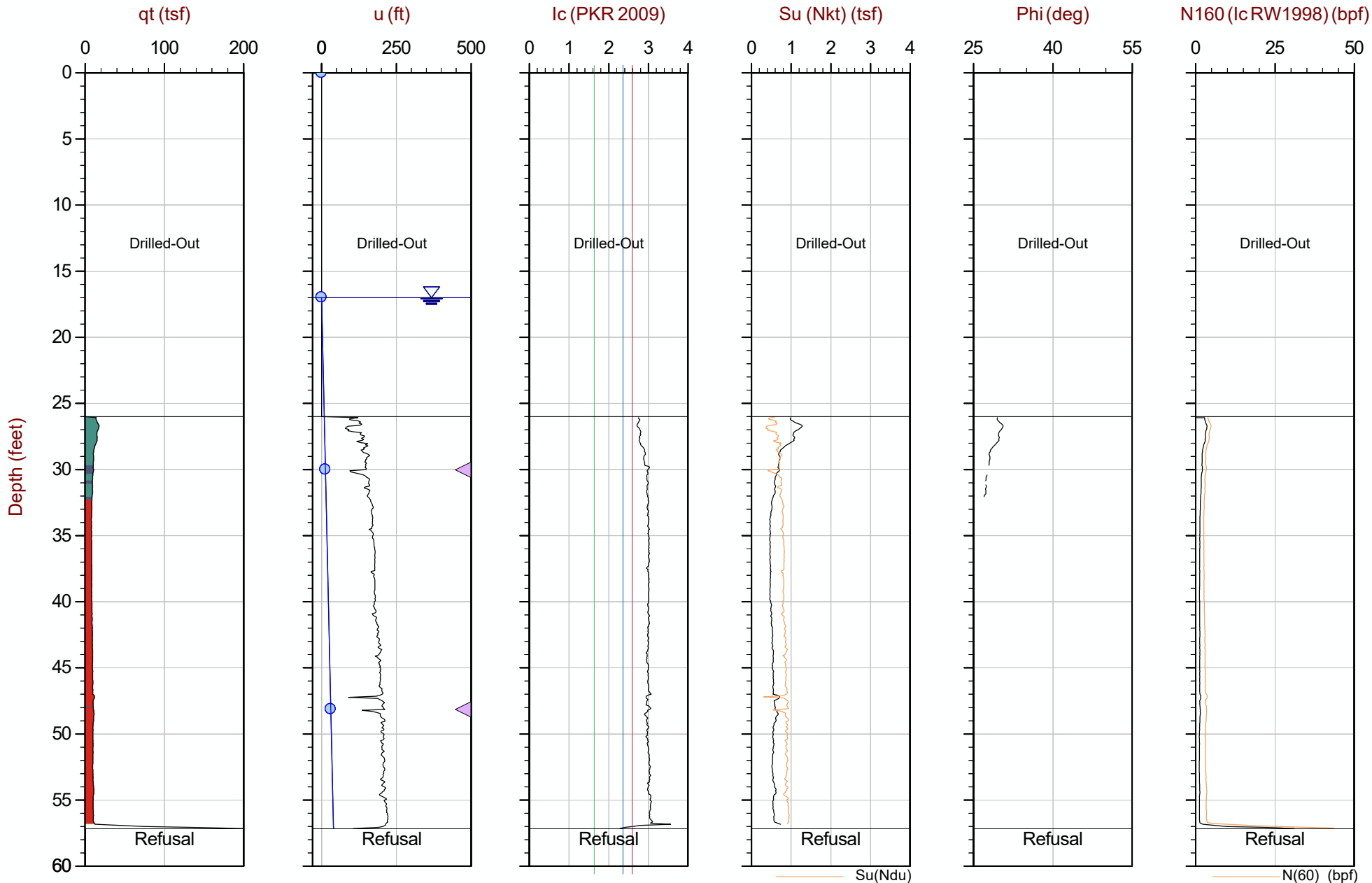
Job No: 20-53-21525

Date: 2020-11-02 10:10

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101B

Cone: 524:T375F10U500



Max Depth: 17.425 m / 57.17 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP101B.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77142 ° Long: -68.72029 °

Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▲ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

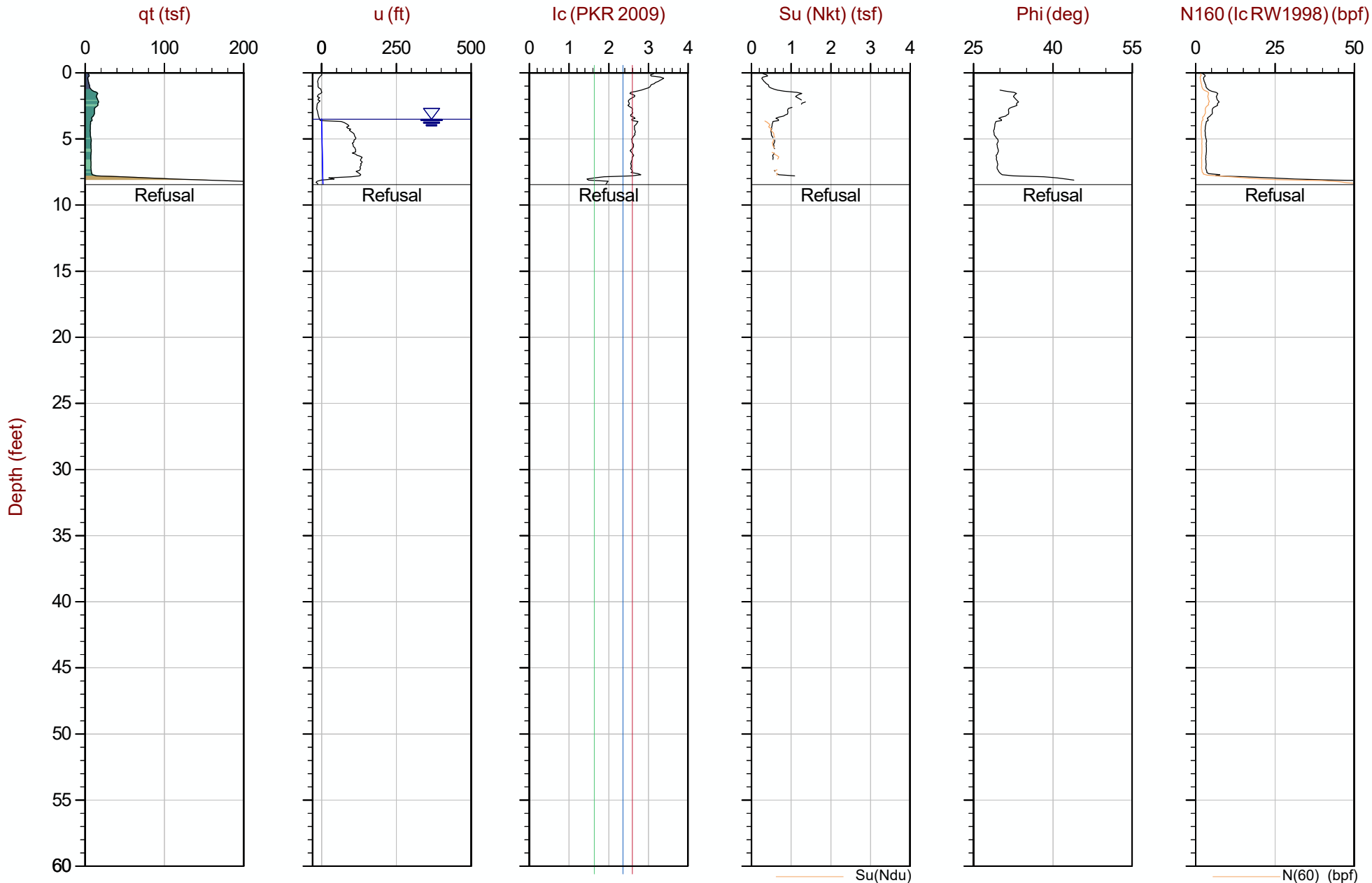
Job No: 20-53-21525

Date: 2020-10-27 14:05

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101

Cone: 524:T375F10U500



Max Depth: 2.575 m / 8.45 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP101.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78975 ° Long: -68.69922 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

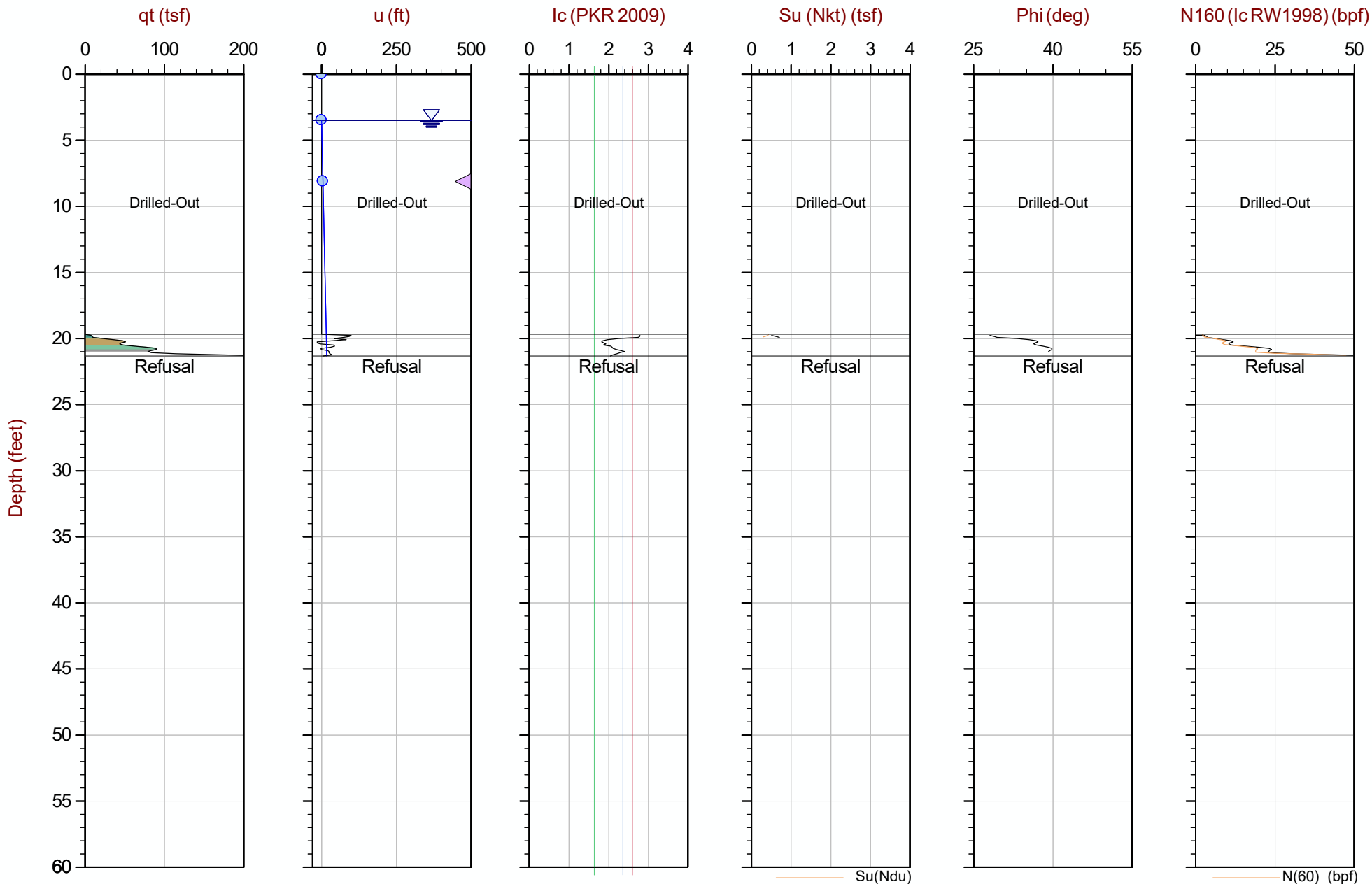
Job No: 20-53-21525

Date: 2020-10-28 08:27

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101B

Cone: 524:T375F10U500



Max Depth: 6.500 m / 21.33 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP101B.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78983 ° Long: -68.69899 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▼ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

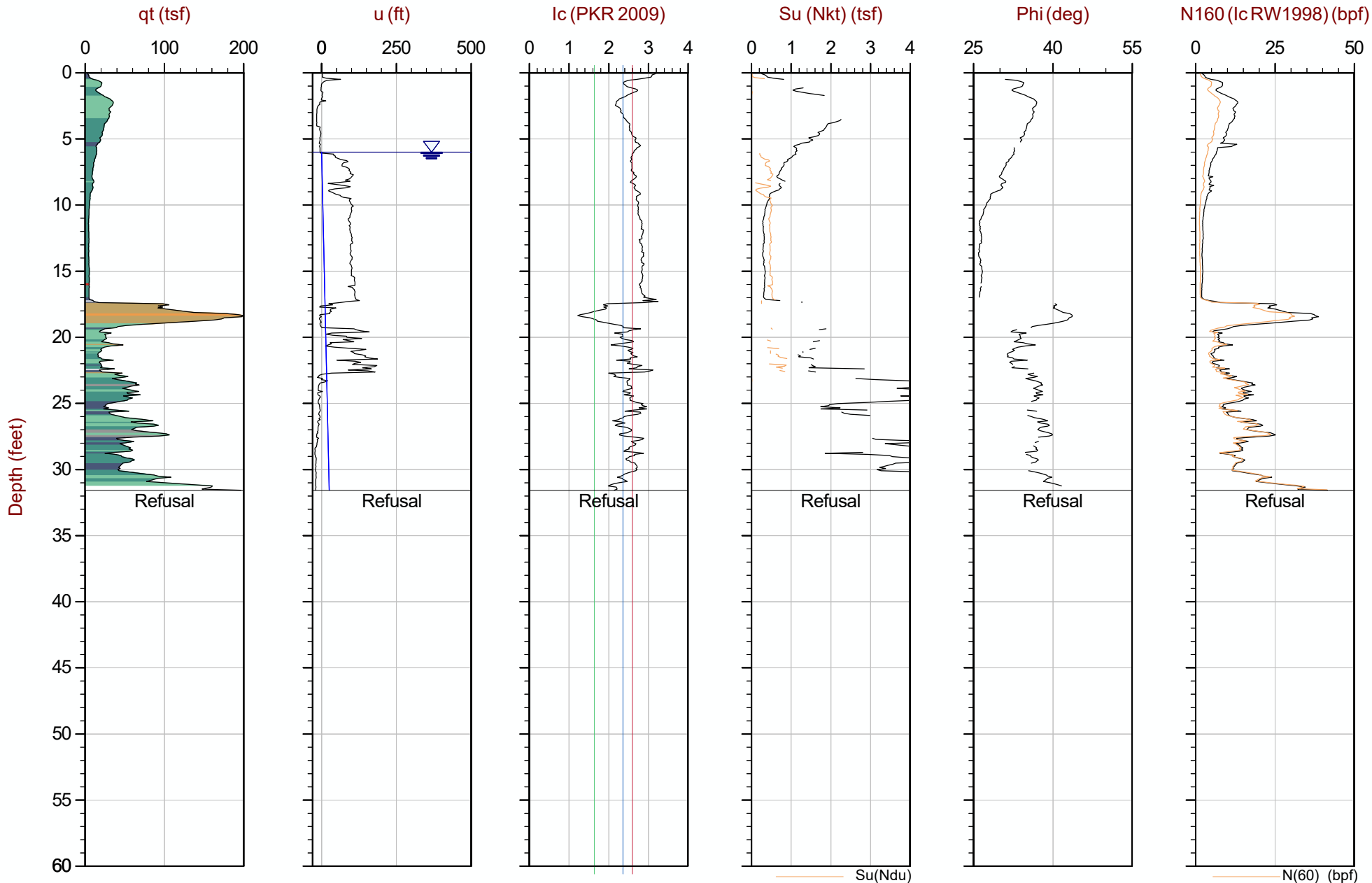
Job No: 20-53-21525

Date: 2020-10-29 08:44

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-102

Cone: 524:T375F10U500



Max Depth: 9.625 m / 31.58 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP102.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79006 ° Long: -68.69877 °

Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▼ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

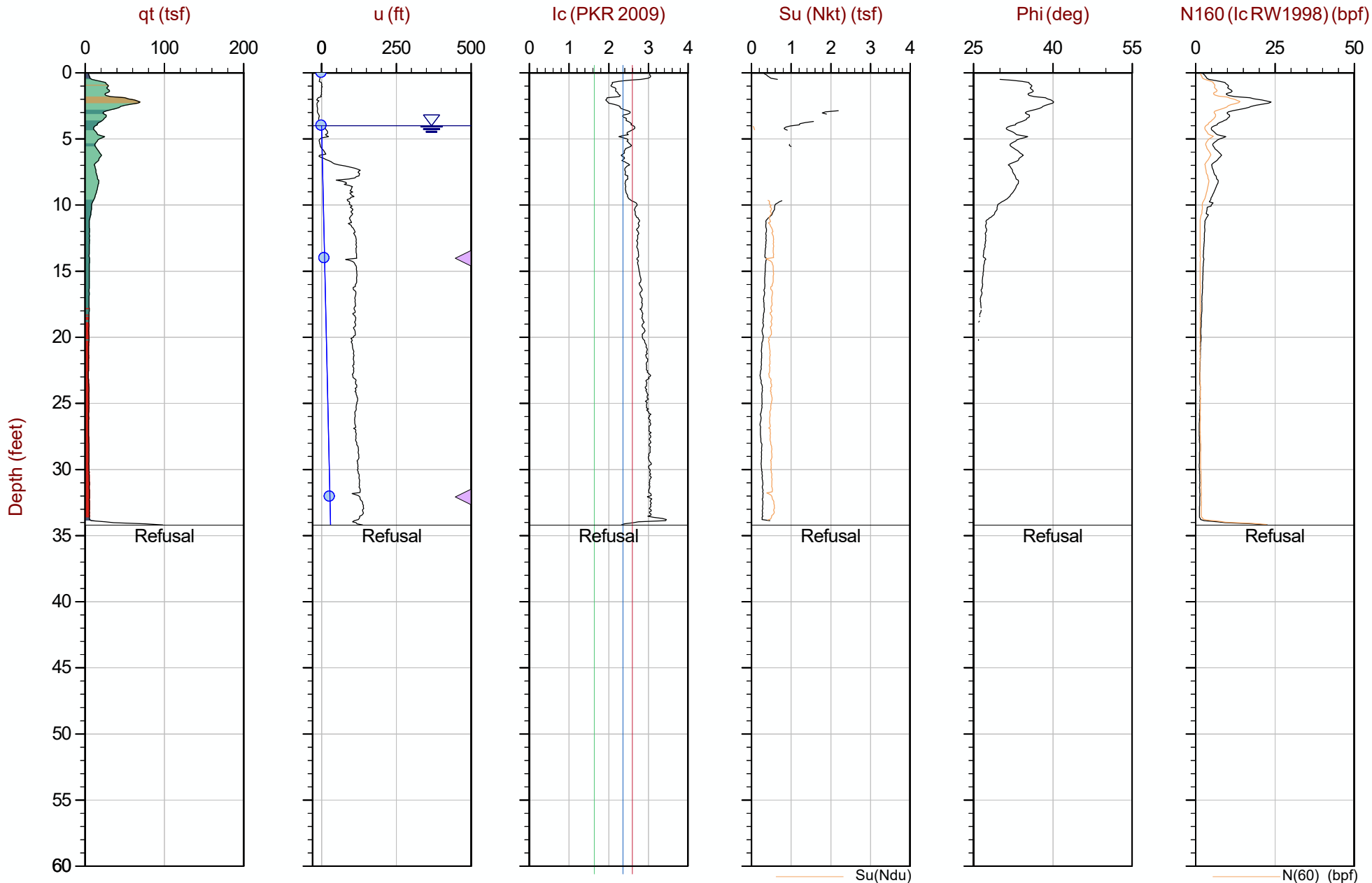
Job No: 20-53-21525

Date: 2020-11-01 12:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103

Cone: 524:T375F10U500



Max Depth: 10.425 m / 34.20 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 SP103.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77204 ° Long: -68.71887 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▼ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

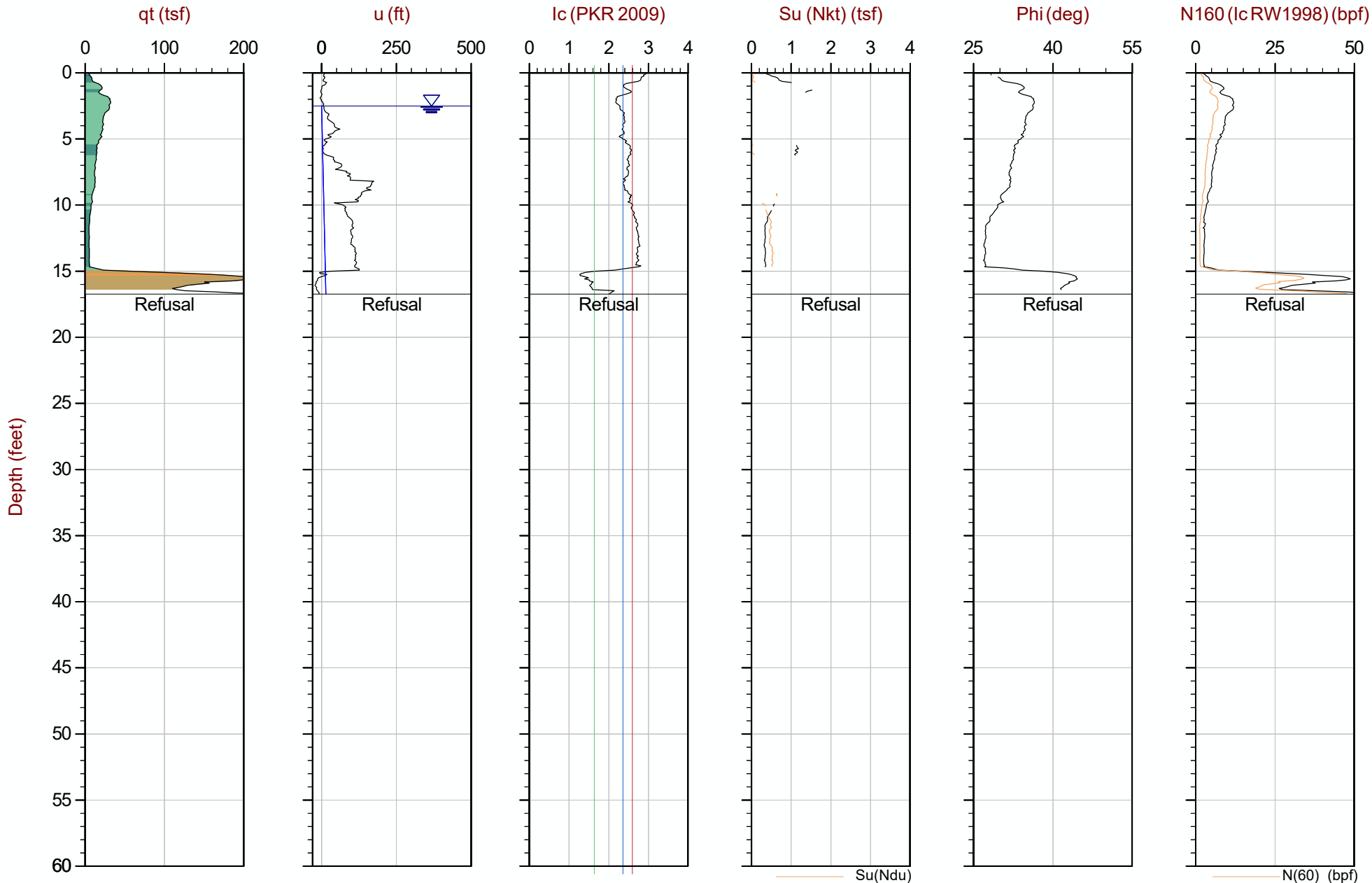
Job No: 20-53-21525

Date: 2020-11-01 12:07

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-104

Cone: 524:T375F10U500



Max Depth: 5.100 m / 16.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP104.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77203 ° Long: -68.71788 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

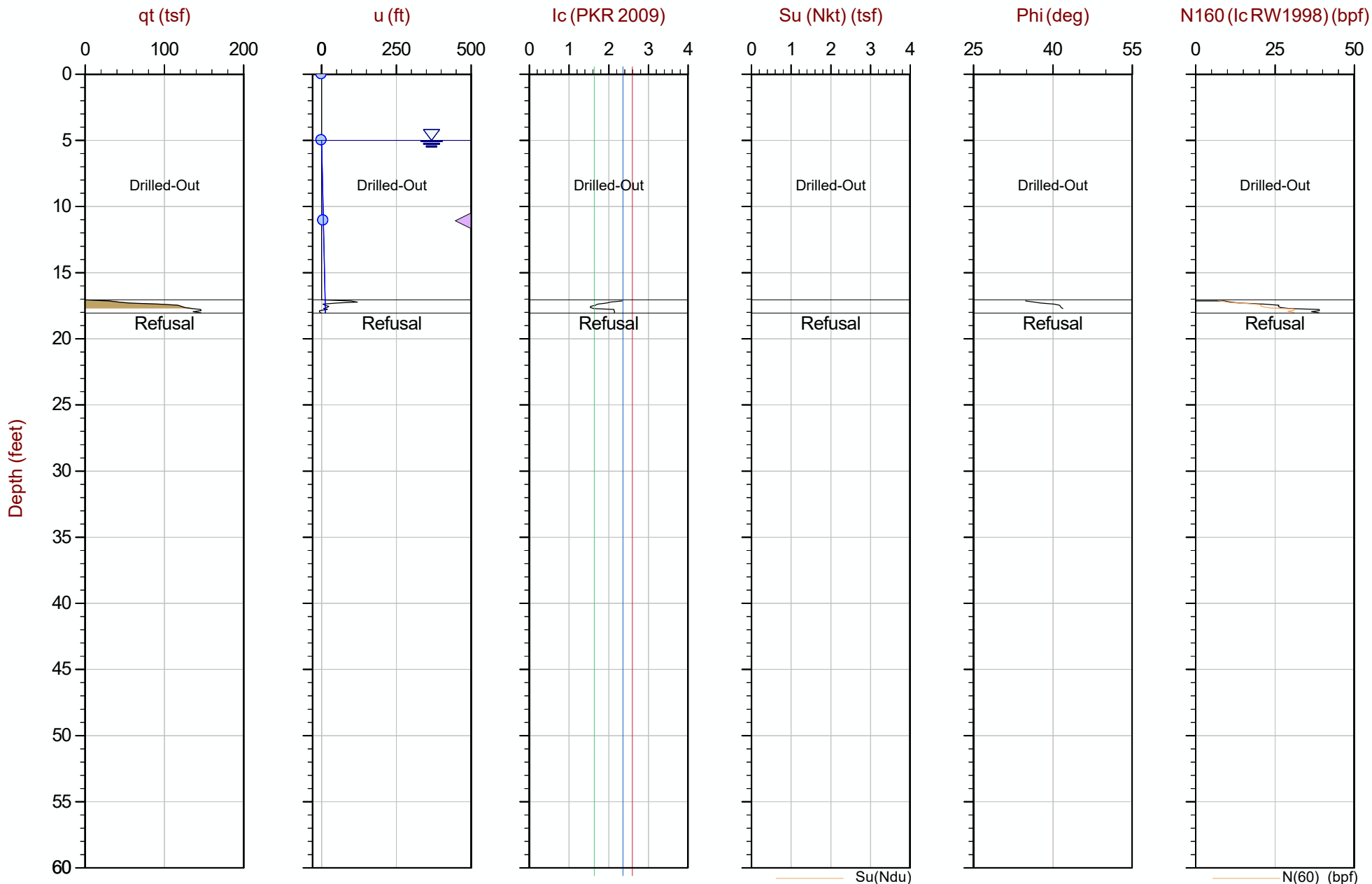
Job No: 20-53-21525

Date: 2020-11-01 08:08

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-104

Cone: 524:T375F10U500





Haley & Aldrich

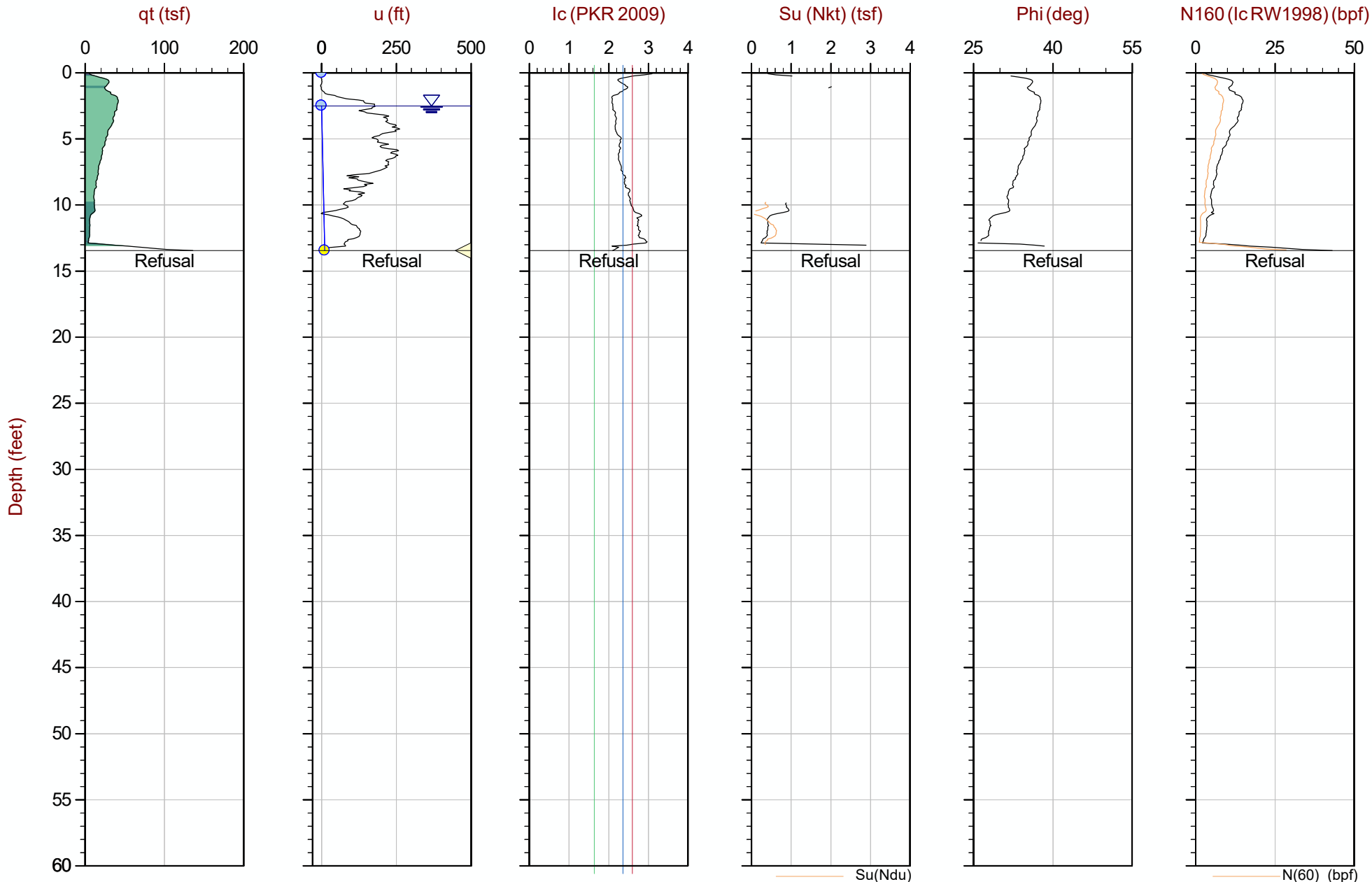
Job No: 20-53-21525

Date: 2020-11-01 10:13

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-105

Cone: 524:T375F10U500



Max Depth: 4.100 m / 13.45 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP105.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77231 ° Long: -68.71711 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

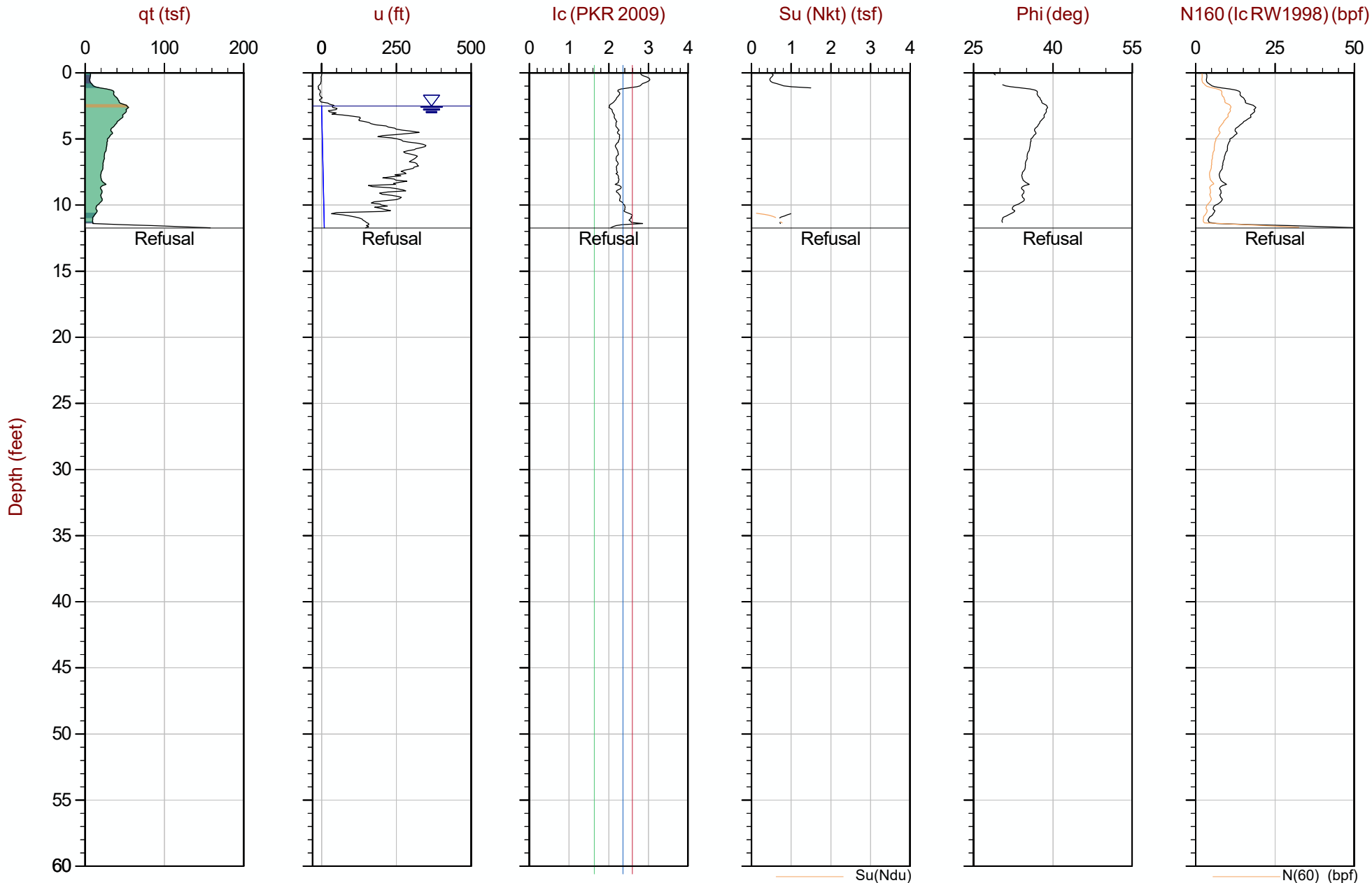
Job No: 20-53-21525

Date: 2020-11-01 09:22

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-106

Cone: 524:T375F10U500



Max Depth: 3.575 m / 11.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP106.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77124 ° Long: -68.71678 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

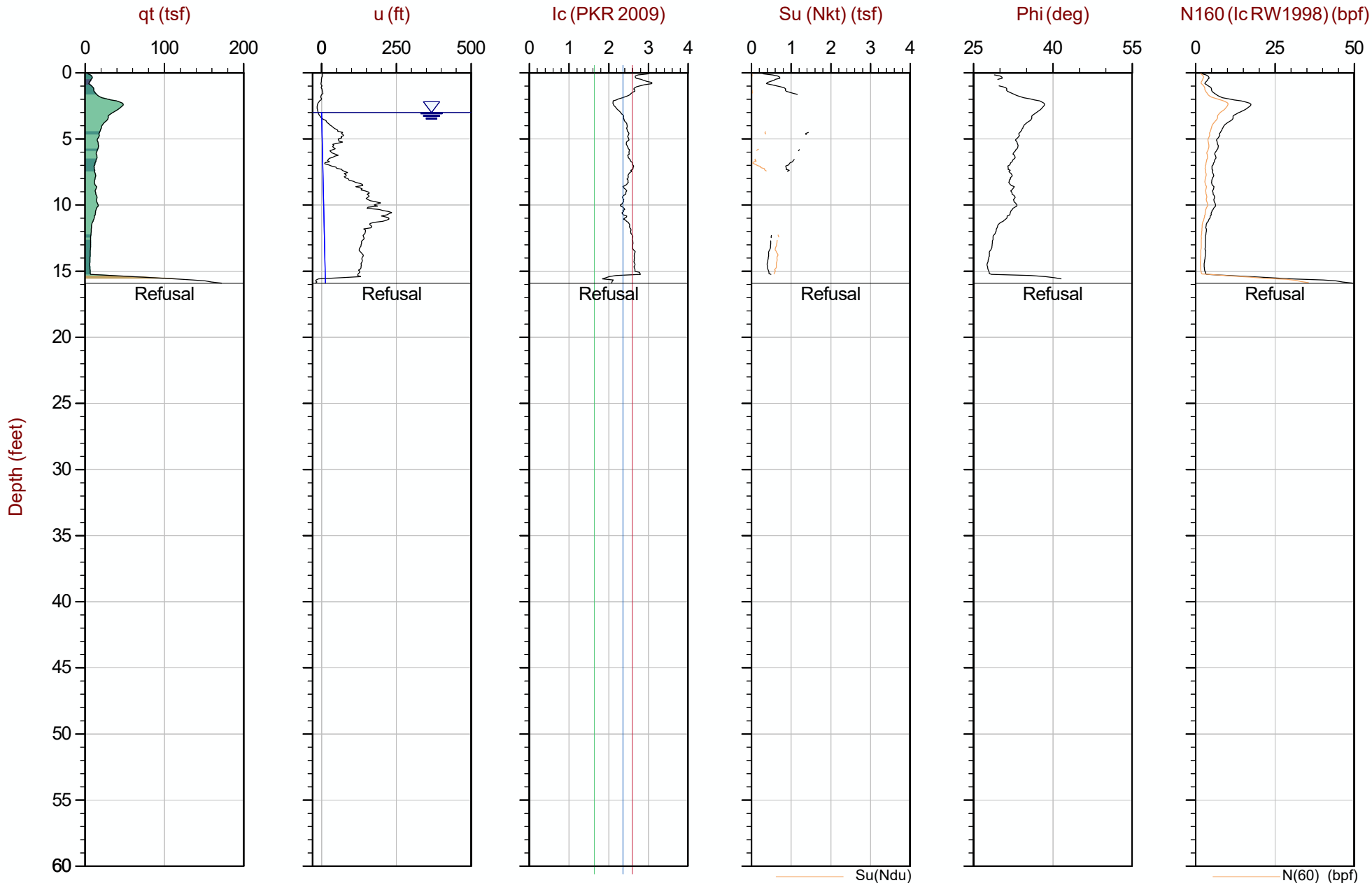
Job No: 20-53-21525

Date: 2020-11-01 11:13

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-108

Cone: 524:T375F10U500



Max Depth: 4.850 m / 15.91 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP108.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77305 ° Long: -68.71653 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

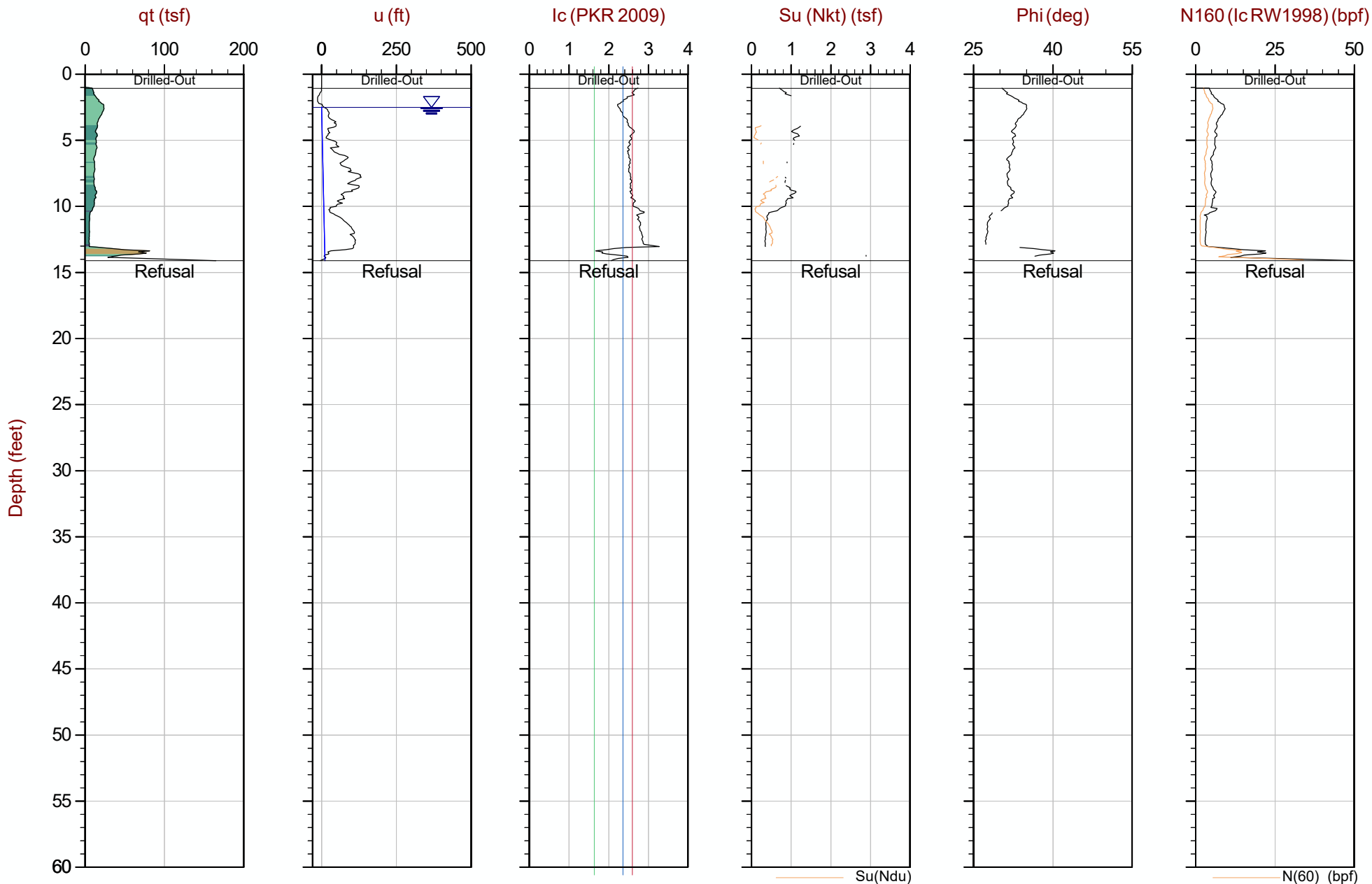
Job No: 20-53-21525

Date: 2020-10-26 09:58

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-109

Cone: 524:T375F10U500



Max Depth: 4.300 m / 14.11 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP109.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77371 ° Long: -68.71513 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

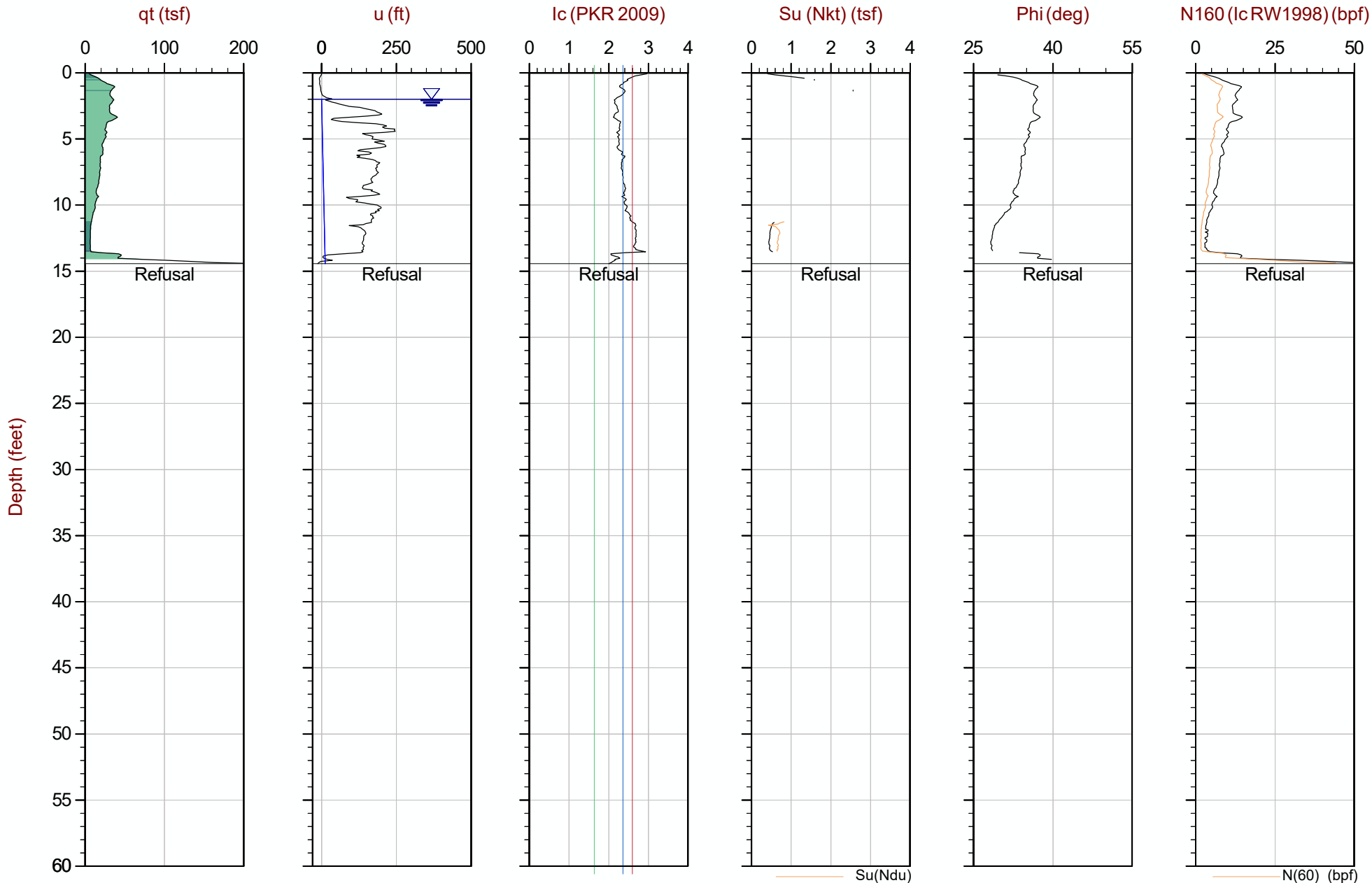
Job No: 20-53-21525

Date: 2020-10-26 11:12

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-110

Cone: 524:T375F10U500



Max Depth: 4.400 m / 14.44 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP110.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77429 ° Long: -68.71409 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

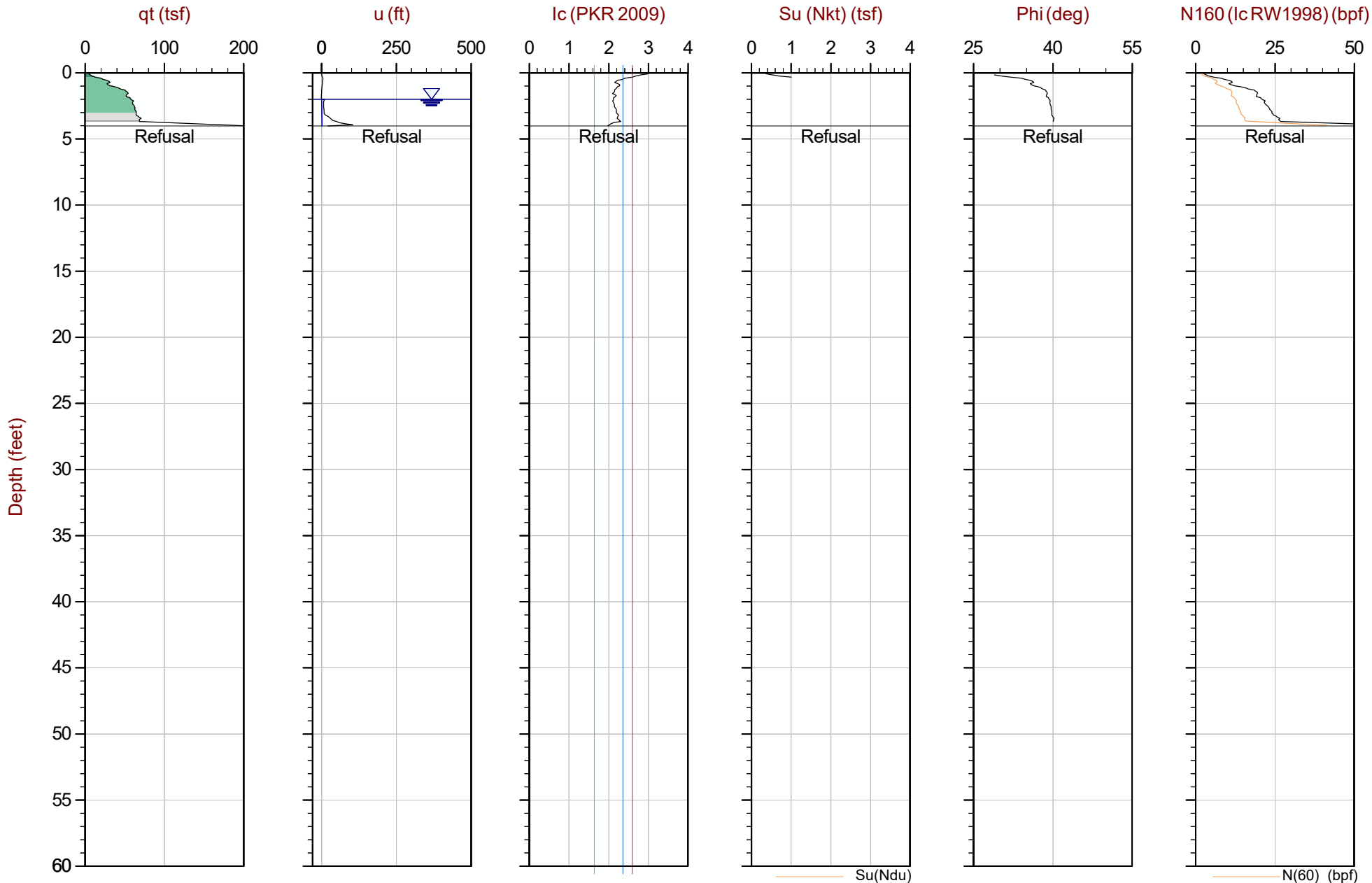
Job No: 20-53-21525

Date: 2020-10-26 12:14

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-111

Cone: 524:T375F10U500



Max Depth: 1.225 m / 4.02 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP111.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77508 ° Long: -68.71296 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

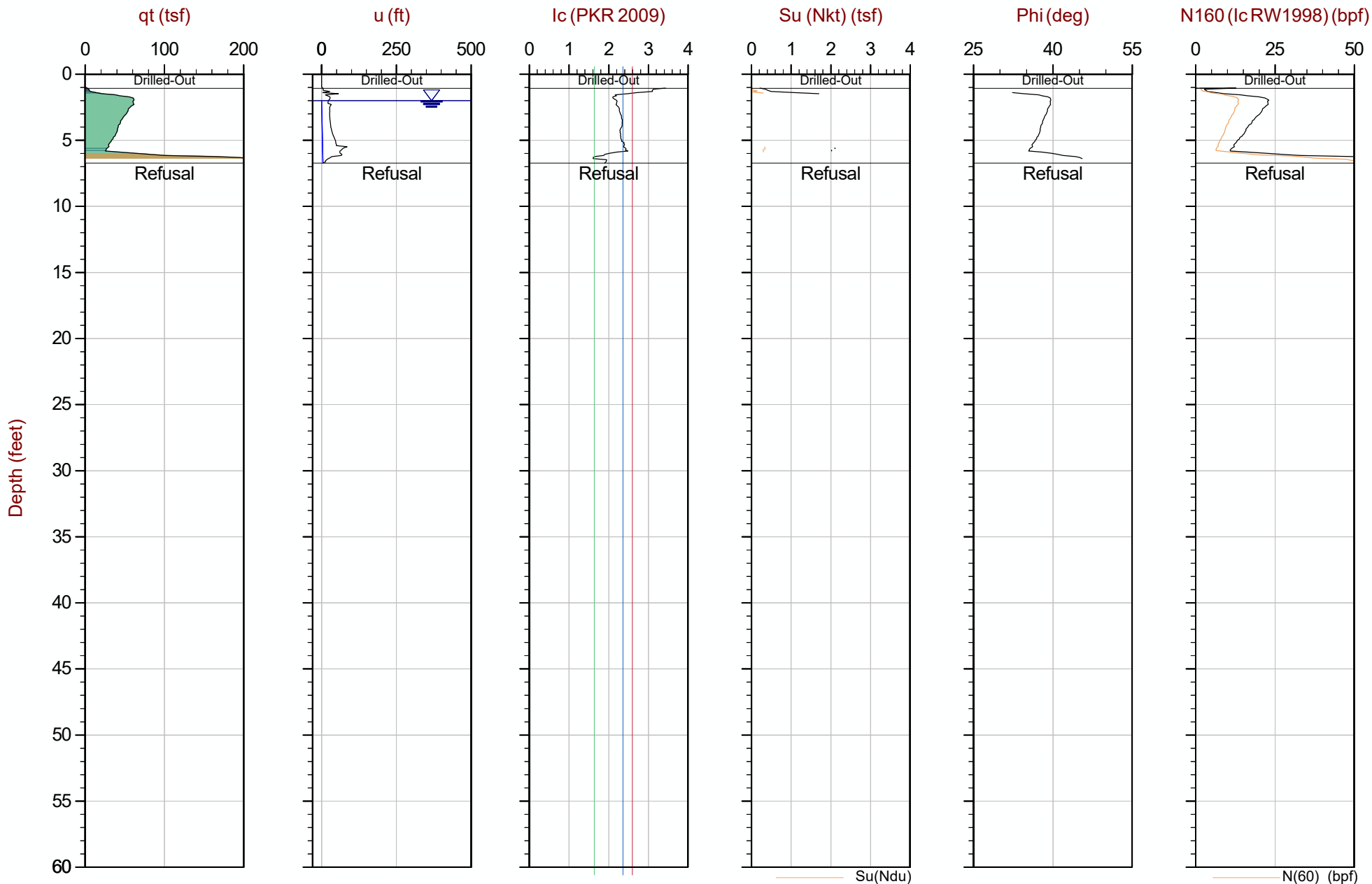
Job No: 20-53-21525

Date: 2020-10-26 12:35

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-111B

Cone: 524:T375F10U500



Max Depth: 2.050 m / 6.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP111B.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77509 ° Long: -68.71294 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

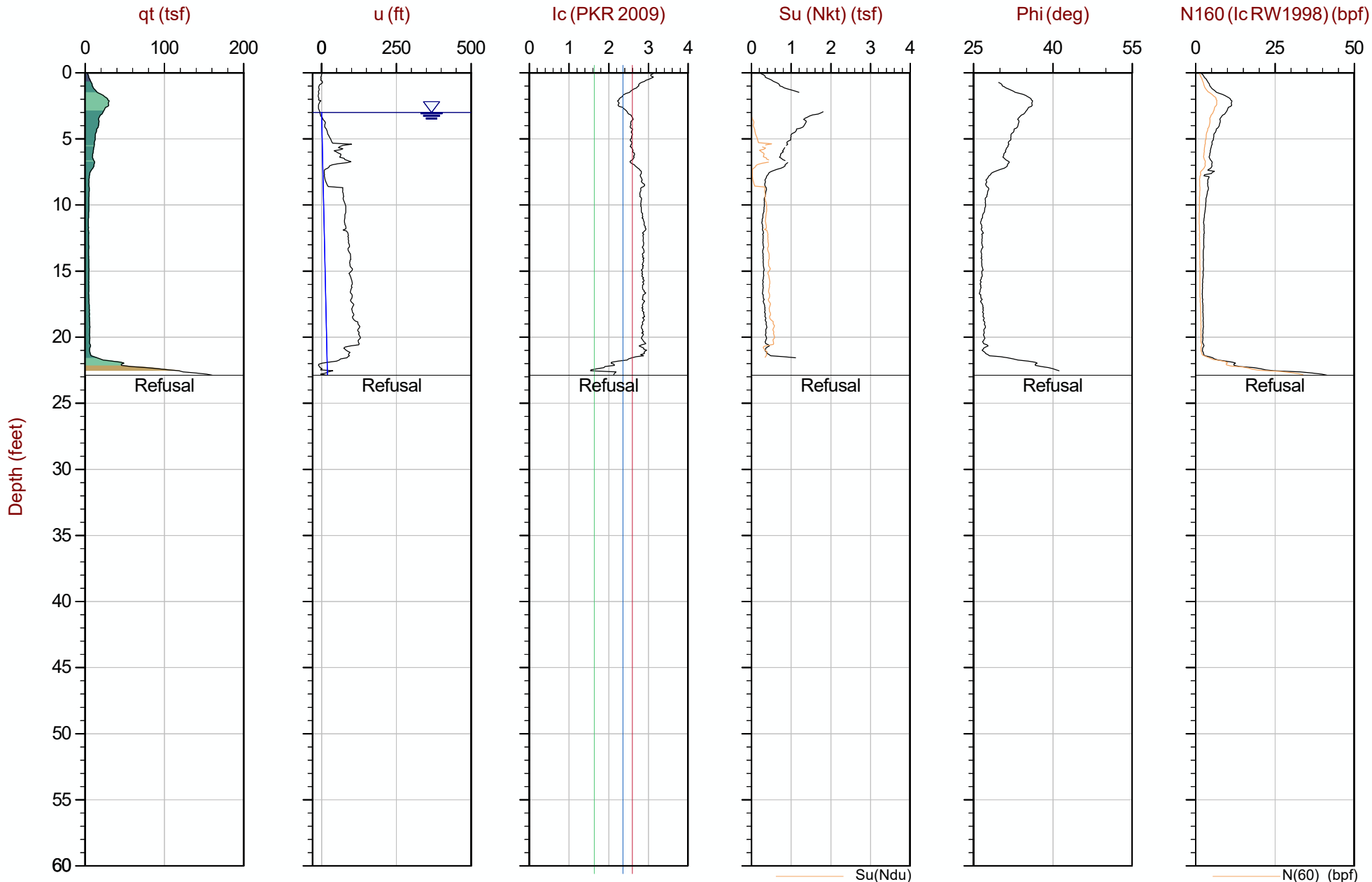
Job No: 20-53-21525

Date: 2020-10-26 13:32

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-112

Cone: 524:T375F10U500



Max Depth: 6.975 m / 22.88 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP112.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77594 ° Long: -68.71206 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

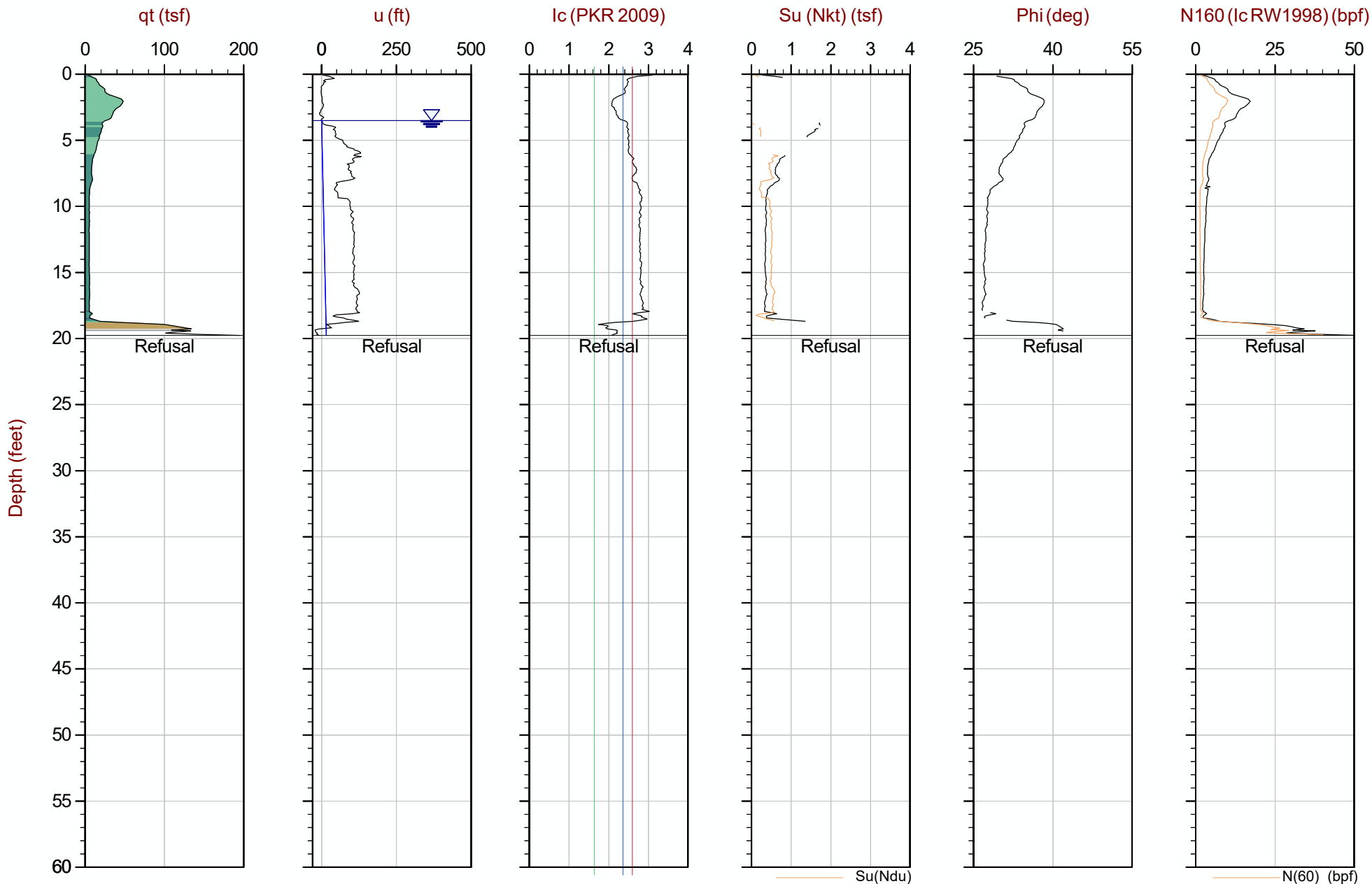
Job No: 20-53-21525

Date: 2020-10-26 14:50

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-113

Cone: 524:T375F10U500



Max Depth: 6.025 m / 19.77 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_CP113.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77673 ° Long: -68.71133 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

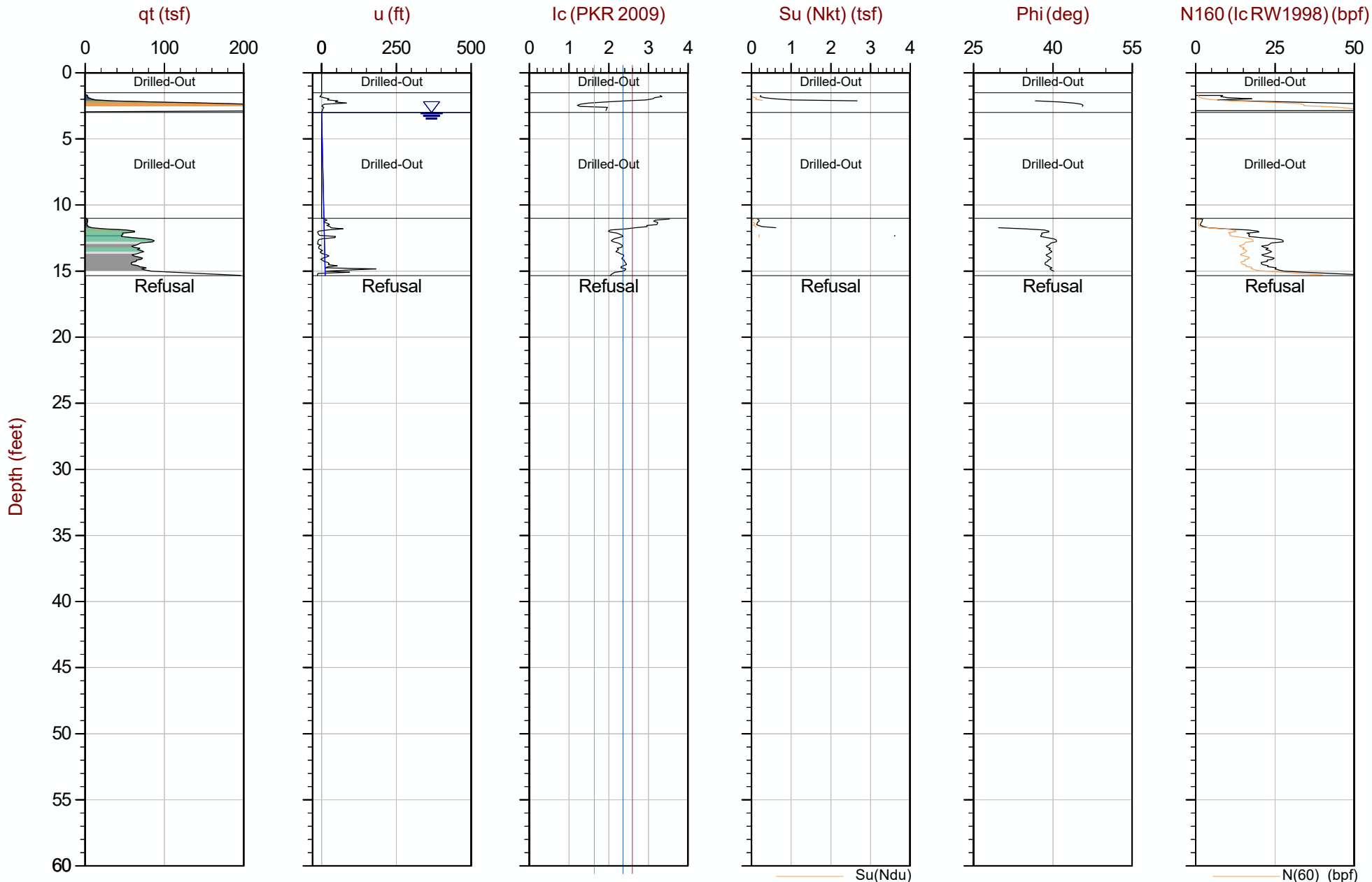
Job No: 20-53-21525

Date: 2020-10-27 10:25

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-114

Cone: 524:T375F10U500



Max Depth: 4.675 m / 15.34 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP114.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78939 ° Long: -68.69967 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

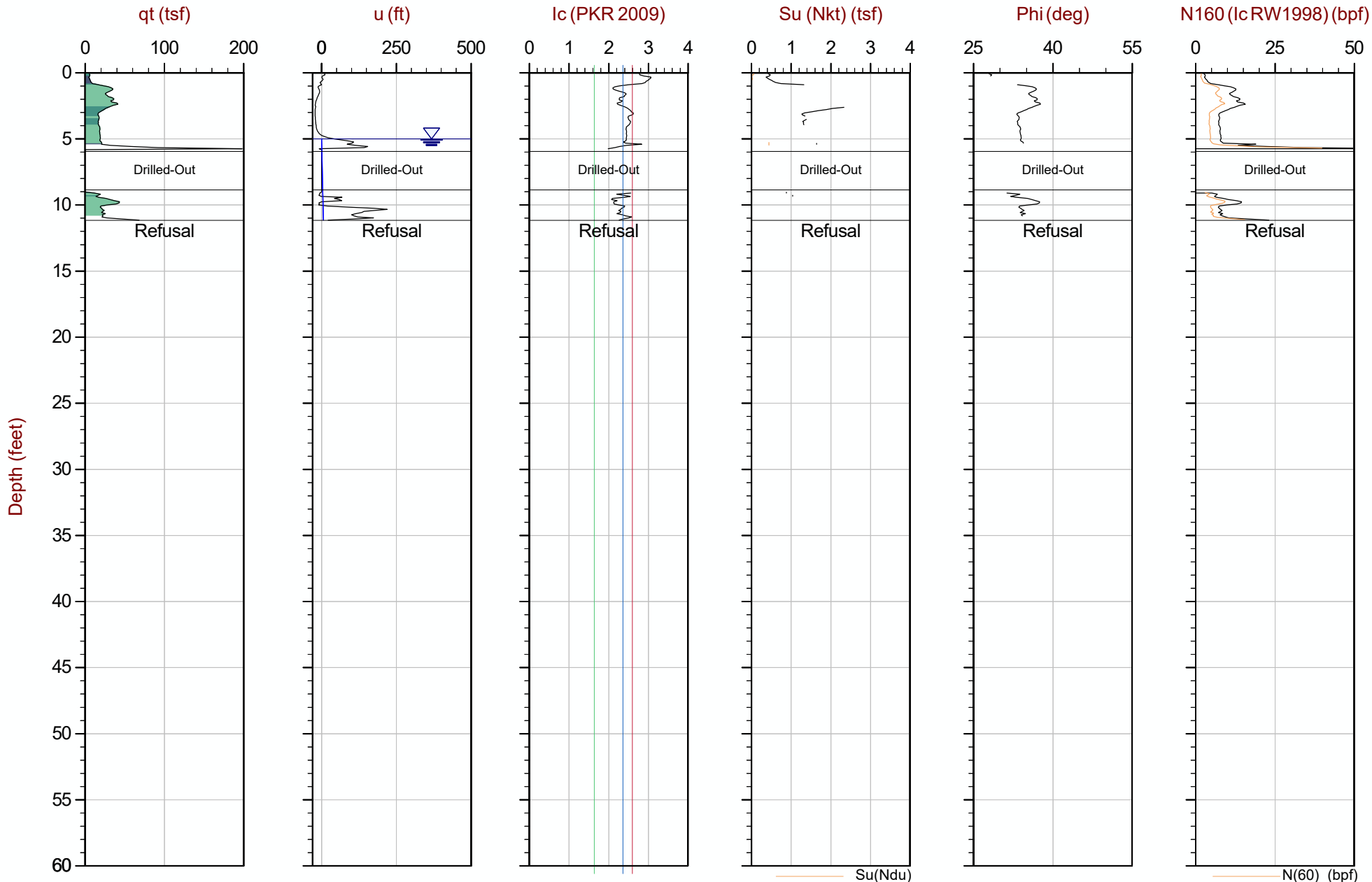
Job No: 20-53-21525

Date: 2020-10-27 12:04

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-115

Cone: 524:T375F10U500



Max Depth: 3.400 m / 11.15 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP115.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78969 ° Long: -68.69938 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

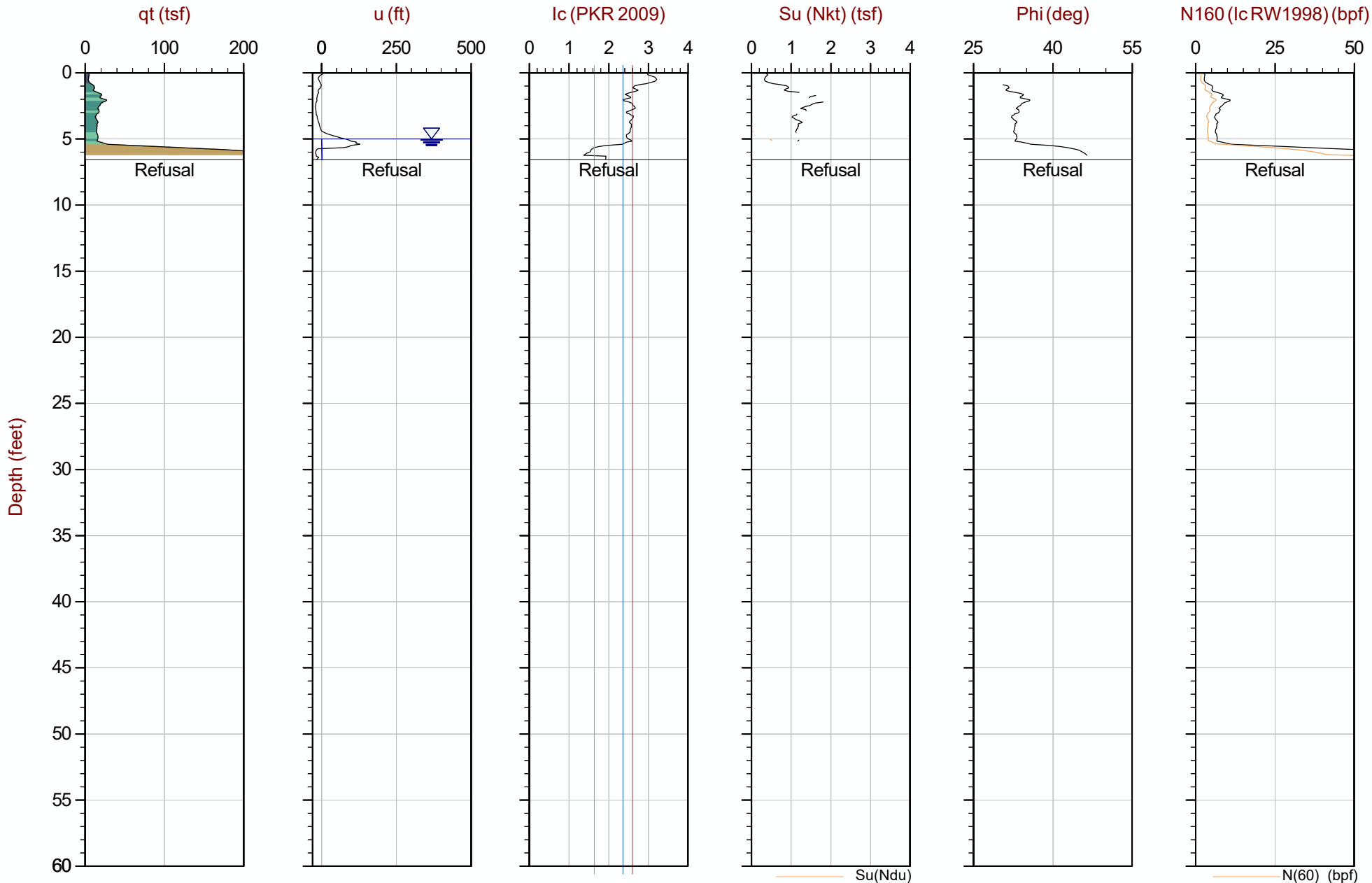
Job No: 20-53-21525

Date: 2020-10-27 13:16

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-115B

Cone: 524:T375F10U500



Max Depth: 2.000 m / 6.56 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP115B.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78971 ° Long: -68.69936 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

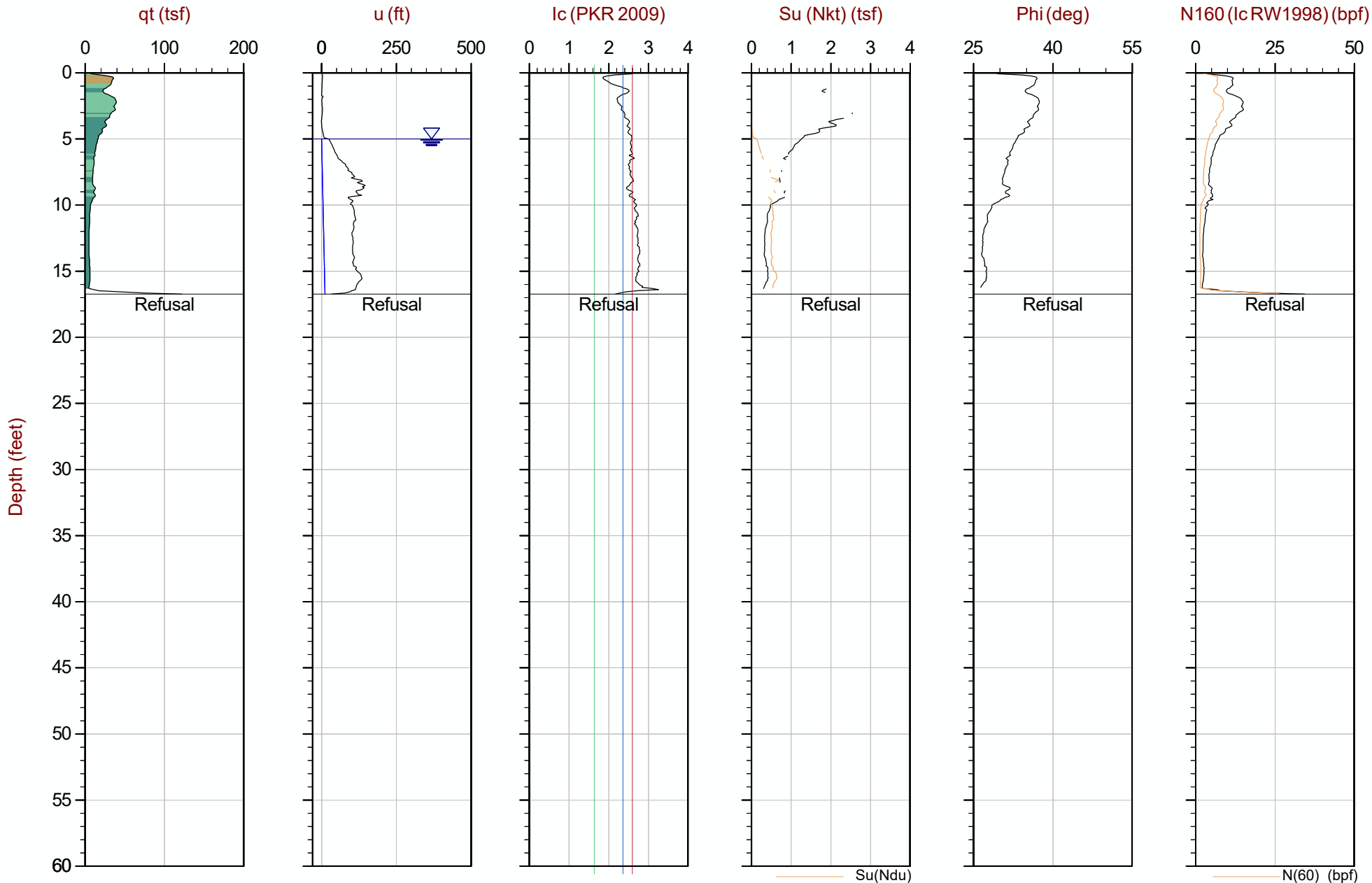
Job No: 20-53-21525

Date: 2020-10-29 10:24

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-116

Cone: 524:T375F10U500



Max Depth: 5.100 m / 16.73 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP116.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79045 ° Long: -68.69834 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

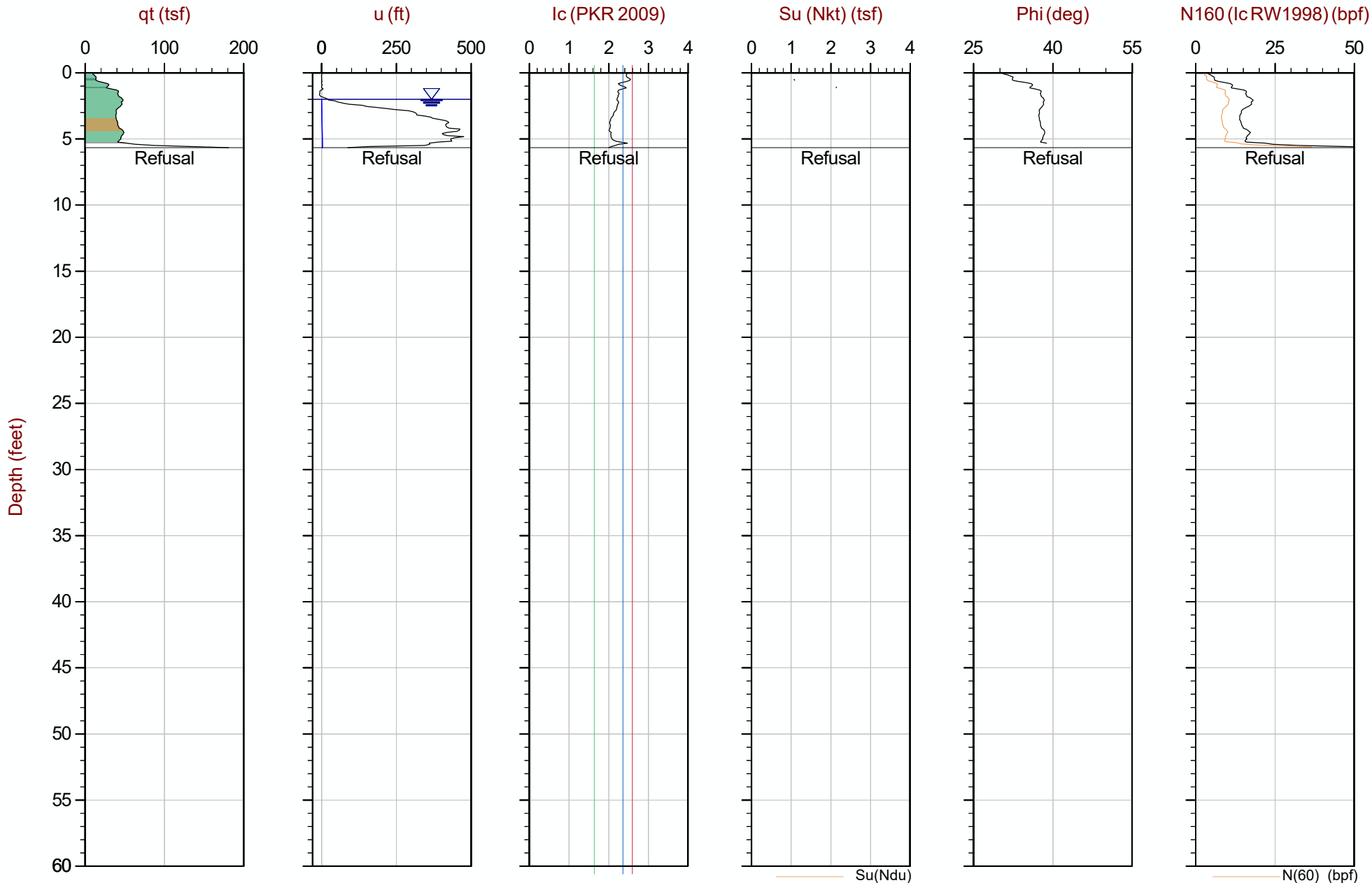
Job No: 20-53-21525

Date: 2020-10-29 11:34

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-117

Cone: 524:T375F10U500



Max Depth: 1.725 m / 5.66 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP117.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79429 ° Long: -68.69303 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

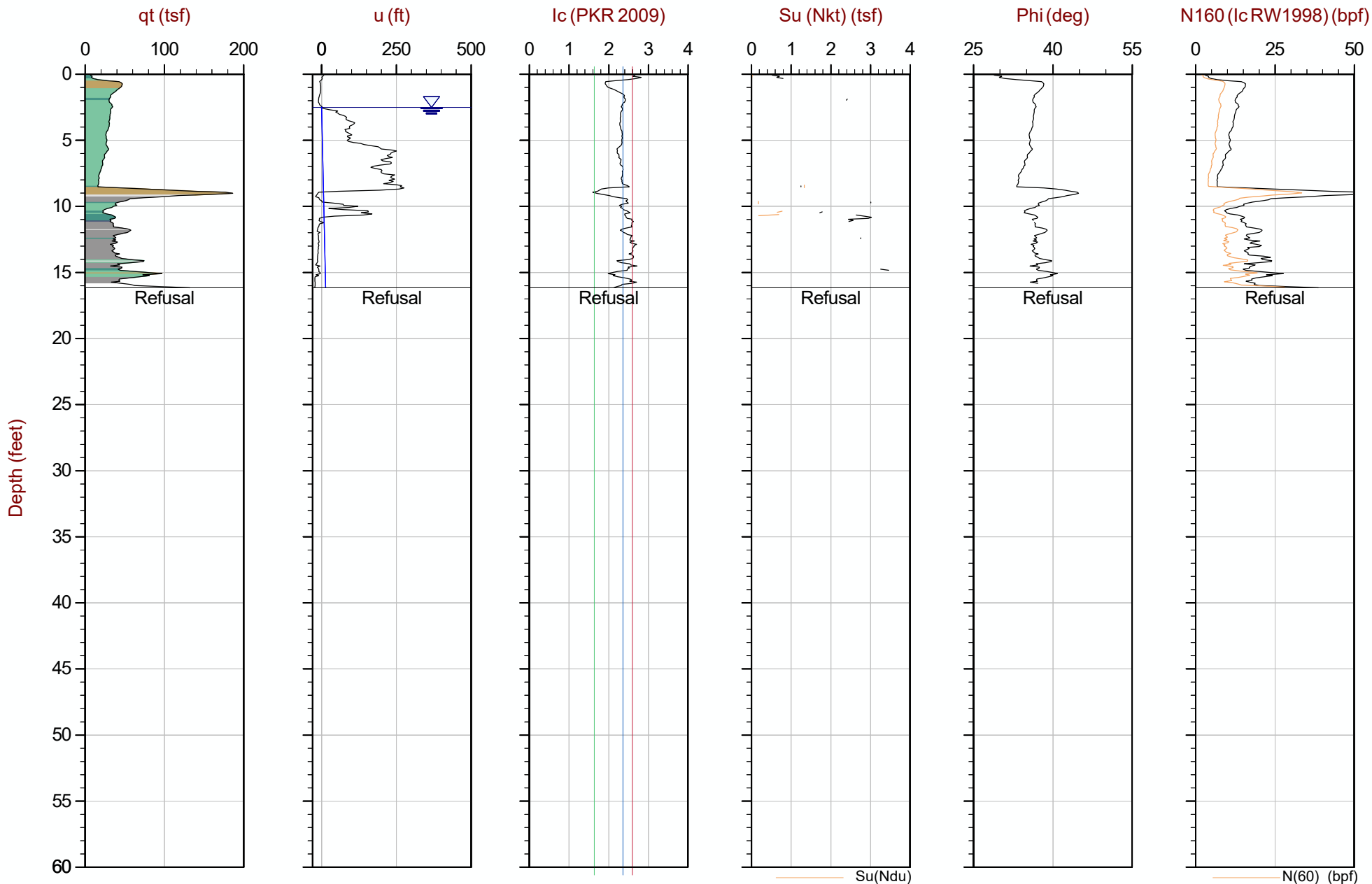
Job No: 20-53-21525

Date: 2020-10-29 12:05

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-118

Cone: 524:T375F10U500



Max Depth: 4.925 m / 16.16 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP118.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79470 ° Long: -68.69252 °

— Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▼ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

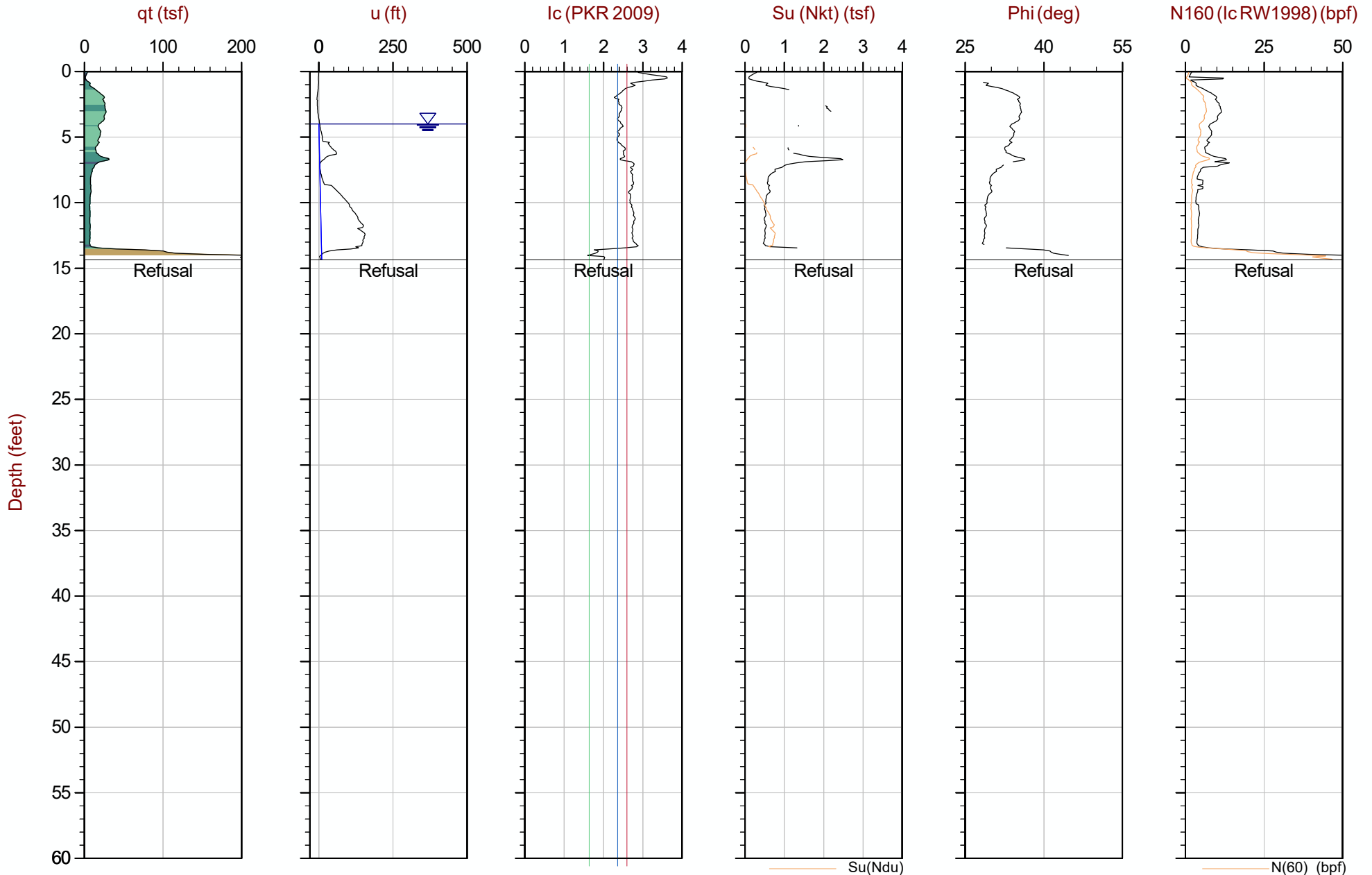
Job No: 20-53-21525

Date: 2020-10-29 13:17

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-119

Cone: 524:T375F10U500



Max Depth: 4.375 m / 14.35 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP119.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79490 ° Long: -68.69217 °

Hydrostatic Line ● Ueq ● Assumed Ueq ▲ PPD, Ueq achieved ▼ PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

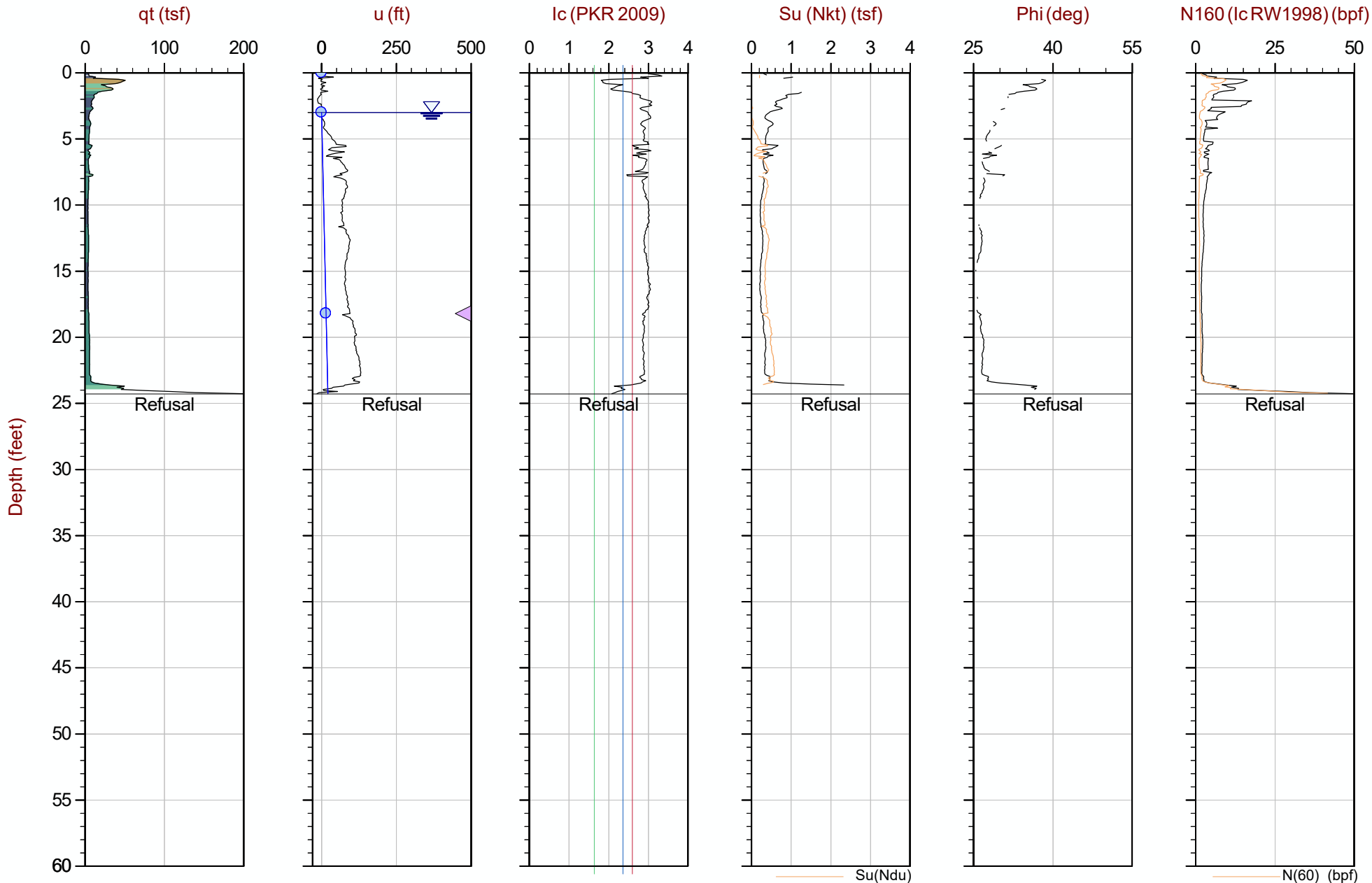
Job No: 20-53-21525

Date: 2020-10-30 12:58

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-122

Cone: 524:T375F10U500



Max Depth: 7.400 m / 24.28 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP122.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79780 ° Long: -68.68837 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

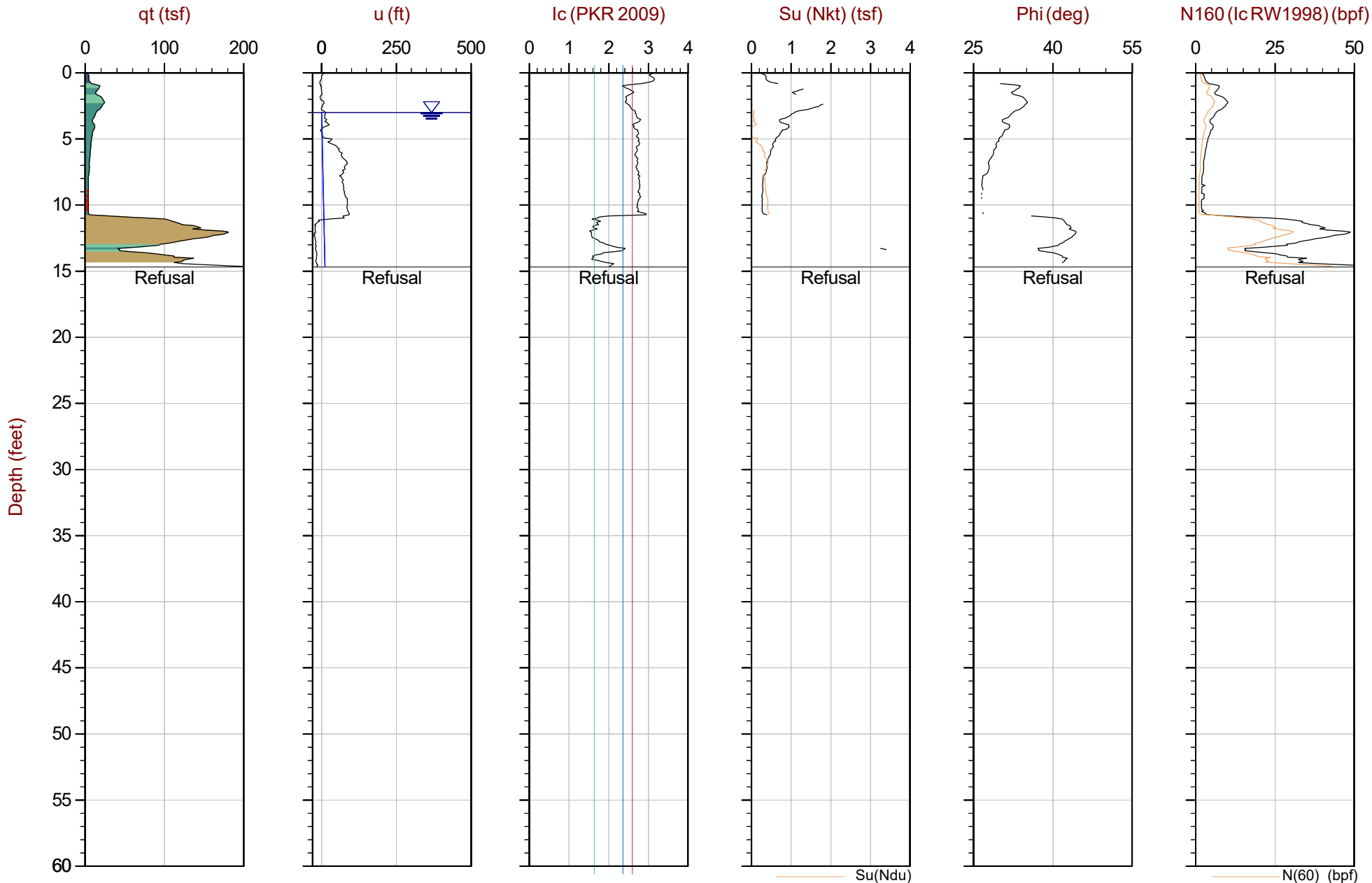
Job No: 20-53-21525

Date: 2020-10-30 11:30

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-123

Cone: 524:T375F10U500



Max Depth: 4.475 m / 14.68 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP123.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79816 ° Long: -68.68809 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

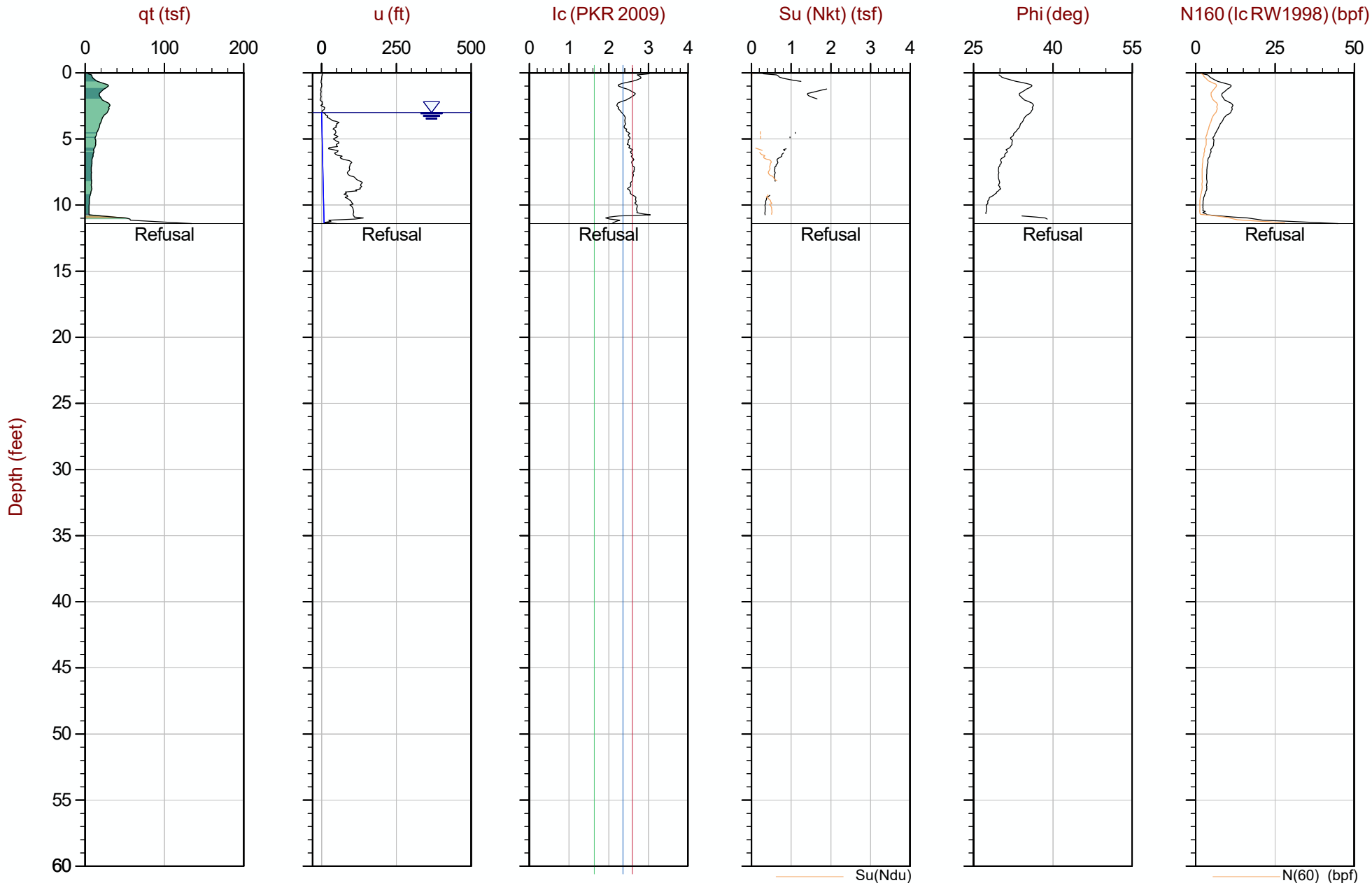
Job No: 20-53-21525

Date: 2020-10-30 10:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-124

Cone: 524:T375F10U500



Max Depth: 3.475 m / 11.40 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP124.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79850 ° Long: -68.68774 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

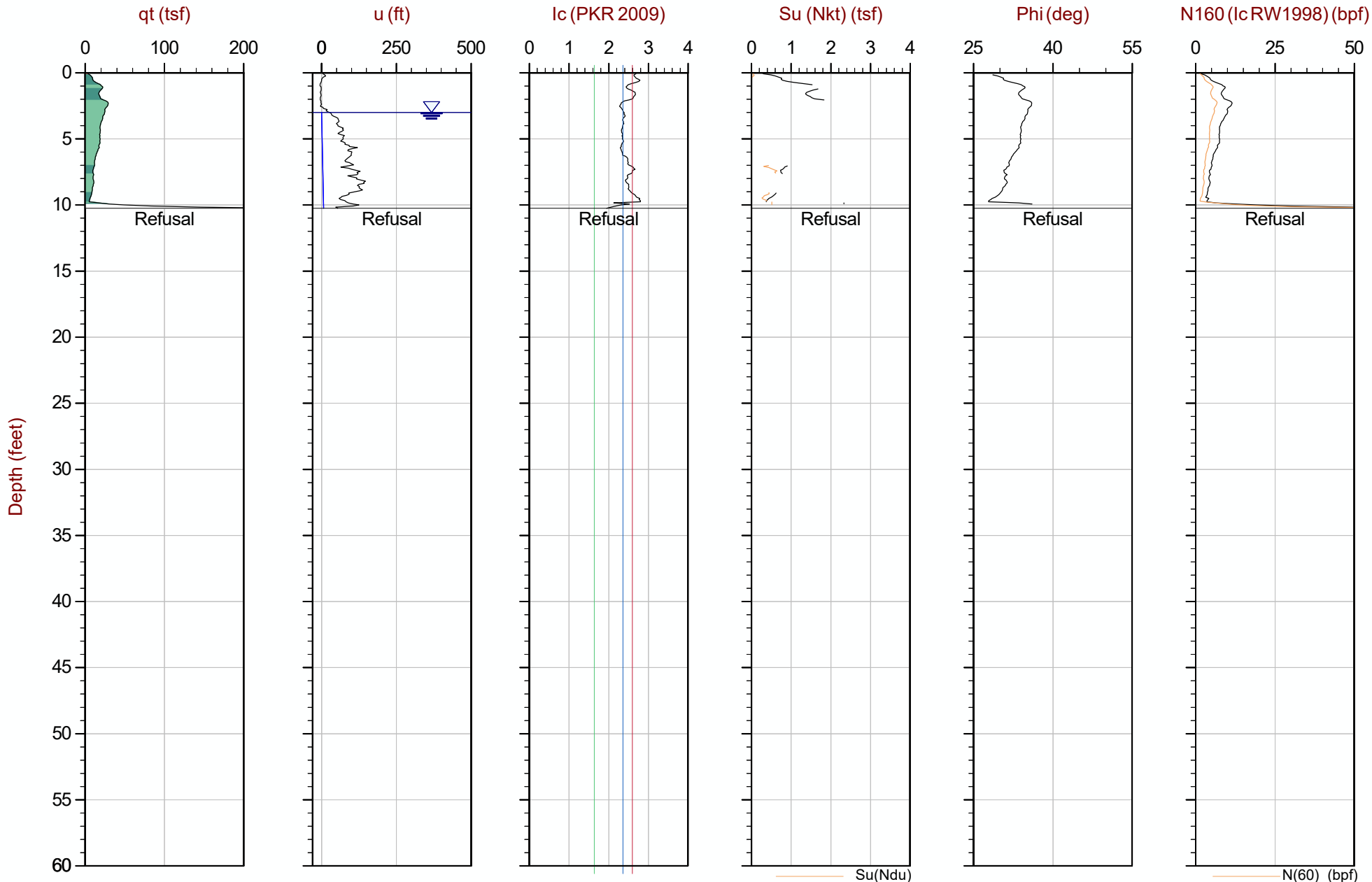
Job No: 20-53-21525

Date: 2020-10-30 10:15

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-125

Cone: 524:T375F10U500



Max Depth: 3.125 m / 10.25 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP125.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79925 ° Long: -68.68700 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

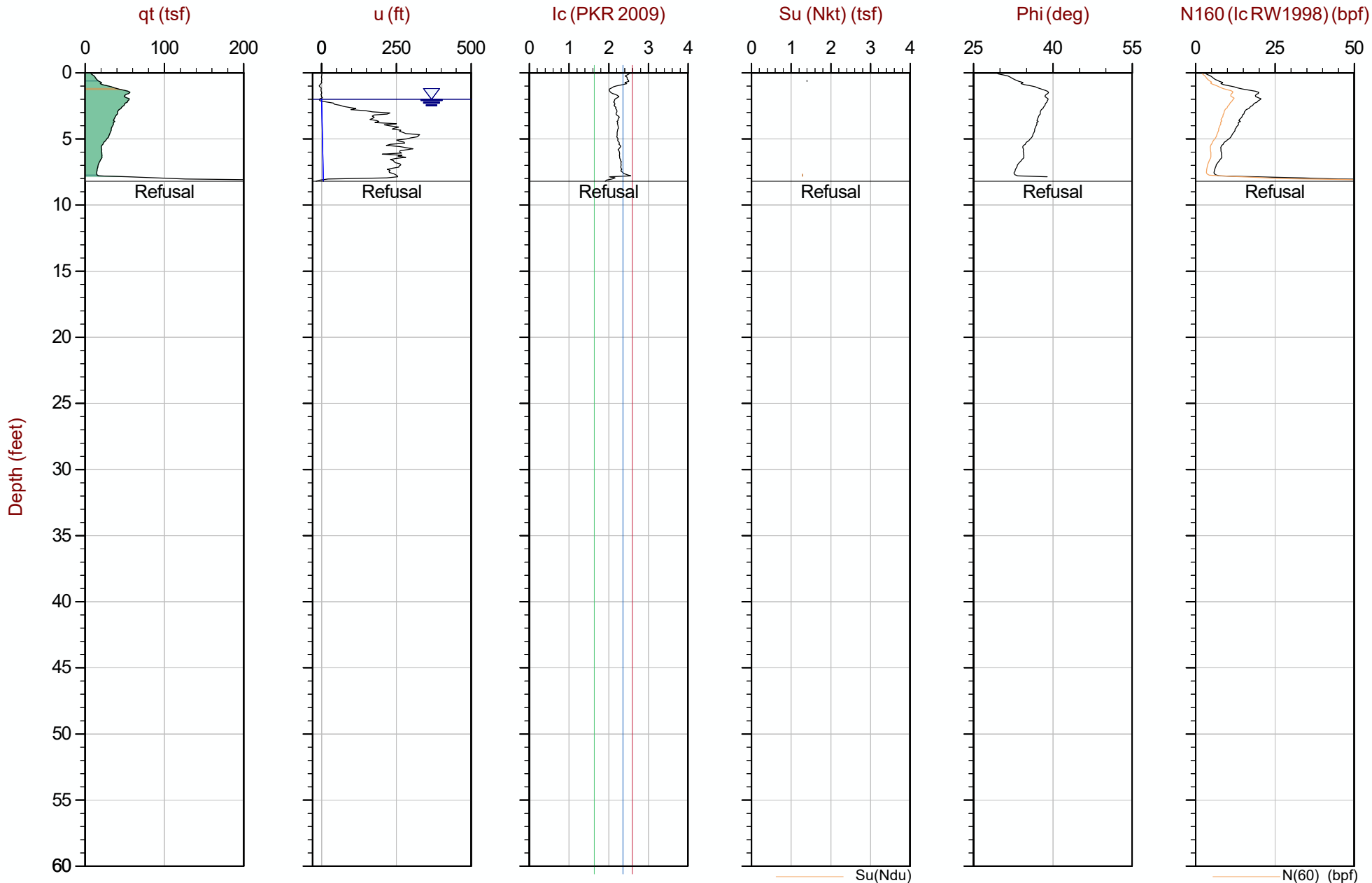
Job No: 20-53-21525

Date: 2020-10-30 09:44

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-126

Cone: 524:T375F10U500



Max Depth: 2.500 m / 8.20 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP126.COR
Unit Wt: SBTQtn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.80002 ° Long: -68.68636 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

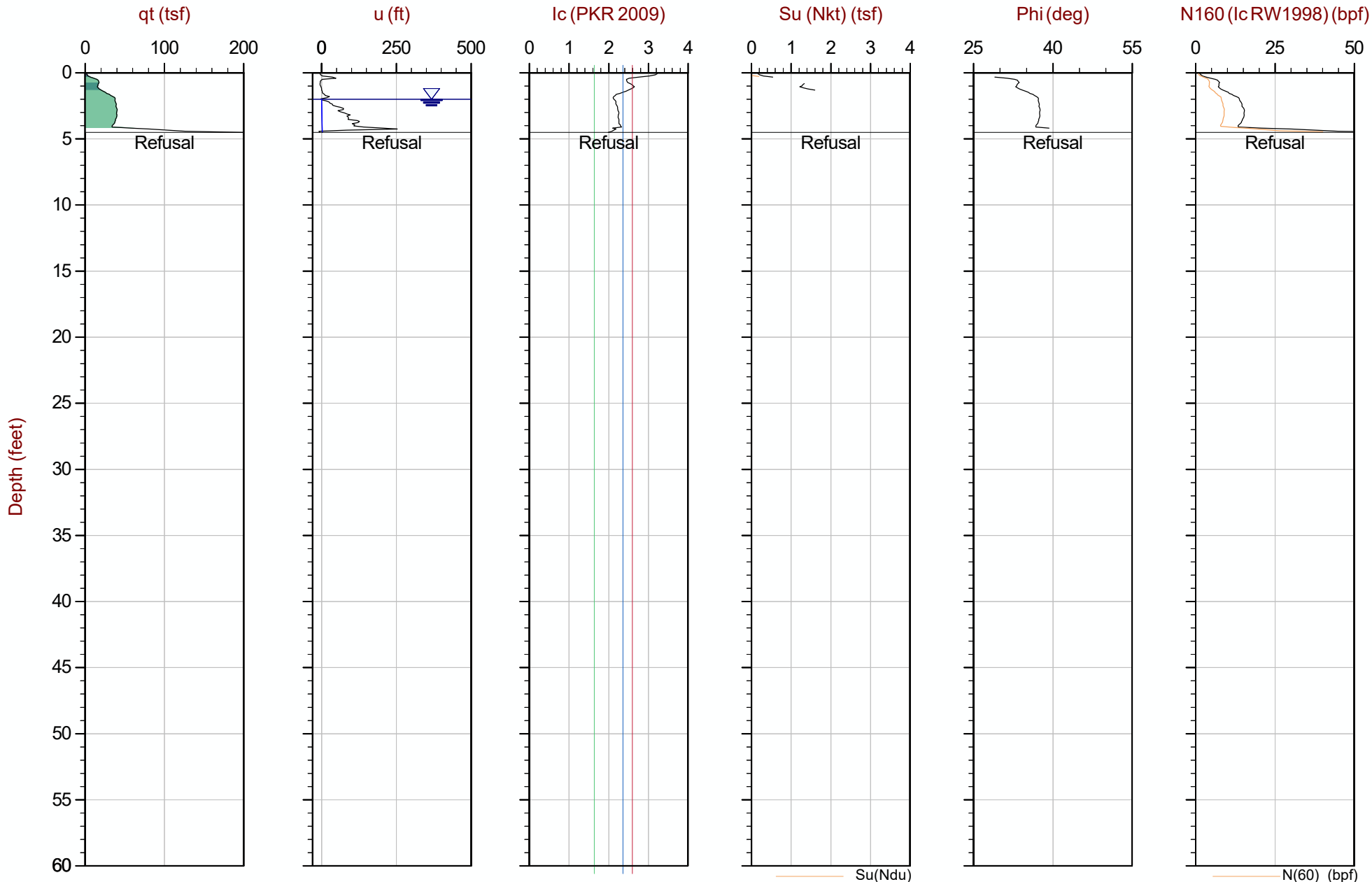
Job No: 20-53-21525

Date: 2020-10-30 09:11

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-127

Cone: 524:T375F10U500



Max Depth: 1.375 m / 4.51 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525 CP127.COR
Unit Wt: SBTQn(PKR2009)
Su Nkt/Ndu: 12.5 / 6.0

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.80037 ° Long: -68.68603 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.

Seismic Cone Penetration Test Plots



Haley & Aldrich

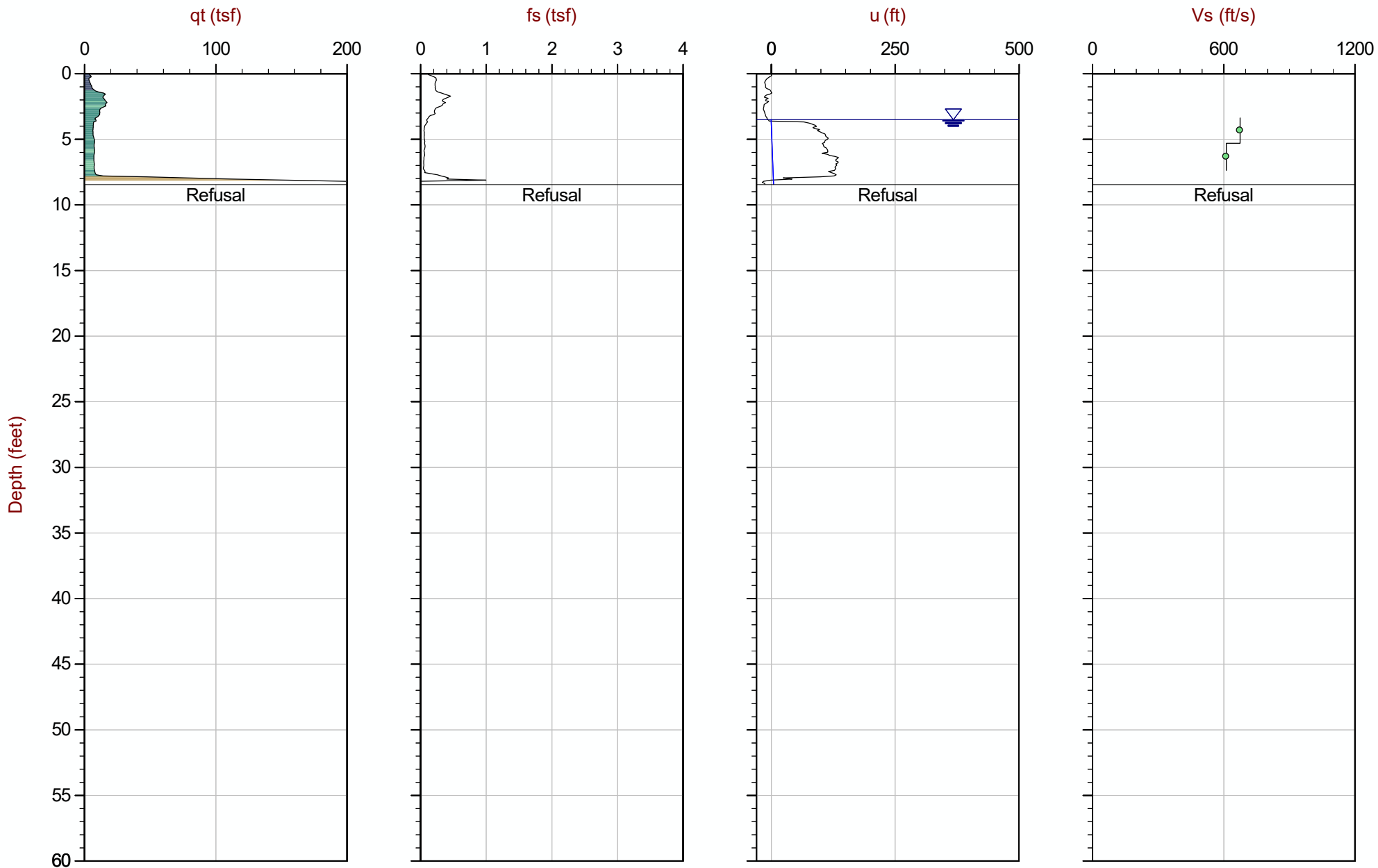
Job No: 20-53-21525

Date: 2020-10-27 14:05

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101

Cone: 524:T375F10U500



Max Depth: 2.575 m / 8.45 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP101.COR
Unit Wt: SBTQn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78975 ° Long: -68.69922 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

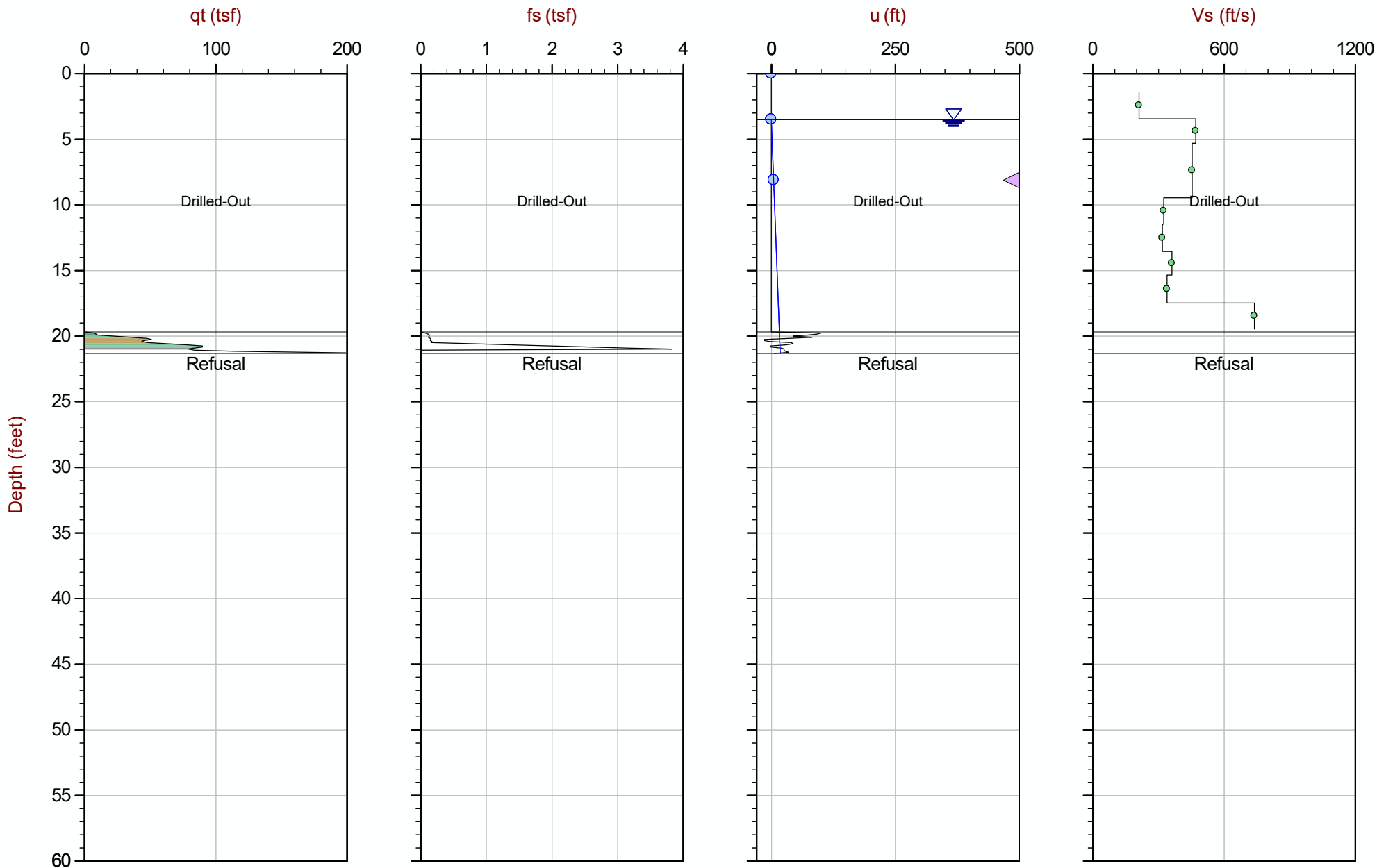
Job No: 20-53-21525

Date: 2020-10-28 08:27

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101B

Cone: 524:T375F10U500



Max Depth: 6.500 m / 21.33 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP101B.COR
Unit Wt: SBTQtn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.78983 ° Long: -68.69899 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

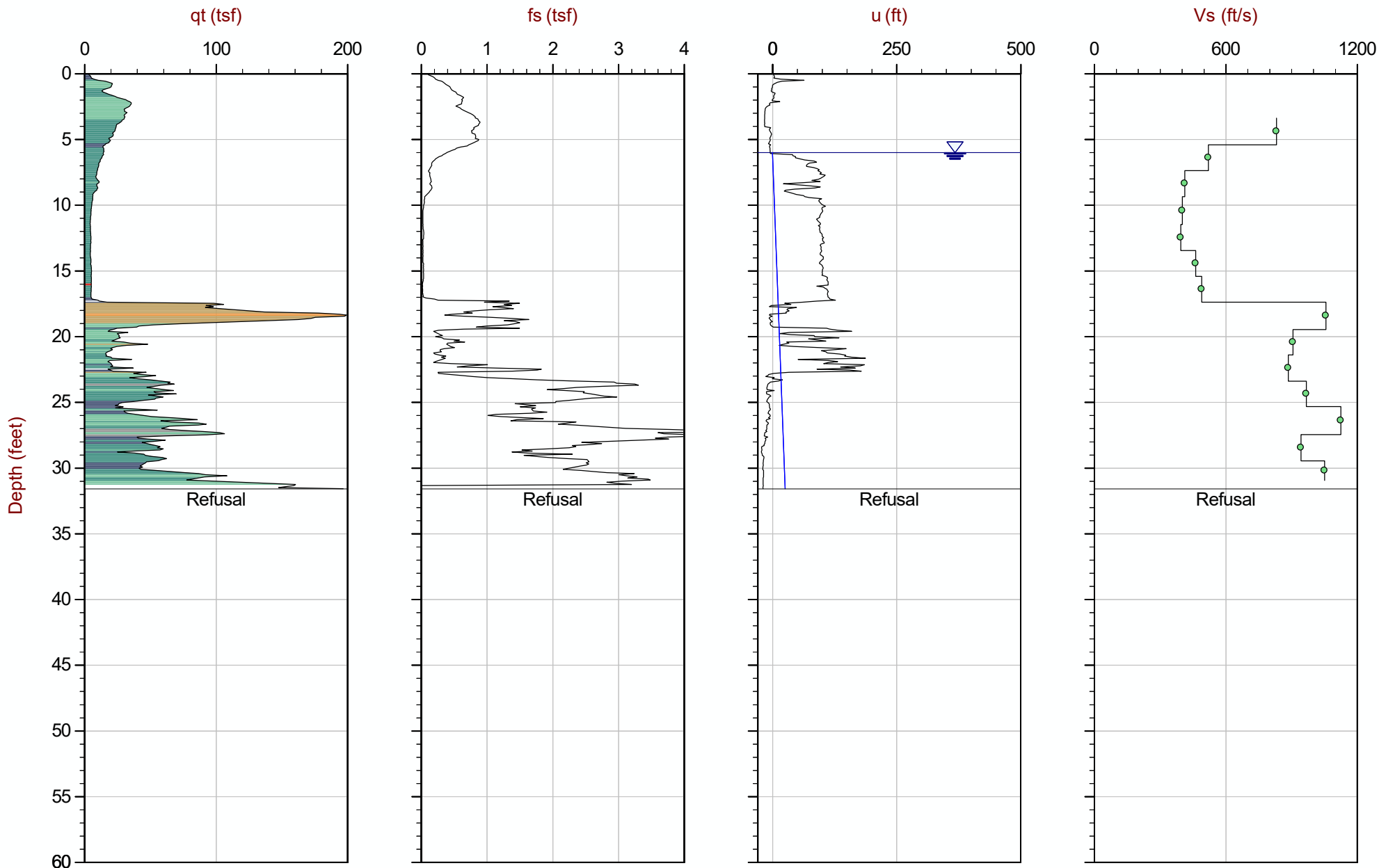
Job No: 20-53-21525

Date: 2020-10-29 08:44

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-102

Cone: 524:T375F10U500



Max Depth: 9.625 m / 31.58 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP102.COR
Unit Wt: SBTQn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.79006 ° Long: -68.69877 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

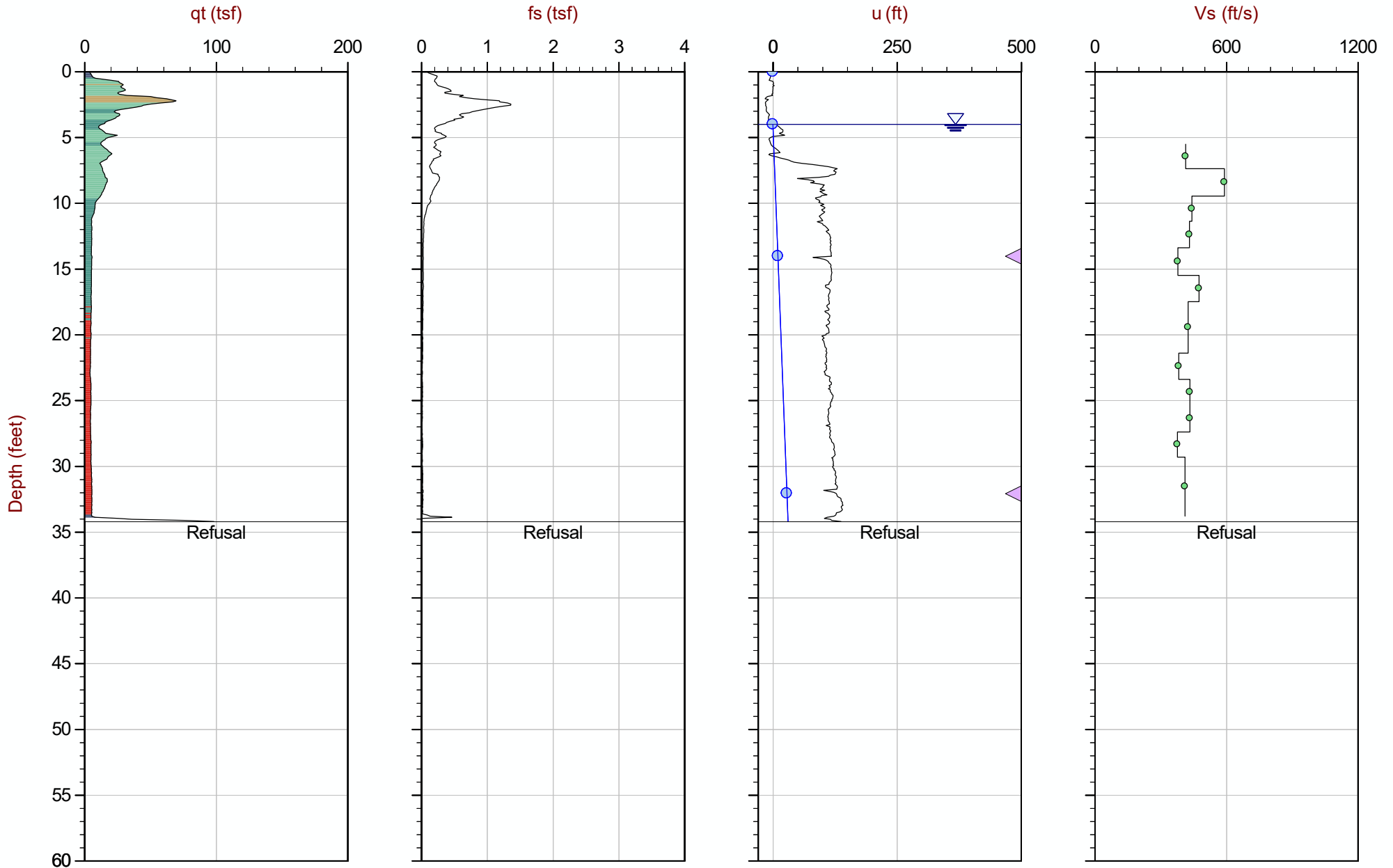
Job No: 20-53-21525

Date: 2020-11-01 12:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103

Cone: 524:T375F10U500



Max Depth: 10.425 m / 34.20 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP103.COR
Unit Wt: SBTQn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77204 ° Long: -68.71887 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



Haley & Aldrich

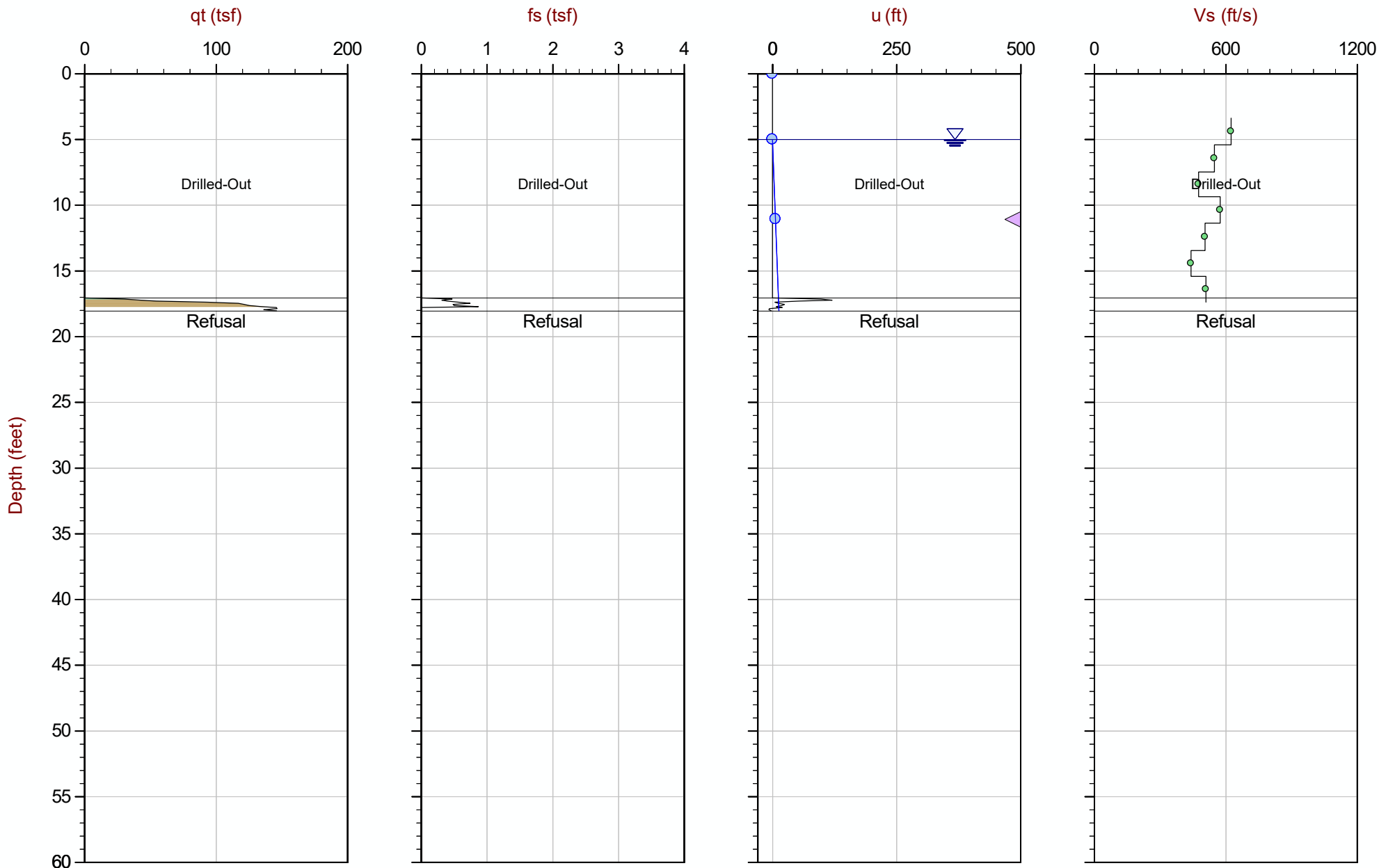
Job No: 20-53-21525

Date: 2020-11-01 08:08

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-104

Cone: 524:T375F10U500



Max Depth: 5.500 m / 18.04 ft
Depth Inc: 0.025 m / 0.082 ft
Avg Int: Every Point

File: 20-53-21525_SP104.COR
Unit Wt: SBTQn(PKR2009)

SBT: Robertson, 2009 and 2010
Coords: Lat: 44.77043 ° Long: -68.71722 °

Hydrostatic Line Ueq Assumed Ueq PPD, Ueq achieved PPD, Ueq not achieved

The reported coordinates were acquired from consumer-grade GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.

Seismic Cone Penetration Test Shear Wave (V_s) Traces



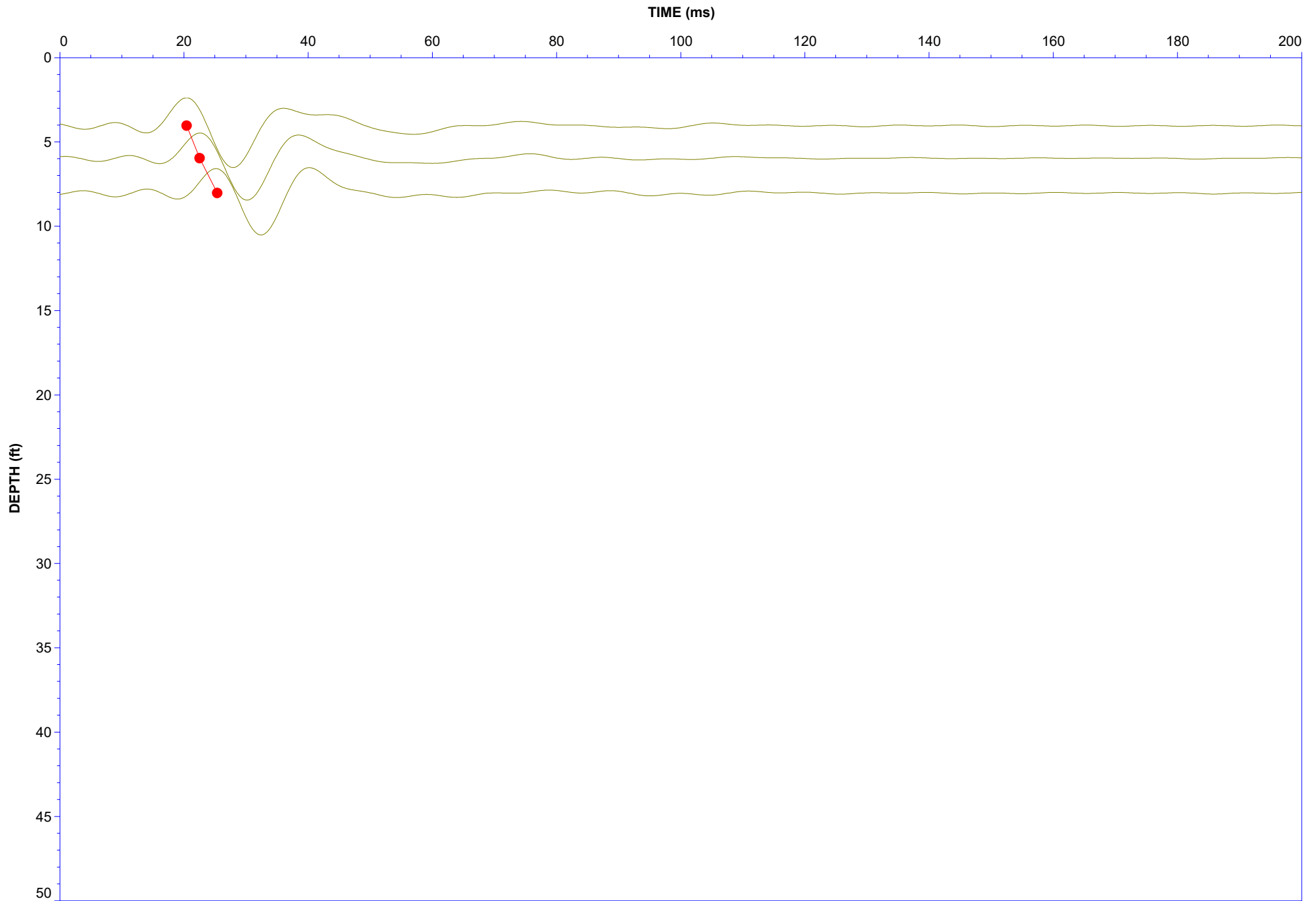
Job No: 20-53-21525
Date: 27-Oct-2020

Client: Haley & Aldrich

Project: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101

Filter: 10-100hz





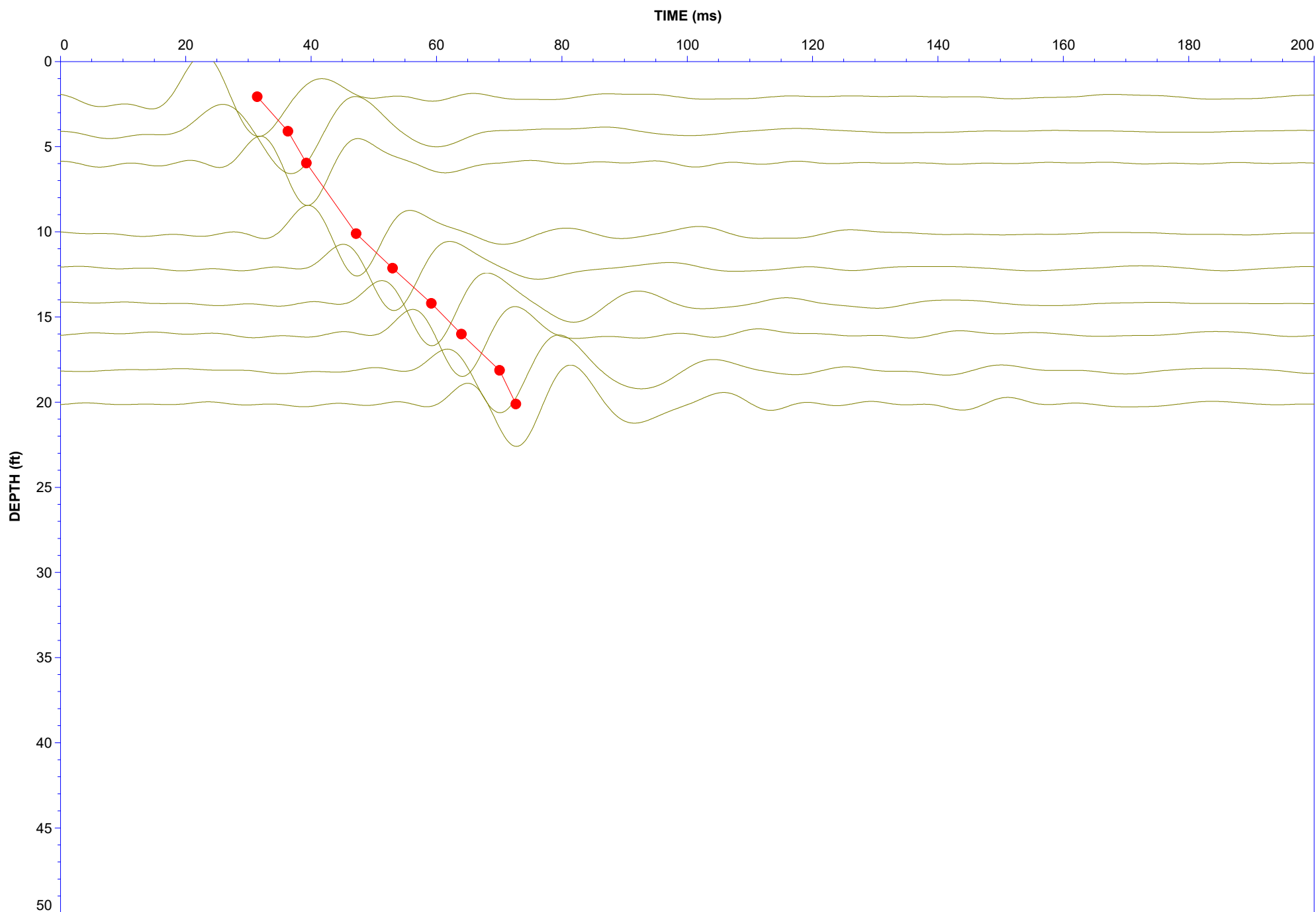
Job No: 20-53-21525
Date: 28-Oct-2020

Client: Haley & Aldrich

Project: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101B

Filter: 10-100hz





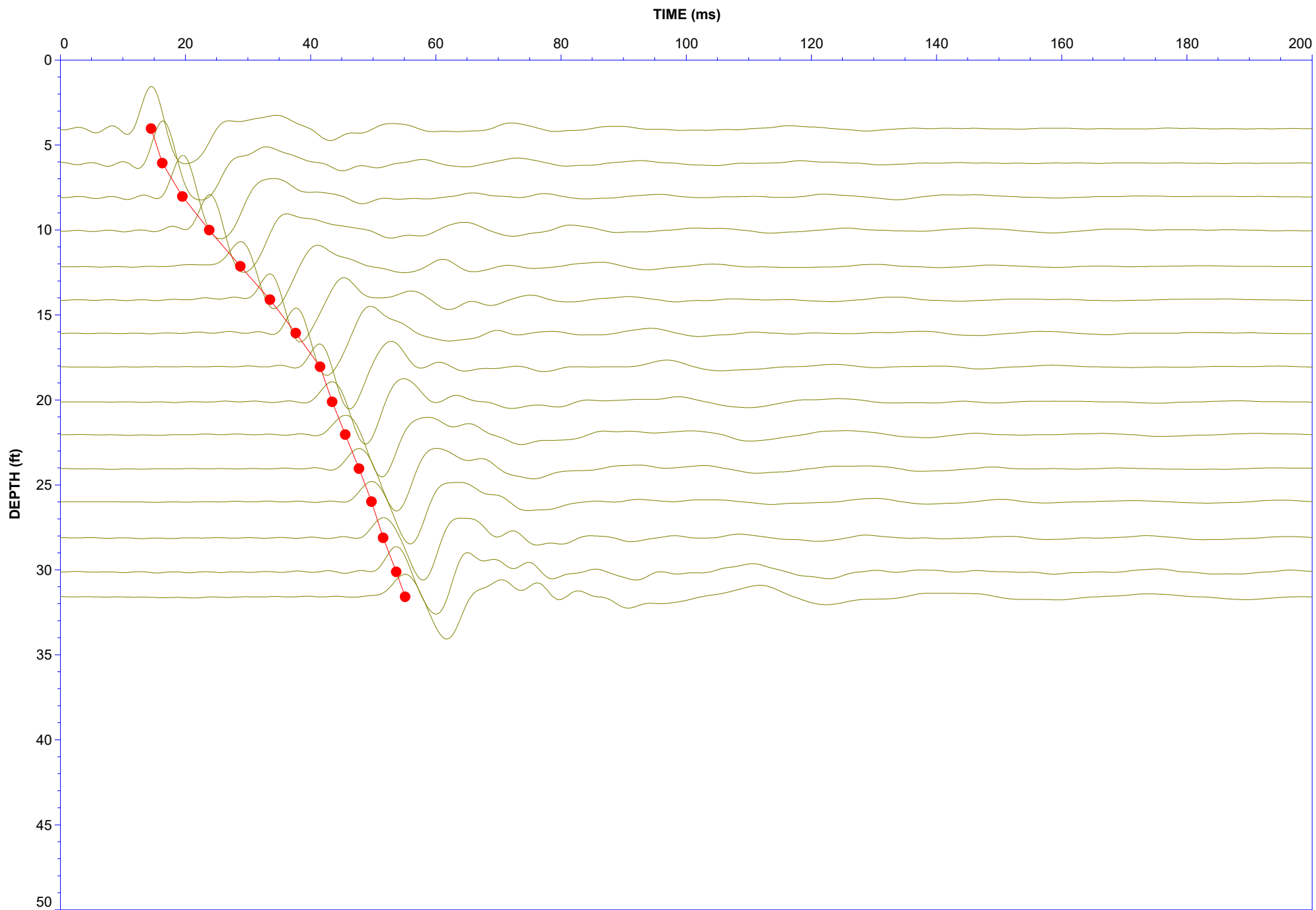
Job No: 20-53-21525
Date: 29-Oct-2020

Client: Haley & Aldrich

Project: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-102

Filter: 20-200hz





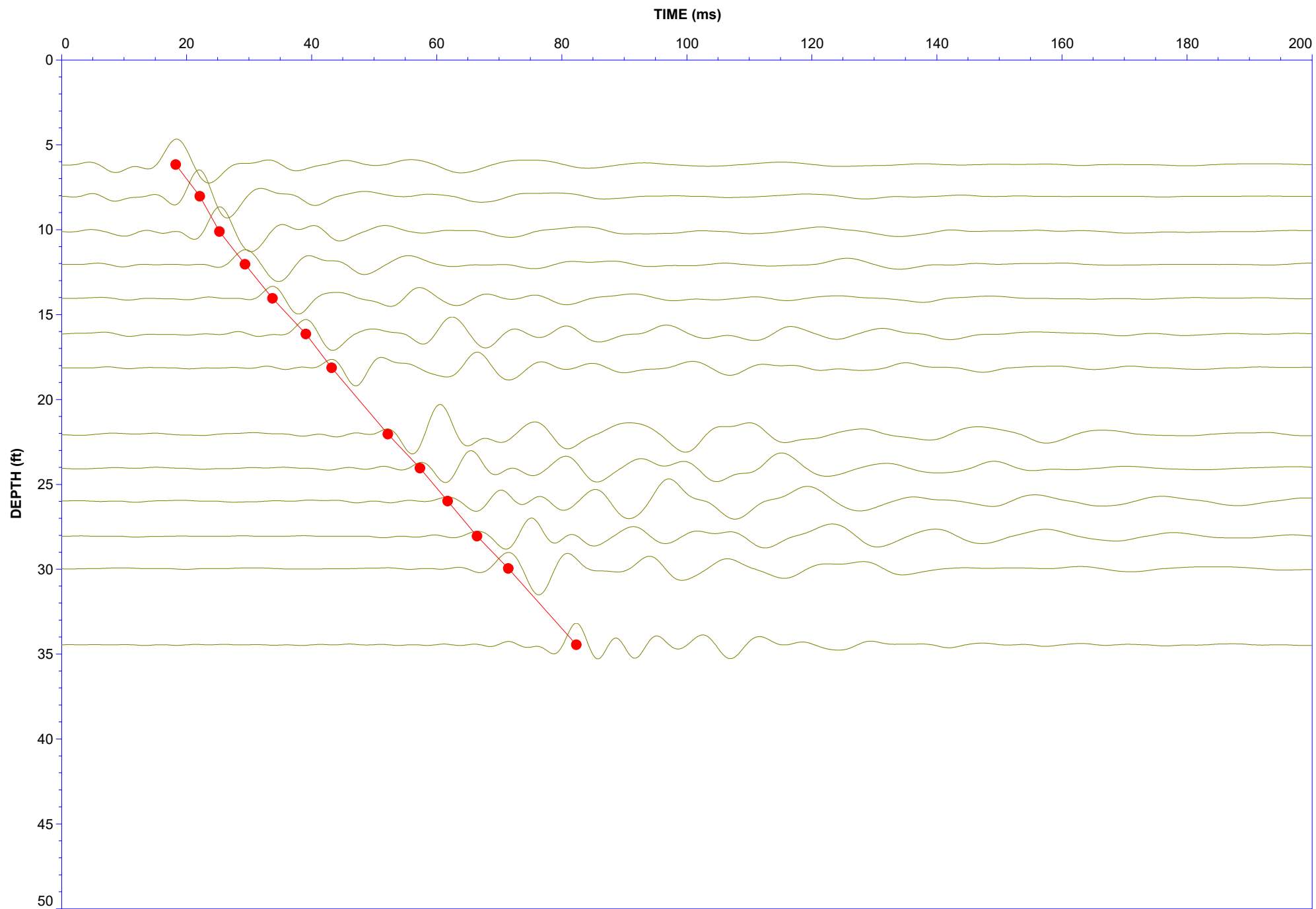
Job No: 20-53-21525
Date: 1-Nov-2020

Client: Haley & Aldrich

Project: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103

Filter: 20-200hz





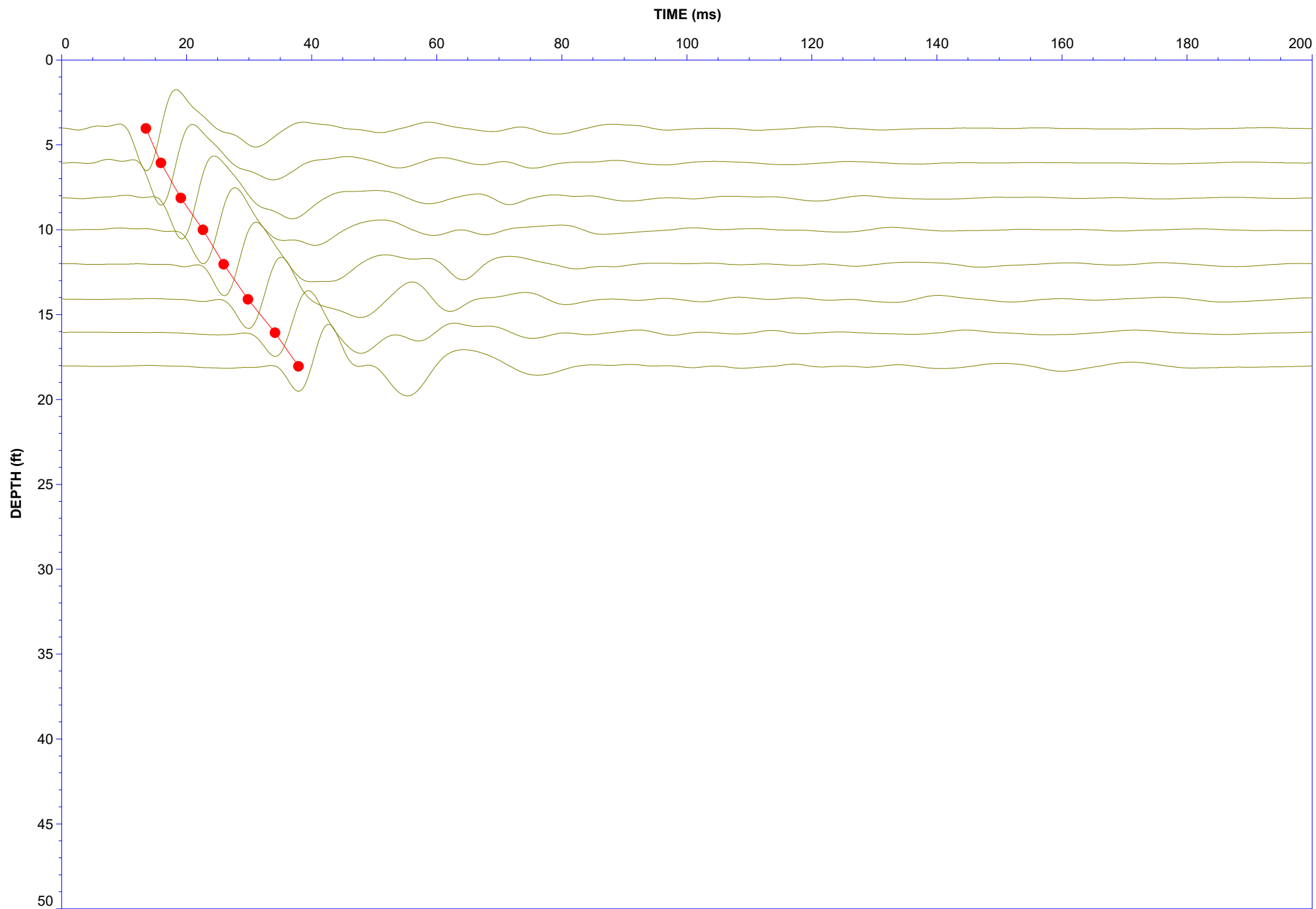
Job No: 20-53-21525
Date: 1-Nov-2020

Client: Haley & Aldrich

Project: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-104

Filter: 20-200hz



Seismic Cone Penetration Test Tabular Results



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Sounding ID: SCPT20-101
Date: 27-Oct-2020

Seismic Source: Beam
Source Offset (ft): 3.98
Source Depth (ft): 0
Geophone Offset (ft): 0.66

SCPT_u SHEAR WAVE VELOCITY TEST RESULTS - V_s

Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
4.04	3.38	5.22			
5.97	5.31	6.64	1.42	2.10	675
8.04	7.38	8.39	1.75	2.85	612



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Sounding ID: SCPT20-101B
Date: 28-Oct-2020

Seismic Source: Beam
Source Offset (ft): 3.98
Source Depth (ft): 0
Geophone Offset (ft): 0.66

SCPT_u SHEAR WAVE VELOCITY TEST RESULTS - V_s

Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
2.07	1.41	4.22			
4.10	3.44	5.26	1.04	4.92	212
5.97	5.31	6.64	1.38	2.92	471
10.10	9.45	10.25	3.61	7.93	455
12.14	11.48	12.15	1.90	5.84	325
14.21	13.55	14.12	1.97	6.17	319
16.01	15.35	15.86	1.74	4.81	362
18.14	17.49	17.93	2.07	6.09	340
20.11	19.46	19.86	1.92	2.60	739



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Sounding ID: SCPT20-102
Date: 29-Oct-2020

Seismic Source: Beam
Source Offset (ft): 3.98
Source Depth (ft): 0
Geophone Offset (ft): 0.66

SCPT_u SHEAR WAVE VELOCITY TEST RESULTS - V_s

Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
4.04	3.38	5.22			
6.07	5.41	6.72	1.50	1.80	832
8.04	7.38	8.39	1.67	3.20	521
10.01	9.35	10.16	1.78	4.30	413
12.14	11.48	12.15	1.99	4.95	402
14.11	13.45	14.03	1.87	4.75	395
16.08	15.42	15.93	1.90	4.10	463
18.04	17.39	17.84	1.91	3.90	490
20.11	19.46	19.86	2.02	1.91	1057
22.05	21.39	21.76	1.90	2.09	907
24.05	23.39	23.73	1.97	2.22	886
25.98	25.33	25.64	1.91	1.97	968
28.12	27.46	27.75	2.11	1.87	1125
30.12	29.46	29.73	1.98	2.10	943
31.59	30.94	31.19	1.46	1.39	1051



Job No: 20-53-21525
Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Sounding ID: SCPT20-103
Date: 01-Nov-2020

Seismic Source: Beam
Source Offset (ft): 3.98
Source Depth (ft): 0
Geophone Offset (ft): 0.66

SCPT_u SHEAR WAVE VELOCITY TEST RESULTS - V_s

Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
6.17	5.51	6.80			
8.04	7.38	8.39	1.59	3.84	414
10.10	9.45	10.25	1.87	3.16	591
12.04	11.38	12.06	1.81	4.08	443
14.04	13.39	13.96	1.90	4.42	431
16.14	15.49	15.99	2.02	5.34	379
18.14	17.49	17.93	1.95	4.10	475
22.05	21.39	21.76	3.82	9.00	425
24.05	23.39	23.73	1.97	5.15	383
25.98	25.33	25.64	1.91	4.41	433
28.05	27.39	27.68	2.04	4.72	433
29.95	29.30	29.57	1.88	5.01	376
34.45	33.79	34.03	4.46	10.86	411



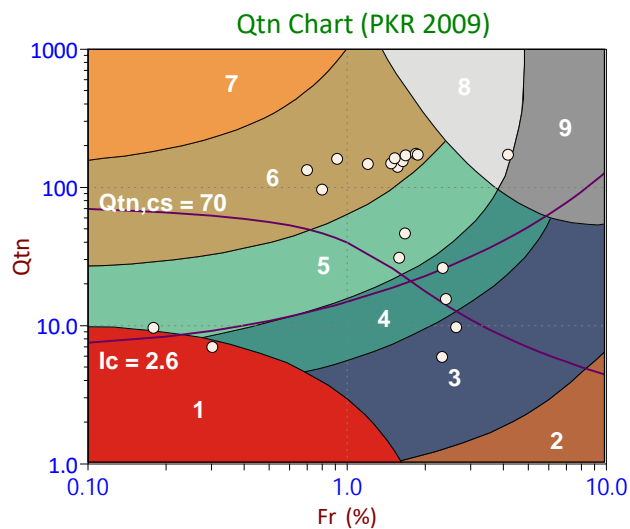
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Client: Haley & Aldrich
Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
Sounding ID: SCPT20-104
Date: 01-Nov-2020

Seismic Source: Beam
Source Offset (ft): 3.98
Source Depth (ft): 0
Geophone Offset (ft): 0.66

SCPT_u SHEAR WAVE VELOCITY TEST RESULTS - V_s

Tip Depth (ft)	Geophone Depth (ft)	Ray Path (ft)	Ray Path Difference (ft)	Travel Time Interval (ms)	Interval Velocity (ft/s)
4.04	3.38	5.22			
6.07	5.41	6.72	1.50	2.40	624
8.14	7.48	8.47	1.75	3.20	548
10.01	9.35	10.16	1.69	3.55	476
12.04	11.38	12.06	1.90	3.30	575
14.11	13.45	14.03	1.97	3.90	505
16.08	15.42	15.93	1.90	4.30	441
18.04	17.39	17.84	1.91	3.75	510

Soil Behavior Type (SBT) Scatter Plots

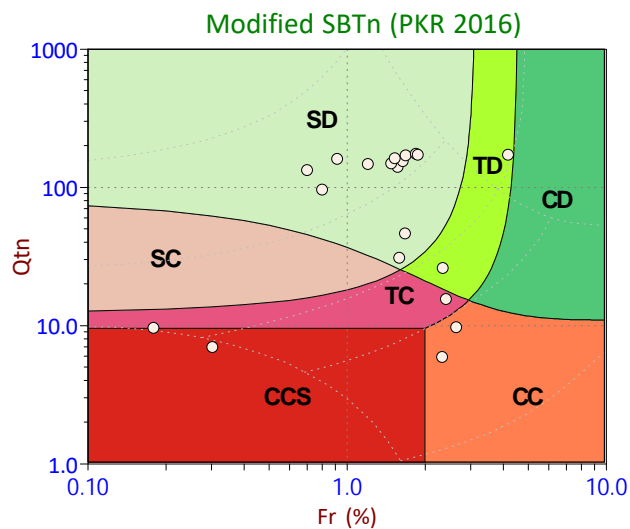


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
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- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

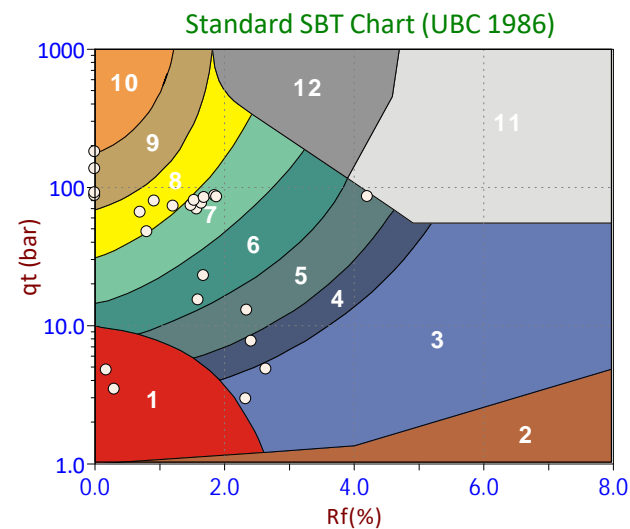
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
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- Silty Sand/Sand
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- Gravelly Sand
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- Cemented Sand



Haley & Aldrich

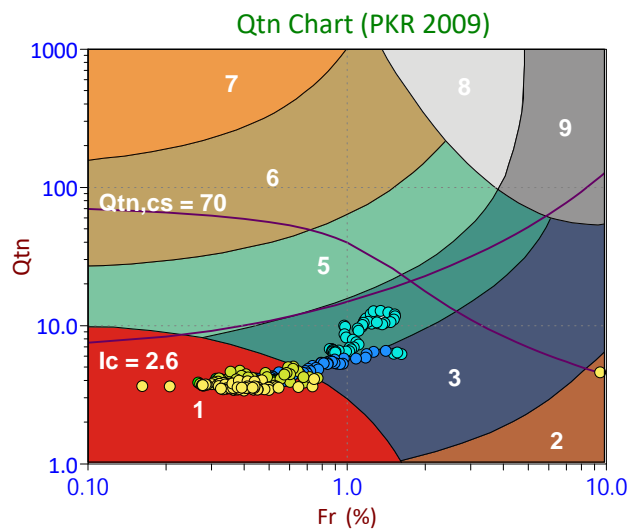
Job No: 20-53-21525

Date: 2020-11-02 10:10

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101B

Cone: 524:T375F10U500

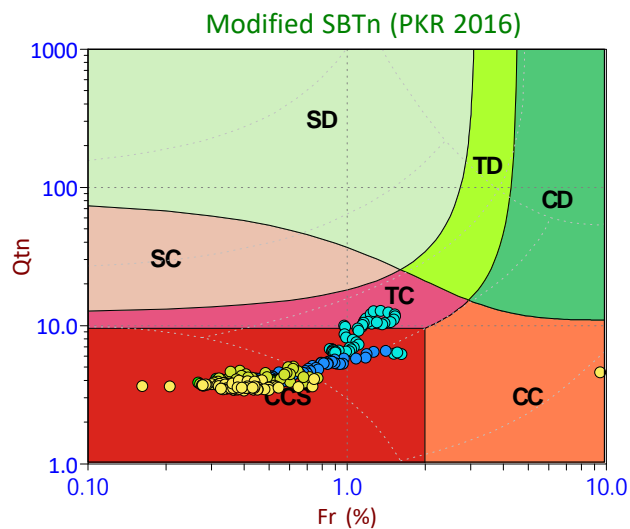


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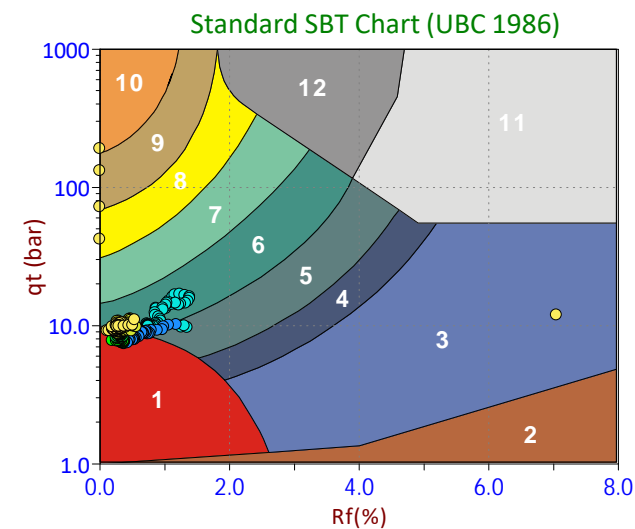
Legend

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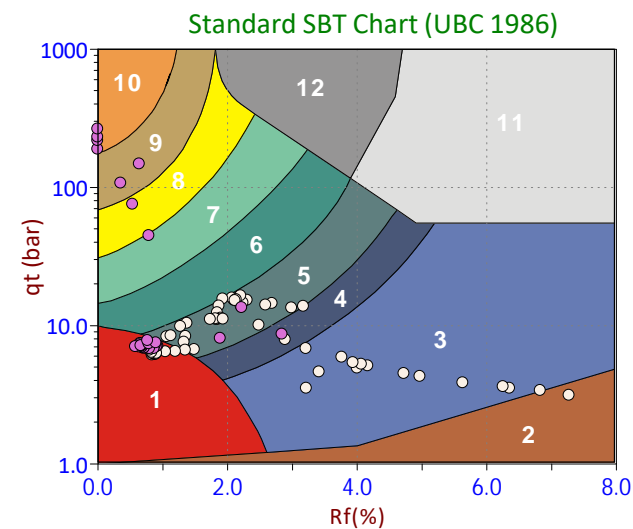
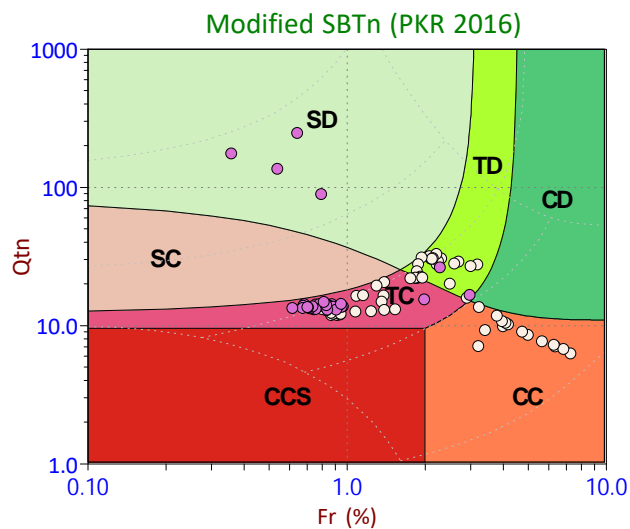
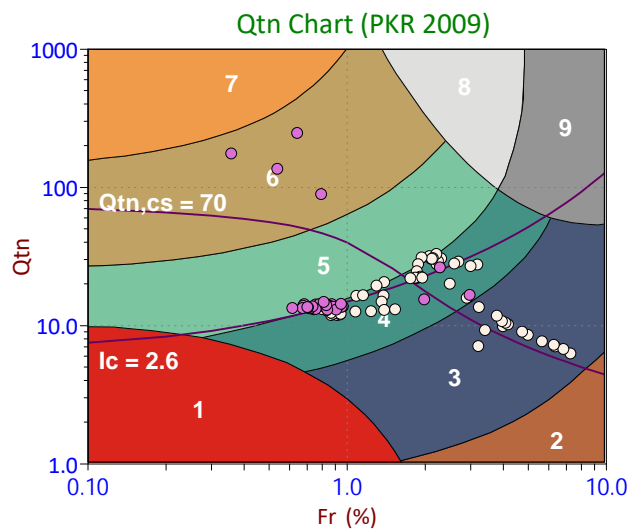
Legend

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Legend

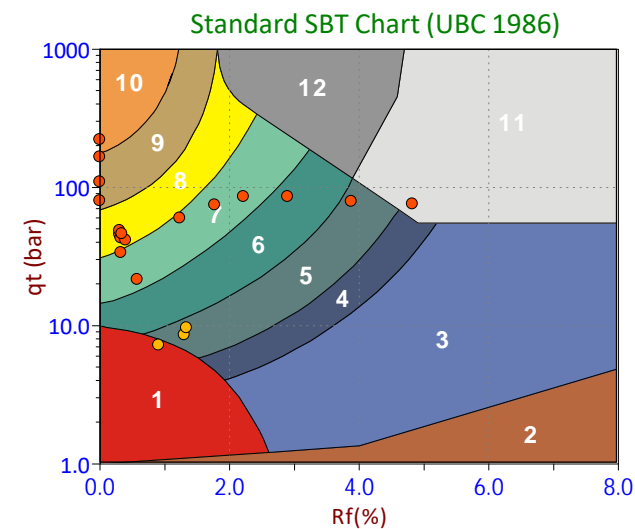
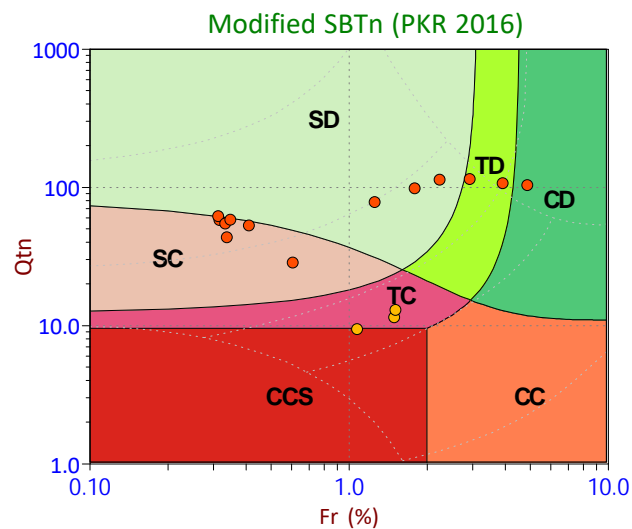
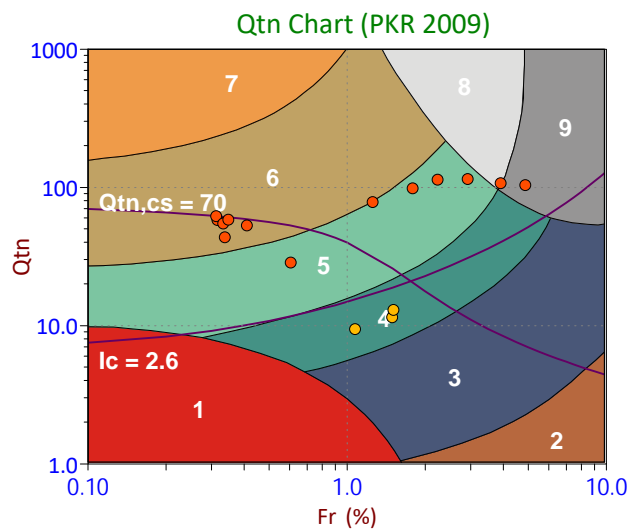
- Sensitive, Fine Grained
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Legend

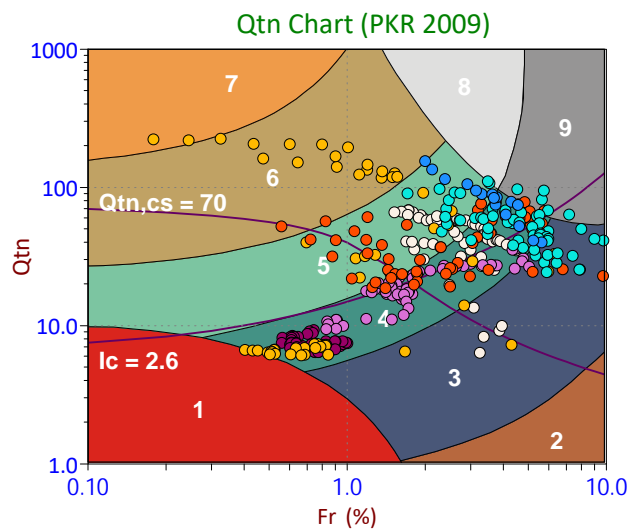
- Sensitive, Fine Grained
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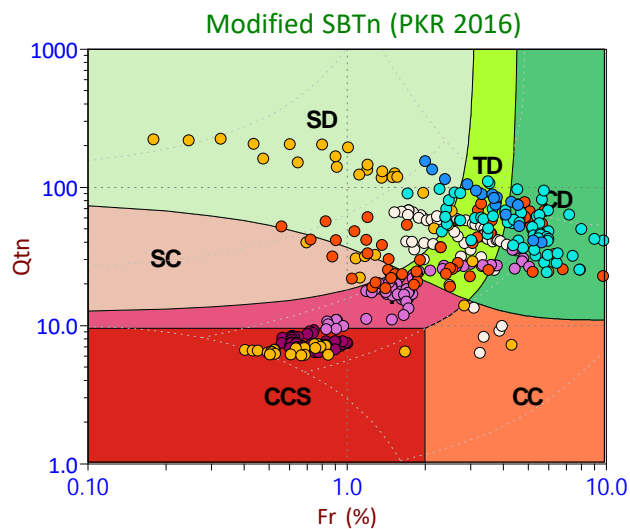


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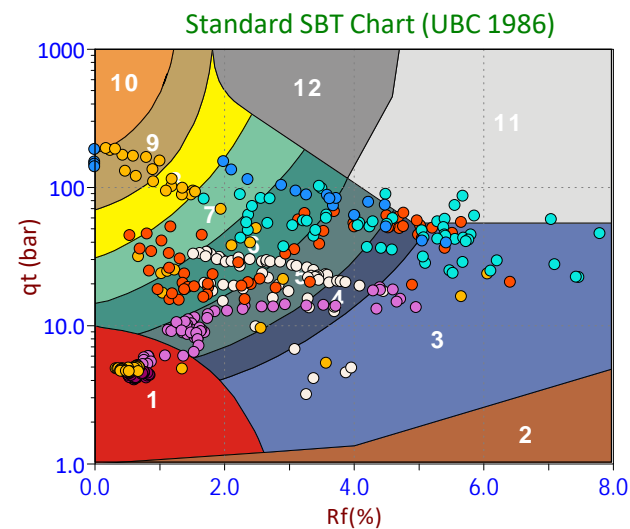
Legend

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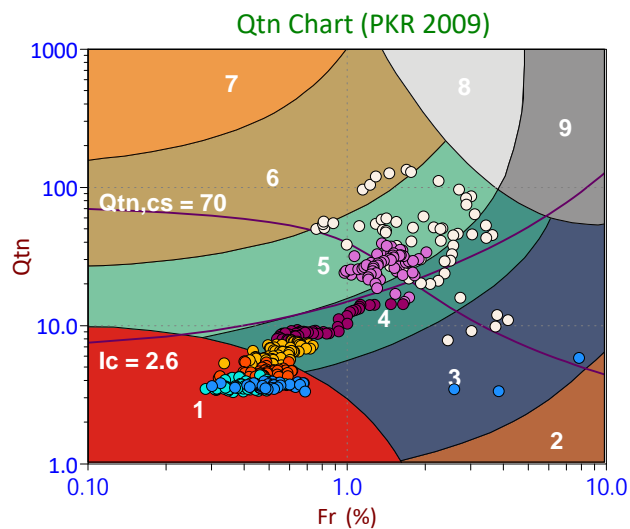
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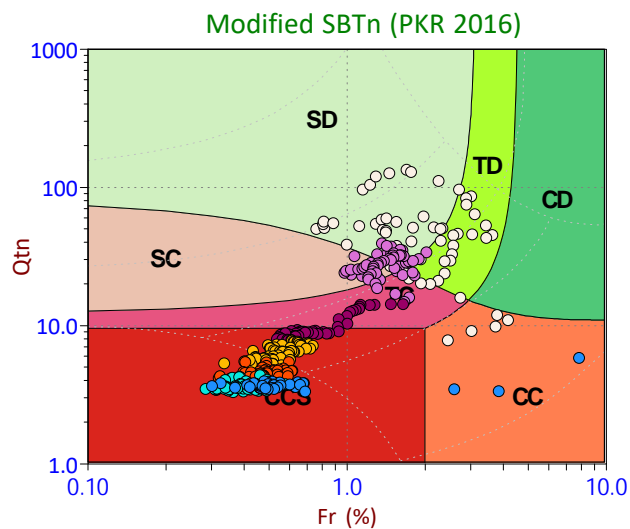


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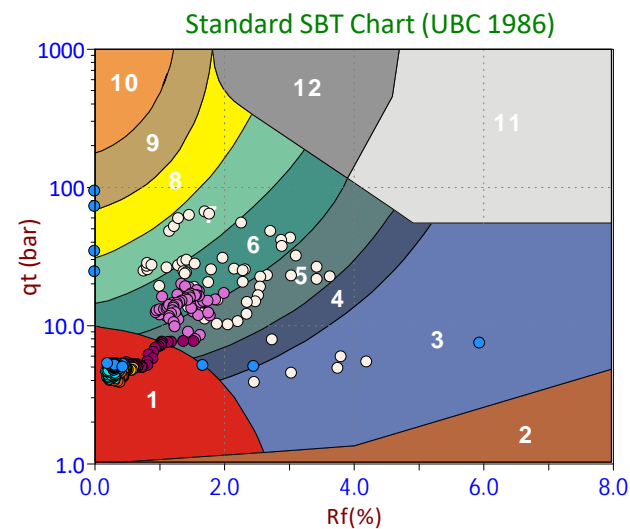
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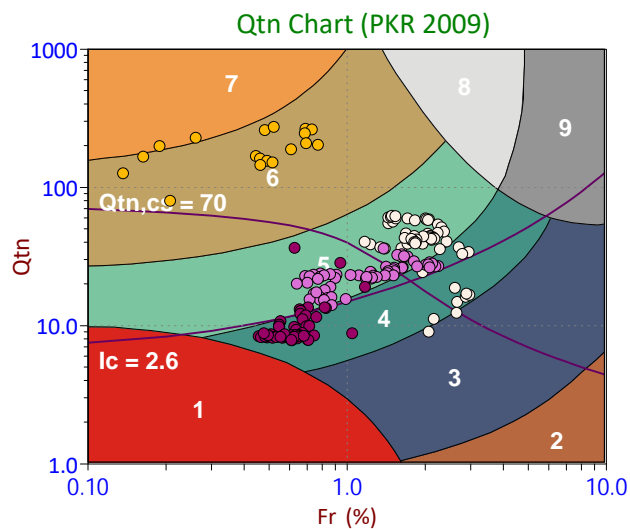
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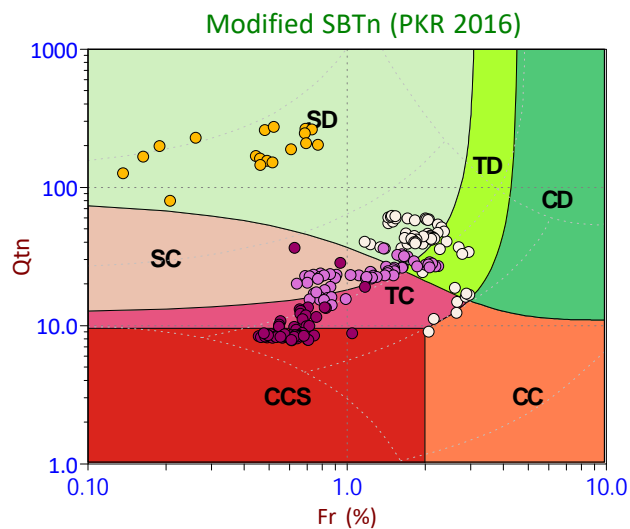


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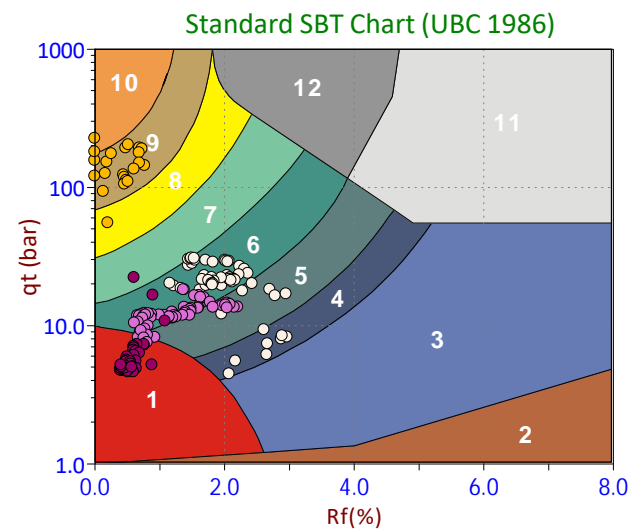
Legend

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Haley & Aldrich

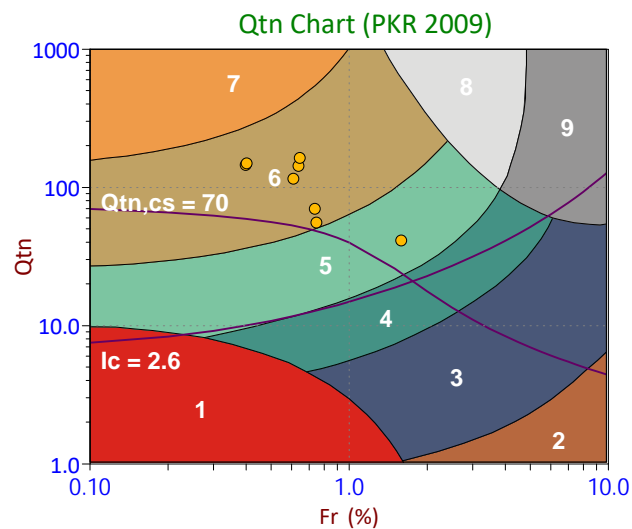
Job No: 20-53-21525

Date: 2020-11-01 08:08

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-104

Cone: 524:T375F10U500

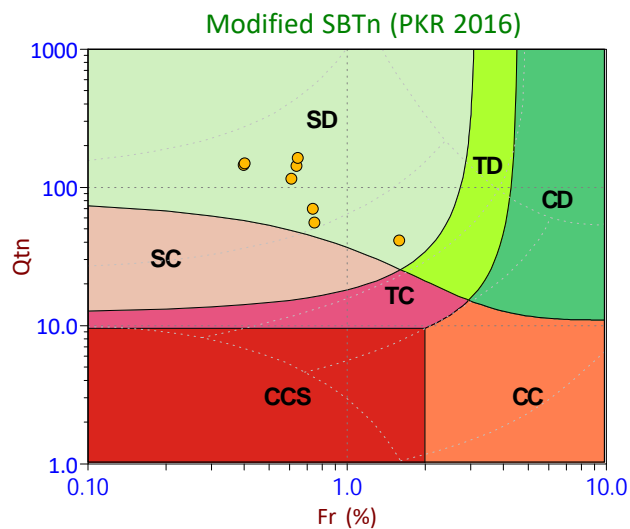


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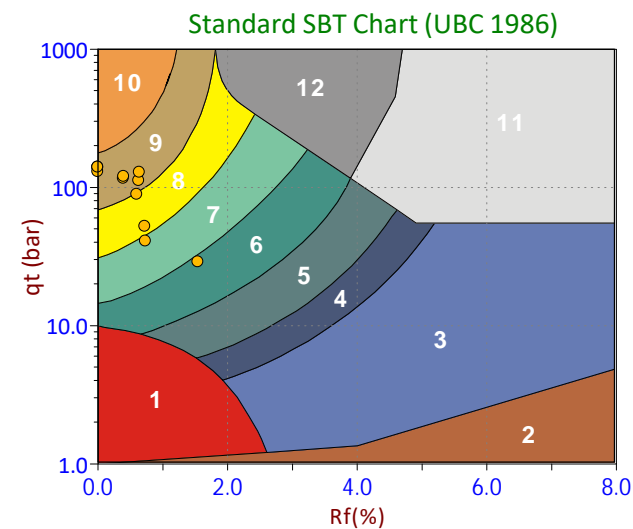
Legend

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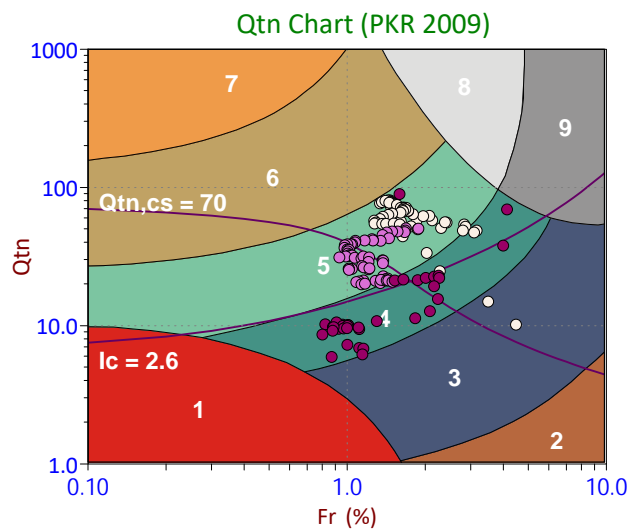
Legend

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Legend

- Sensitive Fines
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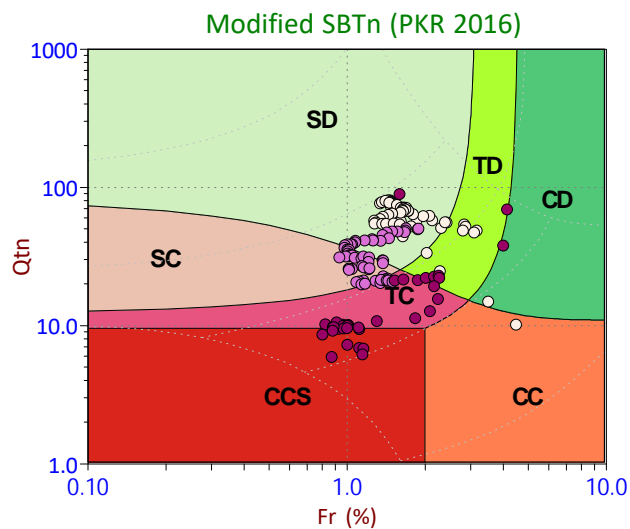


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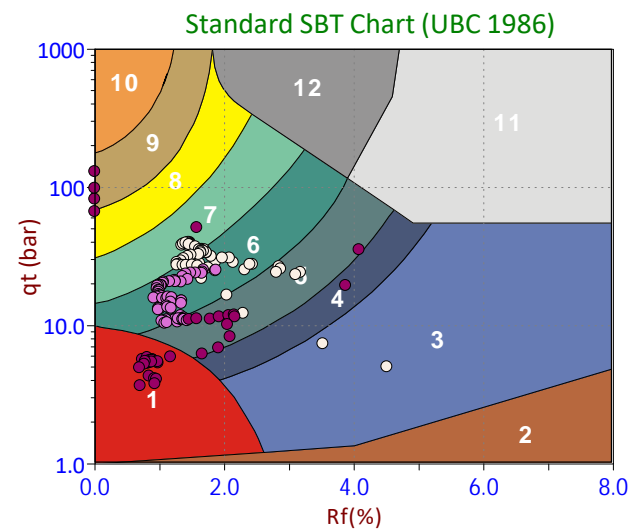
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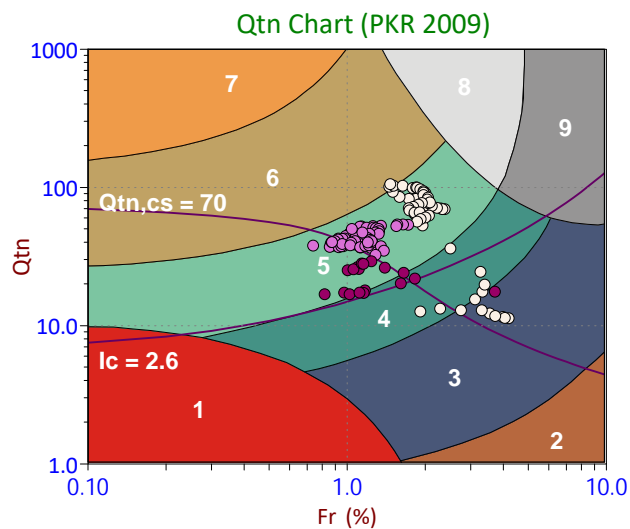
Legend

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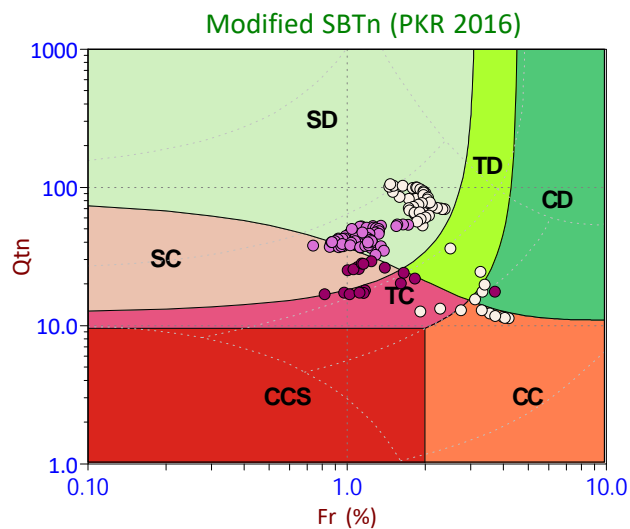


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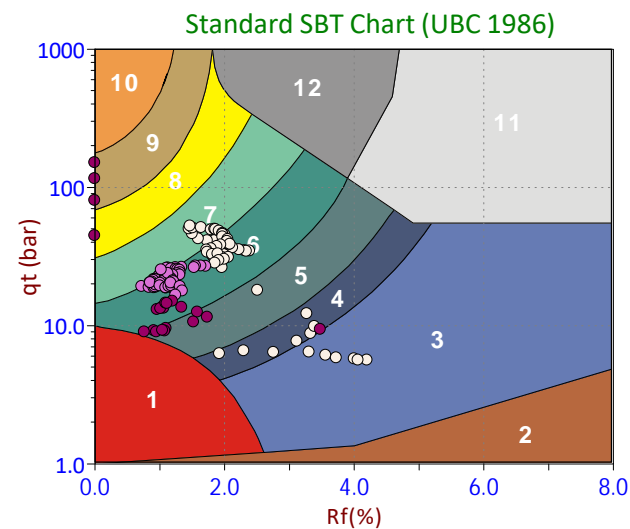
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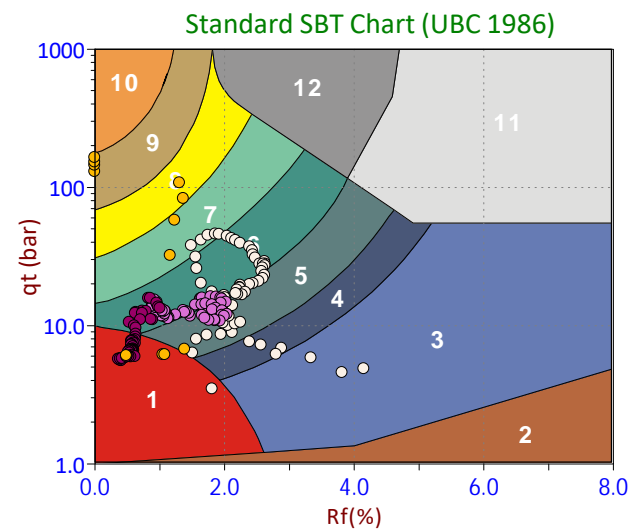
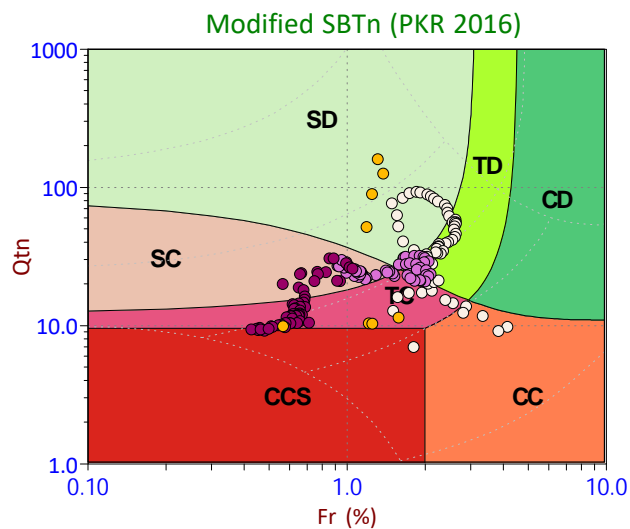
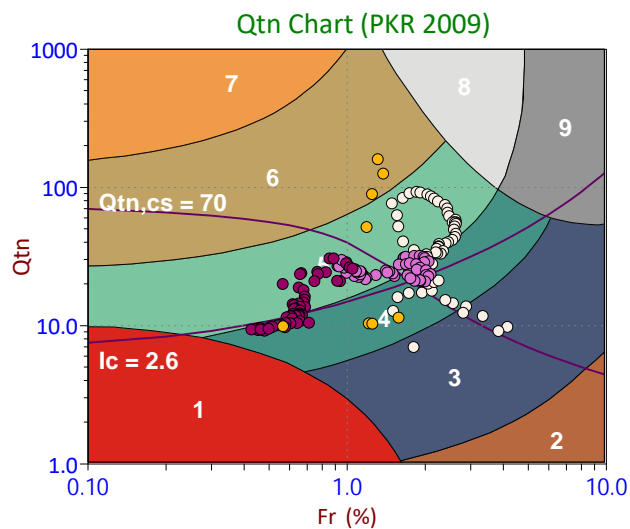
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Legend

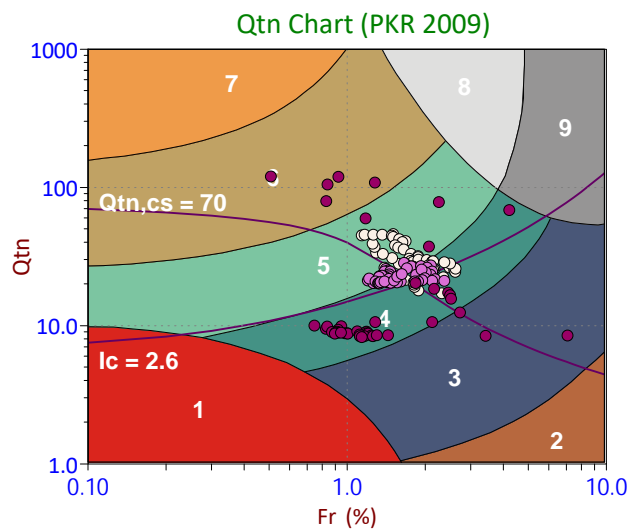
- Sensitive, Fine Grained
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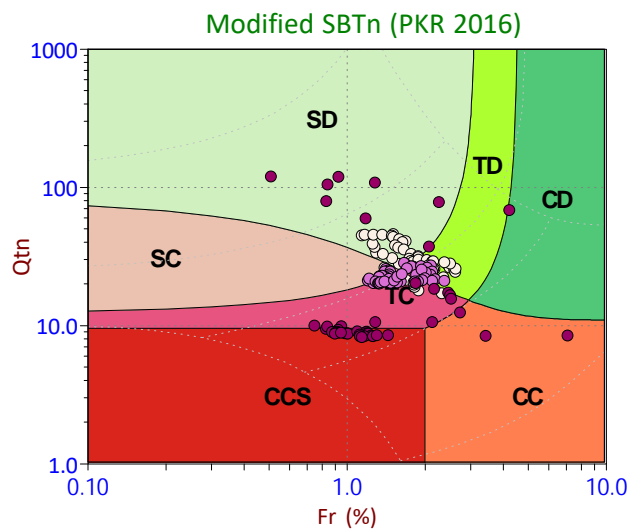


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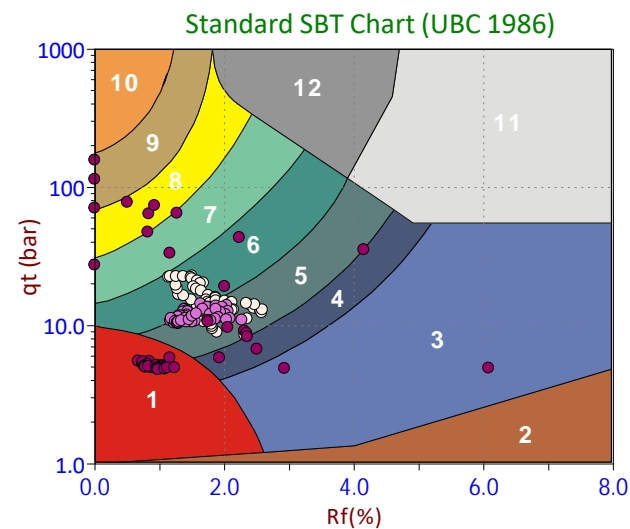
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- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



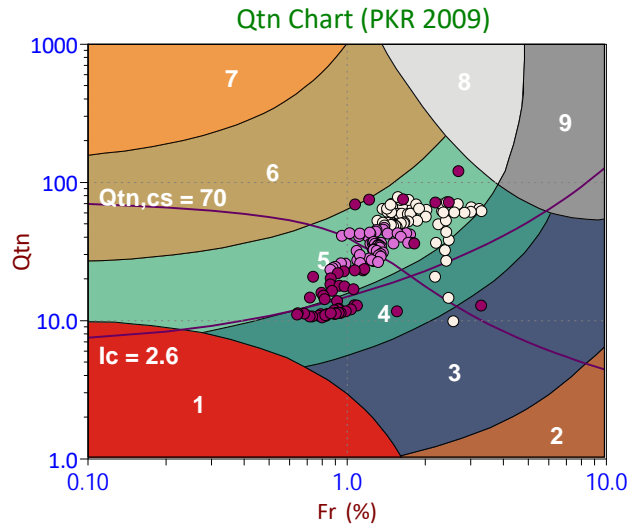
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

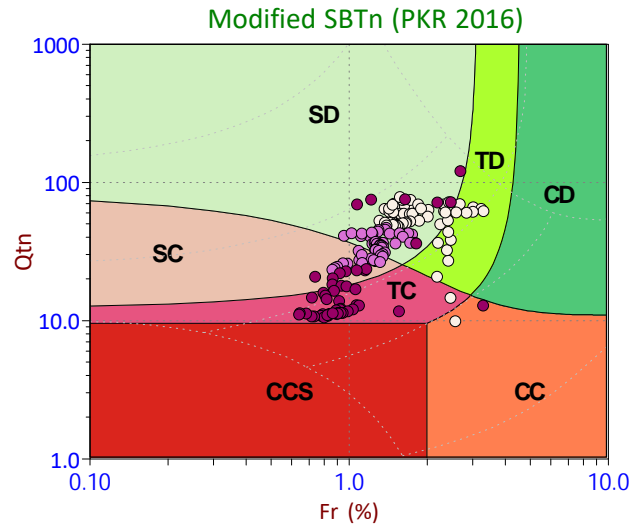


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

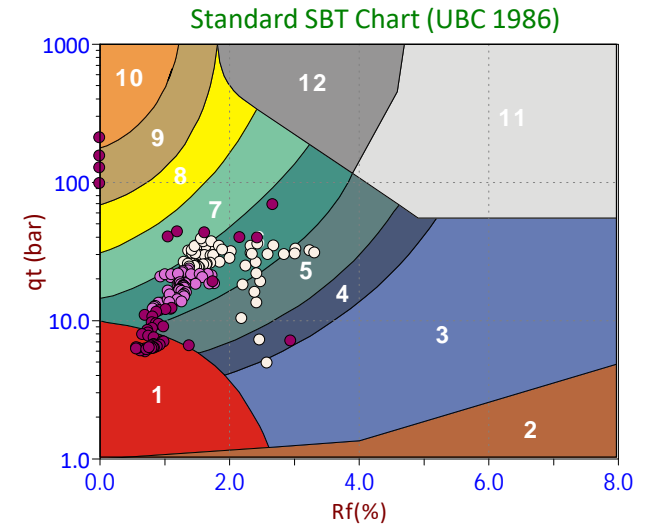
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



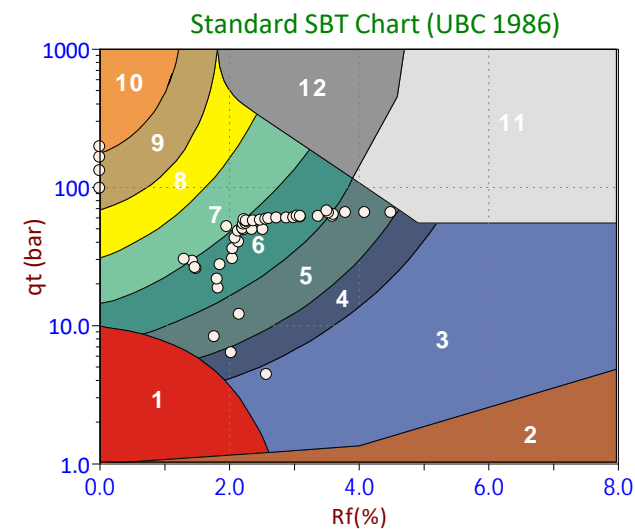
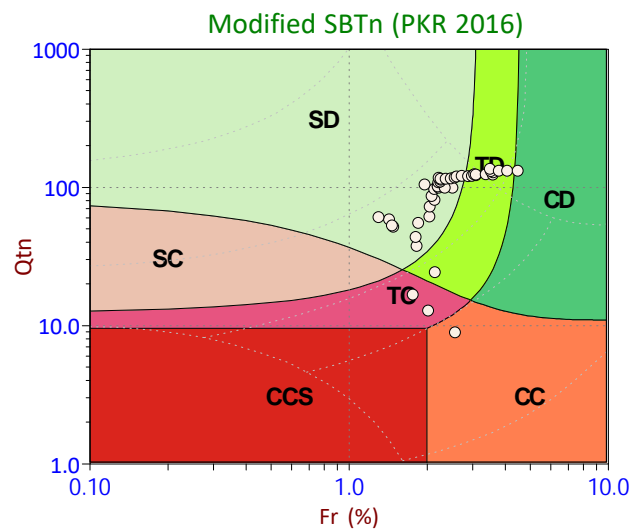
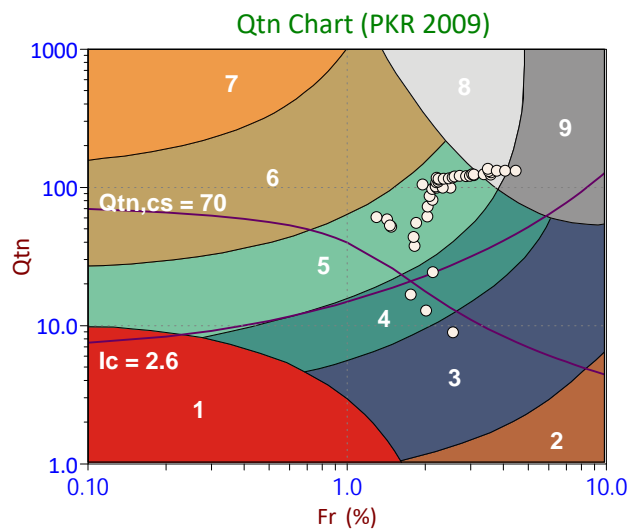
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand



Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

Legend

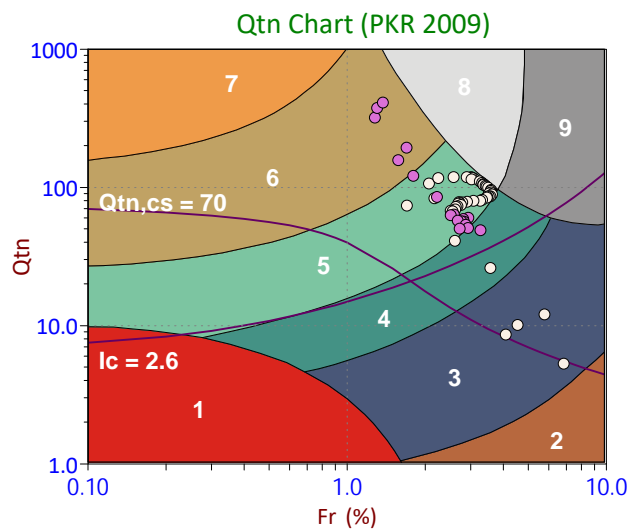
- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained

Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)

Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

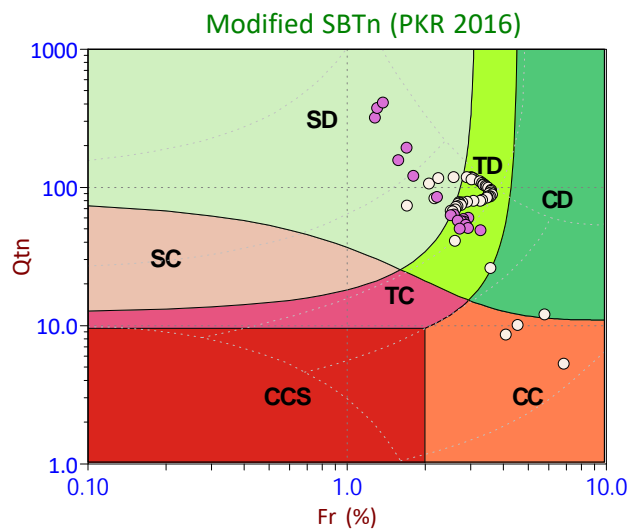


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
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- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

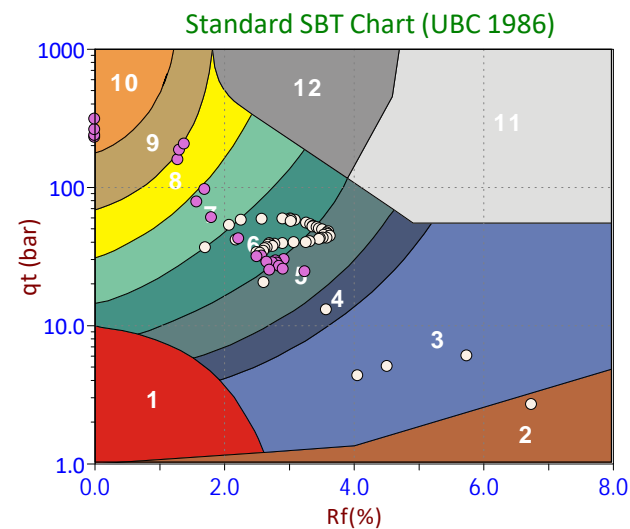
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



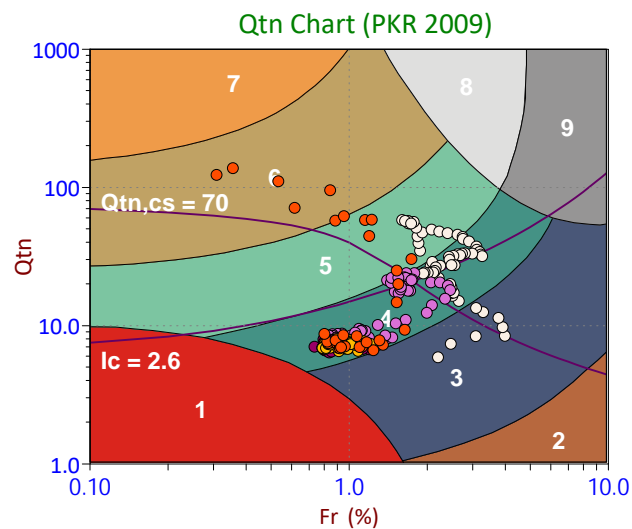
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

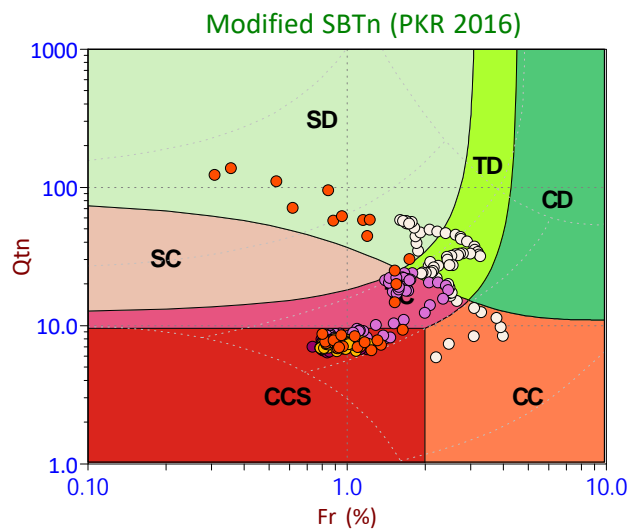


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

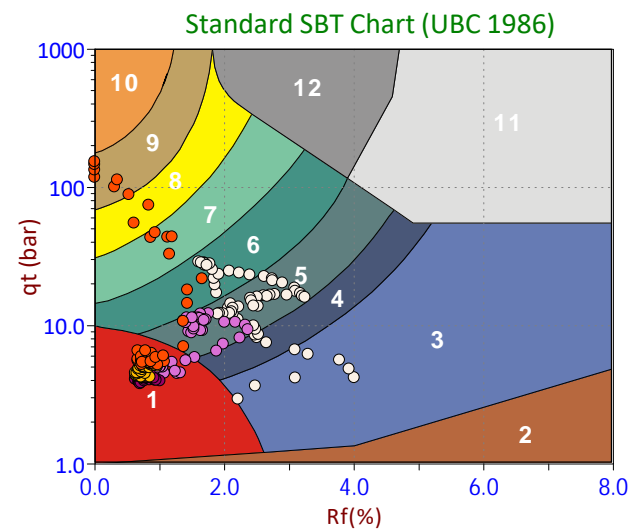
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



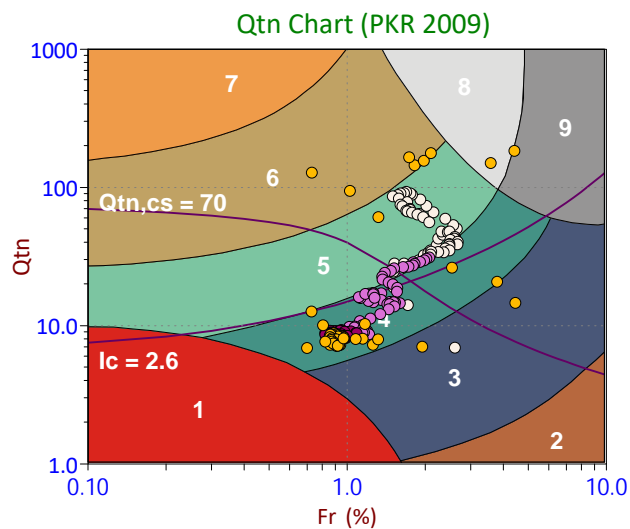
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

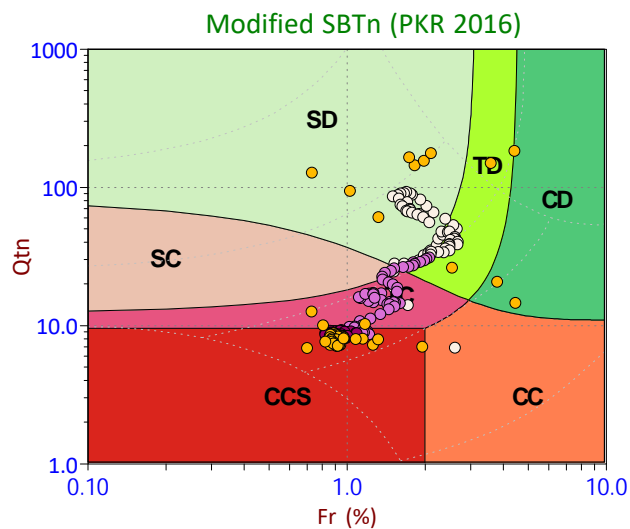


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

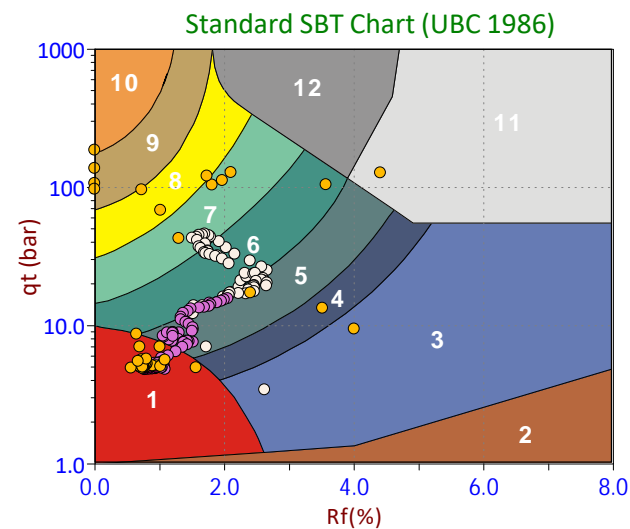
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



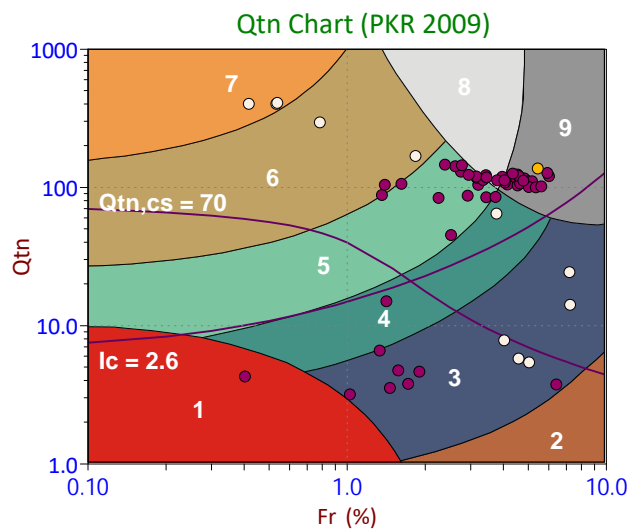
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

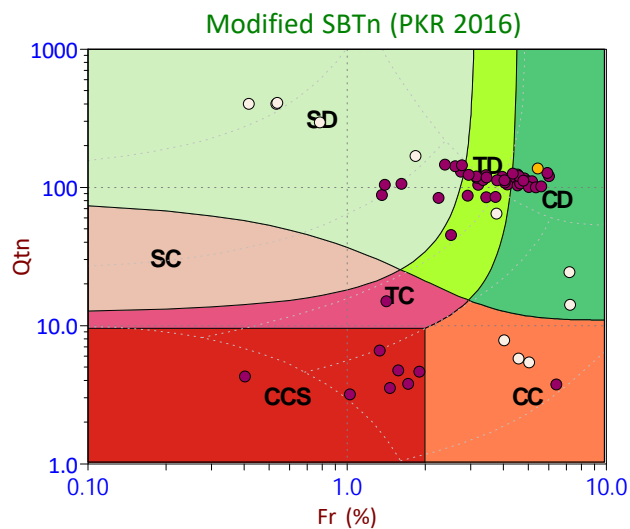


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

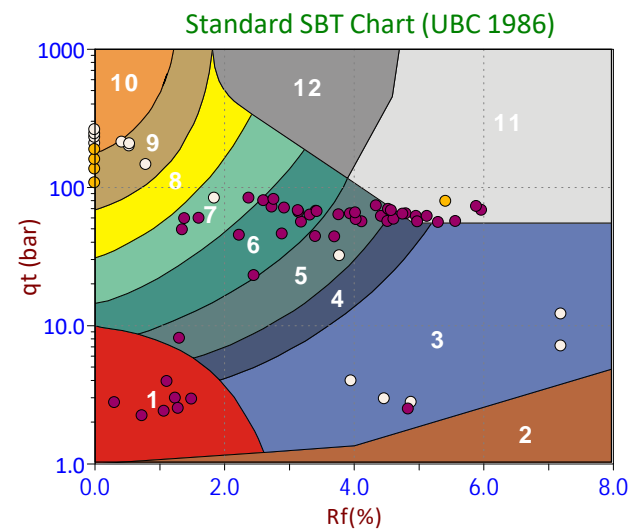
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



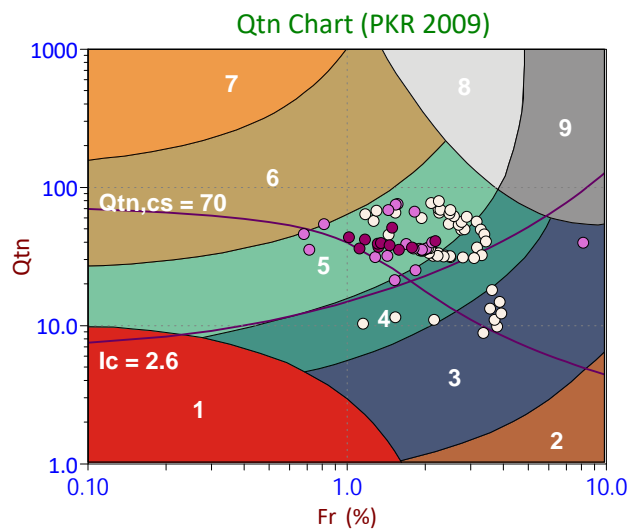
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

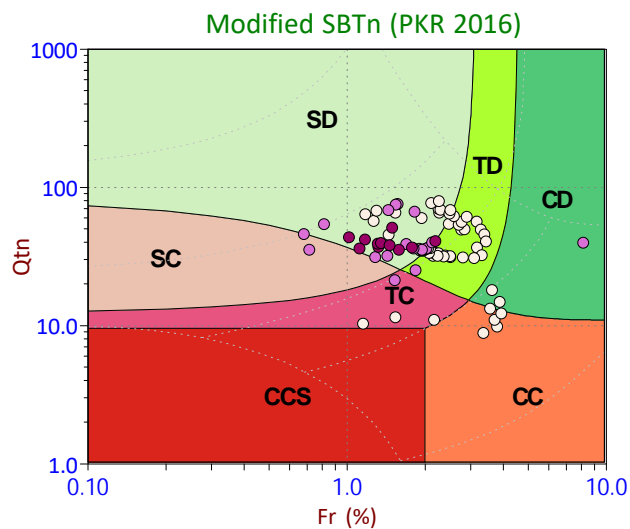


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

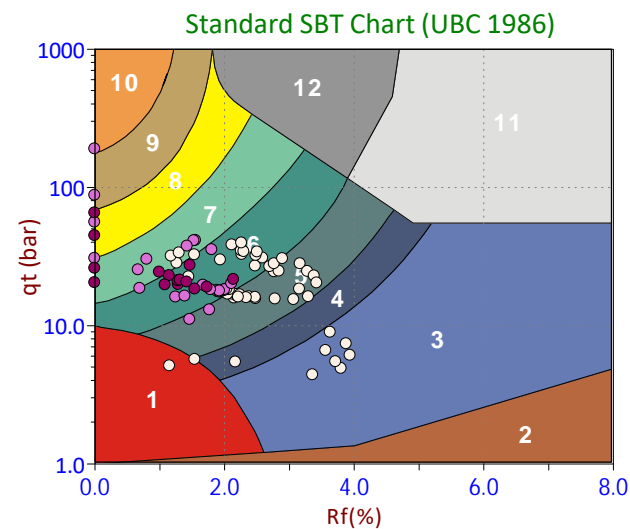
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



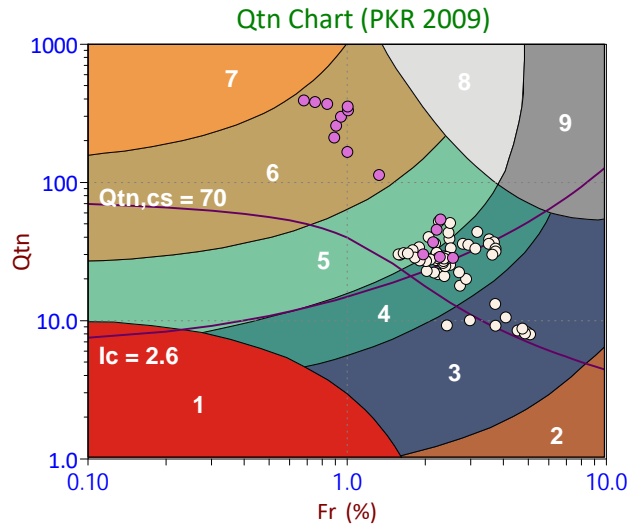
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

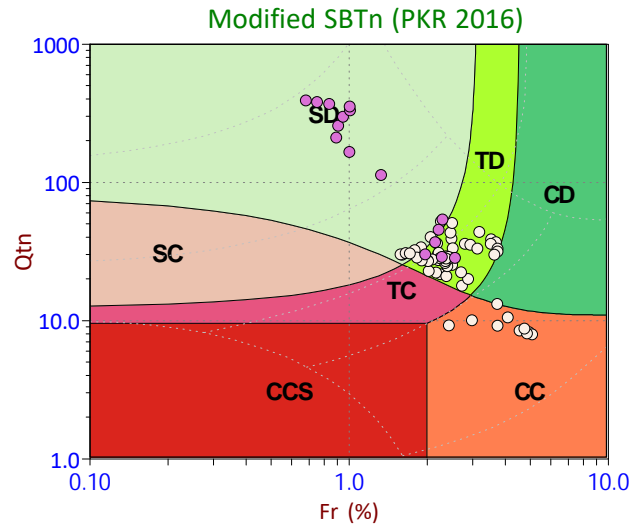


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
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- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

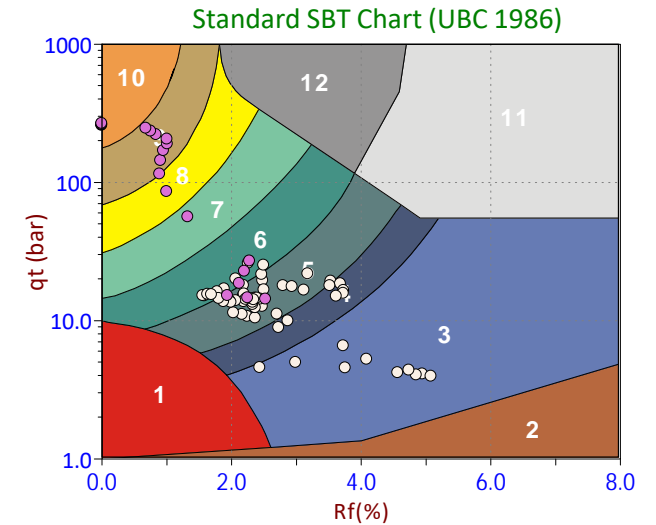
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



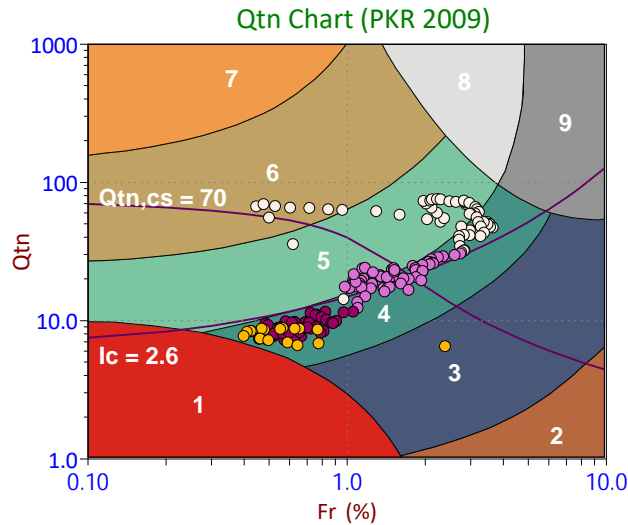
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

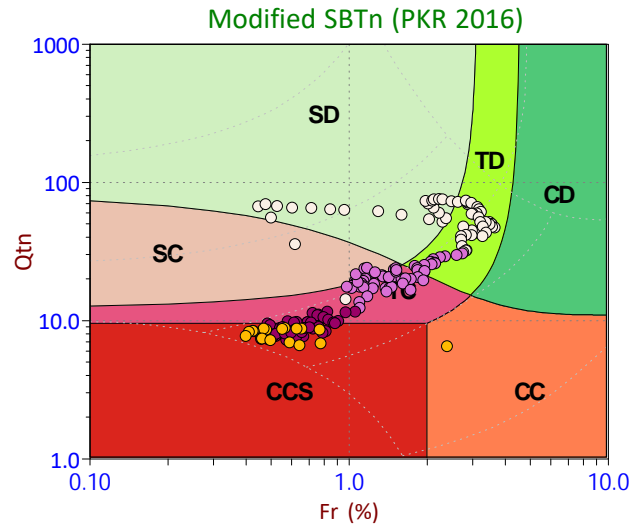


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
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- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

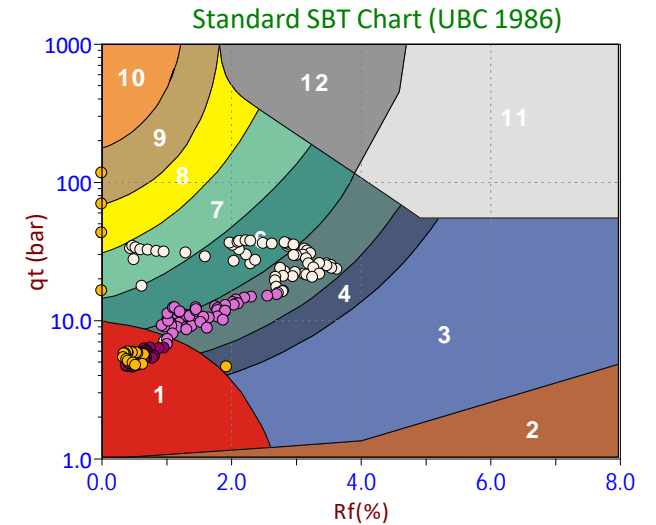
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



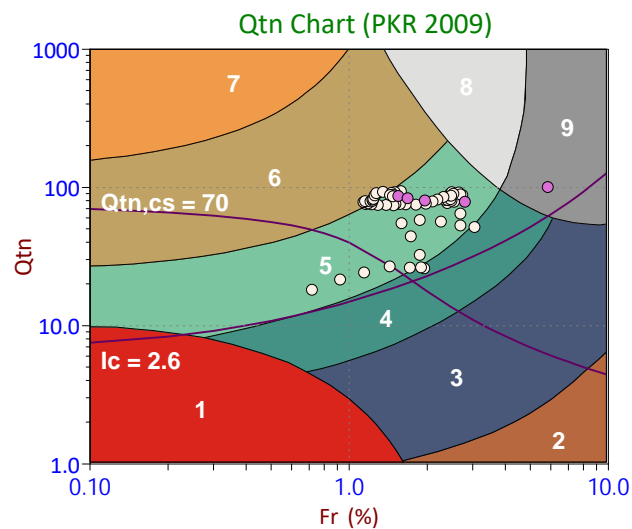
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

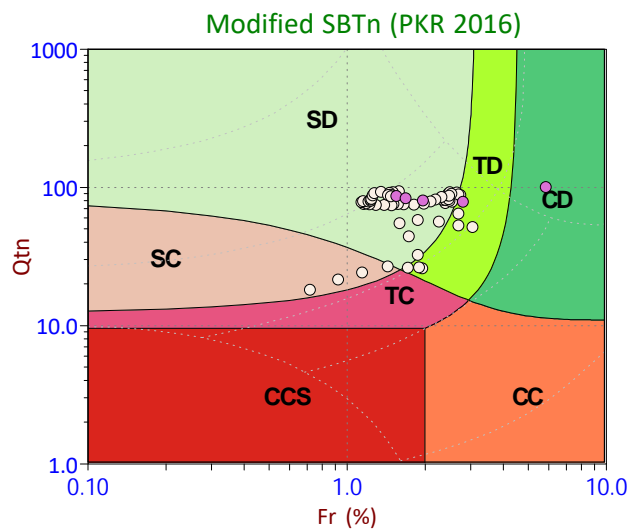


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
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- >50.0 ft

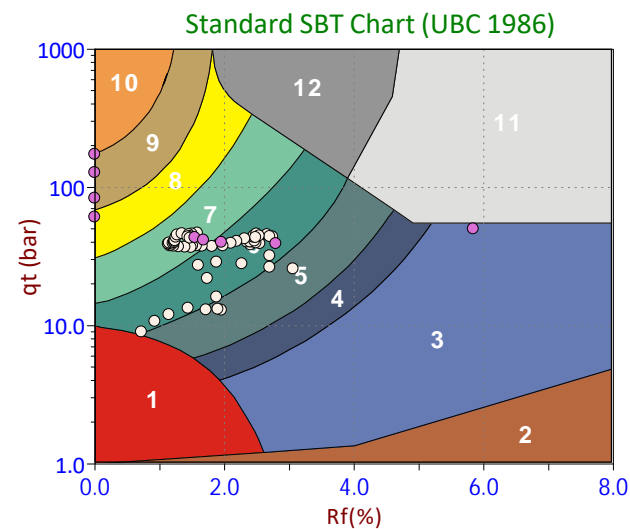
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



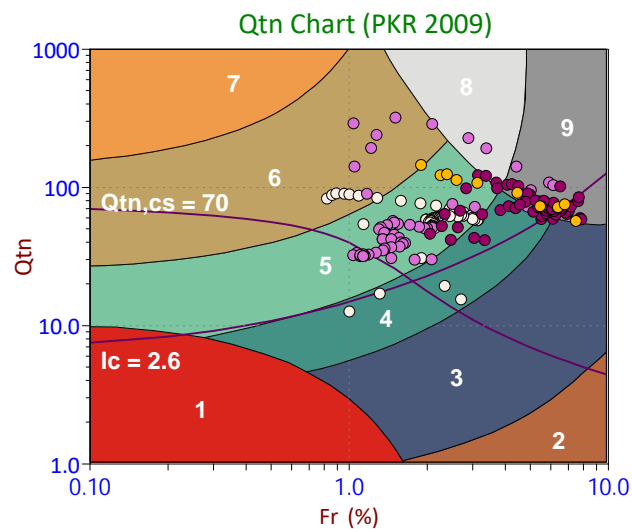
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

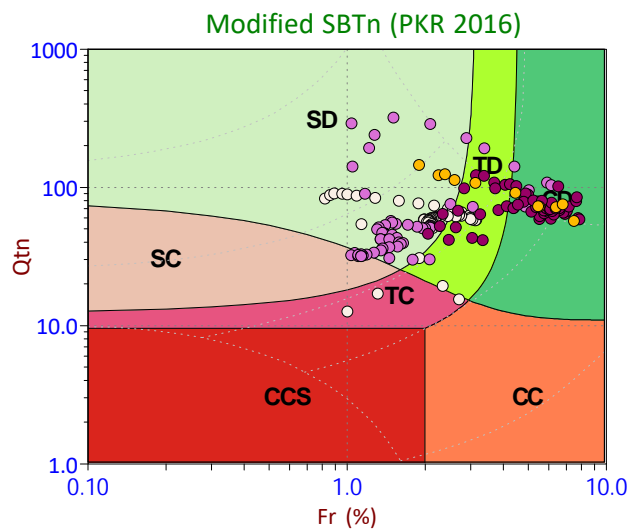


Depth Ranges

- >0.0 to 5.0 ft
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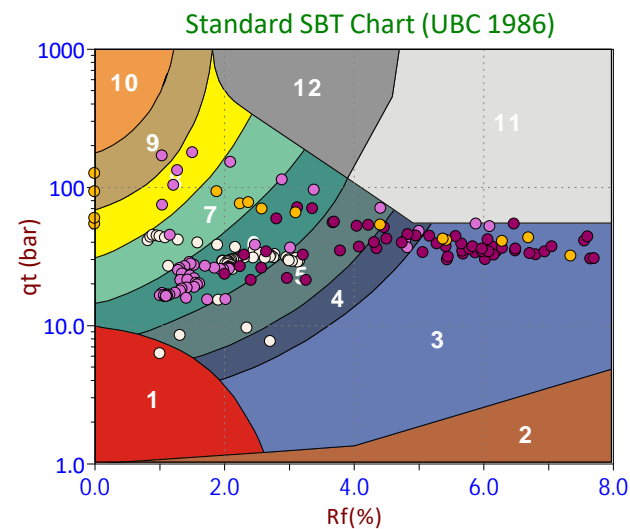
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



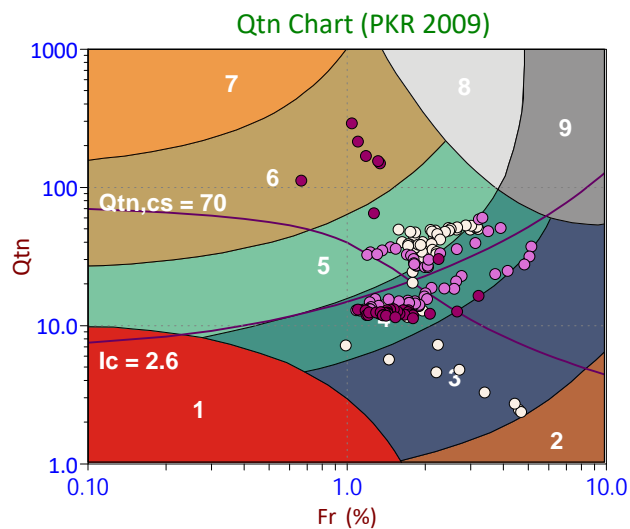
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

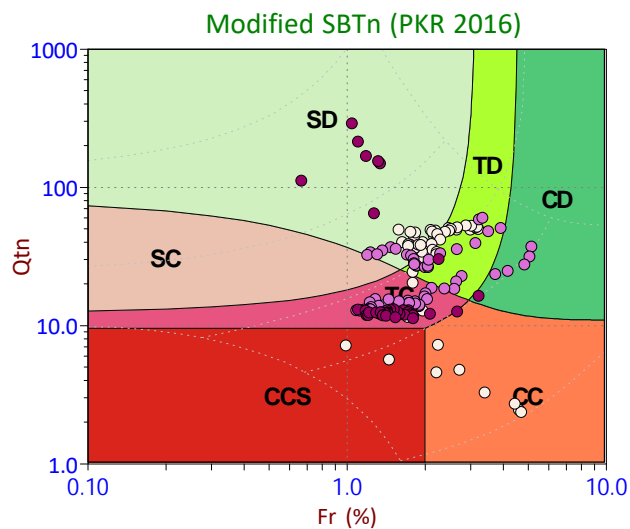


Depth Ranges

- >0.0 to 5.0 ft
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- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

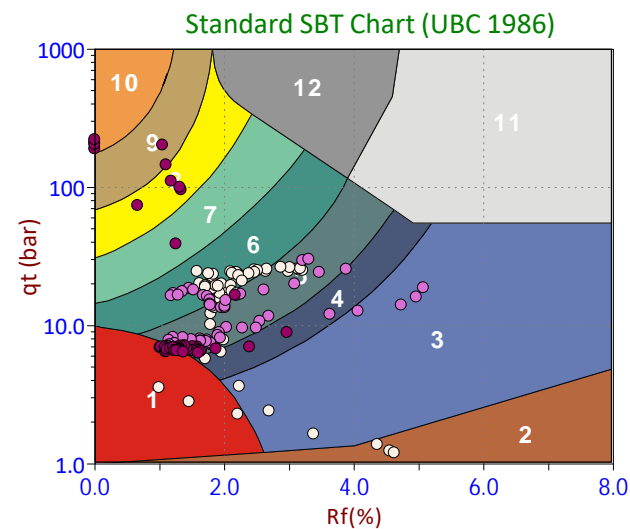
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
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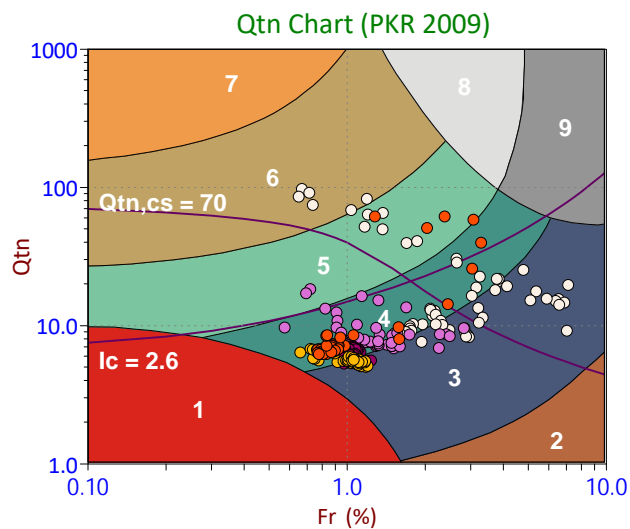
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
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Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

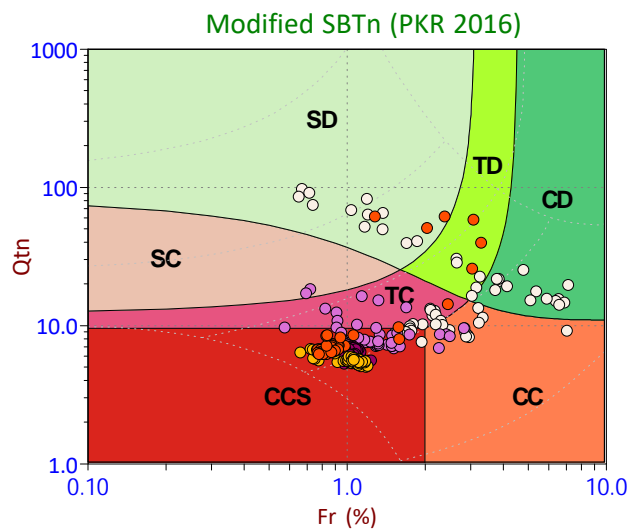


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
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- >20.0 to 25.0 ft
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- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

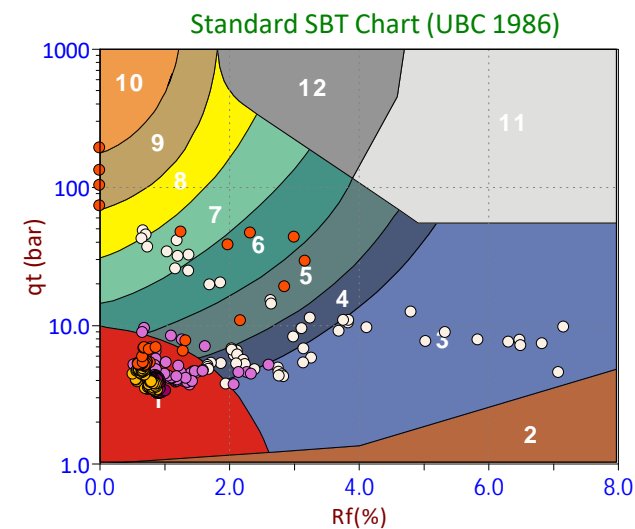
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



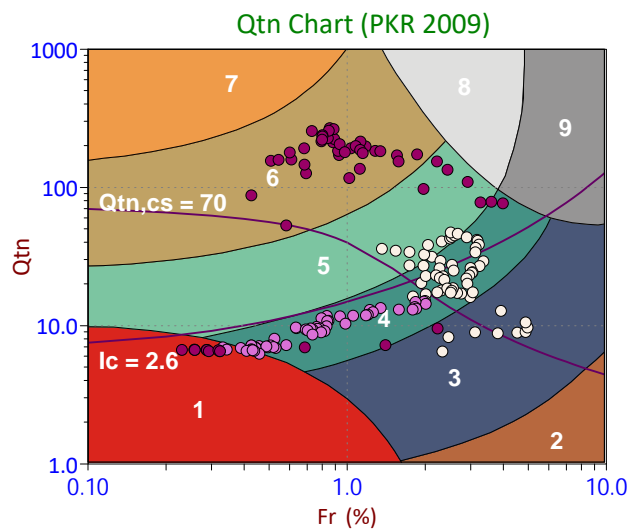
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

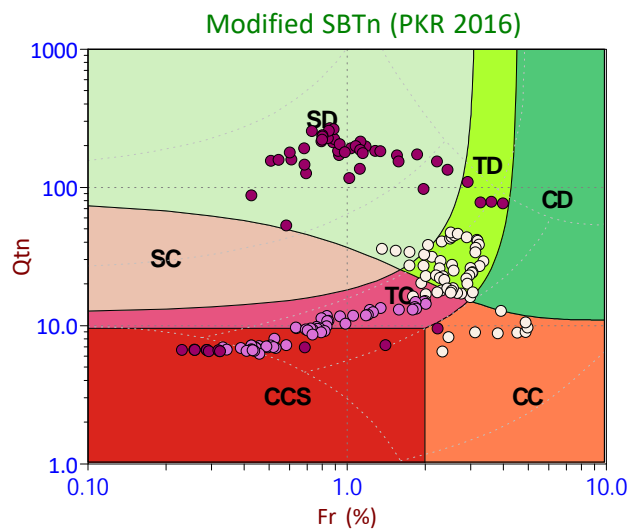


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

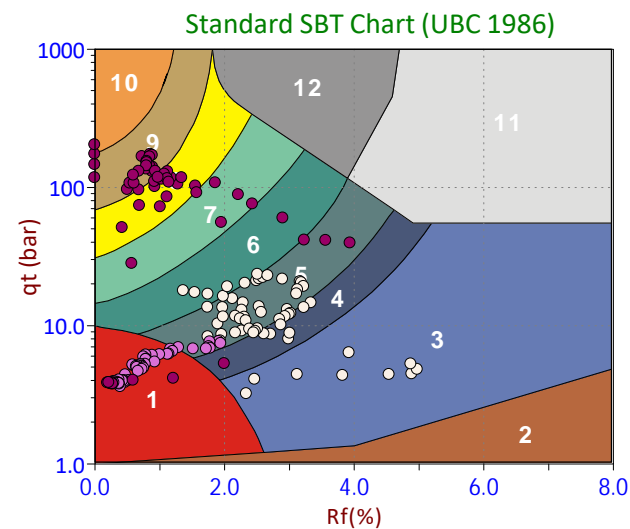
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



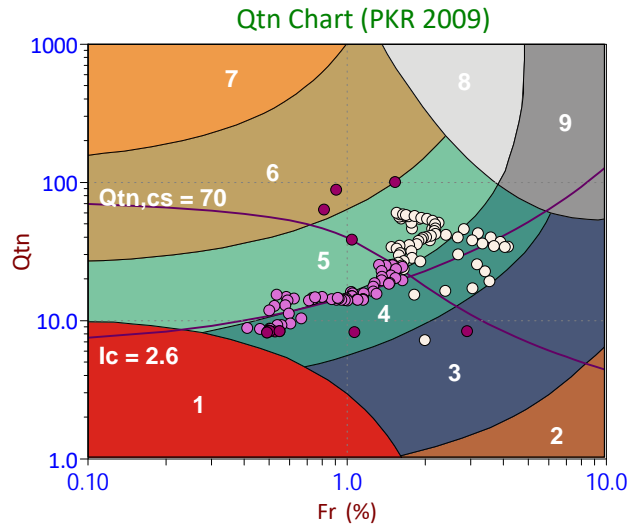
Legend

- CCS (Cont. sensitive clay like)
- CC (Cont. clay like)
- TC (Cont. transitional)
- SC (Cont. sand like)
- CD (Dil. clay like)
- TD (Dil. transitional)
- SD (Dil. sand like)



Legend

- Sensitive Fines
- Organic Soil
- Clay
- Silty Clay
- Clayey Silt
- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

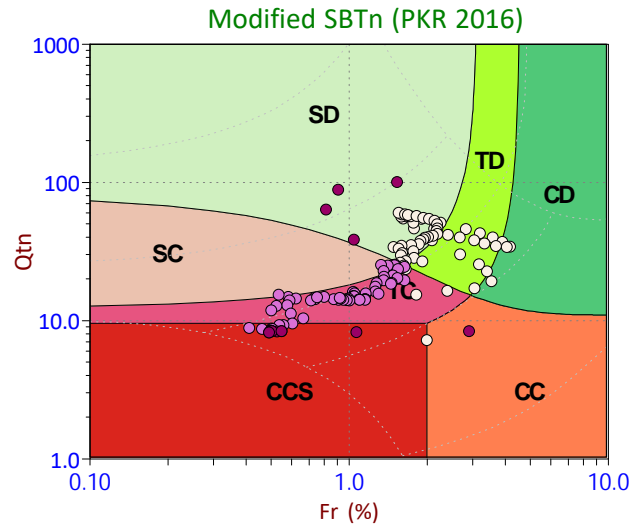


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

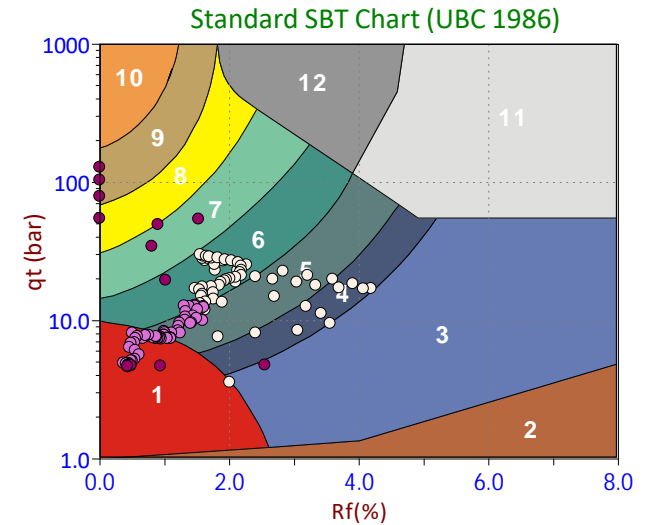
Legend

- Sensitive, Fine Grained
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- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



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- TD (Dil. transitional)
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- Cemented Sand



Haley & Aldrich

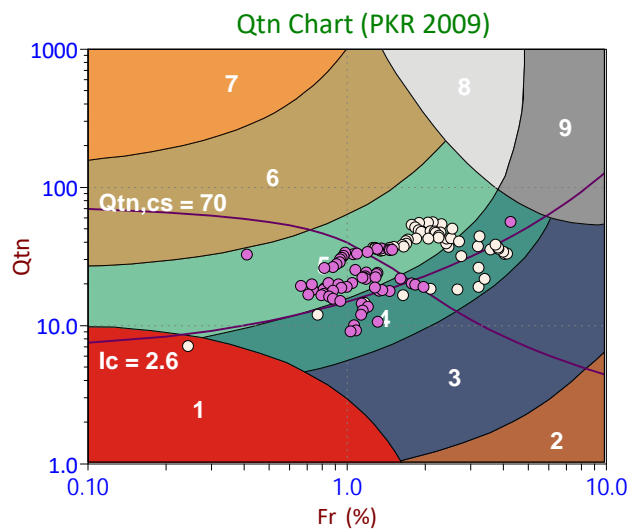
Job No: 20-53-21525

Date: 2020-10-30 10:15

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-125

Cone: 524:T375F10U500

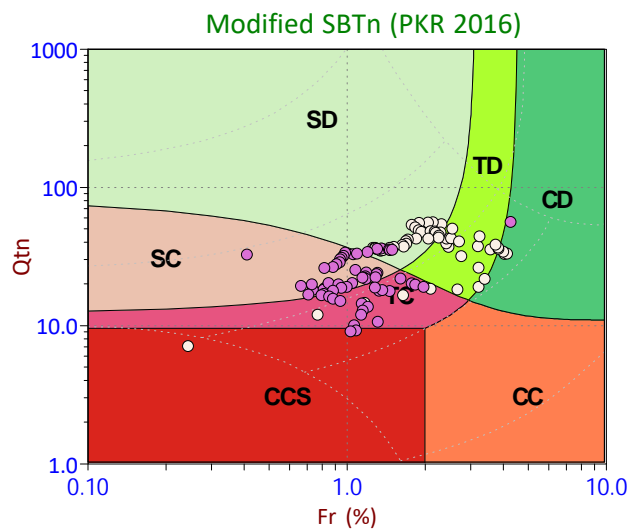


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

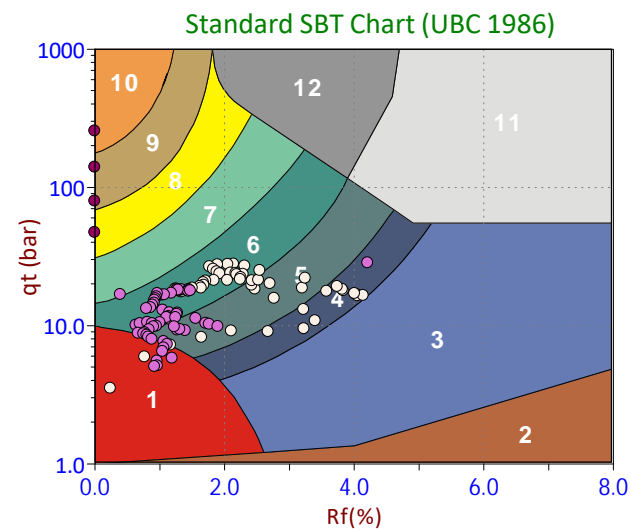
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
- Stiff Sand to Clayey Sand
- Very Stiff Fine Grained



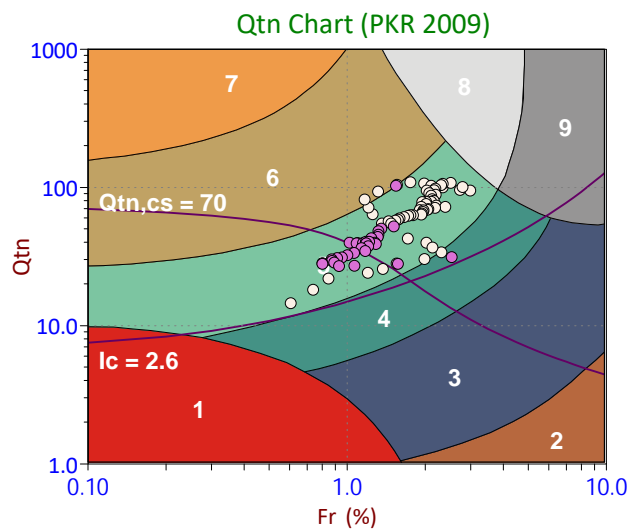
Legend

- CCS (Cont. sensitive clay like)
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Legend

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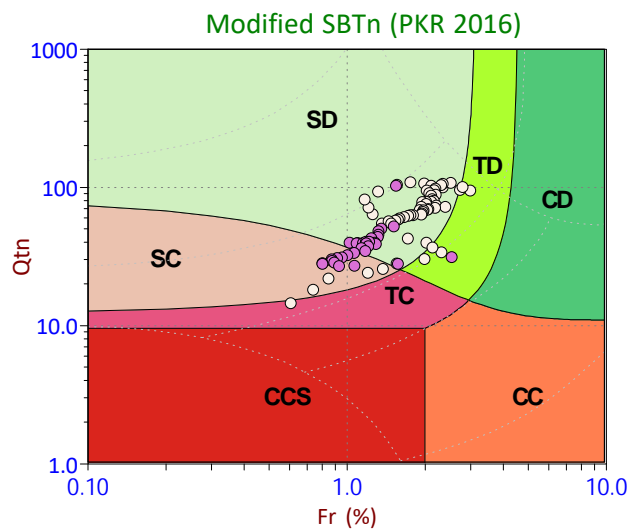


Depth Ranges

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- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

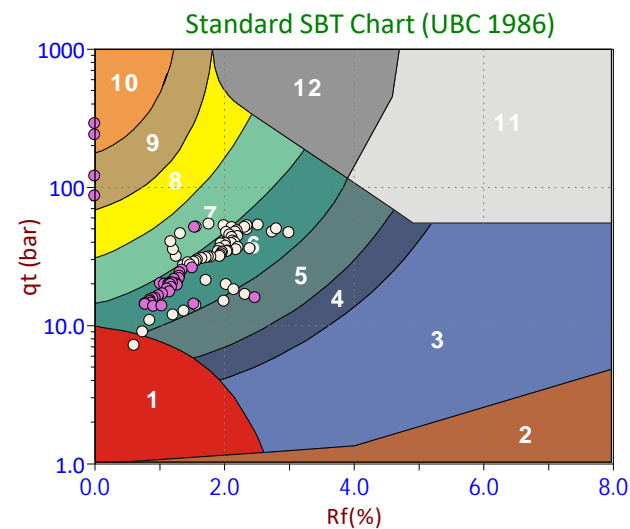
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
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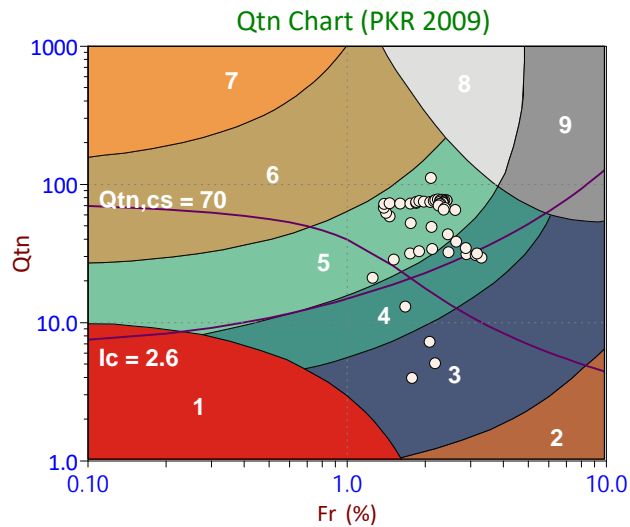
Legend

- CCS (Cont. sensitive clay like)
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- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

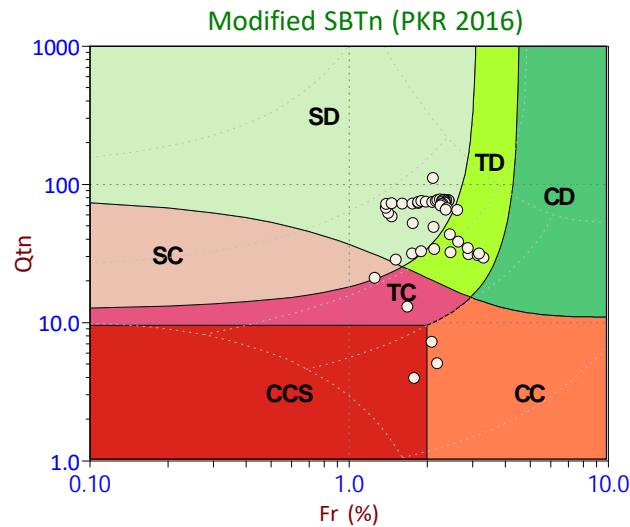


Depth Ranges

- >0.0 to 5.0 ft
- >5.0 to 10.0 ft
- >10.0 to 15.0 ft
- >15.0 to 20.0 ft
- >20.0 to 25.0 ft
- >25.0 to 30.0 ft
- >30.0 to 35.0 ft
- >35.0 to 40.0 ft
- >40.0 to 45.0 ft
- >45.0 to 50.0 ft
- >50.0 ft

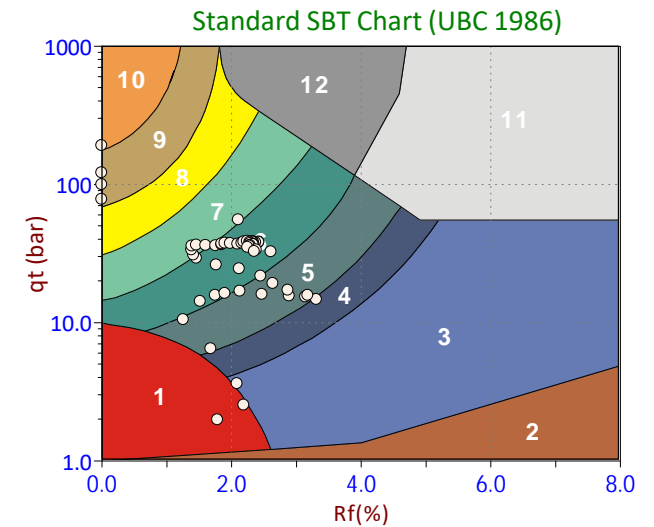
Legend

- Sensitive, Fine Grained
- Organic Soils
- Clays
- Silt Mixtures
- Sand Mixtures
- Sands
- Gravelly Sand to Sand
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- CD (Dil. clay like)
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Legend

- Sensitive Fines
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- Silt
- Sandy Silt
- Silty Sand/Sand
- Sand
- Gravelly Sand
- Stiff Fine Grained
- Cemented Sand

Pore Pressure Dissipation Summary and Pore Pressure Dissipation Plots



Job No: 20-53-21525
 Client: Haley & Aldrich
 Project: I-395 & Route 9 Connector, Brewer & Eddington, ME
 Start Date: 26-Oct-2020
 End Date: 02-Nov-2020

CPT_u PORE PRESSURE DISSIPATION SUMMARY

Sounding ID	File Name	Cone Area (cm ²)	Duration (s)	Test Depth (ft)	Estimated Equilibrium Pore Pressure U _{eq} (ft)	Calculated Phreatic Surface (ft)	Estimated Phreatic Surface (ft)	t ₅₀ ^a (s)	Assumed Rigidity Index (I _r)	c _h ^b (cm ² /min)	Refer to Notation
CPT20-101B	20-53-21525_CP101B	15	4800	30.02	13.0		17.0	2662	100	0.3	
CPT20-101B	20-53-21525_CP101B	15	3595	48.15	31.1		17.0	1572	100	0.4	
SCPT20-101B	20-53-21525_SP101B	15	2700	8.12	4.6		3.5	2670	100	0.3	c
SCPT20-103	20-53-21525_SP103	15	4320	14.03	10.0		4.0	2612	100	0.3	
SCPT20-103	20-53-21525_SP103	15	4060	32.07	28.1		4.0	3354	100	0.2	
SCPT20-104	20-53-21525_SP104	15	1800	11.07	6.1		5.0	942	100	0.7	c
CPT20-105	20-53-21525_CP105	15	305	13.45	10.9	2.5					
CPT20-122	20-53-21525_CP122	15	3600	18.21	15.2		3.0	3160	100	0.2	
Totals	8 dissipations		419.7 min								

a. Time is relative to where umax occurred.

b. Houlsby and Teh, 1991.

c. The pore pressure dissipation test was completed in a split spoon sample hole and the soil is disturbed.



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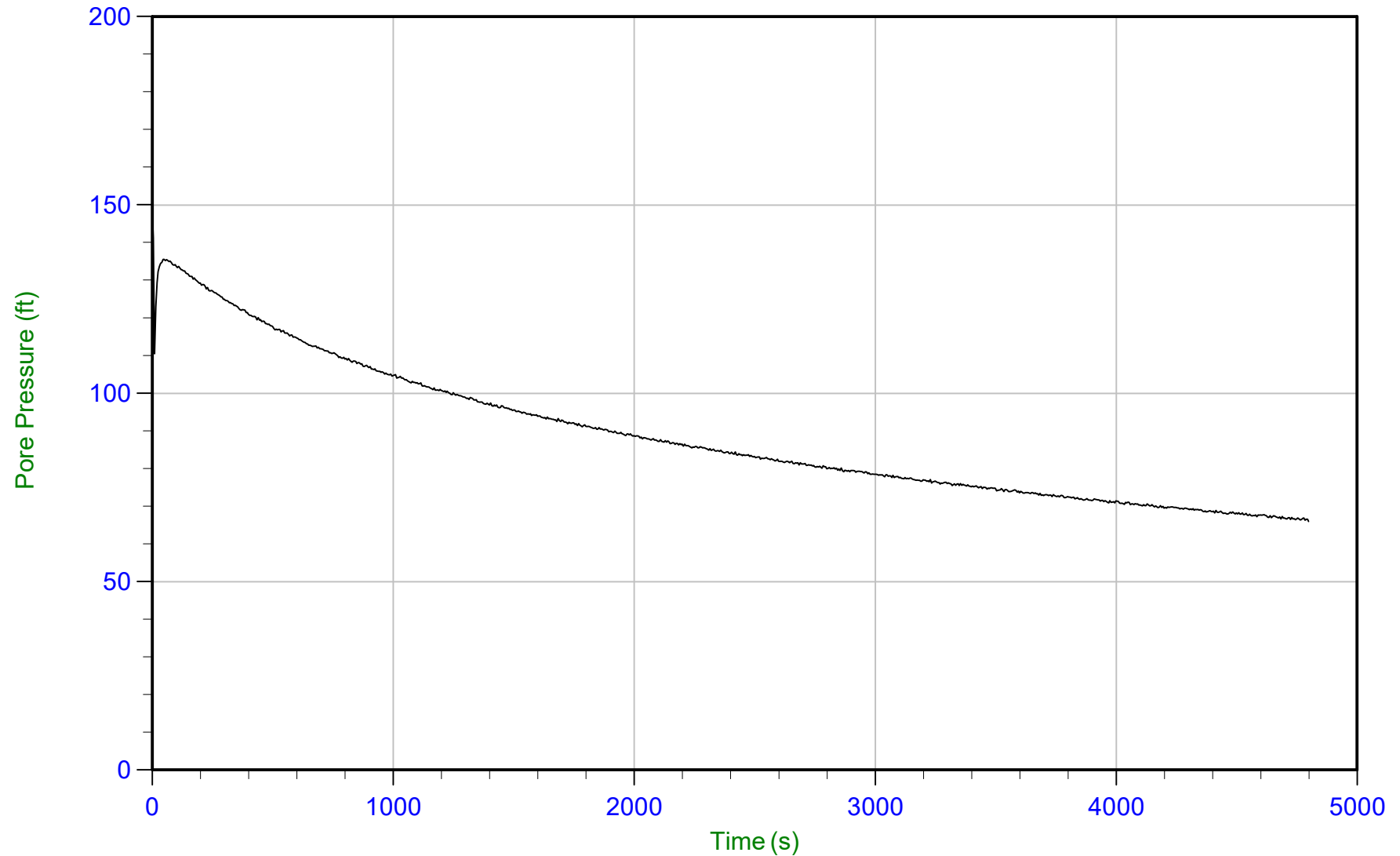
Job No: 20-53-21525

Date: 11/02/2020 10:10

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101B

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_CP101B.PPD

Depth: 9.150 m / 30.019 ft

Duration: 4800.0 s

u Min: 66.0 ft

u Max: 149.7 ft

u Final: 66.0 ft

WT: 5.182 m / 17.000 ft

Ueq: 13.0 ft

U(50): 81.36 ft

T(50): 2661.8 s

Ir: 100

Ch: 0.3 cm²/min



Haley & Aldrich

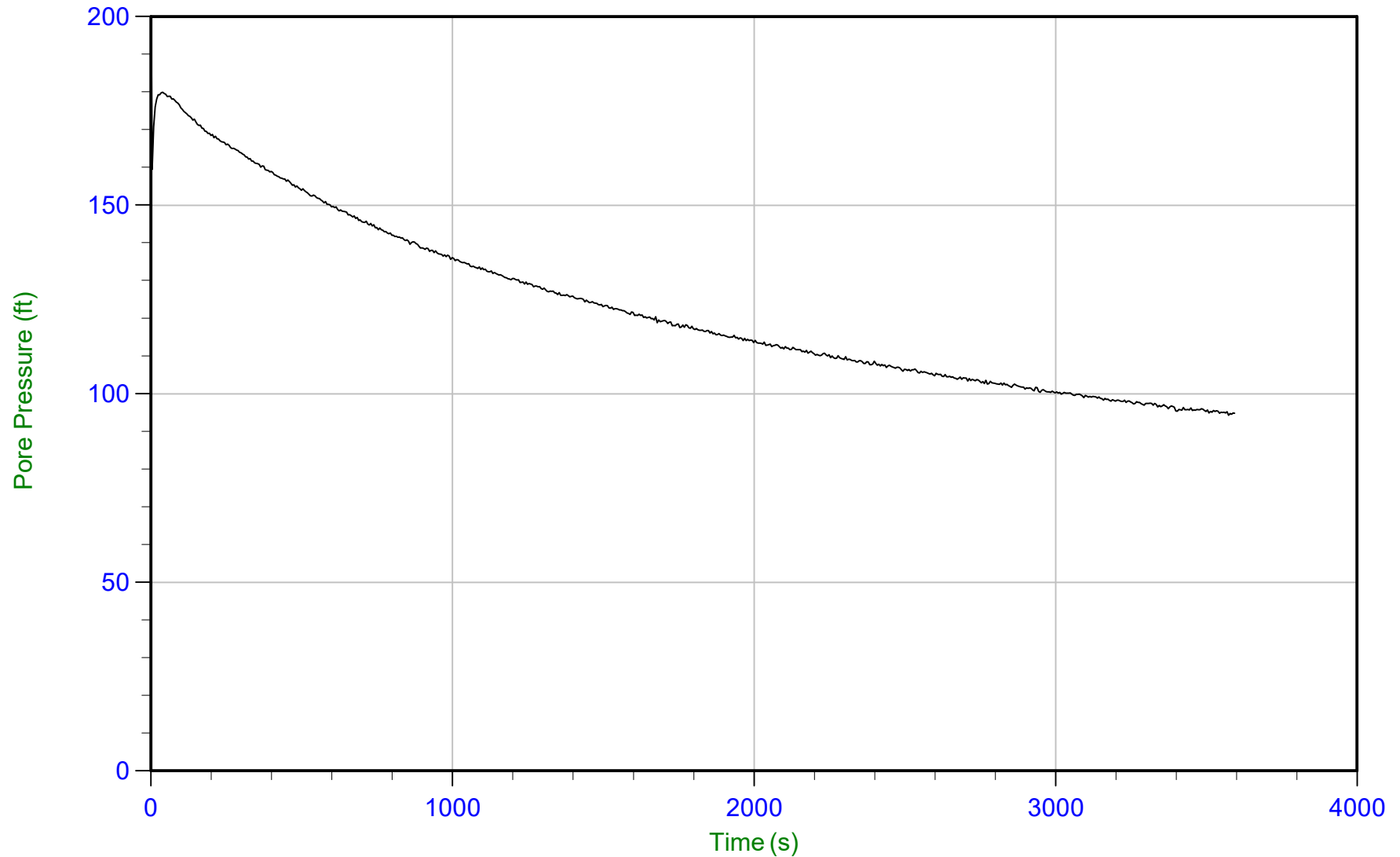
Job No: 20-53-21525

Date: 11/02/2020 10:10

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-101B

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_CP101B.PPD

Depth: 14.675 m / 48.146 ft

Duration: 3595.0 s

u Min: 94.3 ft

u Max: 212.7 ft

u Final: 94.8 ft

WT: 5.182 m / 17.000 ft

Ueq: 31.1 ft

U(50): 121.93 ft

T(50): 1572.3 s

Ir: 100

Ch: 0.4 cm²/min



Haley & Aldrich

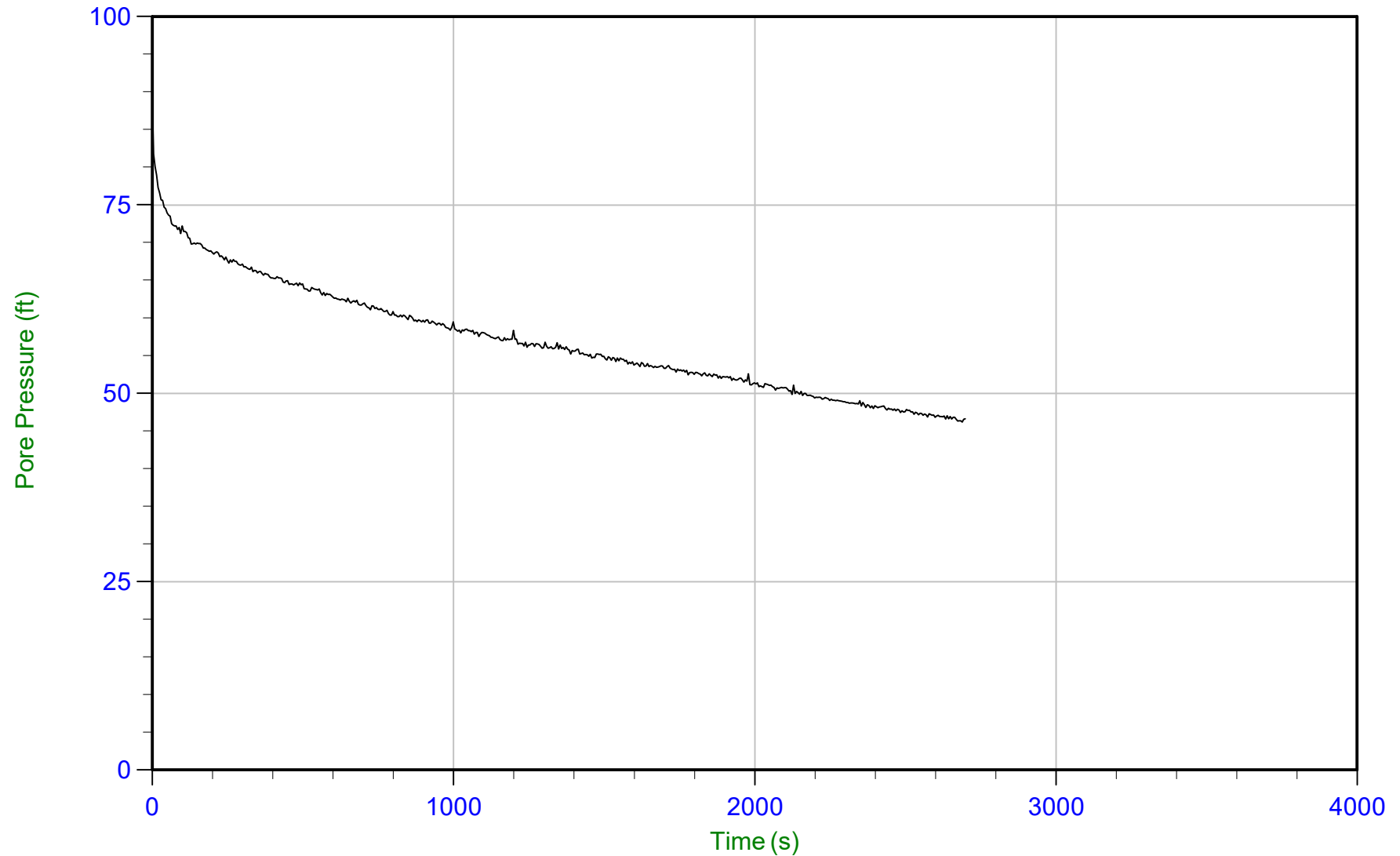
Job No: 20-53-21525

Date: 10/28/2020 08:27

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-101B

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_SP101B.PPD

Depth: 2.475 m / 8.120 ft

Duration: 2700.0 s

u Min: 46.2 ft

u Max: 88.4 ft

u Final: 46.6 ft

WT: 1.067 m / 3.500 ft

Ueq: 4.6 ft

U(50): 46.53 ft

T(50): 2670.1 s

Ir: 100

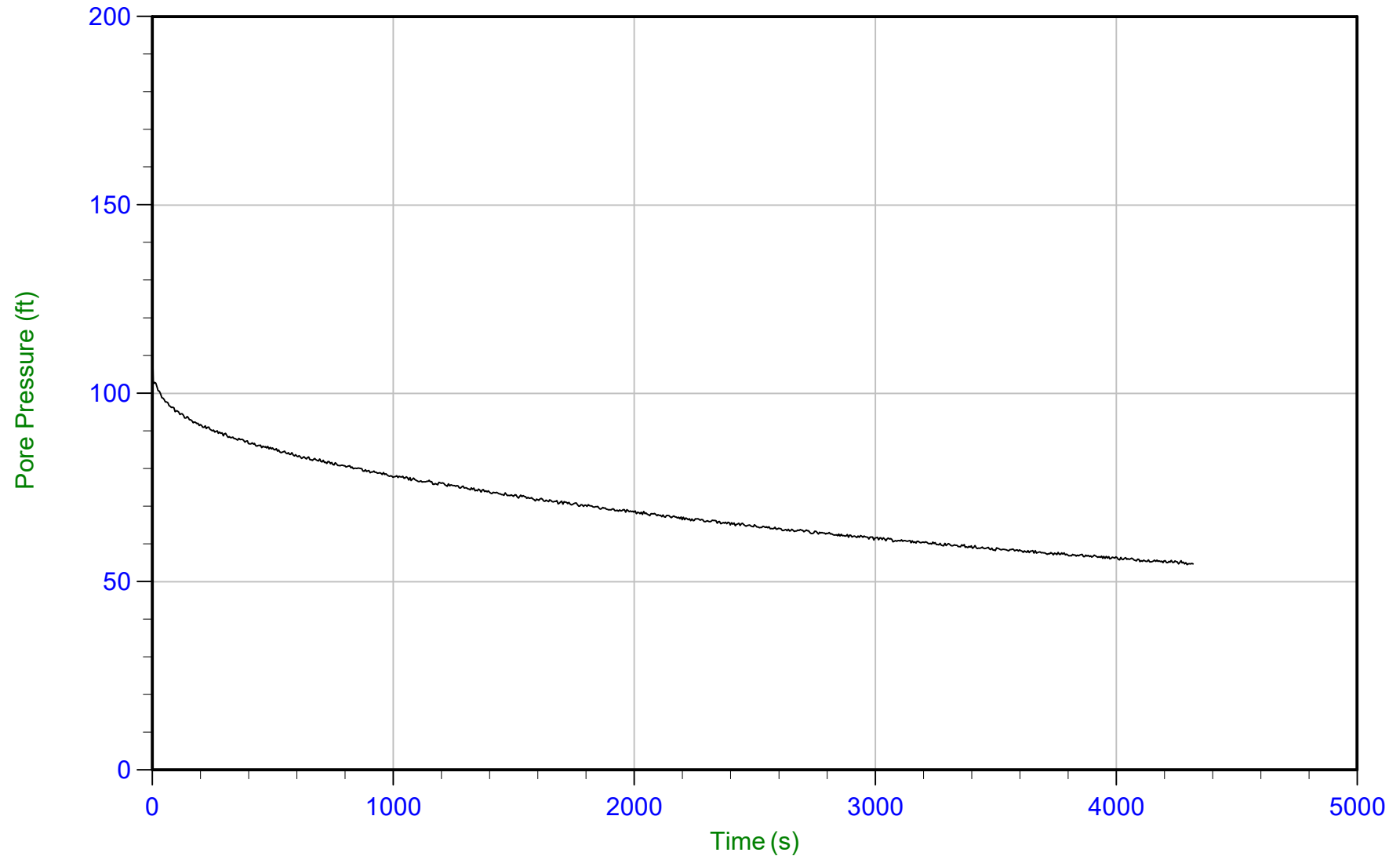
Ch: 0.3 cm²/min



Haley & Aldrich

Job No: 20-53-21525
Date: 11/01/2020 12:46
Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103
Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_SP103.PPD
Depth: 4.275 m / 14.025 ft
Duration: 4320.0 s

u Min: 54.5 ft
u Max: 117.7 ft
u Final: 54.7 ft

WT: 1.219 m / 4.000 ft
Ueq: 10.0 ft
U(50): 63.85 ft

T(50): 2611.9 s
Ir: 100
Ch: 0.3 cm²/min



Haley & Aldrich

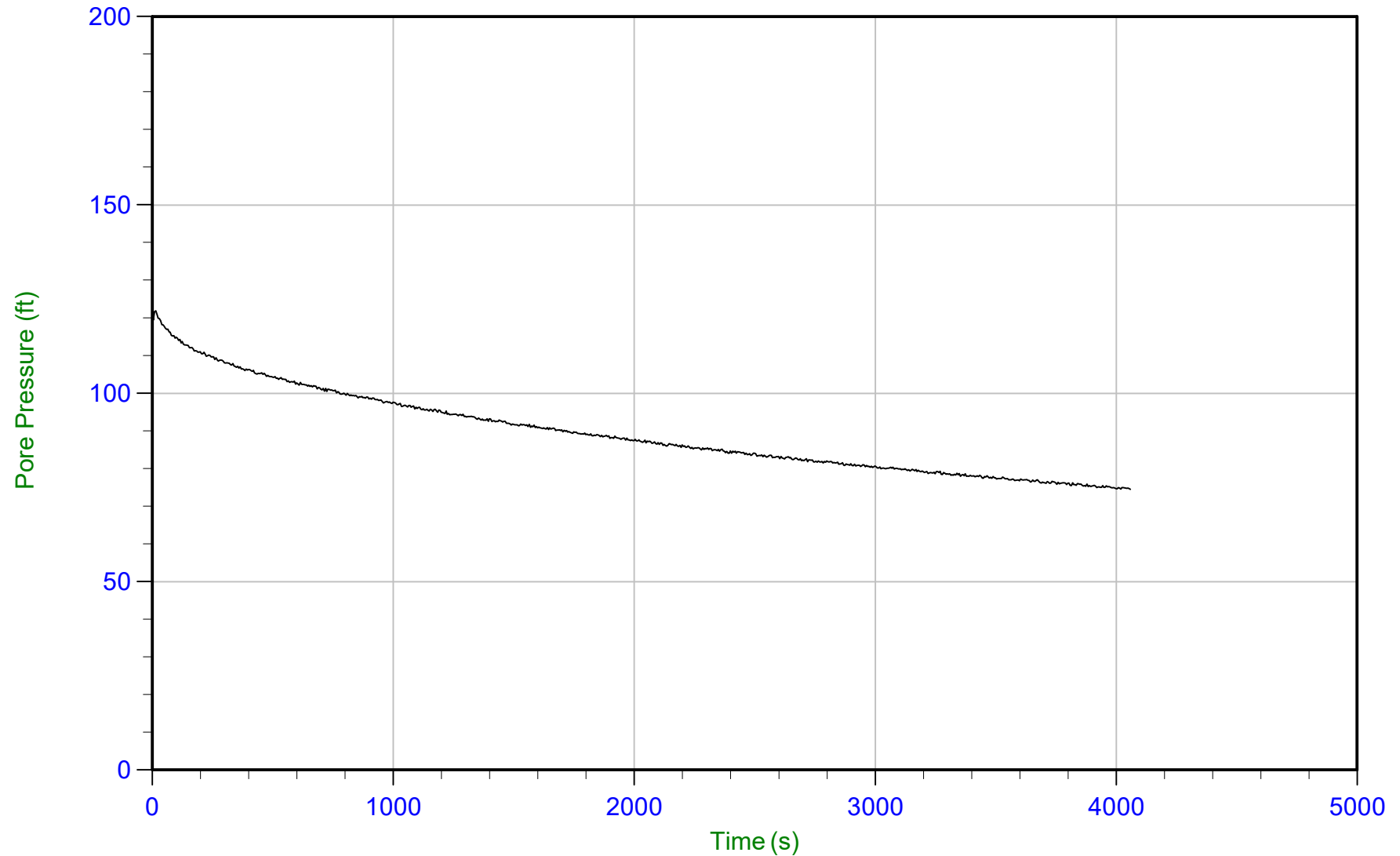
Job No: 20-53-21525

Date: 11/01/2020 12:46

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-103

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_SP103.PPD

Depth: 9.775 m / 32.070 ft

Duration: 4060.0 s

u Min: 74.5 ft

u Max: 128.2 ft

u Final: 74.5 ft

WT: 1.219 m / 4.000 ft

Ueq: 28.1 ft

U(50): 78.15 ft

T(50): 3353.7 s

Ir: 100

Ch: 0.2 cm²/min



Haley & Aldrich

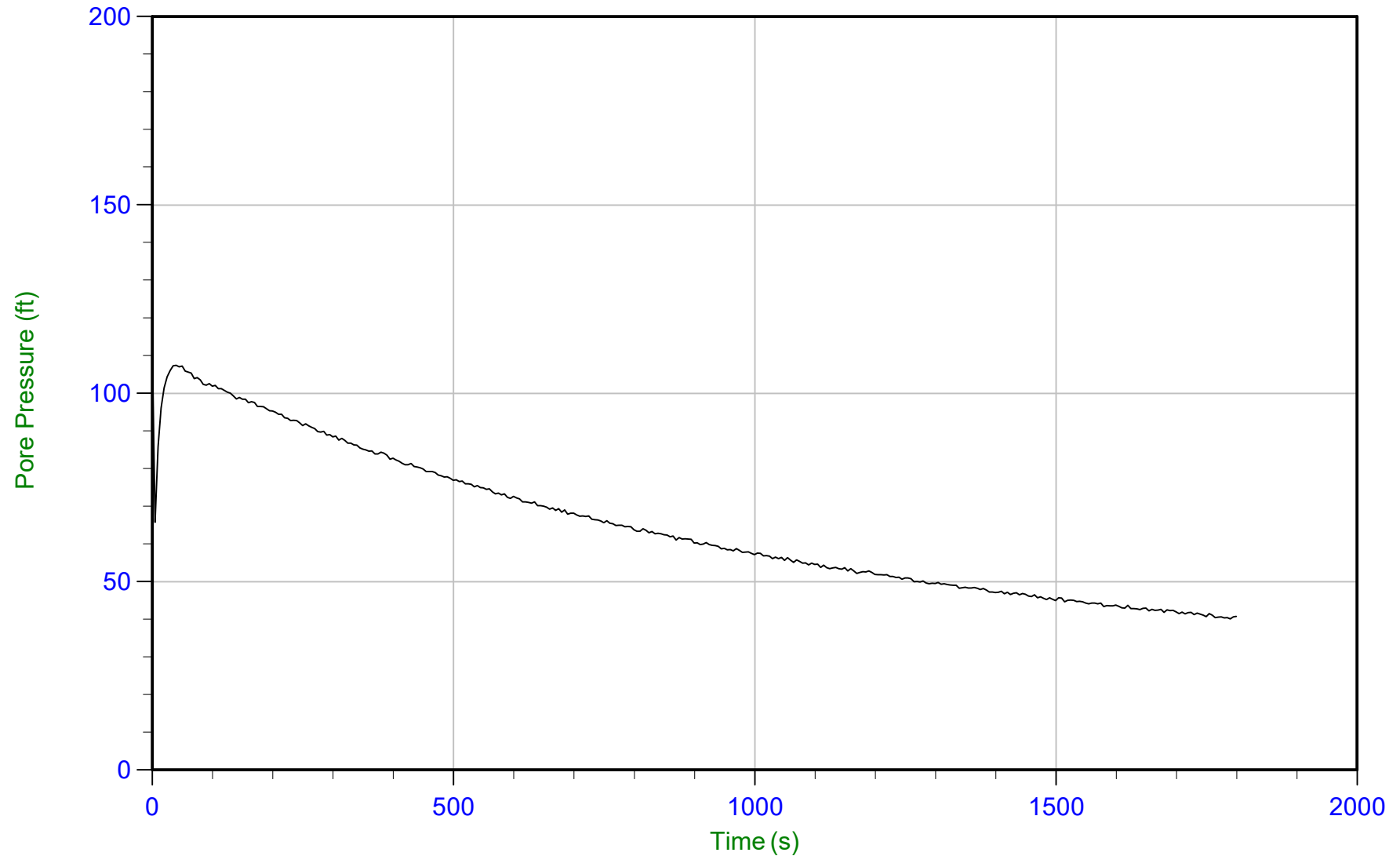
Job No: 20-53-21525

Date: 11/01/2020 08:08

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: SCPT20-104

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_SP104.PPD

Depth: 3.375 m / 11.073 ft

Duration: 1800.0 s

u Min: 40.1 ft

u Max: 112.1 ft

u Final: 40.7 ft

WT: 1.524 m / 5.000 ft

Ueq: 6.1 ft

U(50): 59.11 ft

T(50): 941.8 s

Ir: 100

Ch: 0.7 cm²/min



Haley & Aldrich

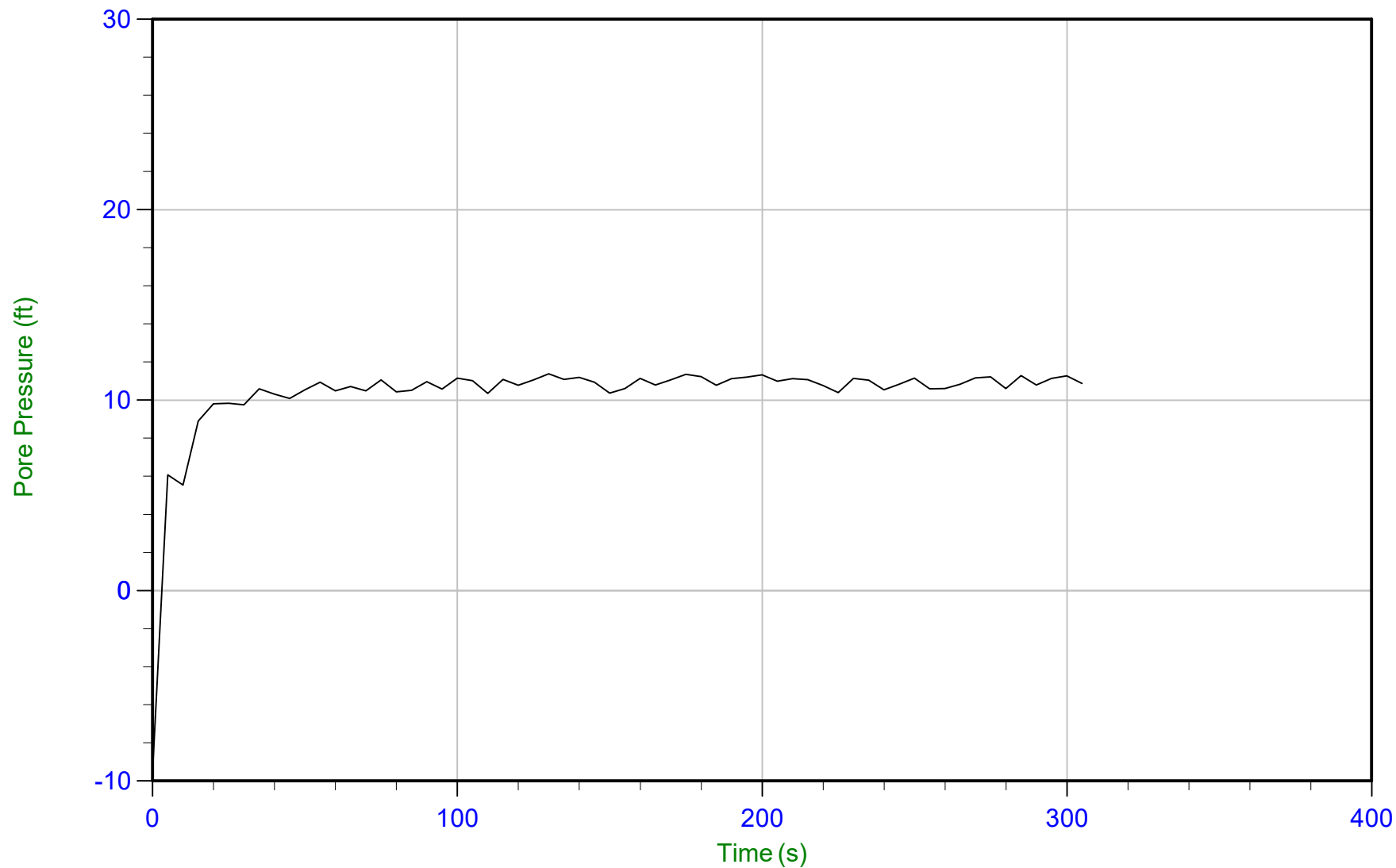
Job No: 20-53-21525

Date: 11/01/2020 10:13

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-105

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_CP105.PPD

Depth: 4.100 m / 13.451 ft

Duration: 305.0 s

u Min: -9.6 ft

u Max: 11.4 ft

u Final: 10.9 ft

WT: 0.763 m / 2.504 ft

Ueq: 10.9 ft



Haley & Aldrich

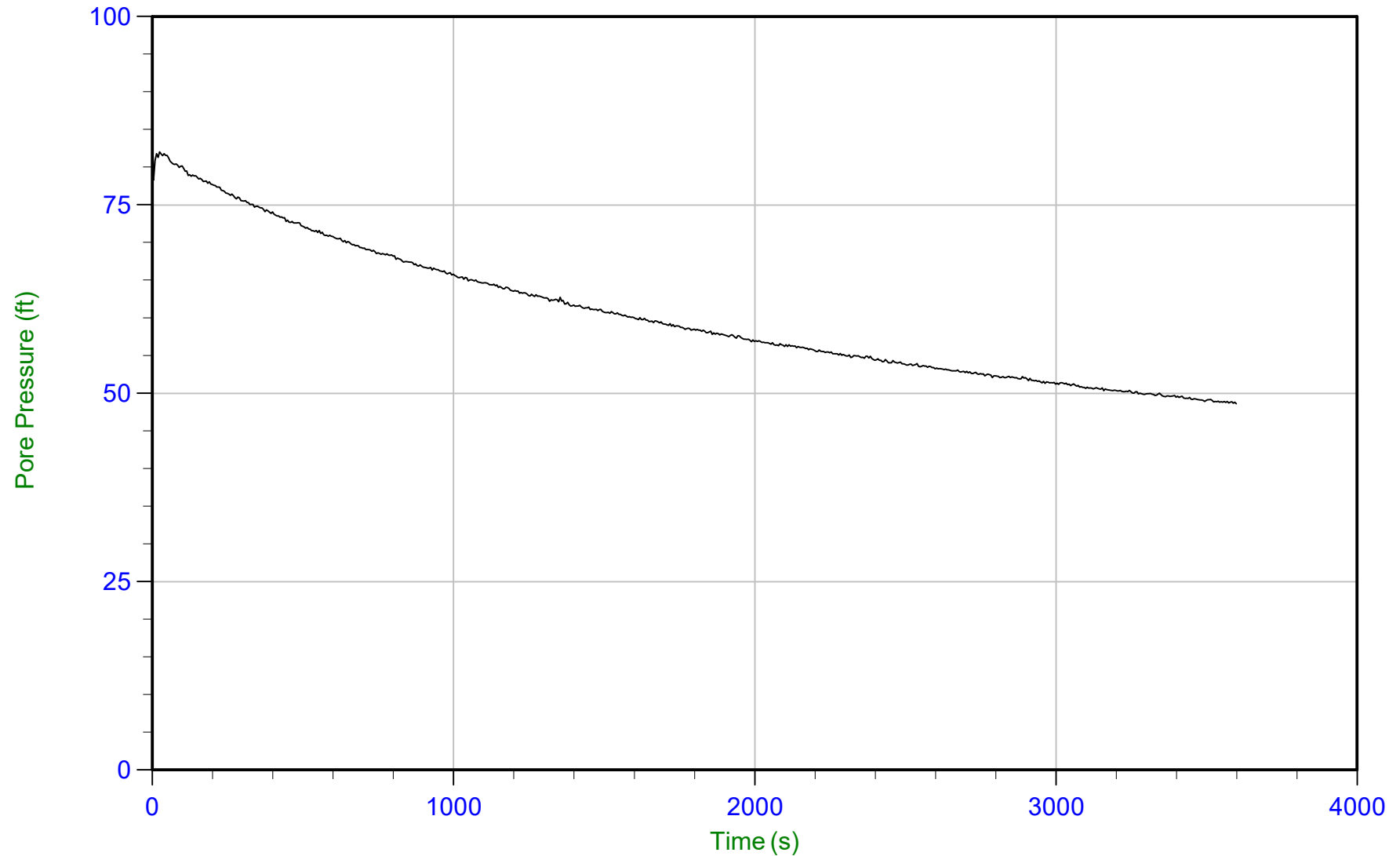
Job No: 20-53-21525

Date: 10/30/2020 12:58

Site: I-395 & Route 9 Connector, Brewer & Eddington, ME

Sounding: CPT20-122

Cone: 524:T375F10U500 Area=15 cm²



Trace Summary:

Filename: 20-53-21525_CP122.PPD

Depth: 5.550 m / 18.208 ft

Duration: 3600.0 s

u Min: 48.6 ft

u Max: 85.5 ft

u Final: 48.6 ft

WT: 0.914 m / 3.000 ft

Ueq: 15.2 ft

U(50): 50.37 ft

T(50): 3159.8 s

Ir: 100

Ch: 0.2 cm²/min

Appendix E

Borehole Geophysical Logging Reports

**BOREHOLE GEOPHYSICAL LOGGING - DATA REPORT
BOREHOLES BB-ECR-202, BB-ECR-205, BB-ELER-202,
BB-ELER-205, HB-BE-231, HB-BE-232, HB-BE-236
BREWER-EDDINGTON I-395/ROUTE 9 CONNECTOR
EDDINGTON, MAINE**

Prepared for:

Haley & Aldrich, Inc.
75 Washington Avenue | Suite 1A
Portland, Maine 04101

Prepared by:

Hager-Richter Geoscience, Inc.
8 Industrial Way - D10
Salem, New Hampshire 03079

File 20RG77
January 2021






Tadpole	Structure Category (Symbol Color)	Description
	Fracture Rank 1 (Light Blue)	Minor Fracture - not distinct and may not be continuous around the borehole
	Fracture Rank 2 (Blue)	Intermediate Fracture - distinct and continuous around the borehole with little or no apparent aperture
	Fracture Rank 3 (Light Green)	Intermediate Fracture - distinct and continuous around the borehole with some apparent aperture
	Fracture Rank 4 (Red)	Major Fracture - distinct with continuous apparent aperture around the borehole
	Foliation or Vein (Orange)	Planar geologic feature interpreted as foliation or a vein

Figure 1. Key to bedrock structure categories.

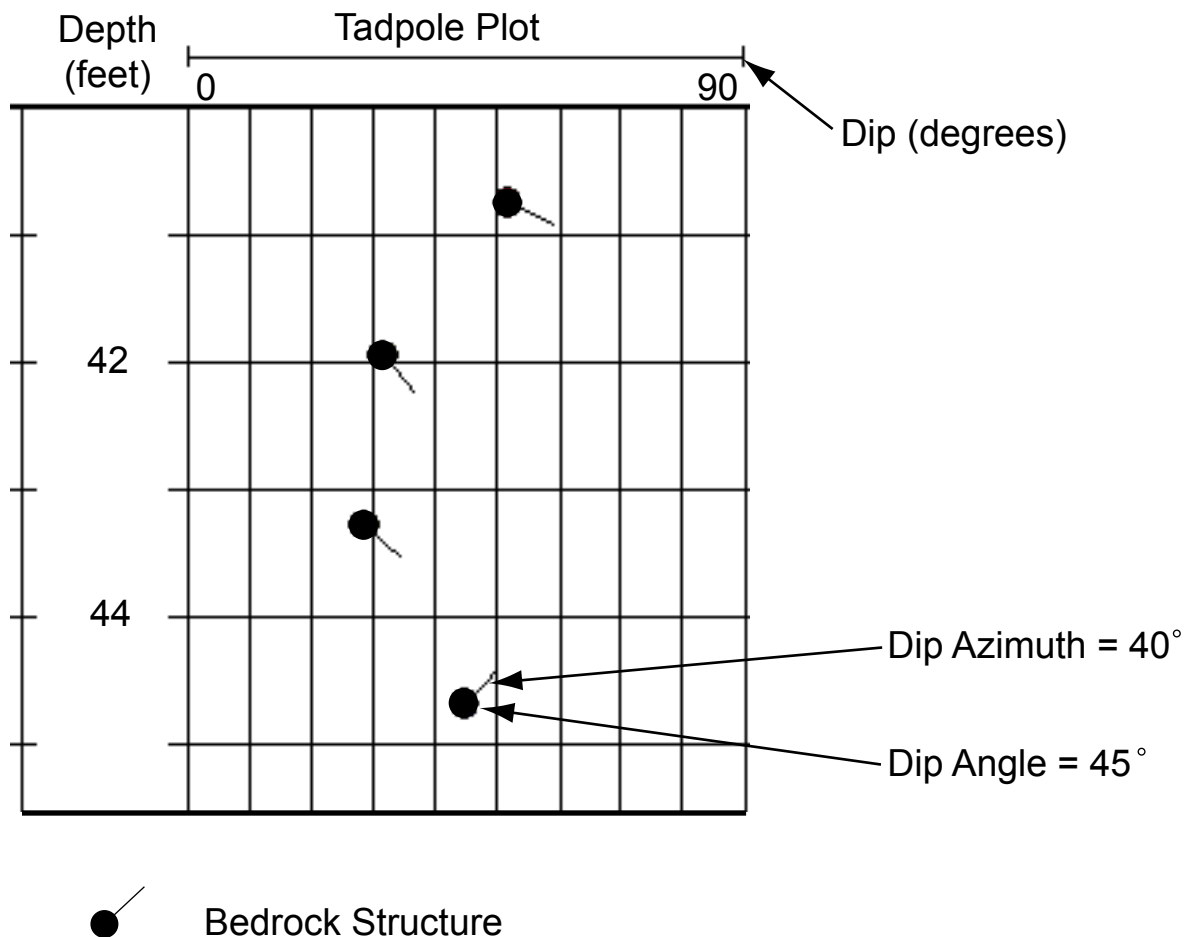


Figure 2. Tadpole plot explanation. The orientation of the bedrock structures is graphically displayed by a tadpole consisting of a circle, the head, and a line, the tail. The position of the head, left to right on the tadpole plot, gives the dip angle of the structure. The left side of the track indicates a dip angle of 0°, and the right side of the track indicates a dip angle of 90° from horizontal. The orientation of the tail gives the dip azimuth of the structure and can be read like a compass. The tail pointing directly up is 0°, north.

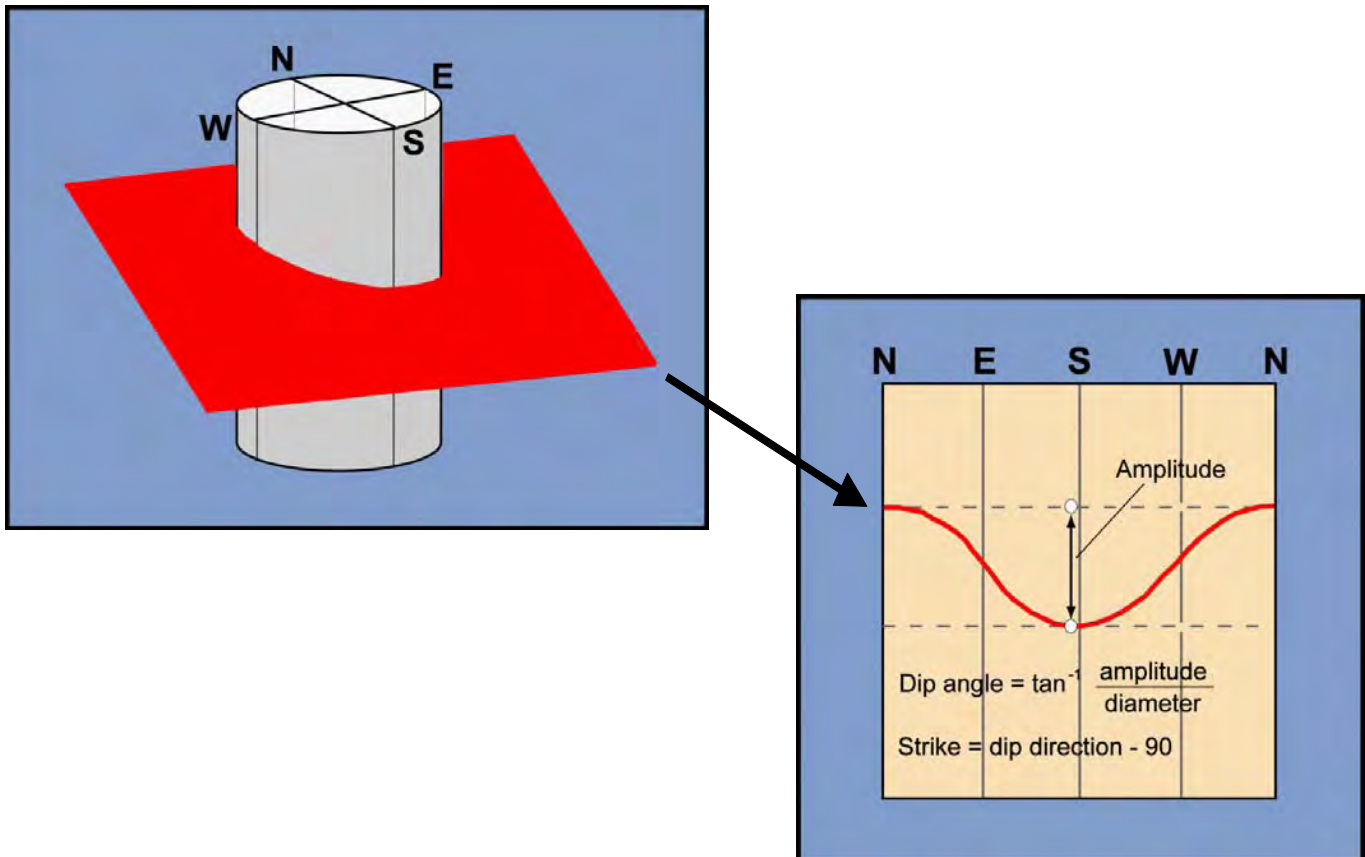


Figure 3. Televiewer Explanation Figure. The image on the left depicts a planar structure in red, such as a fracture or bedding plane, intersected by a borehole. The image on the right depicts the same structure unwrapped as it would be displayed in an optical televiewer (OTV) or acoustic televiewer (ATV) log.

Figure modified from: Garfield, R.L., Day-Lewis, F.D., Gray, M.B., Johnson, C.D., Williams, J.H. and Day-Lewis, A.D.F., 2003, Fractured-Rock Aquifer Characterization within a Regional Geologic Context: Results from the Bucknell University Hydrogeophysics Test Site, GSA Northeastern Section, 38th Annual Meeting, Paper No. 25-19.

HAGER-RICHTER GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

BB-ECR-202 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: November 24, 2020

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Justin Covert
PROJECT REP(S) ON-SITE: Dave Dearden

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

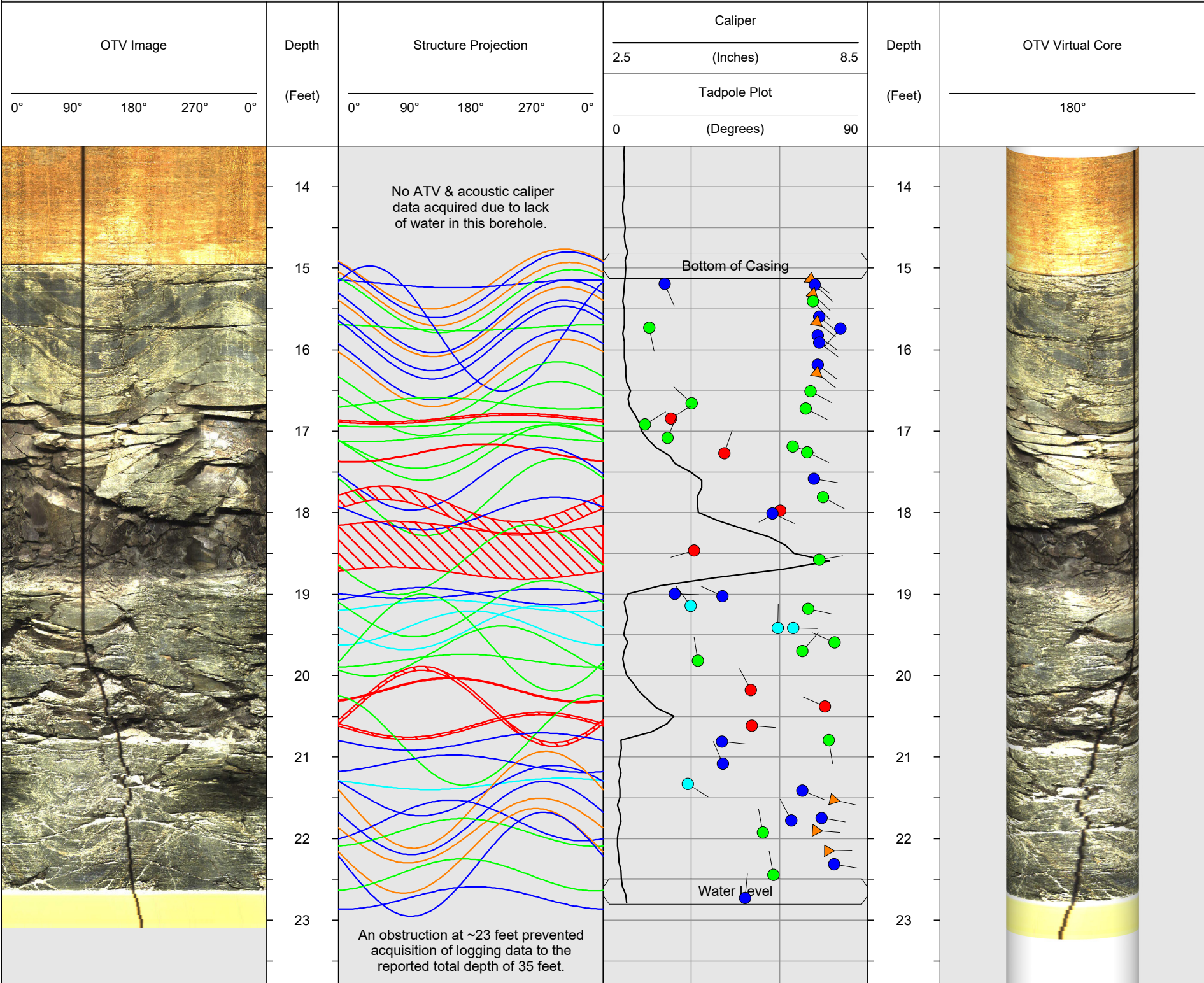
BOREHOLE DIAMETER: 3 Inches

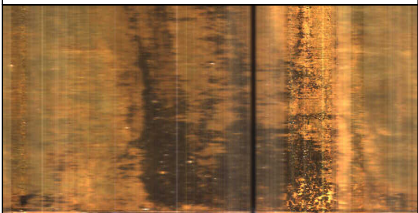
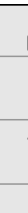
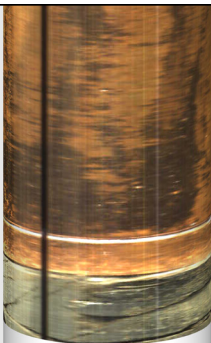
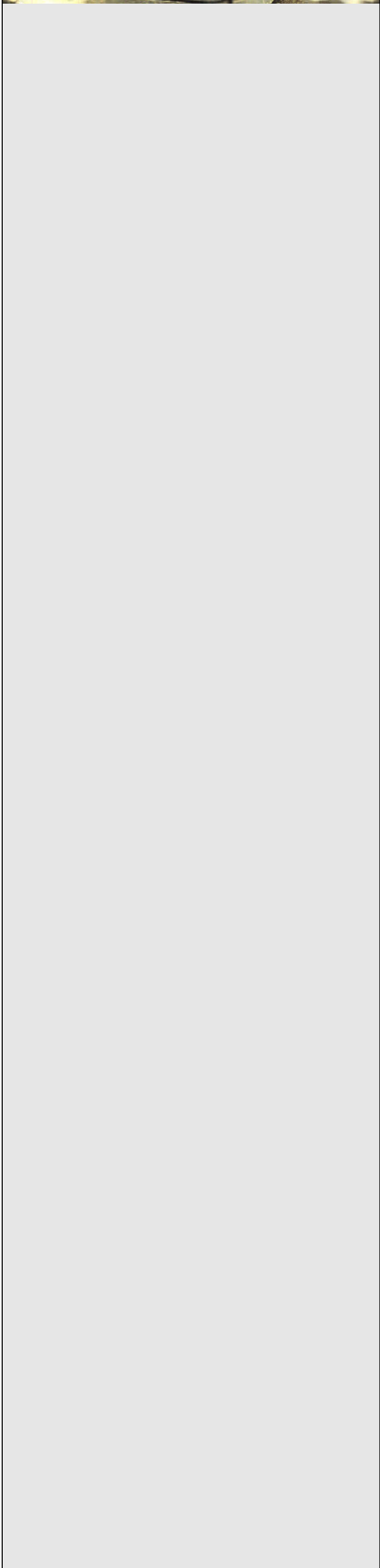
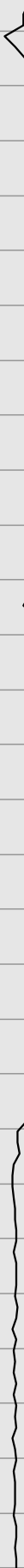

LOGS PROCESSED BY: Robert Garfield P.G. & Nick DeCristofaro

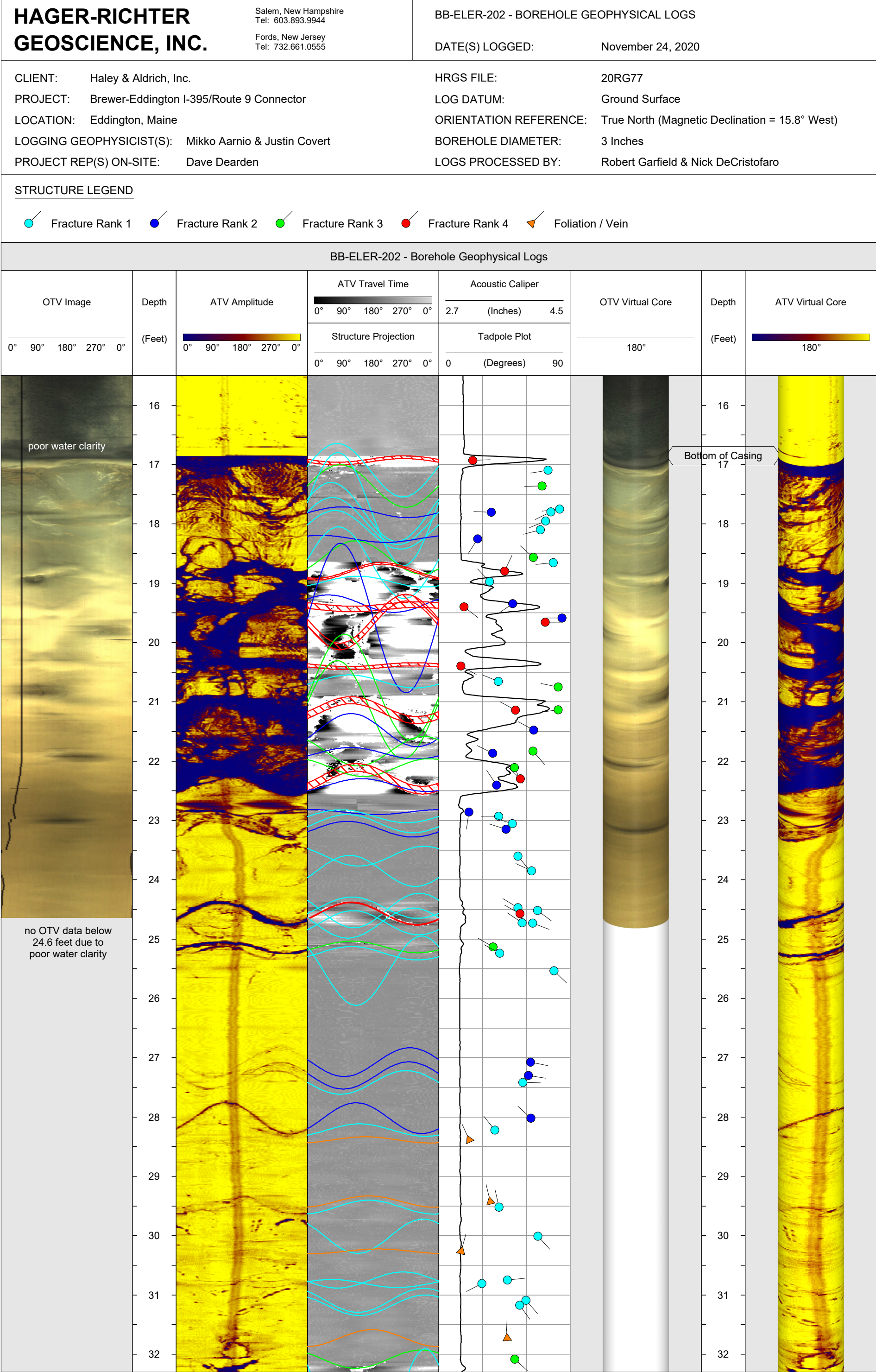
STRUCTURE LEGEND

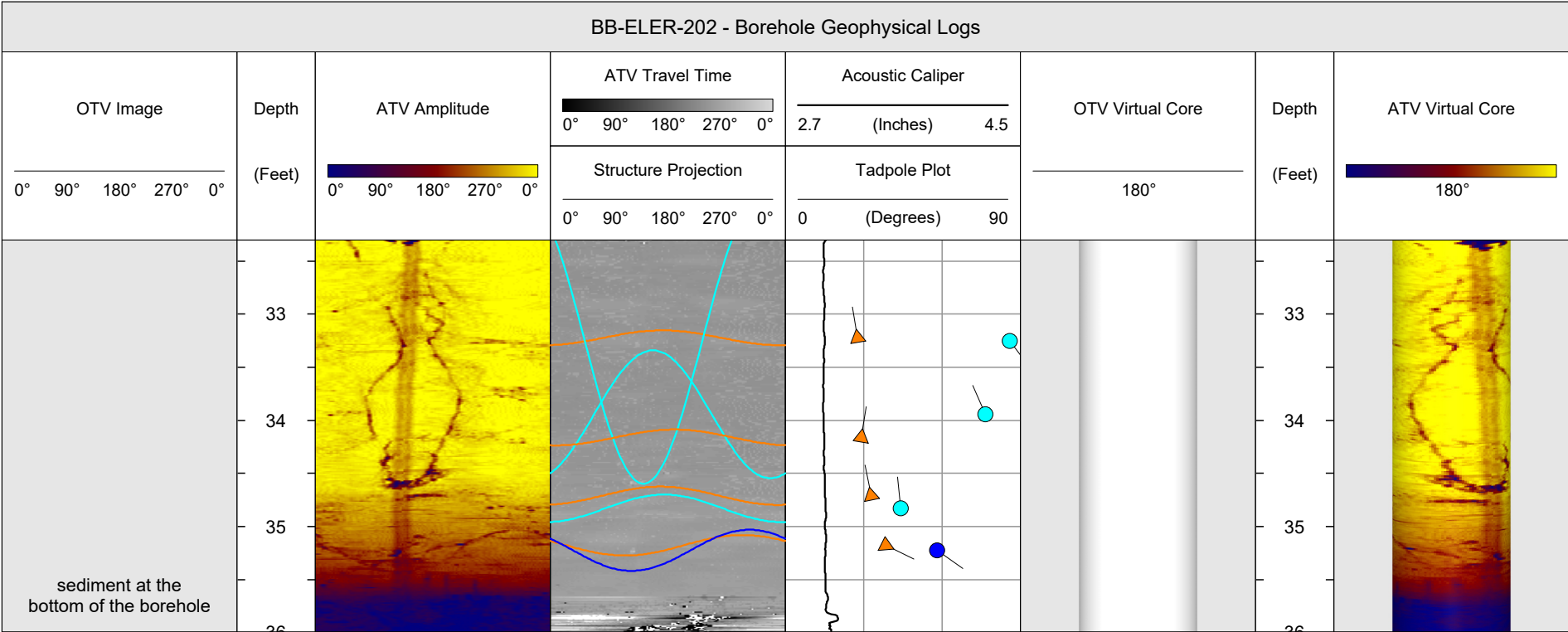
 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

BB-ECR-202 - Borehole Geophysical Logs



<div><div>HAGER-RICHTER GEOSCIENCE, INC.</div><div><div>Salem, New Hampshire Tel: 603.893.9944</div><div>Fords, New Jersey Tel: 732.661.0555</div></div></div>		<div>BB-ECR-205 - BOREHOLE GEOPHYSICAL LOGS</div> <div><div>DATE(S) LOGGED:</div><div>November 24, 2020</div></div>					
<div><div>CLIENT:</div><div>Haley & Aldrich, Inc.</div></div> <div><div>PROJECT:</div><div>Brewer-Eddington I-395/Route 9 Connector</div></div> <div><div>LOCATION:</div><div>Eddington, Maine</div></div> <div><div>LOGGING GEOPHYSICIST(S):</div><div>Mikko Aarnio & Justin Covert</div></div> <div><div>PROJECT REP(S) ON-SITE:</div><div>Dave Dearden</div></div>		<div><div>HRGS FILE:</div><div>20RG77</div></div> <div><div>LOG DATUM:</div><div>Ground Surface</div></div> <div><div>ORIENTATION REFERENCE:</div><div>Data are Unoriented - See Notes Below</div></div> <div><div>BOREHOLE DIAMETER:</div><div>3 Inches</div></div> <div><div>LOGS PROCESSED BY:</div><div>Robert Garfield P.G. & Nick DeCristofaro</div></div>					
<div>NOTE: The OTV data are unoriented due to magnetic interference from the steel casing throughout the unobstructed open bedrock portion of the borehole.</div>							
<div>BB-ECR-205 - Borehole Geophysical Logs</div>							
<div>OTV Image</div>	<div>Depth (Feet)</div>	<div>Comments</div>	<div>Caliper</div>			<div>Depth (Feet)</div>	<div>OTV Virtual Core</div>
			<div>2.5</div>	<div>(Inches)</div>	<div>7</div>		
	15	<div>A partial obstruction at ~17 feet prevented acquisition of OTV & ATV data; only 3-arm caliper data were acquired below ~17 feet.</div> <div>Bottom of Casing</div>				15	
	16					16	
	17					17	
	18					18	
	19					19	
	20					20	
	21					21	
	22					22	
	23					23	
	24					24	
	25					25	
	26					26	
	27					27	
	28					28	
	29					29	
	30					<div>An obstruction at ~30 feet prevented acquisition of 3-arm caliper data to the reported total depth of 35 feet.</div>	





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BB-ELER-205 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED:

November 24, 2020

CLIENT: Haley & Aldrich, Inc.

PROJECT: Brewer-Eddington I-395/Route 9 Connector

LOCATION: Eddington, Maine

LOGGING GEOPHYSICIST(S): Mikko Aarnio & Justin Covert

PROJECT REP(S) ON-SITE: Dave Dearden

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

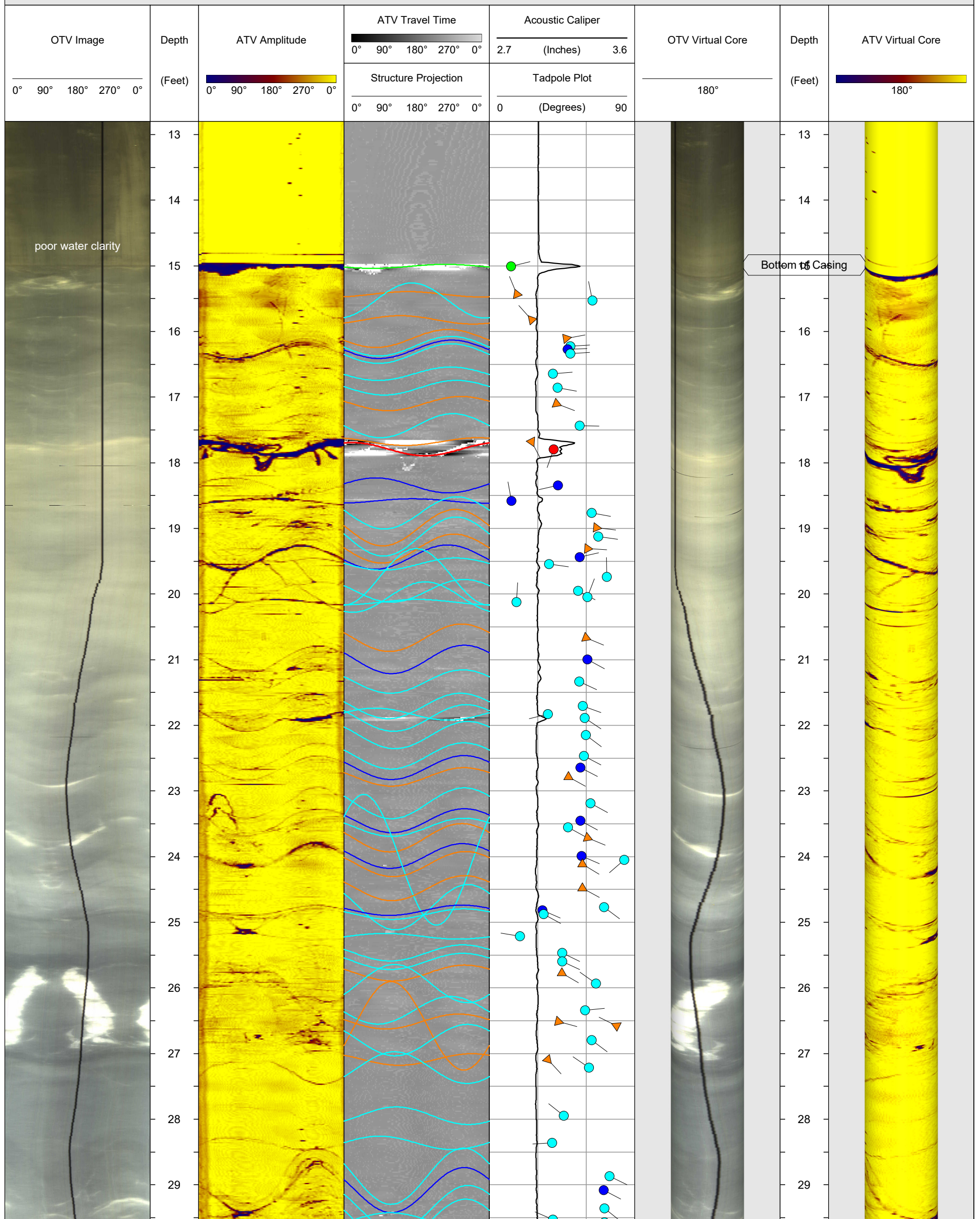
BOREHOLE DIAMETER: 3 Inches

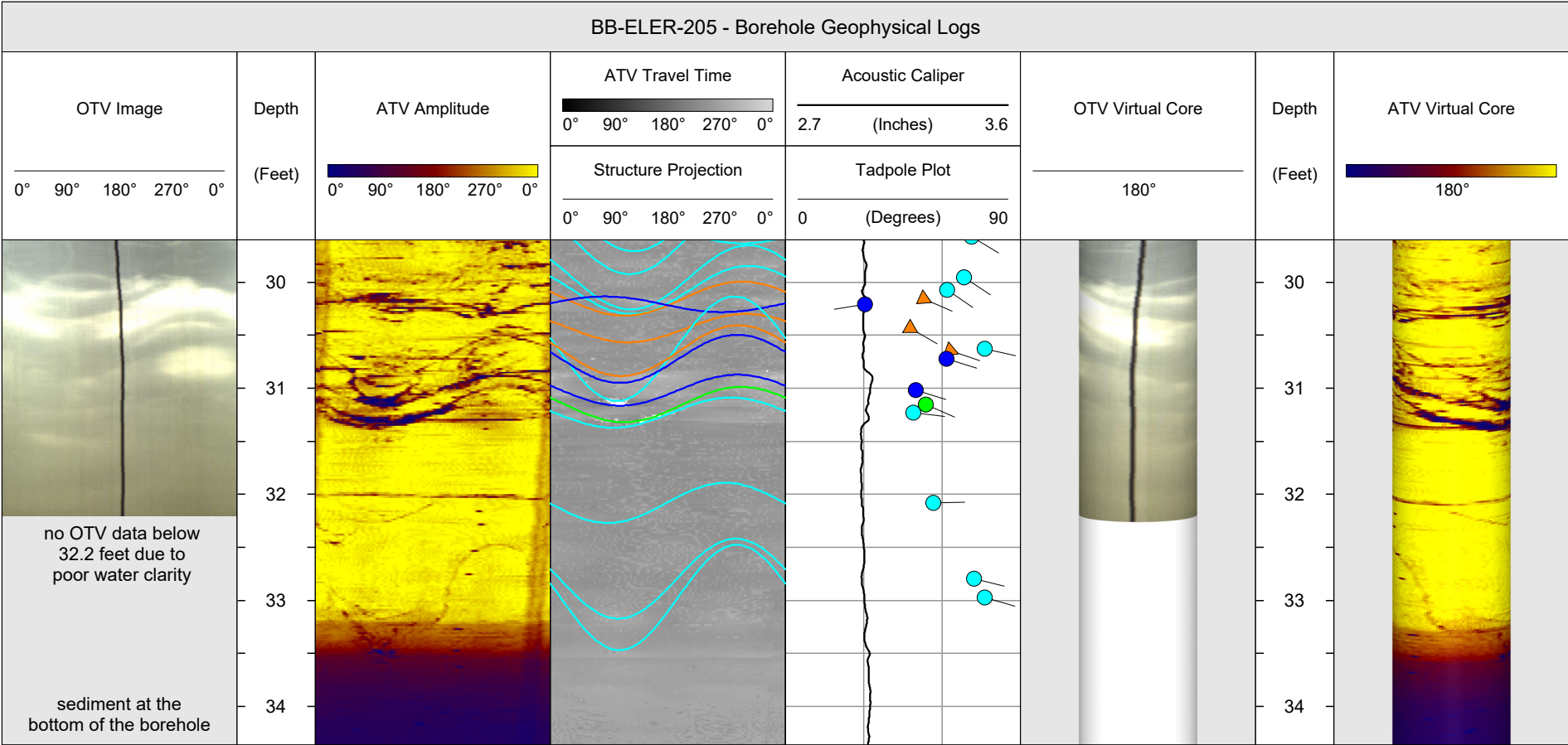
LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

BB-ELER-205 - Borehole Geophysical Logs





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HB-BE-231 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: November 24, 2020

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Justin Covert
PROJECT REP(S) ON-SITE: Dave Dearden

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

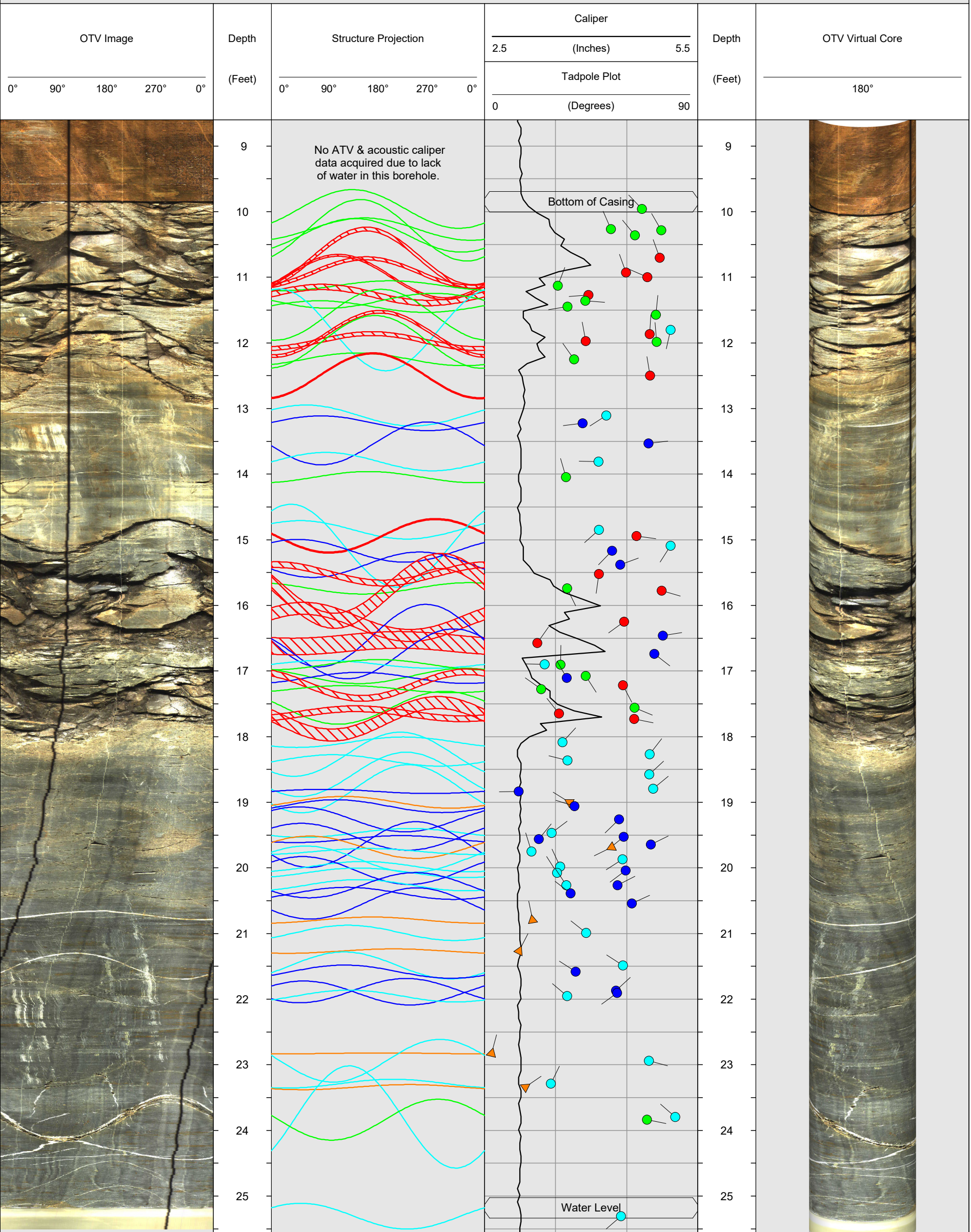
BOREHOLE DIAMETER: 3 Inches

LOGS PROCESSED BY: Robert Garfield P.G. & Nick DeCristofaro

STRUCTURE LEGEND

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

HB-BE-231 - Borehole Geophysical Logs



HB-BE-231 - Borehole Geophysical Logs									
OTV Image	Depth (Feet)	Structure Projection	Caliper			Depth (Feet)	OTV Virtual Core		
			2.5	(Inches)	5.5		180°		
			Tadpole Plot						
			0	(Degrees)	90				
	26	A partial obstruction at ~27.5 feet prevented acquisition of OTV & ATV data to the reported total depth of 35 feet.				26			
	27					27			
	28					28			
	29					29			
	30					30			
	31					31			
	32					32			
	33					33			
	34					34			

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HB-BE-232 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: November 24, 2020

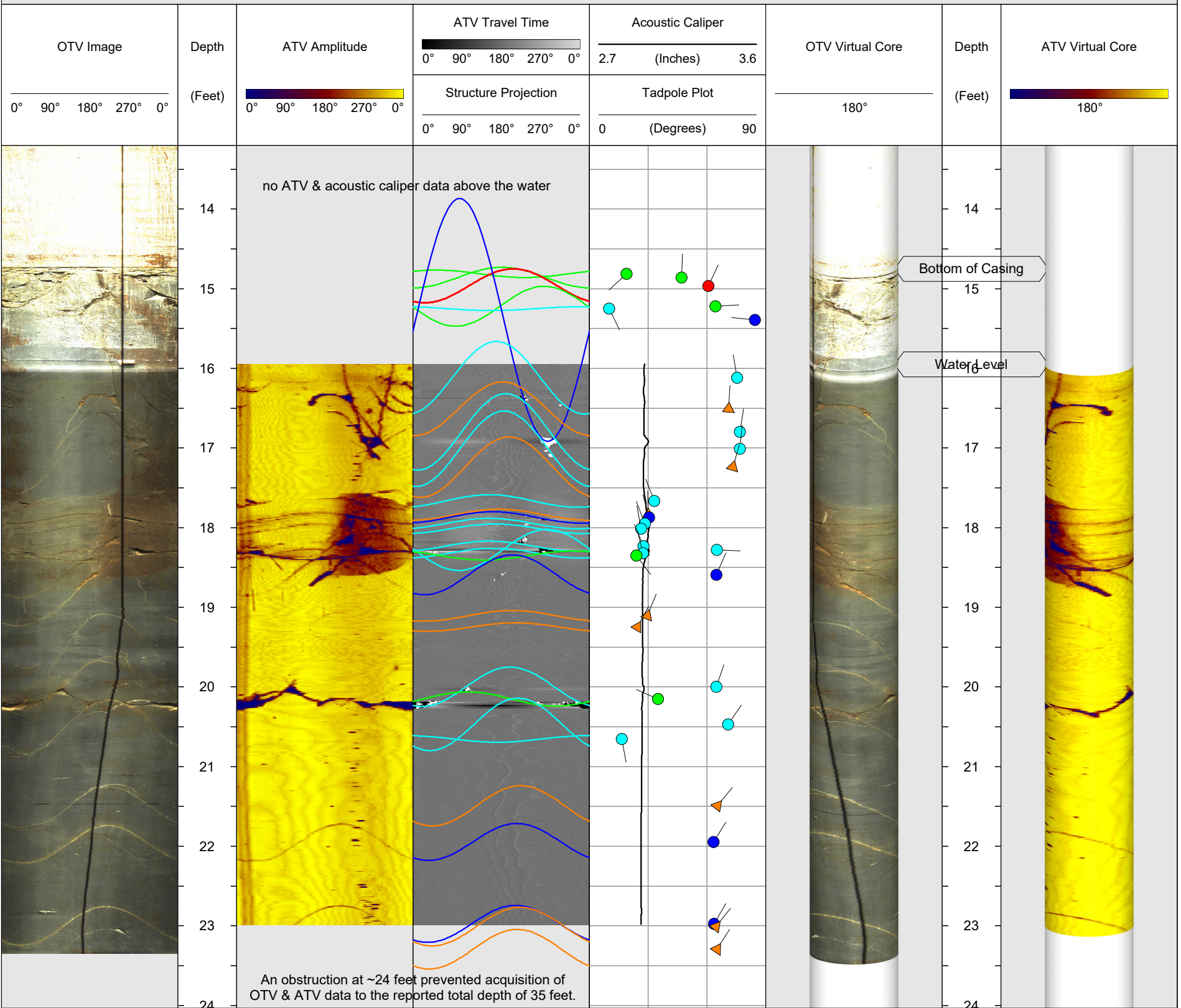
CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Justin Covert
PROJECT REP(S) ON-SITE: Dave Dearden

HRGS FILE: 20RG77
LOG DATUM: Ground Surface
ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)
BOREHOLE DIAMETER: 3 Inches
LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

HB-BE-232 - Borehole Geophysical Logs



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HB-BE-236 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: November 24, 2020

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Justin Covert
PROJECT REP(S) ON-SITE: Dave Dearden

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

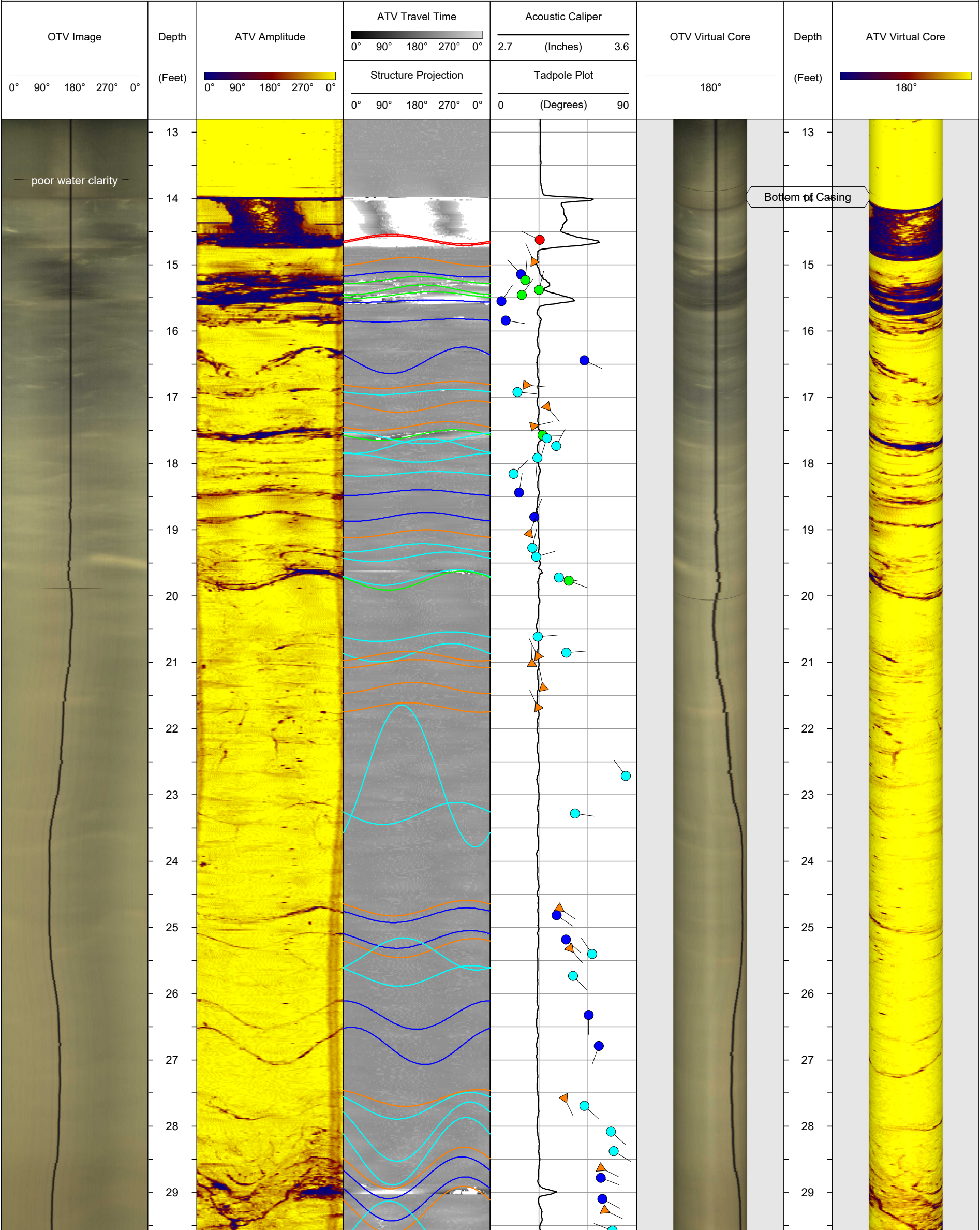
BOREHOLE DIAMETER: 3 Inches

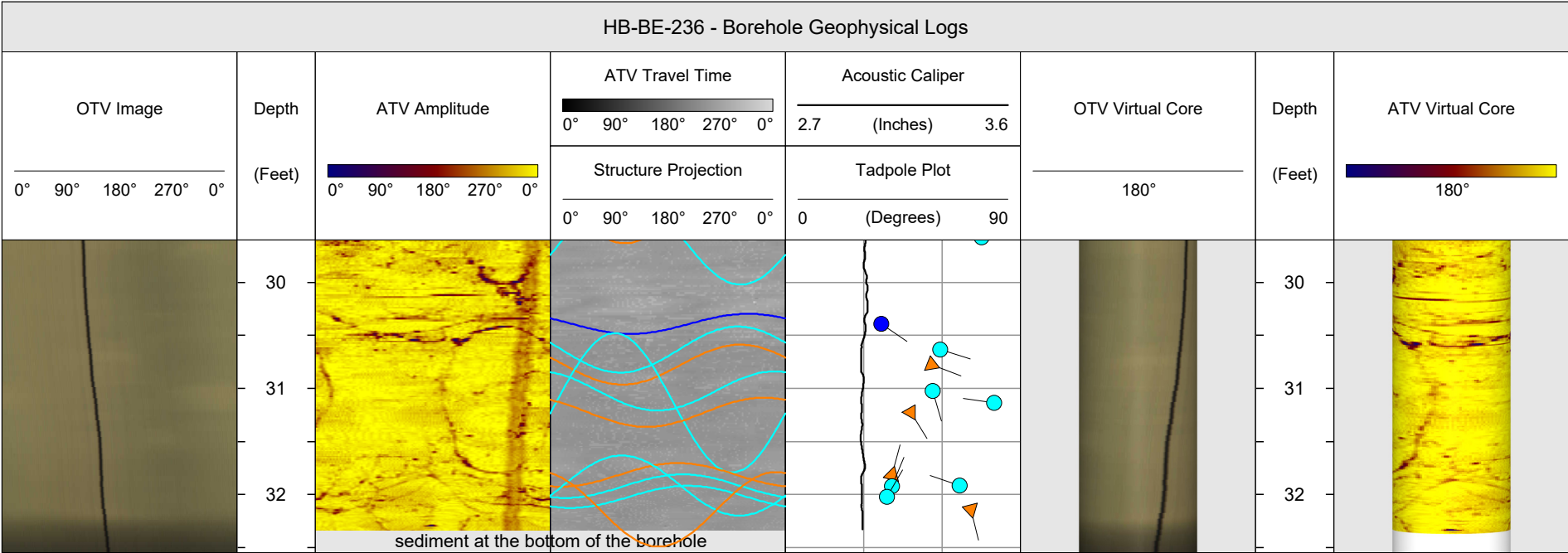
LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

HB-BE-236 - Borehole Geophysical Logs





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ALL BOREHOLES - BEDROCK STRUCTURE STATISTICS PLOTS (BB-ECR-202, BB-ELER-202, BB-ELER-205, HB-BE-231, HB-BE-232, HB-BE-236)

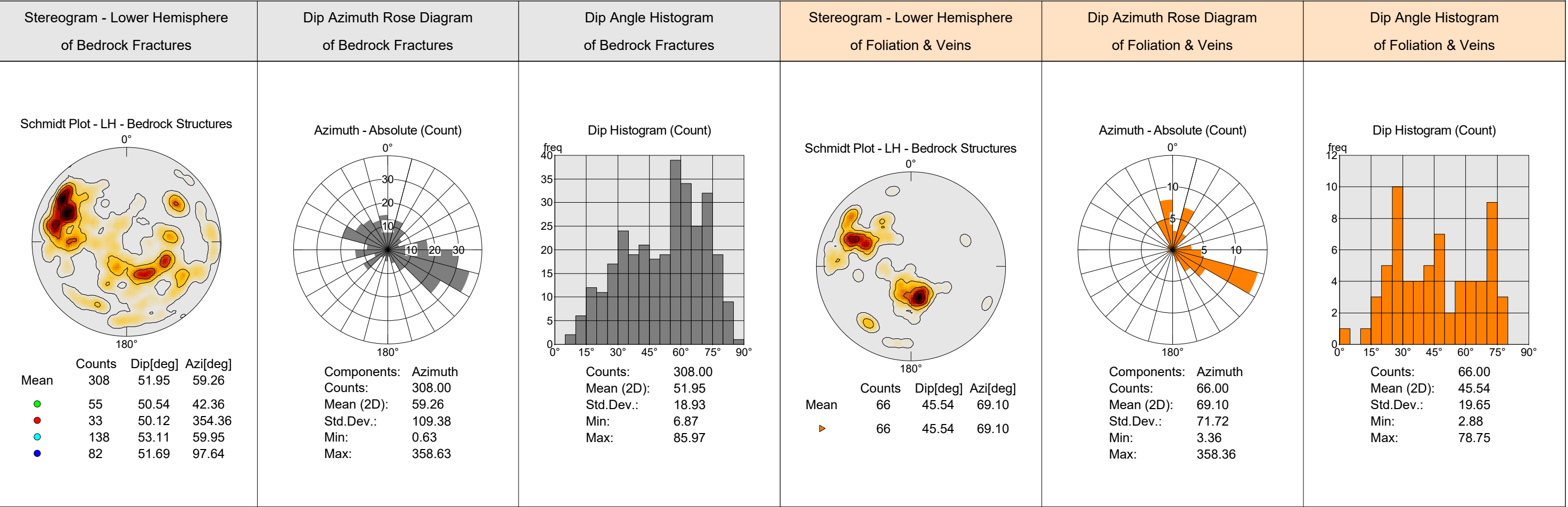
DATE(S) LOGGED: November 24, 2020

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein



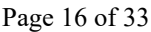
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Tel: 732.661.0555

DATE(S) LOGGED: November 24, 2020

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein



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BB-ELER-202 - BEDROCK STRUCTURE STATISTICS PLOTS

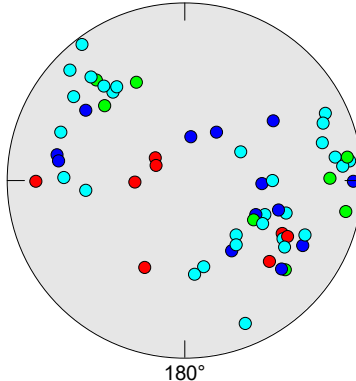
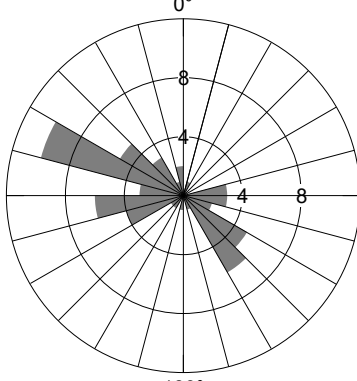
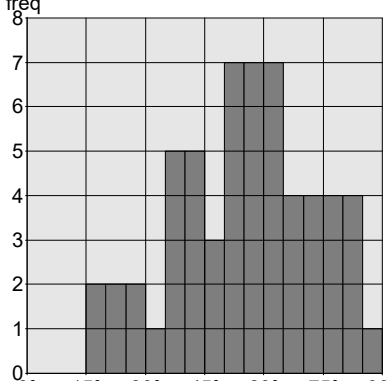
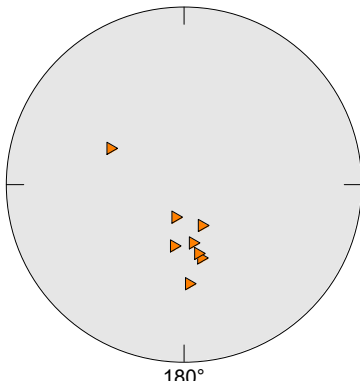
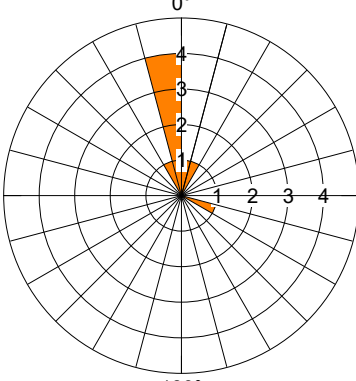
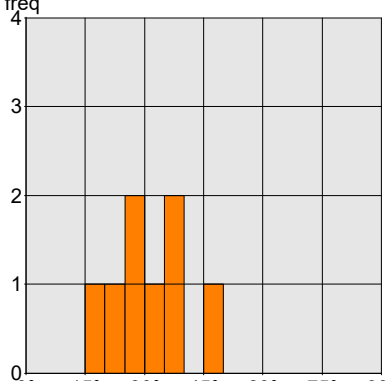
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CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>58</td><td>55.29</td><td>267.86</td></tr><tr><td>●</td><td>8</td><td>42.26</td><td>46.90</td></tr><tr><td>●</td><td>29</td><td>58.92</td><td>267.54</td></tr><tr><td>●</td><td>8</td><td>63.15</td><td>247.48</td></tr><tr><td>●</td><td>13</td><td>50.07</td><td>263.43</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	58	55.29	267.86	●	8	42.26	46.90	●	29	58.92	267.54	●	8	63.15	247.48	●	13	50.07	263.43	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>58.00</td></tr><tr><td>Mean (2D):</td><td>267.86</td></tr><tr><td>Std.Dev.:</td><td>98.21</td></tr><tr><td>Min:</td><td>24.91</td></tr><tr><td>Max:</td><td>353.79</td></tr></table>	Components:	Azimuth	Counts:	58.00	Mean (2D):	267.86	Std.Dev.:	98.21	Min:	24.91	Max:	353.79	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>58.00</th></tr><tr><th>Mean (2D):</th><th>55.29</th></tr><tr><th>Std.Dev.:</th><th>17.84</th></tr><tr><th>Min:</th><th>15.06</th></tr><tr><th>Max:</th><th>85.97</th></tr></table>	Counts:	58.00	Mean (2D):	55.29	Std.Dev.:	17.84	Min:	15.06	Max:	85.97	<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>8</td><td>30.76</td><td>2.29</td></tr><tr><td>▶</td><td>8</td><td>30.76</td><td>2.29</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	8	30.76	2.29	▶	8	30.76	2.29	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>8.00</td></tr><tr><td>Mean (2D):</td><td>2.29</td></tr><tr><td>Std.Dev.:</td><td>38.80</td></tr><tr><td>Min:</td><td>9.59</td></tr><tr><td>Max:</td><td>356.76</td></tr></table>	Components:	Azimuth	Counts:	8.00	Mean (2D):	2.29	Std.Dev.:	38.80	Min:	9.59	Max:	356.76	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>8.00</th></tr><tr><th>Mean (2D):</th><th>30.76</th></tr><tr><th>Std.Dev.:</th><th>9.27</th></tr><tr><th>Min:</th><th>15.46</th></tr><tr><th>Max:</th><th>46.86</th></tr></table>	Counts:	8.00	Mean (2D):	30.76	Std.Dev.:	9.27	Min:	15.46	Max:	46.86
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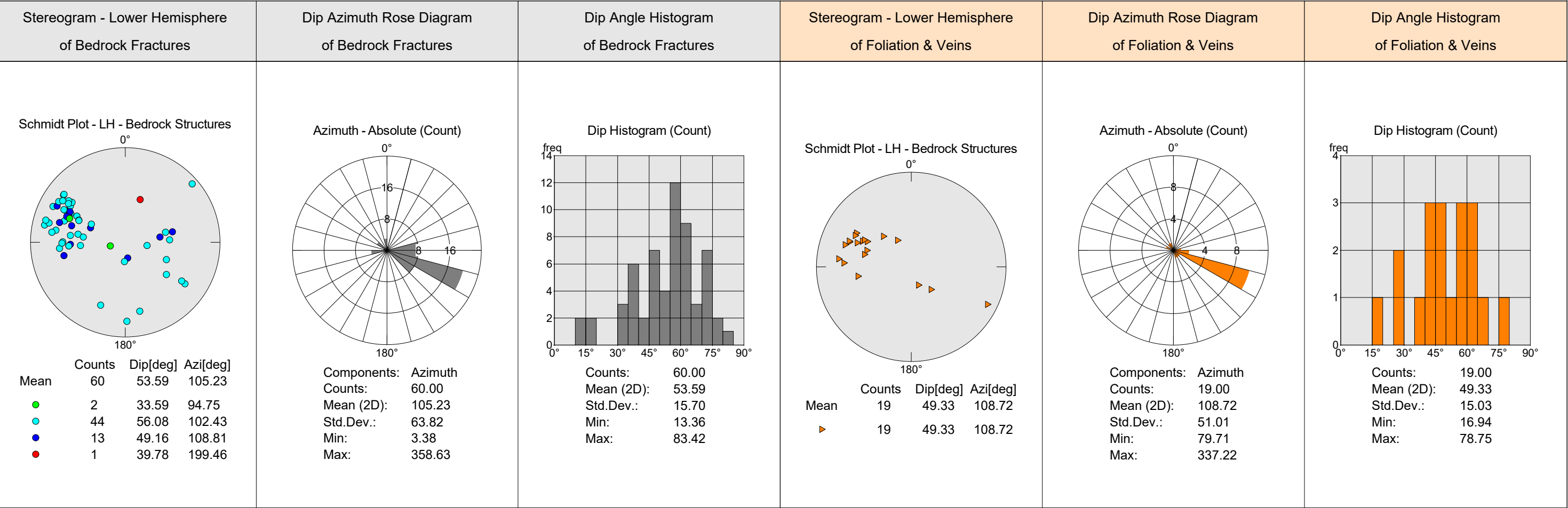
Salem, New Hampshire
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Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: November 24, 2020

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein



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HB-BE-231 - BEDROCK STRUCTURE STATISTICS PLOTS

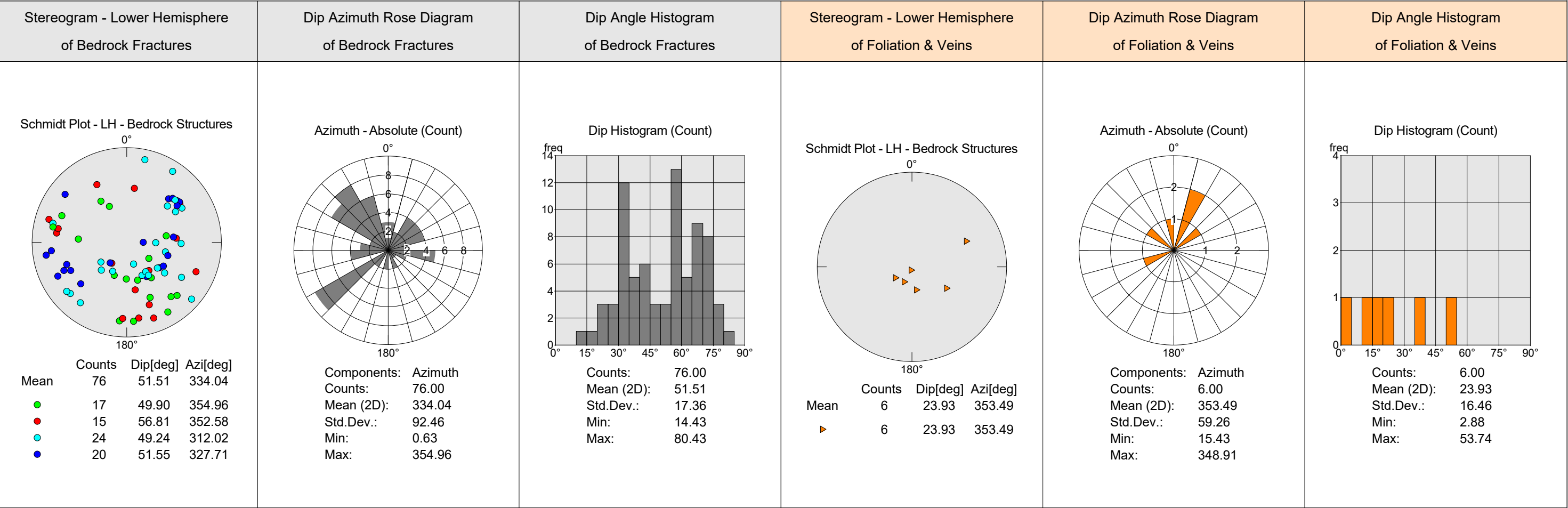
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CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
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STRUCTURE LEGEND

● Fracture Rank 1
 ● Fracture Rank 2
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 ▲ Foliation / Vein



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HB-BE-232 - BEDROCK STRUCTURE STATISTICS PLOTS

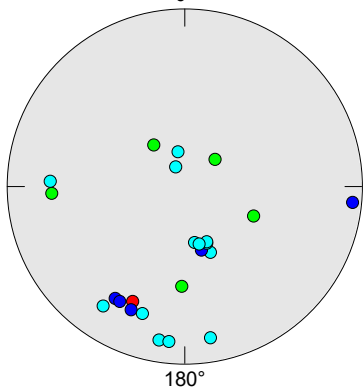
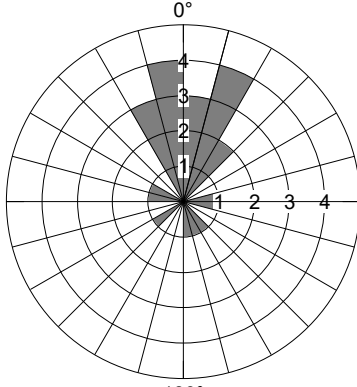
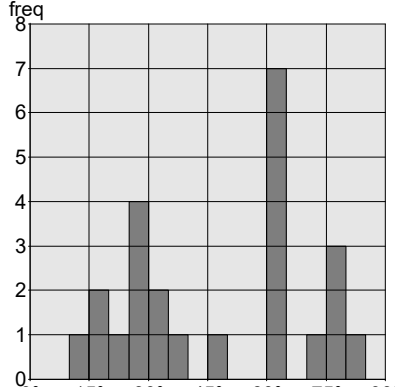
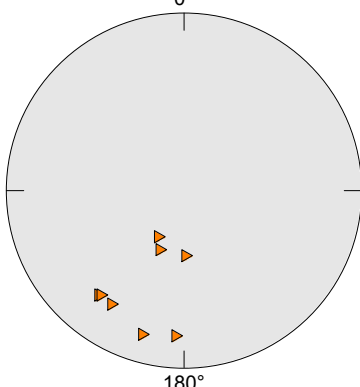
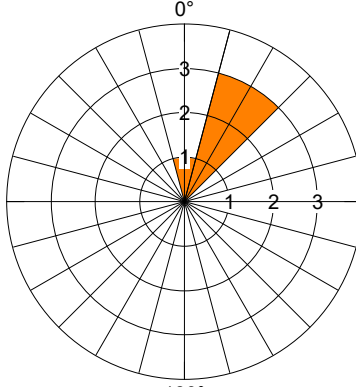
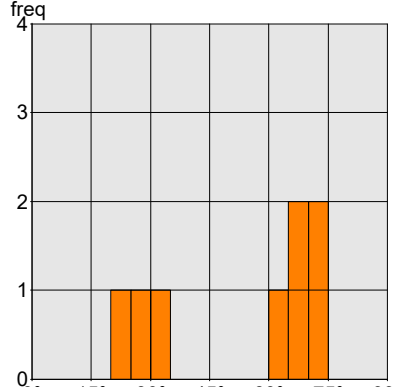
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HRGS FILE: 20RG77
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MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>24</td><td>48.19</td><td>7.76</td></tr><tr><td>●</td><td>5</td><td>37.79</td><td>222.43</td></tr><tr><td>●</td><td>1</td><td>60.78</td><td>24.58</td></tr><tr><td>●</td><td>13</td><td>46.00</td><td>8.17</td></tr><tr><td>●</td><td>5</td><td>61.53</td><td>2.70</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	24	48.19	7.76	●	5	37.79	222.43	●	1	60.78	24.58	●	13	46.00	8.17	●	5	61.53	2.70	<div>Azimuth - Absolute (Count)</div>  <table><tr><td>Components:</td><td>Azimuth</td></tr><tr><td>Counts:</td><td>24.00</td></tr><tr><td>Mean (2D):</td><td>7.76</td></tr><tr><td>Std.Dev.:</td><td>66.45</td></tr><tr><td>Min:</td><td>2.10</td></tr><tr><td>Max:</td><td>350.34</td></tr></table>	Components:	Azimuth	Counts:	24.00	Mean (2D):	7.76	Std.Dev.:	66.45	Min:	2.10	Max:	350.34	<div>Dip Histogram (Count)</div>  <table><tr><td>Counts:</td><td>24.00</td></tr><tr><td>Mean (2D):</td><td>48.19</td></tr><tr><td>Std.Dev.:</td><td>22.83</td></tr><tr><td>Min:</td><td>10.15</td></tr><tr><td>Max:</td><td>84.47</td></tr></table>	Counts:	24.00	Mean (2D):	48.19	Std.Dev.:	22.83	Min:	10.15	Max:	84.47	<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>8</td><td>53.11</td><td>22.64</td></tr><tr><td>▶</td><td>8</td><td>53.11</td><td>22.64</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	8	53.11	22.64	▶	8	53.11	22.64	<div>Azimuth - Absolute (Count)</div>  <table><tr><td>Components:</td><td>Azimuth</td></tr><tr><td>Counts:</td><td>8.00</td></tr><tr><td>Mean (2D):</td><td>22.64</td></tr><tr><td>Std.Dev.:</td><td>14.55</td></tr><tr><td>Min:</td><td>3.36</td></tr><tr><td>Max:</td><td>358.36</td></tr></table>	Components:	Azimuth	Counts:	8.00	Mean (2D):	22.64	Std.Dev.:	14.55	Min:	3.36	Max:	358.36	<div>Dip Histogram (Count)</div>  <table><tr><td>Counts:</td><td>8.00</td></tr><tr><td>Mean (2D):</td><td>53.11</td></tr><tr><td>Std.Dev.:</td><td>19.70</td></tr><tr><td>Min:</td><td>24.49</td></tr><tr><td>Max:</td><td>73.22</td></tr></table>	Counts:	8.00	Mean (2D):	53.11	Std.Dev.:	19.70	Min:	24.49	Max:	73.22
	Counts	Dip[deg]	Azi[deg]																																																																																		
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HAGER-RICHTER
GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

HB-BE-236 - BEDROCK STRUCTURE STATISTICS PLOTS

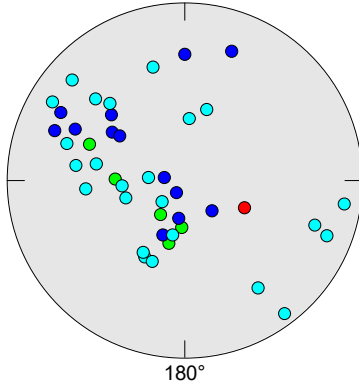
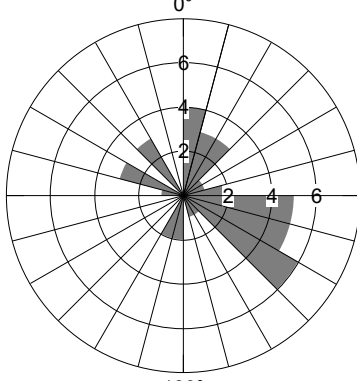
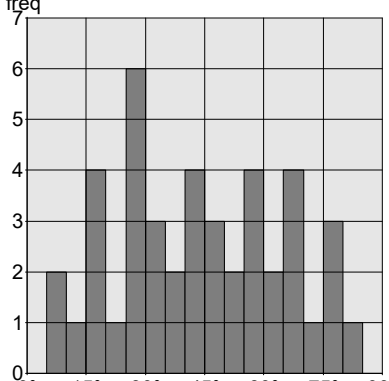
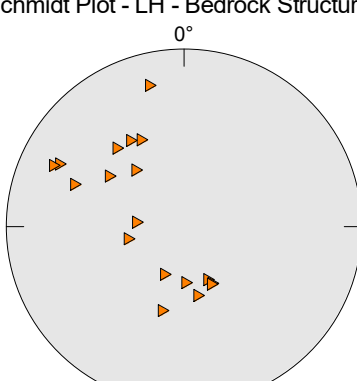
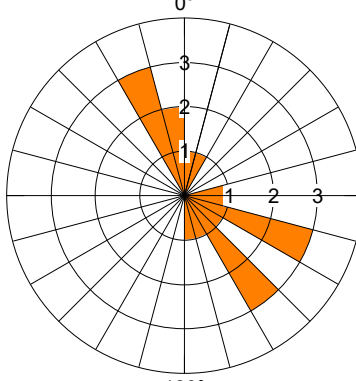
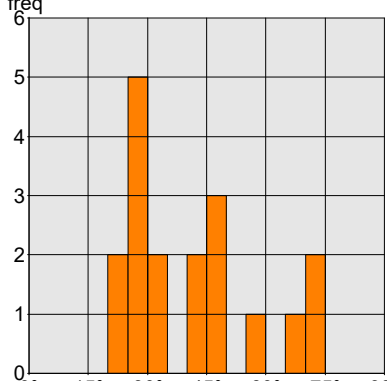
DATE(S) LOGGED: November 24, 2020

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
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	Counts	Dip[deg]	Azi[deg]																																																																																		
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HAGER-RICHTER GEOSCIENCE, INC.	
BB-ECR-202 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ECR-202 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
15.1	127	71	Foliation / Vein
15.2	157	21	Fracture Rank 2
15.2	132	72	Fracture Rank 2
15.3	134	71	Foliation / Vein
15.4	137	71	Fracture Rank 3
15.6	130	74	Fracture Rank 2
15.7	129	73	Foliation / Vein
15.7	169	16	Fracture Rank 3
15.7	222	81	Fracture Rank 2
15.8	127	73	Fracture Rank 2
15.9	127	73	Fracture Rank 2
16.2	127	73	Fracture Rank 2
16.3	129	73	Foliation / Vein
16.5	119	70	Fracture Rank 3
16.7	313	30	Fracture Rank 3
16.7	117	69	Fracture Rank 3
16.9	57	23	Fracture Rank 4
16.9	59	14	Fracture Rank 3
17.1	22	22	Fracture Rank 3
17.2	107	64	Fracture Rank 3
17.3	115	69	Fracture Rank 3
17.3	19	41	Fracture Rank 4
17.6	99	72	Fracture Rank 2
17.8	119	75	Fracture Rank 3
18.0	242	60	Fracture Rank 4
18.0	114	58	Fracture Rank 2
18.5	253	31	Fracture Rank 4
18.6	81	73	Fracture Rank 3
19.0	92	24	Fracture Rank 2
19.0	295	41	Fracture Rank 2
19.1	323	30	Fracture Rank 1
19.2	103	70	Fracture Rank 3
19.4	2	59	Fracture Rank 1
19.4	91	65	Fracture Rank 1
19.6	294	79	Fracture Rank 3

BB-ECR-202 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
19.7	41	68	Fracture Rank 3
19.8	351	32	Fracture Rank 3
20.2	333	50	Fracture Rank 4
20.4	294	75	Fracture Rank 4
20.6	95	51	Fracture Rank 4
20.8	170	77	Fracture Rank 3
20.8	95	40	Fracture Rank 2
21.1	336	41	Fracture Rank 2
21.3	123	29	Fracture Rank 1
21.4	112	68	Fracture Rank 2
21.5	102	78	Foliation / Vein
21.8	99	74	Fracture Rank 2
21.8	333	64	Fracture Rank 2
21.9	94	72	Foliation / Vein
21.9	349	54	Fracture Rank 3
22.2	88	76	Foliation / Vein
22.3	99	79	Fracture Rank 2
22.5	350	58	Fracture Rank 3
22.7	6	48	Fracture Rank 2

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ELER-202 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ELER-202 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
16.9	88	23	Fracture Rank 4
17.1	261	75	Fracture Rank 1
17.4	269	71	Fracture Rank 3
17.8	263	83	Fracture Rank 1
17.8	244	77	Fracture Rank 1
17.8	272	36	Fracture Rank 2
18.0	247	73	Fracture Rank 1
18.1	255	69	Fracture Rank 1
18.3	213	27	Fracture Rank 2
18.6	312	65	Fracture Rank 3
18.7	265	79	Fracture Rank 1
18.8	25	45	Fracture Rank 4
19.0	317	35	Fracture Rank 1
19.3	236	51	Fracture Rank 2
19.4	128	17	Fracture Rank 4
19.6	270	84	Fracture Rank 2
19.7	90	73	Fracture Rank 4
20.4	118	15	Fracture Rank 4
20.7	293	41	Fracture Rank 1
20.8	281	82	Fracture Rank 3
21.1	262	82	Fracture Rank 3
21.1	299	52	Fracture Rank 4
21.5	299	65	Fracture Rank 2
21.8	139	65	Fracture Rank 3
21.9	296	37	Fracture Rank 2
22.1	154	52	Fracture Rank 3
22.3	314	56	Fracture Rank 4
22.4	326	39	Fracture Rank 2
22.9	188	21	Fracture Rank 2
22.9	270	41	Fracture Rank 1
23.1	288	50	Fracture Rank 1
23.2	288	46	Fracture Rank 2
23.6	141	54	Fracture Rank 1
23.9	295	64	Fracture Rank 1
24.5	301	54	Fracture Rank 1

BB-ELER-202 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
24.5	127	68	Fracture Rank 1
24.6	299	56	Fracture Rank 4
24.7	304	57	Fracture Rank 1
24.7	111	64	Fracture Rank 1
25.1	300	37	Fracture Rank 3
25.2	299	42	Fracture Rank 1
25.5	134	79	Fracture Rank 1
27.1	101	63	Fracture Rank 2
27.3	99	61	Fracture Rank 2
27.4	92	58	Fracture Rank 1
28.0	312	63	Fracture Rank 2
28.2	322	39	Fracture Rank 1
28.4	336	21	Foliation / Vein
29.4	347	35	Foliation / Vein
29.5	347	41	Fracture Rank 1
30.0	138	68	Fracture Rank 1
30.3	15	15	Foliation / Vein
30.8	84	47	Fracture Rank 1
30.8	243	29	Fracture Rank 1
31.1	139	60	Fracture Rank 1
31.2	144	55	Fracture Rank 1
31.7	357	47	Foliation / Vein
32.1	133	52	Fracture Rank 3
33.2	351	28	Foliation / Vein
33.3	143	86	Fracture Rank 1
33.9	337	77	Fracture Rank 1
34.2	10	29	Foliation / Vein
34.7	348	33	Foliation / Vein
34.8	354	44	Fracture Rank 1
35.2	116	38	Foliation / Vein
35.2	126	58	Fracture Rank 2

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ELER-205 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ELER-205 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
15.0	76	13	Fracture Rank 3
15.4	337	17	Foliation / Vein
15.5	348	64	Fracture Rank 1
15.8	319	26	Foliation / Vein
16.1	80	48	Foliation / Vein
16.2	86	50	Fracture Rank 1
16.3	88	48	Fracture Rank 2
16.3	86	50	Fracture Rank 1
16.7	86	39	Fracture Rank 1
16.9	100	42	Fracture Rank 1
17.1	111	41	Foliation / Vein
17.4	91	56	Fracture Rank 1
17.7	153	26	Foliation / Vein
17.8	199	40	Fracture Rank 4
18.3	257	42	Fracture Rank 2
18.6	350	14	Fracture Rank 2
18.8	100	63	Fracture Rank 1
19.0	97	66	Foliation / Vein
19.1	98	68	Fracture Rank 1
19.3	94	61	Foliation / Vein
19.4	78	56	Fracture Rank 2
19.5	98	37	Fracture Rank 1
19.7	359	73	Fracture Rank 1
20.0	118	55	Fracture Rank 1
20.1	21	61	Fracture Rank 1
20.1	3	17	Fracture Rank 1
20.7	112	60	Foliation / Vein
21.0	118	61	Fracture Rank 2
21.3	115	56	Fracture Rank 1
21.7	110	58	Fracture Rank 1
21.8	256	36	Fracture Rank 1
21.9	123	59	Fracture Rank 1
22.2	127	60	Fracture Rank 1
22.5	118	59	Fracture Rank 1
22.6	118	56	Fracture Rank 2

BB-ELER-205 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
22.8	118	49	Foliation / Vein
23.2	118	63	Fracture Rank 1
23.5	119	56	Fracture Rank 2
23.6	118	49	Fracture Rank 1
23.7	113	61	Foliation / Vein
24.0	114	57	Fracture Rank 2
24.1	229	83	Fracture Rank 1
24.1	122	58	Foliation / Vein
24.5	119	58	Foliation / Vein
24.8	127	71	Fracture Rank 1
24.8	113	33	Fracture Rank 2
24.9	118	33	Fracture Rank 1
25.2	279	19	Fracture Rank 1
25.5	116	45	Fracture Rank 1
25.6	115	45	Fracture Rank 1
25.8	120	45	Foliation / Vein
25.9	305	66	Fracture Rank 1
26.3	84	59	Fracture Rank 1
26.5	105	43	Foliation / Vein
26.6	296	79	Foliation / Vein
26.8	127	63	Fracture Rank 1
27.1	138	36	Foliation / Vein
27.2	304	62	Fracture Rank 1
28.0	308	46	Fracture Rank 1
28.4	267	39	Fracture Rank 1
28.9	117	74	Fracture Rank 1
29.1	118	71	Fracture Rank 2
29.4	128	71	Fracture Rank 1
29.5	293	39	Fracture Rank 1
29.6	121	71	Fracture Rank 1
30.0	124	68	Fracture Rank 1
30.1	124	62	Fracture Rank 1
30.2	114	53	Foliation / Vein
30.2	261	31	Fracture Rank 2
30.4	120	48	Foliation / Vein
30.6	102	76	Fracture Rank 1
30.7	109	63	Foliation / Vein
30.7	107	62	Fracture Rank 2
31.0	107	50	Fracture Rank 2
31.2	113	54	Fracture Rank 3
31.2	97	49	Fracture Rank 1
32.1	89	57	Fracture Rank 1
32.8	104	72	Fracture Rank 1
33.0	106	76	Fracture Rank 1

HAGER-RICHTER GEOSCIENCE, INC.	
HB-BE-231 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

HB-BE-231 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
10.0	317	66	Fracture Rank 3
10.3	337	53	Fracture Rank 3
10.3	330	75	Fracture Rank 3
10.4	321	63	Fracture Rank 3
10.7	340	74	Fracture Rank 4
10.9	340	60	Fracture Rank 4
11.0	293	69	Fracture Rank 4
11.1	21	31	Fracture Rank 3
11.3	265	44	Fracture Rank 4
11.4	94	43	Fracture Rank 3
11.5	261	35	Fracture Rank 3
11.6	5	72	Fracture Rank 3
11.8	193	78	Fracture Rank 1
11.9	3	70	Fracture Rank 4
12.0	350	43	Fracture Rank 4
12.0	355	73	Fracture Rank 3
12.3	325	38	Fracture Rank 3
12.5	351	70	Fracture Rank 4
13.1	238	51	Fracture Rank 1
13.2	263	41	Fracture Rank 2
13.5	84	69	Fracture Rank 2
13.8	272	48	Fracture Rank 1
14.0	344	34	Fracture Rank 3
14.9	228	48	Fracture Rank 1
14.9	98	64	Fracture Rank 4
15.1	213	79	Fracture Rank 1
15.2	224	54	Fracture Rank 2
15.4	70	57	Fracture Rank 2
15.5	188	48	Fracture Rank 4
15.7	154	35	Fracture Rank 3
15.8	107	75	Fracture Rank 4
16.3	234	59	Fracture Rank 4
16.5	81	75	Fracture Rank 2
16.6	35	22	Fracture Rank 4
16.7	128	72	Fracture Rank 2

HB-BE-231 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
16.9	271	25	Fracture Rank 1
16.9	1	32	Fracture Rank 3
17.1	148	43	Fracture Rank 3
17.1	330	35	Fracture Rank 2
17.2	152	58	Fracture Rank 4
17.3	306	24	Fracture Rank 3
17.6	112	63	Fracture Rank 3
17.7	322	31	Fracture Rank 4
17.7	101	63	Fracture Rank 4
18.1	43	33	Fracture Rank 1
18.3	38	70	Fracture Rank 1
18.4	284	35	Fracture Rank 1
18.6	48	70	Fracture Rank 1
18.8	51	71	Fracture Rank 1
18.8	269	14	Fracture Rank 2
19.0	302	36	Foliation / Vein
19.1	288	38	Fracture Rank 2
19.3	226	57	Fracture Rank 2
19.5	53	28	Fracture Rank 1
19.5	230	59	Fracture Rank 2
19.6	39	23	Fracture Rank 2
19.6	64	70	Fracture Rank 2
19.7	244	54	Foliation / Vein
19.8	342	20	Fracture Rank 1
19.9	238	58	Fracture Rank 1
20.0	335	32	Fracture Rank 1
20.0	232	60	Fracture Rank 2
20.1	327	31	Fracture Rank 1
20.3	326	34	Fracture Rank 1
20.3	63	56	Fracture Rank 2
20.4	308	36	Fracture Rank 2
20.5	66	62	Fracture Rank 2
20.8	349	20	Foliation / Vein
21.0	309	43	Fracture Rank 1
21.3	27	15	Foliation / Vein
21.5	302	58	Fracture Rank 1
21.6	303	38	Fracture Rank 2
21.9	48	55	Fracture Rank 2
21.9	234	56	Fracture Rank 2
22.0	310	35	Fracture Rank 1
22.8	15	3	Foliation / Vein
22.9	105	69	Fracture Rank 1
23.3	26	28	Fracture Rank 1
23.3	56	17	Foliation / Vein
23.8	311	80	Fracture Rank 1

HB-BE-231 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
23.8	102	69	Fracture Rank 3
25.3	229	58	Fracture Rank 1

HAGER-RICHTER GEOSCIENCE, INC.	
HB-BE-232 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

HB-BE-232 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
14.8	228	19	Fracture Rank 3
14.9	2	47	Fracture Rank 3
15.0	25	61	Fracture Rank 4
15.2	87	64	Fracture Rank 3
15.3	154	10	Fracture Rank 1
15.4	275	84	Fracture Rank 2
16.1	350	75	Fracture Rank 1
16.5	3	71	Foliation / Vein
16.8	10	77	Fracture Rank 1
17.0	6	77	Fracture Rank 1
17.2	16	73	Foliation / Vein
17.7	338	33	Fracture Rank 1
17.8	358	30	Foliation / Vein
17.9	345	30	Fracture Rank 2
18.0	339	28	Fracture Rank 1
18.0	350	26	Fracture Rank 1
18.2	338	28	Fracture Rank 1
18.3	93	65	Fracture Rank 1
18.3	346	27	Fracture Rank 1
18.4	143	24	Fracture Rank 3
18.6	24	65	Fracture Rank 2
19.1	23	30	Foliation / Vein
19.3	29	24	Foliation / Vein
20.0	18	65	Fracture Rank 1
20.2	293	35	Fracture Rank 3
20.5	34	71	Fracture Rank 1
20.7	169	17	Fracture Rank 1
21.5	39	65	Foliation / Vein
22.0	32	63	Fracture Rank 2
23.0	30	64	Fracture Rank 2
23.0	39	65	Foliation / Vein
23.3	33	65	Foliation / Vein

HAGER-RICHTER GEOSCIENCE, INC.	
HB-BE-236 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	November 24, 2020
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

HB-BE-236 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
14.6	295	30	Fracture Rank 4
15.0	336	27	Foliation / Vein
15.1	318	19	Fracture Rank 2
15.2	5	22	Fracture Rank 3
15.4	14	30	Fracture Rank 3
15.5	36	20	Fracture Rank 3
15.6	34	7	Fracture Rank 2
15.8	99	10	Fracture Rank 2
16.5	115	58	Fracture Rank 2
16.8	96	22	Foliation / Vein
16.9	96	17	Fracture Rank 1
17.1	140	35	Foliation / Vein
17.4	78	27	Foliation / Vein
17.6	92	32	Fracture Rank 3
17.6	197	35	Fracture Rank 1
17.7	28	40	Fracture Rank 1
17.9	185	29	Fracture Rank 1
18.2	47	14	Fracture Rank 1
18.4	9	18	Fracture Rank 2
18.8	23	27	Fracture Rank 2
19.1	23	24	Foliation / Vein
19.3	13	26	Fracture Rank 1
19.4	74	28	Fracture Rank 1
19.7	101	42	Fracture Rank 1
19.8	111	48	Fracture Rank 3
20.6	85	29	Fracture Rank 1
20.9	85	47	Fracture Rank 1
20.9	334	29	Foliation / Vein
21.0	358	26	Foliation / Vein
21.4	349	33	Foliation / Vein
21.7	335	29	Foliation / Vein
22.7	323	83	Fracture Rank 1
23.3	98	52	Fracture Rank 1
24.7	124	42	Foliation / Vein
24.8	123	41	Fracture Rank 2

HB-BE-236 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
25.2	132	46	Fracture Rank 2
25.3	139	49	Foliation / Vein
25.4	326	63	Fracture Rank 1
25.7	136	51	Fracture Rank 1
26.3	180	60	Fracture Rank 2
26.8	200	67	Fracture Rank 2
27.6	154	46	Foliation / Vein
27.7	133	58	Fracture Rank 1
28.1	132	74	Fracture Rank 1
28.4	121	76	Fracture Rank 1
28.6	117	68	Foliation / Vein
28.8	111	68	Fracture Rank 2
29.1	119	69	Fracture Rank 2
29.3	115	70	Foliation / Vein
29.6	291	75	Fracture Rank 1
30.4	124	37	Fracture Rank 2
30.6	108	59	Fracture Rank 1
30.8	111	56	Foliation / Vein
31.0	164	56	Fracture Rank 1
31.1	278	80	Fracture Rank 1
31.2	148	48	Foliation / Vein
31.8	15	41	Foliation / Vein
31.9	289	67	Fracture Rank 1
31.9	22	41	Fracture Rank 1
32.0	30	39	Fracture Rank 1
32.1	166	71	Foliation / Vein

**BOREHOLE GEOPHYSICAL LOGGING - DATA REPORT
BOREHOLES BB-ECR-201, BB-ECR-203A, BB-ECR-204A,
BB-ECR-206A, BB-ELER-206A, HB-BE-235, HB-BE-237
BREWER-EDDINGTON I-395/ROUTE 9 CONNECTOR
EDDINGTON, MAINE**

Prepared for:

Haley & Aldrich, Inc.
75 Washington Avenue | Suite 1A
Portland, Maine 04101

Prepared by:

Hager-Richter Geoscience, Inc.
8 Industrial Way - D10
Salem, New Hampshire 03079

File 20RG77
March 2021

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Tadpole	Structure Category (Symbol Color)	Description
	Fracture Rank 1 (Light Blue)	Minor Fracture - not distinct and may not be continuous around the borehole
	Fracture Rank 2 (Blue)	Intermediate Fracture - distinct and continuous around the borehole with little or no apparent aperture
	Fracture Rank 3 (Light Green)	Intermediate Fracture - distinct and continuous around the borehole with some apparent aperture
	Fracture Rank 4 (Red)	Major Fracture - distinct with continuous apparent aperture around the borehole
	Foliation or Vein (Orange)	Planar geologic feature interpreted as foliation or a vein

Figure 1. Key to bedrock structure categories.

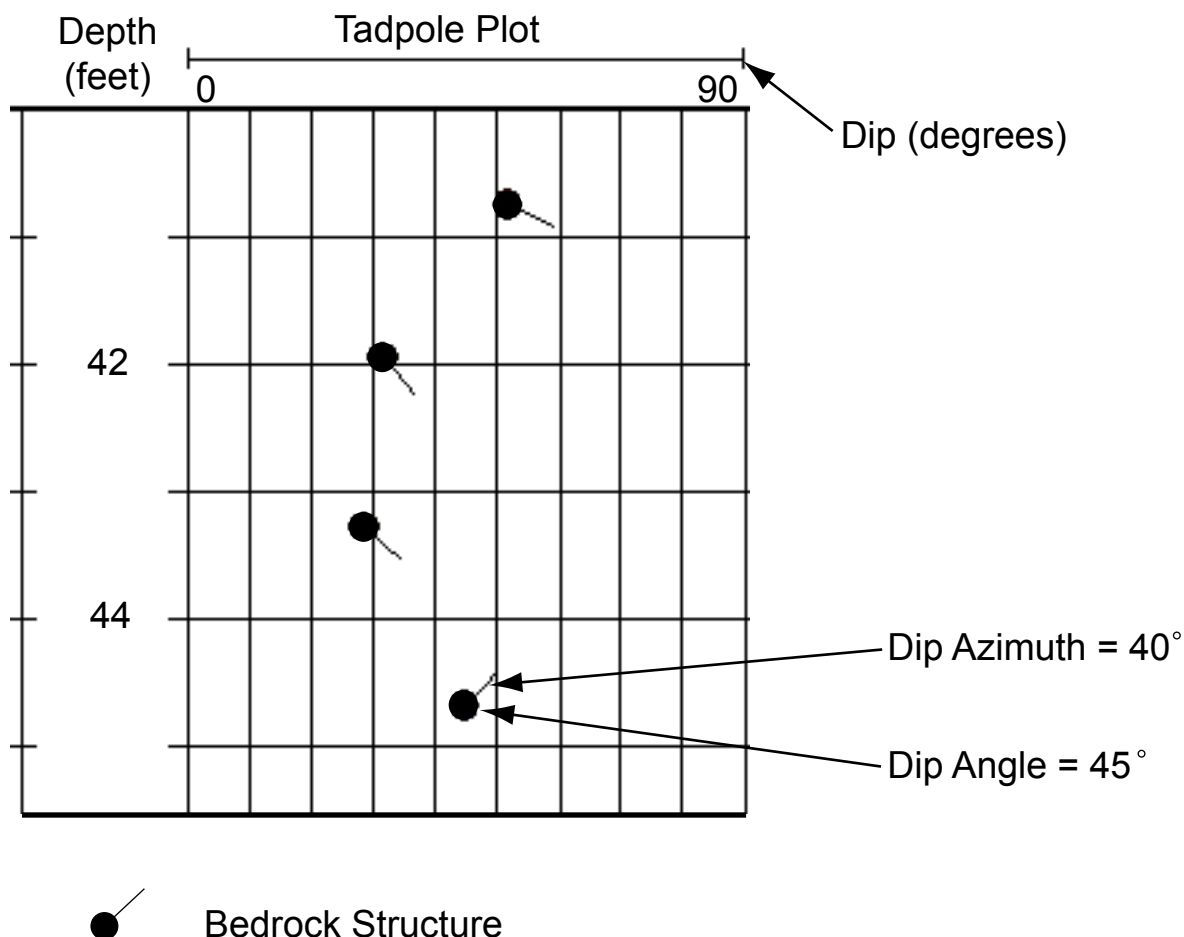


Figure 2. Tadpole plot explanation. The orientation of the bedrock structures is graphically displayed by a tadpole consisting of a circle, the head, and a line, the tail. The position of the head, left to right on the tadpole plot, gives the dip angle of the structure. The left side of the track indicates a dip angle of 0°, and the right side of the track indicates a dip angle of 90° from horizontal. The orientation of the tail gives the dip azimuth of the structure and can be read like a compass. The tail pointing directly up is 0°, north.

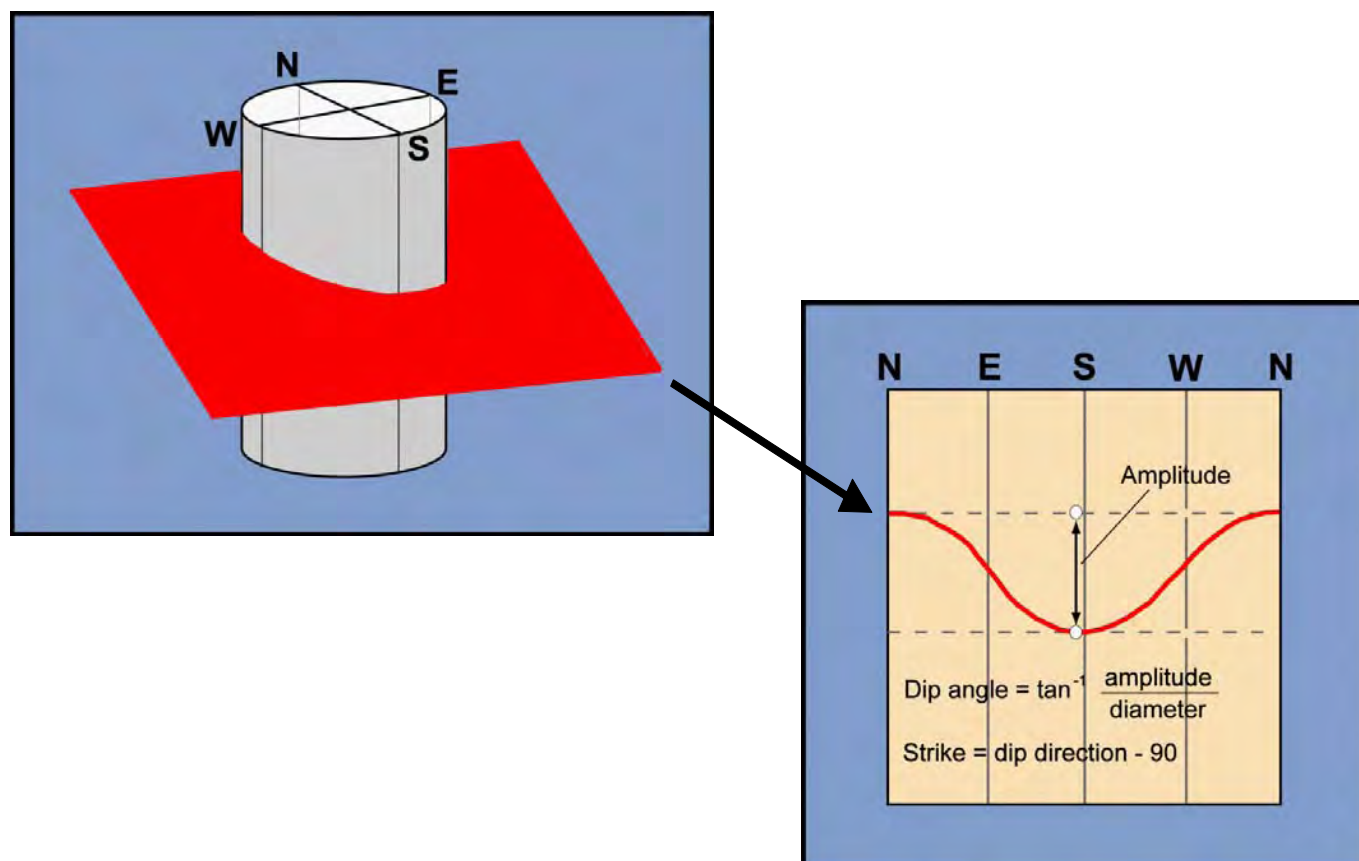


Figure 3. Televiewer Explanation Figure. The image on the left depicts a planar structure in red, such as a fracture or bedding plane, intersected by a borehole. The image on the right depicts the same structure unwrapped as it would be displayed in an optical televiewer (OTV) or acoustic televiewer (ATV) log.

Figure modified from: Garfield, R.L., Day-Lewis, F.D., Gray, M.B., Johnson, C.D., Williams, J.H. and Day-Lewis, A.D.F., 2003, Fractured-Rock Aquifer Characterization within a Regional Geologic Context: Results from the Bucknell University Hydrogeophysics Test Site, GSA Northeastern Section, 38th Annual Meeting, Paper No. 25-19.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: March 3, 2021

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

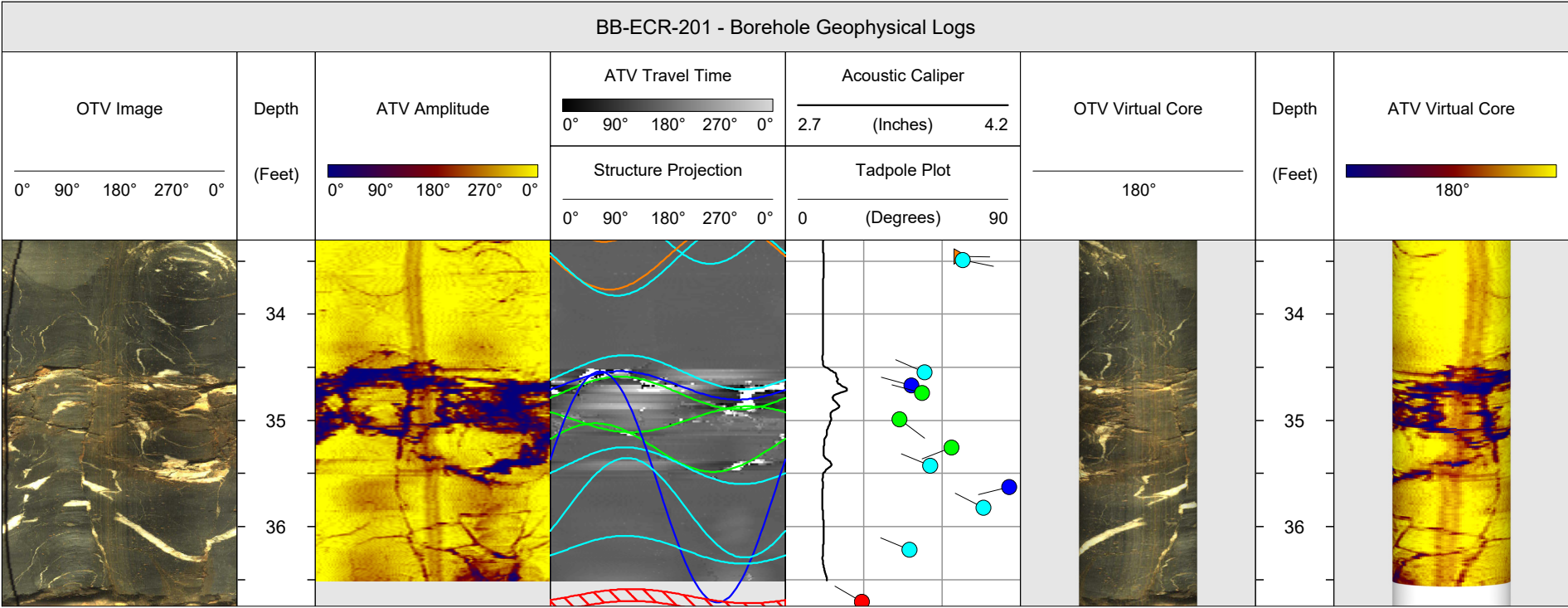
ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

BOREHOLE DIAMETER: 3.8 Inches

LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

OTV Image	Depth (Feet)	ATV Amplitude	ATV Travel Time	Acoustic Caliper	OTV Virtual Core	Depth (Feet)	ATV Virtual Core
0° 90° 180° 270° 0°		0° 90° 180° 270° 0°	0° 90° 180° 270° 0°	2.7 (Inches) 4.2	180°		0° 90° 180° 270° 0°
			Structure Projection	Tadpole Plot			
			0° 90° 180° 270° 0°	0 (Degrees) 90			0° 90° 180° 270° 0°
	17					Bottom of Casing	
	18						
	19						
	20						
	21						
	22						
	23						
	24						
	25						
	26						
	27						
	28						
	29						
	30						
	31						
	32						
	33						



Salem, New Hampshire
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Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: March 3, 2021

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

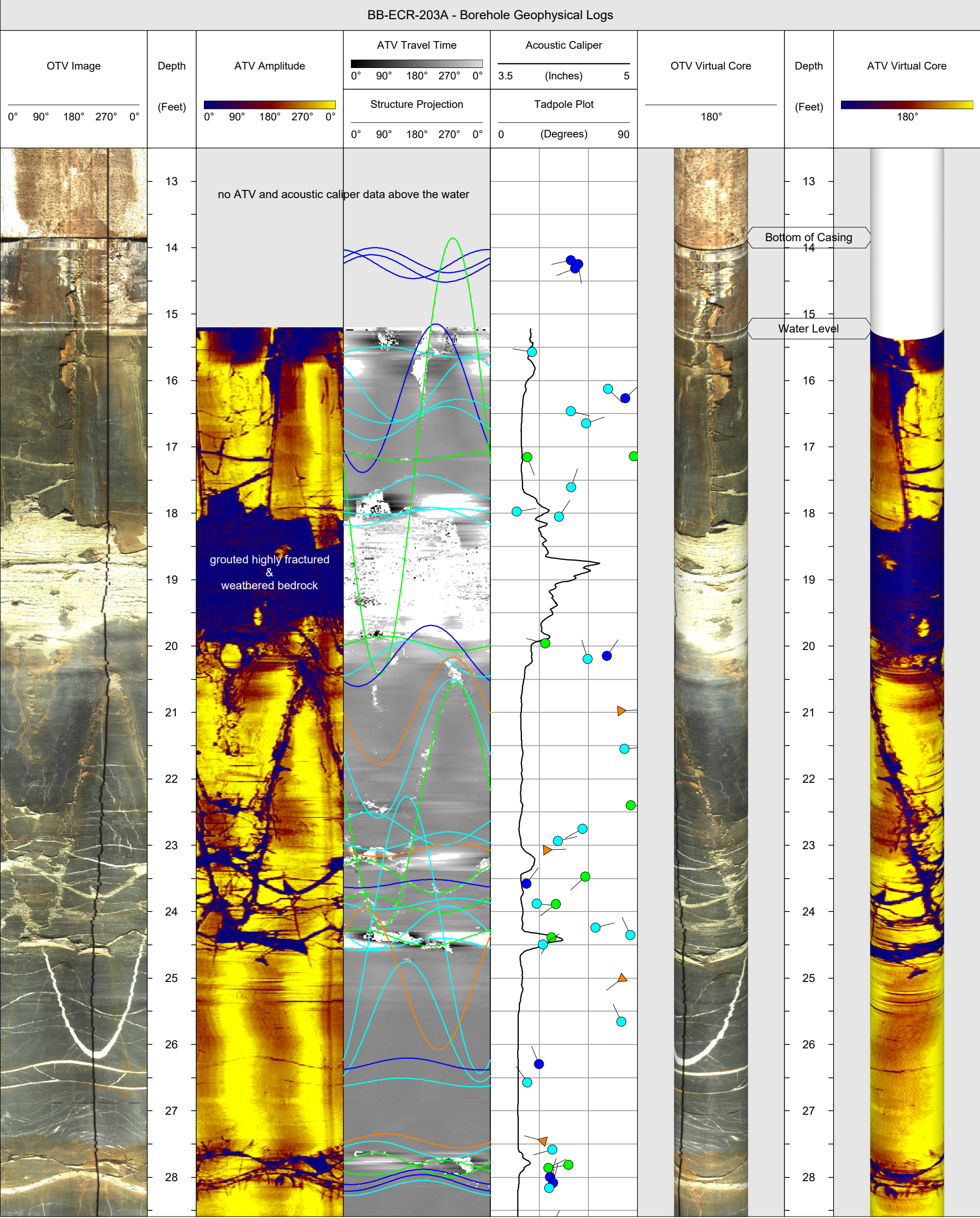
ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

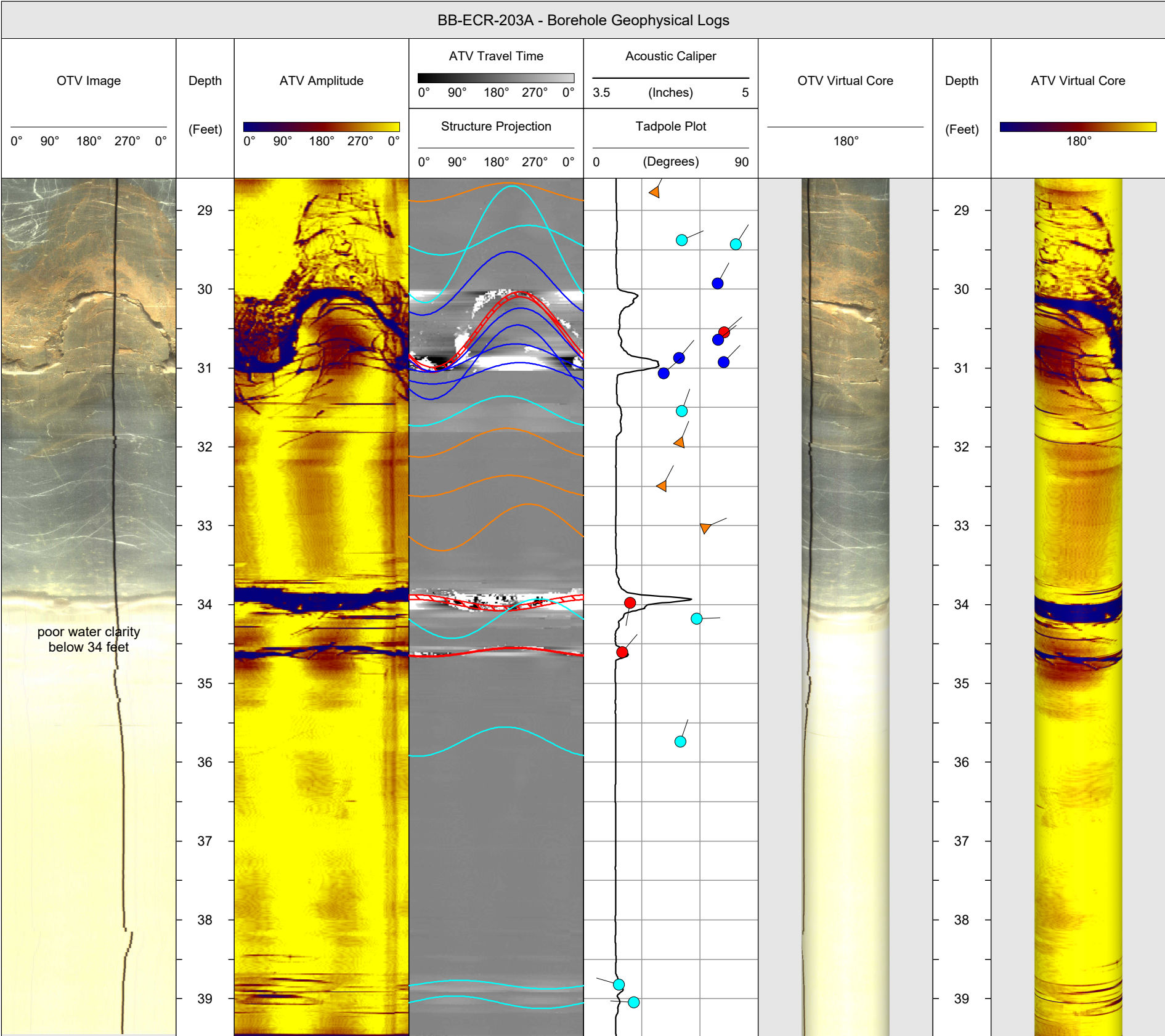
BOREHOLE DIAMETER: 3.8 Inches

LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

NOTES: Due to unstable, highly fractured and weathered, bedrock at ~18-20 feet in borehole BB-ECR-203A encountered during drilling, the drilling was suspended at a depth of ~25 feet to grout the bedrock. The grout stabilized bedrock was then drilled through and the drilling of the borehole was completed to total depth.





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Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: March 3, 2021

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

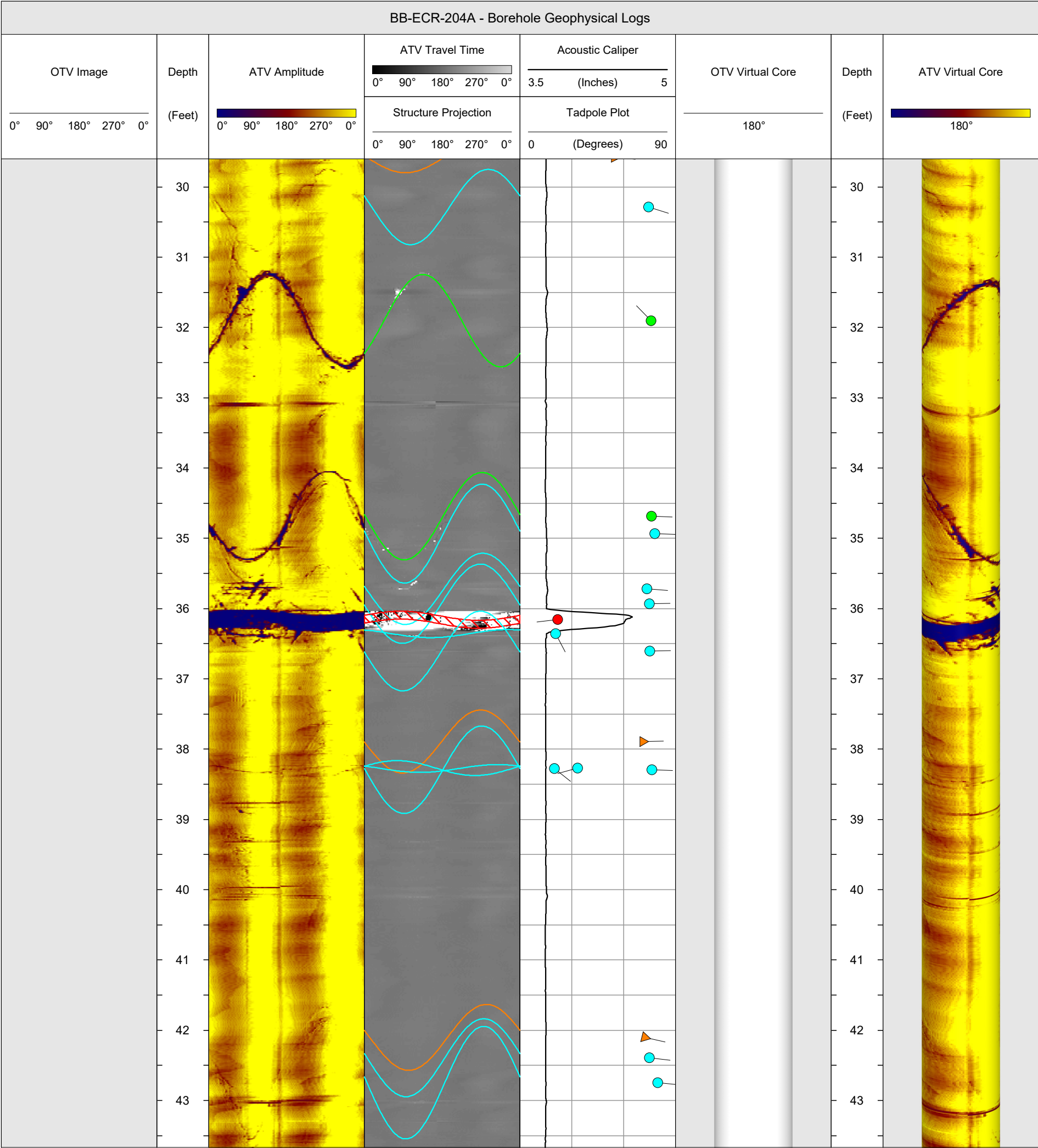
ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

BOREHOLE DIAMETER: 3.8 Inches

LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

OTV Image	Depth (Feet)	ATV Amplitude 0° 90° 180° 270° 0°	ATV Travel Time 0° 90° 180° 270° 0° Structure Projection 0° 90° 180° 270° 0°	Acoustic Caliper 3.5 (Inches) 5 Tadpole Plot 0 (Degrees) 90	OTV Virtual Core 180°	Depth (Feet)	ATV Virtual Core 180°
	13					13	
	14					14	
	15					15	
	16					16	
	17					17	
	18					18	
	19					19	
poor water clarity	20					20	
no OTV data below 21 feet due to poor water clarity	21					21	
	22					22	
	23					23	
	24					24	
	25					25	
	26					26	
	27					27	
	28					28	
	29					29	



HAGER-RICHTER GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

BB-ECR-206A - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: March 3, 2021

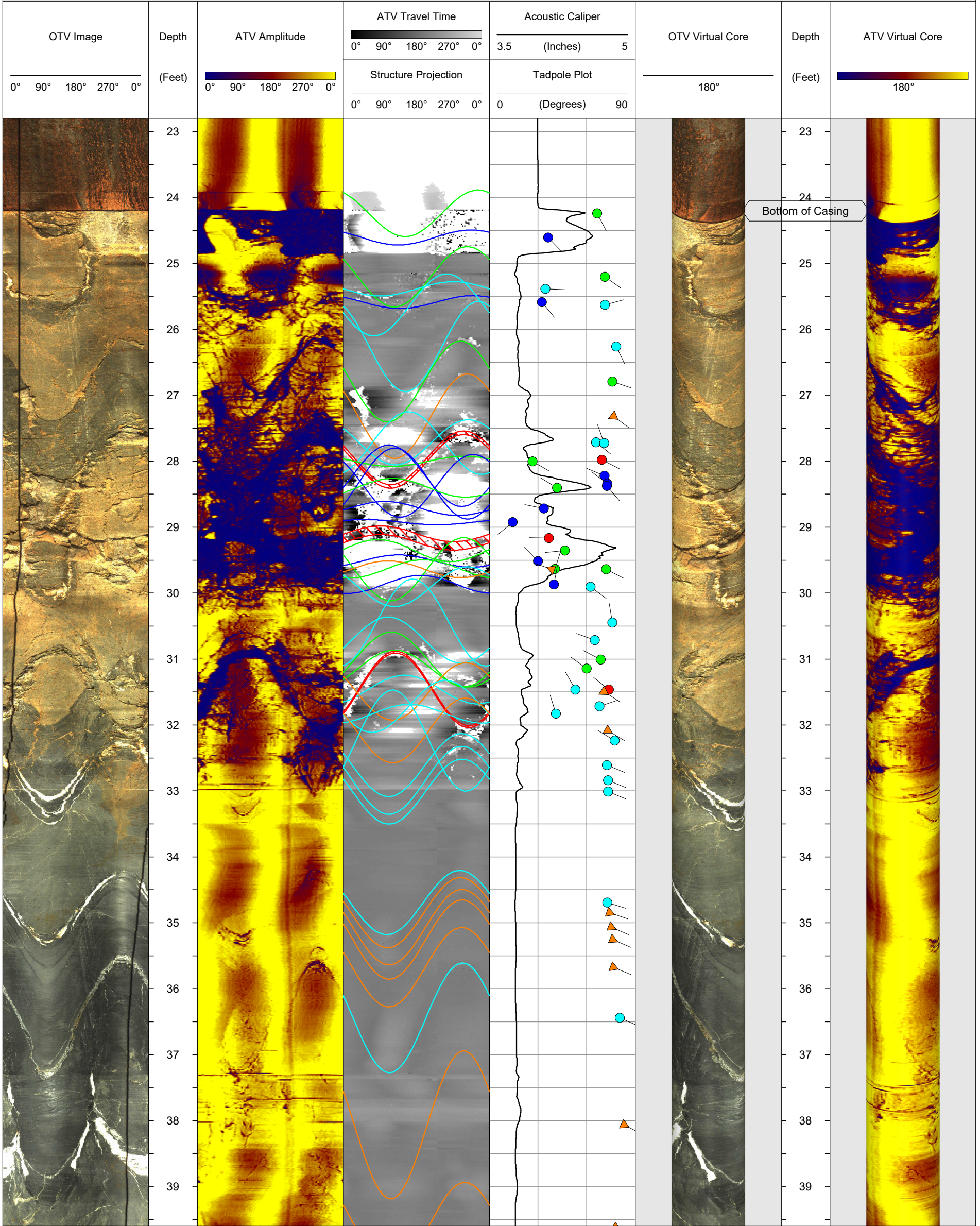
CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Mark Jones
PROJECT REP(S) ON-SITE: Josh Fletcher

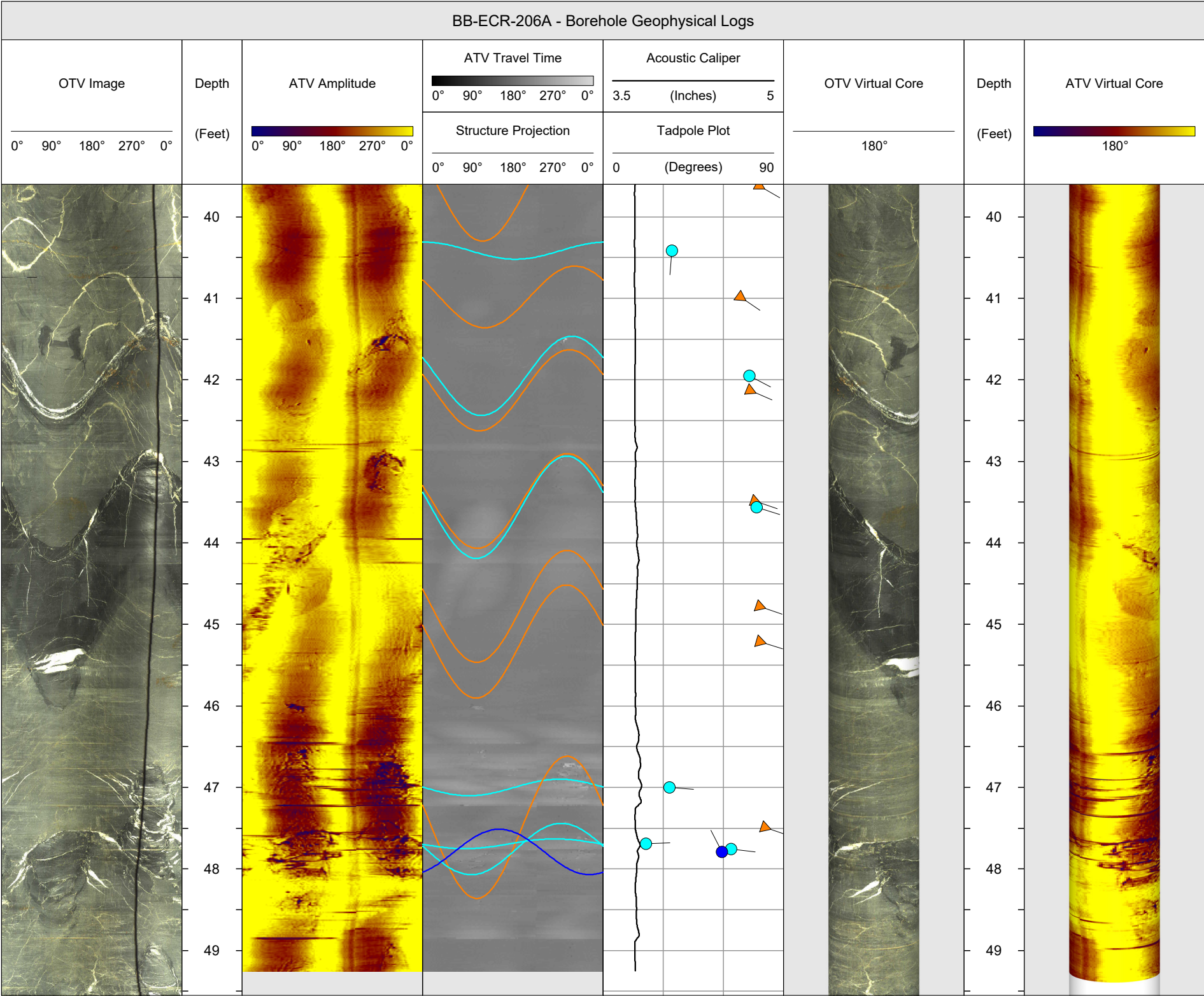
HRGS FILE:	20RG77
LOG DATUM:	Ground Surface
ORIENTATION REFERENCE:	True North (Magnetic Declination = 15.8° West)
BOREHOLE DIAMETER:	3.8 Inches
LOGS PROCESSED BY:	Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

BB-ECR-206A - Borehole Geophysical Logs





HAGER-RICHTER GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

BB-ELER-206A - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: March 3, 2021

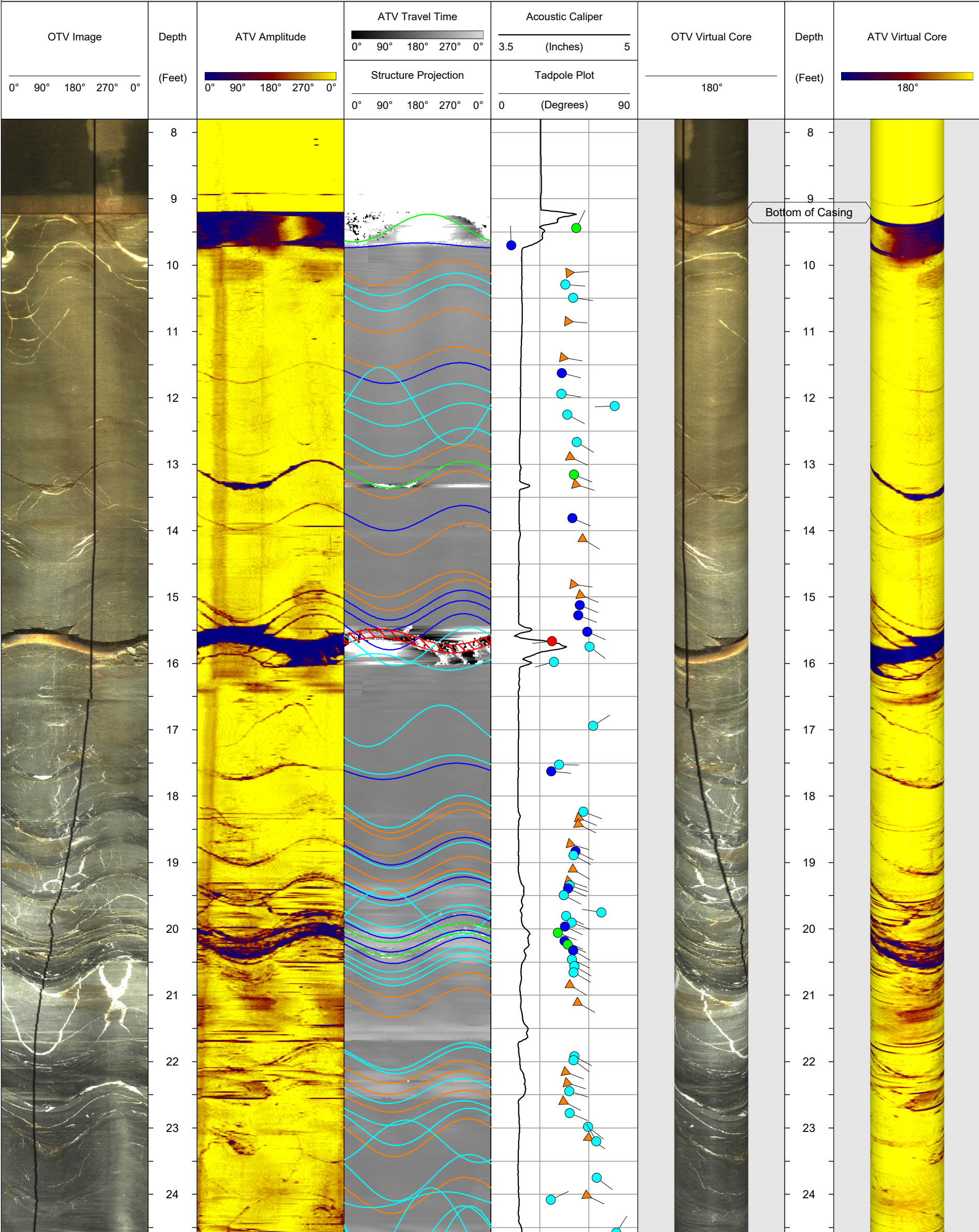
CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Mark Jones
PROJECT REP(S) ON-SITE: Josh Fletcher

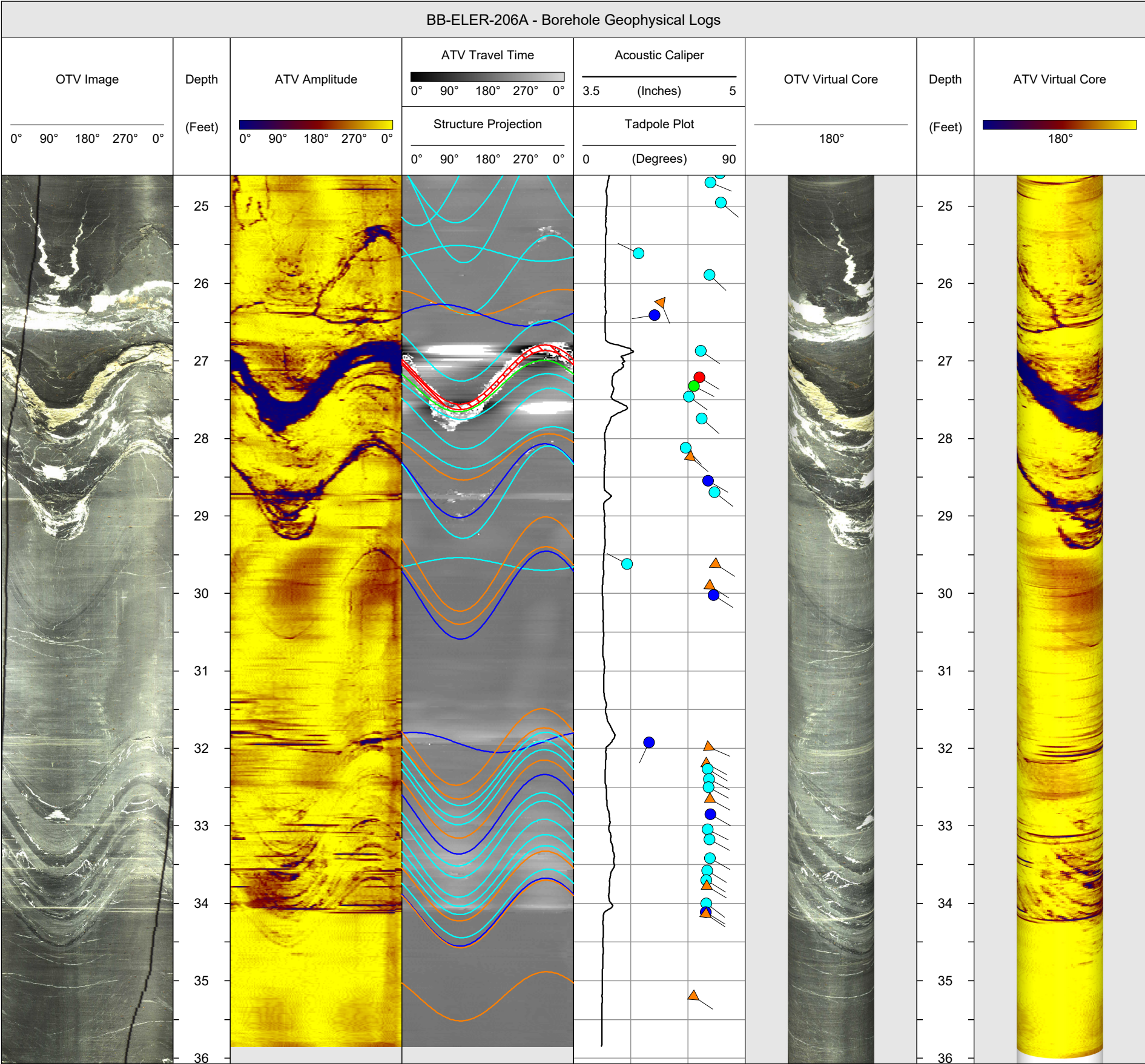
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LOG DATUM:	Ground Surface
ORIENTATION REFERENCE:	True North (Magnetic Declination = 15.8° West)
BOREHOLE DIAMETER:	3.8 Inches
LOGS PROCESSED BY:	Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

BB-ELER-206A - Borehole Geophysical Logs





HAGER-RICHTER GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

HB-BE-235 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine
LOGGING GEOPHYSICIST(S): Mikko Aarnio & Mark Jones
PROJECT REP(S) ON-SITE: Josh Fletcher

HRGS FILE: 20RG77

LOG DATUM: Ground Surface

ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

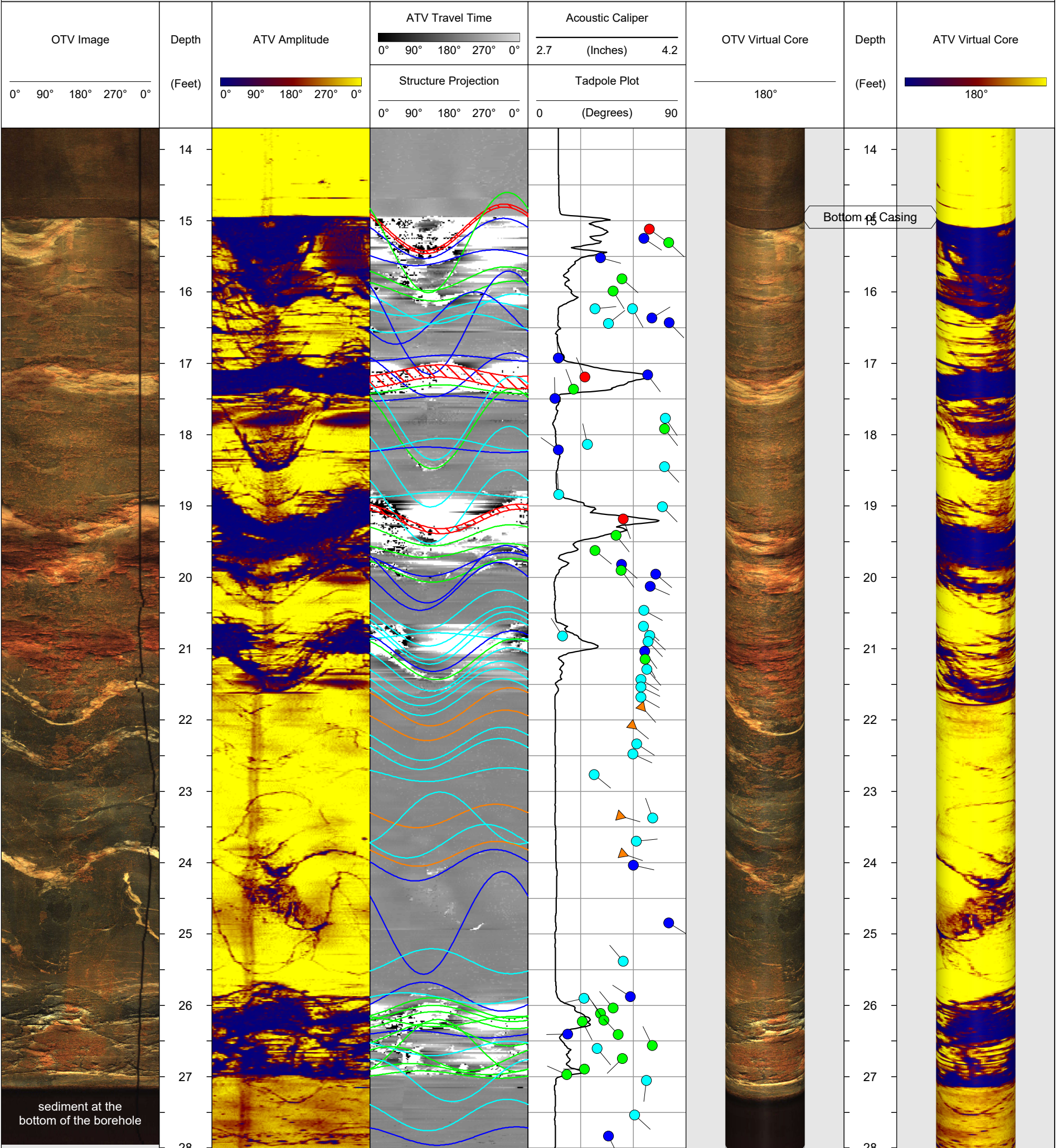
BOREHOLE DIAMETER: 3 Inches

LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND

 Fracture Rank 1
 Fracture Rank 2
 Fracture Rank 3
 Fracture Rank 4
 Foliation / Vein

HB-BE-235 - Borehole Geophysical Logs



HAGER-RICHTER GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

HB-BE-237 - BOREHOLE GEOPHYSICAL LOGS

DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.

PROJECT: Brewer-Eddington I-395/Route 9 Connector

LOCATION: Eddington, Maine

LOGGING GEOPHYSICIST(S): Mikko Aarnio & Mark Jones

PROJECT REP(S) ON-SITE: Josh Fletcher

HRGS FILE: 20RG77

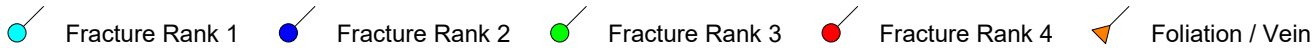
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ORIENTATION REFERENCE: True North (Magnetic Declination = 15.8° West)

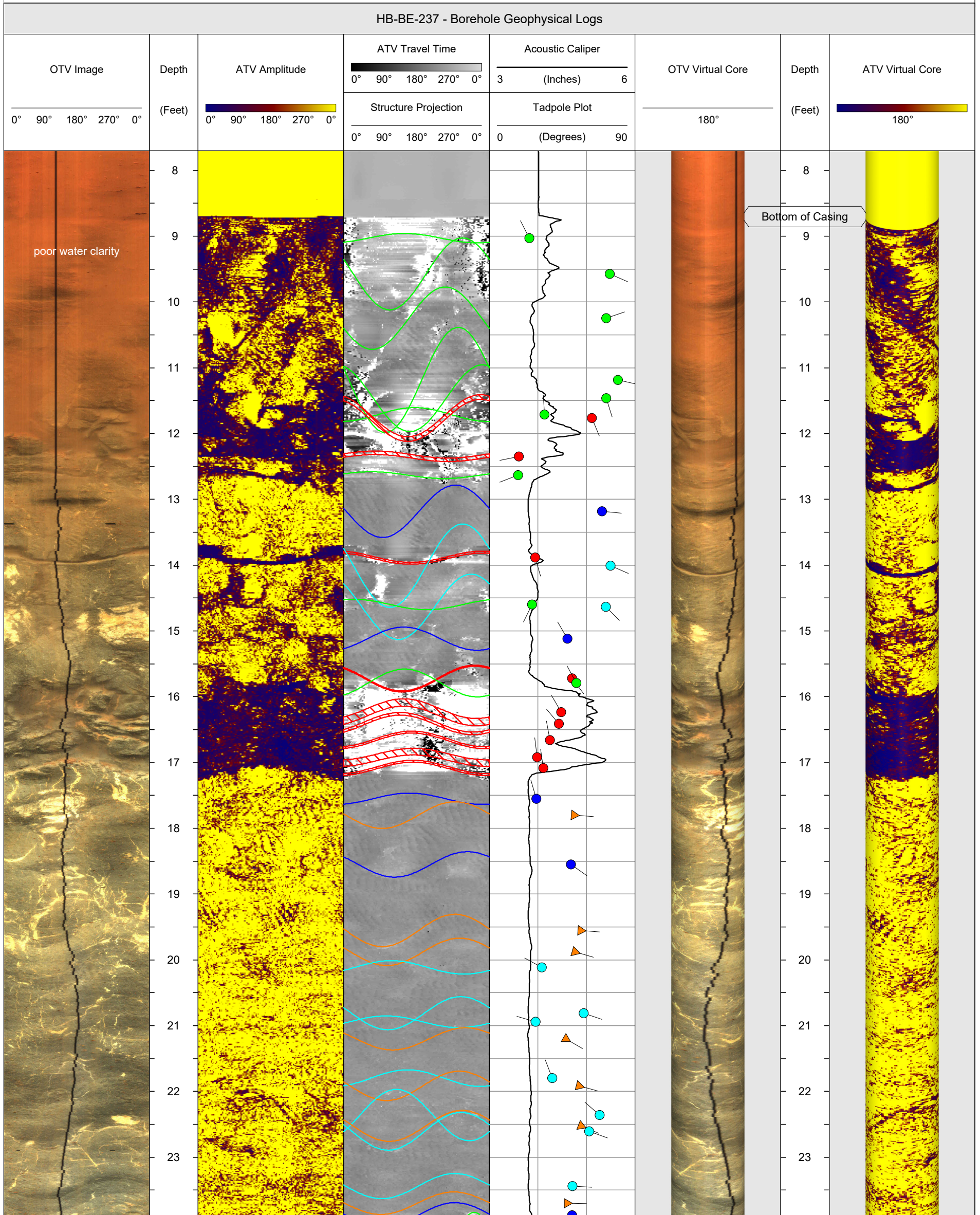
BOREHOLE DIAMETER: 3.8 Inches

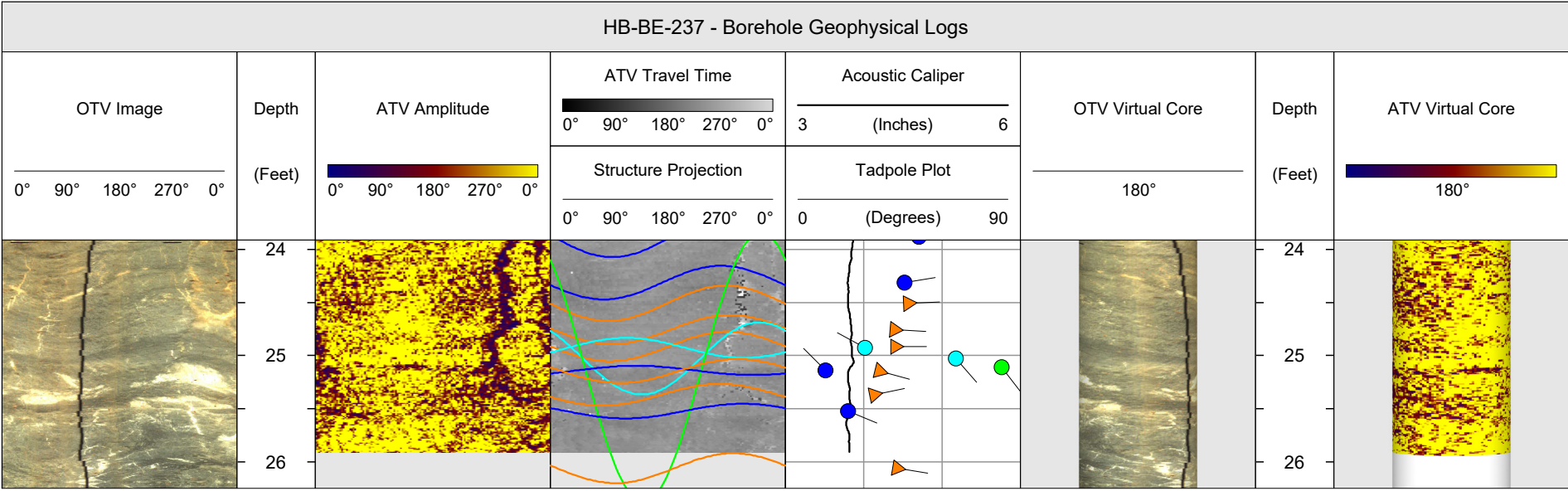
LOGS PROCESSED BY: Robert Garfield & Nick DeCristofaro

STRUCTURE LEGEND



NOTE: The borehole was reamed to approximately 3.8 inches after NQ coring.





HAGER-RICHTER
GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

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Tel: 732.661.0555

ALL BOREHOLES FROM NOVEMBER 2020 & MARCH 2021 - BEDROCK STRUCTURE STATISTICS PLOTS

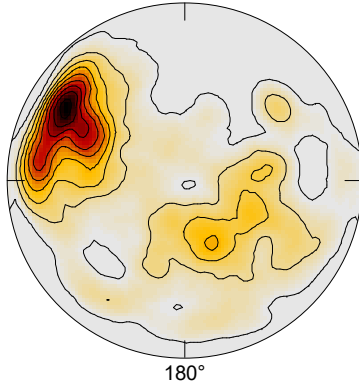
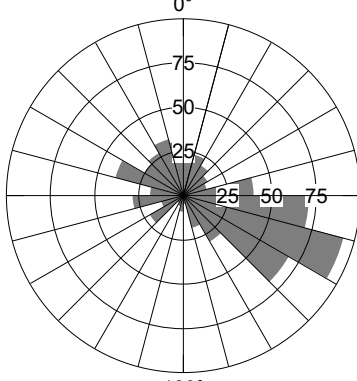
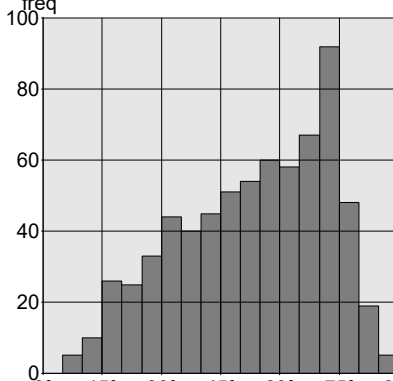
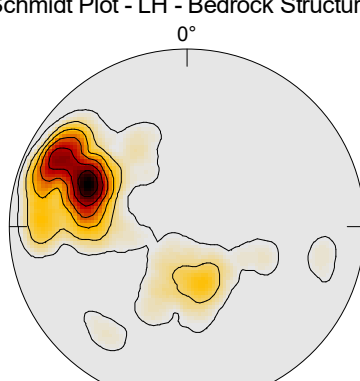
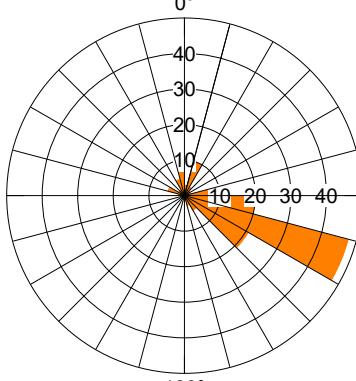
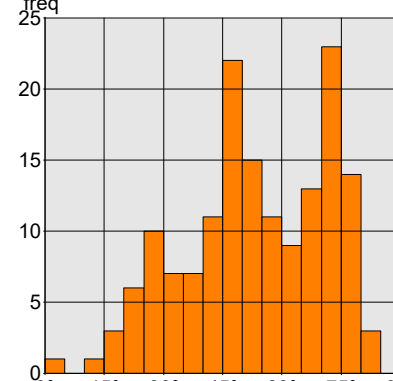
DATE(S) LOGGED: November 24, 2020 & March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>682</td><td>53.27</td><td>83.43</td></tr><tr><td>●</td><td>134</td><td>51.71</td><td>72.78</td></tr><tr><td>●</td><td>165</td><td>52.25</td><td>86.53</td></tr><tr><td>●</td><td>319</td><td>55.92</td><td>87.55</td></tr><tr><td>●</td><td>64</td><td>45.74</td><td>333.67</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	682	53.27	83.43	●	134	51.71	72.78	●	165	52.25	86.53	●	319	55.92	87.55	●	64	45.74	333.67	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>682.00</td></tr><tr><td>Mean (2D):</td><td>83.43</td></tr><tr><td>Std.Dev.:</td><td>99.02</td></tr><tr><td>Min:</td><td>0.63</td></tr><tr><td>Max:</td><td>359.03</td></tr></table>	Components:	Azimuth	Counts:	682.00	Mean (2D):	83.43	Std.Dev.:	99.02	Min:	0.63	Max:	359.03	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>682.00</th></tr><tr><th>Mean (2D):</th><th>53.27</th></tr><tr><th>Std.Dev.:</th><th>19.01</th></tr><tr><th>Min:</th><th>5.57</th></tr><tr><th>Max:</th><th>87.81</th></tr></table>	Counts:	682.00	Mean (2D):	53.27	Std.Dev.:	19.01	Min:	5.57	Max:	87.81	<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>156</td><td>53.44</td><td>93.26</td></tr><tr><td>▶</td><td>156</td><td>53.44</td><td>93.26</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	156	53.44	93.26	▶	156	53.44	93.26	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>156.00</td></tr><tr><td>Mean (2D):</td><td>93.26</td></tr><tr><td>Std.Dev.:</td><td>59.00</td></tr><tr><td>Min:</td><td>3.36</td></tr><tr><td>Max:</td><td>358.36</td></tr></table>	Components:	Azimuth	Counts:	156.00	Mean (2D):	93.26	Std.Dev.:	59.00	Min:	3.36	Max:	358.36	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>156.00</th></tr><tr><th>Mean (2D):</th><th>53.44</th></tr><tr><th>Std.Dev.:</th><th>17.87</th></tr><tr><th>Min:</th><th>2.88</th></tr><tr><th>Max:</th><th>83.09</th></tr></table>	Counts:	156.00	Mean (2D):	53.44	Std.Dev.:	17.87	Min:	2.88	Max:	83.09
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HAGER-RICHTER
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BB-ECR-201 - BEDROCK STRUCTURE STATISTICS PLOTS

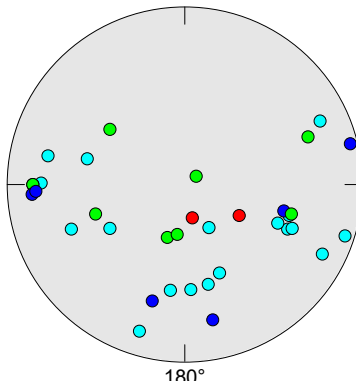
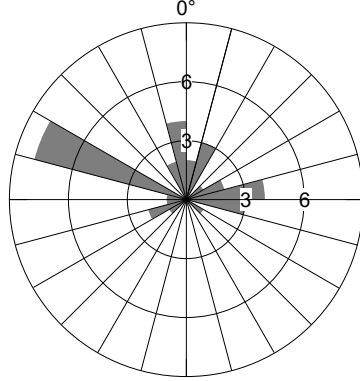
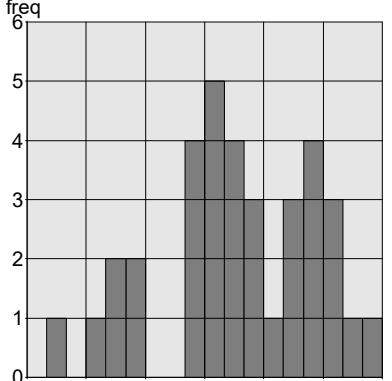
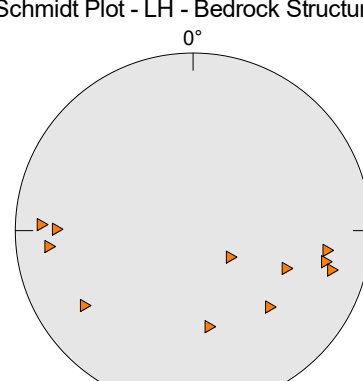
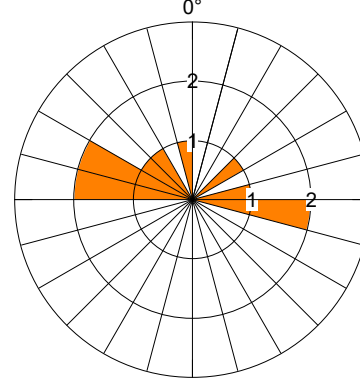
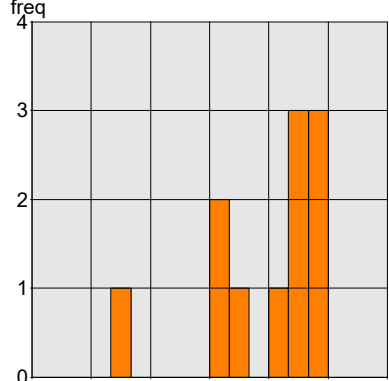
DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
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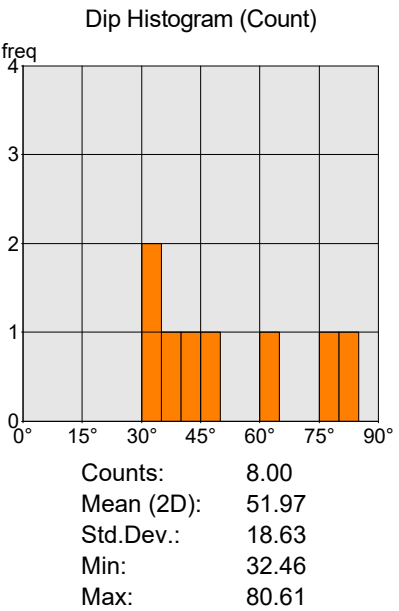
Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: March 3, 2021

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein

Dip Angle Histogram of Foliation & Veins



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BB-ECR-204A - BEDROCK STRUCTURE STATISTICS PLOTS

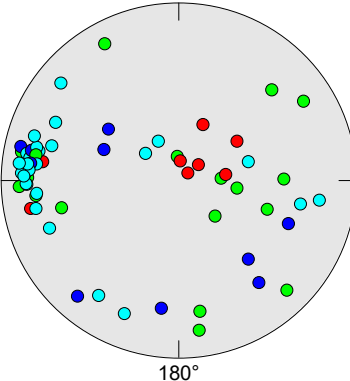
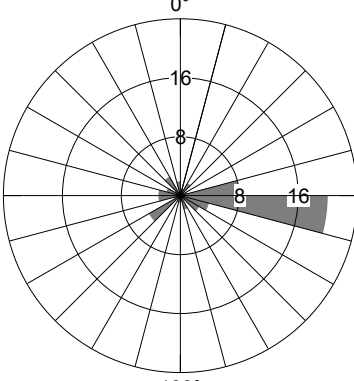
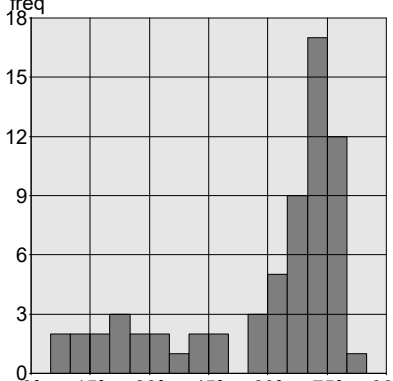
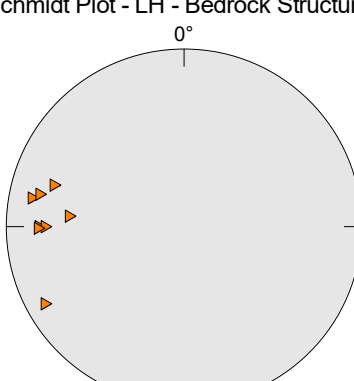
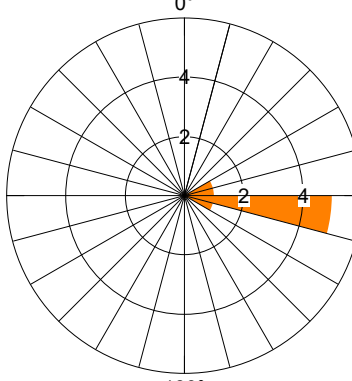
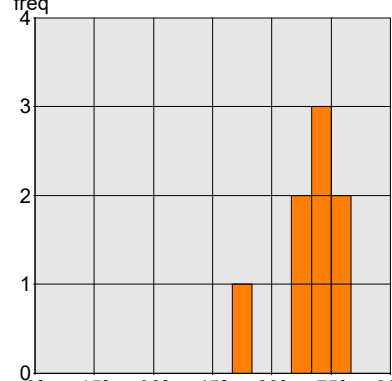
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BB-ECR-206A - BEDROCK STRUCTURE STATISTICS PLOTS

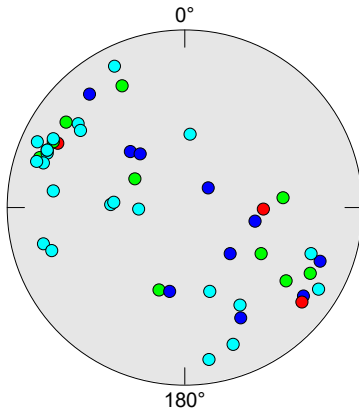
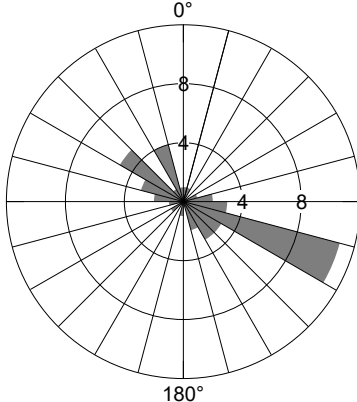
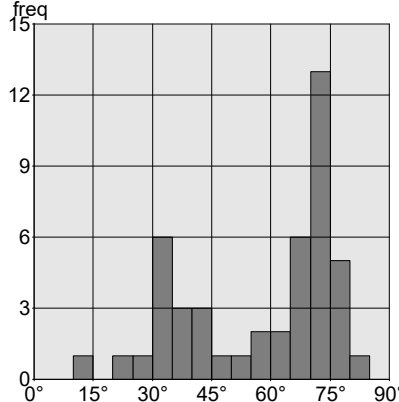
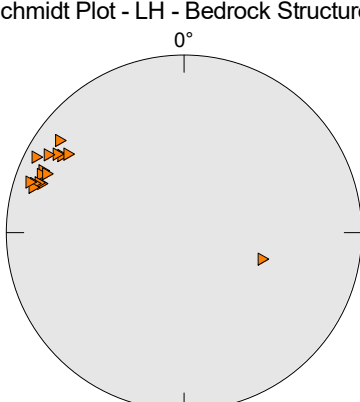
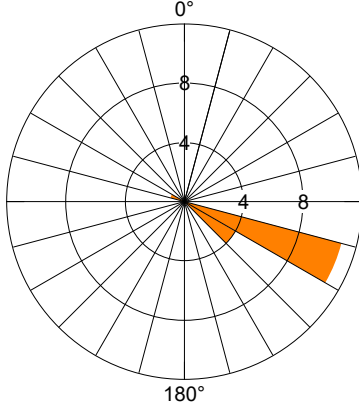
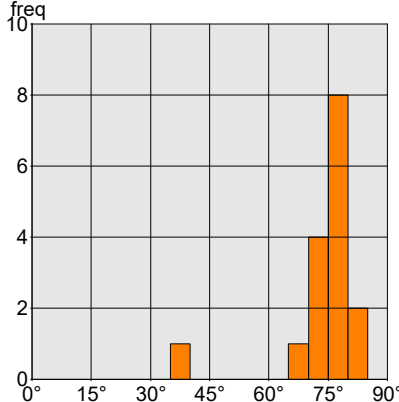
DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>46</td><td>57.81</td><td>90.09</td></tr><tr><td>●</td><td>10</td><td>57.10</td><td>113.80</td></tr><tr><td>●</td><td>10</td><td>46.12</td><td>288.93</td></tr><tr><td>●</td><td>23</td><td>62.79</td><td>93.78</td></tr><tr><td>●</td><td>3</td><td>60.06</td><td>283.03</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	46	57.81	90.09	●	10	57.10	113.80	●	10	46.12	288.93	●	23	62.79	93.78	●	3	60.06	283.03	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>46.00</td></tr><tr><td>Mean (2D):</td><td>90.09</td></tr><tr><td>Std.Dev.:</td><td>104.75</td></tr><tr><td>Min:</td><td>10.44</td></tr><tr><td>Max:</td><td>350.99</td></tr></table>	Components:	Azimuth	Counts:	46.00	Mean (2D):	90.09	Std.Dev.:	104.75	Min:	10.44	Max:	350.99	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>46.00</th></tr><tr><th>Mean (2D):</th><th>57.81</th></tr><tr><th>Std.Dev.:</th><th>18.59</th></tr><tr><th>Min:</th><th>14.35</th></tr><tr><th>Max:</th><th>80.32</th></tr></table>	Counts:	46.00	Mean (2D):	57.81	Std.Dev.:	18.59	Min:	14.35	Max:	80.32	<div>Schmidt Plot - LH - Bedrock Structures</div>  <table><tr><th></th><th>Counts</th><th>Dip[deg]</th><th>Azi[deg]</th></tr><tr><td>Mean</td><td>16</td><td>73.59</td><td>115.47</td></tr><tr><td>▶</td><td>16</td><td>73.59</td><td>115.47</td></tr></table>		Counts	Dip[deg]	Azi[deg]	Mean	16	73.59	115.47	▶	16	73.59	115.47	<div>Azimuth - Absolute (Count)</div>  <table><tr><th>Components:</th><th>Azimuth</th></tr><tr><td>Counts:</td><td>16.00</td></tr><tr><td>Mean (2D):</td><td>115.47</td></tr><tr><td>Std.Dev.:</td><td>30.26</td></tr><tr><td>Min:</td><td>106.88</td></tr><tr><td>Max:</td><td>289.11</td></tr></table>	Components:	Azimuth	Counts:	16.00	Mean (2D):	115.47	Std.Dev.:	30.26	Min:	106.88	Max:	289.11	<div>Dip Histogram (Count)</div>  <table><tr><th>Counts:</th><th>16.00</th></tr><tr><th>Mean (2D):</th><th>73.59</th></tr><tr><th>Std.Dev.:</th><th>9.59</th></tr><tr><th>Min:</th><th>38.46</th></tr><tr><th>Max:</th><th>83.09</th></tr></table>	Counts:	16.00	Mean (2D):	73.59	Std.Dev.:	9.59	Min:	38.46	Max:	83.09
	Counts	Dip[deg]	Azi[deg]																																																																																		
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HAGER-RICHTER
GEOSCIENCE, INC.

Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

BB-ELER-206A - BEDROCK STRUCTURE STATISTICS PLOTS

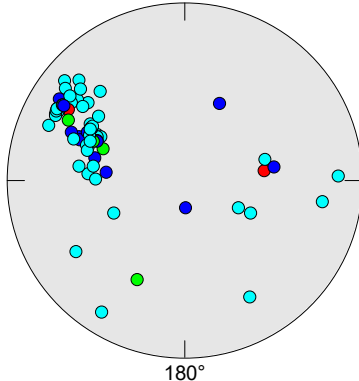
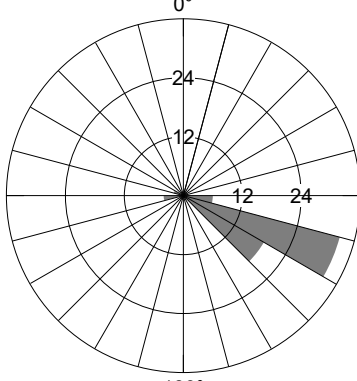
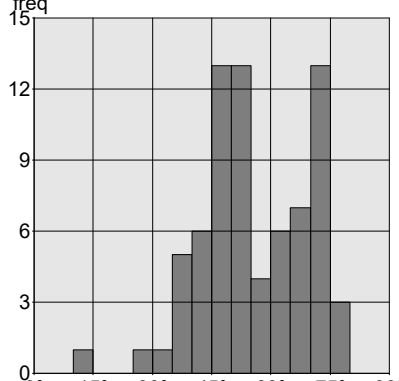
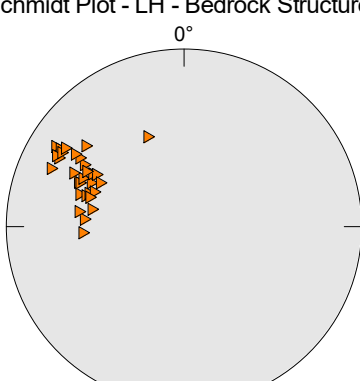
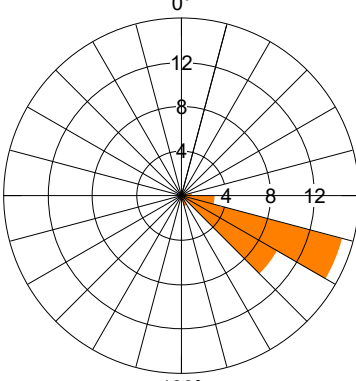
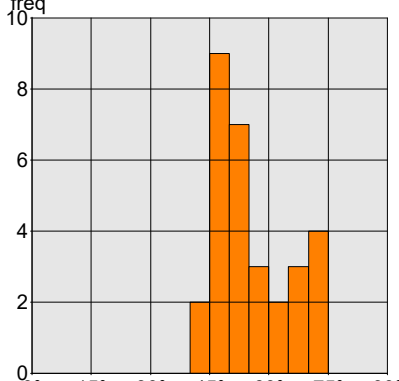
DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

Fracture Rank 1 Fracture Rank 2 Fracture Rank 3 Fracture Rank 4 Foliation / Vein

Stereogram - Lower Hemisphere of Bedrock Fractures	Dip Azimuth Rose Diagram of Bedrock Fractures	Dip Angle Histogram of Bedrock Fractures	Stereogram - Lower Hemisphere of Foliation & Veins	Dip Azimuth Rose Diagram of Foliation & Veins	Dip Angle Histogram of Foliation & Veins																																																																																
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Salem, New Hampshire
Tel: 603.893.9944

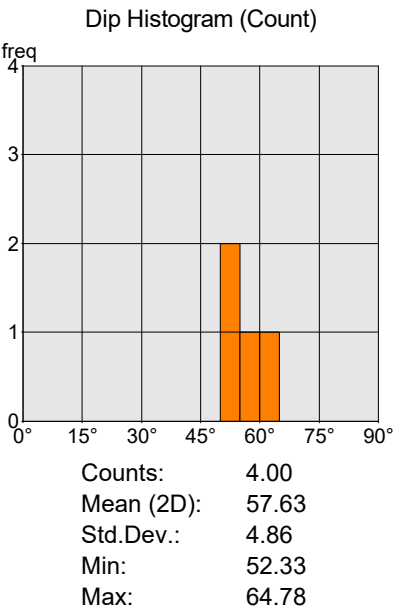
Fords, New Jersey
Tel: 732.661.0555

DATE(S) LOGGED: March 3, 2021

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein

Dip Angle Histogram of Foliation & Veins



Salem, New Hampshire
Tel: 603.893.9944

Fords, New Jersey
Tel: 732.661.0555

HB-BE-237 - BEDROCK STRUCTURE STATISTICS PLOTS

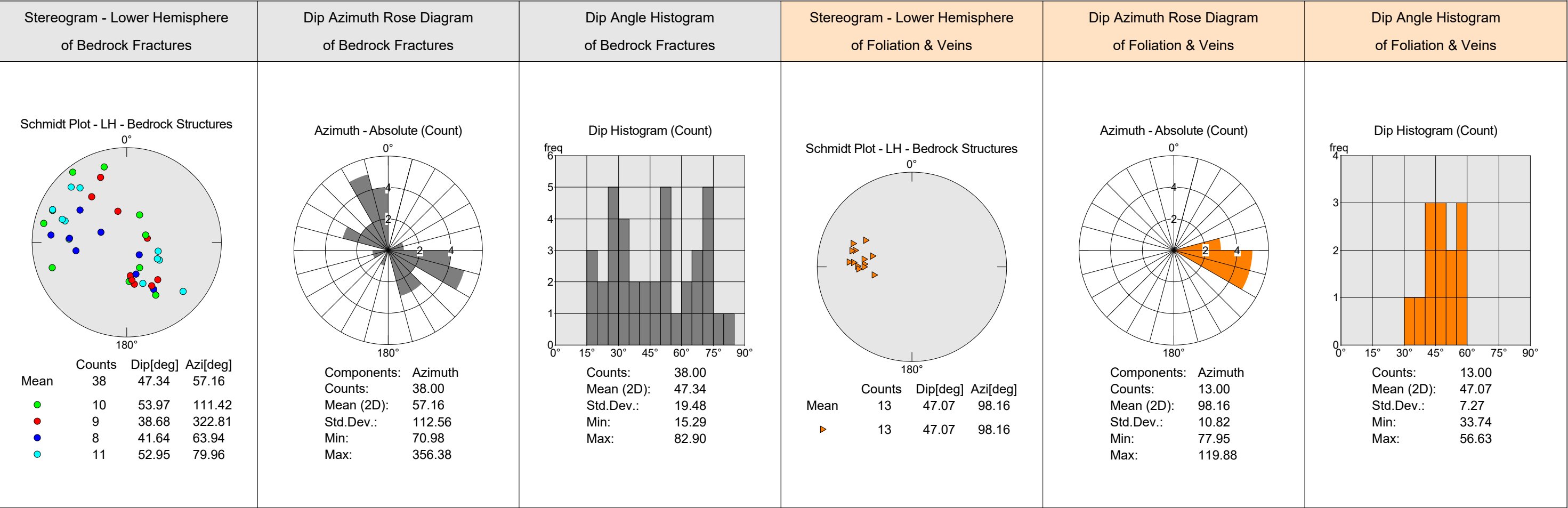
DATE(S) LOGGED: March 3, 2021

CLIENT: Haley & Aldrich, Inc.
PROJECT: Brewer-Eddington I-395/Route 9 Connector
LOCATION: Eddington, Maine

HRGS FILE: 20RG77
ORIENTATION REFERENCE: True North
MAGNETIC DECLINATION: 15.8° West

STRUCTURE LEGEND

● Fracture Rank 1 ● Fracture Rank 2 ● Fracture Rank 3 ● Fracture Rank 4 ▲ Foliation / Vein



HAGER-RICHTER GEOSCIENCE, INC.	
BB-ECR-201 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ECR-201 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
17.3	55	64	Foliation / Vein
18.0	347	48	Fracture Rank 1
19.5	233	7	Fracture Rank 3
20.1	330	23	Fracture Rank 1
20.1	348	67	Fracture Rank 2
22.1	278	65	Foliation / Vein
22.4	283	66	Foliation / Vein
22.9	17	75	Fracture Rank 1
24.6	287	52	Fracture Rank 1
25.3	292	47	Foliation / Vein
25.7	286	70	Foliation / Vein
26.4	18	26	Fracture Rank 3
26.7	315	51	Foliation / Vein
27.1	306	21	Foliation / Vein
27.2	346	16	Fracture Rank 4
27.4	8	51	Fracture Rank 1
28.0	9	23	Fracture Rank 3
28.1	16	58	Fracture Rank 2
28.2	357	50	Fracture Rank 1
28.3	351	46	Foliation / Vein
28.5	105	48	Fracture Rank 1
29.3	91	70	Fracture Rank 1
29.8	72	44	Fracture Rank 3
29.8	93	75	Foliation / Vein
29.9	60	41	Fracture Rank 1
30.1	90	75	Fracture Rank 4
30.2	90	75	Fracture Rank 3
30.5	86	75	Fracture Rank 2
30.6	68	58	Fracture Rank 1
31.7	288	84	Fracture Rank 1
31.9	338	45	Fracture Rank 1
32.0	87	73	Fracture Rank 2
32.9	84	71	Foliation / Vein
33.2	245	73	Fracture Rank 1
33.5	91	66	Foliation / Vein

BB-ECR-201 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
33.5	102	68	Fracture Rank 1
34.6	294	53	Fracture Rank 1
34.7	285	48	Fracture Rank 2
34.7	285	52	Fracture Rank 3
35.0	126	44	Fracture Rank 3
35.3	249	64	Fracture Rank 3
35.4	292	55	Fracture Rank 1
35.6	256	86	Fracture Rank 2
35.8	297	76	Fracture Rank 1
36.2	293	47	Fracture Rank 1
36.7	300	29	Fracture Rank 4

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ECR-203A - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ECR-203A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
14.2	257	49	Fracture Rank 2
14.3	171	54	Fracture Rank 2
14.3	249	52	Fracture Rank 2
15.6	279	25	Fracture Rank 1
16.1	134	72	Fracture Rank 1
16.3	47	83	Fracture Rank 2
16.5	104	49	Fracture Rank 1
16.7	72	59	Fracture Rank 1
17.1	88	88	Fracture Rank 3
17.2	158	23	Fracture Rank 3
17.6	19	49	Fracture Rank 1
18.0	81	16	Fracture Rank 1
18.1	33	42	Fracture Rank 1
20.0	284	34	Fracture Rank 3
20.2	35	71	Fracture Rank 2
20.2	343	59	Fracture Rank 1
21.0	87	80	Foliation / Vein
21.6	85	82	Fracture Rank 1
22.4	97	86	Fracture Rank 3
22.8	242	56	Fracture Rank 1
22.9	77	41	Fracture Rank 1
23.1	87	34	Foliation / Vein
23.5	227	58	Fracture Rank 3
23.6	37	22	Fracture Rank 2
23.9	96	28	Fracture Rank 1
23.9	233	40	Fracture Rank 3
24.2	76	64	Fracture Rank 1
24.4	335	86	Fracture Rank 1
24.4	212	38	Fracture Rank 3
24.5	57	32	Fracture Rank 1
25.0	233	81	Foliation / Vein
25.7	335	80	Fracture Rank 1
26.3	338	30	Fracture Rank 2
26.6	326	22	Fracture Rank 1
27.5	286	32	Foliation / Vein

BB-ECR-203A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
27.6	254	38	Fracture Rank 1
27.8	260	48	Fracture Rank 3
27.9	64	36	Fracture Rank 3
28.0	18	37	Fracture Rank 2
28.1	14	38	Fracture Rank 2
28.2	15	36	Fracture Rank 1
28.8	26	37	Foliation / Vein
29.4	66	51	Fracture Rank 1
29.4	33	79	Fracture Rank 1
29.9	28	69	Fracture Rank 2
30.5	49	73	Fracture Rank 4
30.6	51	69	Fracture Rank 2
30.9	40	49	Fracture Rank 2
30.9	44	72	Fracture Rank 2
31.1	49	41	Fracture Rank 2
31.5	20	51	Fracture Rank 1
32.0	23	50	Foliation / Vein
32.5	28	41	Foliation / Vein
33.0	67	63	Foliation / Vein
34.0	191	24	Fracture Rank 4
34.2	88	58	Fracture Rank 1
34.6	40	20	Fracture Rank 4
35.7	19	50	Fracture Rank 1
38.8	286	18	Fracture Rank 1
39.0	274	26	Fracture Rank 1

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ECR-204A - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ECR-204A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
14.4	8	62	Fracture Rank 2
14.9	22	70	Fracture Rank 1
14.9	292	56	Fracture Rank 2
14.9	61	79	Foliation / Vein
14.9	88	79	Fracture Rank 3
15.3	278	69	Fracture Rank 1
15.6	281	59	Fracture Rank 1
16.0	288	44	Fracture Rank 3
16.1	269	50	Fracture Rank 3
16.4	231	12	Fracture Rank 4
16.6	204	28	Fracture Rank 4
16.6	351	64	Fracture Rank 3
16.9	278	27	Fracture Rank 3
16.9	266	20	Fracture Rank 3
17.0	352	74	Fracture Rank 3
17.6	126	41	Fracture Rank 2
17.7	319	50	Fracture Rank 2
17.9	91	74	Fracture Rank 3
17.9	322	63	Fracture Rank 2
18.4	226	63	Fracture Rank 3
18.7	41	76	Fracture Rank 2
18.8	112	38	Fracture Rank 2
18.9	152	77	Fracture Rank 3
19.0	238	72	Fracture Rank 3
19.3	79	74	Fracture Rank 4
19.8	236	33	Fracture Rank 4
19.9	98	66	Fracture Rank 4
20.0	100	79	Fracture Rank 3
20.6	95	74	Fracture Rank 1
20.9	101	76	Foliation / Vein
21.2	102	80	Fracture Rank 2
21.3	100	76	Fracture Rank 1
21.7	99	74	Fracture Rank 1
22.1	77	58	Fracture Rank 3
22.2	84	70	Fracture Rank 3

BB-ECR-204A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
22.4	79	71	Fracture Rank 1
22.6	90	71	Foliation / Vein
22.9	115	66	Fracture Rank 1
23.5	70	67	Fracture Rank 1
23.8	90	68	Foliation / Vein
24.2	85	69	Fracture Rank 1
24.7	35	68	Fracture Rank 1
24.8	105	64	Fracture Rank 1
25.1	176	11	Fracture Rank 3
25.2	130	75	Fracture Rank 1
25.8	108	66	Foliation / Vein
25.9	102	70	Fracture Rank 1
26.3	102	74	Fracture Rank 2
26.5	228	6	Fracture Rank 4
26.5	103	71	Fracture Rank 1
26.6	100	71	Fracture Rank 3
26.7	97	70	Fracture Rank 1
27.0	96	73	Fracture Rank 1
27.9	97	73	Fracture Rank 2
28.1	315	24	Fracture Rank 3
28.3	185	9	Fracture Rank 4
29.6	95	55	Foliation / Vein
30.3	107	74	Fracture Rank 1
31.9	315	76	Fracture Rank 3
34.7	92	76	Fracture Rank 3
34.9	93	78	Fracture Rank 1
35.7	94	73	Fracture Rank 1
35.9	88	75	Fracture Rank 1
36.2	263	22	Fracture Rank 4
36.4	152	21	Fracture Rank 1
36.6	89	75	Fracture Rank 1
37.9	89	71	Foliation / Vein
38.3	255	33	Fracture Rank 1
38.3	129	20	Fracture Rank 1
38.3	92	76	Fracture Rank 1
42.1	103	72	Foliation / Vein
42.4	97	75	Fracture Rank 1
42.7	96	80	Fracture Rank 1

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ECR-206A - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ECR-206A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
24.2	153	66	Fracture Rank 3
24.6	136	36	Fracture Rank 2
25.2	126	71	Fracture Rank 3
25.4	93	35	Fracture Rank 1
25.6	140	32	Fracture Rank 2
25.6	75	71	Fracture Rank 1
26.3	154	78	Fracture Rank 1
26.8	109	76	Fracture Rank 3
27.3	127	77	Foliation / Vein
27.7	128	66	Fracture Rank 1
27.7	341	71	Fracture Rank 1
28.0	117	69	Fracture Rank 4
28.0	120	27	Fracture Rank 3
28.2	292	71	Fracture Rank 2
28.3	307	73	Fracture Rank 2
28.4	140	73	Fracture Rank 2
28.4	301	42	Fracture Rank 3
28.7	281	34	Fracture Rank 2
28.9	230	14	Fracture Rank 2
29.2	272	37	Fracture Rank 4
29.4	264	46	Fracture Rank 3
29.5	315	30	Fracture Rank 2
29.6	17	40	Fracture Rank 3
29.6	117	72	Fracture Rank 3
29.6	289	38	Foliation / Vein
29.9	10	40	Fracture Rank 2
29.9	127	62	Fracture Rank 1
30.4	351	76	Fracture Rank 1
30.7	290	65	Fracture Rank 1
31.0	298	69	Fracture Rank 3
31.1	306	60	Fracture Rank 3
31.5	331	53	Fracture Rank 1
31.5	309	74	Fracture Rank 4
31.5	122	70	Foliation / Vein
31.7	72	68	Fracture Rank 1

BB-ECR-206A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
31.8	343	41	Fracture Rank 1
32.1	122	73	Foliation / Vein
32.2	301	77	Fracture Rank 1
32.6	112	72	Fracture Rank 1
32.8	113	73	Fracture Rank 1
33.0	113	73	Fracture Rank 1
34.7	107	73	Fracture Rank 1
34.9	109	74	Foliation / Vein
35.1	113	75	Foliation / Vein
35.3	114	76	Foliation / Vein
35.7	112	76	Foliation / Vein
36.4	114	80	Fracture Rank 1
38.1	117	83	Foliation / Vein
39.6	120	78	Foliation / Vein
40.4	184	34	Fracture Rank 1
41.0	124	68	Foliation / Vein
42.0	118	73	Fracture Rank 1
42.1	113	73	Foliation / Vein
43.5	109	76	Foliation / Vein
43.6	107	77	Fracture Rank 1
44.8	108	78	Foliation / Vein
45.2	107	78	Foliation / Vein
47.0	95	33	Fracture Rank 1
47.5	108	81	Foliation / Vein
47.7	88	21	Fracture Rank 1
47.8	97	64	Fracture Rank 1
47.8	333	59	Fracture Rank 2

HAGER-RICHTER GEOSCIENCE, INC.	
BB-ELER-206A - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

BB-ELER-206A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
9.4	26	52	Fracture Rank 3
9.7	358	13	Fracture Rank 2
10.1	86	48	Foliation / Vein
10.3	94	46	Fracture Rank 1
10.5	98	50	Fracture Rank 1
10.8	95	47	Foliation / Vein
11.4	101	44	Foliation / Vein
11.6	104	44	Fracture Rank 2
11.9	99	43	Fracture Rank 1
12.1	268	76	Fracture Rank 1
12.3	118	47	Fracture Rank 1
12.7	121	52	Fracture Rank 1
12.9	115	49	Foliation / Vein
13.2	113	51	Fracture Rank 3
13.3	108	52	Foliation / Vein
13.8	113	50	Fracture Rank 2
14.1	122	56	Foliation / Vein
14.8	99	50	Foliation / Vein
15.0	113	55	Foliation / Vein
15.1	113	54	Fracture Rank 2
15.3	111	54	Fracture Rank 2
15.5	113	59	Fracture Rank 2
15.7	263	37	Fracture Rank 4
15.8	127	60	Fracture Rank 1
16.0	255	39	Fracture Rank 1
16.9	57	63	Fracture Rank 1
17.5	91	42	Fracture Rank 1
17.6	96	37	Fracture Rank 2
18.2	111	57	Fracture Rank 1
18.3	113	54	Foliation / Vein
18.4	116	54	Foliation / Vein
18.7	108	49	Foliation / Vein
18.8	117	52	Fracture Rank 2
18.9	119	51	Fracture Rank 1
19.1	120	50	Foliation / Vein

BB-ELER-206A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
19.3	109	48	Foliation / Vein
19.3	109	48	Fracture Rank 1
19.4	114	47	Fracture Rank 2
19.5	118	45	Fracture Rank 1
19.8	279	68	Fracture Rank 1
19.8	117	46	Fracture Rank 1
19.9	110	50	Fracture Rank 1
20.0	115	45	Fracture Rank 2
20.1	111	41	Fracture Rank 3
20.2	114	45	Fracture Rank 2
20.2	113	47	Fracture Rank 3
20.3	120	50	Fracture Rank 2
20.5	116	50	Fracture Rank 1
20.6	120	51	Fracture Rank 1
20.7	121	51	Fracture Rank 1
20.9	121	48	Foliation / Vein
21.1	119	53	Foliation / Vein
21.9	119	51	Fracture Rank 1
22.0	127	51	Fracture Rank 1
22.2	111	45	Foliation / Vein
22.3	107	47	Foliation / Vein
22.5	107	48	Fracture Rank 1
22.6	118	44	Foliation / Vein
22.8	113	48	Fracture Rank 1
23.0	129	59	Fracture Rank 1
23.2	123	60	Foliation / Vein
23.2	331	64	Fracture Rank 1
23.8	127	65	Fracture Rank 1
24.0	116	59	Foliation / Vein
24.1	65	37	Fracture Rank 1
24.6	32	77	Fracture Rank 1
24.7	112	72	Fracture Rank 1
25.0	130	77	Fracture Rank 1
25.6	296	34	Fracture Rank 1
25.9	134	71	Fracture Rank 1
26.2	158	46	Foliation / Vein
26.4	261	42	Fracture Rank 2
26.9	124	67	Fracture Rank 1
27.2	121	66	Fracture Rank 4
27.3	118	63	Fracture Rank 3
27.5	126	61	Fracture Rank 1
27.7	131	67	Fracture Rank 1
28.1	136	59	Fracture Rank 1
28.2	129	61	Foliation / Vein
28.5	120	70	Fracture Rank 2

BB-ELER-206A - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
28.7	128	74	Fracture Rank 1
29.6	297	28	Fracture Rank 1
29.6	122	74	Foliation / Vein
29.9	122	71	Foliation / Vein
30.0	123	73	Fracture Rank 2
31.9	205	40	Fracture Rank 2
32.0	114	71	Foliation / Vein
32.2	119	70	Foliation / Vein
32.3	122	70	Fracture Rank 1
32.4	117	71	Fracture Rank 1
32.5	119	71	Fracture Rank 1
32.7	119	71	Foliation / Vein
32.9	120	72	Fracture Rank 2
33.1	117	70	Fracture Rank 1
33.2	119	71	Fracture Rank 1
33.4	120	72	Fracture Rank 1
33.6	122	70	Fracture Rank 1
33.7	122	70	Fracture Rank 1
33.8	121	70	Foliation / Vein
34.0	126	70	Fracture Rank 1
34.1	122	69	Fracture Rank 2
34.1	123	69	Foliation / Vein
35.2	124	63	Foliation / Vein

HAGER-RICHTER GEOSCIENCE, INC.	
HB-BE-235 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

HB-BE-235 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
15.1	126	69	Fracture Rank 4
15.3	124	66	Fracture Rank 2
15.3	133	80	Fracture Rank 3
15.5	105	41	Fracture Rank 2
15.8	131	53	Fracture Rank 3
16.0	148	49	Fracture Rank 3
16.2	85	38	Fracture Rank 1
16.2	151	59	Fracture Rank 1
16.4	59	71	Fracture Rank 2
16.4	136	80	Fracture Rank 2
16.4	51	46	Fracture Rank 1
16.9	356	17	Fracture Rank 2
17.2	142	68	Fracture Rank 2
17.2	336	32	Fracture Rank 4
17.4	339	26	Fracture Rank 3
17.5	359	15	Fracture Rank 2
17.8	146	78	Fracture Rank 1
17.9	142	78	Fracture Rank 3
18.1	347	34	Fracture Rank 1
18.2	305	17	Fracture Rank 2
18.5	138	78	Fracture Rank 1
18.8	357	17	Fracture Rank 1
19.0	136	77	Fracture Rank 1
19.2	160	54	Fracture Rank 4
19.4	141	50	Fracture Rank 3
19.6	130	38	Fracture Rank 3
19.8	131	53	Fracture Rank 2
19.9	143	53	Fracture Rank 3
20.0	130	73	Fracture Rank 2
20.1	114	70	Fracture Rank 2
20.5	118	66	Fracture Rank 1
20.7	132	66	Fracture Rank 1
20.8	133	69	Fracture Rank 1
20.8	327	20	Fracture Rank 1
20.9	137	68	Fracture Rank 1

HB-BE-235 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
21.0	144	66	Fracture Rank 2
21.2	151	67	Fracture Rank 3
21.3	139	68	Fracture Rank 1
21.4	122	64	Fracture Rank 1
21.5	116	64	Fracture Rank 1
21.7	120	64	Fracture Rank 1
21.8	138	65	Foliation / Vein
22.1	129	59	Foliation / Vein
22.3	123	62	Fracture Rank 1
22.5	115	60	Fracture Rank 1
22.8	129	38	Fracture Rank 1
23.3	106	52	Foliation / Vein
23.4	340	71	Fracture Rank 1
23.7	84	62	Fracture Rank 1
23.9	109	54	Foliation / Vein
24.0	103	60	Fracture Rank 2
24.9	122	80	Fracture Rank 2
25.4	320	54	Fracture Rank 1
25.9	302	58	Fracture Rank 2
25.9	257	32	Fracture Rank 1
26.0	321	48	Fracture Rank 3
26.1	324	41	Fracture Rank 3
26.2	327	43	Fracture Rank 3
26.2	152	31	Fracture Rank 3
26.4	268	23	Fracture Rank 2
26.4	317	51	Fracture Rank 3
26.6	334	71	Fracture Rank 3
26.6	140	39	Fracture Rank 1
26.8	226	54	Fracture Rank 3
26.9	270	32	Fracture Rank 3
27.0	293	22	Fracture Rank 3
27.1	187	67	Fracture Rank 1
27.5	132	61	Fracture Rank 1
27.8	153	46	Fracture Rank 2

HAGER-RICHTER GEOSCIENCE, INC.	
HB-BE-237 - TABLE OF BEDROCK STRUCTURES	
CLIENT	Haley & Aldrich, Inc.
PROJECT	Brewer-Eddington I-395/Route 9 Connector
LOCATION	Eddington, Maine
HRGS FILE	20RG77
DATE LOGGED	March 3, 2021
LOG DATUM	Ground Surface
DIP AZIMUTH	True North (Magnetic Declination = 15.8° West)
DIP ANGLE	Measured from Horizontal

HB-BE-237 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
9.0	333	25	Fracture Rank 3
9.6	113	74	Fracture Rank 3
10.2	71	72	Fracture Rank 3
11.2	103	79	Fracture Rank 3
11.5	163	72	Fracture Rank 3
11.7	356	34	Fracture Rank 3
11.8	158	63	Fracture Rank 4
12.4	258	18	Fracture Rank 4
12.6	249	18	Fracture Rank 3
13.2	96	70	Fracture Rank 2
13.9	164	28	Fracture Rank 4
14.0	114	75	Fracture Rank 1
14.6	206	26	Fracture Rank 3
14.6	135	72	Fracture Rank 1
15.1	330	48	Fracture Rank 2
15.7	143	51	Fracture Rank 4
15.8	331	54	Fracture Rank 3
16.2	330	44	Fracture Rank 4
16.4	320	43	Fracture Rank 4
16.7	350	37	Fracture Rank 4
16.9	353	29	Fracture Rank 4
17.1	352	33	Fracture Rank 4
17.6	343	29	Fracture Rank 2
17.8	94	52	Foliation / Vein
18.6	125	50	Fracture Rank 2
19.6	95	57	Foliation / Vein
19.9	106	53	Foliation / Vein
20.1	299	32	Fracture Rank 1
20.8	109	58	Fracture Rank 1
20.9	286	29	Fracture Rank 1
21.2	120	47	Foliation / Vein
21.8	339	39	Fracture Rank 1
21.9	105	55	Foliation / Vein
22.4	311	68	Fracture Rank 1
22.5	112	57	Foliation / Vein

HB-BE-237 - TABLE OF BEDROCK STRUCTURES

Depth (Feet)	Dip Azimuth (Degrees)	Dip Angle (Degrees)	Bedrock Structure Category
22.6	110	62	Fracture Rank 1
23.4	93	51	Fracture Rank 1
23.7	91	48	Foliation / Vein
23.9	94	51	Fracture Rank 2
24.3	81	46	Fracture Rank 2
24.5	88	47	Foliation / Vein
24.8	93	42	Foliation / Vein
24.9	90	42	Foliation / Vein
24.9	298	30	Fracture Rank 1
25.0	139	65	Fracture Rank 1
25.1	142	83	Fracture Rank 3
25.1	315	15	Fracture Rank 2
25.1	105	36	Foliation / Vein
25.4	78	34	Foliation / Vein
25.5	112	24	Fracture Rank 2
26.1	99	43	Foliation / Vein

Appendix F

Shear Wave Velocity Measurement Report

**SHEAR WAVE VELOCITY TESTING
ROUTE 9/I-395 CONNECTOR
BREWER AND EDDINGTON, MAINE**

Prepared for:

Haley & Aldrich, Inc.
75 Washington Avenue, Suite 1A
Portland, Maine 04101

Prepared by:

Hager-Richter Geoscience, Inc.
8 Industrial Way - D10
Salem, New Hampshire 03079

File 20RG77
January 2021

HAGER-RICHTER GEOSCIENCE, INC.

GEOPHYSICS FOR THE ENGINEERING COMMUNITY
SALEM, NEW HAMPSHIRE
Tel: 603.893.9944
FORDS, NEW JERSEY
Tel: 732.661.0555

January 14, 2021
File 20RG77

Erin A. Force, P.E.
Senior Project Manager
Haley & Aldrich, Inc.
75 Washington Avenue, Suite 1A
Portland, Maine 04101

Main: (207) 482.4626
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RE: Shear Wave Velocity Testing
Route 9/I-395 Connector
Brewer and Eddington, Maine

Dear Ms. Force:

In this report, we summarize the results of shear wave velocity testing conducted by Hager-Richter Geoscience, Inc. (HRGS) in December 2020 at the above referenced site located in Brewer, Maine for Haley & Aldrich, Inc. (H&A). The scope of the survey and the area of interest were specified by H&A.

INTRODUCTION

The site is the proposed alignment for a connector highway between I-395 and Route 9 in Brewer and Eddington, Maine. The general location of the Site is shown in Figure 1. As part of a geotechnical investigation for the project, H&A requires site-specific shear wave velocity information as a function of depth for the soil and bedrock to a depth of 100 feet for determining seismic site class. Figure 2 is an aerial photo showing the locations of the five (5) shear wave velocity test lines and center points, along with the approximate location of some borings installed by H&A.

H&A provided boring logs for seven borings installed near the locations of the test lines at the site. The boring logs report marine clays and silts over till and bedrock in the area of interest at the Site. Bedrock is described as siltstone and was encountered as shallow as 22 feet below ground surface at the Site.

OBJECTIVE

The objective of the geophysical survey was to provide shear wave velocity information as a function of depth to a depth of 100 feet at five locations at the Site.

THE SURVEY

Jeffrey Reid, P.G., and Sean Reid of HRGS conducted the field operations on December 30, 2020. The project was coordinated with Ms. Erin Force, P.E. of H&A. Ms. Force was on-site for the geophysical testing and specified the locations for the testing. Data analysis and interpretation were completed at the HRGS offices. Original data and field notes will be retained in the HRGS files for a minimum of three years.

Shear wave velocity testing was conducted using the passive shear wave seismic (pVs) method, also called the Refraction Microtremor (ReMi™) method along five (5) test lines. The locations of the test lines and center points are shown in Figure 2.

METHODS AND EQUIPMENT

As indicated above, the passive shear wave seismic (pVs) method, also called the Refraction Microtremor method, or ReMi™ was used to determine the shear wave velocity as a function of depth.

The passive shear wave seismic (pVs) method is a geophysical method to determine a vertical shear-wave velocity profile at a single location by analyzing a particular type of seismic wave recorded on a multichannel record. The name pVs is derived from p for passive and Vs for velocity of shear waves. The pVs method, also called the Refraction Microtremor method, or ReMi™, uses Rayleigh waves, a particular kind of wave first described by Lord Rayleigh in 1885. Such waves are dispersive (meaning that the velocity is a function of the wavelength), and the amplitude of such waves decreases with depth. The Rayleigh wave velocity depends primarily on the shear wave velocities and layering of the subsurface material.

Rayleigh waves are a significant part of the ambient subsurface noise at most, if not all, sites. There are many sources of such noise, including, but not limited to, wind, pedestrian and vehicular traffic, surface and subway trains, and construction activities. Although such noise can be troublesome for most seismic methods, it is the source of signals for the pVs method, and the higher the noise level, the better the results for this method.

Low frequency (4.5 Hz) geophones are installed 4 ft apart along a straight line and connected to a seismograph. The ambient noise is recorded for 30 seconds two or three times and examined to be sure that noise of sufficiently low frequency is present. If the noise is sufficient, then 15 to 20 such records are acquired. If the noise spectra do not reach sufficiently low frequencies, then one walks or runs along the test line during data acquisition to add low frequency noise to the ambient noise. The surface waves used in the pVs method, considered noise in seismic refraction and reflection surveys, are enhanced during data acquisition and processing for the pVs method. The seismic data are analyzed using SeisOpt® ReMi™, a commercially licensed software package developed by Optim, Inc. located at the University of Nevada at Reno. Results are

normally presented as 1-D plots or in tabular form showing shear wave velocity as a function of depth at the center of the seismic line.

It should be noted that the method produces a single velocity profile (V_s as a function of depth Z) at one location (namely, the center of the line) for each line. The software also calculates the average shear wave velocity using the following equation (taken from the International Building Code):

$$V_{avg} = \left(\sum_{i=1}^N d_i \right) / \sum_{i=1}^N d_i / V_i \quad \text{Eq. 1}$$

where V_{avg} is average shear wave velocity
 d_i is thickness of the i^{th} layer
 V_i is the shear wave velocity of the i^{th} layer
 N is the number of layers

The Seismic Site Class, based solely on average shear wave velocity, is defined by the IBC as follows:

Site Class	Soil Profile Name	Soil Shear Wave Velocity (ft/s)
A	Hard rock	$V_s > 5000$
B	Rock	$2500 < V_s \leq 5000$
C	Very dense soil and soft rock	$1200 < V_s \leq 2500$
D	Stiff soil profile	$600 \leq V_s \leq 1200$
E	Soft soil profile	$V_s < 600$

Although the IBC provides other methods to determine the Site Class, such as standard penetration resistance (N-values) and soil undrained shear strength, this report provides site specific data for shear wave velocity only. Furthermore, there is no consideration of other factors that may affect a site such as liquefaction. **The final determination of seismic site class should be made by the project engineer.**

Site Specific. We used two 24-channel digital seismographs (Geometrics Geode) coupled to 48 geophones to acquire the pVs data. We used 4.5-Hz frequency vertical geophones for the subject shear wave velocity testing.

The locations of the five (5) test lines are shown in Figure 2. The pVs data were acquired using 48 geophones and a geophone spacing of 4 feet for all pVs test lines. The pVs method yields a single vertical velocity profile at the mid points of the test lines, shown in Figure 2. The seismic source for the pVs test was ambient noise and random hammer striking while acquiring the data to enhance the high frequency content of the seismic signal.

LIMITATIONS OF THE METHOD

pVs. As with all physical measurements, there is experimental error in the velocities that are determined using the passive shear wave velocity seismic method. For the *pVs* method, the accuracy of V_{avg} is stated by Optim, Inc. to be 5-15%.

The depth of investigation is a function of the noise spectrum, and long wavelengths (low frequencies) are required to determine velocity at large depths. Noise levels can be improved by a person running along the seismic spread during data acquisition.

RESULTS

The geophysical survey consisted of passive shear wave seismic (*pVs*) testing. The surface shear wave velocity testing was conducted along five (5) test lines, designated as *pVs* Lines 1-5. The *pVs* test line locations and center points for the velocity profiles are shown in Figure 2, along with the approximate locations of borings installed by H&A.

H&A provided boring logs for seven borings installed near the locations of the test lines at the site. The boring logs report marine clays and silts over till and bedrock in the area of interest at the Site. Bedrock is described as siltstone and was encountered as shallow as 22 feet to more than 37 feet below ground surface at the Site.

The results of the *pVs* testing are reported in Table 1. For modeling purposes, the subsurface stratigraphy was broken into three or four discrete units. The velocity units do not necessarily correlate with specific lithologic units identified in the borings. We note that boring logs do not provide a complete stratigraphic constraint, therefore, the number of layers and the thickness that provides the best statistical fit to the respective dispersion curve was used for each line independently. The root mean square error for the fit of the dispersion curve versus the measured data using the model velocities is provided in Table 1 and ranges from 1.2% to 4.2%.

No attempt was made to “force” a specific model to the data. The velocities for the units to the maximum depth investigated, and the average values of the velocity of shear waves, V_{s100} , determined by Equation 1 for the depth interval of 0 – 100 ft are also reported in Table 1 for the *pVs* test lines.

LIMITATIONS ON USE OF THIS REPORT

This letter report was prepared for the exclusive use Haley & Aldrich (Client). No other party shall be entitled to rely on this Report, or any information, documents, records, data, interpretations, advice or opinions given to Client by Hager-Richter Geoscience, Inc. (HRGS) in the performance of its work. The Report relates solely to the specific project for which HRGS has been retained and shall not be used or relied upon by Client or any third party for any variation or extension of this project, any other project or any other purpose without the express

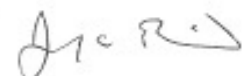
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HRGS has used reasonable care, skill, competence and judgment in the performance of its services for this project consistent with professional standards for those providing similar services at the same time, in the same locale, and under like circumstances. Unless otherwise stated, the work performed by HRGS should be understood to be exploratory and interpretational in character and any results, findings or recommendations contained in this Report or resulting from the work proposed may include decisions which are judgmental in nature and not necessarily based solely on pure science or engineering. It should be noted that our conclusions might be modified if subsurface conditions were better delineated with additional subsurface exploration including, but not limited to, test pits, soil borings with collection of soil and water samples, and laboratory testing.

Except as expressly provided in this limitations section, HRGS makes no other representation or warranty of any kind whatsoever, oral or written, expressed or implied; and all implied warranties of merchantability and fitness for a particular purpose, are hereby disclaimed.

If you have any questions or comments regarding this report, please contact us at your convenience. It has been a pleasure to work with Haley & Aldrich on this project. We look forward to working with you again in the future.

Sincerely,
HAGER-RICHTER GEOSCIENCE, INC.



Jeffrey Reid, P.G.
Owner / Principal Geophysicist

Attachments: Table 1, Figures 1 & 2

TABLE 1 - pVs TESTING RESULTS

Geologic Unit*	pVs Test Line 1	
	Depth Interval (ft)	Vs** (ft/s)
Marine Clay	0 – 5.1	383
Marine Clay	5.1 – 30.0	510
Dense Till/Weathered Bedrock	30.0+	2865
V _{S 100} (ft/s)	1155	
RMS Error (%)	3.4%	

Geologic Unit*	pVs Test Line 2	
	Depth Interval (ft)	Vs** (ft/s)
Marine Silt	0 – 12.8	429
Marine Clay	12.8 – 18.7	485
Marine Clay	18.7 – 35.3	621
Dense Till/Weathered Bedrock	35.3+	2341
V _{S 100} (ft/s)	1037	
RMS Error (%)	2.5%	

Geologic Unit*	pVs Test Line 3	
	Depth Interval (ft)	Vs** (ft/s)
Marine Silt	0 – 11.2	475
Marine Clay	11.2 – 19.0	836
Till	19.0 – 27.2	1624
Dense Till/Weathered Bedrock	27.2+	2794
V _{S 100} (ft/s)	1562	
RMS Error (%)	1.4%	

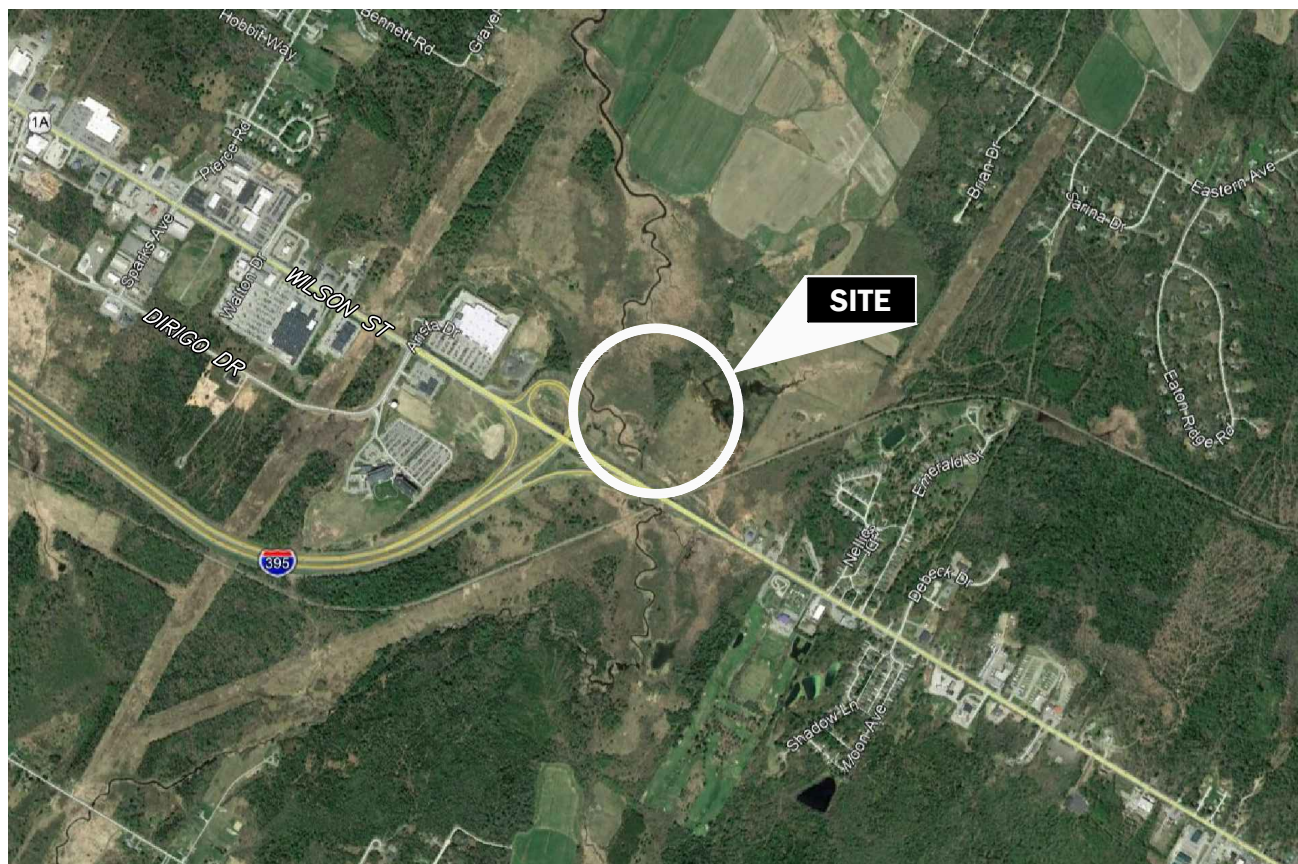
* Stratigraphy is roughly based on Draft Boring Logs provided by H&A
** Shear wave velocity profile is determined for the mid-point of the test line

**TABLE 1 - pVs TESTING RESULTS
(cont.)**

Geologic Unit*	pVs Test Line 4	
	Depth Interval (ft)	V_s** (ft/s)
Marine Silt	0 – 7.6	459
Marine Clay	7.6 – 12.6	610
Till	12.6 – 20.2	993
Dense Till/Weathered Bedrock	20.2+	2943
V _{S 100} (ft/s)	1680	
RMS Error (%)	4.2%	

Geologic Unit*	pVs Test Line 5	
	Depth Interval (ft)	V_s** (ft/s)
Marine Clay	0 – 11.1	501
Till	11.1 – 29.8	1382
Dense Till	29.8 – 33.5	2204
Dense Till/Weathered Bedrock	33.5+	3504
V _{S 100} (ft/s)	1775	
RMS Error (%)	1.2%	

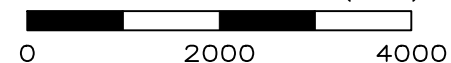
- * Stratigraphy is roughly based on Draft Boring Logs provided by H&A
** Shear wave velocity profile is determined for the mid-point of the test line



N



APPROXIMATE SCALE (feet)



LOCATION

NOTE:

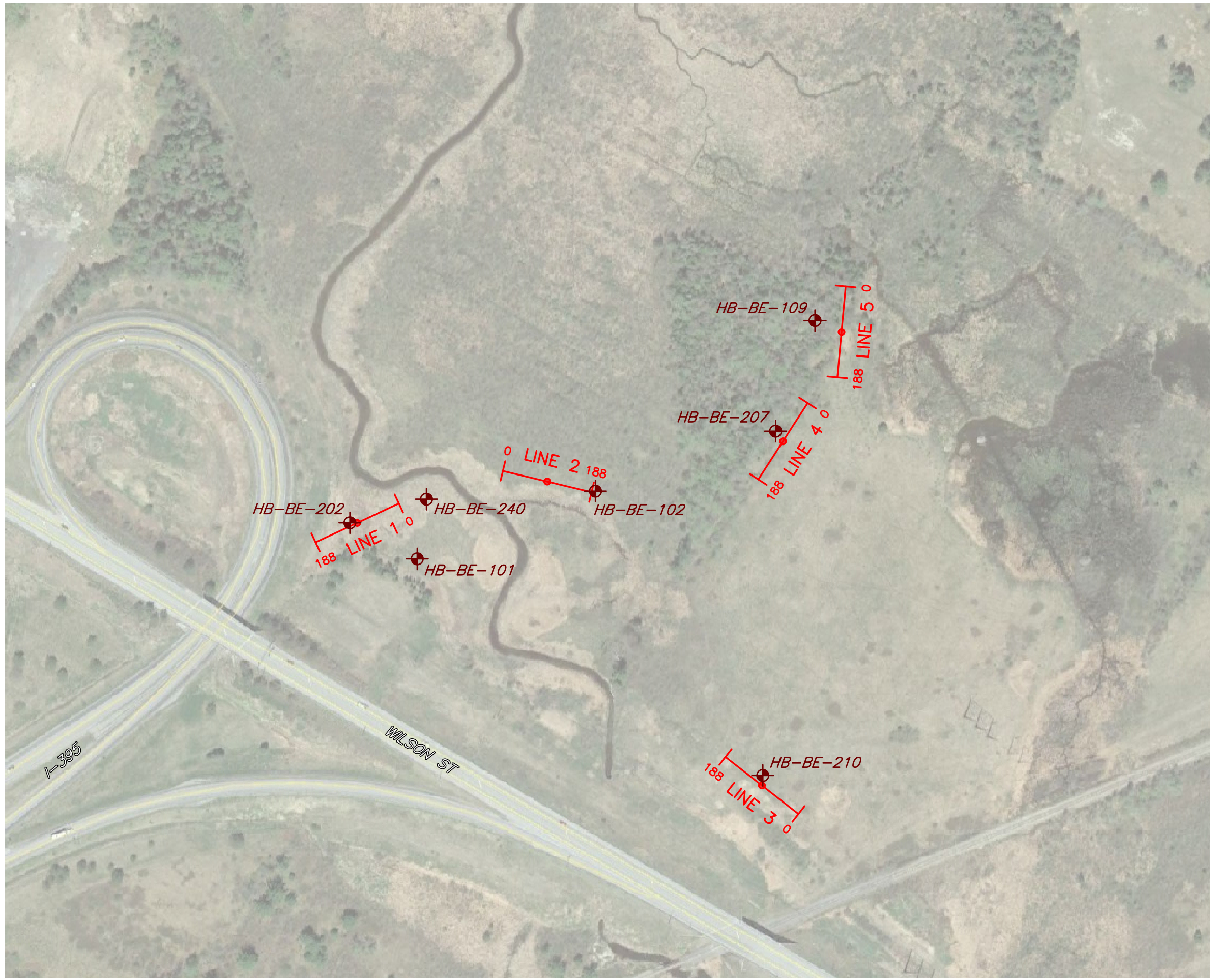
Modified from Google Earth Pro aerial photograph.

Figure 1
General Site Location
Route 9/I-395 Connector
Brewer/Eddington, Maine

File 20RG77

January, 2021

HAGER-RICHTER
Salem, NH | Fords, NJ



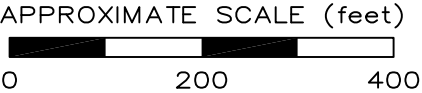
LEGEND



pVs TEST LINE
WITH MIDPOINT



APPROXIMATE
BORING
LOCATION



NOTE:
Modified from Google Earth Pro aerial photograph.

Figure 2
pVs Test Line Locations
Route 9/I-395 Connector
Brewer/Eddington, Maine

File 20RG77 | January, 2021

HAGER-RICHTER
Salem, NH | Fords, NJ

Appendix G

Laboratory Test Results

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME		Project No: GTX-308855
Boring ID: ---	Sample Type: ---	Tested By:	GA
Sample ID: ---	Test Date: 10/03/18	Checked By:	emm
Depth : ---	Test Id: 474421		

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-BEA-101	3D	5-7 ft	Moist, grayish brown silty sand with gravel	7.6
BB-BEA-101	4D	10-11.4 ft	Moist, grayish brown silty sand with gravel	8.5
BB-BEA-101	5D	15-17 ft	Moist, grayish brown silty sand with gravel	11.3
BB-BEA-102	4D	10-12 ft	Moist, dark grayish brown sandy silt	10.9
BB-BEA-102	5D	15-17 ft	Moist, dark grayish brown silty gravel with sand	9.7

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 2)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313196
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	03/22/21
Depth :	---	Test Id:	611809
		Tested By:	md
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-BEB-202	U1	5-7 ft	Moist, gray clay	32.7
BB-BEB-202	U2	15-17 ft	Wet, gray clay	36.5
BB-BEB-204	U1	5-7 ft	Moist, light yellowish brown clay	29.3
BB-BEB-205	U1	10-12 ft	Moist, gray clay	36.8

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	03/23/21
Depth :	---	Test Id:	611463
		Tested By:	md
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-BFB1-202	U1	10-12 ft	Moist, gray clay	35.2
BB-BFB1-202	U2	15-17 ft	Moist, gray clay	38.9
BB-BFB1-204	U1	10-12 ft	Moist, gray clay	35.3
BB-BFB-201	U2	23-25 ft	Moist, gray clay	33.7
BB-BFB-202	U1	18-20 ft	Wet, gray clay	42.0
BB-BFB-202	U2	25-27 ft	Moist, gray clay	36.0
BB-BFB2-201	U1	5-7 ft	Moist, olive gray clay	33.6
BB-BFB2-202	U1	10-12 ft	Moist, gray clay	35.6
BB-BST1-201	U1	15-17 ft	Moist, olive gray	38.0

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID: ---	Sample Type: ---	Tested By:	GA
Sample ID: ---	Test Date: 10/02/18	Checked By:	emm
Depth : ---	Test Id: 474558		

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-BWS-103	3D	5-7 ft	Moist, olive sandy silty clay with gravel	13.1
BB-BWS-103	4D	10-12 ft	Moist, olive silty clay with sand	16.0
BB-BWS-103	5D	15-17 ft	Moist, olive gray silty clay	15.4
BB-BWS-104	4D	6-8 ft	Moist, olive silty clay	17.4
BB-BWS-104	5D	10-12 ft	Moist, olive gray silty clay	15.4
BB-BWS-104	6D	15-17 ft	Moist, olive gray silty clay with sand	12.2
BB-BWS-104	7D	20-22 ft	Moist, dark gray silty clay	14.7

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Clewleyville Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308854
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	10/03/18
Depth :	---	Test Id:	474398
		Tested By:	jbr
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-ECR-101	3D	4.3-6.3 ft	Moist, brown clay with sand	13.3
BB-ECR-101	4D	10-12 ft	Moist, brown sandy clay	12.1
BB-ECR-102	3D	4.3-6.3	Moist, brown sandy clay	13.6

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Levenseller Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308857
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	10/02/18
Depth :	---	Test Id:	474534
		Tested By:	jbr
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
BB-ELER-101	3D	4-6 ft	Moist, yellowish brown sandy clay	12.4
BB-ELER-102	3D	4.3-6.3 ft	Moist, brown sandy silt	13.9
BB-ELER-102	4D	10-12 ft	Moist, brown sandy clay with gravel	10.2

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	10/12/18
Depth :	---	Test Id:	474364
		Tested By:	GA
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-101	1U	5-7 ft	Moist, dark gray clay	35.1
HB-BE-102	1U	10-11.3 ft	Moist, dark olive gray clay	27.6
HB-BE-105	1U	10-12 ft	Moist, olive gray clay	30.3
HB-BE-105	2U	14-16 ft	Moist, dark gray clay	36.7
HB-BE-107A	1U	10-12 ft	Wet, dark gray clay	39.3
HB-BE-115	2D	2-4 ft	Moist, brownish yellow silty sand with gravel	4.9
HB-BE-115	3D	5-7 ft	Moist, olive brown silty sand with gravel	6.7
HB-BE-115	6D	14-16 ft	Moist, olive silty sand with gravel	9.7
HB-BE-115	8D	18-19.6 ft	Moist, olive silty sand	11.4

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	09/28/18
Depth :	---	Test Id:	474375
		Tested By:	GA
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-116	3D	5-7 ft	Moist, olive sandy silt	11.5
HB-BE-116	4D	10-11.8 ft	Moist, olive silty sand with gravel	8.6
HB-BE-116	5D	15-16.4 ft	Moist, olive silty sand with gravel	10.1
HB-BE-116	6D	18-20 ft	Moist, olive silty sand with gravel	9.3
HB-BE-116	8D	22-24 ft	Moist, olive sandy silt	11.9
HB-BE-133	3D	5-7 ft	Moist, olive gray sandy clay	10.1
HB-BE-133	5D	15-17 ft	Moist, olive gray clay with sand	12.1
HB-BE-133	7D	25-27 ft	Moist, dark gray sandy clay	12.3

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	10/12/18
Depth :	---	Test Id:	474378
		Tested By:	GA
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-144	2D	2-4 ft	Moist, light olive brown sandy silt with gravel	7.7
HB-BE-144	3D	5-7 ft	Moist, olive sandy clay	10.0
HB-BE-145	3D	5-6.8 ft	Moist, dark grayish brown silty sand	8.5
HB-BE-148	2D	2-4 ft	Moist, pale olive sandy silt with gravel	10.5
HB-BE-148	5D	12-14 ft	Moist, olive sandy clay	13.7
HB-BE-148	6D	14-15.3 ft	Moist, olive sandy clay	16.0

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	10/17/18
Depth :	---	Test Id:	474317
		Tested By:	md
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-151	2D	2-3.7 ft	Moist, olive sandy clay	13.7
HB-BE-151	3D	4-6 ft	Moist, olive sandy clay	13.4
HB-BE-151	4D	10-10.9 ft	Moist, olive gray sandy clay	12.4
HB-BE-151	5D	15-16.2 ft	Moist, olive sandy clay	11.3
HB-BE-151	6D+7D+8D	17-21.5 ft	Moist, olive brown gravel with clay and sand	7.9
HB-BFB-101	1U	5-7 ft	Moist, very dark gray clay	36.1
HB-BFB-101	2U	12-14 ft	Wet, very dark greenish gray clay	40.4
HB-BFB-101	4U	30-32 ft	Moist, dark gray clay	35.4

Notes: Temperature of Drying : 110° Celsius

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID: ---	Sample Type: ---	Tested By:	md
Sample ID: ---	Test Date: 03/15/21	Checked By:	emm
Depth : ---	Test Id: 611469		

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-201	U1	30-32 ft	Moist, olive gray and very dark gray clay	54.4
HB-BE-201	U2	46-48 ft	Moist, black and light olive silt	36.3
HB-BE-202	U2	18-20 ft	Moist, gray clay	38.3
HB-BE-204	U1	13-15 ft	Moist, gray clay	33.8
HB-BE-205	U1	12-14 ft	Moist, gray clay	36.1
HB-BE-206	U1	12-14 ft	Moist, gray clay	37.7
HB-BE-207	U1	5-7 ft	Moist, olive gray and brownish yellow clay	26.5
HB-BE-208	U1	10-12 ft	Moist, gray and olive yellow clay	32.2
HB-BE-210	U1	15-17 ft	Moist, gray clay	33.3
HB-BE-215	U1	10-12 ft	Moist, gray clay	36.0

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	03/15/21
Depth :	---	Test Id:	611471
		Tested By:	md
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-216	U1	8-10 ft	Moist, olive gray clay	30.6
HB-BE-239	U1	13-15 ft	Moist, gray clay	36.0
HB-BE-239	U2	23-25 ft	Moist, gray clay	37.3
HB-BE-240	U1	10-12 ft	Moist, gray clay	27.4
HB-BE-240	U2	20-22 ft	Moist, gray clay	37.9
HB-BE-242A	U1	9-11 ft	Moist, gray clay	31.2

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 3)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313197
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	04/02/21
Depth :	---	Test Id:	613073
		Tested By:	md
		Checked By:	emm

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BE-223	U1(19.2")	12-14	Wet, dark gray clay	42.0
HB-BE-223A	U1(20.4")	8-10	Moist, gray clay	36.4
HB-BE-224	U1(21.6")	8-10	Moist, dark gray clay	34.2

Notes: Temperature of Drying : 110° Celsius



Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 4)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313198
Boring ID:	---	Sample Type:	---
Sample ID:	---	Test Date:	03/22/21
Depth :	---	Test Id:	611820
		Tested By:	md
		Checked By:	emm

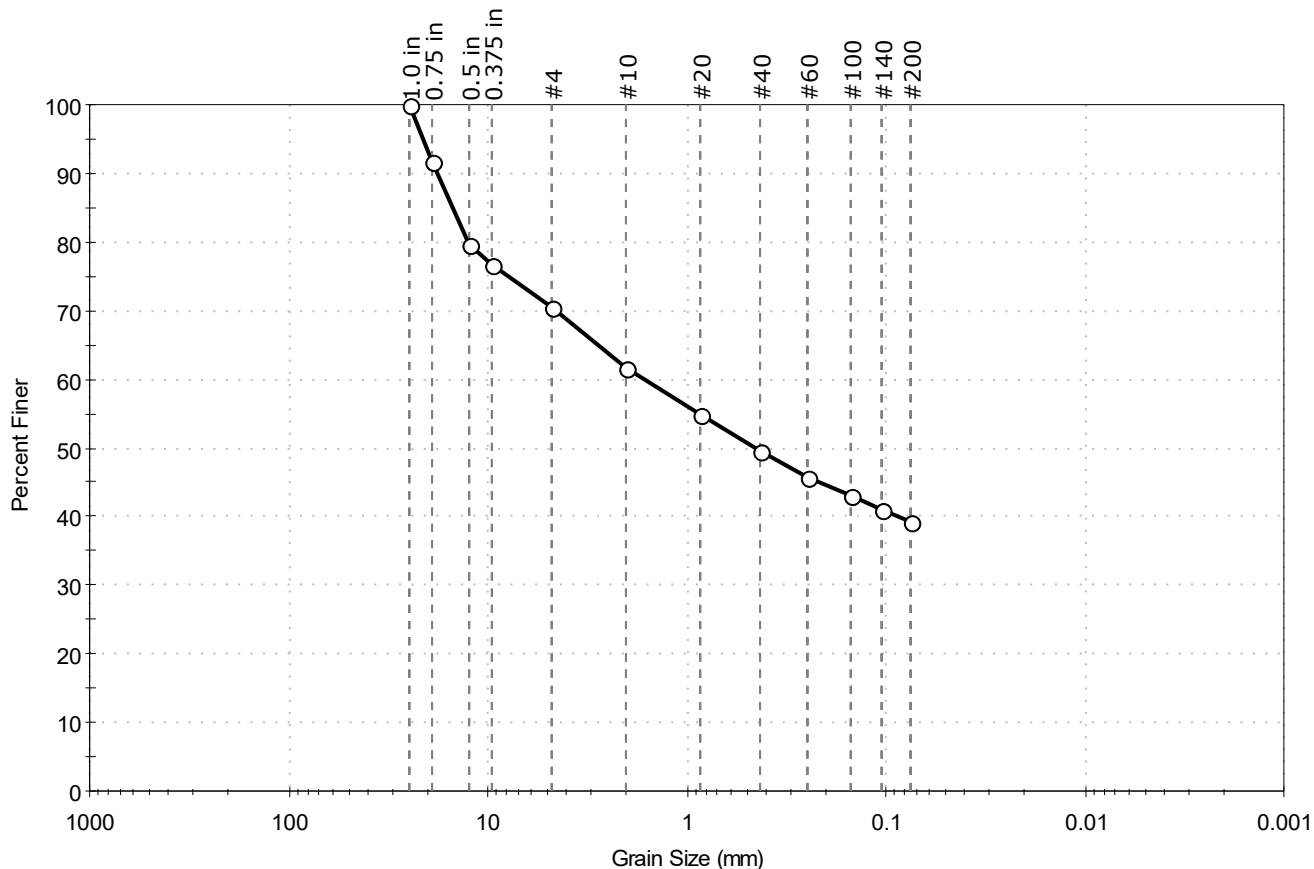
Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
HB-BB-226	U1	6-8 ft	Moist, olive gray clay	30.9
HB-BB-227	U1	5-7 ft	Moist, olive gray clay	31.4
HB-BE-225	U1	8-10 ft	Moist, gray clay	32.3

Notes: Temperature of Drying : 110° Celsius

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308855
Boring ID:	BB-BEA-101	Sample Type:	jar
Sample ID:	3D	Test Date:	10/03/18
Depth :	5-7 ft	Test Id:	474412
Test Comment:	---		
Visual Description:	Moist, grayish brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	29.5	31.3	39.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	92		
0.5 in	12.50	80		
0.375 in	9.50	77		
#4	4.75	70		
#10	2.00	62		
#20	0.85	55		
#40	0.42	50		
#60	0.25	46		
#100	0.15	43		
#140	0.11	41		
#200	0.075	39		

Coefficients

$D_{85} = 15.0398 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 1.6089 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.4451 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

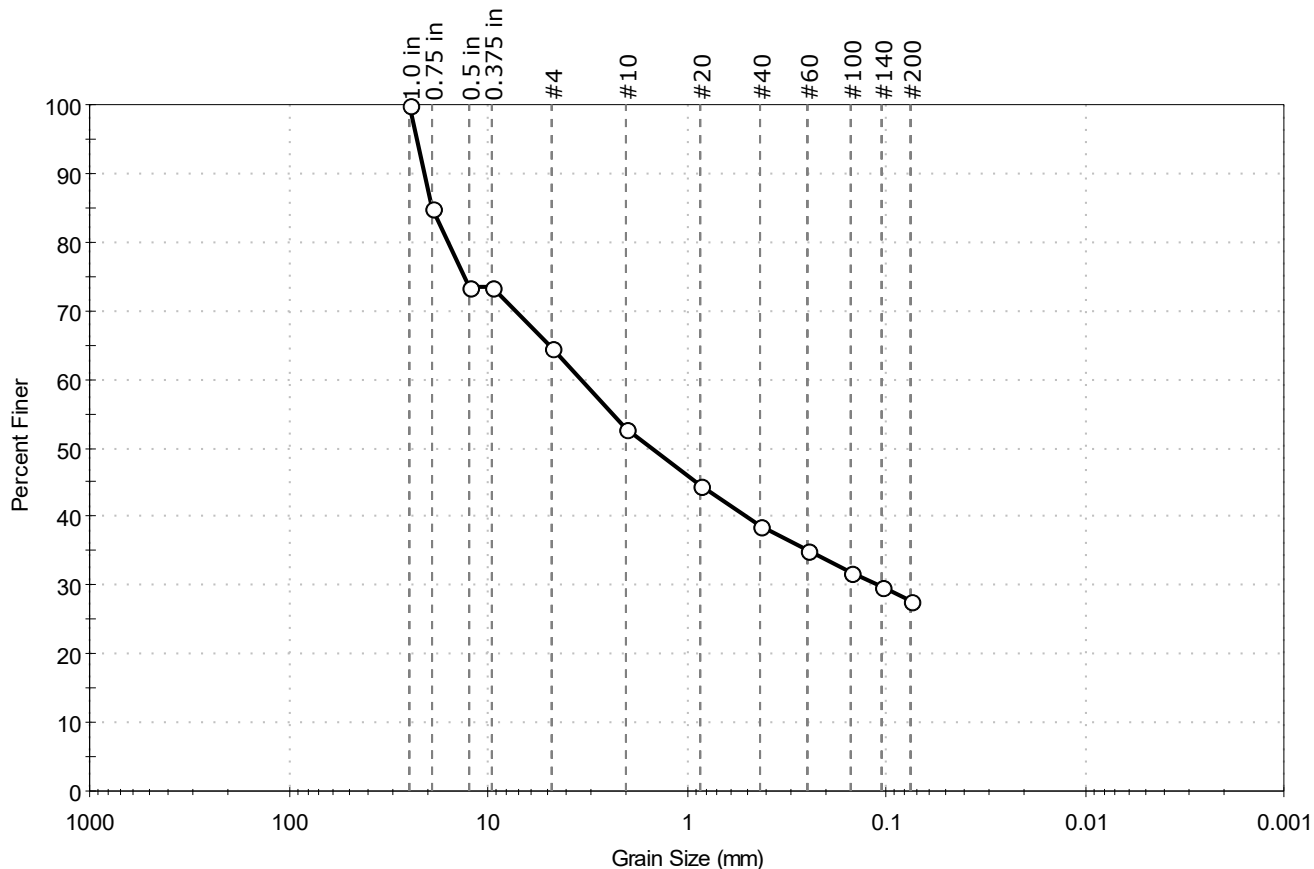
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308855
Boring ID:	BB-BEA-101	Sample Type:	jar
Sample ID:	4D	Test Date:	10/03/18
Depth :	10-11.4 ft	Test Id:	474413
Test Comment:	---		
Visual Description:	Moist, grayish brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	35.4	36.8	27.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	85		
0.5 in	12.50	74		
0.375 in	9.50	74		
#4	4.75	65		
#10	2.00	53		
#20	0.85	44		
#40	0.42	39		
#60	0.25	35		
#100	0.15	32		
#140	0.11	30		
#200	0.075	28		

Coefficients

$D_{85} = 19.0103 \text{ mm}$ $D_{30} = 0.1113 \text{ mm}$
 $D_{60} = 3.3917 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 1.4948 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

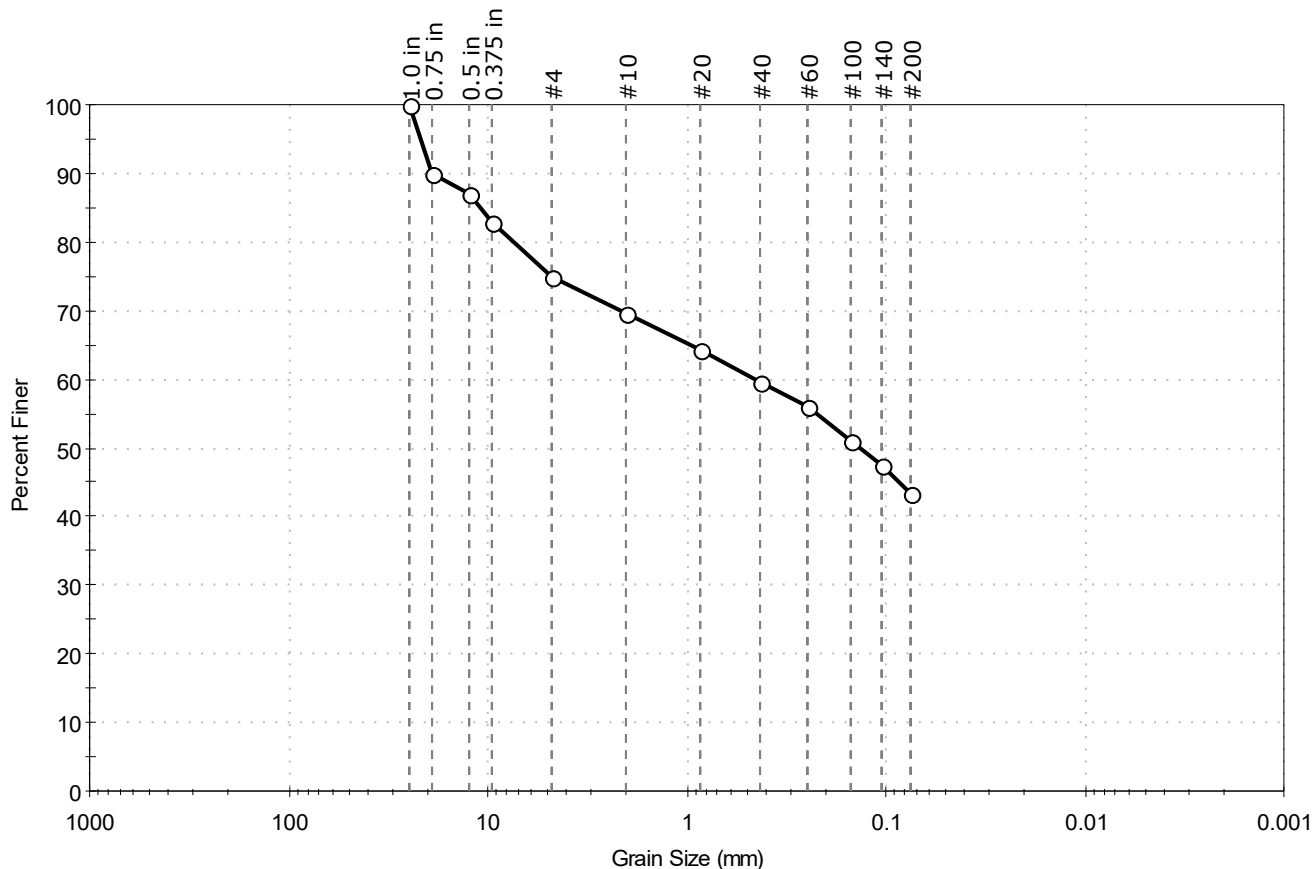
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308855
Boring ID:	BB-BEA-101	Sample Type:	jar
Sample ID:	5D	Test Date:	10/03/18
Depth :	15-17 ft	Test Id:	474414
Test Comment:	---		
Visual Description:	Moist, grayish brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	25.0	31.6	43.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	90		
0.5 in	12.50	87		
0.375 in	9.50	83		
#4	4.75	75		
#10	2.00	70		
#20	0.85	64		
#40	0.42	60		
#60	0.25	56		
#100	0.15	51		
#140	0.11	47		
#200	0.075	43		

Coefficients

$D_{85} = 10.9699$ mm $D_{30} = \text{N/A}$
 $D_{60} = 0.4428$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.1358$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

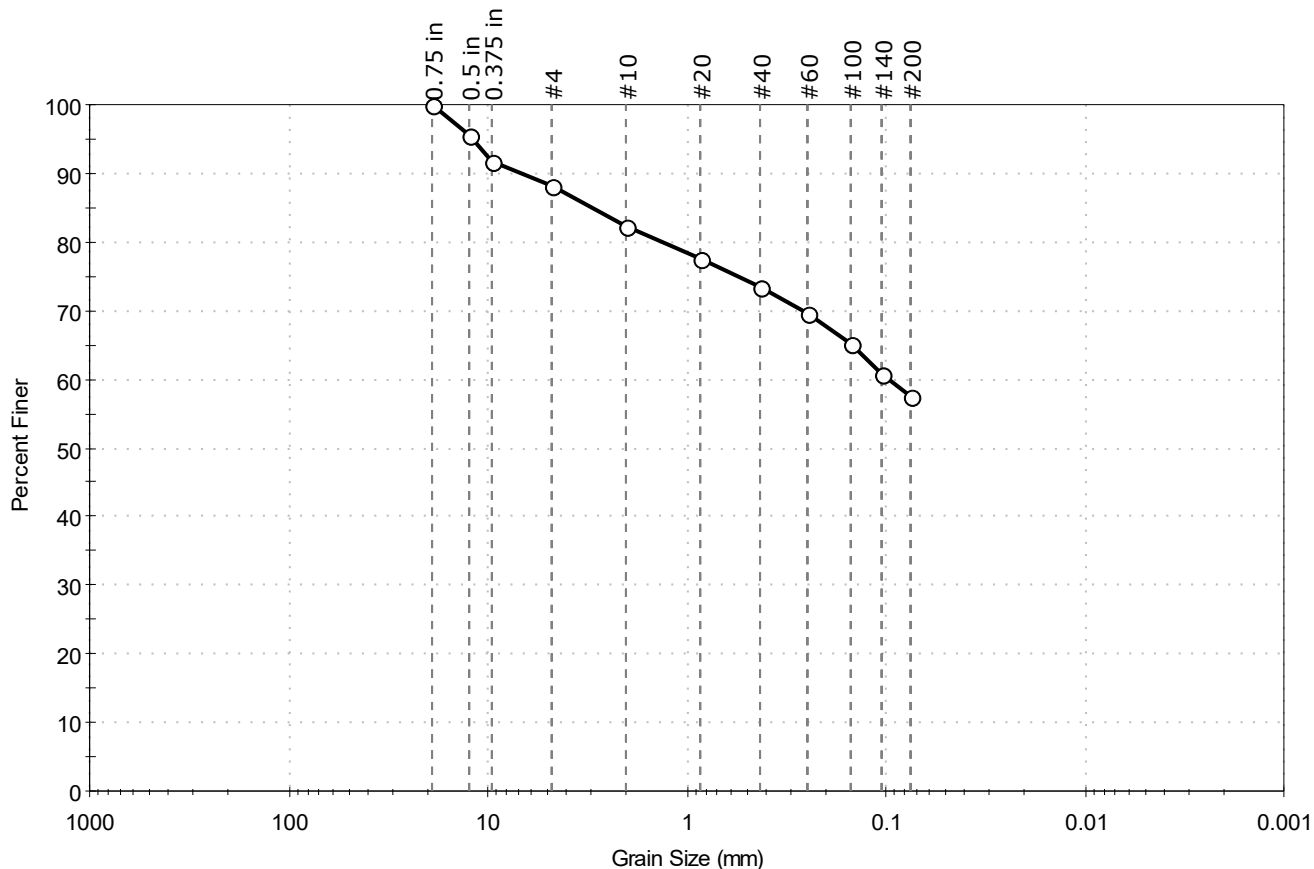
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308855
Boring ID:	BB-BEA-102	Sample Type:	jar
Sample ID:	4D	Test Date:	10/03/18
Depth :	10-12 ft	Test Id:	474415
Test Comment:	---		
Visual Description:	Moist, dark grayish brown sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	11.8	30.6	57.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	96		
0.375 in	9.50	92		
#4	4.75	88		
#10	2.00	82		
#20	0.85	78		
#40	0.42	73		
#60	0.25	70		
#100	0.15	65		
#140	0.11	61		
#200	0.075	58		

Coefficients

D ₈₅ = 2.9612 mm	D ₃₀ = N/A
D ₆₀ = 0.0973 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

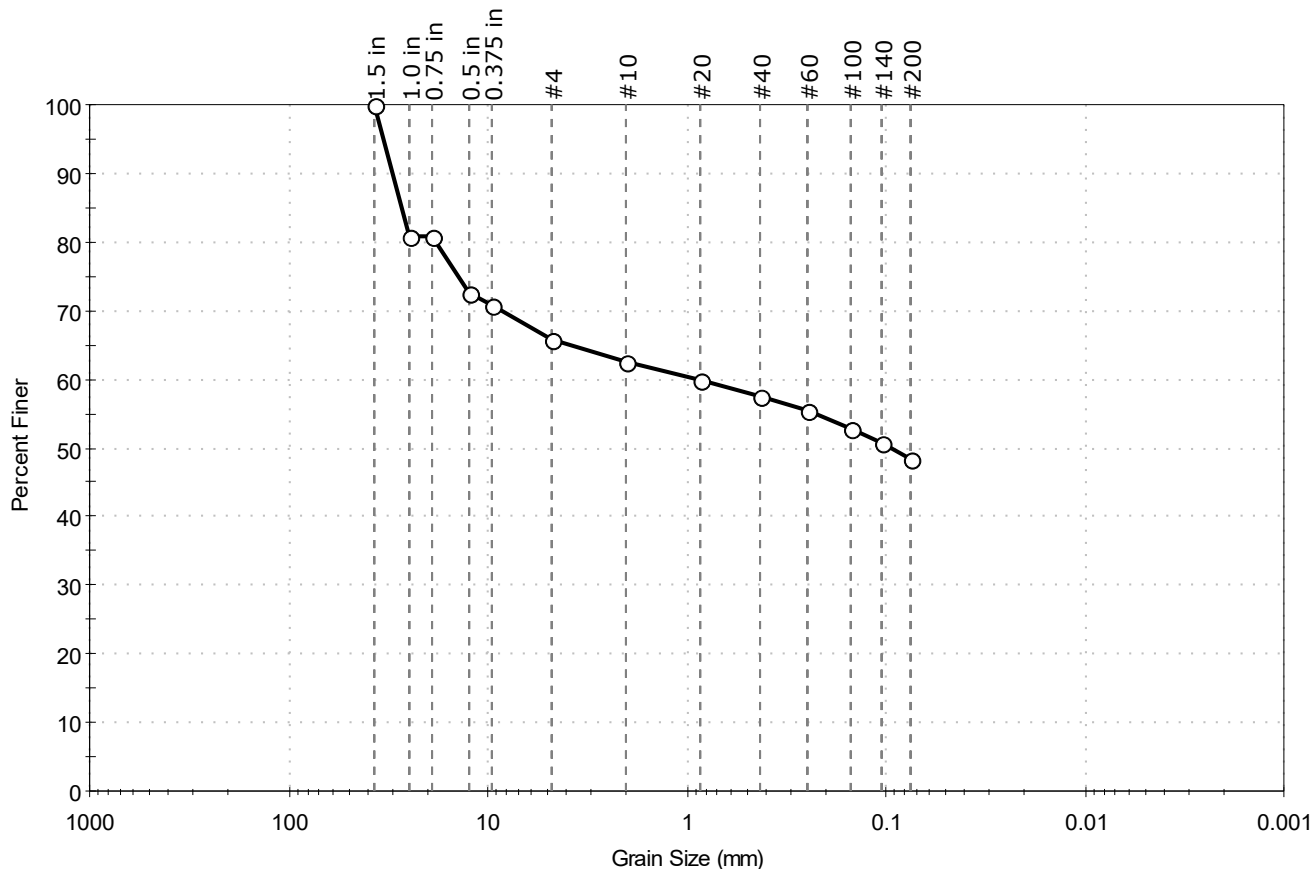
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Eastern Ave Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308855
Boring ID:	BB-BEA-102	Sample Type:	jar
Sample ID:	5D	Test Date:	10/03/18
Depth :	15-17 ft	Test Id:	474416
Test Comment:	---		
Visual Description:	Moist, dark grayish brown silty gravel with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	34.2	17.5	48.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1.0 in	25.00	81		
0.75 in	19.00	81		
0.5 in	12.50	72		
0.375 in	9.50	71		
#4	4.75	66		
#10	2.00	62		
#20	0.85	60		
#40	0.42	57		
#60	0.25	55		
#100	0.15	53		
#140	0.11	51		
#200	0.075	48		

Coefficients

$D_{85} = 27.3552 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.8581 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.0939 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

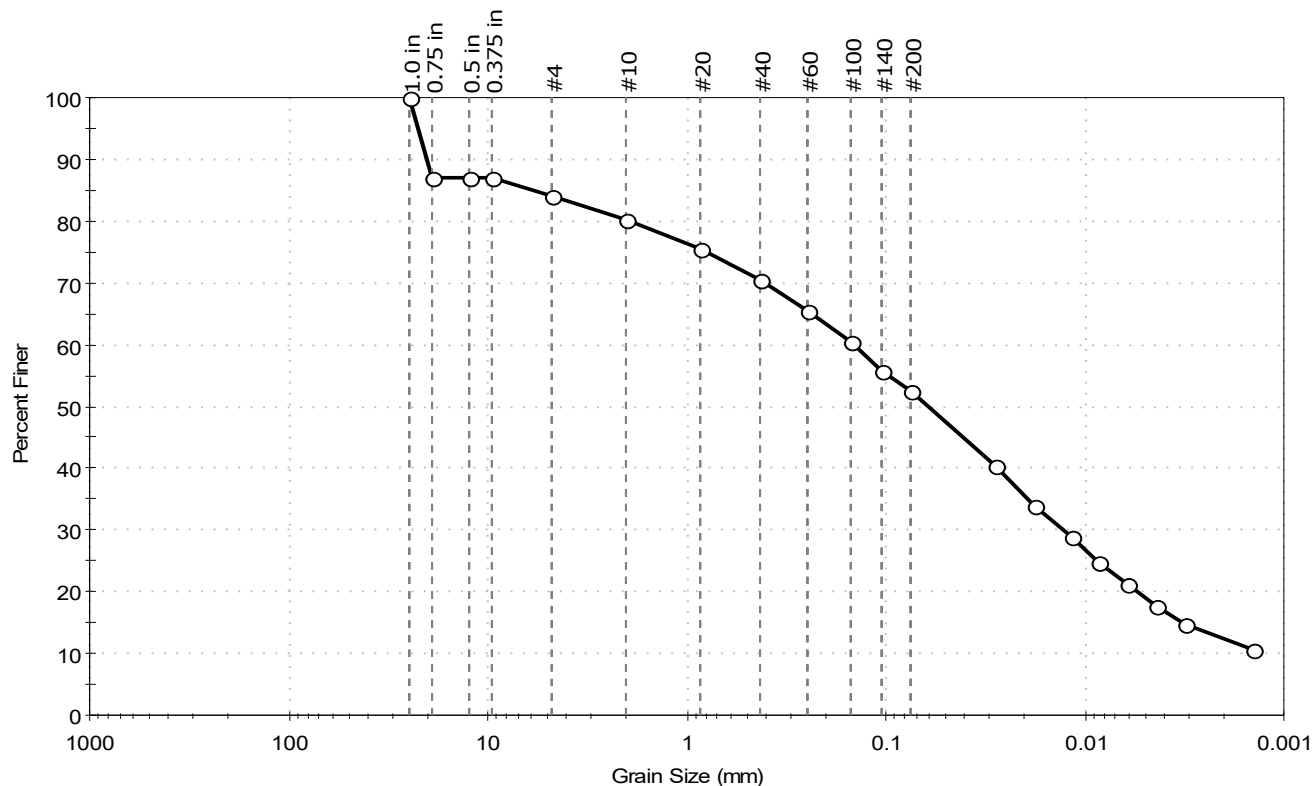
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar
Sample ID:	3D	Test Date:	10/02/18
Depth :	5-7 ft	Test Id:	474545
Test Comment:	---		
Visual Description:	Moist, olive sandy silty clay with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	15.9	31.7	52.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	87		
0.5 in	12.50	87		
0.375 in	9.50	87		
#4	4.75	84		
#10	2.00	80		
#20	0.85	76		
#40	0.42	70		
#60	0.25	65		
#100	0.15	60		
#140	0.11	56		
#200	0.075	52		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0284	40		
---	0.0180	34		
---	0.0117	29		
---	0.0085	25		
---	0.0061	21		
---	0.0044	18		
---	0.0032	15		
---	0.0014	10		

Coefficients

D₈₅ = 5.8569 mm D₃₀ = 0.0127 mm
 D₆₀ = 0.1462 mm D₁₅ = 0.0033 mm
 D₅₀ = 0.0619 mm D₁₀ = N/A
 C_u = N/A C_c = N/A

Classification

ASTM Sandy Silty CLAY with Gravel (CL-ML)

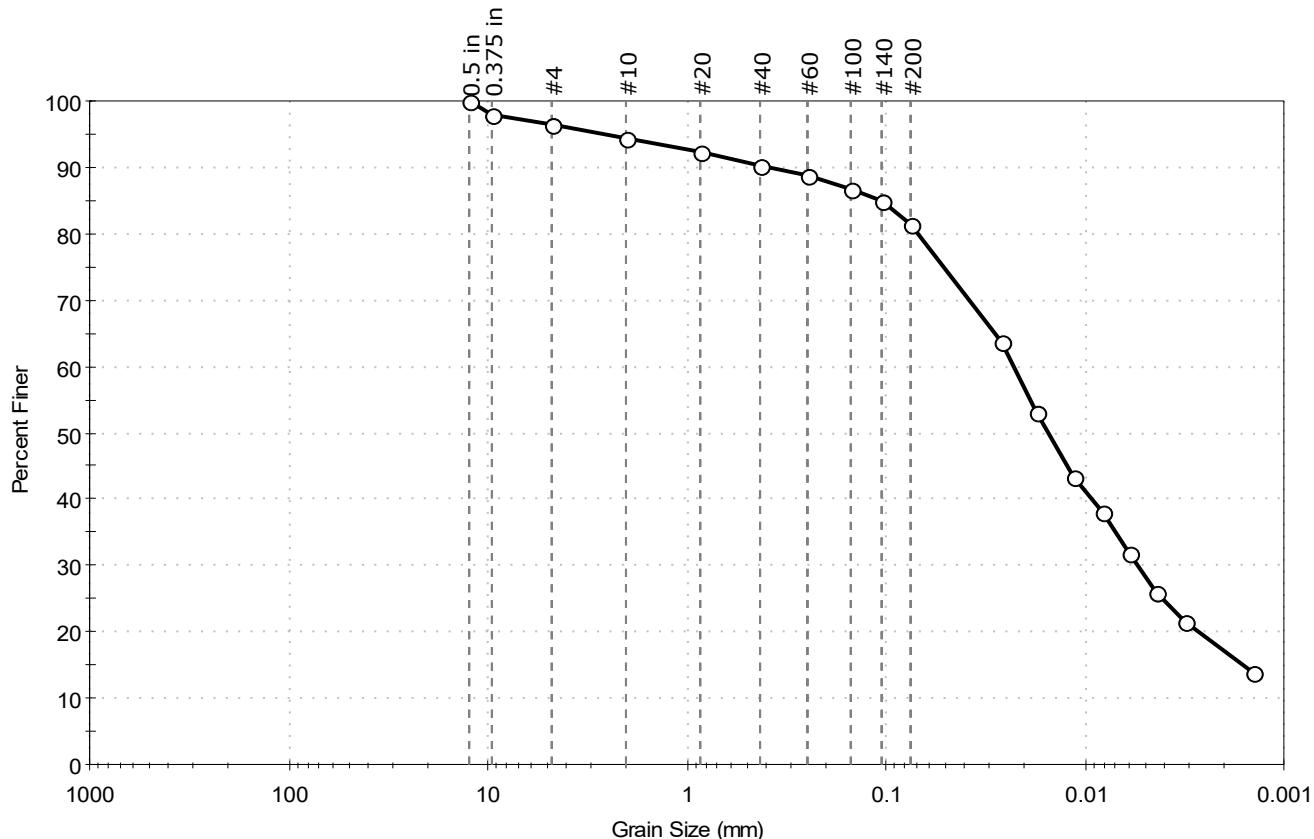
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar
Sample ID:	4D	Test Date:	10/02/18
Depth :	10-12 ft	Test Id:	474546
Test Comment:	---		
Visual Description:	Moist, olive silty clay with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	3.5	15.1	81.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	97		
#10	2.00	94		
#20	0.85	92		
#40	0.42	90		
#60	0.25	89		
#100	0.15	87		
#140	0.11	85		
#200	0.075	81		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0263	64		
---	0.0176	53		
---	0.0114	43		
---	0.0082	38		
---	0.0060	32		
---	0.0044	26		
---	0.0031	21		
---	0.0014	14		

Coefficients

$D_{85} = 0.1068$ mm $D_{30} = 0.0054$ mm
 $D_{60} = 0.0229$ mm $D_{15} = 0.0016$ mm
 $D_{50} = 0.0153$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY with Sand (CL-ML)

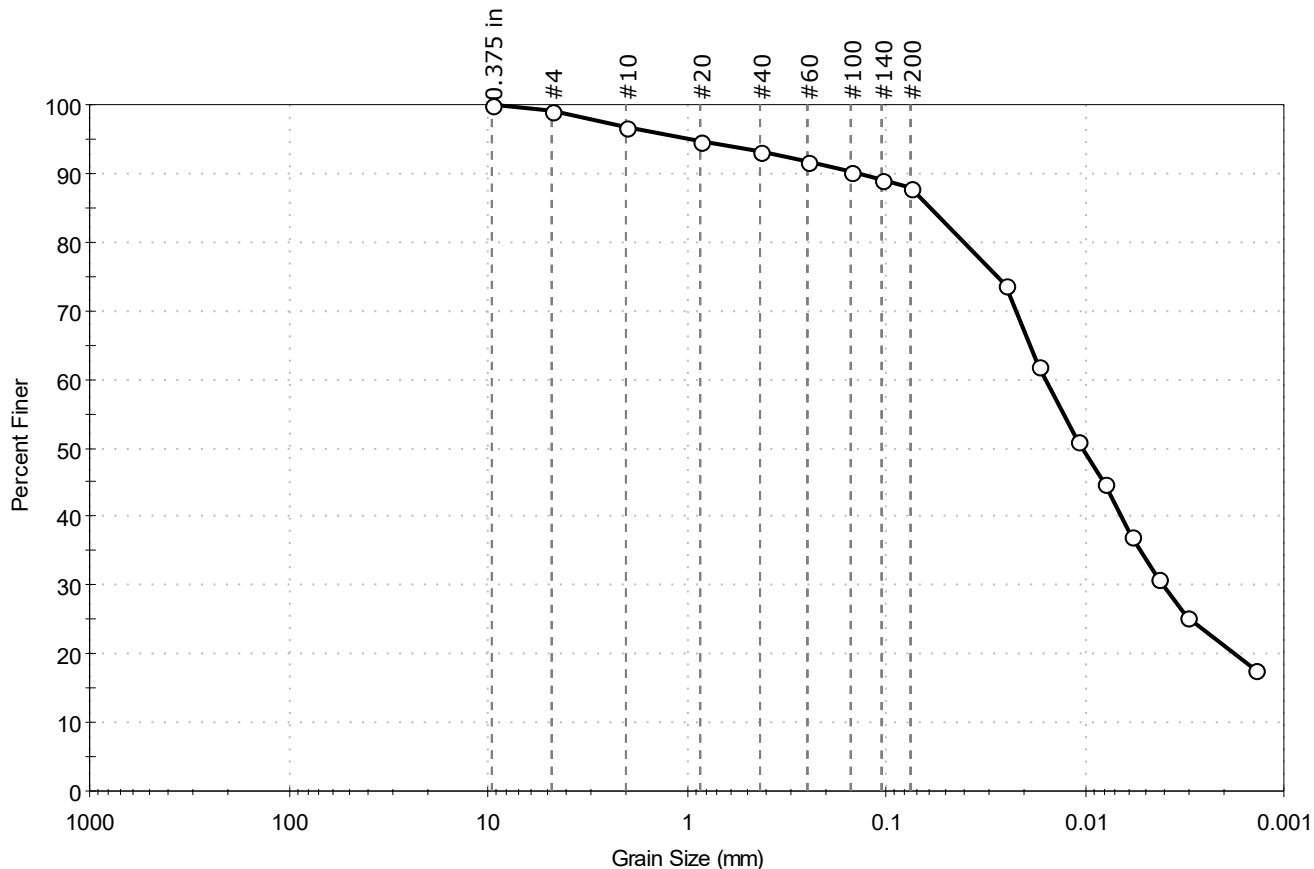
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar
Sample ID:	5D	Test Date:	10/02/18
Depth :	15-17 ft	Test Id:	474547
Test Comment:	---		
Visual Description:	Moist, olive gray silty clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.8	11.3	87.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	97		
#20	0.85	95		
#40	0.42	93		
#60	0.25	92		
#100	0.15	90		
#140	0.11	89		
#200	0.075	88		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0253	74		
---	0.0170	62		
---	0.0110	51		
---	0.0081	45		
---	0.0059	37		
---	0.0043	31		
---	0.0031	25		
---	0.0014	18		

Coefficients

$D_{85} = 0.0599$ mm $D_{30} = 0.0041$ mm
 $D_{60} = 0.0157$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0104$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY (CL-ML)

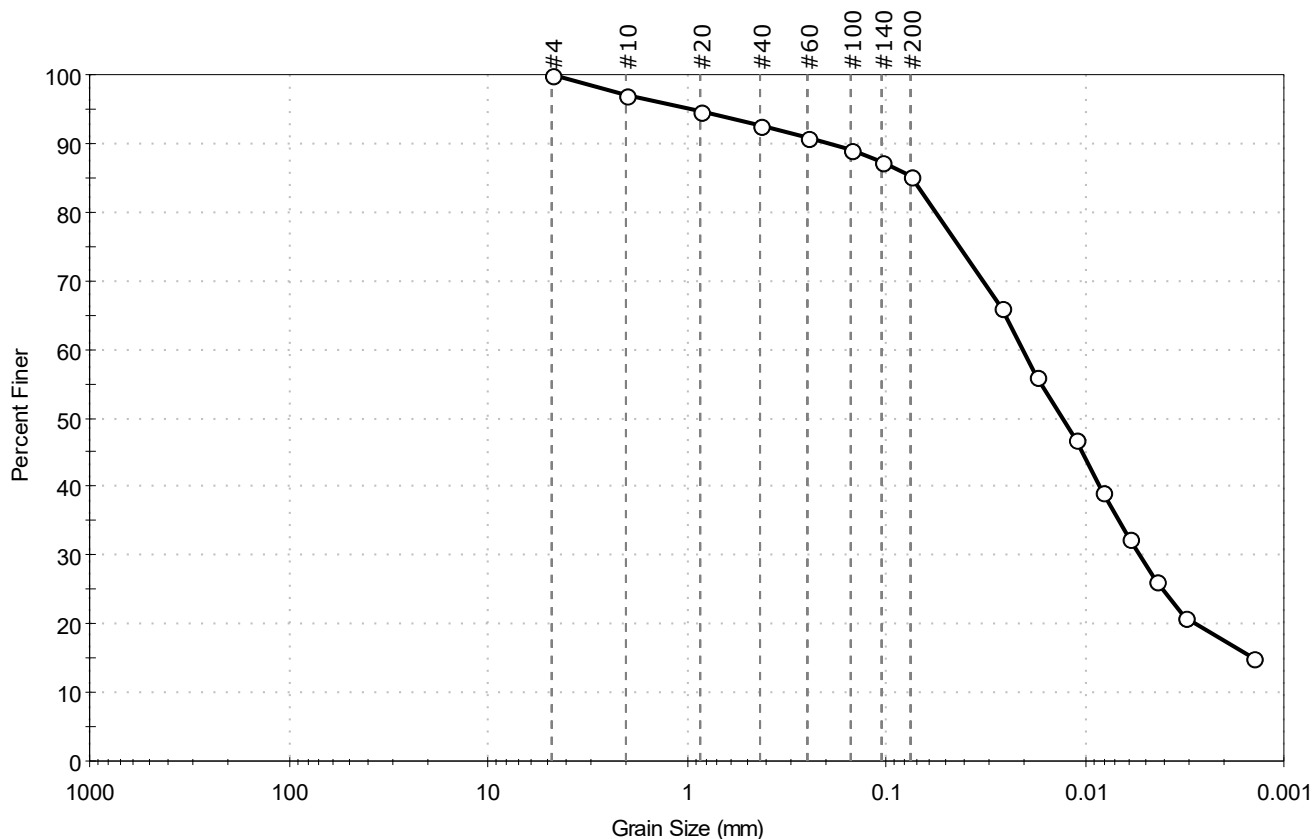
AASHTO Silty Soils (A-4 (4))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	4D	Test Date:	10/02/18
Depth :	6-8 ft	Test Id:	474548
Test Comment:	---		
Visual Description:	Moist, olive silty clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	14.8	85.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	97		
#20	0.85	95		
#40	0.42	93		
#60	0.25	91		
#100	0.15	89		
#140	0.11	87		
#200	0.075	85		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0264	66		
---	0.0175	56		
---	0.0112	47		
---	0.0082	39		
---	0.0060	32		
---	0.0043	26		
---	0.0031	21		
---	0.0014	15		

Coefficients

$D_{85} = 0.0743$ mm $D_{30} = 0.0053$ mm
 $D_{60} = 0.0206$ mm $D_{15} = 0.0014$ mm
 $D_{50} = 0.0130$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY (CL-ML)

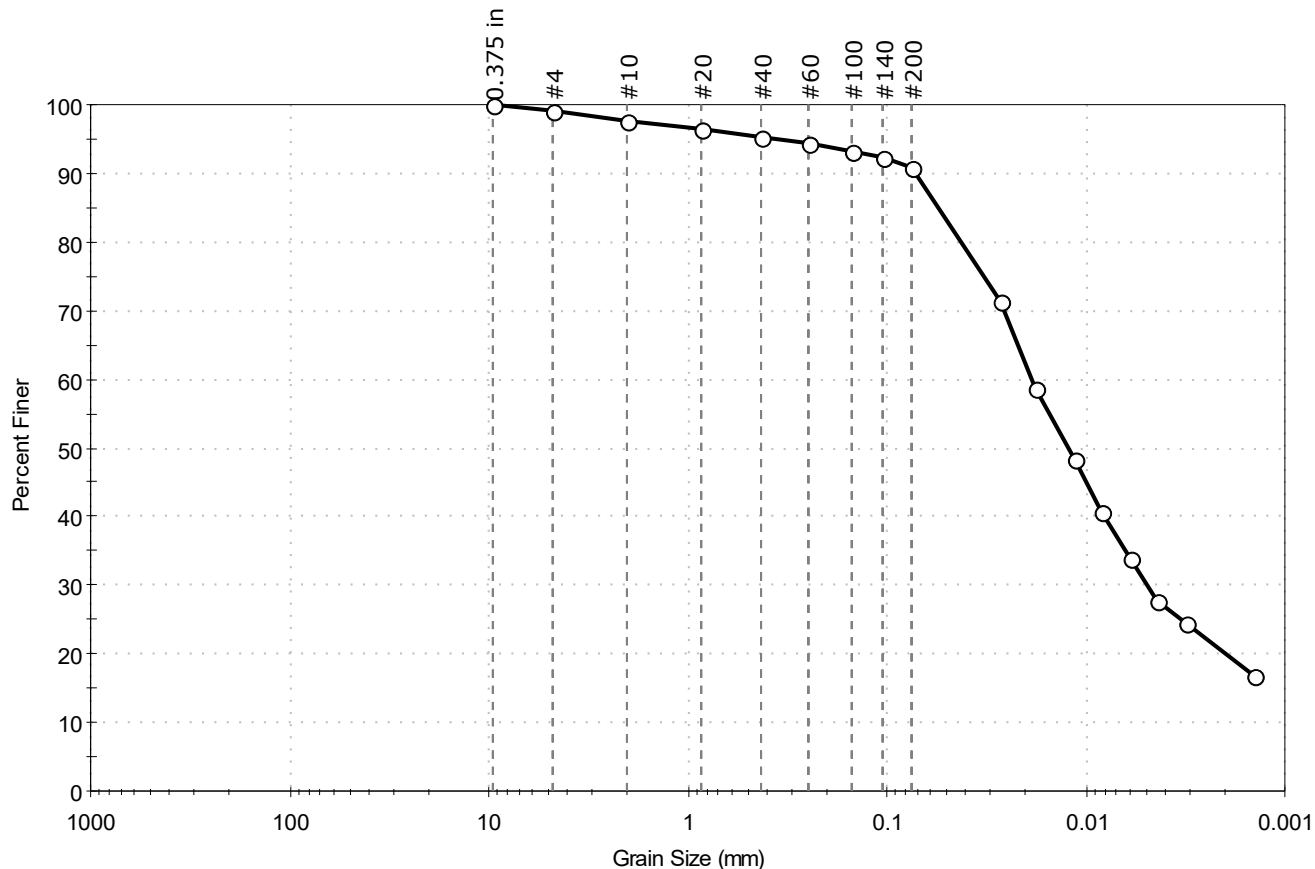
AASHTO Silty Soils (A-4 (2))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	5D	Test Date:	10/02/18
Depth :	10-12 ft	Test Id:	474549
Test Comment:	---		
Visual Description:	Moist, olive gray silty clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	1.0	8.2	90.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	98		
#20	0.85	96		
#40	0.42	95		
#60	0.25	94		
#100	0.15	93		
#140	0.11	92		
#200	0.075	91		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0266	71		
---	0.0178	59		
---	0.0114	48		
---	0.0083	41		
---	0.0061	34		
---	0.0044	28		
---	0.0031	24		
---	0.0014	17		

Coefficients

$D_{85} = 0.0550$ mm $D_{30} = 0.0049$ mm
 $D_{60} = 0.0186$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0122$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY (CL-ML)

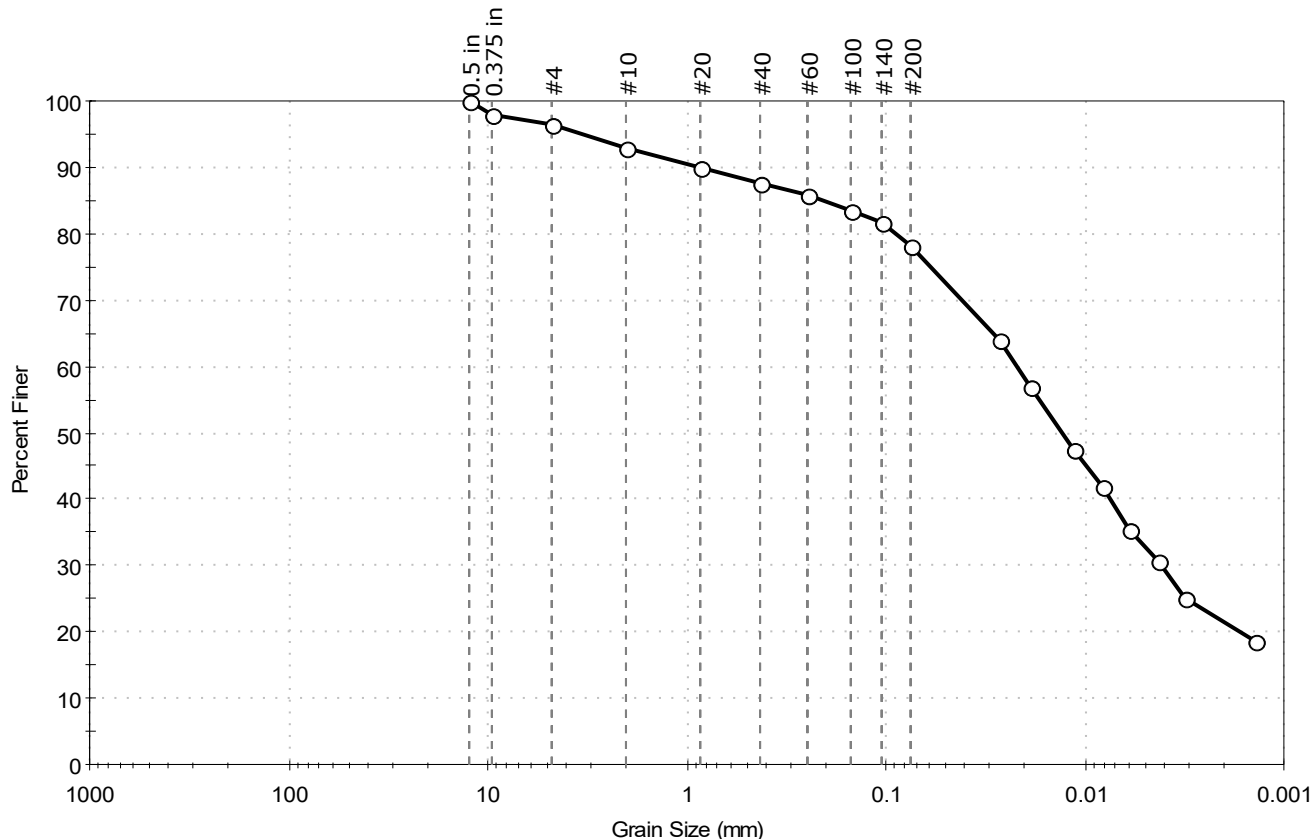
AASHTO Silty Soils (A-4 (4))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	6D	Test Date:	10/02/18
Depth :	15-17 ft	Test Id:	474550
Test Comment:	---		
Visual Description:	Moist, olive gray silty clay with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	3.5	18.4	78.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	97		
#10	2.00	93		
#20	0.85	90		
#40	0.42	88		
#60	0.25	86		
#100	0.15	83		
#140	0.11	82		
#200	0.075	78		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0271	64		
---	0.0188	57		
---	0.0113	47		
---	0.0082	42		
---	0.0060	35		
---	0.0043	31		
---	0.0031	25		
---	0.0014	19		

Coefficients

$D_{85} = 0.2105$ mm $D_{30} = 0.0041$ mm
 $D_{60} = 0.0220$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0130$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY with Sand (CL-ML)

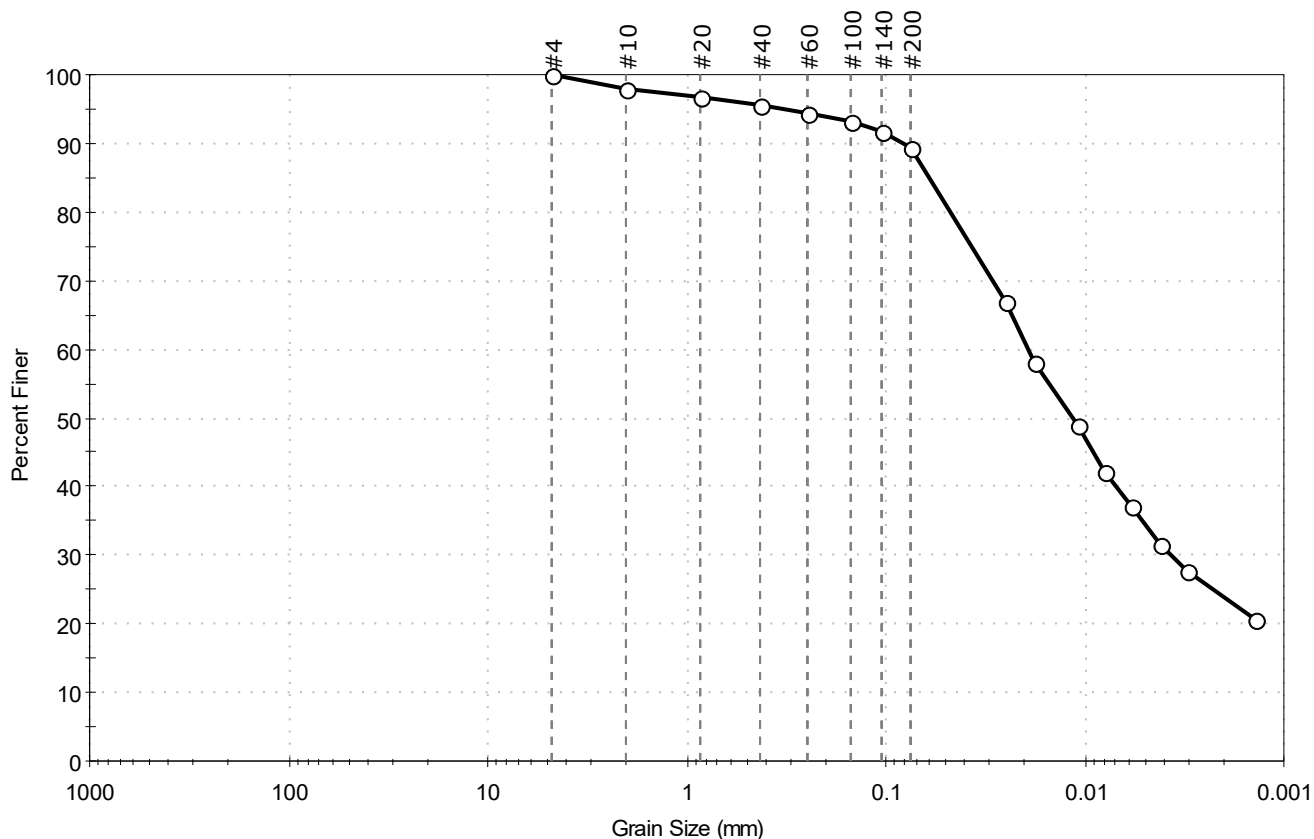
AASHTO Silty Soils (A-4 (3))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	7D	Test Date:	10/02/18
Depth :	20-22 ft	Test Id:	474551
Test Comment:	---		
Visual Description:	Moist, dark gray silty clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	10.7	89.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	98		
#20	0.85	97		
#40	0.42	96		
#60	0.25	94		
#100	0.15	93		
#140	0.11	92		
#200	0.075	89		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0248	67		
---	0.0180	58		
---	0.0109	49		
---	0.0080	42		
---	0.0058	37		
---	0.0042	31		
---	0.0030	28		
---	0.0014	21		

Coefficients

$D_{85} = 0.0607$ mm $D_{30} = 0.0037$ mm
 $D_{60} = 0.0192$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0116$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Silty CLAY (CL-ML)

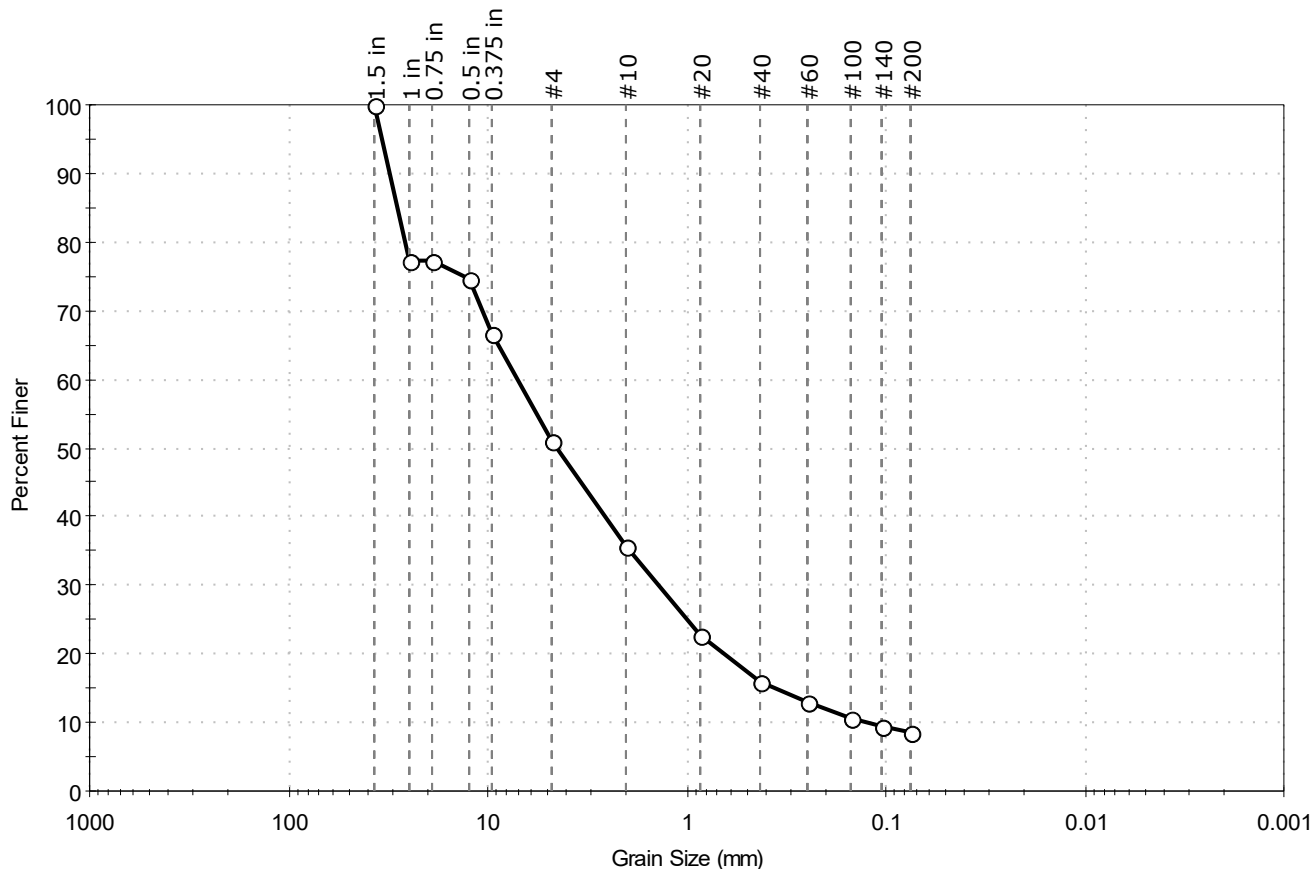
AASHTO Silty Soils (A-4 (2))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-205	Sample Type:	jar
Sample ID:	2D	Test Date:	03/03/20
Depth :	2-4 ft	Test Id:	545432
Test Comment:	---		
Visual Description:	Moist, brown gravel with silt and sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	48.9	42.7	8.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1 in	25.00	77		
0.75 in	19.00	77		
0.5 in	12.50	75		
0.375 in	9.50	67		
#4	4.75	51		
#10	2.00	36		
#20	0.85	23		
#40	0.42	16		
#60	0.25	13		
#100	0.15	11		
#140	0.11	9		
#200	0.075	8.4		

Coefficients

$D_{85} = 28.7280 \text{ mm}$ $D_{30} = 1.3819 \text{ mm}$
 $D_{60} = 7.0347 \text{ mm}$ $D_{15} = 0.3570 \text{ mm}$
 $D_{50} = 4.4483 \text{ mm}$ $D_{10} = 0.1254 \text{ mm}$
 $C_u = 56.098$ $C_c = 2.165$

Classification

ASTM N/A

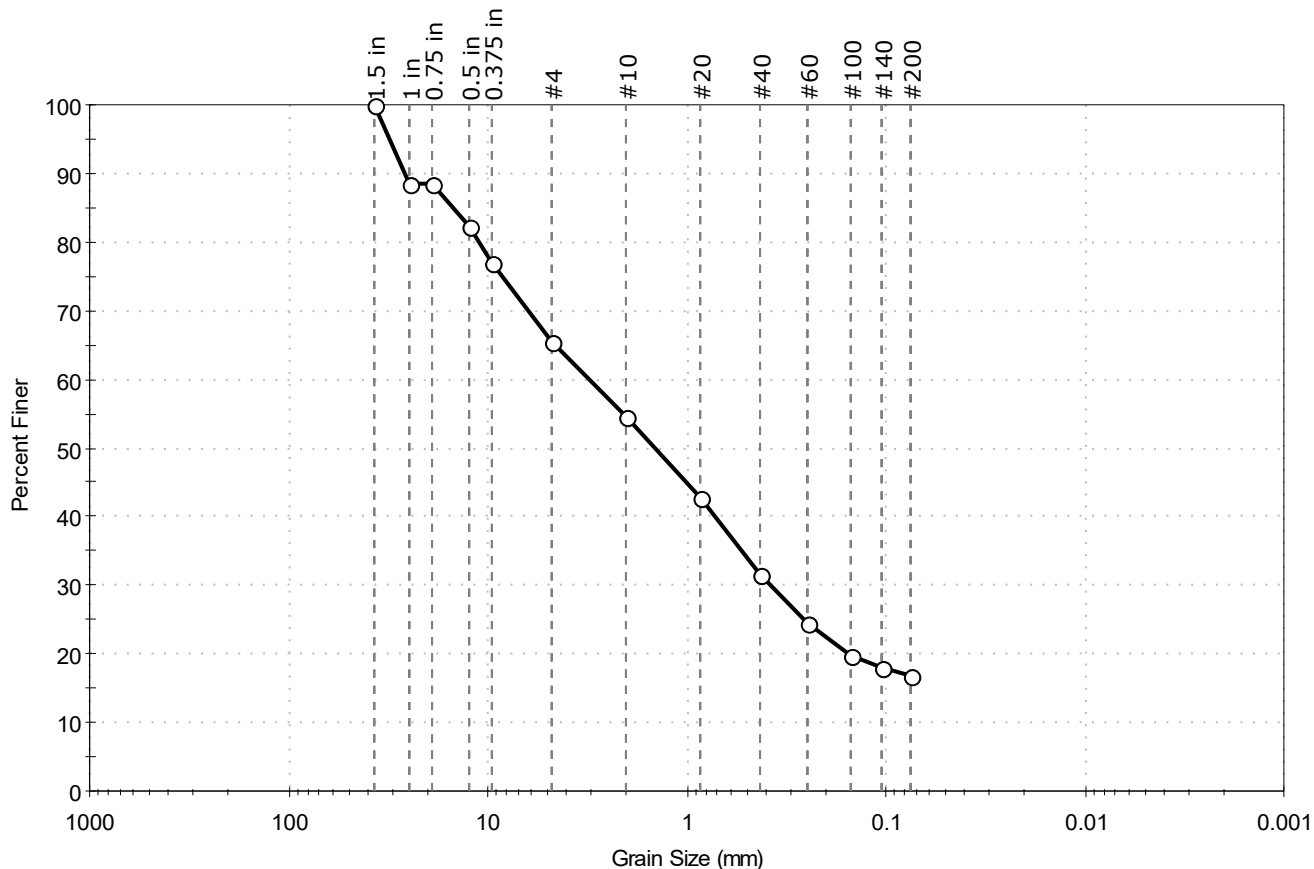
AASHTO Stone Fragments, Gravel and Sand (A-1-a (1))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-205	Sample Type:	jar
Sample ID:	4D	Test Date:	03/03/20
Depth :	6-8 ft	Test Id:	545434
Test Comment:	---		
Visual Description:	Moist, dark olive brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	34.5	48.8	16.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1 in	25.00	88		
0.75 in	19.00	88		
0.5 in	12.50	82		
0.375 in	9.50	77		
#4	4.75	66		
#10	2.00	55		
#20	0.85	43		
#40	0.42	32		
#60	0.25	24		
#100	0.15	20		
#140	0.11	18		
#200	0.075	17		

Coefficients

$D_{85} = 15.0568 \text{ mm}$ $D_{30} = 0.3755 \text{ mm}$
 $D_{60} = 3.0554 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 1.4239 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

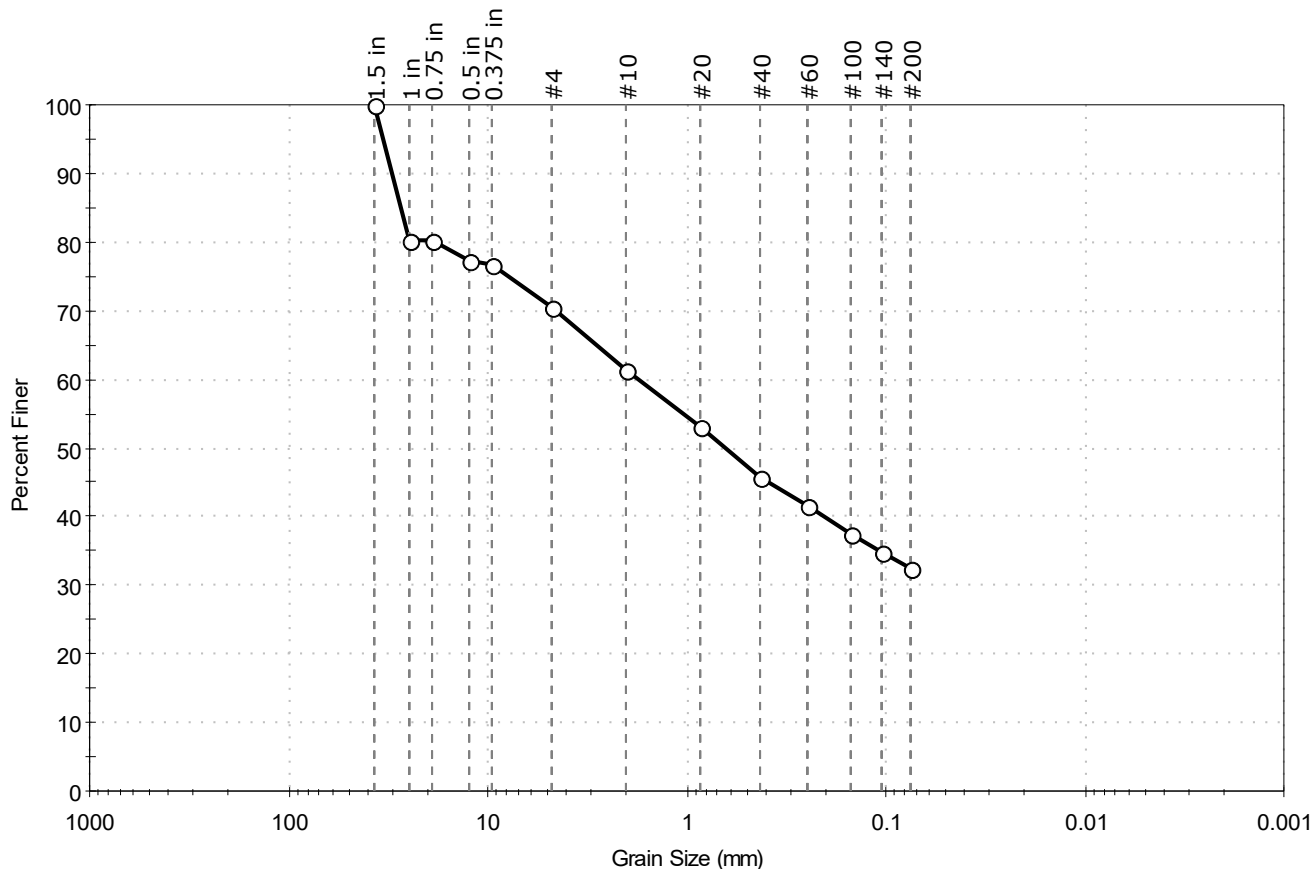
AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-205	Sample Type:	jar
Sample ID:	5D	Test Date:	03/03/20
Depth :	8-10 ft	Test Id:	545435
Test Comment:	---		
Visual Description:	Moist, olive brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	29.7	37.9	32.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1 in	25.00	80		
0.75 in	19.00	80		
0.5 in	12.50	77		
0.375 in	9.50	77		
#4	4.75	70		
#10	2.00	61		
#20	0.85	53		
#40	0.42	46		
#60	0.25	42		
#100	0.15	37		
#140	0.11	35		
#200	0.075	32		

Coefficients

$D_{85} = 27.5656$ mm $D_{30} = \text{N/A}$
 $D_{60} = 1.7284$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.6363$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

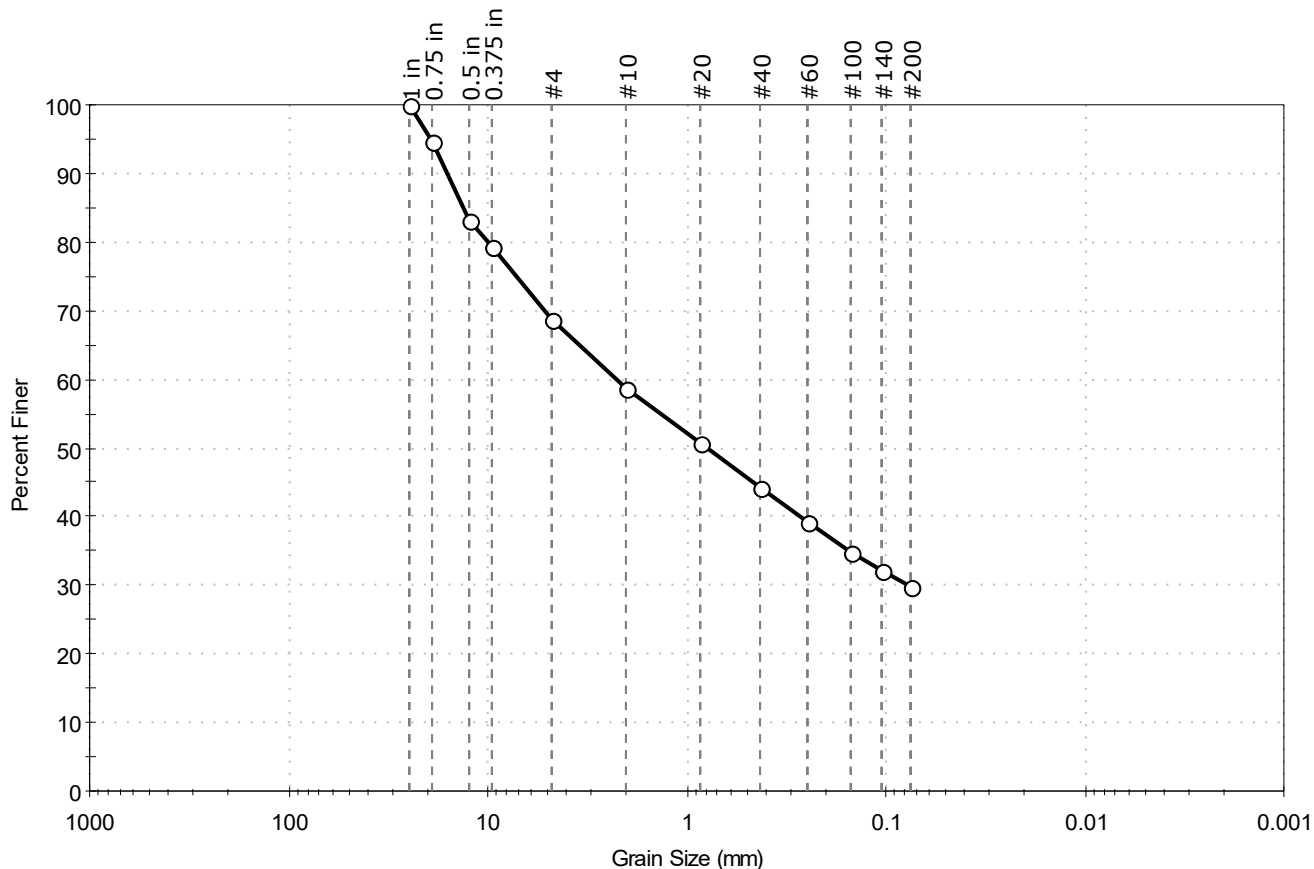
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-205	Sample Type:	jar
Sample ID:	6D	Test Date:	03/03/20
Depth :	10-12 ft	Test Id:	545436
Test Comment:	---		
Visual Description:	Moist, olive brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	31.4	38.9	29.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	95		
0.5 in	12.50	83		
0.375 in	9.50	79		
#4	4.75	69		
#10	2.00	59		
#20	0.85	51		
#40	0.42	44		
#60	0.25	39		
#100	0.15	35		
#140	0.11	32		
#200	0.075	30		

Coefficients

$D_{85} = 13.3002 \text{ mm}$ $D_{30} = 0.0785 \text{ mm}$
 $D_{60} = 2.2265 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.7889 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

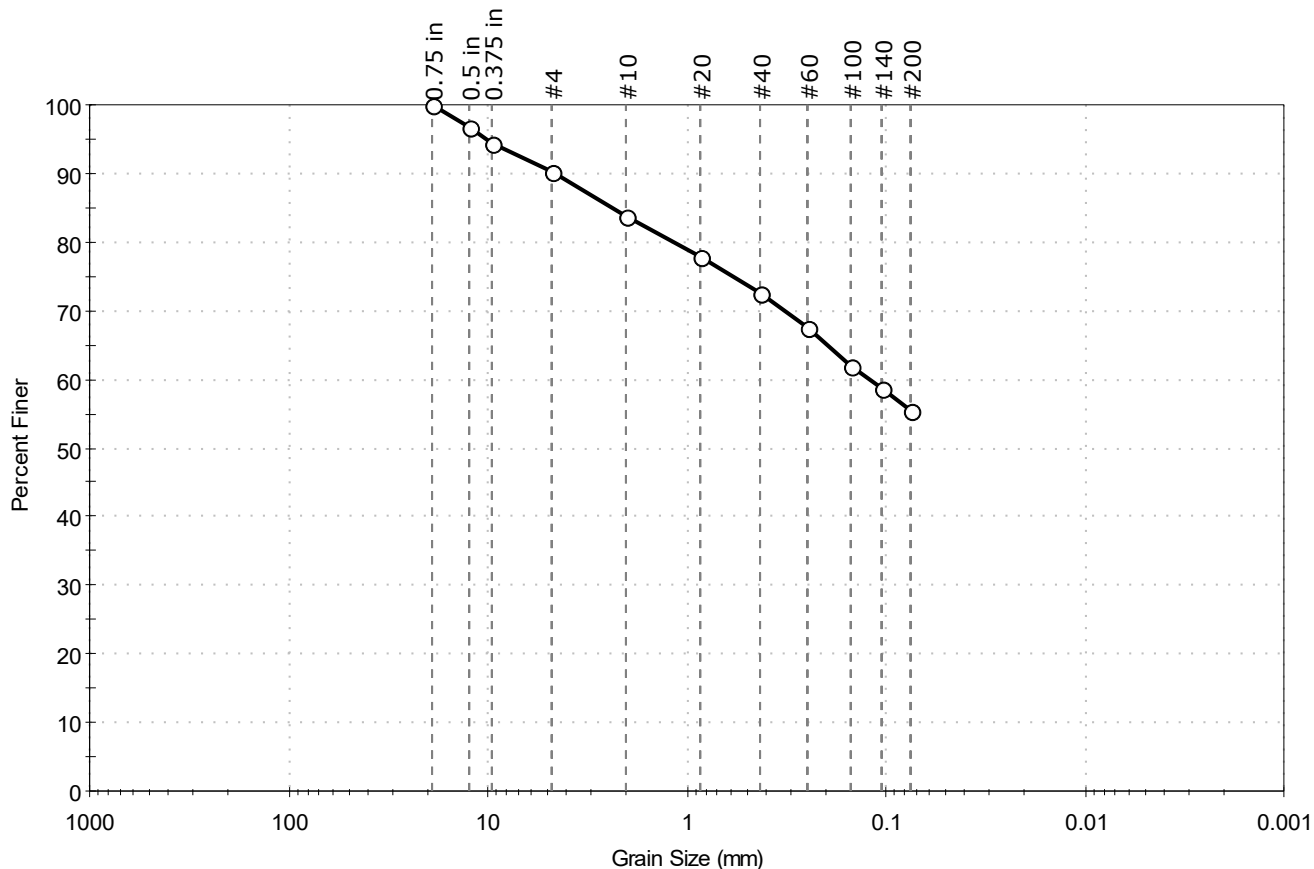
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-205	Sample Type:	jar
Sample ID:	10D	Test Date:	03/03/20
Depth :	18-20 ft	Test Id:	545439
Test Comment:	---		
Visual Description:	Moist, olive brown sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	9.8	34.8	55.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	97		
0.375 in	9.50	94		
#4	4.75	90		
#10	2.00	84		
#20	0.85	78		
#40	0.42	73		
#60	0.25	67		
#100	0.15	62		
#140	0.11	59		
#200	0.075	55		

Coefficients

$D_{85} = 2.3331 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1227 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

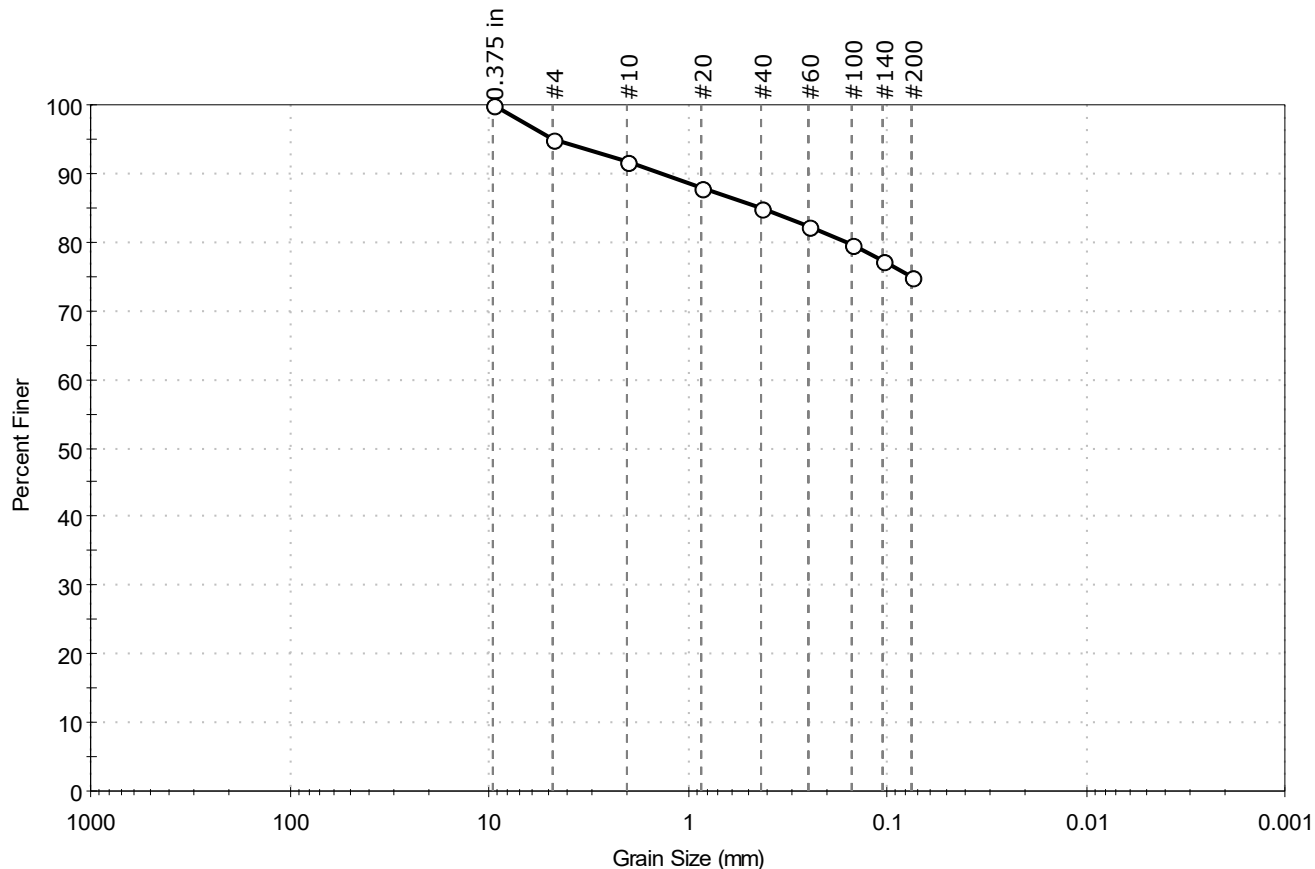
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	jar
Sample ID:	2D	Test Date:	03/03/20
Depth :	5-7 ft	Test Id:	545430
Test Comment:	---		
Visual Description:	Moist, olive silt with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	5.1	19.9	75.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	95		
#10	2.00	92		
#20	0.85	88		
#40	0.42	85		
#60	0.25	82		
#100	0.15	80		
#140	0.11	77		
#200	0.075	75		

Coefficients

$D_{85} = 0.4320$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

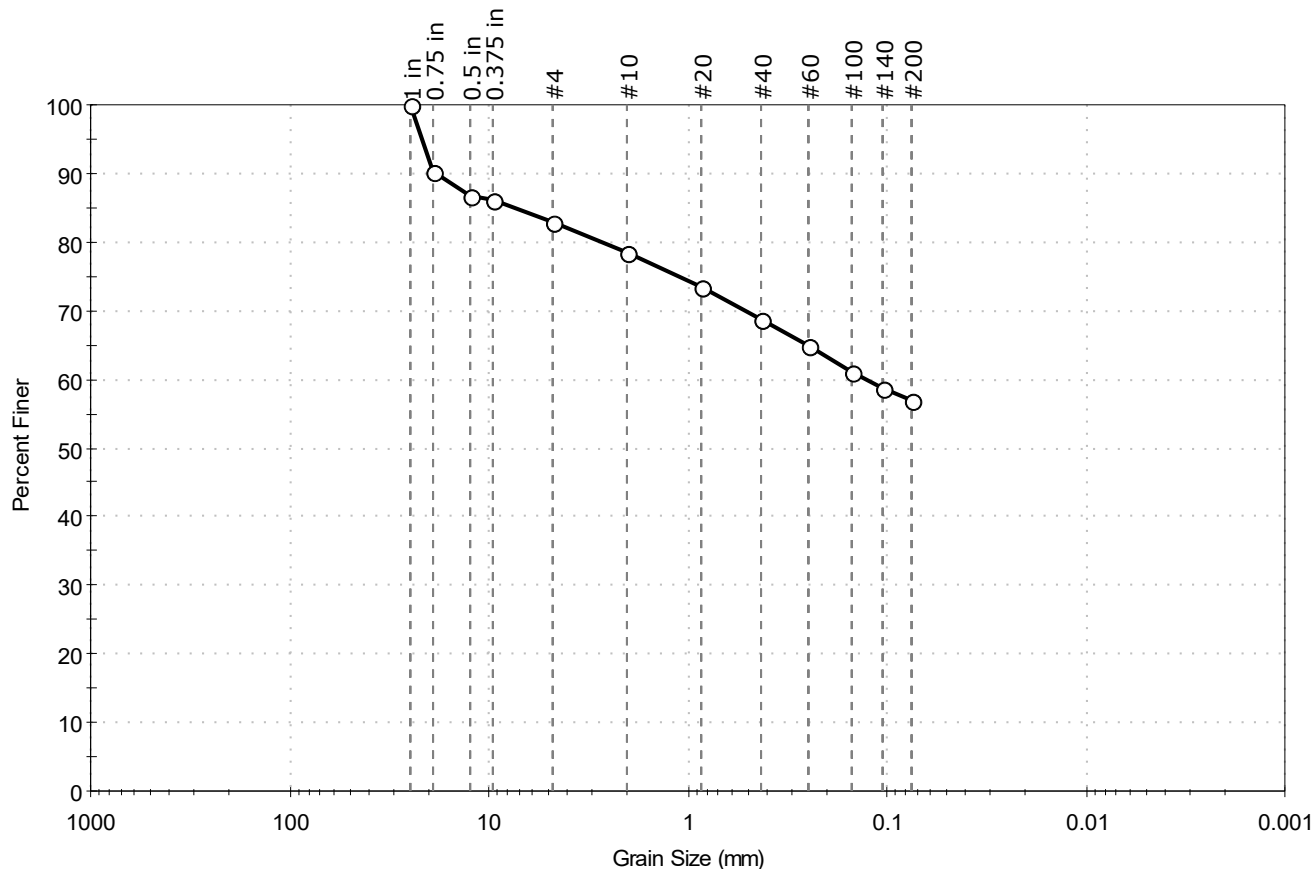
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	jar
Sample ID:	3D	Test Date:	03/03/20
Depth :	10-12 ft	Test Id:	545431
Test Comment:	---		
Visual Description:	Moist, olive gray sandy silt with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	17.0	26.0	57.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	90		
0.5 in	12.50	87		
0.375 in	9.50	86		
#4	4.75	83		
#10	2.00	78		
#20	0.85	73		
#40	0.42	69		
#60	0.25	65		
#100	0.15	61		
#140	0.11	59		
#200	0.075	57		

Coefficients

$D_{85} = 7.5034 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1269 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

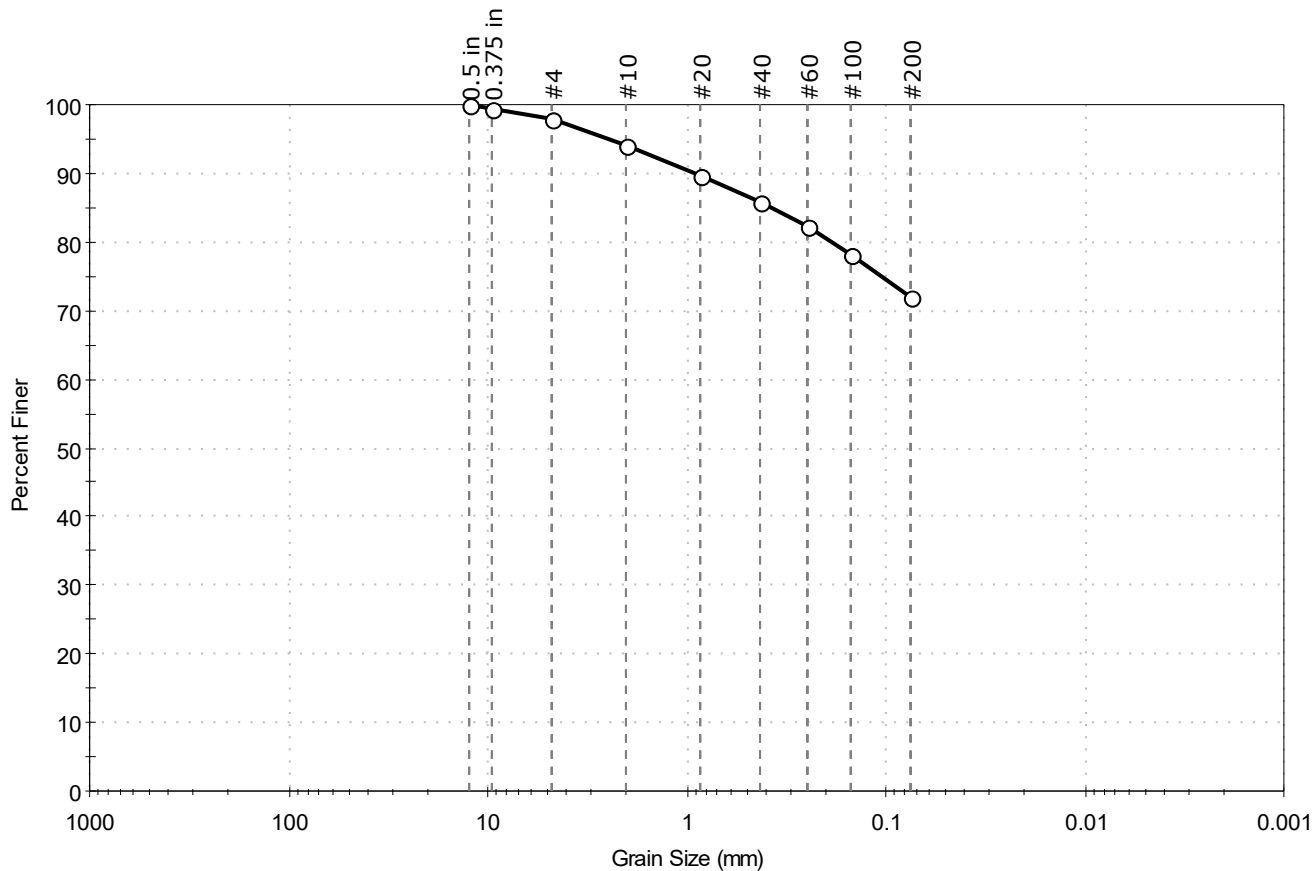
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : **ROUNDED**
 Sand/Gravel Hardness : **HARD**

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Clewleyville Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308854
Boring ID:	BB-ECR-101	Sample Type:	jar
Sample ID:	3D	Test Date:	10/03/18
Depth :	4.3-6.3 ft	Test Id:	474393
Test Comment:	---		
Visual Description:	Moist, brown clay with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	2.2	25.8	72.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	99		
#4	4.75	98		
#10	2.00	94		
#20	0.85	90		
#40	0.42	86		
#60	0.25	82		
#100	0.15	78		
#200	0.075	72		

Coefficients

$D_{85} = 0.3677 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

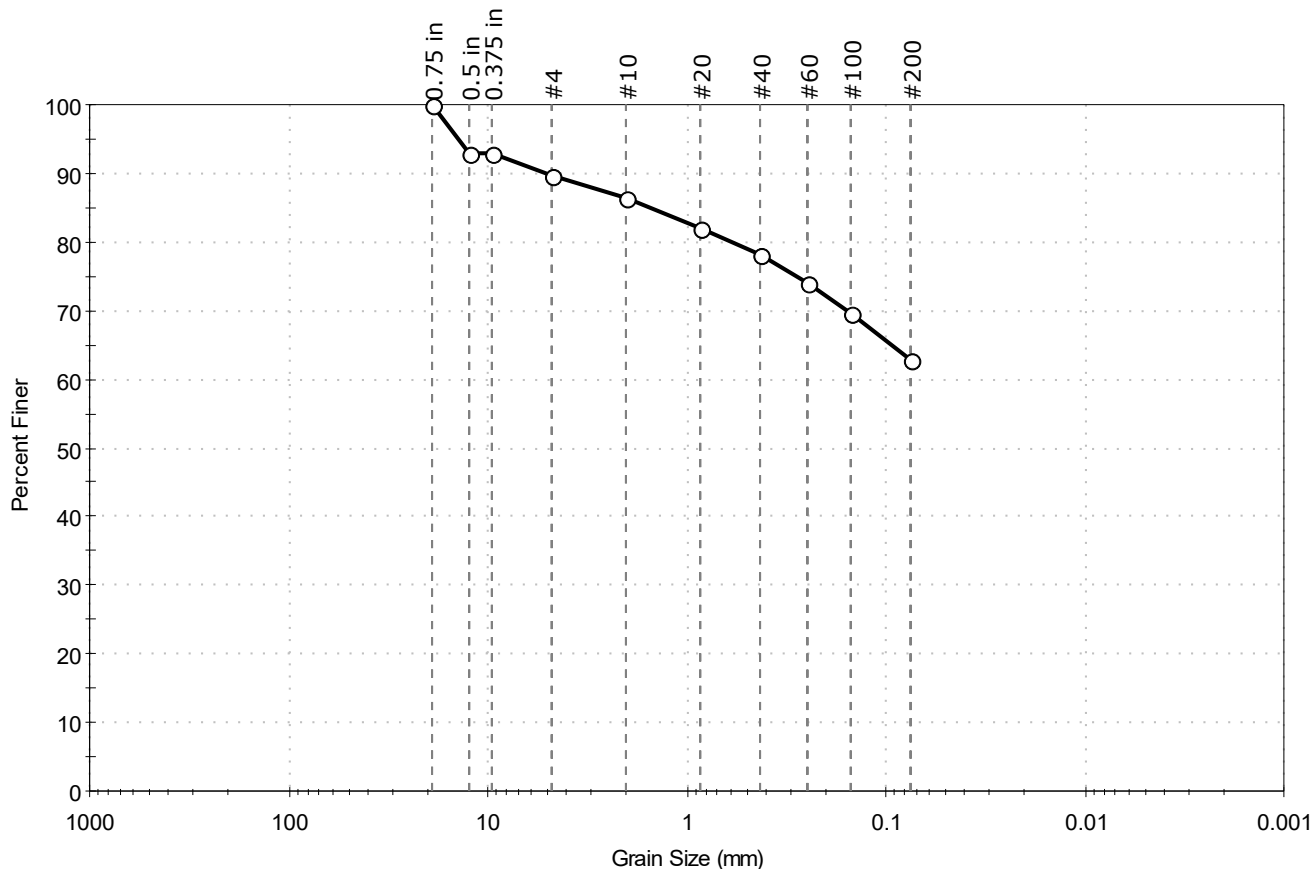
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Clewleyville Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308854
Boring ID:	BB-ECR-101	Sample Type:	jar
Sample ID:	4D	Test Date:	10/02/18
Depth :	10-12 ft	Test Id:	474394
Test Comment:	---		
Visual Description:	Moist, brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	10.3	26.9	62.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	93		
#4	4.75	90		
#10	2.00	86		
#20	0.85	82		
#40	0.42	78		
#60	0.25	74		
#100	0.15	70		
#200	0.075	63		

Coefficients

$D_{85} = 1.5132 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

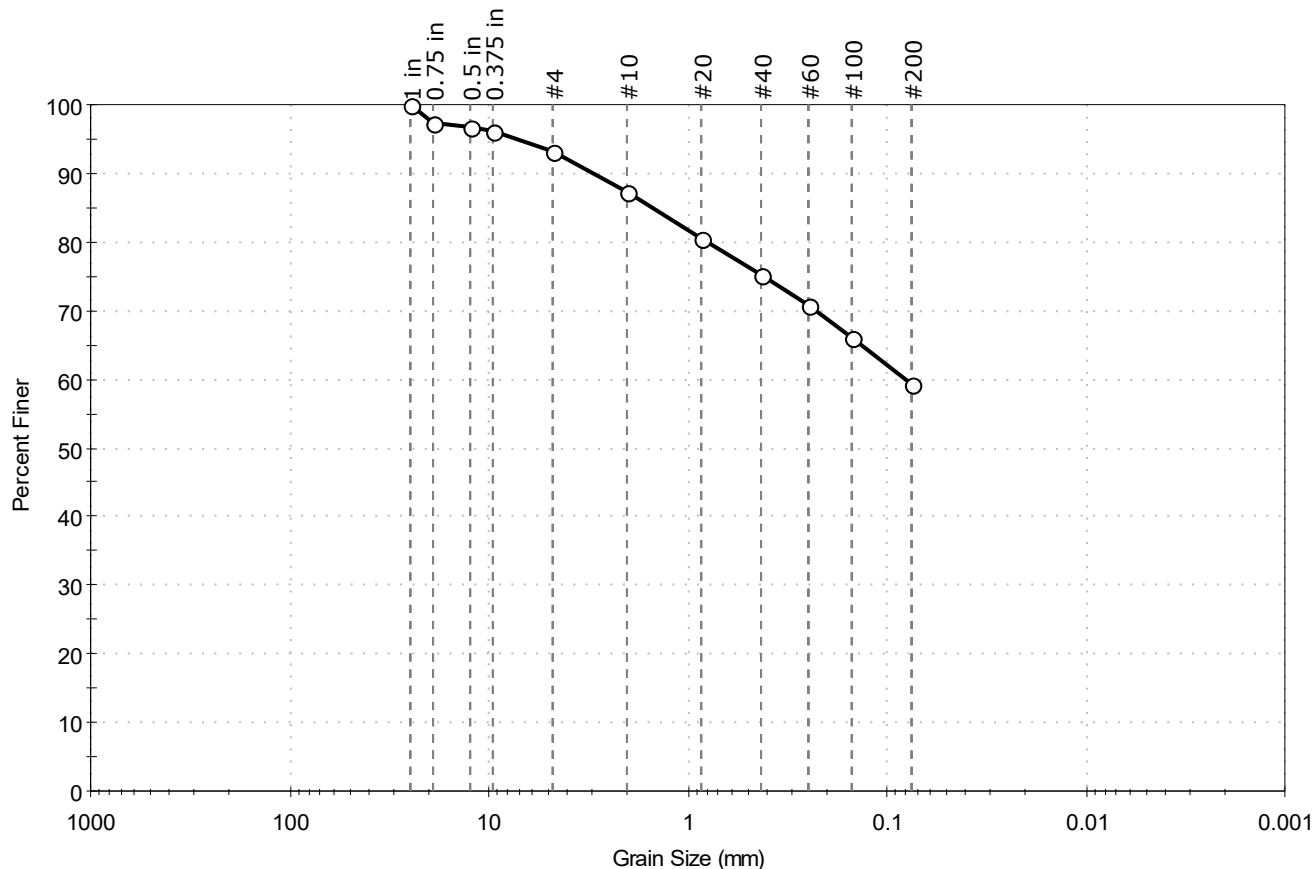
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : **ROUNDED**
 Sand/Gravel Hardness : **HARD**

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Clewleyville Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308854
Boring ID:	BB-ECR-102	Sample Type:	jar
Sample ID:	3D	Test Date:	10/01/18
Depth :	4.3-6.3	Test Id:	474395
Test Comment:	---		
Visual Description:	Moist, brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.8	34.0	59.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	97		
0.5 in	12.50	97		
0.375 in	9.50	96		
#4	4.75	93		
#10	2.00	87		
#20	0.85	81		
#40	0.42	75		
#60	0.25	71		
#100	0.15	66		
#200	0.075	59		

Coefficients

$D_{85} = 1.4750 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.0812 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

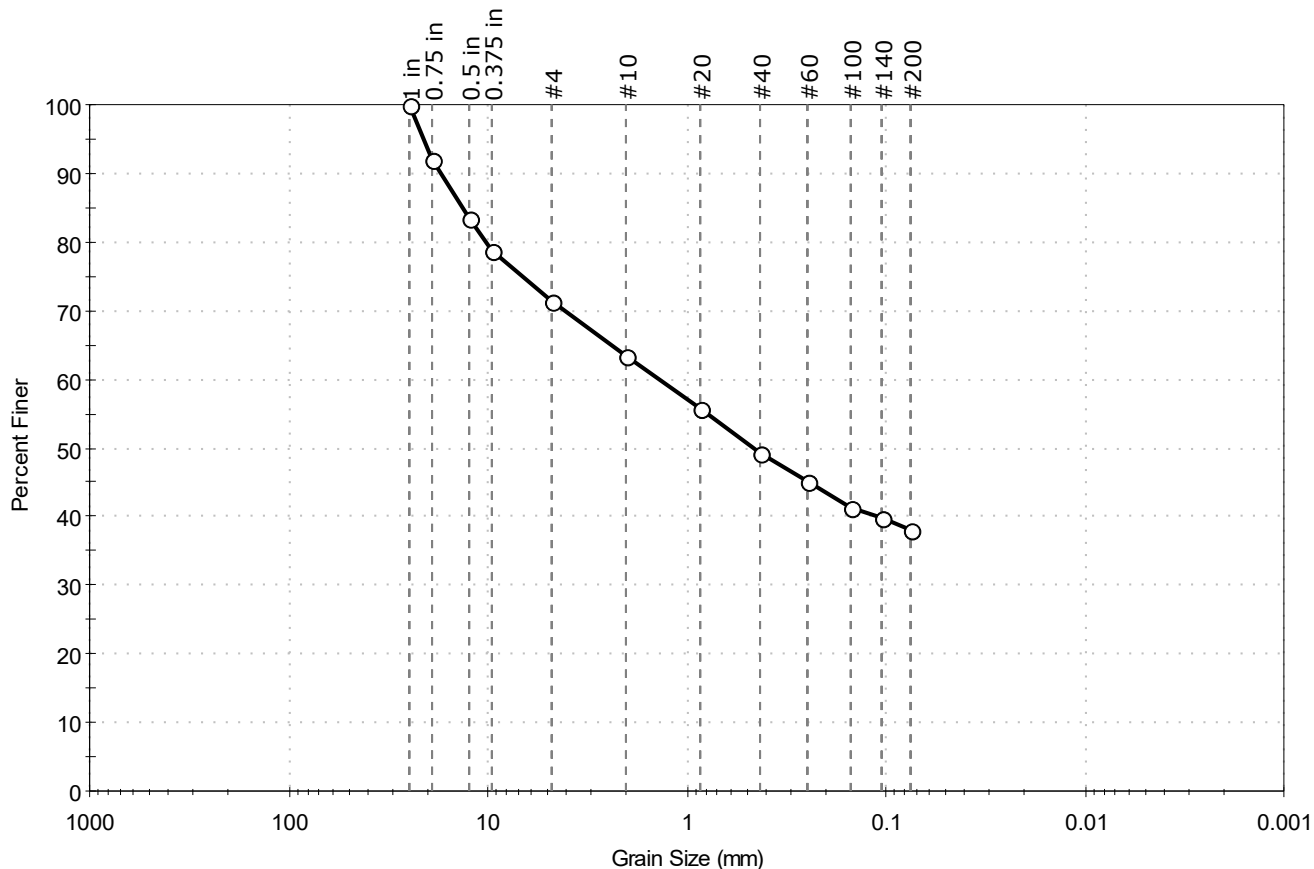
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : **ROUNDED**
 Sand/Gravel Hardness : **HARD**

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Bridge (Lambert Rd)		
Location:	Eddington, ME	Project No:	GTX-313323
Boring ID:	BB-ELAR-201A	Sample Type:	jar
Sample ID:	2D	Test Date:	03/19/21
Depth :	2-4	Test Id:	613355
Test Comment:	---		
Visual Description:	Moist, dark brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	28.6	33.5	37.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	92		
0.5 in	12.50	83		
0.375 in	9.50	79		
#4	4.75	71		
#10	2.00	63		
#20	0.85	56		
#40	0.42	49		
#60	0.25	45		
#100	0.15	41		
#140	0.11	40		
#200	0.075	38		

Coefficients

$D_{85} = 13.4687 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 1.3581 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.4577 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

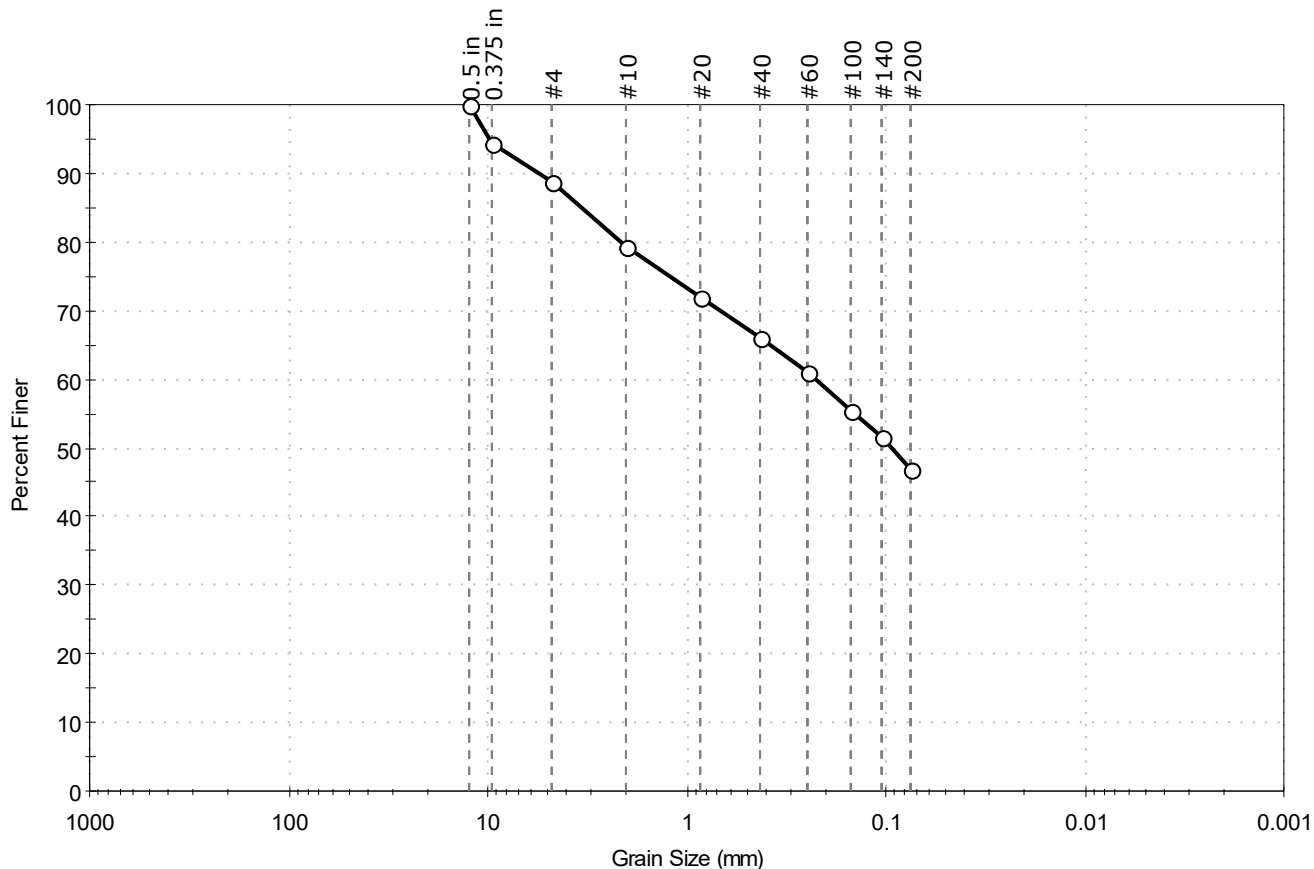
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Bridge (Lambert Rd)		
Location:	Eddington, ME	Project No:	GTX-313323
Boring ID:	BB-ELAR-203A	Sample Type:	jar
Sample ID:	3D	Test Date:	03/22/21
Depth :	4-5.5	Test Id:	613356
Test Comment:	---		
Visual Description:	Moist, olive sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	11.3	41.8	46.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	95		
#4	4.75	89		
#10	2.00	79		
#20	0.85	72		
#40	0.42	66		
#60	0.25	61		
#100	0.15	56		
#140	0.11	52		
#200	0.075	47		

Coefficients

$D_{85} = 3.3596 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.2274 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.0945 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

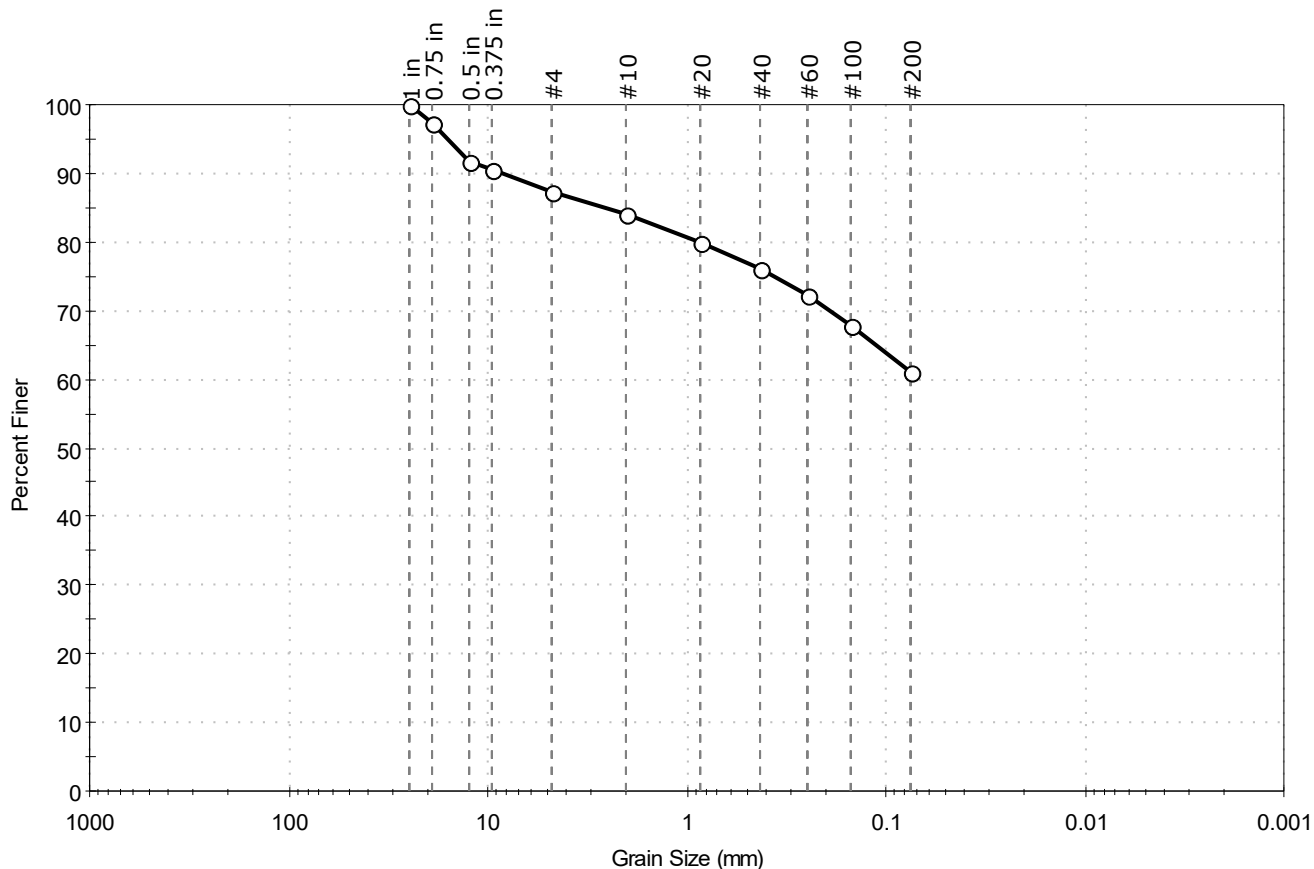
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Levenseller Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308857
Boring ID:	BB-ELER-101	Sample Type:	jar
Sample ID:	3D	Test Date:	10/01/18
Depth :	4-6 ft	Test Id:	474527
Test Comment:	---		
Visual Description:	Moist, yellowish brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	12.6	26.4	61.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	97		
0.5 in	12.50	92		
0.375 in	9.50	91		
#4	4.75	87		
#10	2.00	84		
#20	0.85	80		
#40	0.42	76		
#60	0.25	72		
#100	0.15	68		
#200	0.075	61		

Coefficients

$D_{85} = 2.5324 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

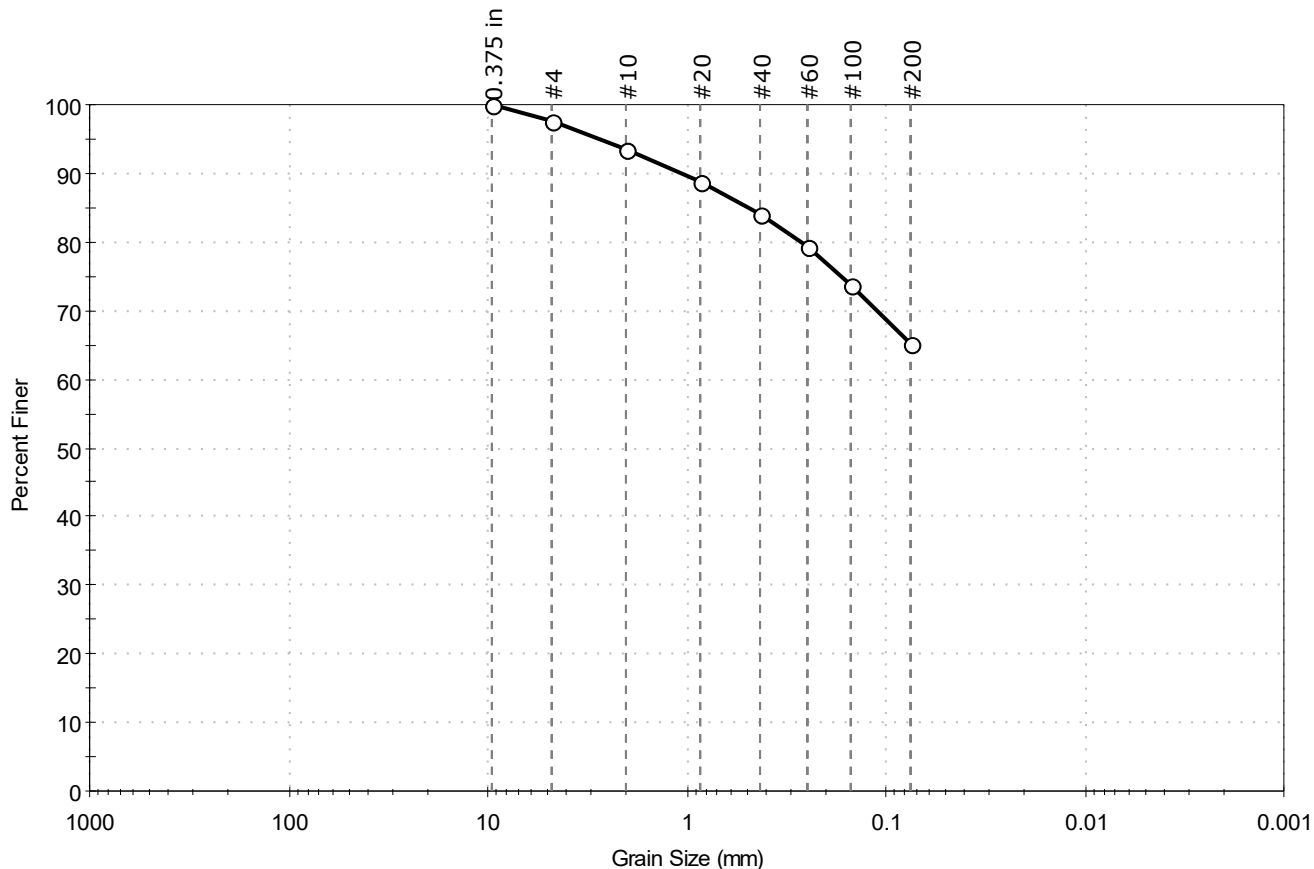
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Levenseller Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308857
Boring ID:	BB-ELER-102	Sample Type:	jar
Sample ID:	3D	Test Date:	10/01/18
Depth :	4.3-6.3 ft	Test Id:	474529
Test Comment:	---		
Visual Description:	Moist, brown sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	2.3	32.4	65.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	98		
#10	2.00	94		
#20	0.85	89		
#40	0.42	84		
#60	0.25	79		
#100	0.15	74		
#200	0.075	65		

Coefficients

$D_{85} = 0.4909 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

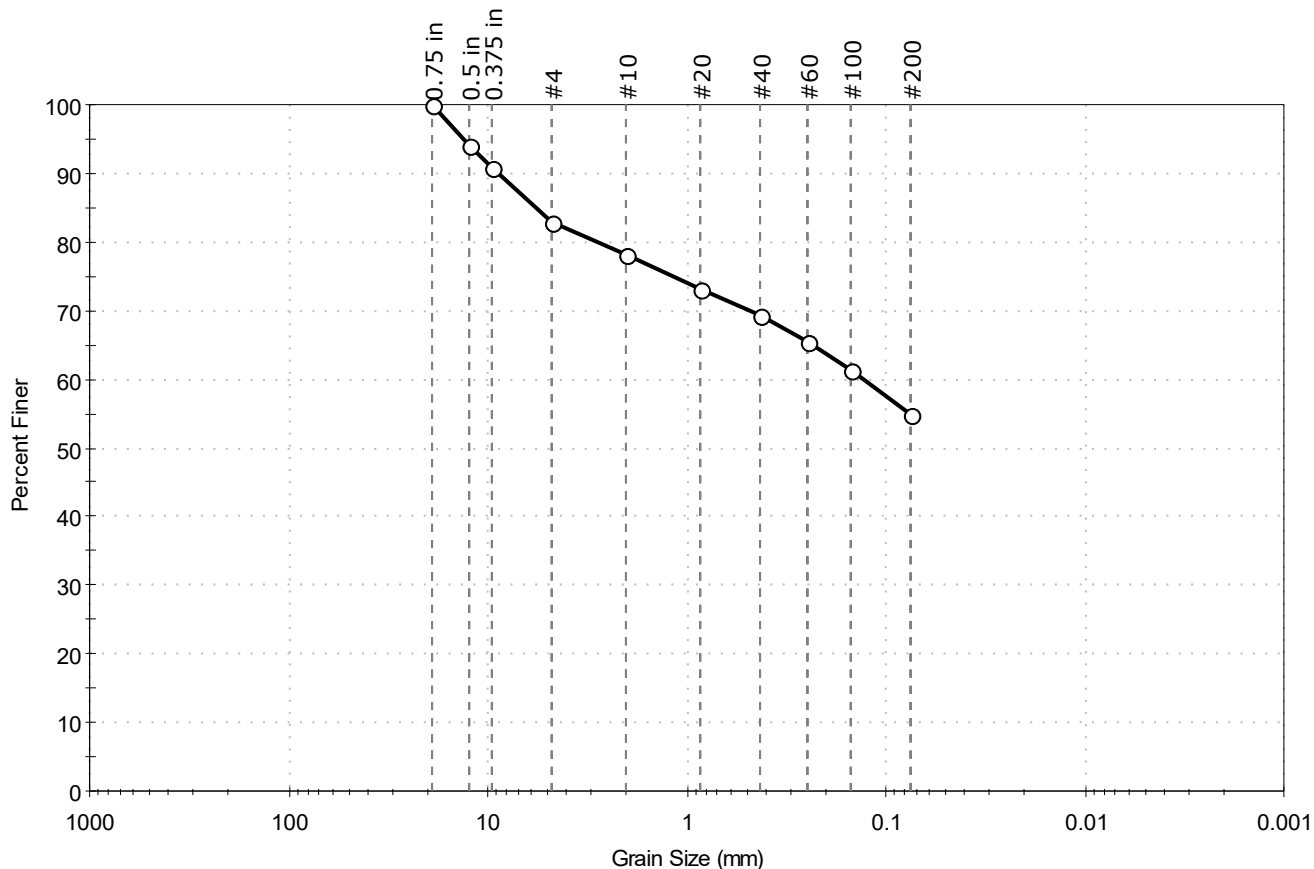
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : **ROUNDED**
 Sand/Gravel Hardness : **HARD**

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Levenseller Rd Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308857
Boring ID:	BB-ELER-102	Sample Type:	jar
Sample ID:	4D	Test Date:	10/03/18
Depth :	10-12 ft	Test Id:	474530
Test Comment:	---		
Visual Description:	Moist, brown sandy clay with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	17.1	28.1	54.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	94		
0.375 in	9.50	91		
#4	4.75	83		
#10	2.00	78		
#20	0.85	73		
#40	0.42	69		
#60	0.25	66		
#100	0.15	61		
#200	0.075	55		

Coefficients

$D_{85} = 5.7232 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1311 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

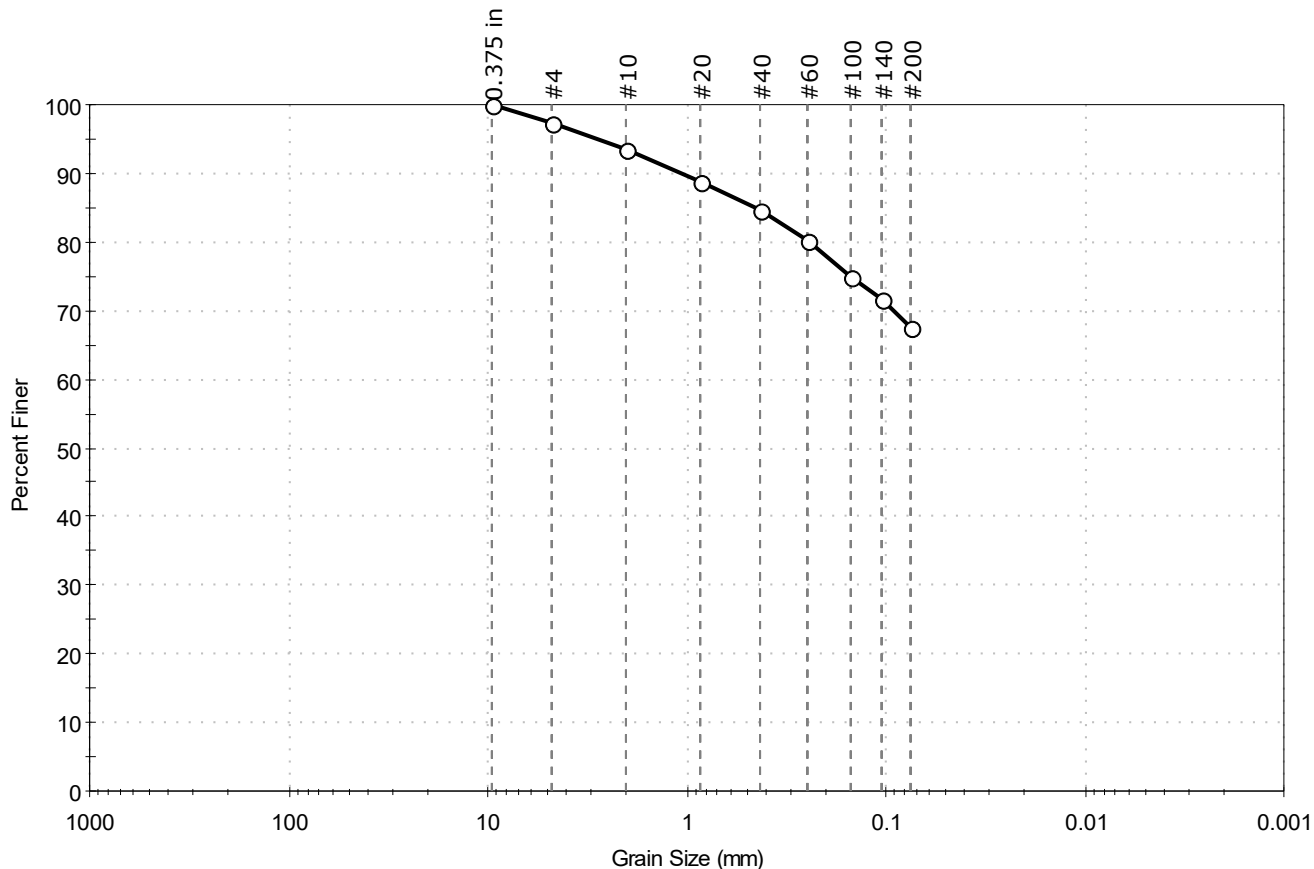
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Bridge (Levenseller Rd)		
Location:	Eddington, ME	Project No:	GTX-313322
Boring ID:	BB-ELER-202	Sample Type:	jar
Sample ID:	3D	Test Date:	03/19/21
Depth :	10-12	Test Id:	613357
Test Comment:	---		
Visual Description:	Moist, olive brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	2.7	29.8	67.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	97		
#10	2.00	93		
#20	0.85	89		
#40	0.42	85		
#60	0.25	80		
#100	0.15	75		
#140	0.11	72		
#200	0.075	68		

Coefficients

$D_{85} = 0.4528$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

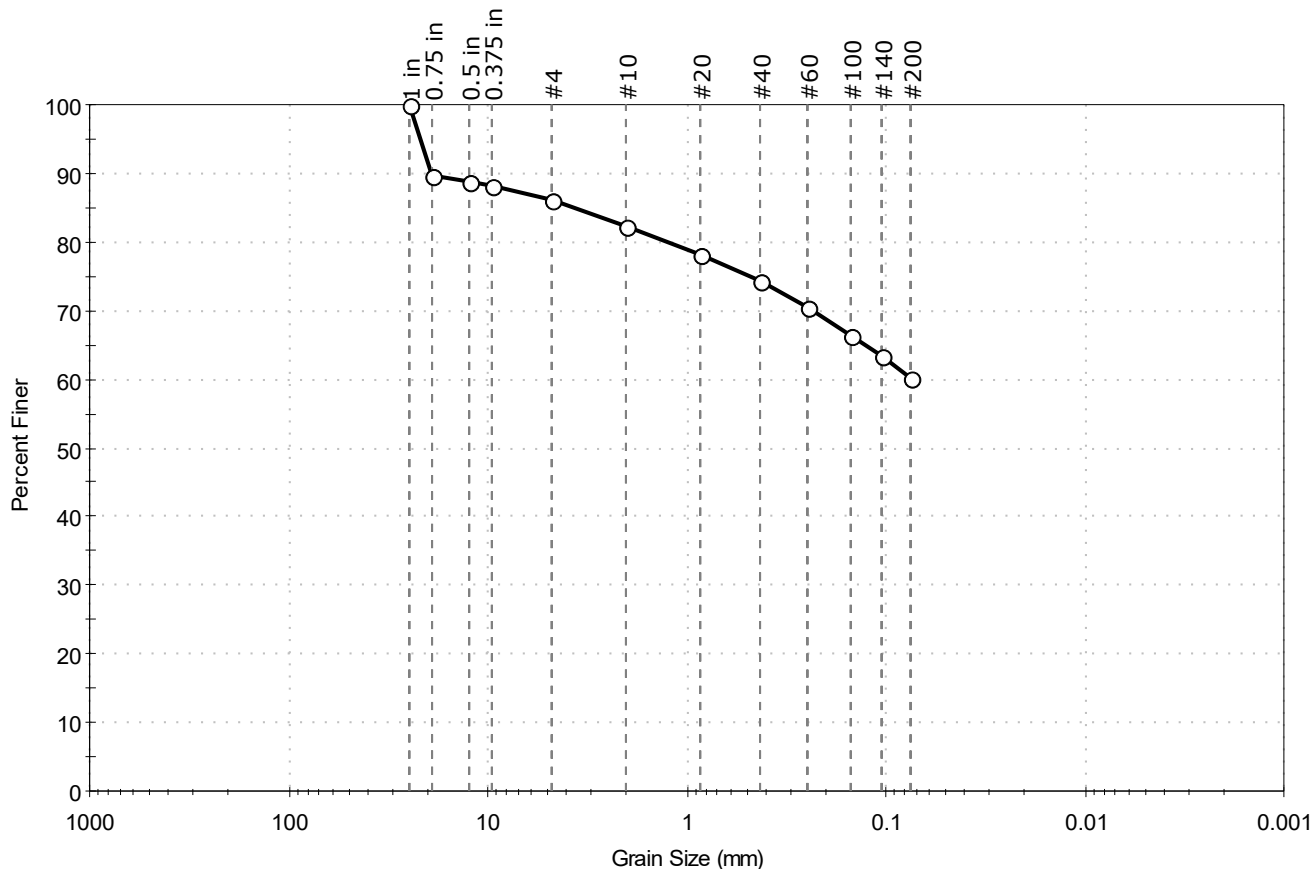
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Bridge (Levenseller Rd)		
Location:	Eddington, ME	Project No:	GTX-313322
Boring ID:	BB-ELER-203	Sample Type:	jar
Sample ID:	2D	Test Date:	03/22/21
Depth :	5-7	Test Id:	613358
Test Comment:	---		
Visual Description:	Moist, gray sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	13.9	25.9	60.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	90		
0.5 in	12.50	89		
0.375 in	9.50	88		
#4	4.75	86		
#10	2.00	82		
#20	0.85	78		
#40	0.42	74		
#60	0.25	71		
#100	0.15	66		
#140	0.11	63		
#200	0.075	60		

Coefficients

D ₈₅ = 3.6803 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

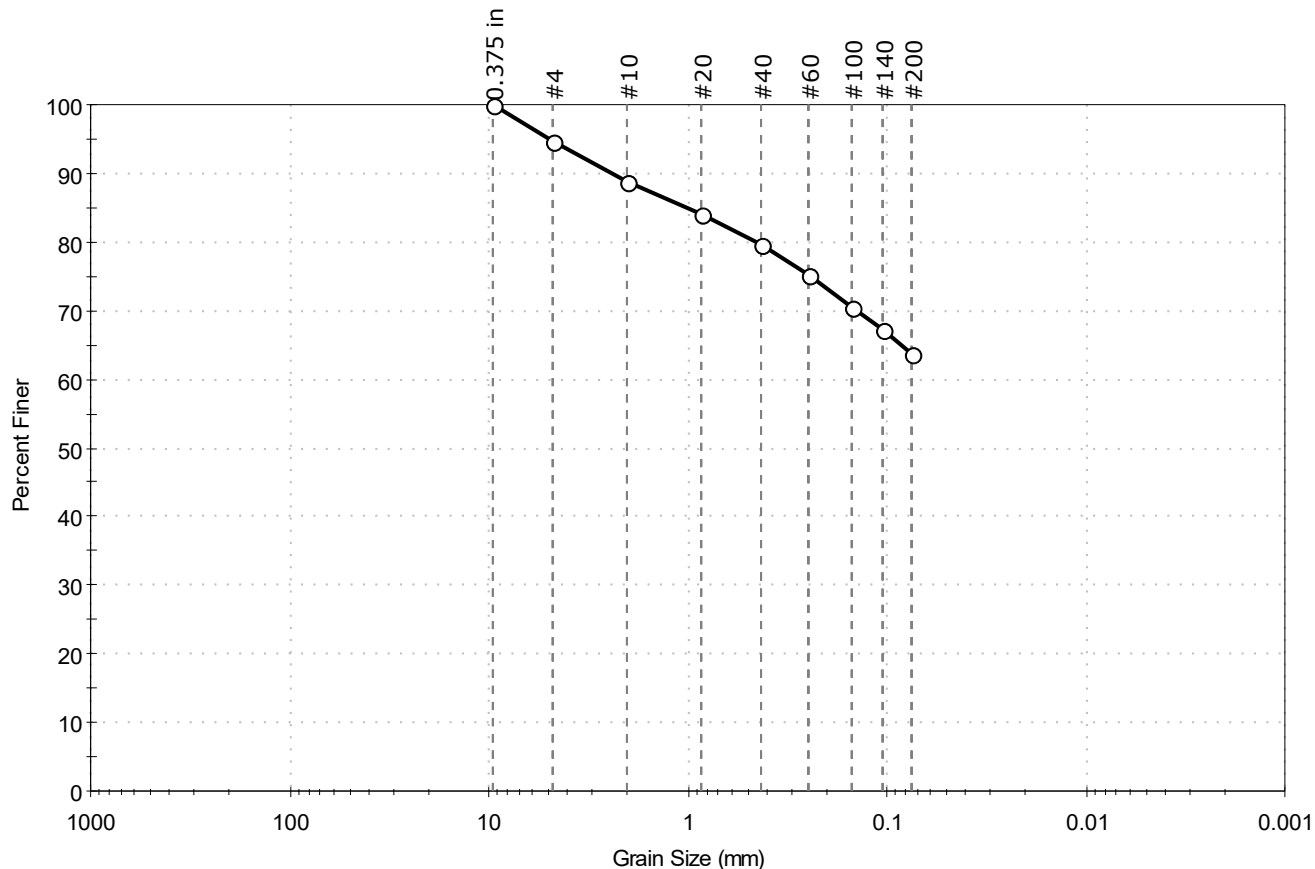
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Bridge (Levenseller Rd)		
Location:	Eddington, ME	Project No:	GTX-313322
Boring ID:	BB-ELER-205	Sample Type:	jar
Sample ID:	3D	Test Date:	03/19/21
Depth :	10-12	Test Id:	613359
Test Comment:	---		
Visual Description:	Moist, gray sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	5.3	31.2	63.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	95		
#10	2.00	89		
#20	0.85	84		
#40	0.42	80		
#60	0.25	75		
#100	0.15	71		
#140	0.11	67		
#200	0.075	64		

Coefficients

D ₈₅ = 1.0065 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

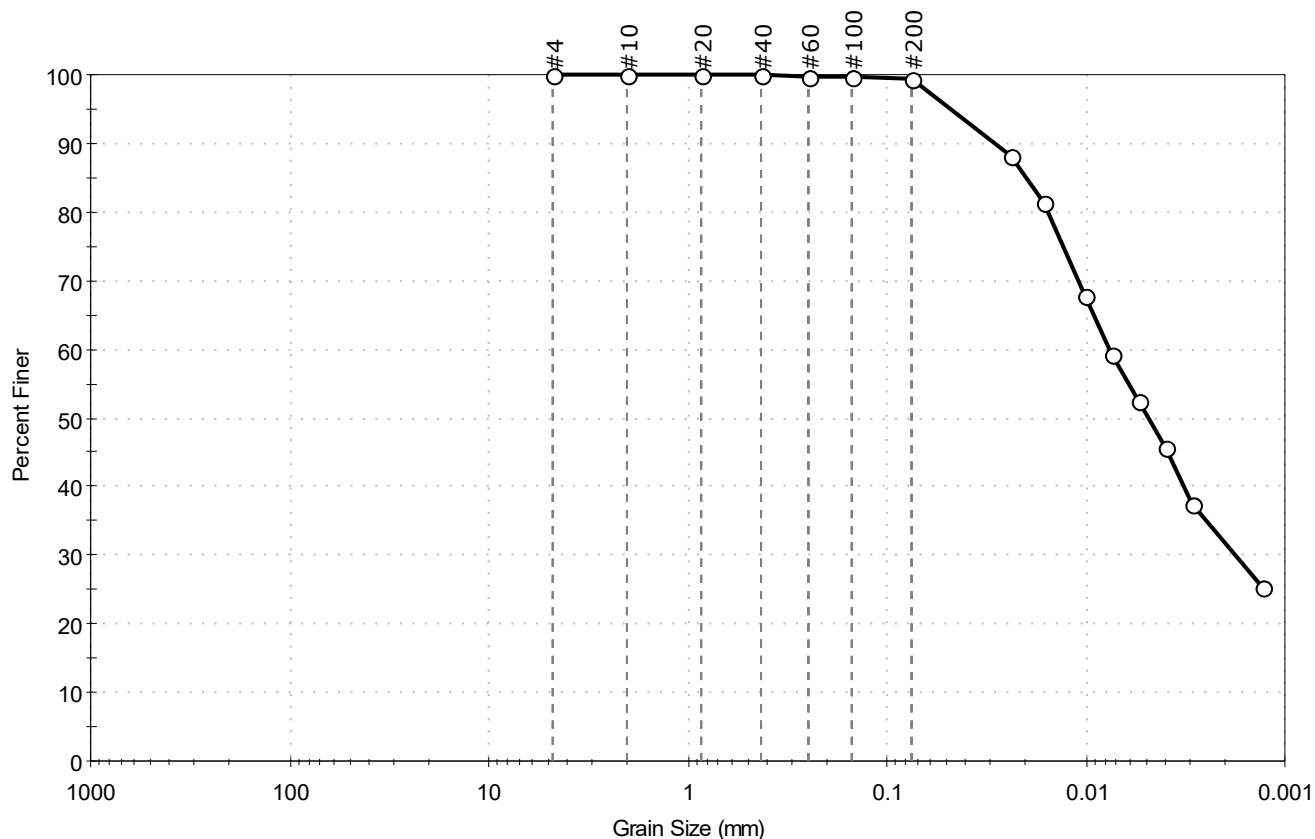
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-308853	
Project: Rt 9/I-395 Connector		
Location: Brewer and Eddington, ME		
Boring ID: HB-BE-108	Sample Type: bag	Tested By: ckg
Sample ID: 2D	Test Date: 07/18/19	Checked By: bfs
Depth: 5-7	Test Id: 513338	
Test Comment: ---		
Visual Description: Moist, mottled reddish brown and dark gray clay		
Sample Comment: ---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	0.7	99.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	100		
#200	0.075	99		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0239	88		
---	0.0163	81		
---	0.0101	68		
---	0.0074	59		
---	0.0055	53		
---	0.0040	46		
---	0.0029	37		
---	0.0013	25		

Coefficients

$D_{85} = 0.0199$ mm $D_{30} = 0.0018$ mm
 $D_{60} = 0.0075$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0049$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

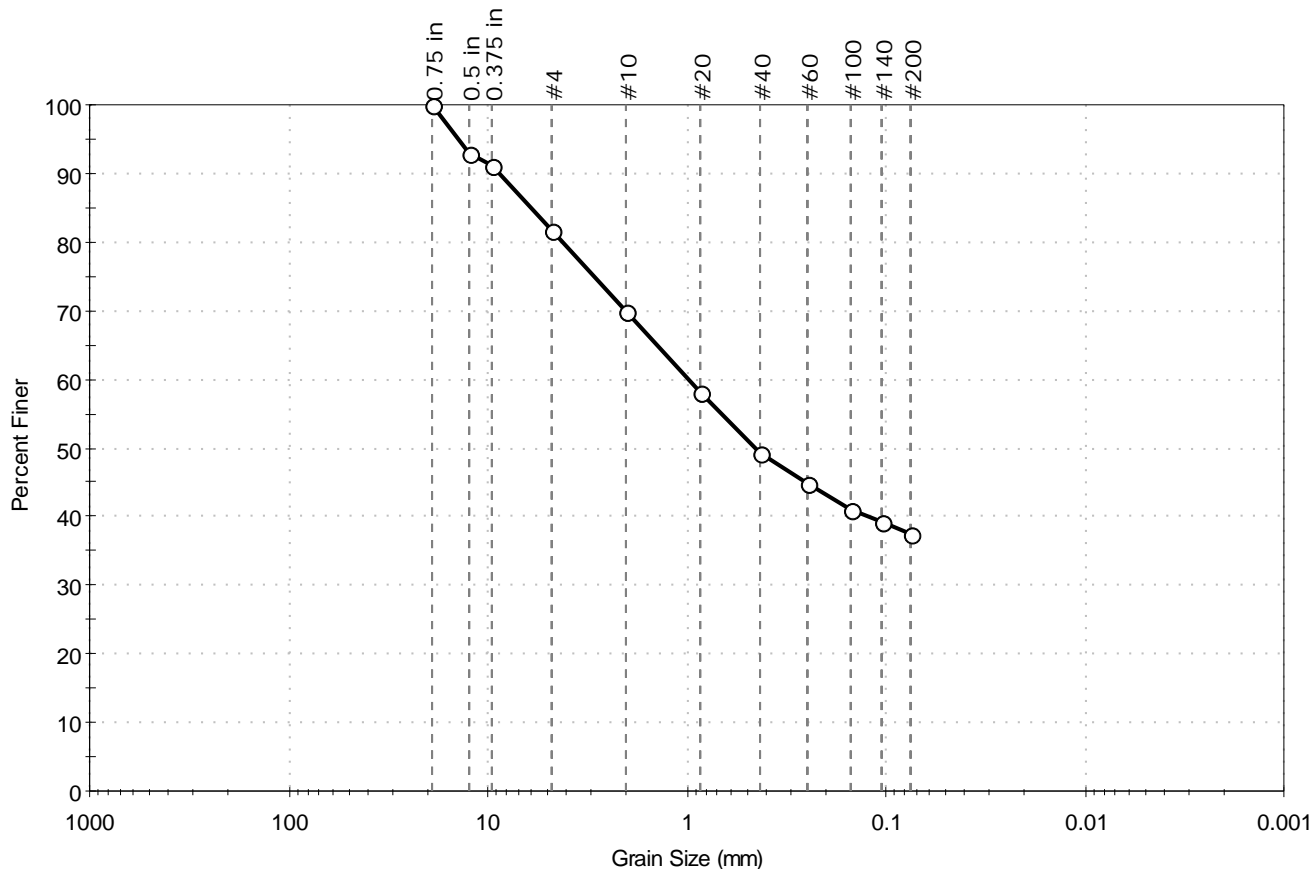
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-115	Sample Type:	jar
Sample ID:	2D	Test Date:	10/12/18
Depth :	2-4 ft	Test Id:	474338
Test Comment:	---		
Visual Description:	Moist, brownish yellow silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	18.2	44.5	37.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	91		
#4	4.75	82		
#10	2.00	70		
#20	0.85	58		
#40	0.425	49		
#60	0.25	45		
#100	0.15	41		
#140	0.11	39		
#200	0.075	37		

Coefficients

D ₈₅ = 6.0196 mm	D ₃₀ = N/A
D ₆₀ = 0.9839 mm	D ₁₅ = N/A
D ₅₀ = 0.4479 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

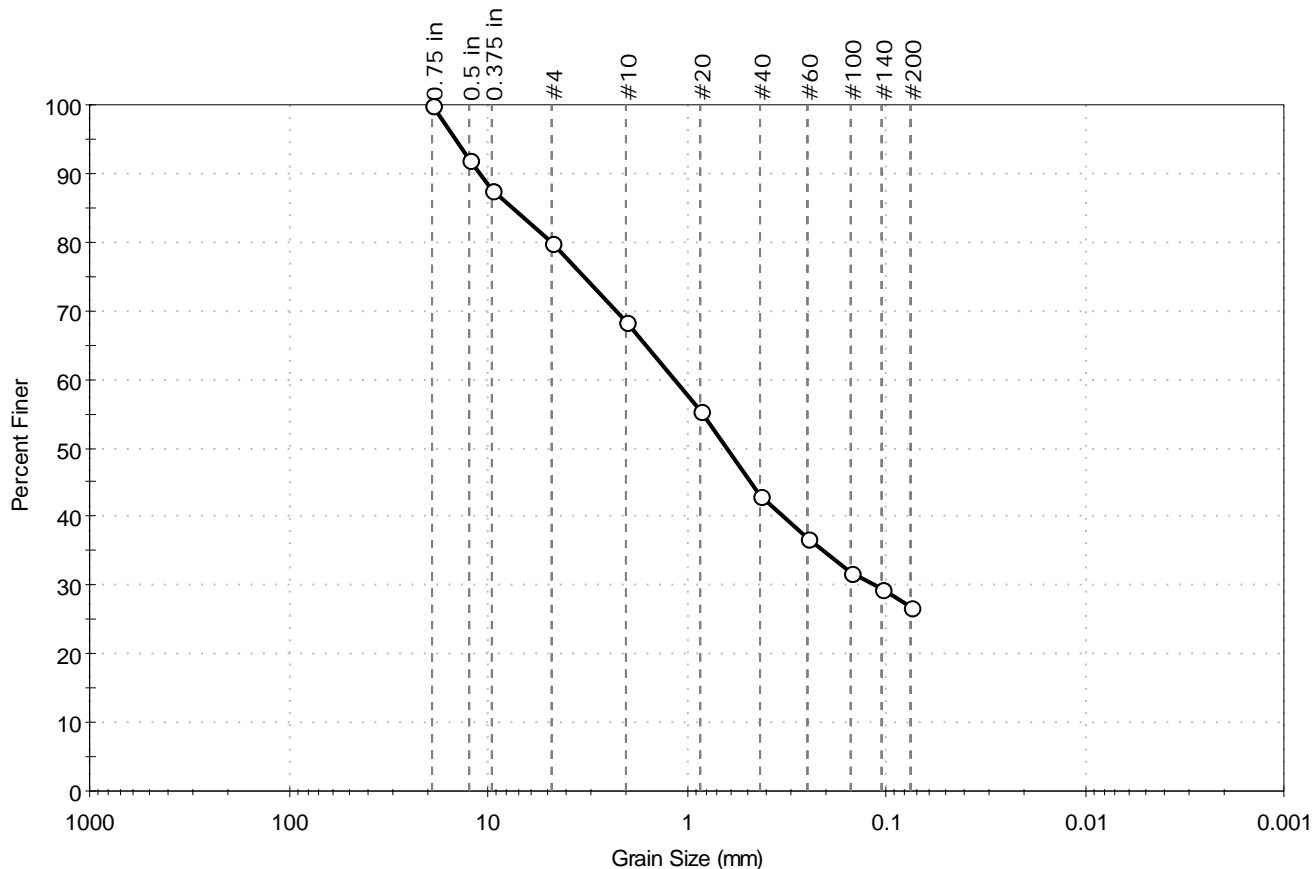
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-115	Sample Type:	jar
Sample ID:	3D	Test Date:	10/12/18
Depth :	5-7 ft	Test Id:	474339
Test Comment:	---		
Visual Description:	Moist, olive brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	20.0	53.2	26.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	92		
0.375 in	9.50	88		
#4	4.75	80		
#10	2.00	68		
#20	0.85	55		
#40	0.425	43		
#60	0.25	37		
#100	0.15	32		
#140	0.11	29		
#200	0.075	27		

Coefficients

$D_{85} = 7.4799 \text{ mm}$ $D_{30} = 0.1147 \text{ mm}$
 $D_{60} = 1.1529 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.6253 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

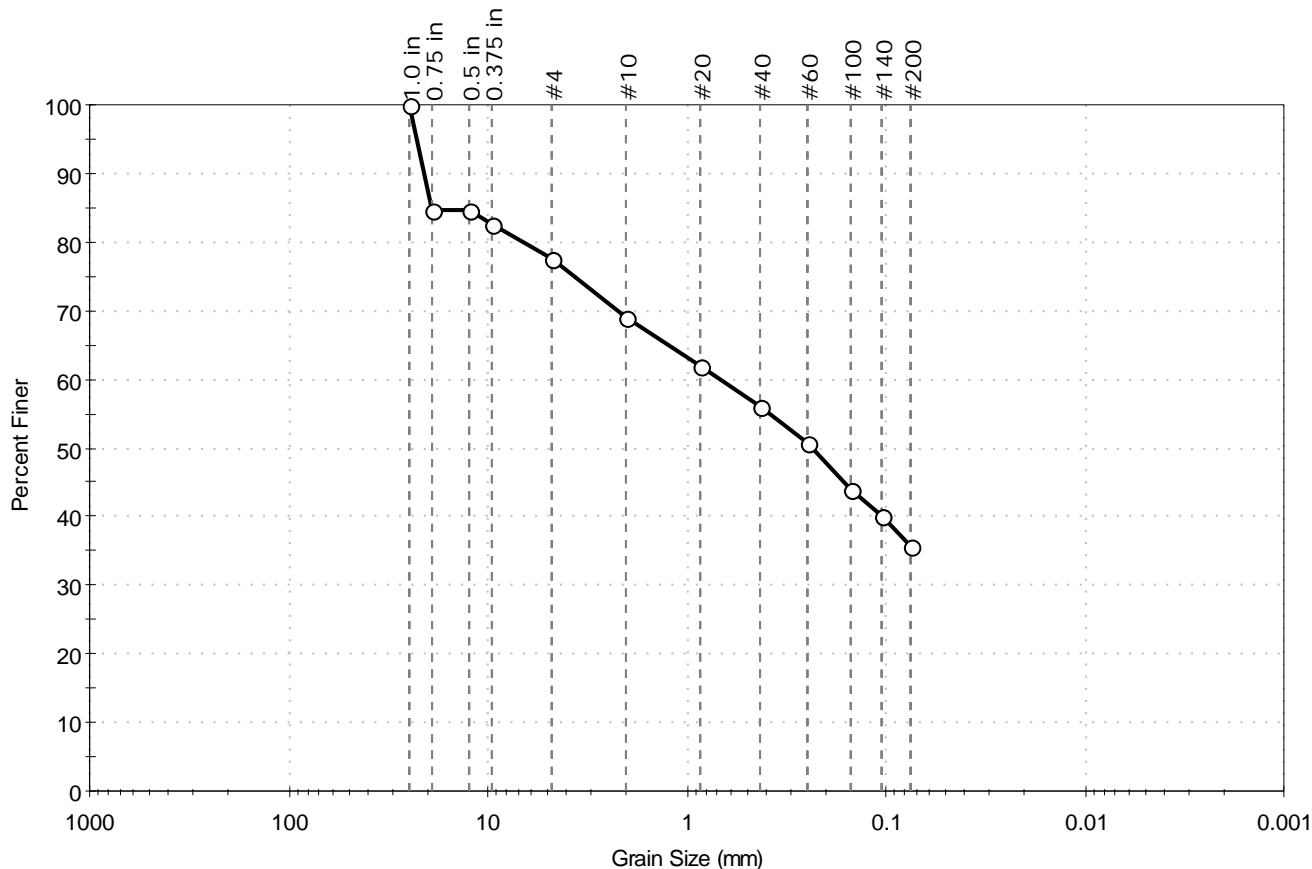
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-115	Sample Type:	jar
Sample ID:	6D	Test Date:	10/12/18
Depth :	14-16 ft	Test Id:	474340
Test Comment:	---		
Visual Description:	Moist, olive silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	22.4	41.9	35.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	85		
0.5 in	12.50	85		
0.375 in	9.50	83		
#4	4.75	78		
#10	2.00	69		
#20	0.85	62		
#40	0.42	56		
#60	0.25	51		
#100	0.15	44		
#140	0.11	40		
#200	0.075	36		

Coefficients

D ₈₅ = 19.1608 mm	D ₃₀ = N/A
D ₆₀ = 0.6725 mm	D ₁₅ = N/A
D ₅₀ = 0.2361 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

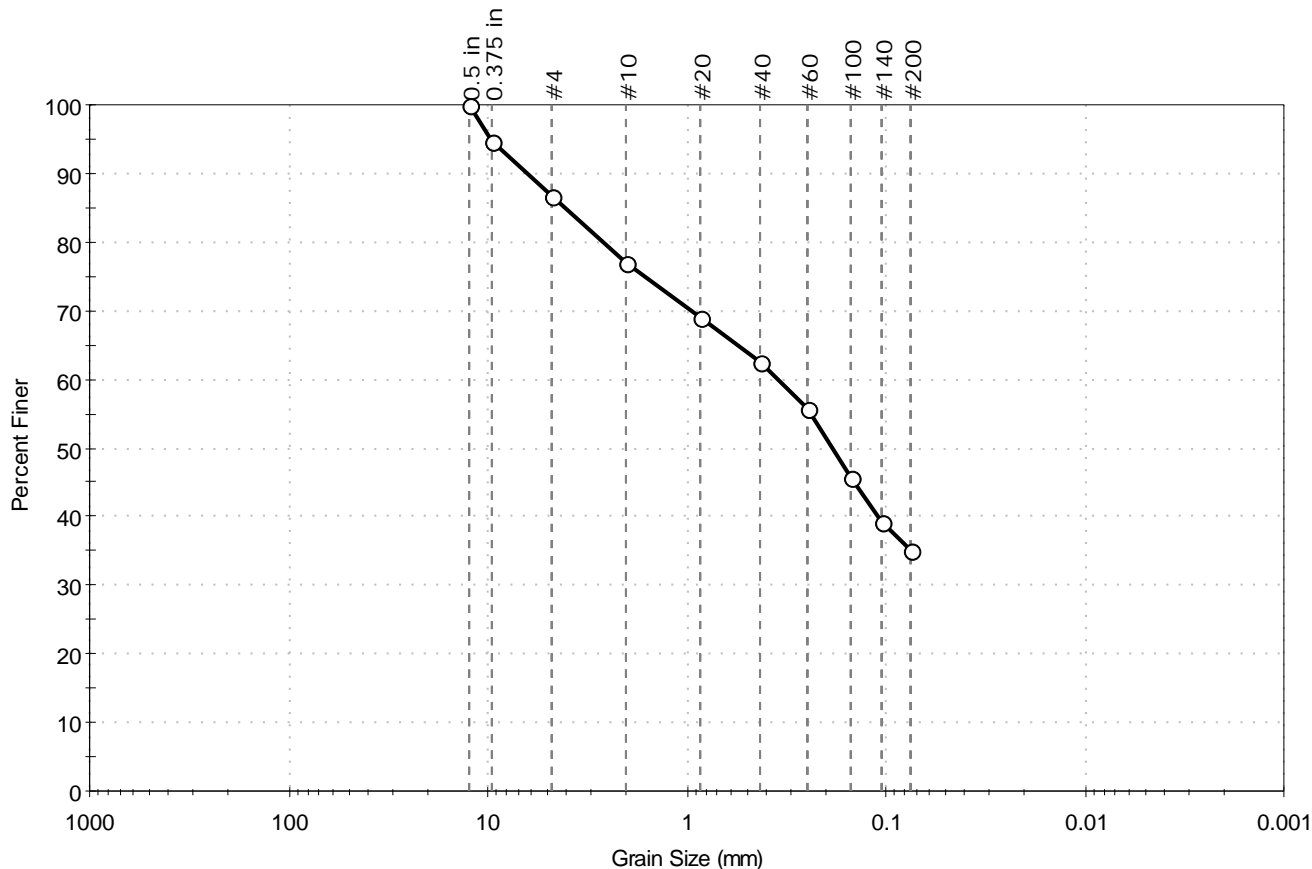
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-115	Sample Type:	jar
Sample ID:	8D	Test Date:	10/12/18
Depth :	18-19.6 ft	Test Id:	474341
Test Comment:	---		
Visual Description:	Moist, olive silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	13.3	51.6	35.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	95		
#4	4.75	87		
#10	2.00	77		
#20	0.85	69		
#40	0.42	63		
#60	0.25	56		
#100	0.15	46		
#140	0.11	39		
#200	0.075	35		

Coefficients

D ₈₅ = 4.0985 mm	D ₃₀ = N/A
D ₆₀ = 0.3490 mm	D ₁₅ = N/A
D ₅₀ = 0.1857 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

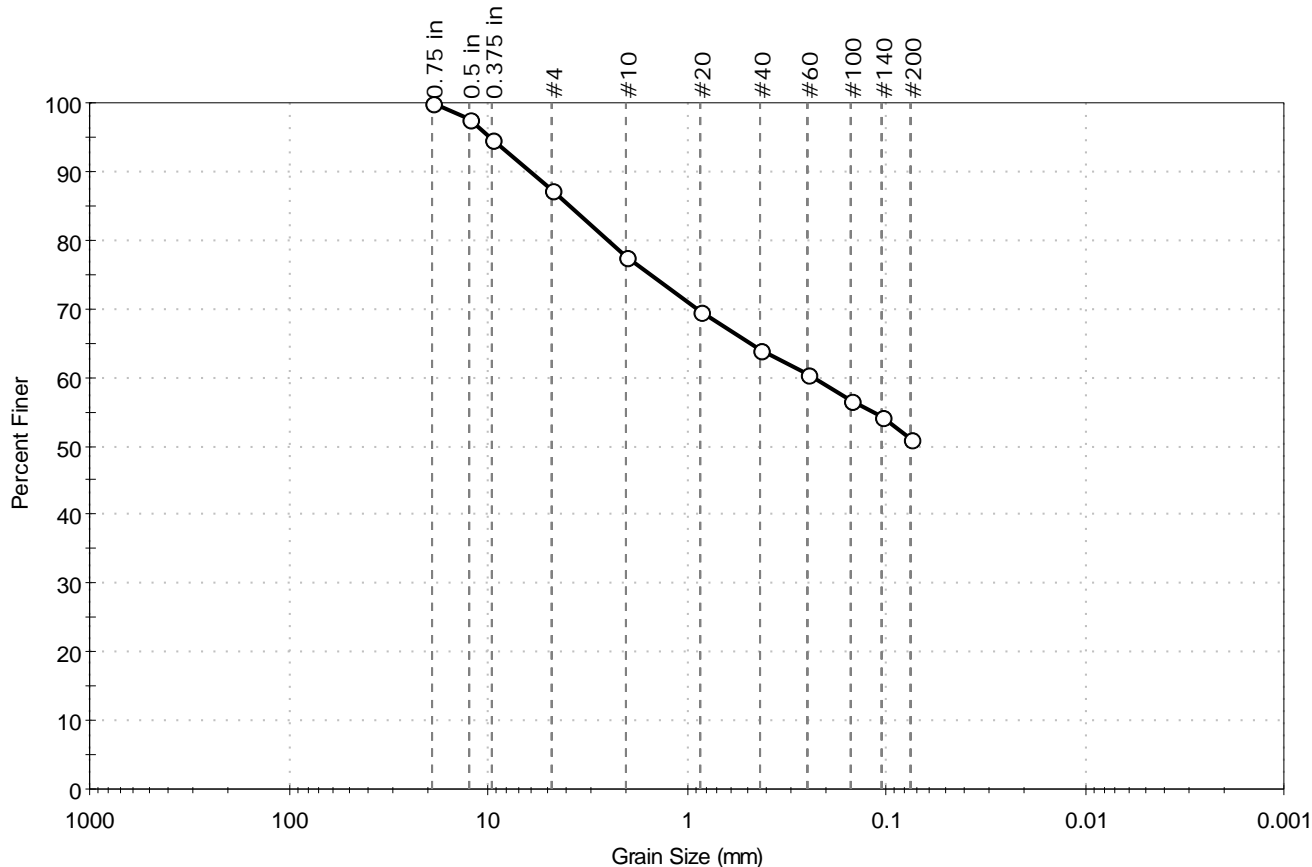
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-116	Sample Type:	jar
Sample ID:	3D	Test Date:	10/12/18
Depth :	5-7 ft	Test Id:	474342
Test Comment:	---		
Visual Description:	Moist, olive sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	12.7	36.2	51.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	95		
#4	4.75	87		
#10	2.00	78		
#20	0.85	70		
#40	0.42	64		
#60	0.25	61		
#100	0.15	57		
#140	0.11	54		
#200	0.075	51		

Coefficients

D ₈₅ = 3.8808 mm	D ₃₀ = N/A
D ₆₀ = 0.2327 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

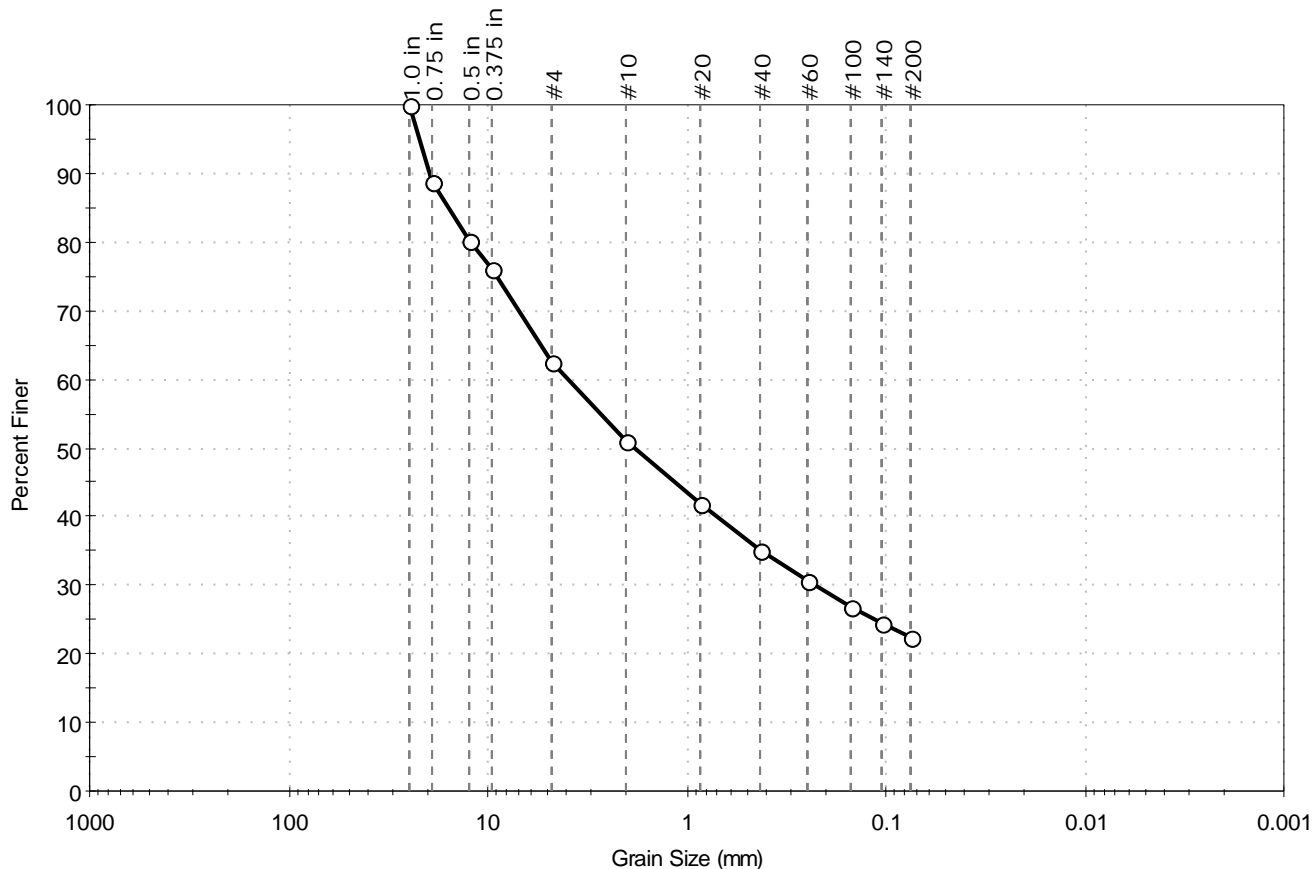
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-116	Sample Type:	jar
Sample ID:	4D	Test Date:	10/12/18
Depth :	10-11.8 ft	Test Id:	474343
Test Comment:	---		
Visual Description:	Moist, olive silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	37.3	40.2	22.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	89		
0.5 in	12.50	80		
0.375 in	9.50	76		
#4	4.75	63		
#10	2.00	51		
#20	0.85	42		
#40	0.42	35		
#60	0.25	31		
#100	0.15	27		
#140	0.11	25		
#200	0.075	22		

Coefficients

D ₈₅ = 15.7055 mm	D ₃₀ = 0.2278 mm
D ₆₀ = 3.8974 mm	D ₁₅ = N/A
D ₅₀ = 1.8216 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

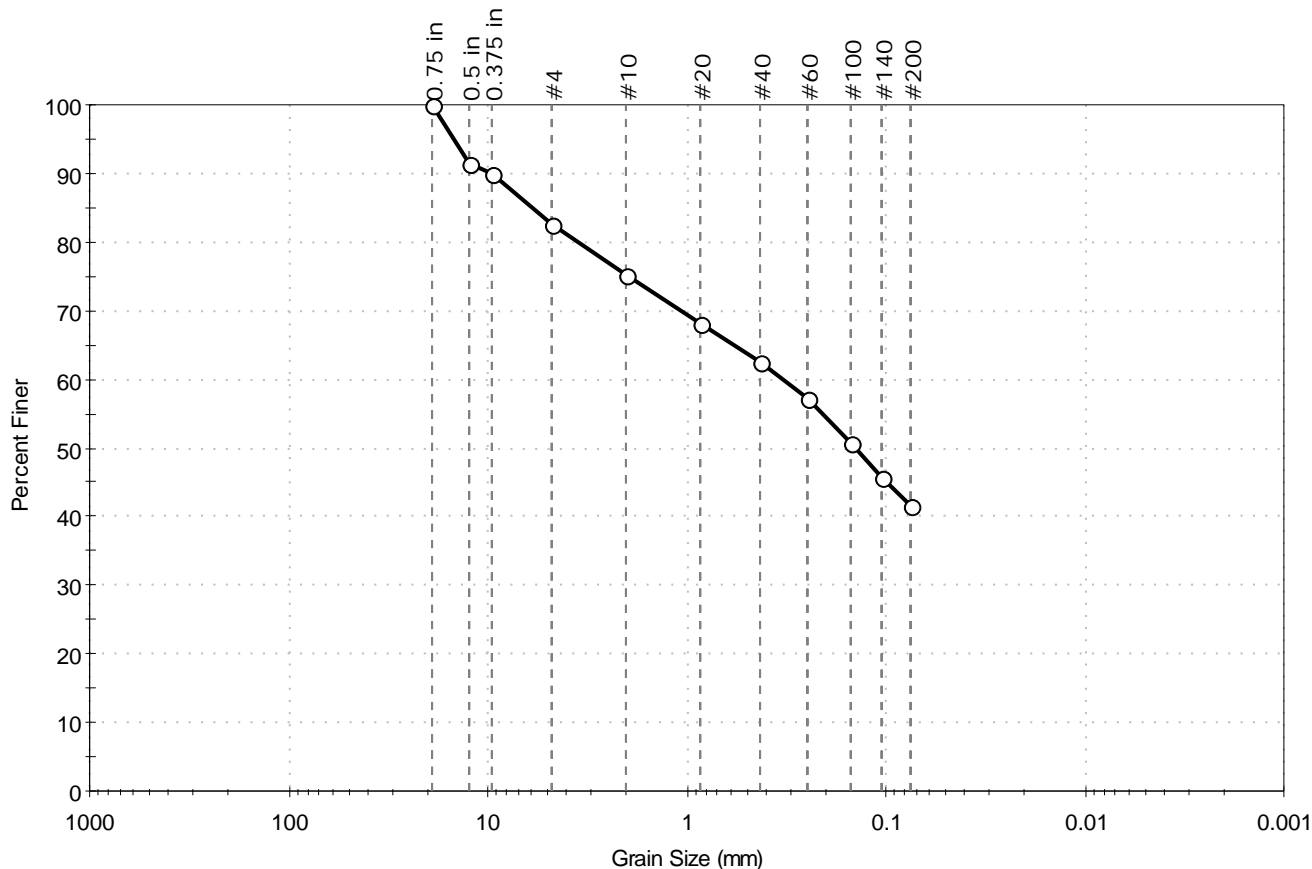
AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-116	Sample Type:	jar
Sample ID:	5D	Test Date:	10/12/18
Depth :	15-16.4 ft	Test Id:	474344
Test Comment:	---		
Visual Description:	Moist, olive silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	17.3	41.2	41.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	92		
0.375 in	9.50	90		
#4	4.75	83		
#10	2.00	75		
#20	0.85	68		
#40	0.42	63		
#60	0.25	57		
#100	0.15	51		
#140	0.11	46		
#200	0.075	41		

Coefficients

D ₈₅ = 5.9240 mm	D ₃₀ = N/A
D ₆₀ = 0.3313 mm	D ₁₅ = N/A
D ₅₀ = 0.1411 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

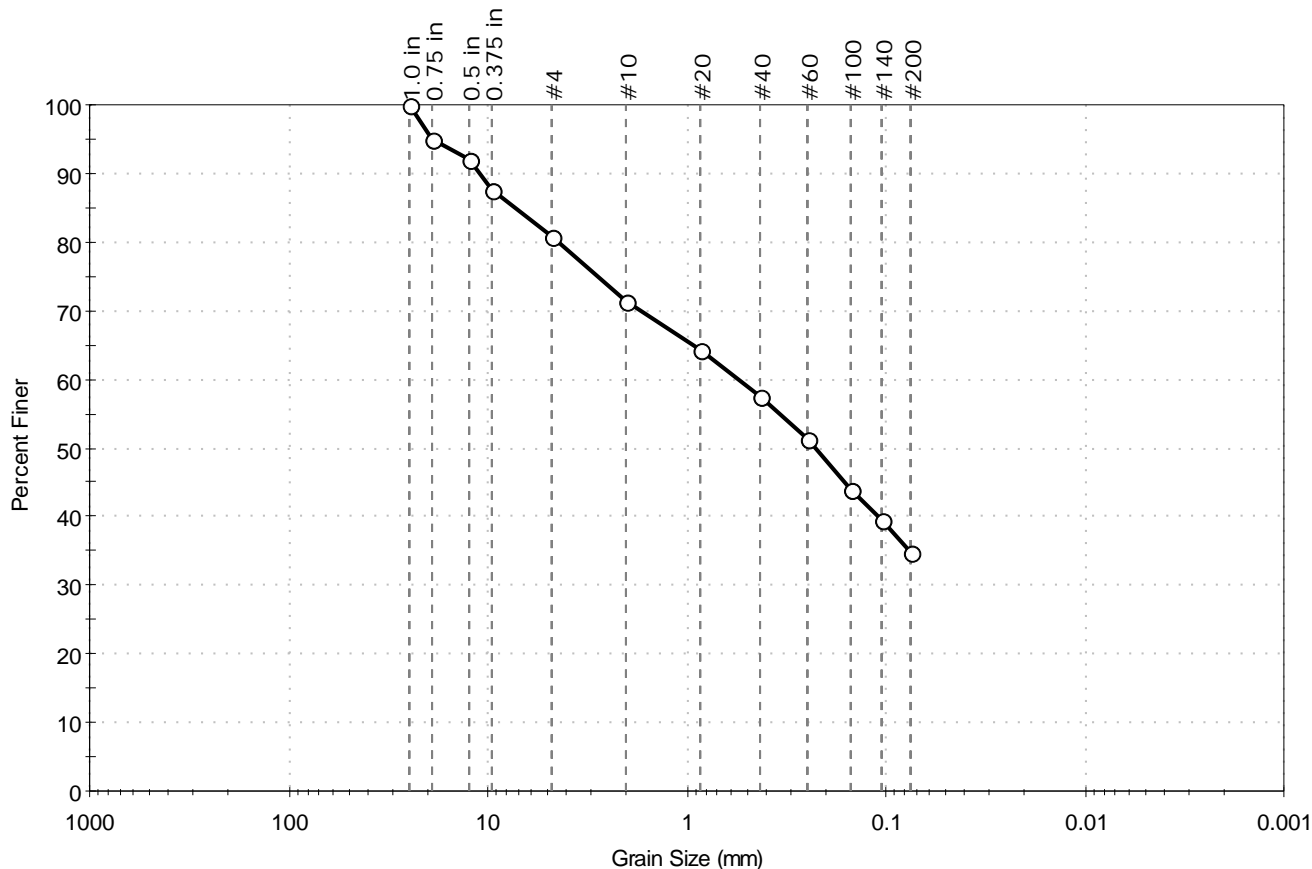
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-116	Sample Type:	jar
Sample ID:	6D	Test Date:	10/12/18
Depth :	18-20 ft	Test Id:	474345
Test Comment:	---		
Visual Description:	Moist, olive silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	19.3	46.0	34.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	95		
0.5 in	12.50	92		
0.375 in	9.50	87		
#4	4.75	81		
#10	2.00	71		
#20	0.85	64		
#40	0.42	58		
#60	0.25	51		
#100	0.15	44		
#140	0.11	40		
#200	0.075	35		

Coefficients

D ₈₅ = 7.3740 mm	D ₃₀ = N/A
D ₆₀ = 0.5451 mm	D ₁₅ = N/A
D ₅₀ = 0.2271 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

AASHTO Silty Gravel and Sand (A-2-4 (0))

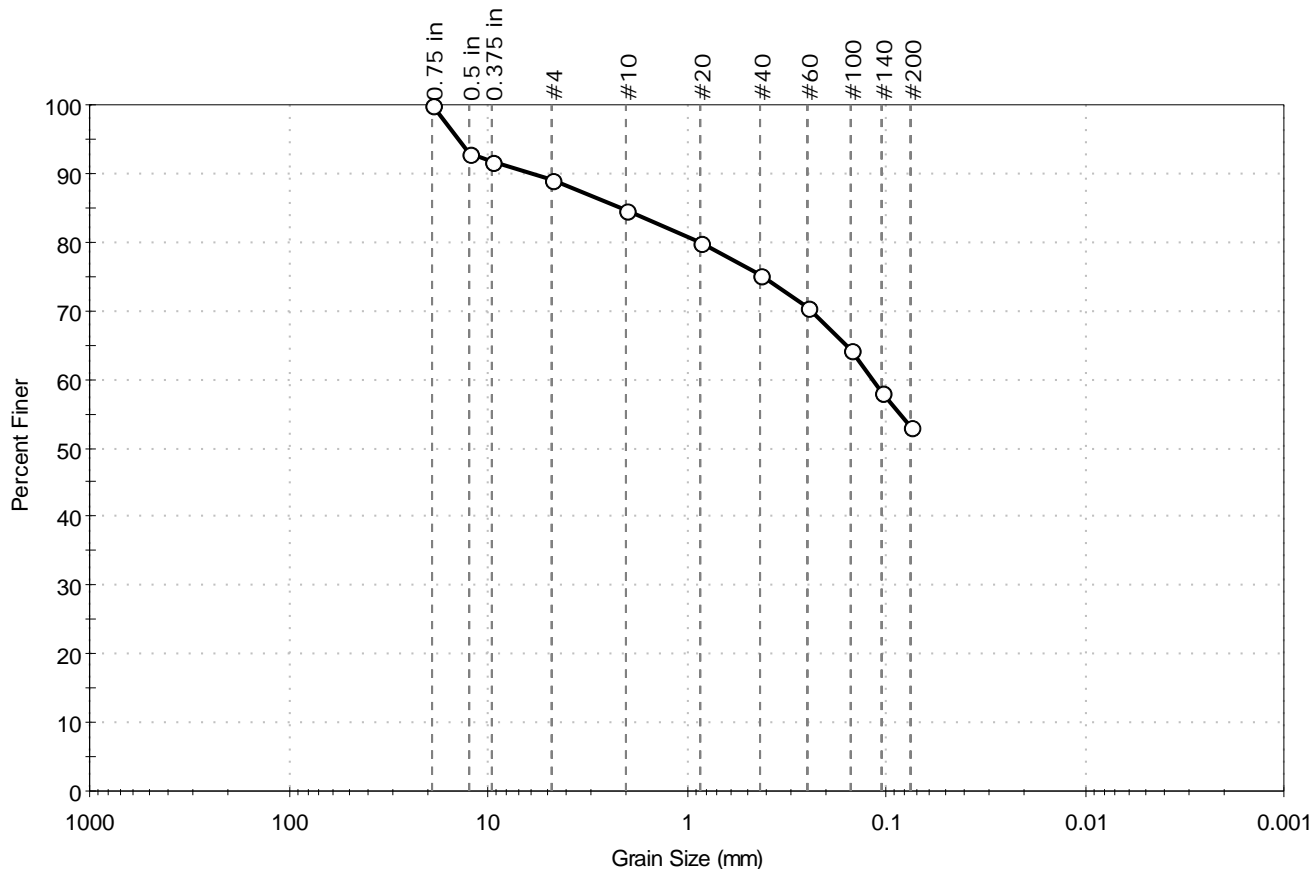
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	HB-BE-116	Sample Type:	jar	Tested By:	GA
Sample ID:	8D	Test Date:	10/12/18	Checked By:	emm
Depth :	22-24 ft	Test Id:	474346		
Test Comment:	---				
Visual Description:	Moist, olive sandy silt				
Sample Comment:	---				

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	10.9	35.9	53.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	92		
#4	4.75	89		
#10	2.00	85		
#20	0.85	80		
#40	0.42	75		
#60	0.25	71		
#100	0.15	64		
#140	0.11	58		
#200	0.075	53		

Coefficients

D ₈₅ = 2.1502 mm	D ₃₀ = N/A
D ₆₀ = 0.1176 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

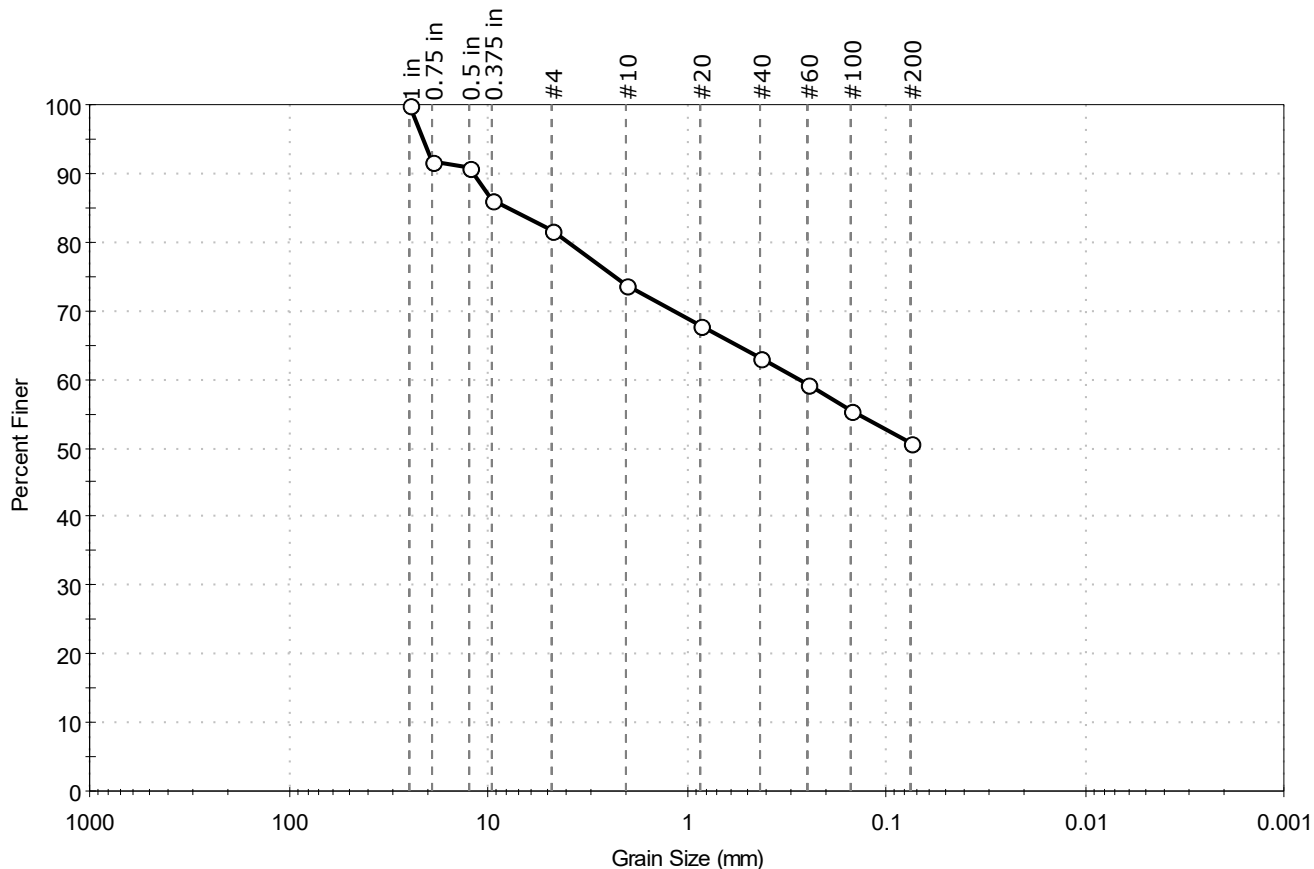
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-117	Sample Type:	bag
Sample ID:	4D	Test Date:	07/15/19
Depth :	15-17	Test Id:	513342
Test Comment:	---		
Visual Description:	Moist, olive sandy clay with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	18.4	30.7	50.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	92		
0.5 in	12.50	91		
0.375 in	9.50	86		
#4	4.75	82		
#10	2.00	74		
#20	0.85	68		
#40	0.42	63		
#60	0.25	59		
#100	0.15	55		
#200	0.075	51		

Coefficients

D ₈₅ = 7.9158 mm	D ₃₀ = N/A
D ₆₀ = 0.2715 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

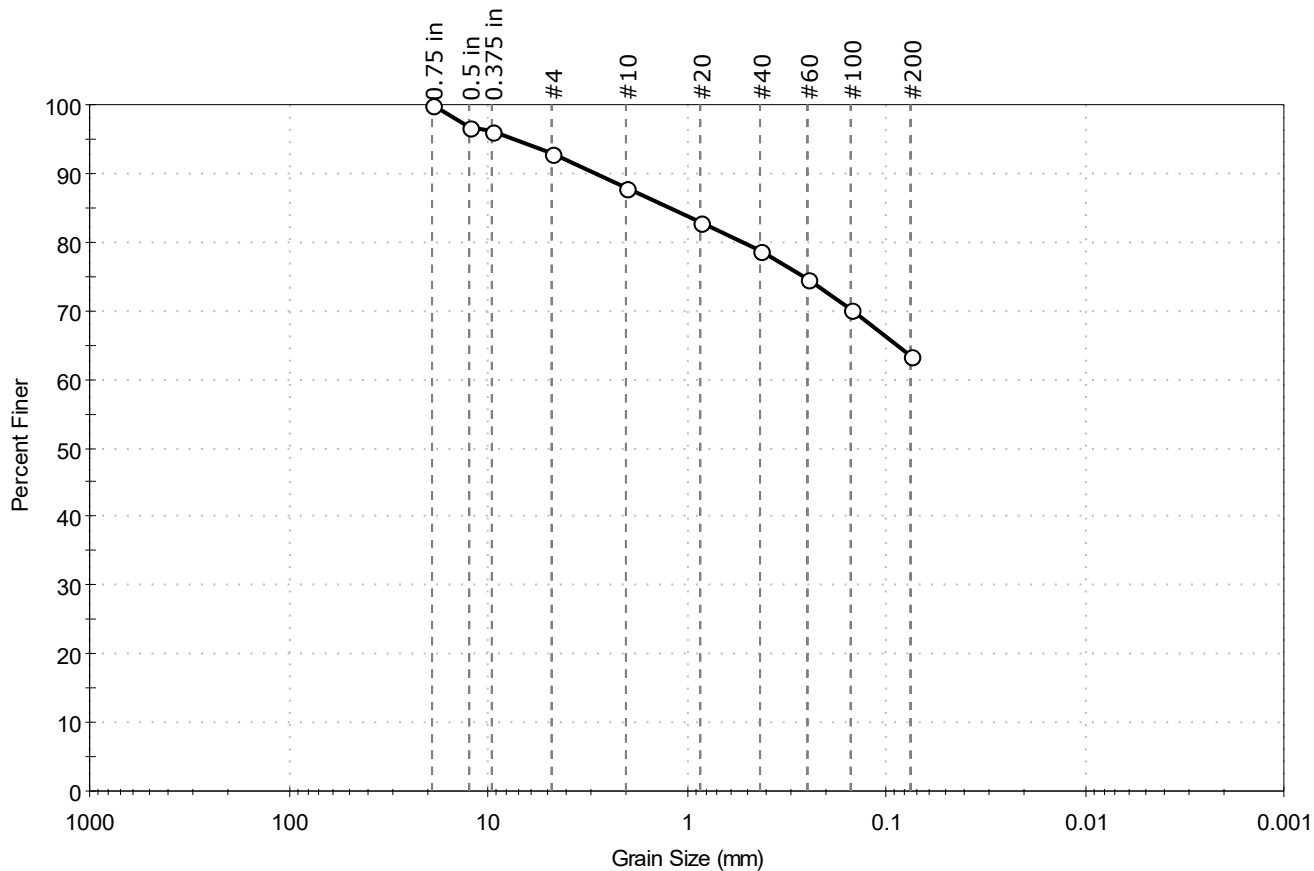
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-117	Sample Type:	bag
Sample ID:	8D	Test Date:	07/19/19
Depth :	35-35.9	Test Id:	513343
Test Comment:	---		
Visual Description:	Moist, dark olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	7.1	29.4	63.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	97		
0.375 in	9.50	96		
#4	4.75	93		
#10	2.00	88		
#20	0.85	83		
#40	0.42	79		
#60	0.25	75		
#100	0.15	70		
#200	0.075	64		

Coefficients

D ₈₅ = 1.2071 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

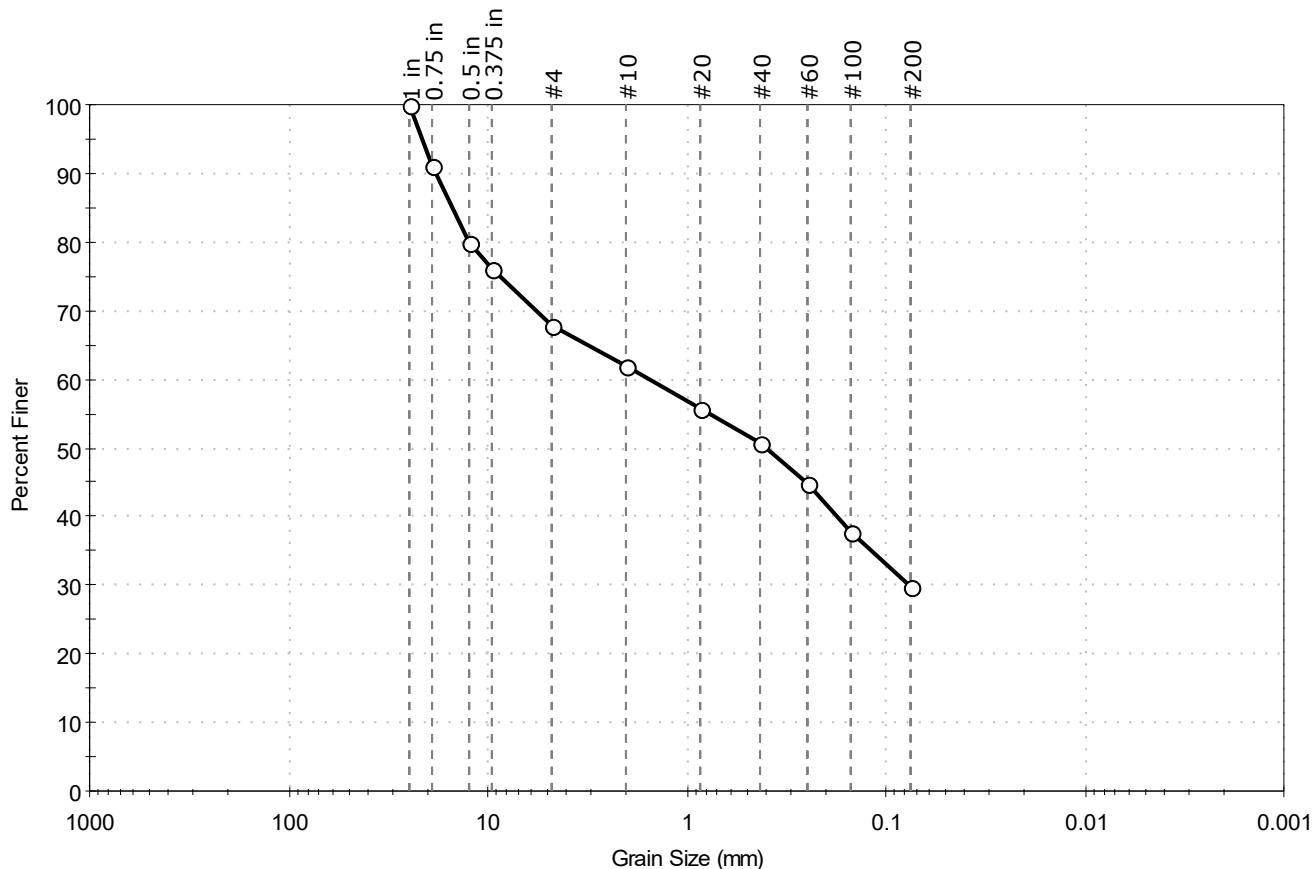
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-120	Sample Type:	bag
Sample ID:	4D	Test Date:	07/19/19
Depth :	15-17	Checked By:	bfs
		Test Id:	513344
Test Comment:	---		
Visual Description:	Moist, yellowish brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	32.0	38.2	29.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	91		
0.5 in	12.50	80		
0.375 in	9.50	76		
#4	4.75	68		
#10	2.00	62		
#20	0.85	56		
#40	0.42	51		
#60	0.25	45		
#100	0.15	38		
#200	0.075	30		

Coefficients

$D_{85} = 15.1017 \text{ mm}$ $D_{30} = 0.0761 \text{ mm}$
 $D_{60} = 1.5322 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.4010 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

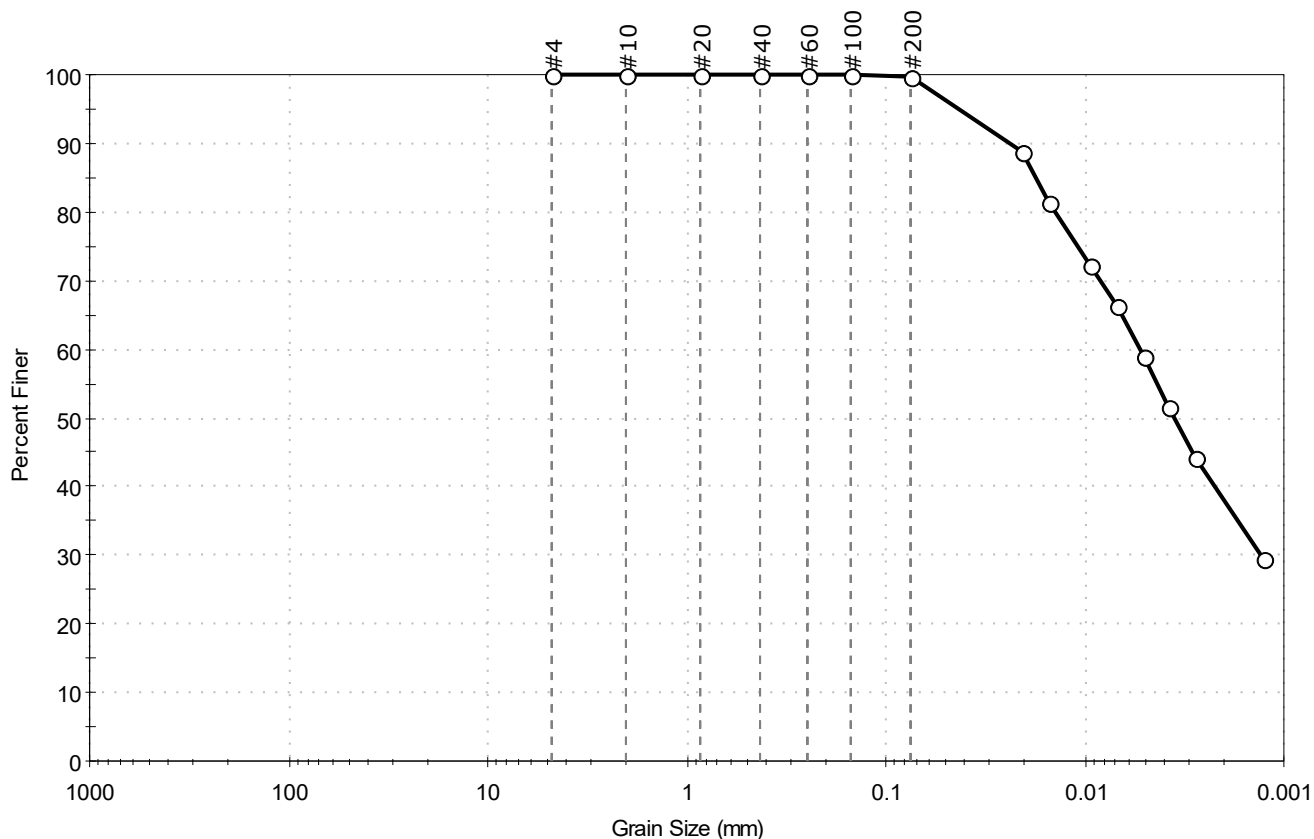
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-123	Sample Type:	bag
Sample ID:	1D	Test Date:	07/18/19
Depth :	0-2	Test Id:	513341
Test Comment:	---		
Visual Description:	Moist, dark olive clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	0.2	99.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	100		
#200	0.075	100		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0205	89		
---	0.0151	81		
---	0.0094	72		
---	0.0069	66		
---	0.0051	59		
---	0.0038	52		
---	0.0028	44		
---	0.0013	30		

Coefficients

$D_{85} = 0.0176$ mm $D_{30} = 0.0013$ mm
 $D_{60} = 0.0053$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.0035$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

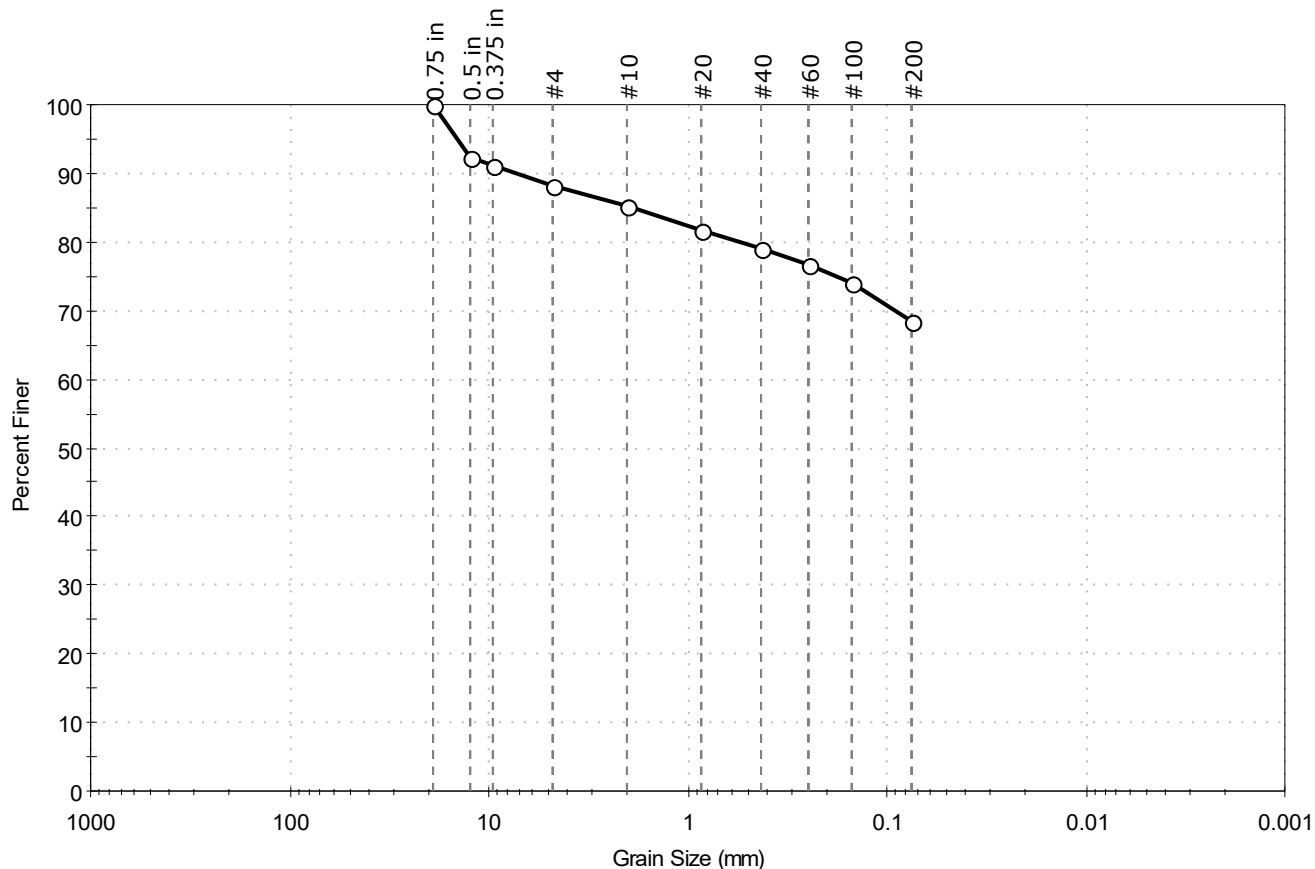
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-131	Sample Type:	bag
Sample ID:	5D	Test Date:	07/15/19
Depth :	20-22	Test Id:	513345
Test Comment:	---		
Visual Description:	Moist, light olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	11.7	19.8	68.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	92		
0.375 in	9.50	91		
#4	4.75	88		
#10	2.00	85		
#20	0.85	82		
#40	0.42	79		
#60	0.25	77		
#100	0.15	74		
#200	0.075	69		

Coefficients

D ₈₅ = 1.9377 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

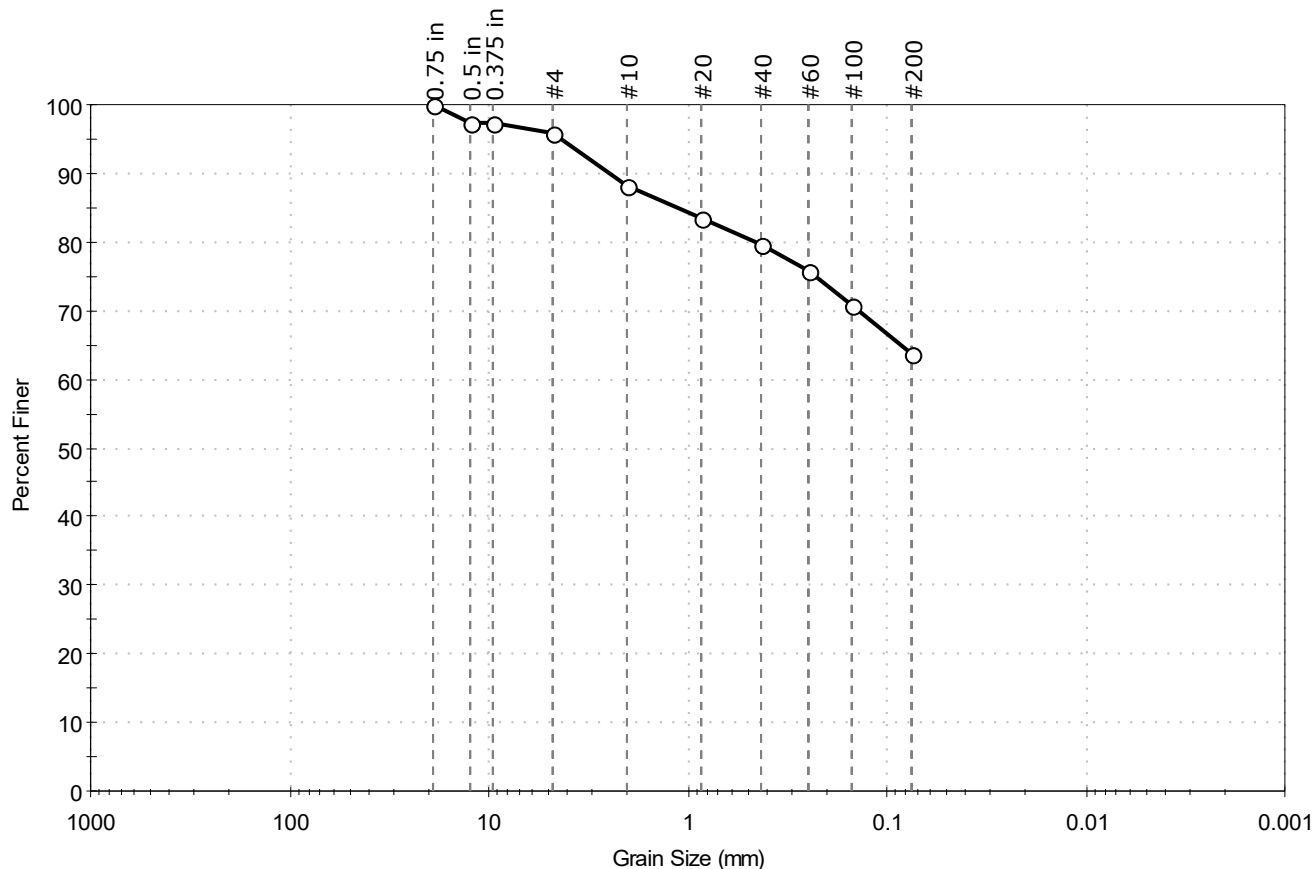
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-131	Sample Type:	bag
Sample ID:	8D	Test Date:	07/19/19
Depth :	30-32	Test Id:	513346
Test Comment:	---		
Visual Description:	Moist, light olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	4.3	32.1	63.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	97		
0.375 in	9.50	97		
#4	4.75	96		
#10	2.00	88		
#20	0.85	83		
#40	0.42	80		
#60	0.25	76		
#100	0.15	71		
#200	0.075	64		

Coefficients

D ₈₅ = 1.1196 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

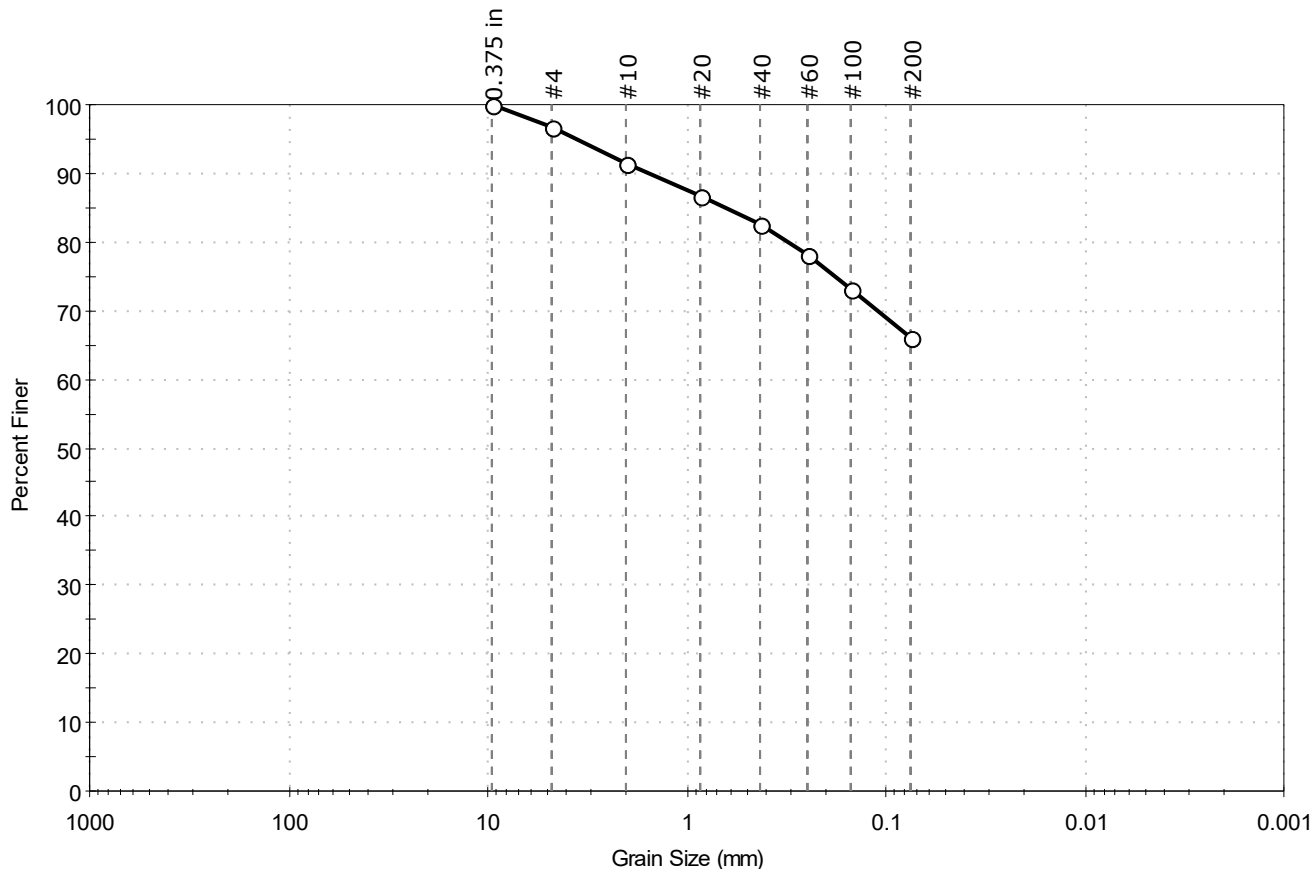
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-308853	
Project: Rt 9/I-395 Connector		
Location: Brewer and Eddington, ME		
Boring ID: HB-BE-132	Sample Type: bag	Tested By: ckg
Sample ID: 3D	Test Date: 07/19/19	Checked By: bfs
Depth: 10-12	Test Id: 513335	
Test Comment: ---		
Visual Description: Moist, olive brown sandy silt		
Sample Comment: ---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	3.3	30.7	66.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	97		
#10	2.00	91		
#20	0.85	87		
#40	0.42	83		
#60	0.25	78		
#100	0.15	73		
#200	0.075	66		

Coefficients

$D_{85} = 0.6444$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

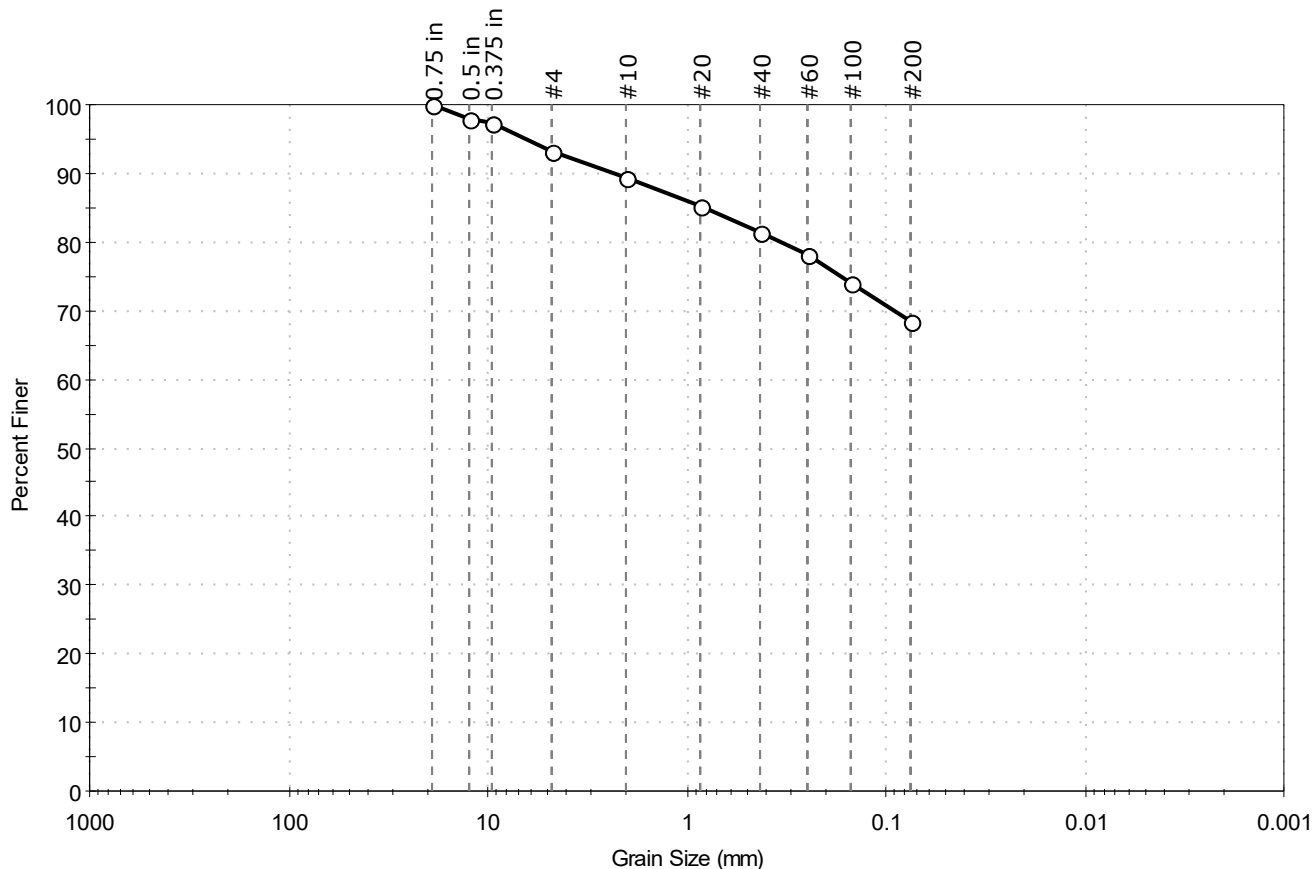
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-132	Sample Type:	bag
Sample ID:	7D	Test Date:	07/19/19
Depth :	30-32	Test Id:	513336
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.7	24.9	68.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	97		
#4	4.75	93		
#10	2.00	89		
#20	0.85	85		
#40	0.42	82		
#60	0.25	78		
#100	0.15	74		
#200	0.075	68		

Coefficients

D ₈₅ = 0.8015 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

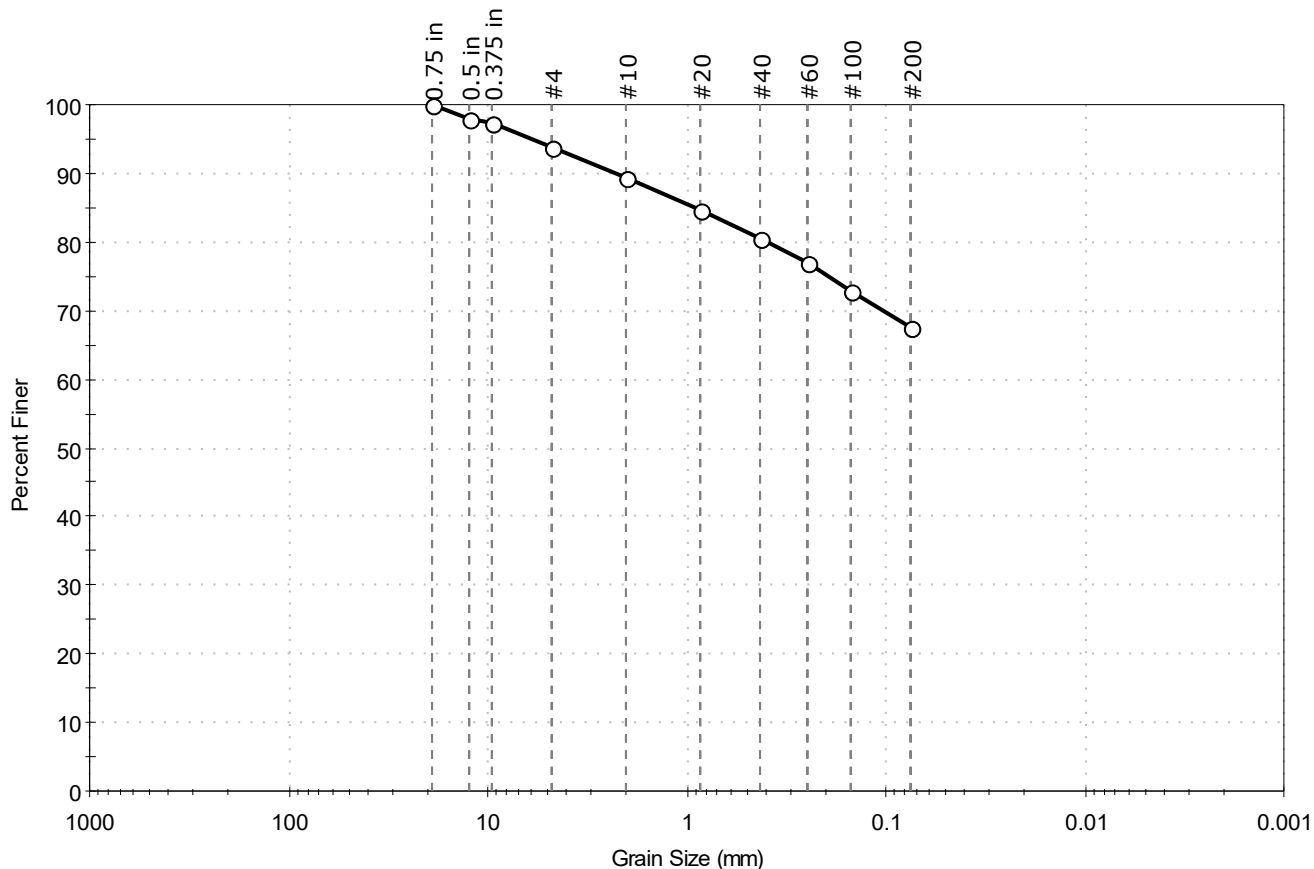
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-132	Sample Type:	bag
Sample ID:	12D	Test Date:	07/15/19
Depth :	48.5-50.5	Test Id:	513337
Test Comment:	---		
Visual Description:	Moist, dark olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.1	26.4	67.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	97		
#4	4.75	94		
#10	2.00	89		
#20	0.85	85		
#40	0.42	81		
#60	0.25	77		
#100	0.15	73		
#200	0.075	67		

Coefficients

$D_{85} = 0.9089 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

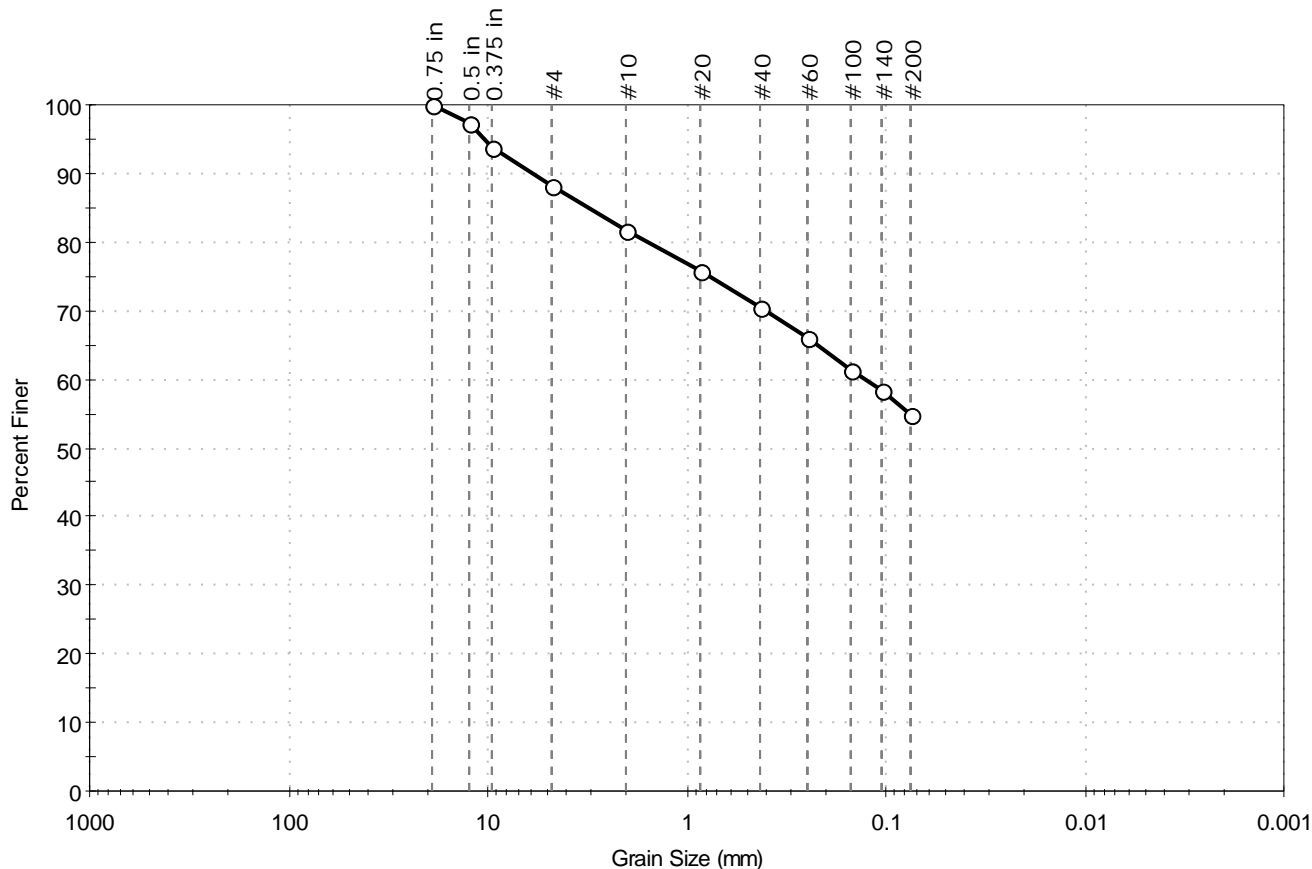
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : **ROUNDED**
 Sand/Gravel Hardness : **HARD**

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-133	Sample Type:	jar
Sample ID:	3D	Test Date:	10/12/18
Depth :	5-7 ft	Test Id:	474350
Test Comment:	---		
Visual Description:	Moist, olive gray sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	11.7	33.5	54.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	97		
0.375 in	9.50	94		
#4	4.75	88		
#10	2.00	82		
#20	0.85	76		
#40	0.42	70		
#60	0.25	66		
#100	0.15	61		
#140	0.11	58		
#200	0.075	55		

Coefficients

D ₈₅ = 3.0756 mm	D ₃₀ = N/A
D ₆₀ = 0.1278 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

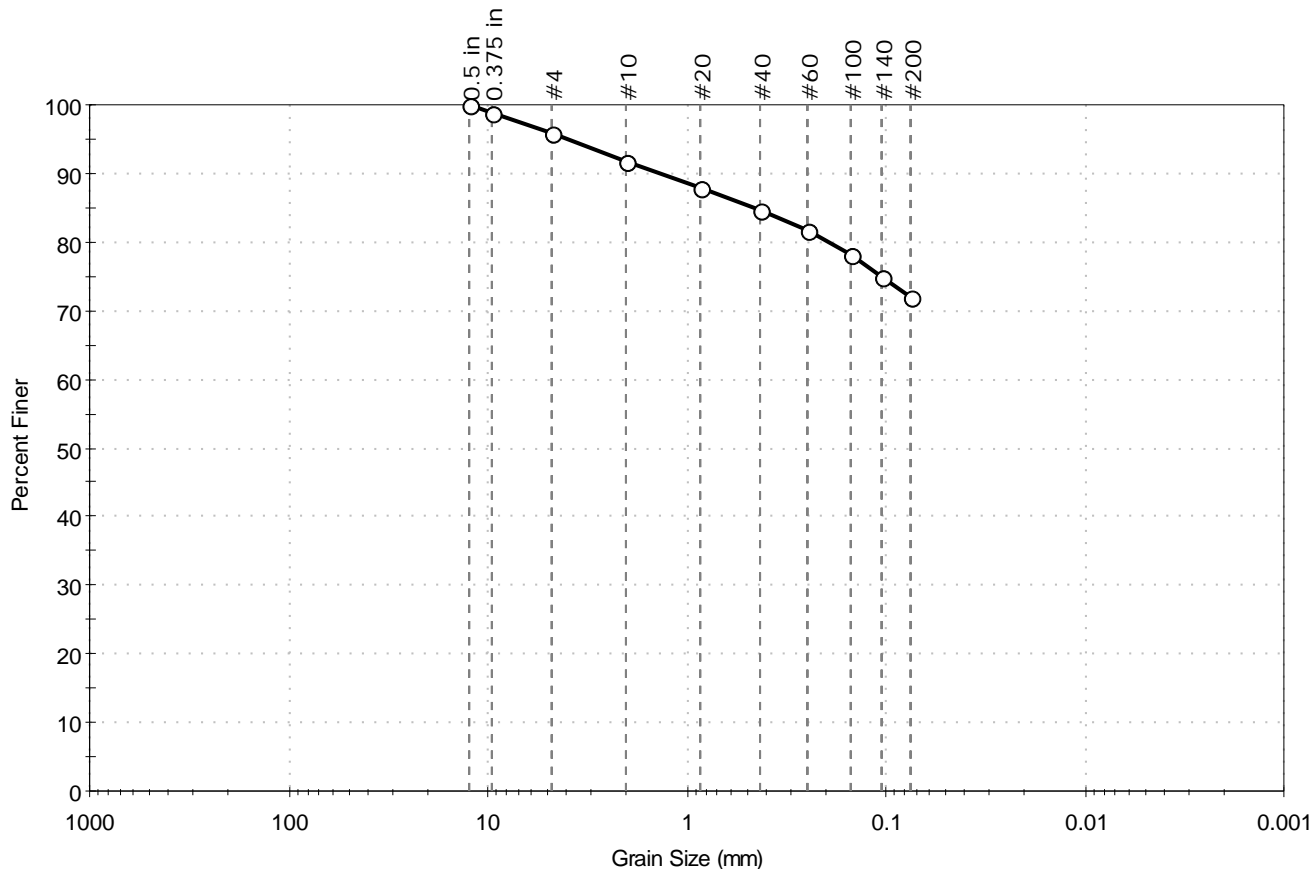
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-133	Sample Type:	jar
Sample ID:	5D	Test Date:	10/12/18
Depth :	15-17 ft	Test Id:	474351
Test Comment:	---		
Visual Description:	Moist, olive gray clay with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	4.2	23.7	72.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	99		
#4	4.75	96		
#10	2.00	92		
#20	0.85	88		
#40	0.42	85		
#60	0.25	82		
#100	0.15	78		
#140	0.11	75		
#200	0.075	72		

Coefficients

$D_{85} = 0.4536 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

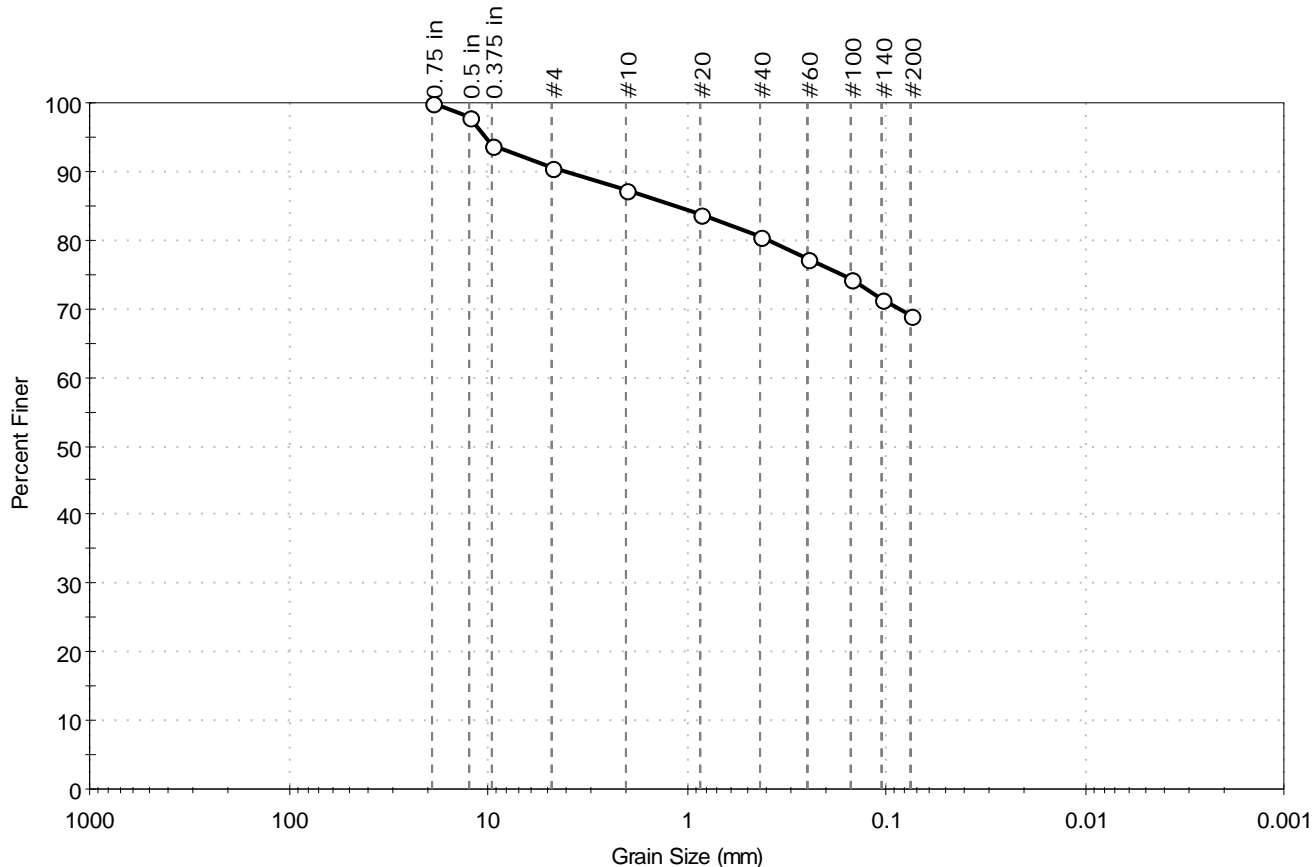
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	HB-BE-133	Sample Type:	jar	Tested By:	GA
Sample ID:	7D	Test Date:	09/28/18	Checked By:	emm
Depth :	25-27 ft	Test Id:	474352		
Test Comment:	---				
Visual Description:	Moist, dark gray sandy clay				
Sample Comment:	---				

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	9.4	21.5	69.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	94		
#4	4.75	91		
#10	2.00	87		
#20	0.85	84		
#40	0.42	80		
#60	0.25	77		
#100	0.15	74		
#140	0.11	71		
#200	0.075	69		

Coefficients

D ₈₅ = 1.1626 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

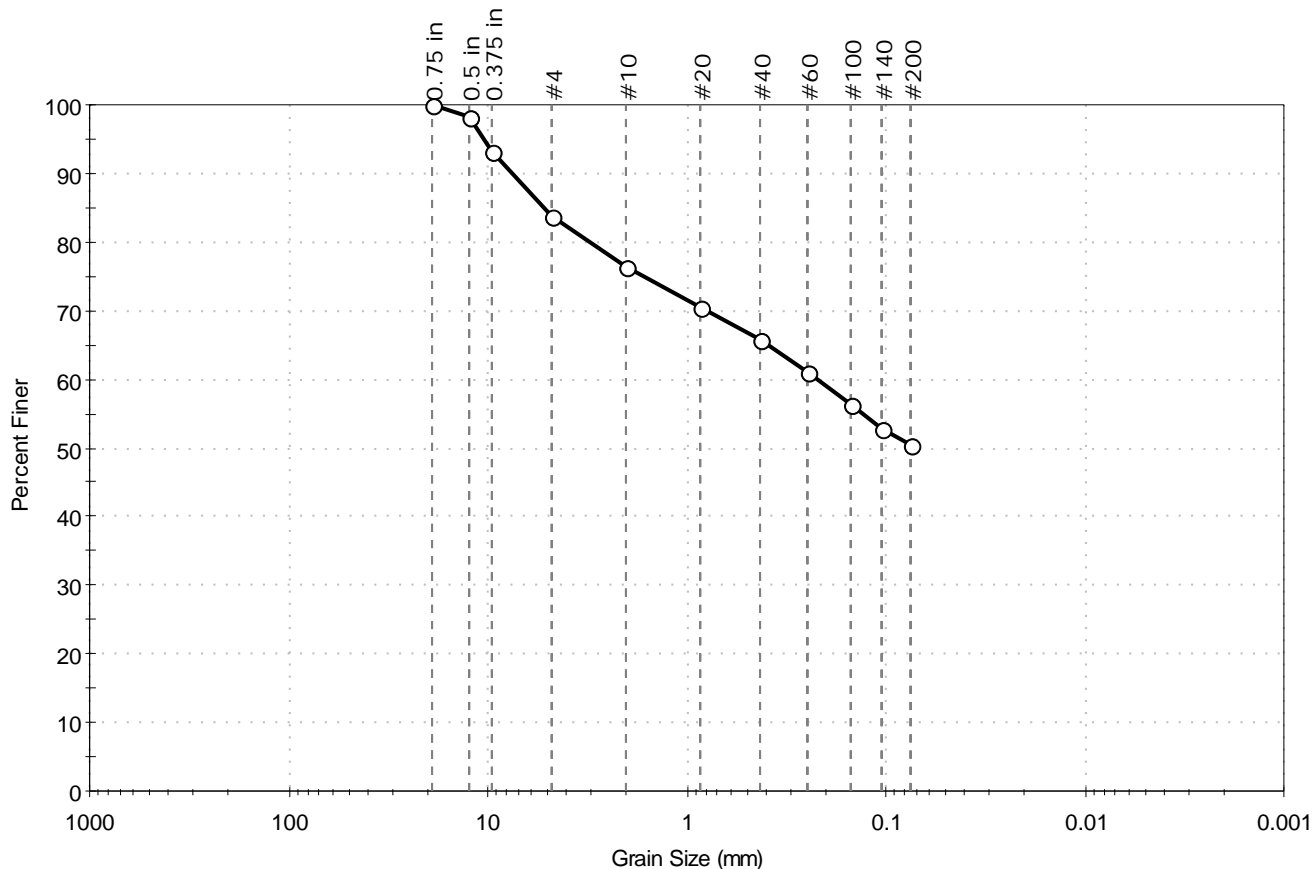
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.			
Project:	Rt 9/I-395 Connector			
Location:	Brewer and Eddington, ME		Project No:	GTX-308853
Boring ID:	HB-BE-144	Sample Type:	jar	Tested By: GA
Sample ID:	2D	Test Date:	10/12/18	Checked By: emm
Depth :	2-4 ft	Test Id:	474347	
Test Comment:	---			
Visual Description:	Moist, light olive brown sandy silt with gravel			
Sample Comment:	---			

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	16.4	33.3	50.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	93		
#4	4.75	84		
#10	2.00	76		
#20	0.85	70		
#40	0.42	66		
#60	0.25	61		
#100	0.15	56		
#140	0.11	53		
#200	0.075	50		

Coefficients

D ₈₅ = 5.2444 mm	D ₃₀ = N/A
D ₆₀ = 0.2227 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

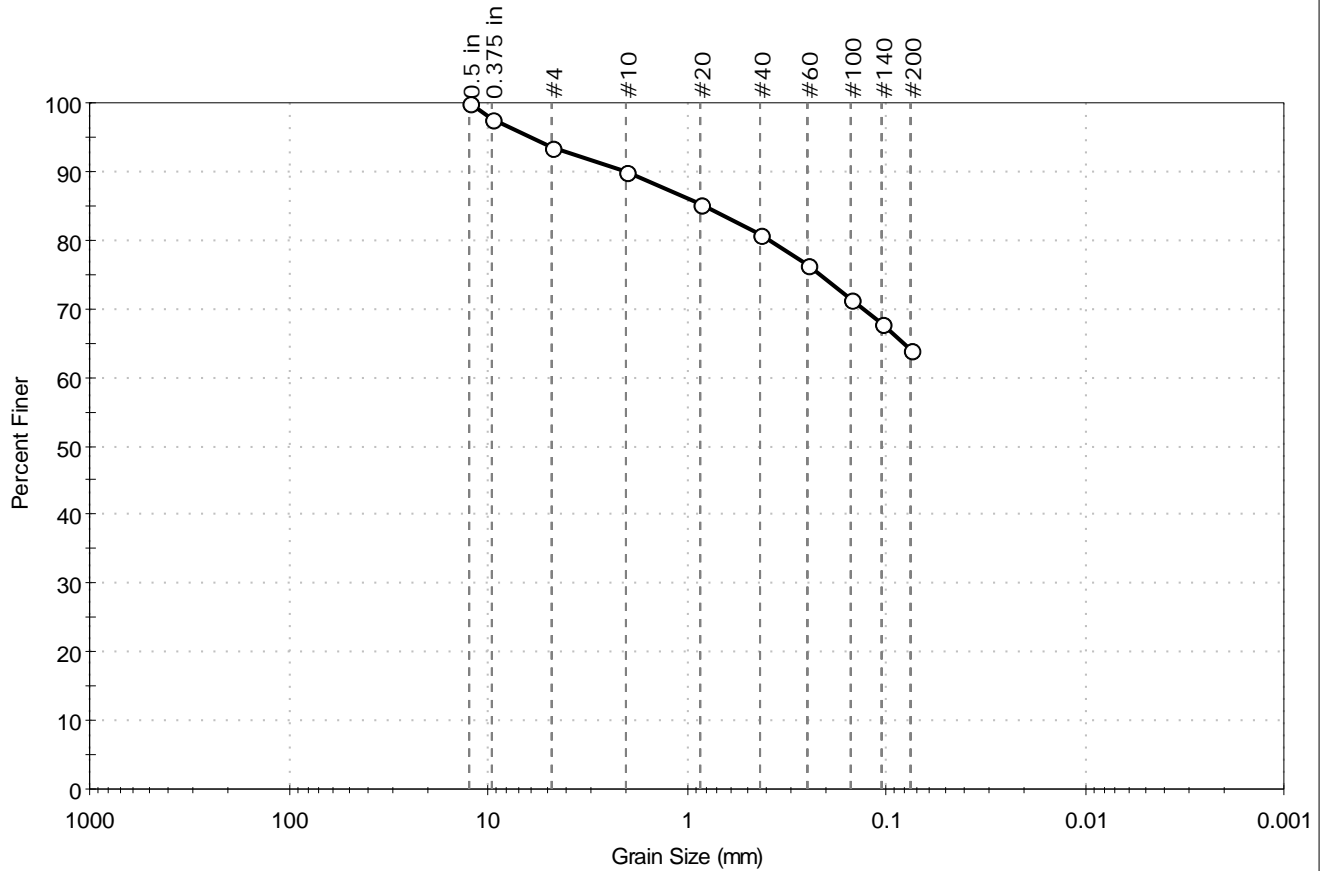
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-144	Sample Type:	jar	Tested By:	GA
Sample ID:	3D	Test Date:	10/12/18	Checked By:	emm
Depth :	5-7 ft	Test Id:	474348		
Test Comment:	---				
Visual Description:	Moist, olive sandy clay				
Sample Comment:	---				

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	6.5	29.5	64.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	93		
#10	2.00	90		
#20	0.85	85		
#40	0.42	81		
#60	0.25	76		
#100	0.15	71		
#140	0.11	68		
#200	0.075	64		

Coefficients

D ₈₅ = 0.8302 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

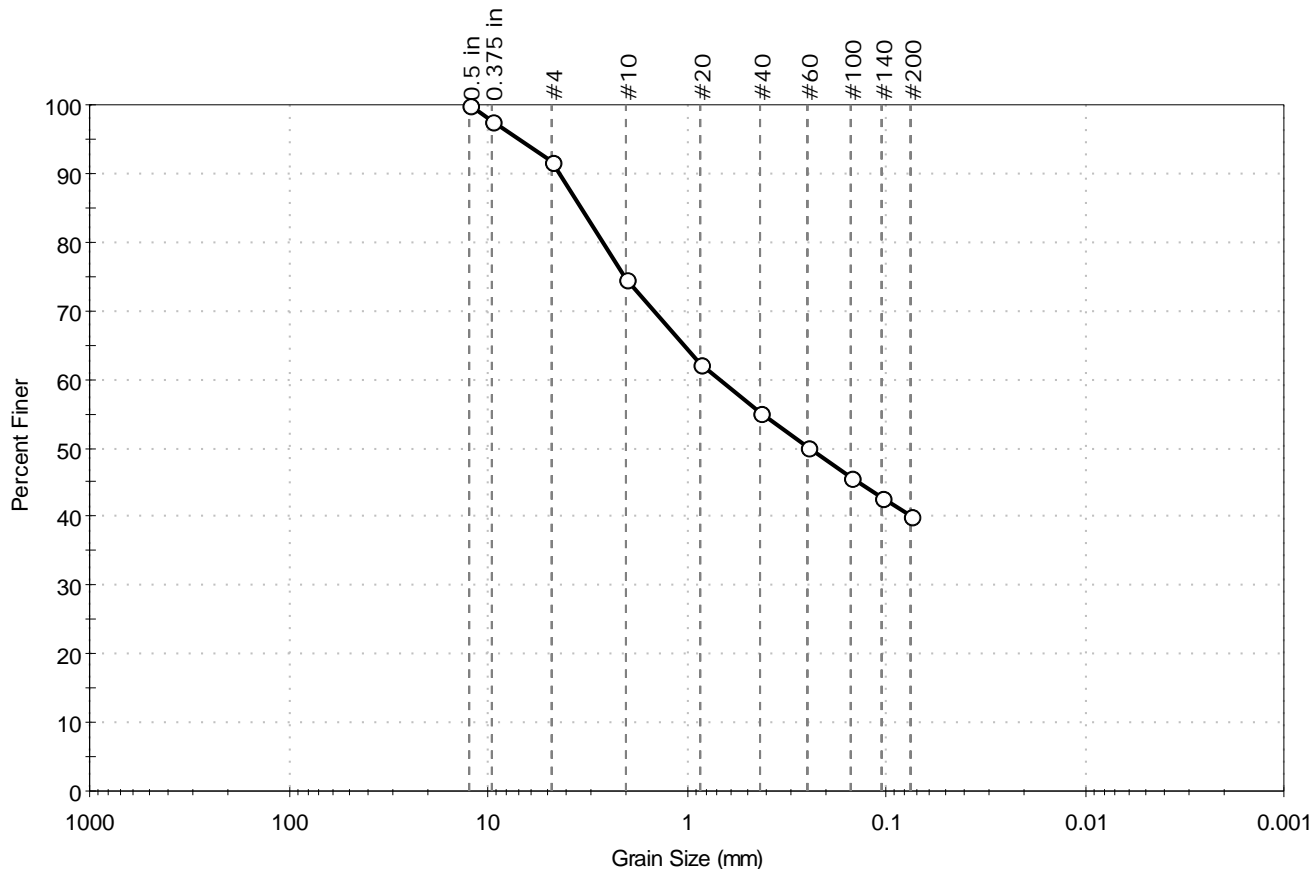
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR

Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-145	Sample Type:	jar
Sample ID:	3D	Test Date:	09/28/18
Depth :	5-6.8 ft	Test Id:	474349
Test Comment:	---		
Visual Description:	Moist, dark grayish brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	8.4	51.5	40.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	92		
#10	2.00	75		
#20	0.85	62		
#40	0.42	55		
#60	0.25	50		
#100	0.15	46		
#140	0.11	43		
#200	0.075	40		

Coefficients

D ₈₅ = 3.3888 mm	D ₃₀ = N/A
D ₆₀ = 0.6777 mm	D ₁₅ = N/A
D ₅₀ = 0.2476 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

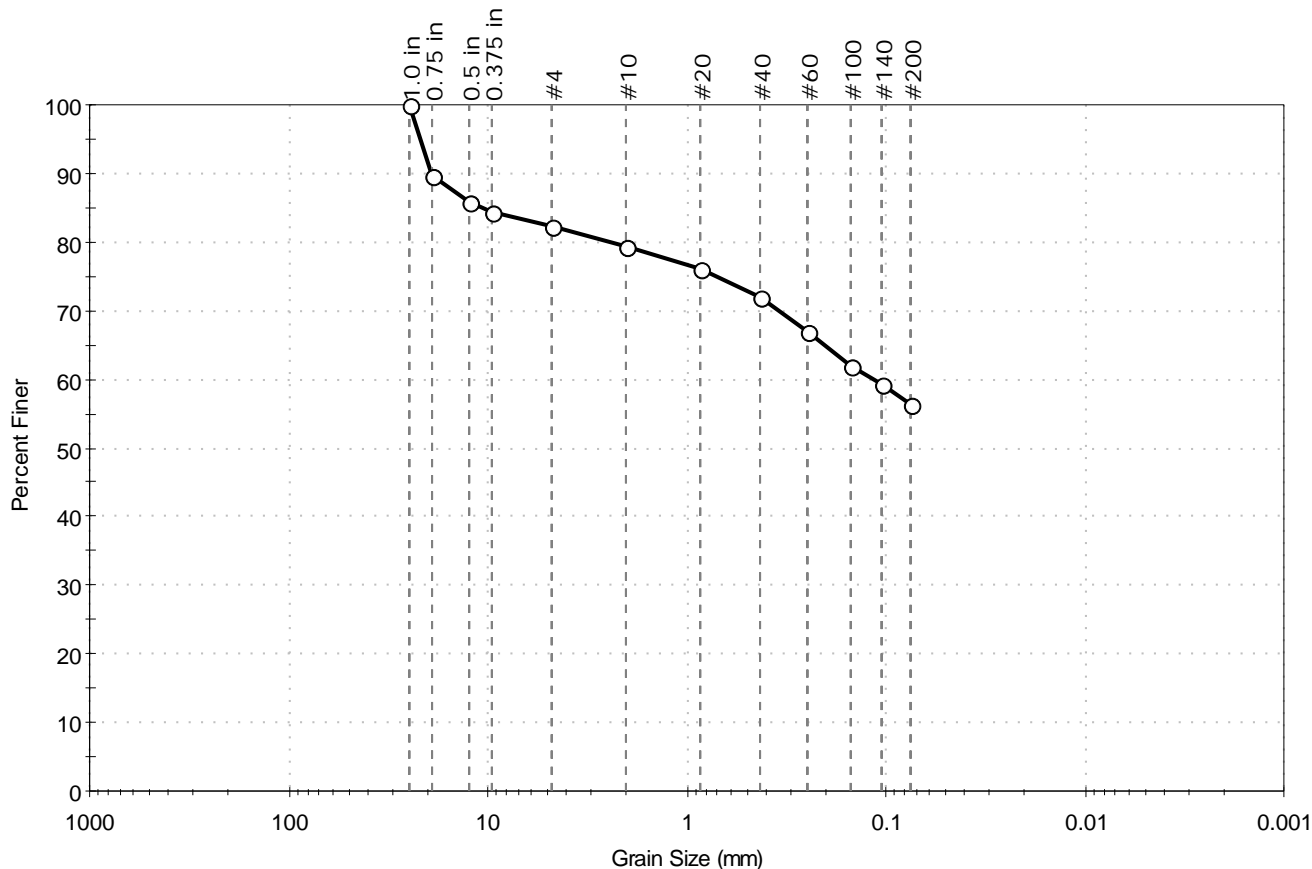
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-148	Sample Type:	jar
Sample ID:	2D	Test Date:	10/12/18
Depth :	2-4 ft	Test Id:	474353
Test Comment:	---		
Visual Description:	Moist, pale olive sandy silt with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	17.8	25.9	56.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	90		
0.5 in	12.50	86		
0.375 in	9.50	84		
#4	4.75	82		
#10	2.00	79		
#20	0.85	76		
#40	0.42	72		
#60	0.25	67		
#100	0.15	62		
#140	0.11	59		
#200	0.075	56		

Coefficients

D ₈₅ = 10.7406 mm	D ₃₀ = N/A
D ₆₀ = 0.1160 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

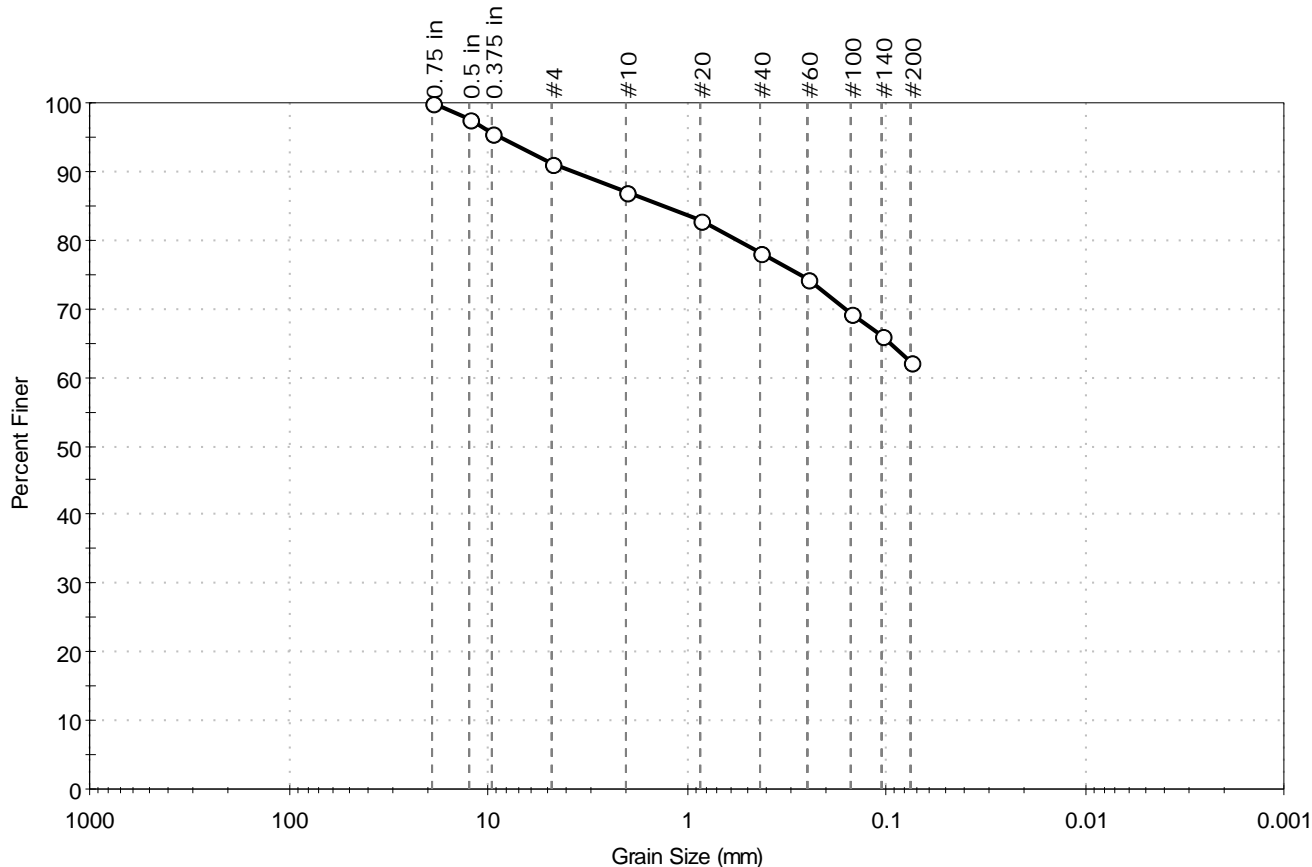
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-148	Sample Type:	jar
Sample ID:	5D	Test Date:	10/12/18
Depth :	12-14 ft	Test Id:	474354
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	8.8	29.0	62.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	96		
#4	4.75	91		
#10	2.00	87		
#20	0.85	83		
#40	0.42	78		
#60	0.25	74		
#100	0.15	69		
#140	0.11	66		
#200	0.075	62		

Coefficients

$D_{85} = 1.3240 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Sandy Lean CLAY (CL)

AASHTO Clayey Soils (A-6 (4))

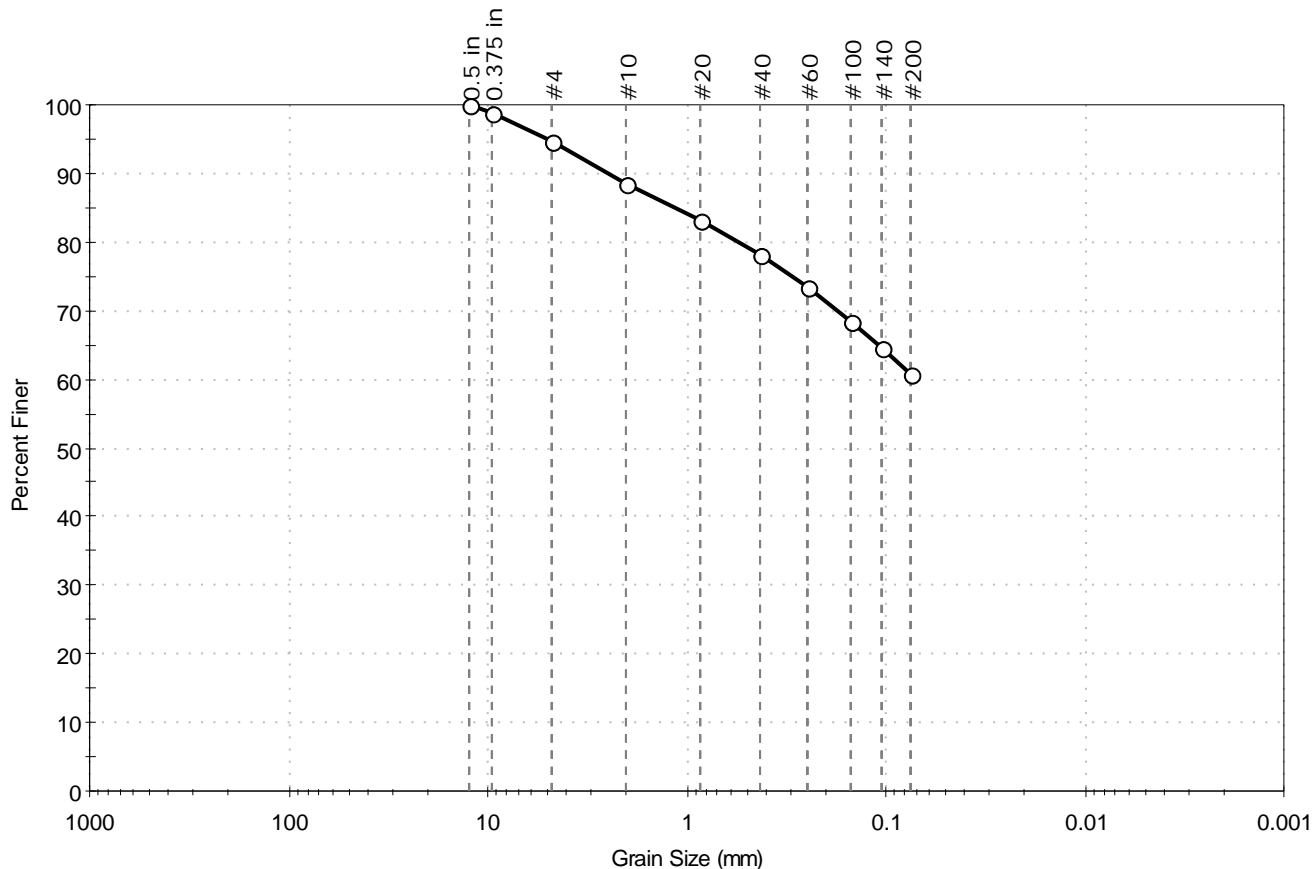
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.			
Project:	Rt 9/I-395 Connector			
Location:	Brewer and Eddington, ME		Project No:	GTX-308853
Boring ID:	HB-BE-148	Sample Type:	jar	Tested By: GA
Sample ID:	6D	Test Date:	10/12/18	Checked By: emm
Depth :	14-15.3 ft	Test Id:	474355	
Test Comment:	---			
Visual Description:	Moist, olive sandy clay			
Sample Comment:	---			

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	5.4	33.7	60.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	99		
#4	4.75	95		
#10	2.00	89		
#20	0.85	83		
#40	0.42	78		
#60	0.25	73		
#100	0.15	69		
#140	0.11	65		
#200	0.075	61		

Coefficients

D ₈₅ = 1.1369 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM Sandy Lean CLAY (CL)

AASHTO Silty Soils (A-4 (3))

Sample/Test Description

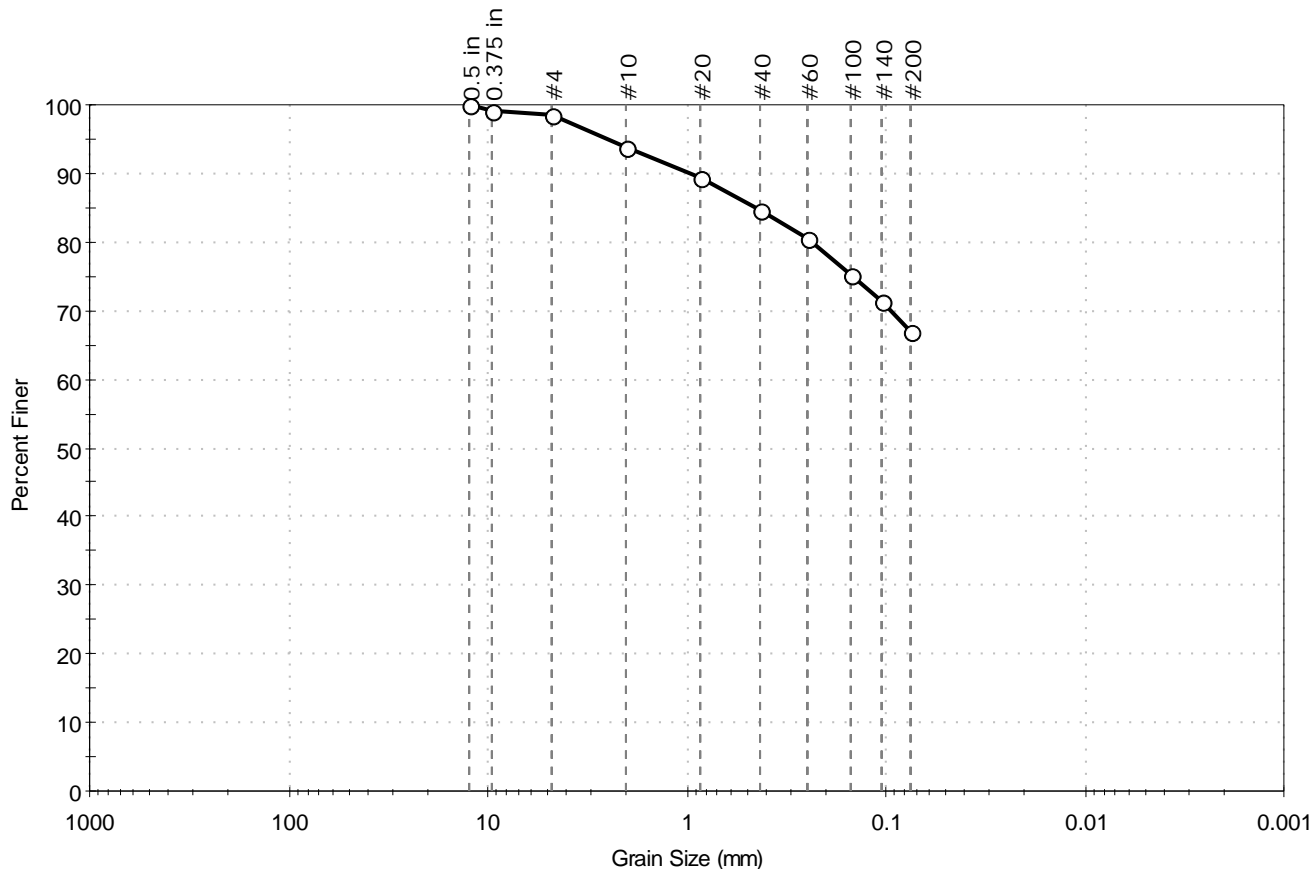
Sand/Gravel Particle Shape : ANGULAR

Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.	Project No:	GTX-308853
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME		
Boring ID:	HB-BE-151	Sample Type:	jar
Sample ID:	2D	Test Date:	10/12/18
Depth :	2-3.7 ft	Test Id:	474356
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	1.6	31.5	66.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	99		
#4	4.75	98		
#10	2.00	94		
#20	0.85	89		
#40	0.42	85		
#60	0.25	81		
#100	0.15	75		
#140	0.11	71		
#200	0.075	67		

Coefficients

D ₈₅ = 0.4417 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

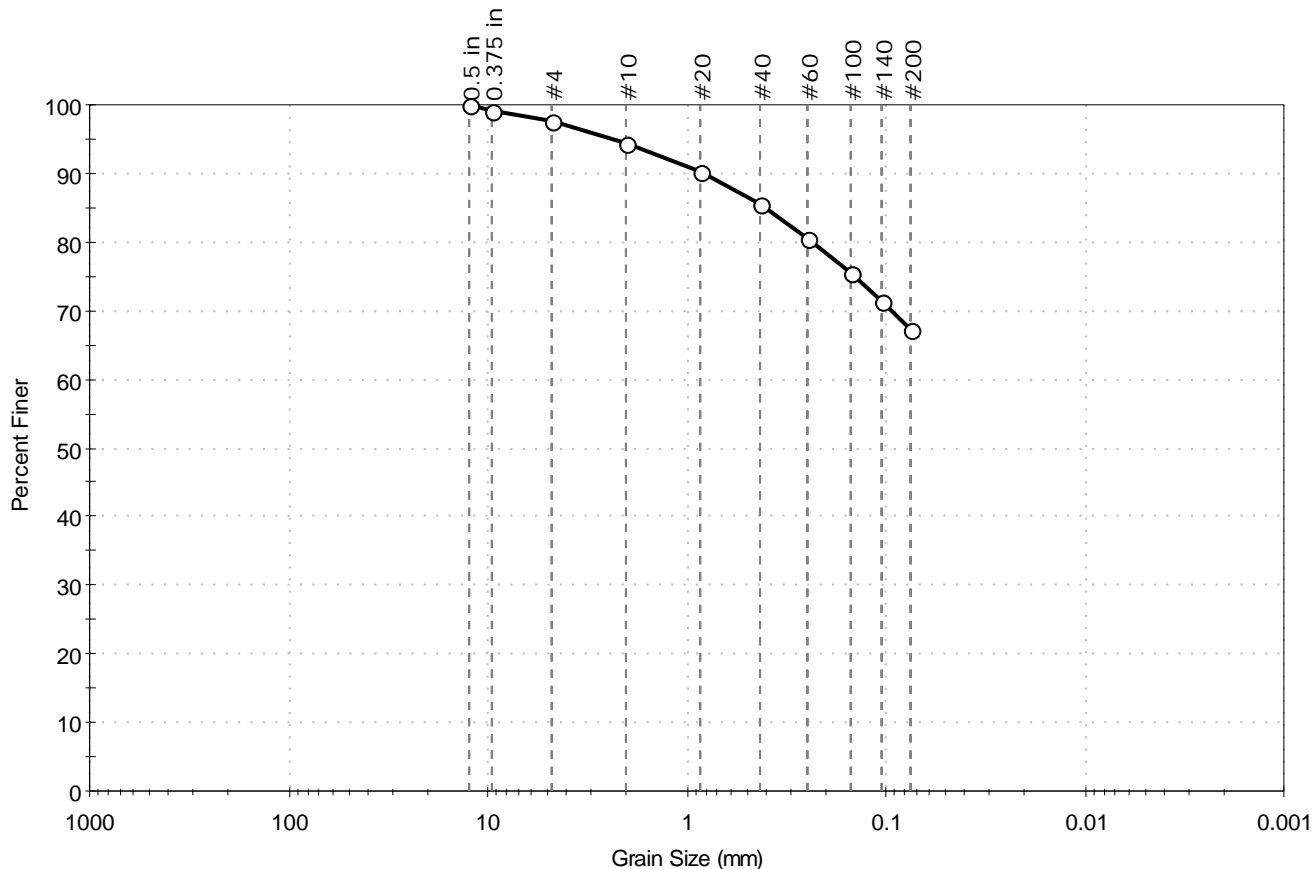
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-151	Sample Type:	jar
Sample ID:	3D	Test Date:	10/12/18
Depth :	4-6 ft	Test Id:	474357
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	2.2	30.6	67.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	99		
#4	4.75	98		
#10	2.00	95		
#20	0.85	90		
#40	0.42	86		
#60	0.25	81		
#100	0.15	75		
#140	0.11	71		
#200	0.075	67		

Coefficients

$D_{85} = 0.3970 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

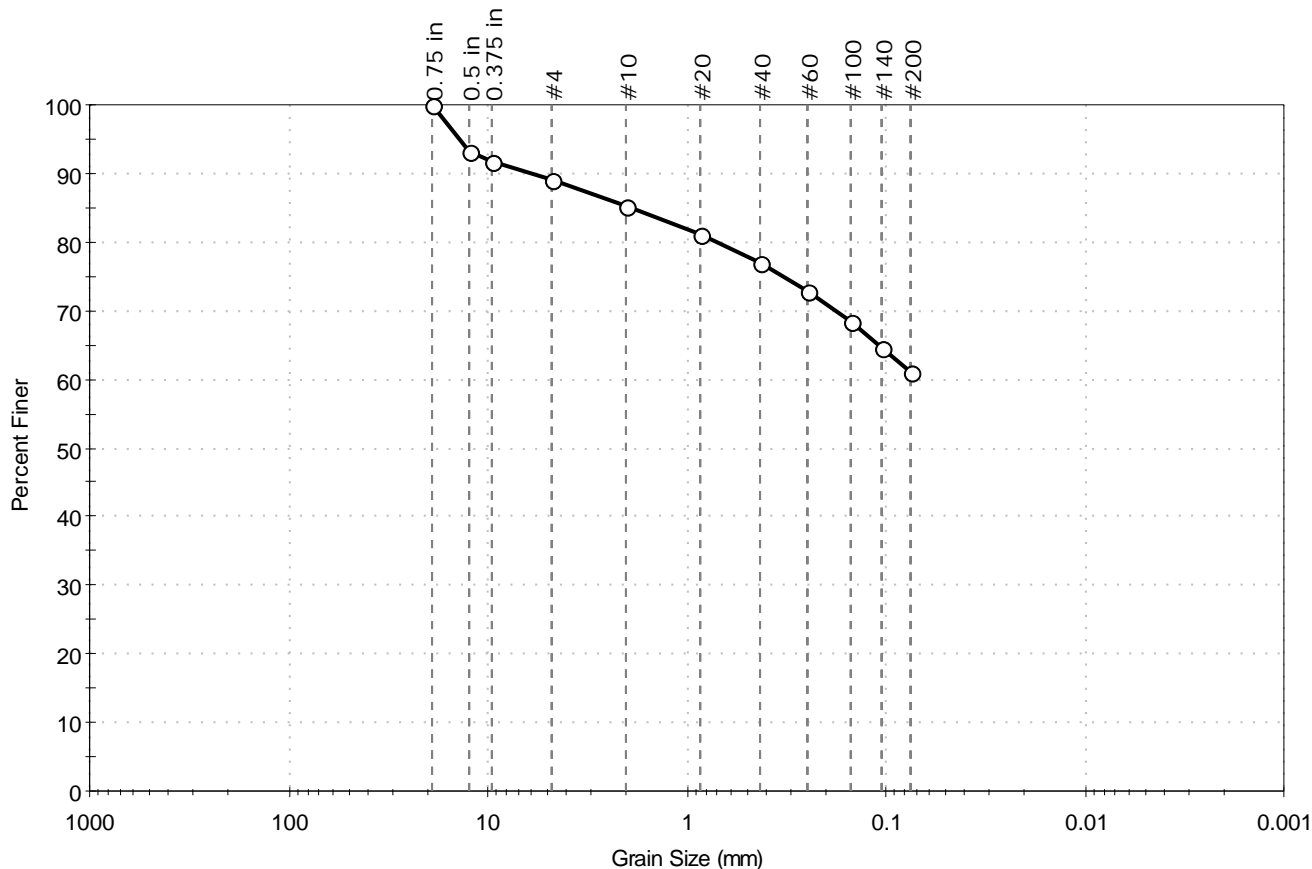
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-151	Sample Type:	jar
Sample ID:	4D	Test Date:	10/12/18
Depth :	10-10.9 ft	Test Id:	474358
Test Comment:	---		
Visual Description:	Moist, olive gray sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	10.9	28.0	61.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	92		
#4	4.75	89		
#10	2.00	85		
#20	0.85	81		
#40	0.42	77		
#60	0.25	73		
#100	0.15	69		
#140	0.11	65		
#200	0.075	61		

Coefficients

D ₈₅ = 1.9341 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

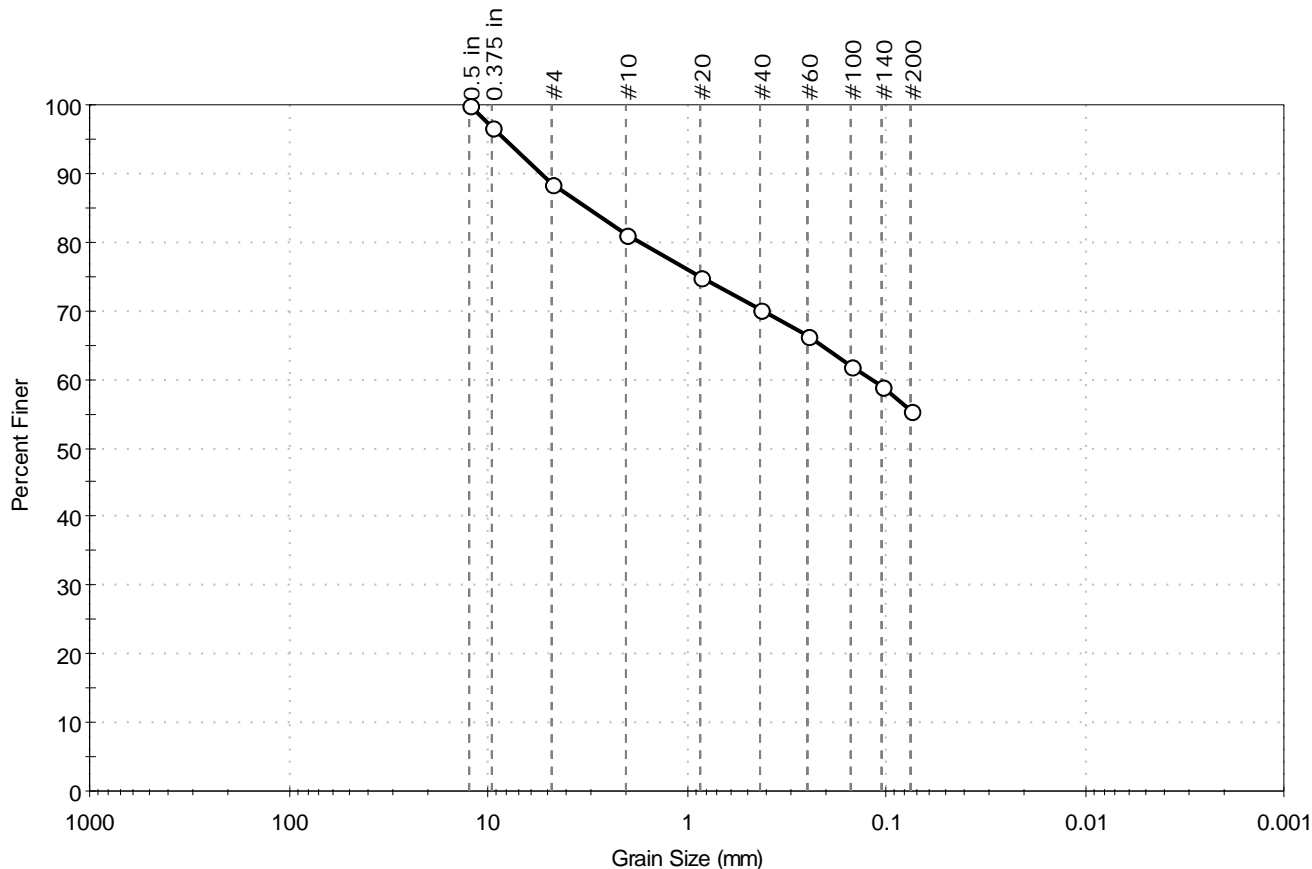
Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD



Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-151	Sample Type:	jar
Sample ID:	5D	Test Date:	10/12/18
Depth :	15-16.2 ft	Test Id:	474359
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	11.4	33.0	55.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	97		
#4	4.75	89		
#10	2.00	81		
#20	0.85	75		
#40	0.42	70		
#60	0.25	66		
#100	0.15	62		
#140	0.11	59		
#200	0.075	56		

Coefficients

$D_{85} = 3.1608 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1199 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM Sandy Lean CLAY (CL)

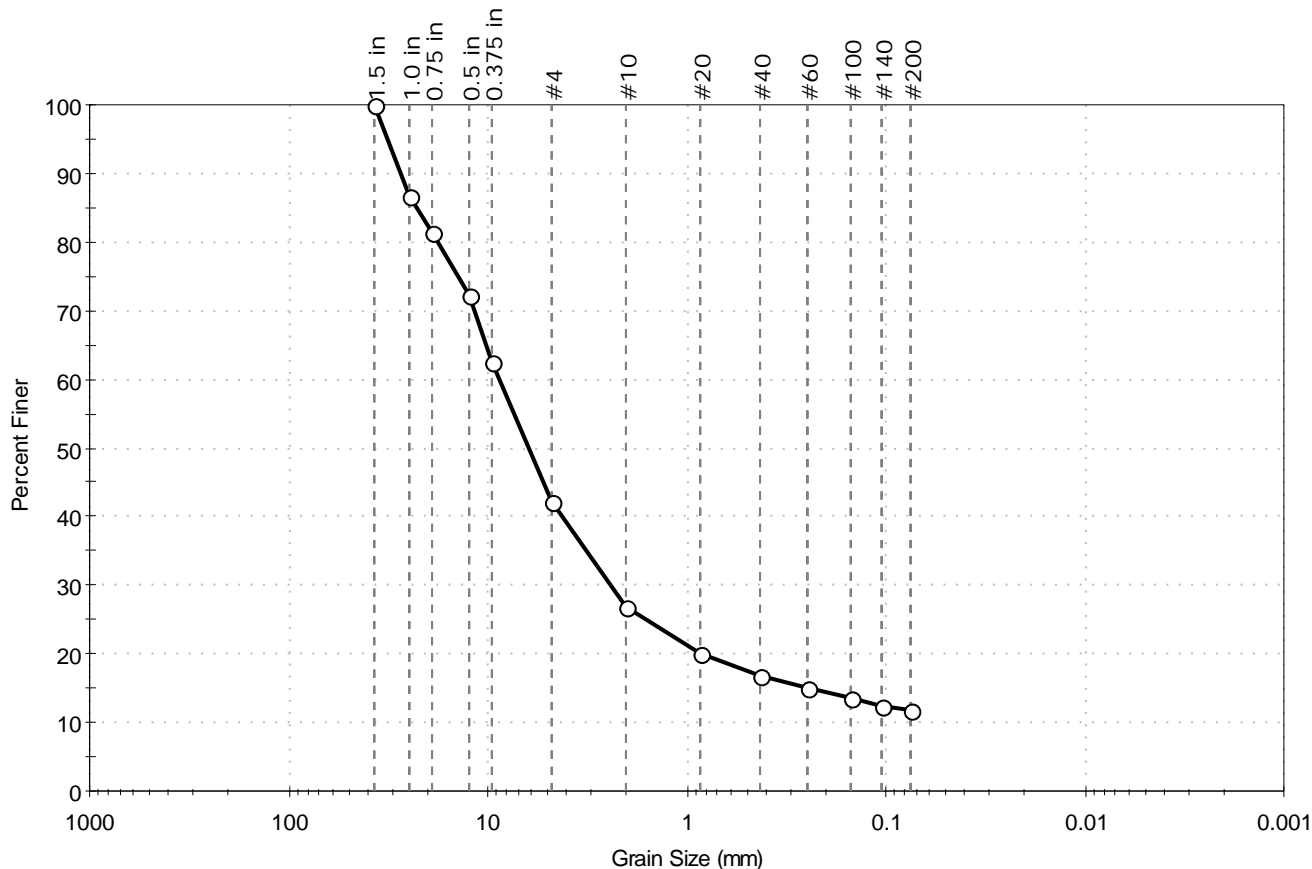
AASHTO Clayey Soils (A-6 (3))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-151	Sample Type:	jar
Sample ID:	6D+7D+8D	Test Date:	10/12/18
Depth :	17-21.5 ft	Test Id:	474360
Test Comment:	---		
Visual Description:	Moist, olive brown gravel with clay and sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
---	57.7	30.6	11.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1.0 in	25.00	87		
0.75 in	19.00	82		
0.5 in	12.50	72		
0.375 in	9.50	63		
#4	4.75	42		
#10	2.00	27		
#20	0.85	20		
#40	0.42	17		
#60	0.25	15		
#100	0.15	14		
#140	0.11	13		
#200	0.075	12		

Coefficients

D ₈₅ = 22.8430 mm	D ₃₀ = 2.3793 mm
D ₆₀ = 8.6744 mm	D ₁₅ = 0.2570 mm
D ₅₀ = 6.1704 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

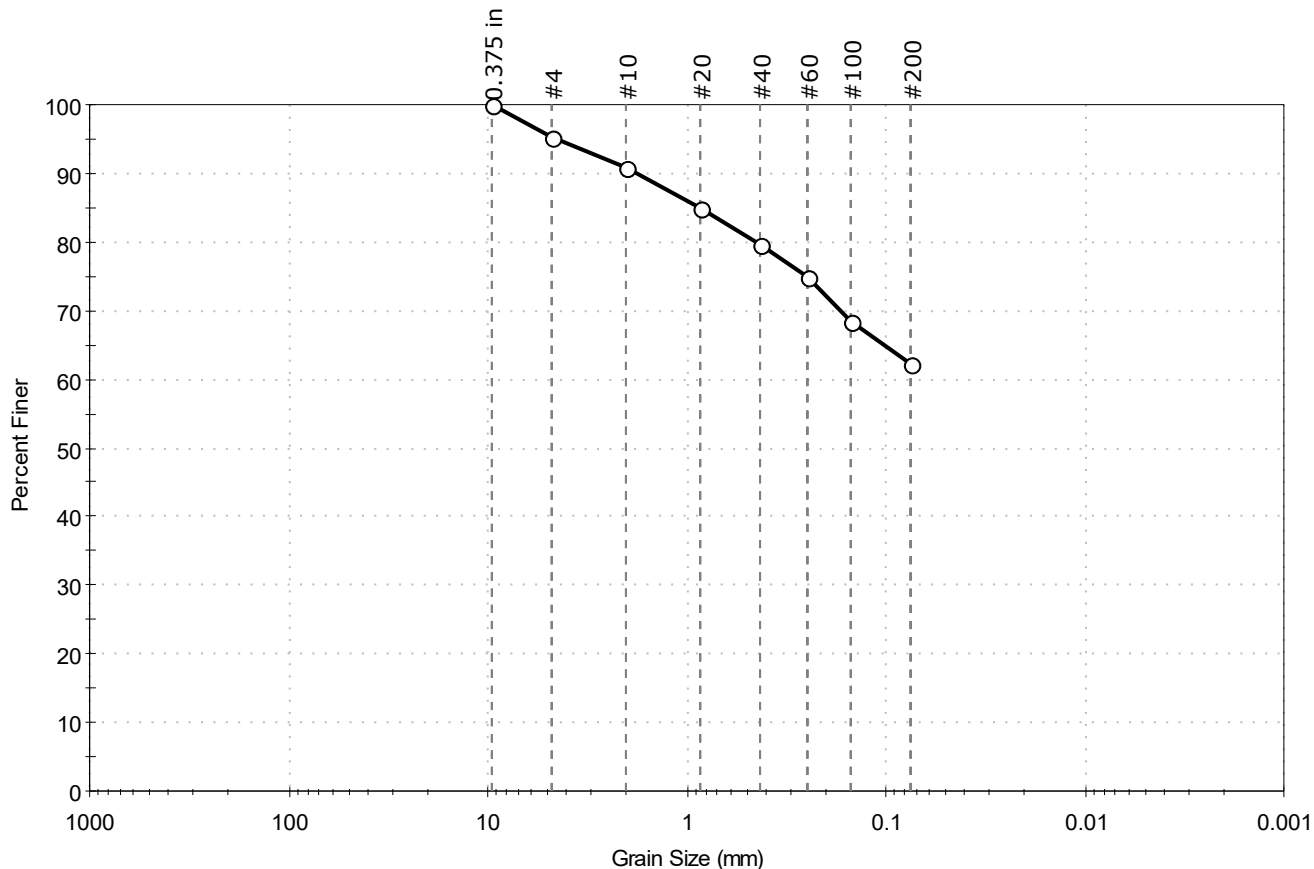
AASHTO Stone Fragments, Gravel and Sand (A-1-a (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-152	Sample Type:	bag
Sample ID:	2D	Test Date:	07/15/19
Depth :	5-7	Test Id:	513326
Test Comment:	---		
Visual Description:	Moist, dark olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	4.7	33.0	62.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	95		
#10	2.00	91		
#20	0.85	85		
#40	0.42	80		
#60	0.25	75		
#100	0.15	69		
#200	0.075	62		

Coefficients

$D_{85} = 0.8606 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

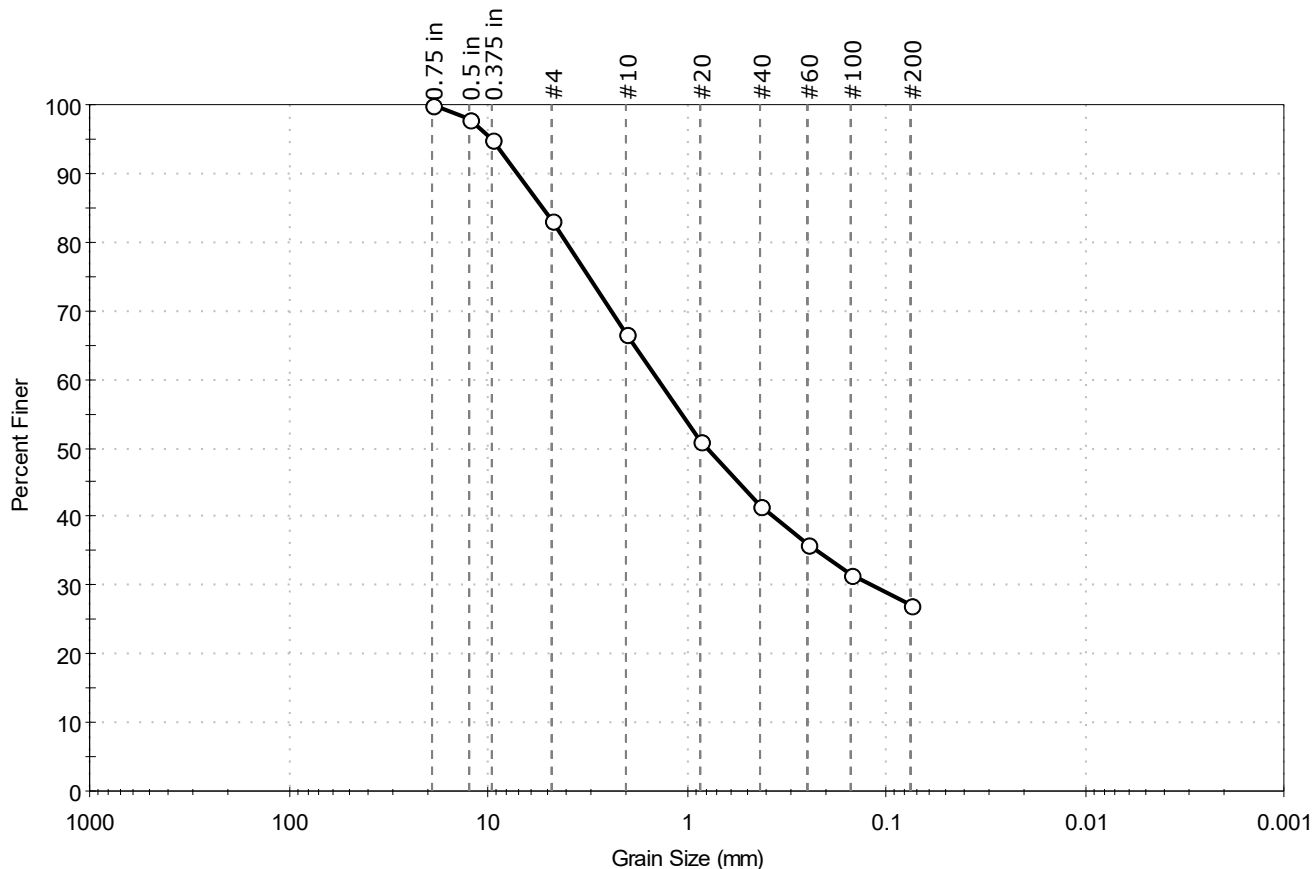
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-152	Sample Type:	bag
Sample ID:	5D	Test Date:	07/15/19
Depth :	17-18.7	Test Id:	513327
Test Comment:	---		
Visual Description:	Moist, dark brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	16.7	56.1	27.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	98		
0.375 in	9.50	95		
#4	4.75	83		
#10	2.00	67		
#20	0.85	51		
#40	0.42	41		
#60	0.25	36		
#100	0.15	31		
#200	0.075	27		

Coefficients

$D_{85} = 5.2597 \text{ mm}$ $D_{30} = 0.1185 \text{ mm}$
 $D_{60} = 1.3892 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.7859 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

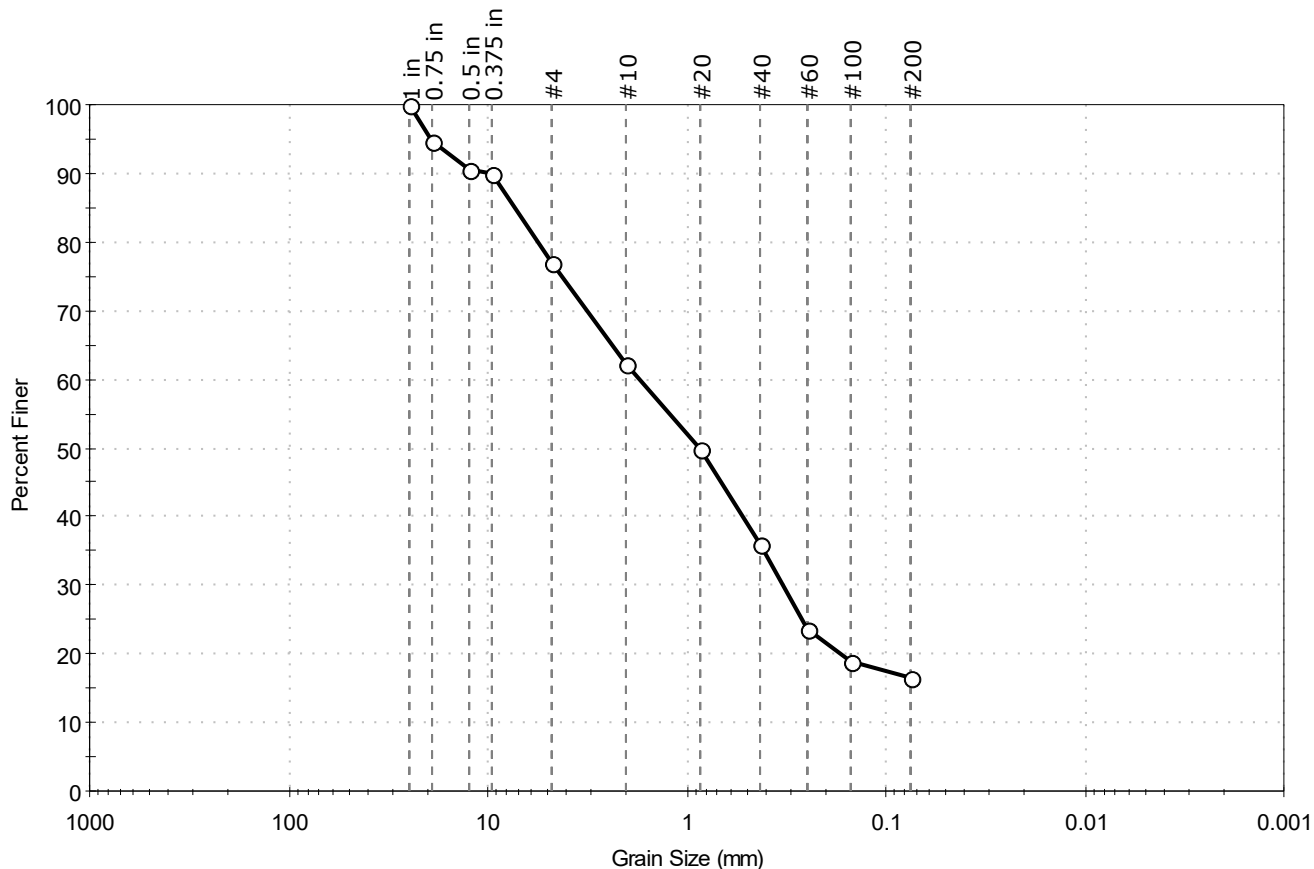
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-153	Sample Type:	bag
Sample ID:	1D	Test Date:	07/19/19
Depth :	0-2	Test Id:	513328
Test Comment:	---		
Visual Description:	Moist, dark reddish brown silty sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	23.1	60.3	16.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	95		
0.5 in	12.50	91		
0.375 in	9.50	90		
#4	4.75	77		
#10	2.00	62		
#20	0.85	50		
#40	0.42	36		
#60	0.25	24		
#100	0.15	19		
#200	0.075	17		

Coefficients

D ₈₅ = 7.3110 mm	D ₃₀ = 0.3292 mm
D ₆₀ = 1.7170 mm	D ₁₅ = N/A
D ₅₀ = 0.8523 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

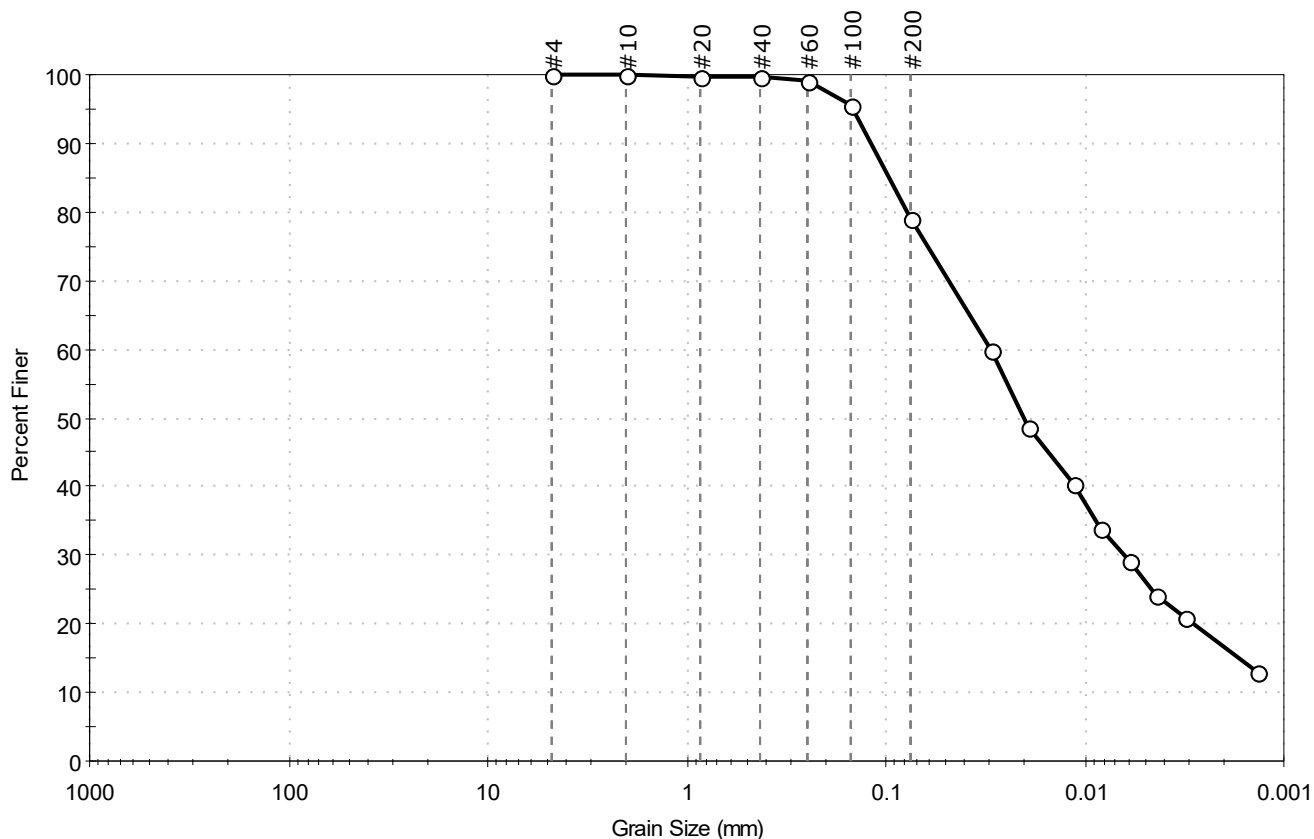
AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-308853	
Project: Rt 9/I-395 Connector		
Location: Brewer and Eddington, ME		
Boring ID: HB-BE-163	Sample Type: bag	Tested By: ckg
Sample ID: 2D	Test Date: 07/18/19	Checked By: bfs
Depth: 5-7	Test Id: 513339	
Test Comment: ---		
Visual Description: Moist, mottled olive and reddish brown clay with sand		
Sample Comment: ---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	20.9	79.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	99		
#100	0.15	96		
#200	0.075	79		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0295	60		
---	0.0194	49		
---	0.0115	41		
---	0.0084	34		
---	0.0060	29		
---	0.0044	24		
---	0.0031	21		
---	0.0014	13		

Coefficients

$D_{85} = 0.0961$ mm $D_{30} = 0.0063$ mm
 $D_{60} = 0.0296$ mm $D_{15} = 0.0017$ mm
 $D_{50} = 0.0204$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

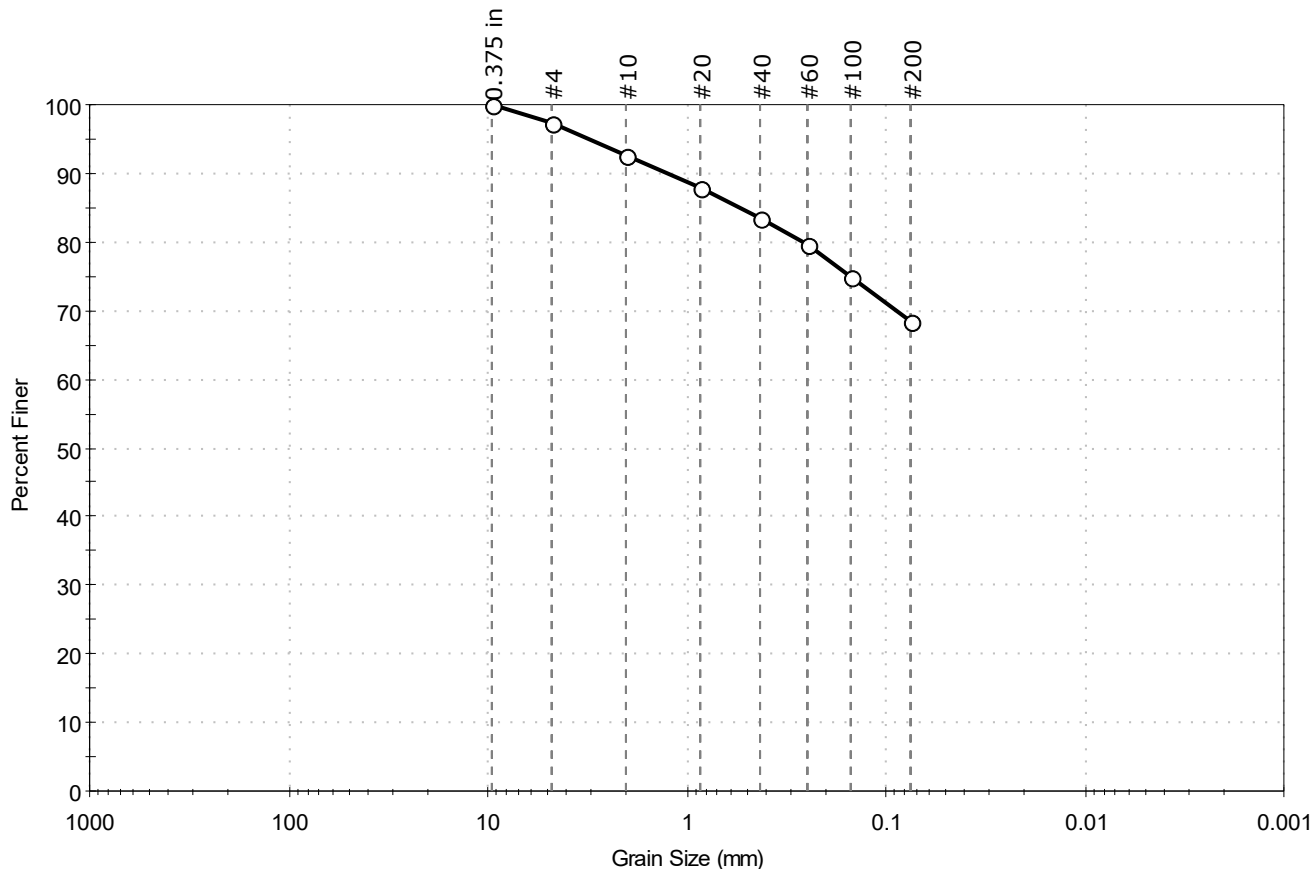
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client: Haley & Aldrich, Inc.	Project No: GTX-308853	
Project: Rt 9/I-395 Connector		
Location: Brewer and Eddington, ME		
Boring ID: HB-BE-164	Sample Type: bag	Tested By: ckg
Sample ID: 2D	Test Date: 07/15/19	Checked By: bfs
Depth: 5-7	Test Id: 513329	
Test Comment: ---		
Visual Description: Moist, olive sandy clay		
Sample Comment: ---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	2.7	28.7	68.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	97		
#10	2.00	93		
#20	0.85	88		
#40	0.42	84		
#60	0.25	80		
#100	0.15	75		
#200	0.075	69		

Coefficients

$D_{85} = 0.5323 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

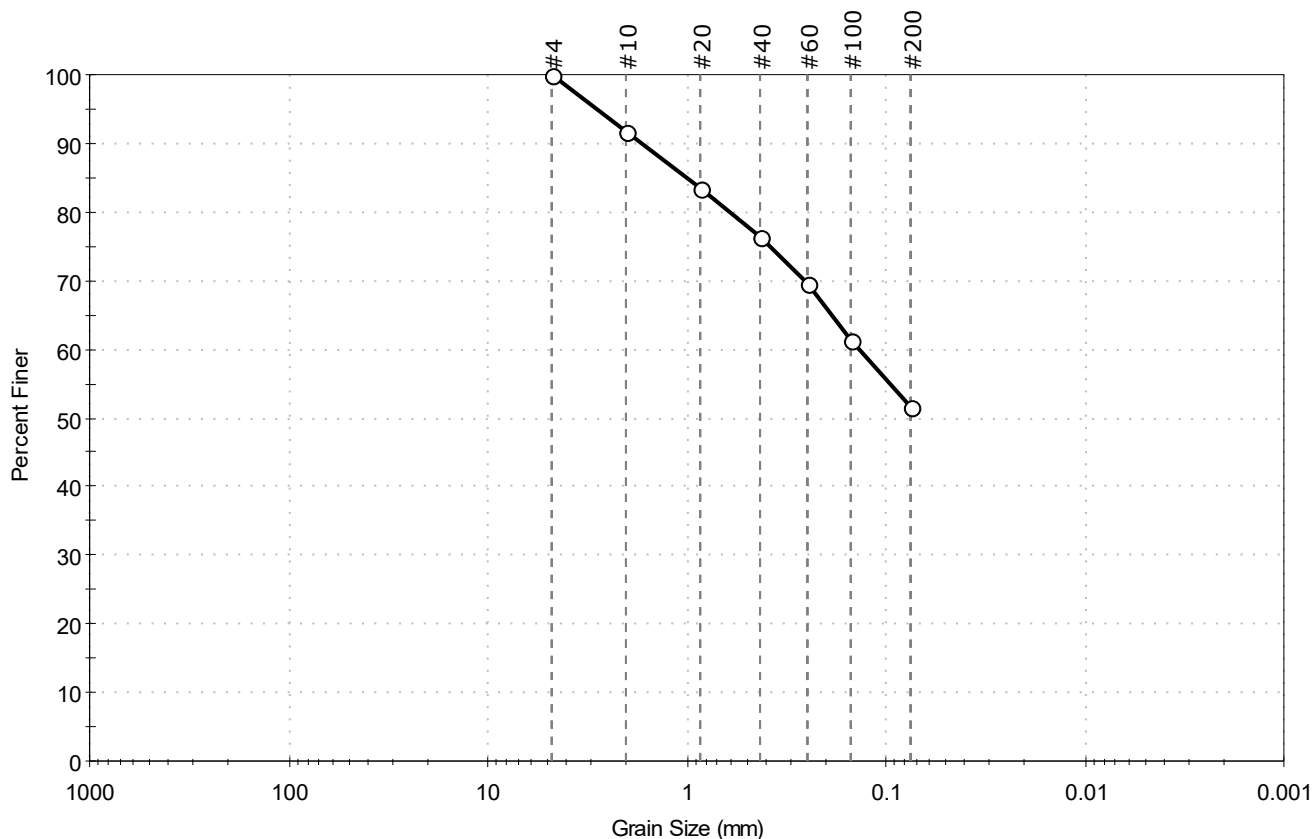
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-164	Sample Type:	bag
Sample ID:	3D	Test Date:	07/19/19
Depth :	10-12	Test Id:	513330
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	48.4	51.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	92		
#20	0.85	84		
#40	0.42	76		
#60	0.25	70		
#100	0.15	61		
#200	0.075	52		

Coefficients

$D_{85} = 0.9864 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1355 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

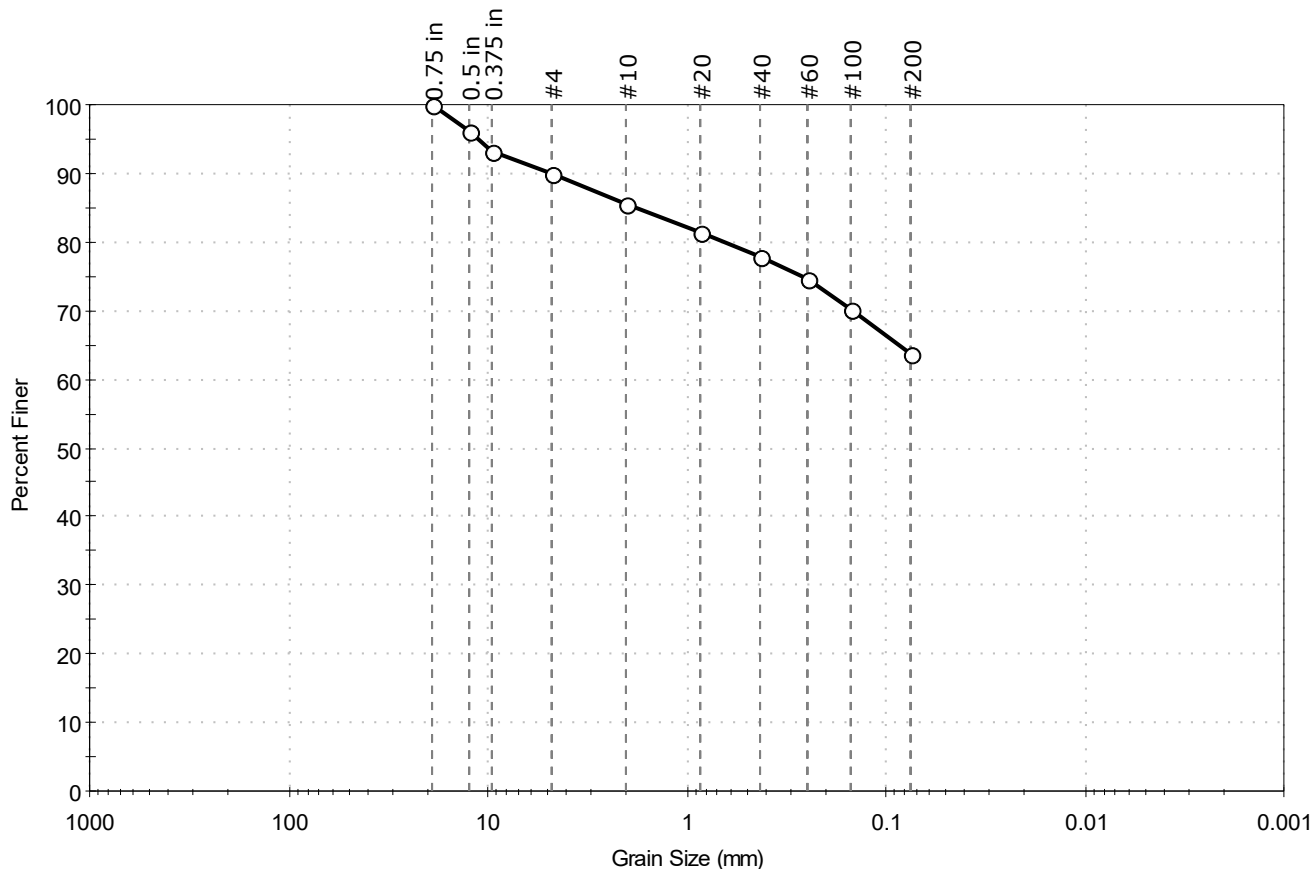
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-165	Sample Type:	bag
Sample ID:	2D	Test Date:	07/15/19
Depth :	5-7	Test Id:	513331
Test Comment:	---		
Visual Description:	Moist, dark olive brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	9.9	26.3	63.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	96		
0.375 in	9.50	93		
#4	4.75	90		
#10	2.00	86		
#20	0.85	81		
#40	0.42	78		
#60	0.25	75		
#100	0.15	70		
#200	0.075	64		

Coefficients

D ₈₅ = 1.8007 mm	D ₃₀ = N/A
D ₆₀ = N/A	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

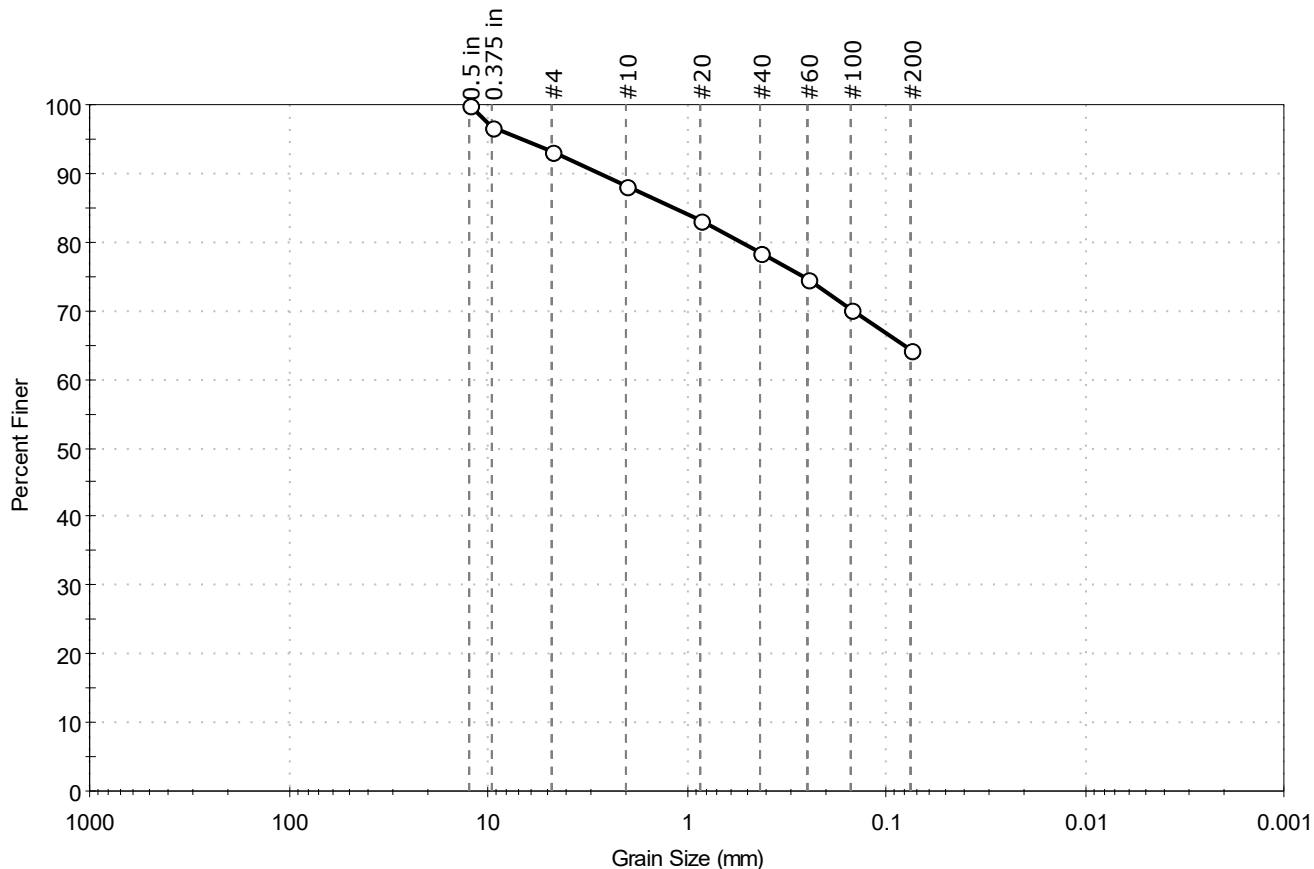
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-165	Sample Type:	bag
Sample ID:	5D	Test Date:	07/19/19
Depth :	14-16	Test Id:	513332
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.8	29.0	64.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	97		
#4	4.75	93		
#10	2.00	88		
#20	0.85	83		
#40	0.42	78		
#60	0.25	75		
#100	0.15	70		
#200	0.075	64		

Coefficients

$D_{85} = 1.1652 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

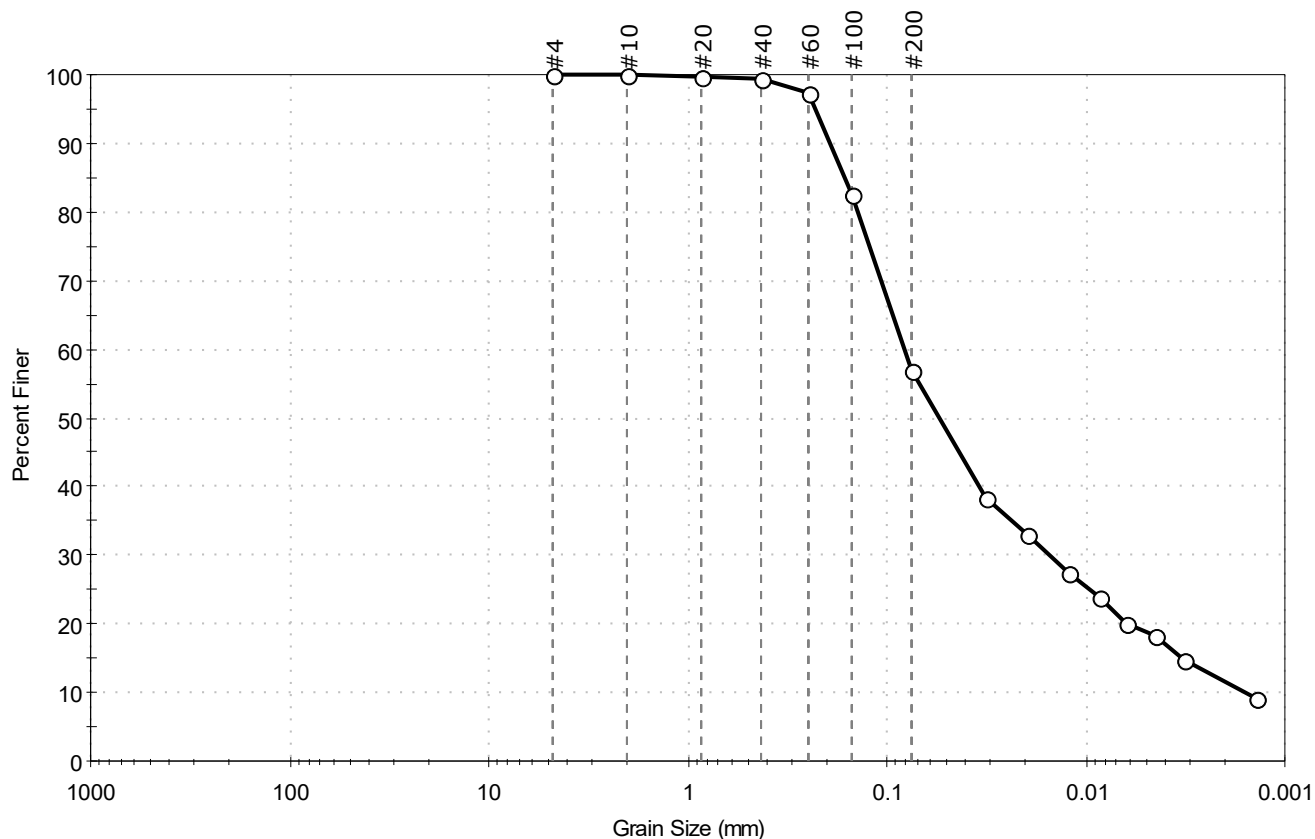
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-308853	
Project: Rt 9/I-395 Connector		
Location: Brewer and Eddington, ME		
Boring ID: HB-BE-166	Sample Type: bag	Tested By: ckg
Sample ID: 2D	Test Date: 07/18/19	Checked By: bfs
Depth: 5-7	Test Id: 513340	
Test Comment: ---		
Visual Description: Moist, olive sandy clay		
Sample Comment: ---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	42.9	57.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	97		
#100	0.15	83		
#200	0.075	57		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0315	38		
---	0.0198	33		
---	0.0122	27		
---	0.0086	24		
---	0.0062	20		
---	0.0045	18		
---	0.0032	15		
---	0.0014	9		

Coefficients

$D_{85} = 0.1627$ mm $D_{30} = 0.0152$ mm
 $D_{60} = 0.0812$ mm $D_{15} = 0.0033$ mm
 $D_{50} = 0.0540$ mm $D_{10} = 0.0016$ mm
 $C_u = 50.750$ $C_c = 1.778$

Classification

ASTM N/A

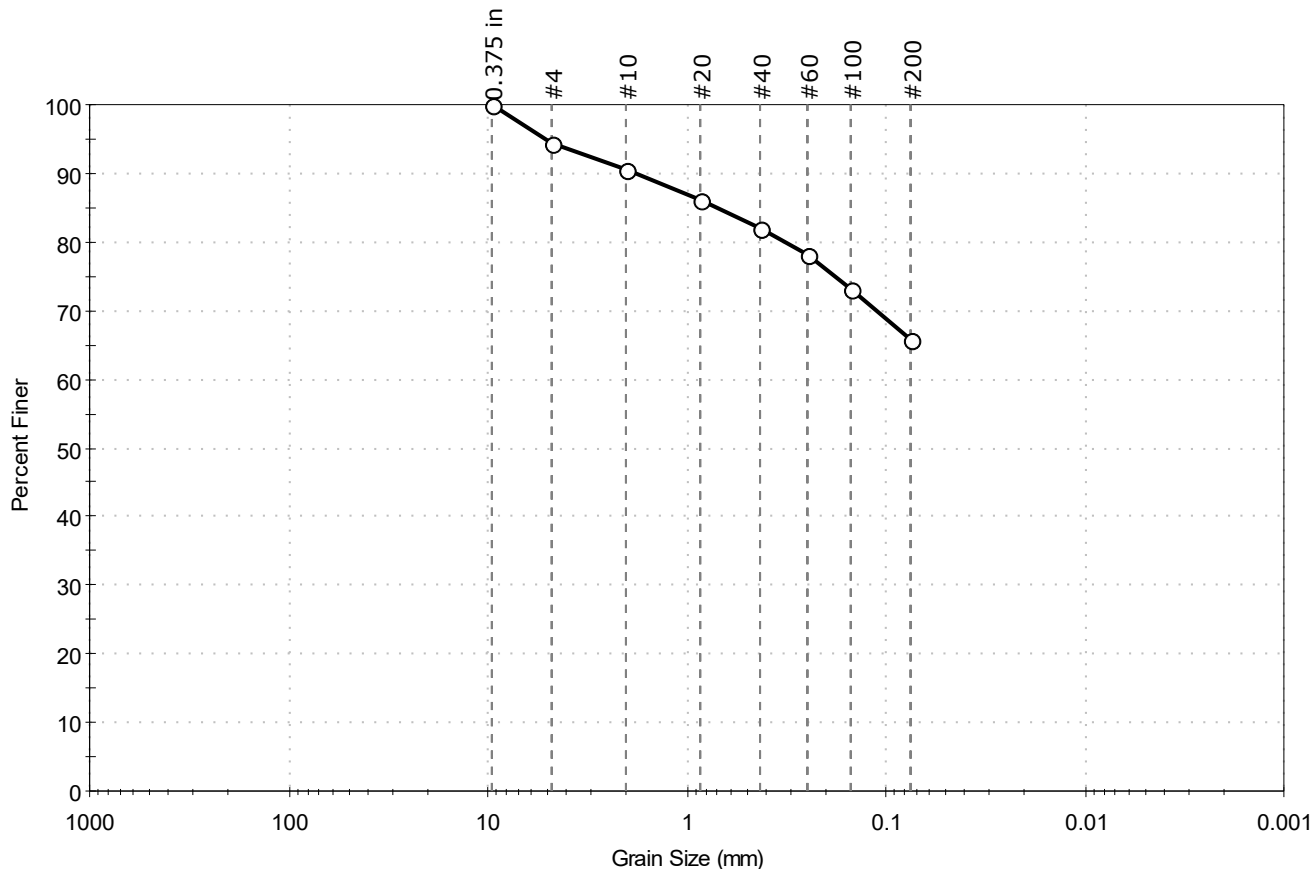
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ---
 Sand/Gravel Hardness : ---
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #200 Sieve

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-168	Sample Type:	bag
Sample ID:	2D	Test Date:	07/15/19
Depth :	2-4	Test Id:	513333
Test Comment:	---		
Visual Description:	Moist, olive sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	5.5	28.8	65.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	95		
#10	2.00	91		
#20	0.85	86		
#40	0.42	82		
#60	0.25	78		
#100	0.15	73		
#200	0.075	66		

Coefficients

$D_{85} = 0.7086$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

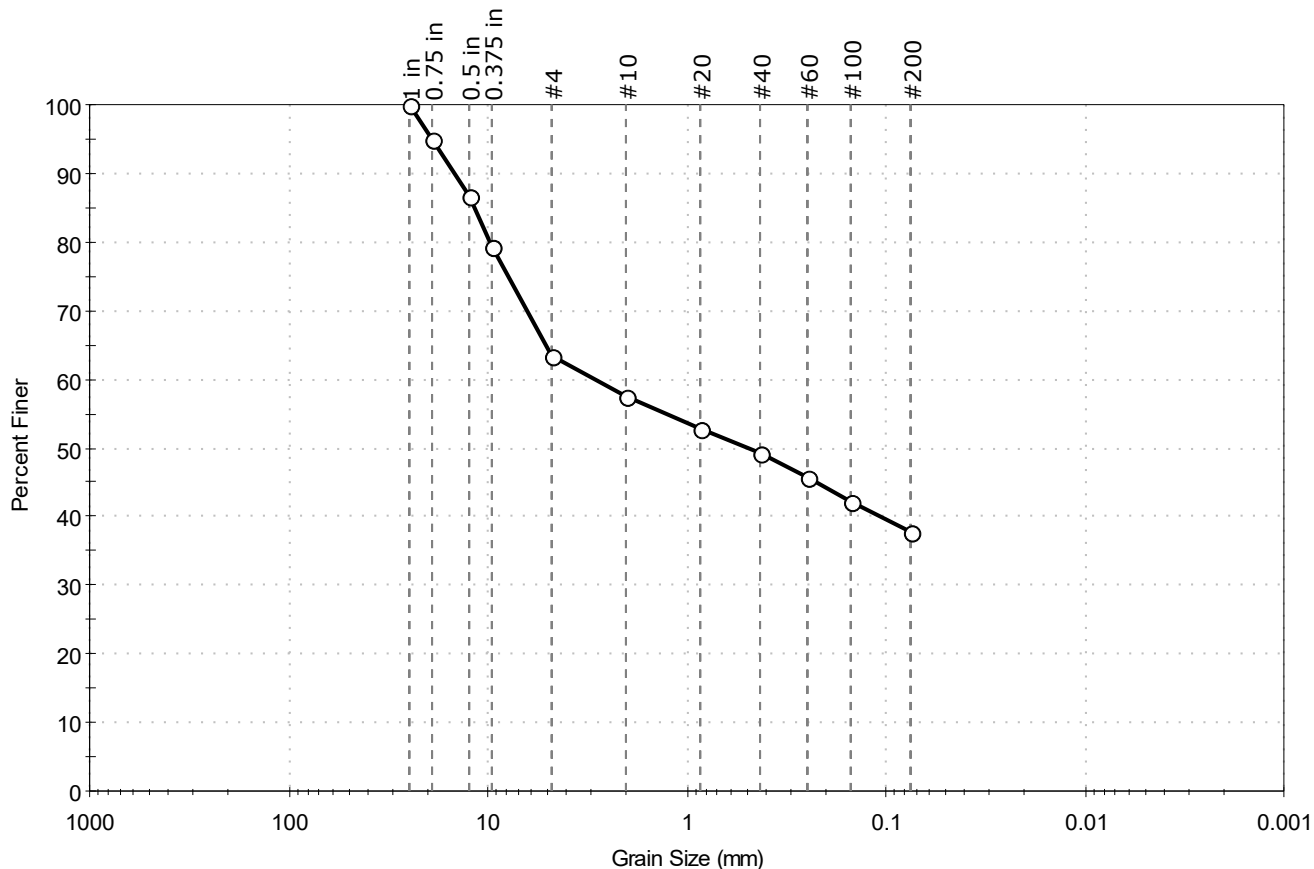
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-168	Sample Type:	bag
Sample ID:	4D	Test Date:	07/19/19
Depth :	6-8	Test Id:	513334
Test Comment:	---		
Visual Description:	Moist, dark olive clayey gravel with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D422



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	36.7	25.5	37.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1 in	25.00	100		
0.75 in	19.00	95		
0.5 in	12.50	87		
0.375 in	9.50	79		
#4	4.75	63		
#10	2.00	57		
#20	0.85	53		
#40	0.42	49		
#60	0.25	46		
#100	0.15	42		
#200	0.075	38		

Coefficients

$D_{85} = 11.6956 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 2.9215 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.4863 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

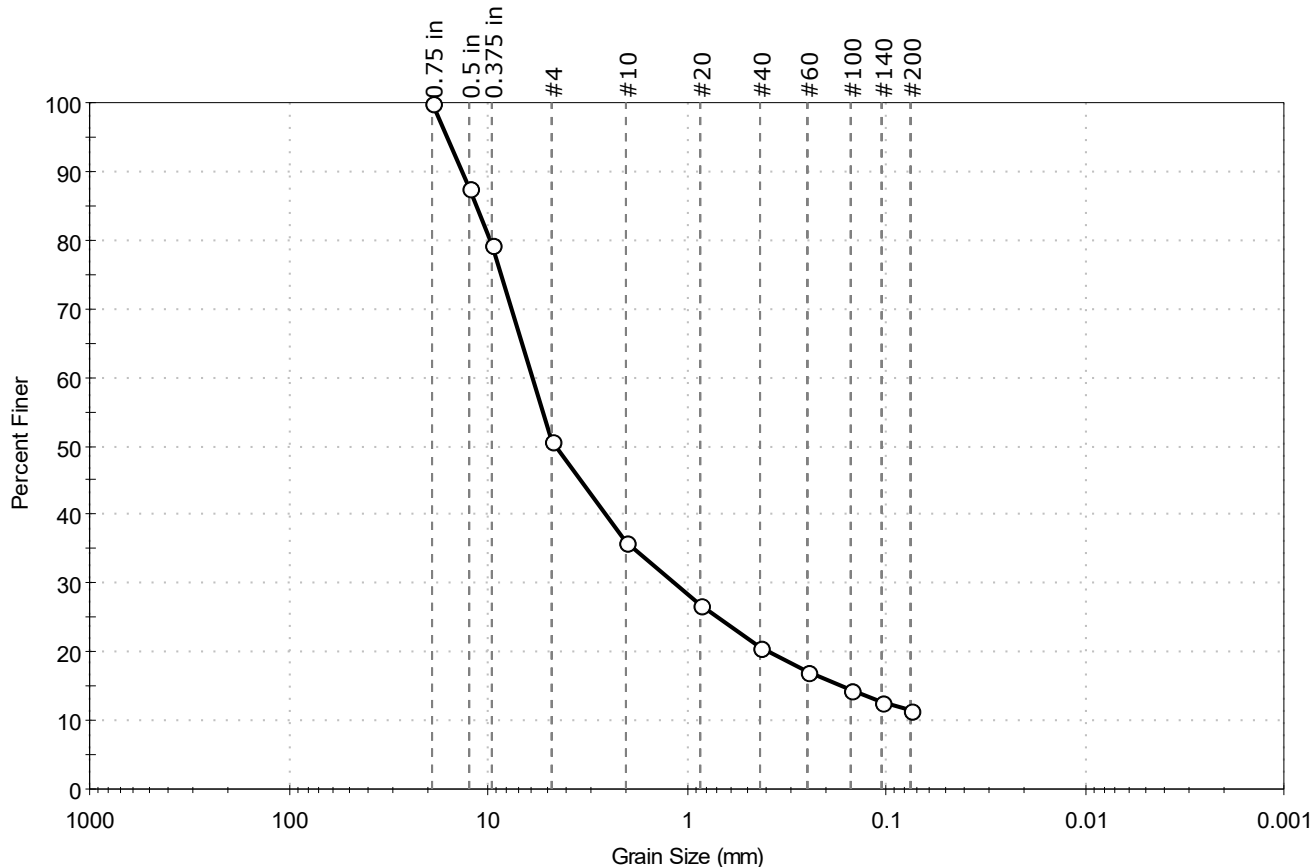
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-212	Sample Type:	jar
Sample ID:	7D	Test Date:	03/29/21
Depth :	30-32	Test Id:	613869
Test Comment:	---		
Visual Description:	Moist, gray gravel with silt and sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	49.2	39.3	11.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	88		
0.375 in	9.50	79		
#4	4.75	51		
#10	2.00	36		
#20	0.85	27		
#40	0.42	21		
#60	0.25	17		
#100	0.15	14		
#140	0.11	13		
#200	0.075	11		

Coefficients

D ₈₅ = 11.4629 mm	D ₃₀ = 1.1389 mm
D ₆₀ = 5.9326 mm	D ₁₅ = 0.1698 mm
D ₅₀ = 4.5243 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

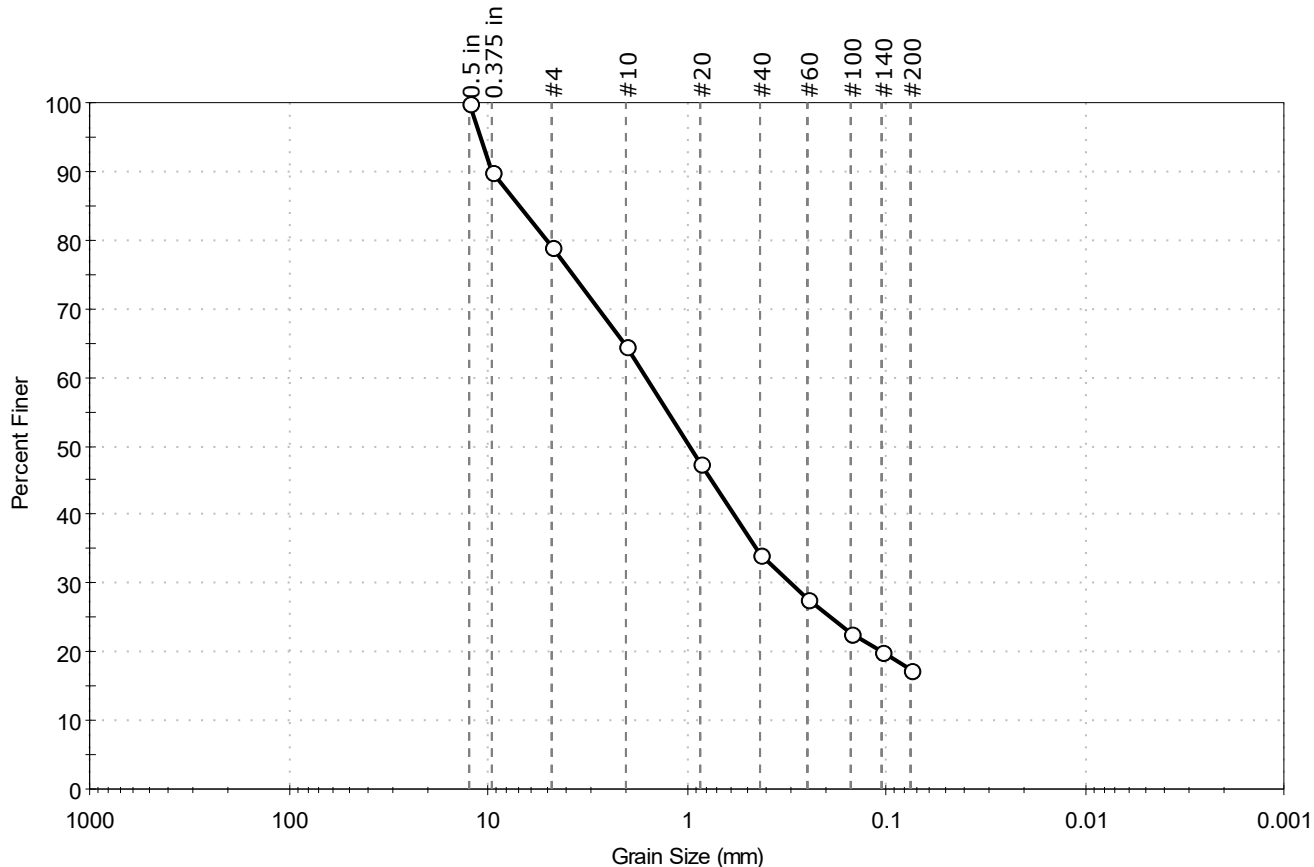
AASHTO Stone Fragments, Gravel and Sand (A-1-a (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-313370	
Project: I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location: Brewer, ME	Sample Type: jar	Tested By: GA
Boring ID: HB-BE-217	Test Date: 03/29/21	Checked By: emm
Sample ID: 2D	Test Id: 613870	
Depth: 5-6.8		
Test Comment: ---		
Visual Description: Moist, yellowish brown silty sand with gravel		
Sample Comment: ---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	20.9	61.8	17.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	90		
#4	4.75	79		
#10	2.00	65		
#20	0.85	47		
#40	0.42	34		
#60	0.25	28		
#100	0.15	23		
#140	0.11	20		
#200	0.075	17		

Coefficients

D ₈₅ = 6.8992 mm	D ₃₀ = 0.3021 mm
D ₆₀ = 1.5938 mm	D ₁₅ = N/A
D ₅₀ = 0.9676 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

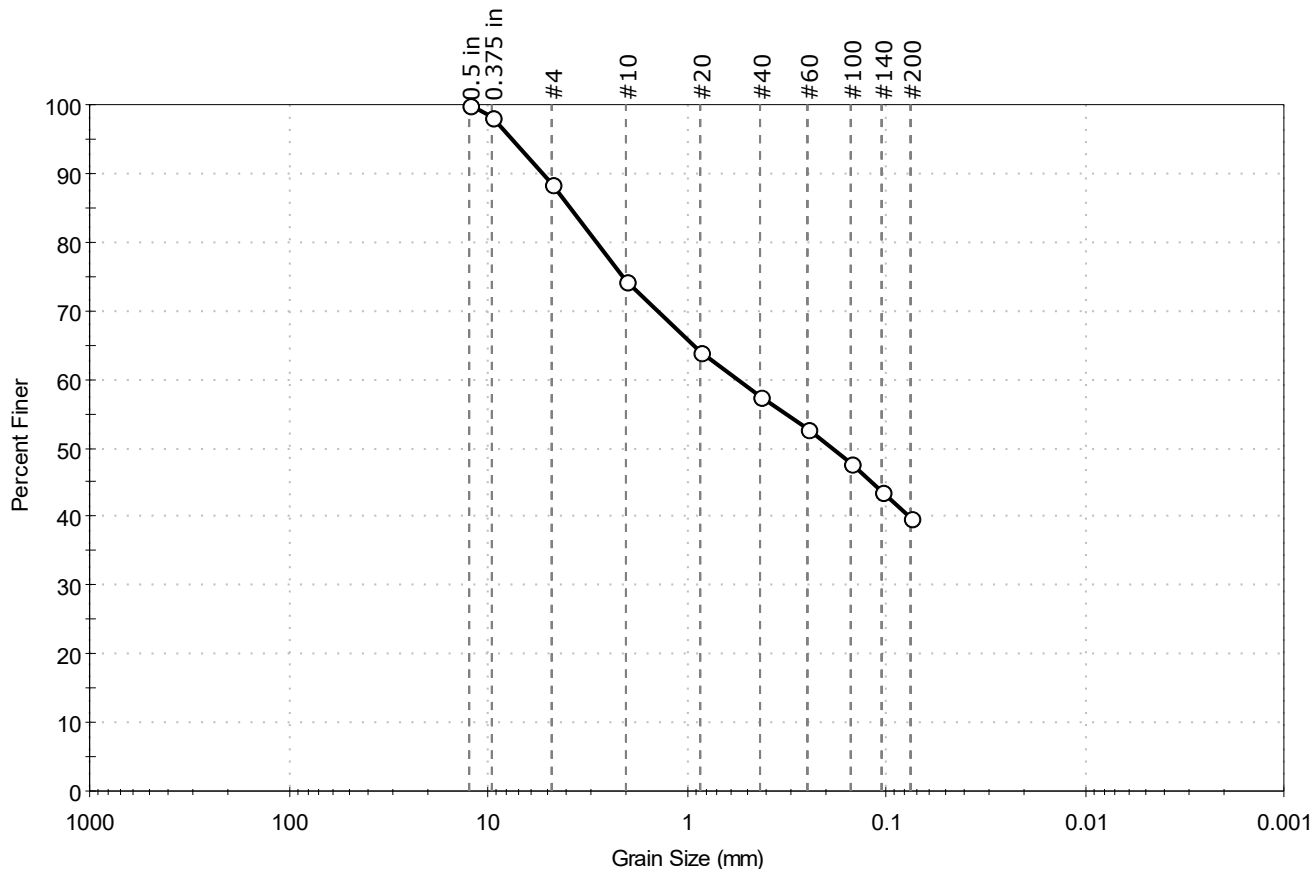
AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-217	Sample Type:	jar
Sample ID:	3D	Test Date:	03/29/21
Depth :	10-12	Test Id:	613871
Test Comment:	---		
Visual Description:	Moist, brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	11.5	48.8	39.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	89		
#10	2.00	74		
#20	0.85	64		
#40	0.42	57		
#60	0.25	53		
#100	0.15	48		
#140	0.11	44		
#200	0.075	40		

Coefficients

$D_{85} = 3.8378 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.5524 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.1894 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

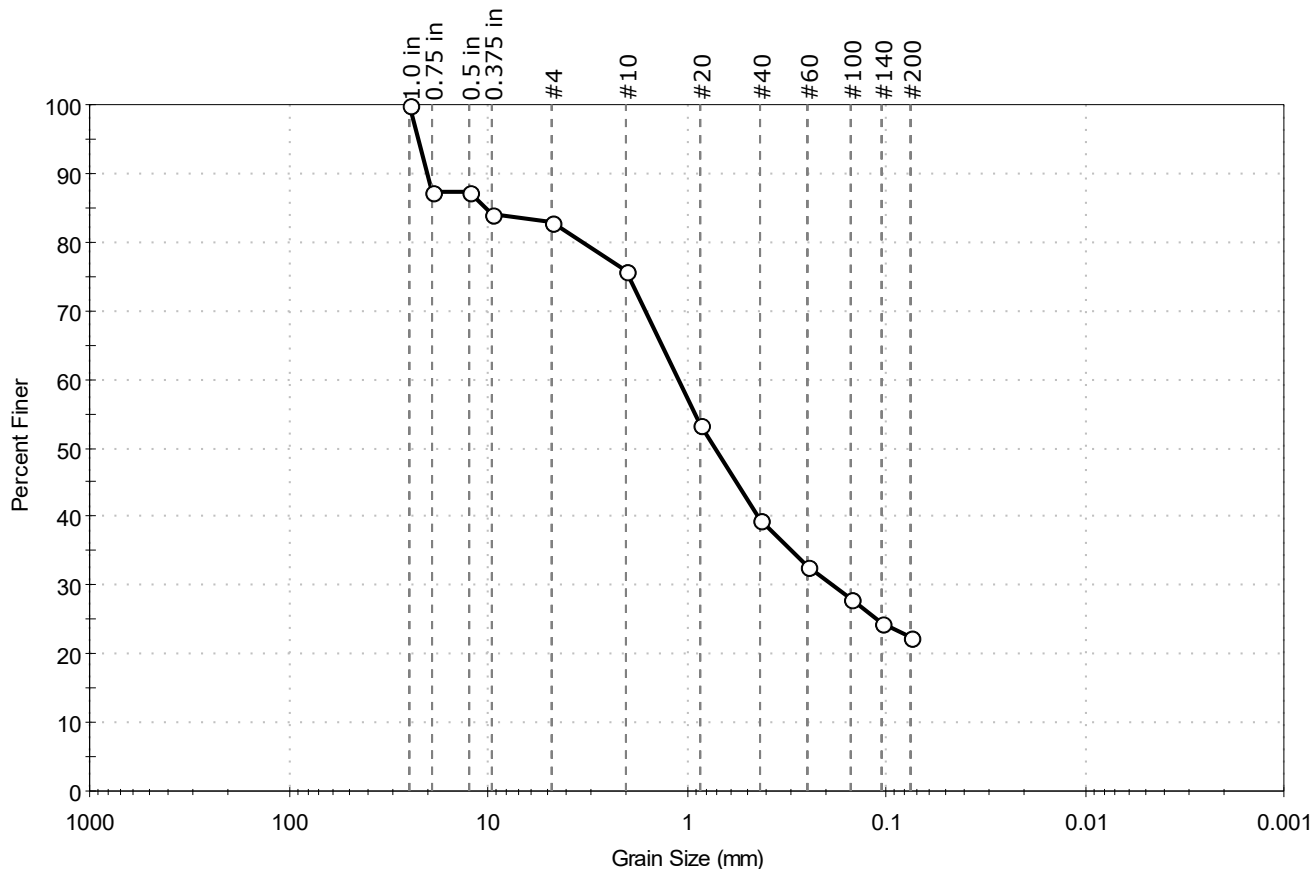
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-217	Sample Type:	jar
Sample ID:	4D	Test Date:	03/29/21
Depth :	15-17	Test Id:	613872
Test Comment:	---		
Visual Description:	Moist, olive brown clayey sand with gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	17.1	60.6	22.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	87		
0.5 in	12.50	87		
0.375 in	9.50	84		
#4	4.75	83		
#10	2.00	76		
#20	0.85	53		
#40	0.42	40		
#60	0.25	33		
#100	0.15	28		
#140	0.11	25		
#200	0.075	22		

Coefficients

D ₈₅ = 10.2490 mm	D ₃₀ = 0.1860 mm
D ₆₀ = 1.0932 mm	D ₁₅ = N/A
D ₅₀ = 0.7175 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

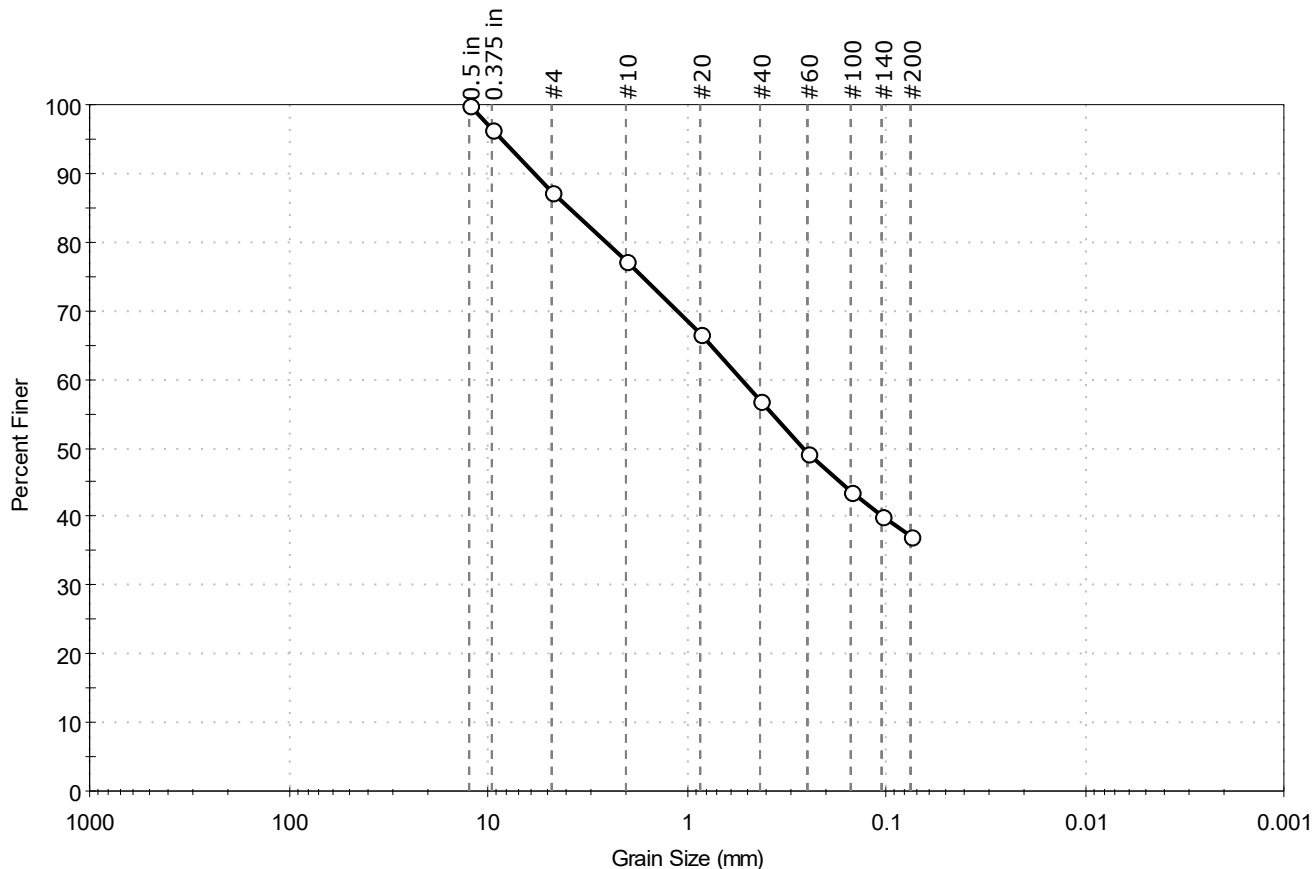
AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-217	Sample Type:	jar
Sample ID:	5D	Test Date:	03/29/21
Depth :	20-22	Test Id:	613873
Test Comment:	---		
Visual Description:	Moist, grayish brown clayey sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	12.7	50.1	37.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	96		
#4	4.75	87		
#10	2.00	77		
#20	0.85	67		
#40	0.42	57		
#60	0.25	49		
#100	0.15	44		
#140	0.11	40		
#200	0.075	37		

Coefficients

$D_{85} = 3.9135 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.5275 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.2613 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

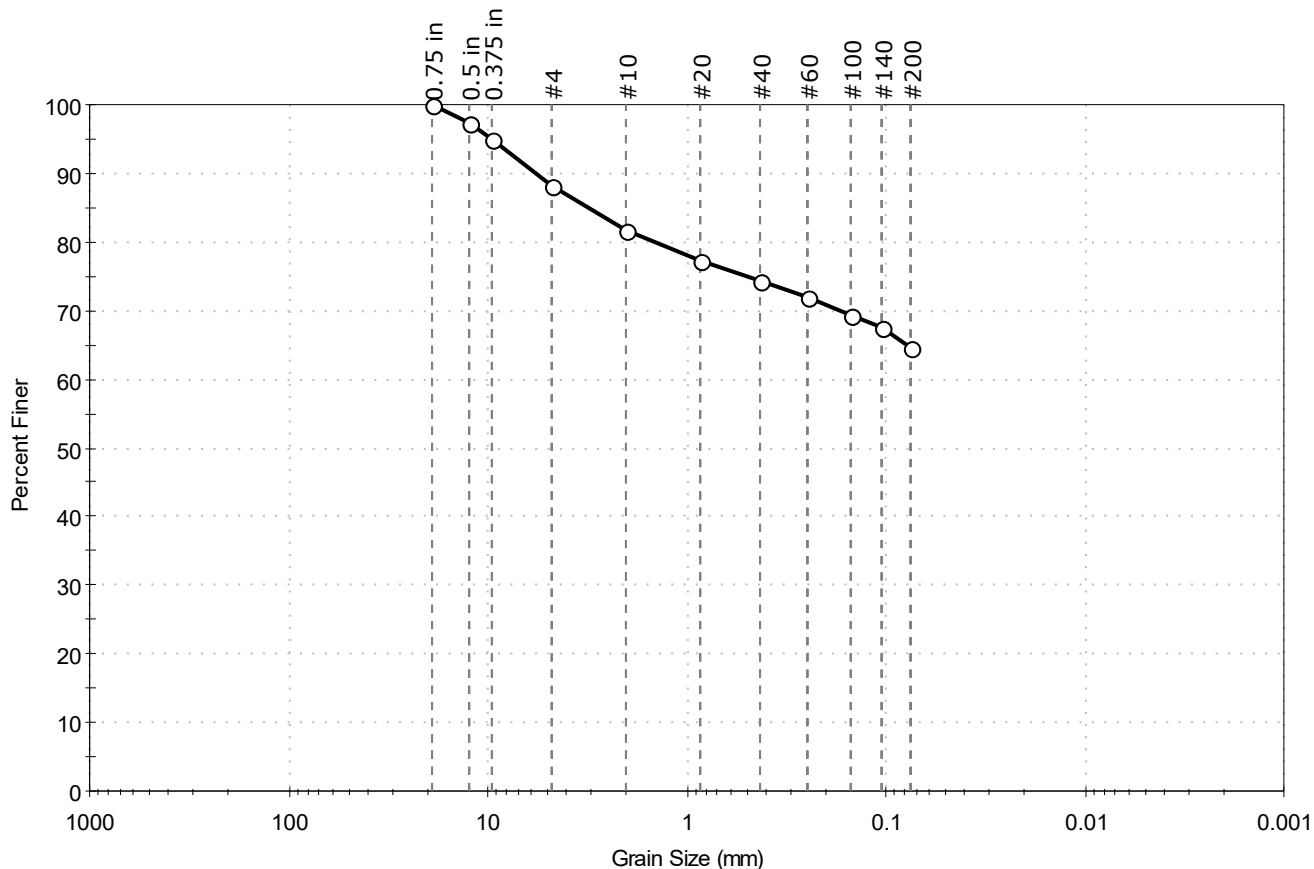
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-218	Sample Type:	jar
Sample ID:	2D	Test Date:	03/25/21
Depth :	5-7	Test Id:	613874
Test Comment:	---		
Visual Description:	Moist, light olive brown sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	11.7	23.6	64.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	97		
0.375 in	9.50	95		
#4	4.75	88		
#10	2.00	82		
#20	0.85	77		
#40	0.42	74		
#60	0.25	72		
#100	0.15	69		
#140	0.11	67		
#200	0.075	65		

Coefficients

$D_{85} = 3.0778 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

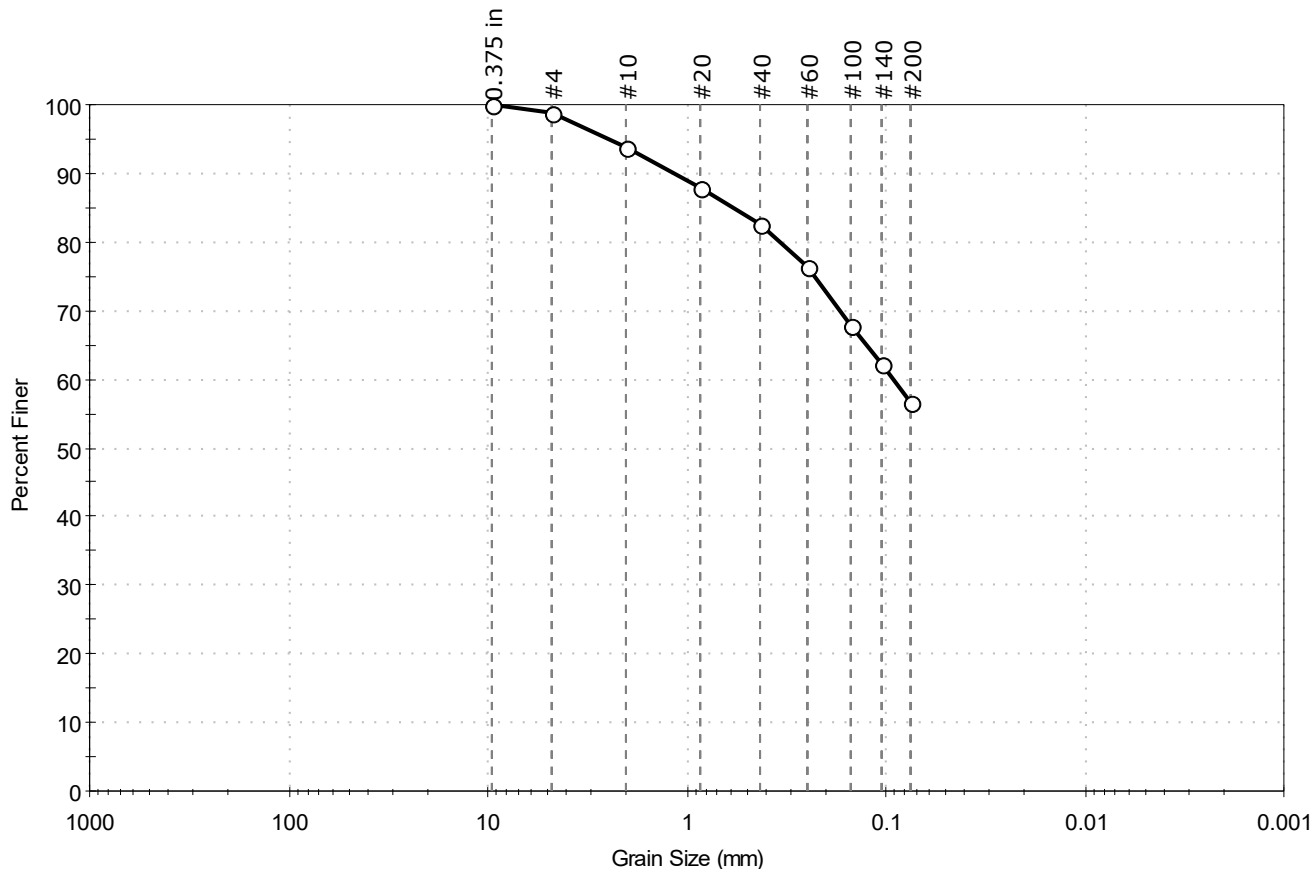
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-218	Sample Type:	jar
Sample ID:	3D	Test Date:	03/24/21
Depth :	10-12	Test Id:	613875
Test Comment:	---		
Visual Description:	Moist, brown sandy silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	1.1	42.1	56.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	94		
#20	0.85	88		
#40	0.42	83		
#60	0.25	76		
#100	0.15	68		
#140	0.11	62		
#200	0.075	57		

Coefficients

$D_{85} = 0.5901$ mm $D_{30} = \text{N/A}$
 $D_{60} = 0.0921$ mm $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

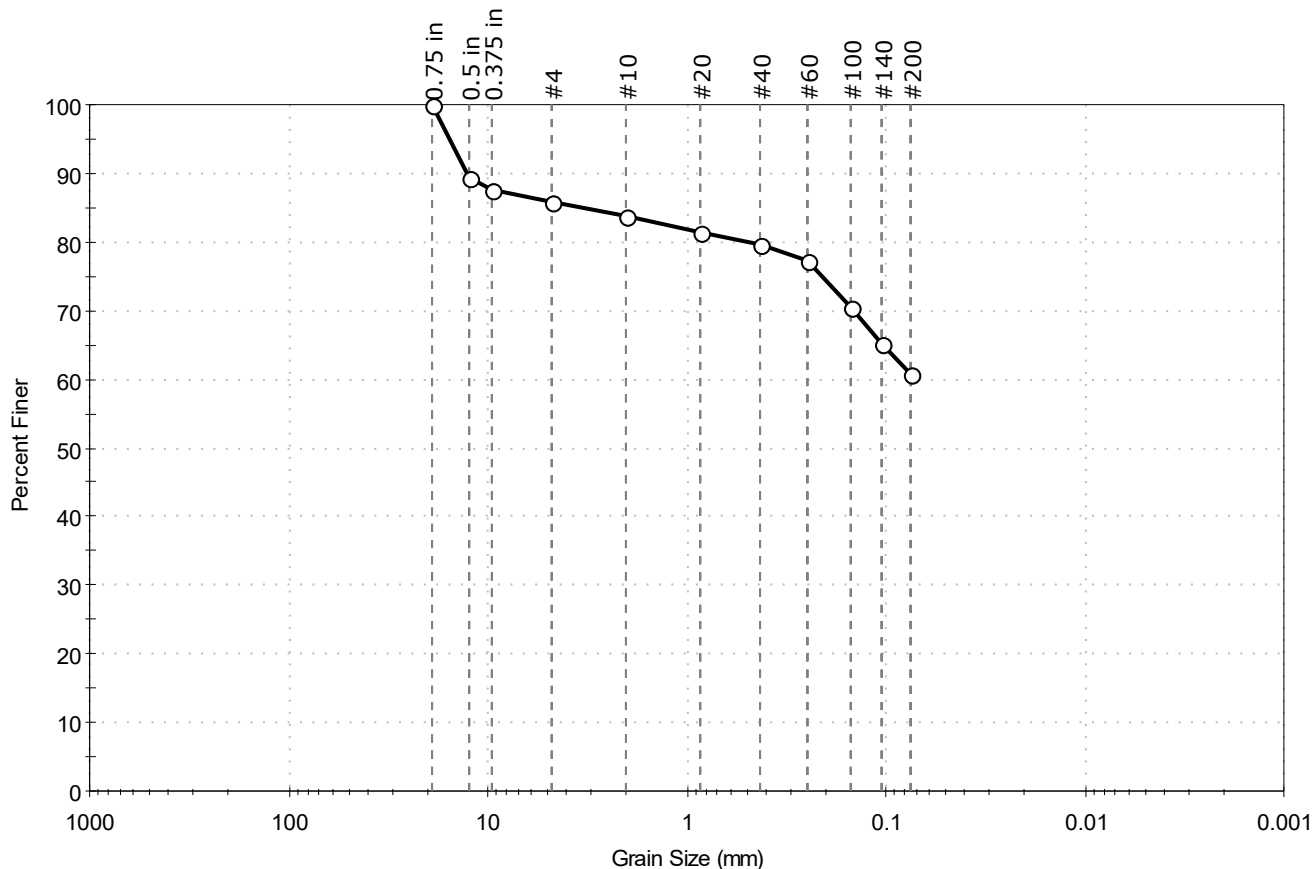
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client: Haley & Aldrich, Inc.	Project No: GTX-313370	
Project: I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location: Brewer, ME	Sample Type: jar	Tested By: ckg
Boring ID: HB-BE-218	Test Date: 03/24/21	Checked By: emm
Sample ID: 4D	Test Id: 613876	
Depth: 15-17		
Test Comment: ---		
Visual Description: Moist, grayish brown sandy silt		
Sample Comment: ---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	14.1	25.2	60.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	90		
0.375 in	9.50	88		
#4	4.75	86		
#10	2.00	84		
#20	0.85	81		
#40	0.42	80		
#60	0.25	77		
#100	0.15	70		
#140	0.11	65		
#200	0.075	61		

Coefficients

$D_{85} = 3.2272 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

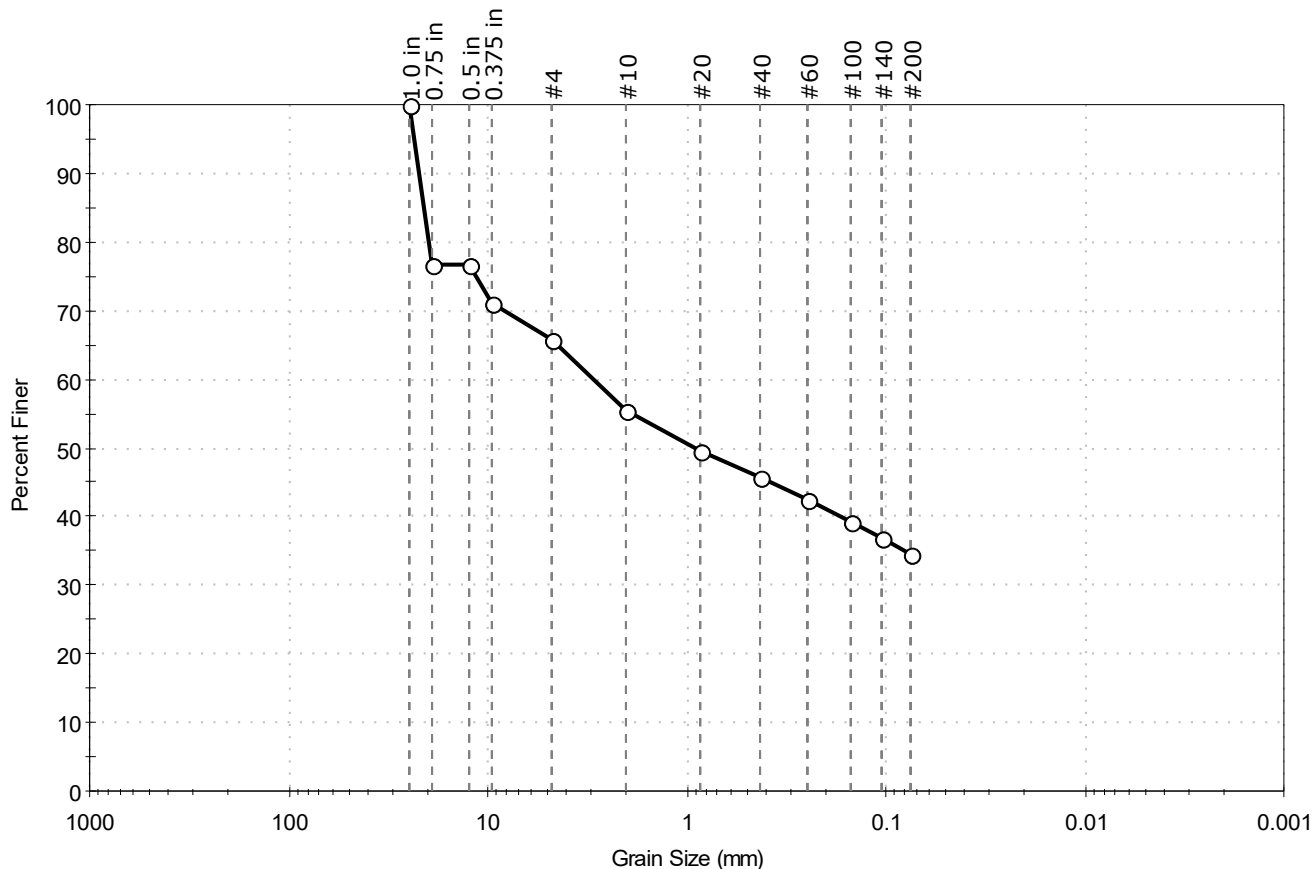
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-218	Sample Type:	jar
Sample ID:	5D	Test Date:	03/29/21
Depth :	20-22	Test Id:	613877
Test Comment:	---		
Visual Description:	Moist, grayish brown clayey gravel with sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	34.3	31.3	34.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	77		
0.5 in	12.50	77		
0.375 in	9.50	71		
#4	4.75	66		
#10	2.00	55		
#20	0.85	50		
#40	0.42	46		
#60	0.25	42		
#100	0.15	39		
#140	0.11	37		
#200	0.075	34		

Coefficients

$D_{85} = 20.9485 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 2.9426 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.8887 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

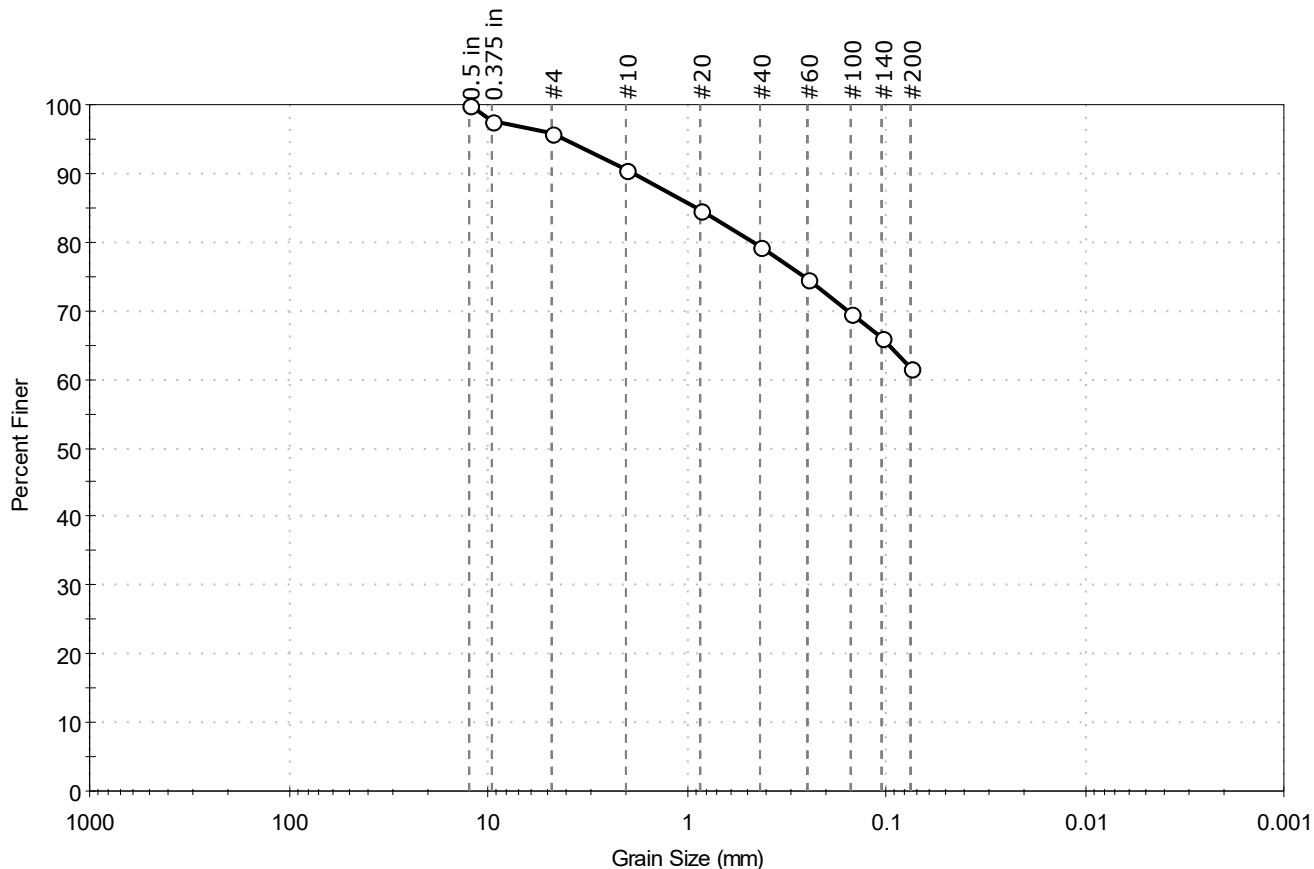
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-228	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-7	Test Id:	613878
Test Comment:	---		
Visual Description:	Moist, olive brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	4.2	34.2	61.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	96		
#10	2.00	91		
#20	0.85	85		
#40	0.42	79		
#60	0.25	75		
#100	0.15	70		
#140	0.11	66		
#200	0.075	62		

Coefficients

$D_{85} = 0.8808 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

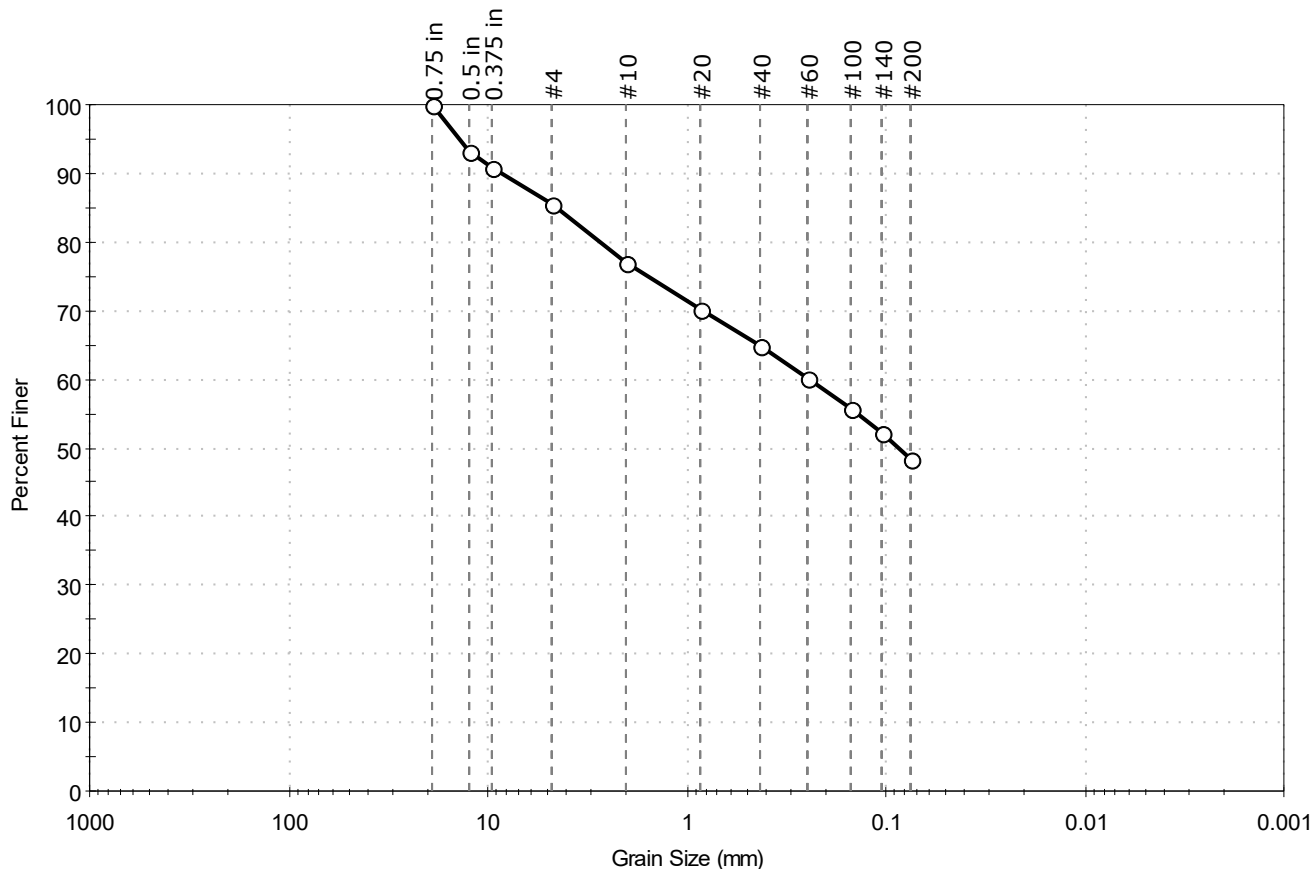
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-228	Sample Type:	jar
Sample ID:	3D	Test Date:	03/29/21
Depth :	10-10.3	Test Id:	613879
Test Comment:	---		
Visual Description:	Moist, pale brown clayey sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	14.4	37.2	48.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	91		
#4	4.75	86		
#10	2.00	77		
#20	0.85	70		
#40	0.42	65		
#60	0.25	60		
#100	0.15	56		
#140	0.11	52		
#200	0.075	48		

Coefficients

$D_{85} = 4.4645 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.2426 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.0867 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

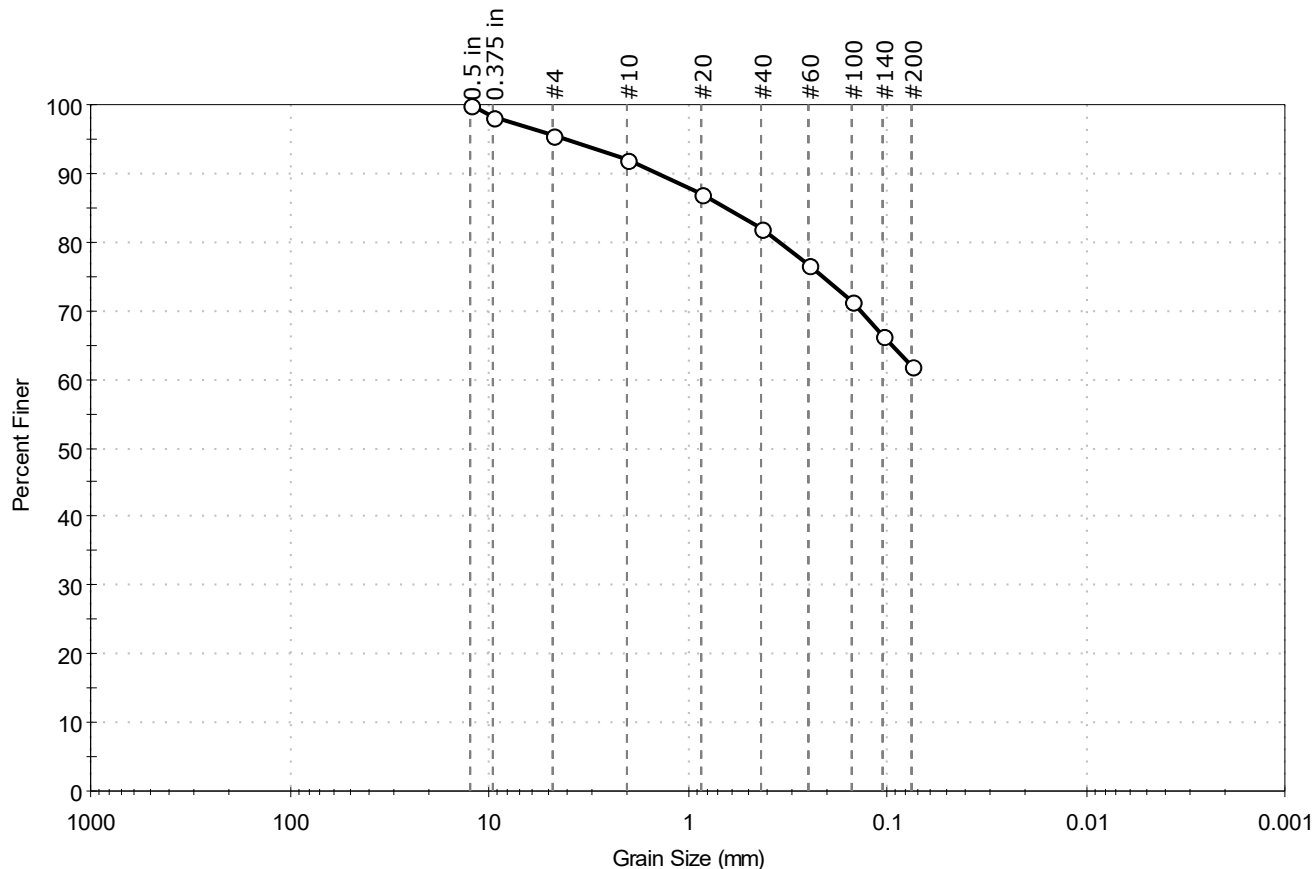
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-229	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-7	Test Id:	613880
Test Comment:	---		
Visual Description:	Moist, olive brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	4.3	33.6	62.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	98		
#4	4.75	96		
#10	2.00	92		
#20	0.85	87		
#40	0.42	82		
#60	0.25	77		
#100	0.15	71		
#140	0.11	66		
#200	0.075	62		

Coefficients

$D_{85} = 0.6508 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

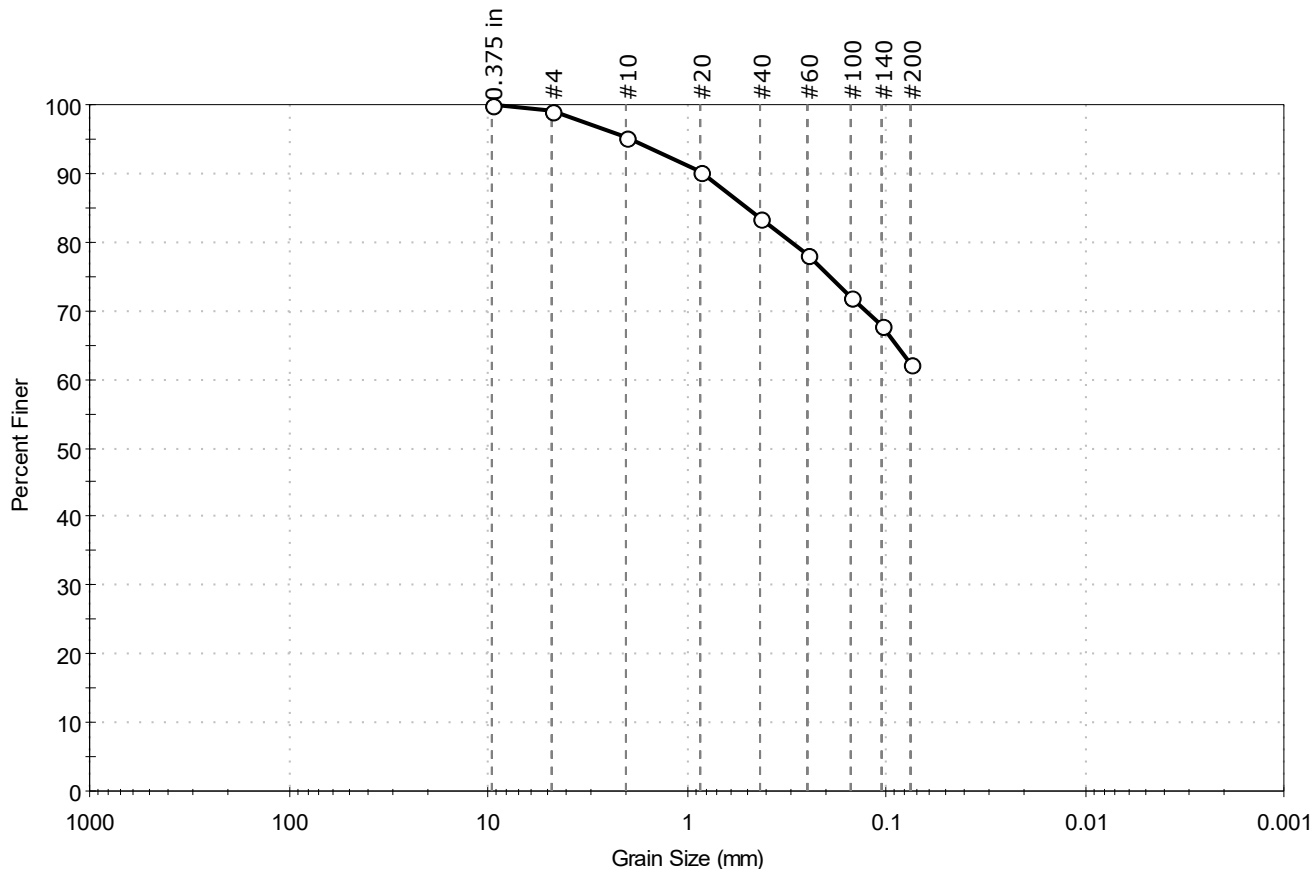
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-230	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-6.7	Test Id:	613881
Test Comment:	---		
Visual Description:	Moist, yellowish brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	1.0	36.8	62.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	95		
#20	0.85	90		
#40	0.42	84		
#60	0.25	78		
#100	0.15	72		
#140	0.11	68		
#200	0.075	62		

Coefficients

$D_{85} = 0.4948$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

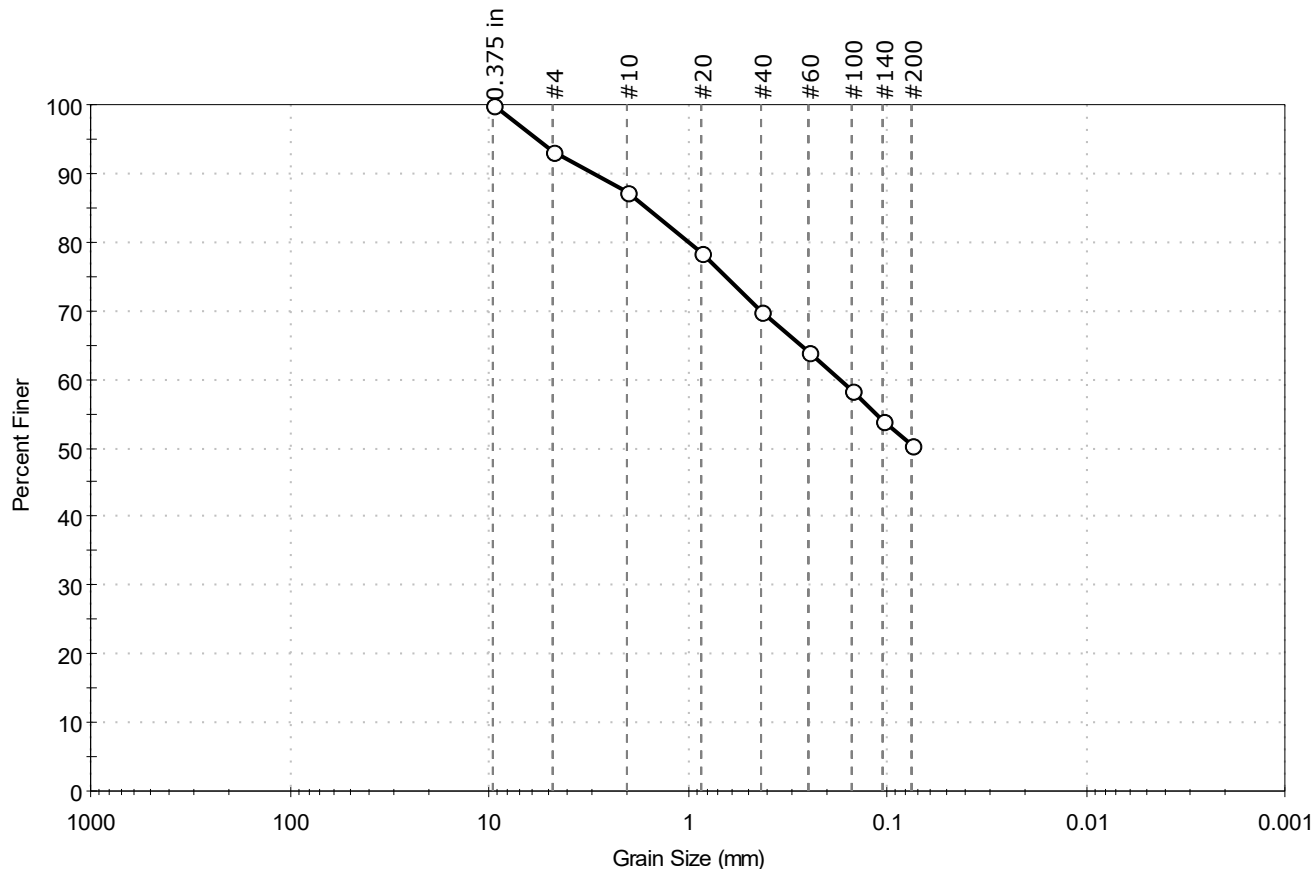
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-231	Sample Type:	jar
Sample ID:	2D	Test Date:	03/26/21
Depth :	5-7	Test Id:	613882
Test Comment:	---		
Visual Description:	Moist, pale brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.9	42.7	50.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	93		
#10	2.00	87		
#20	0.85	78		
#40	0.42	70		
#60	0.25	64		
#100	0.15	58		
#140	0.11	54		
#200	0.075	50		

Coefficients

D ₈₅ = 1.5905 mm	D ₃₀ = N/A
D ₆₀ = 0.1731 mm	D ₁₅ = N/A
D ₅₀ = N/A	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification

ASTM N/A

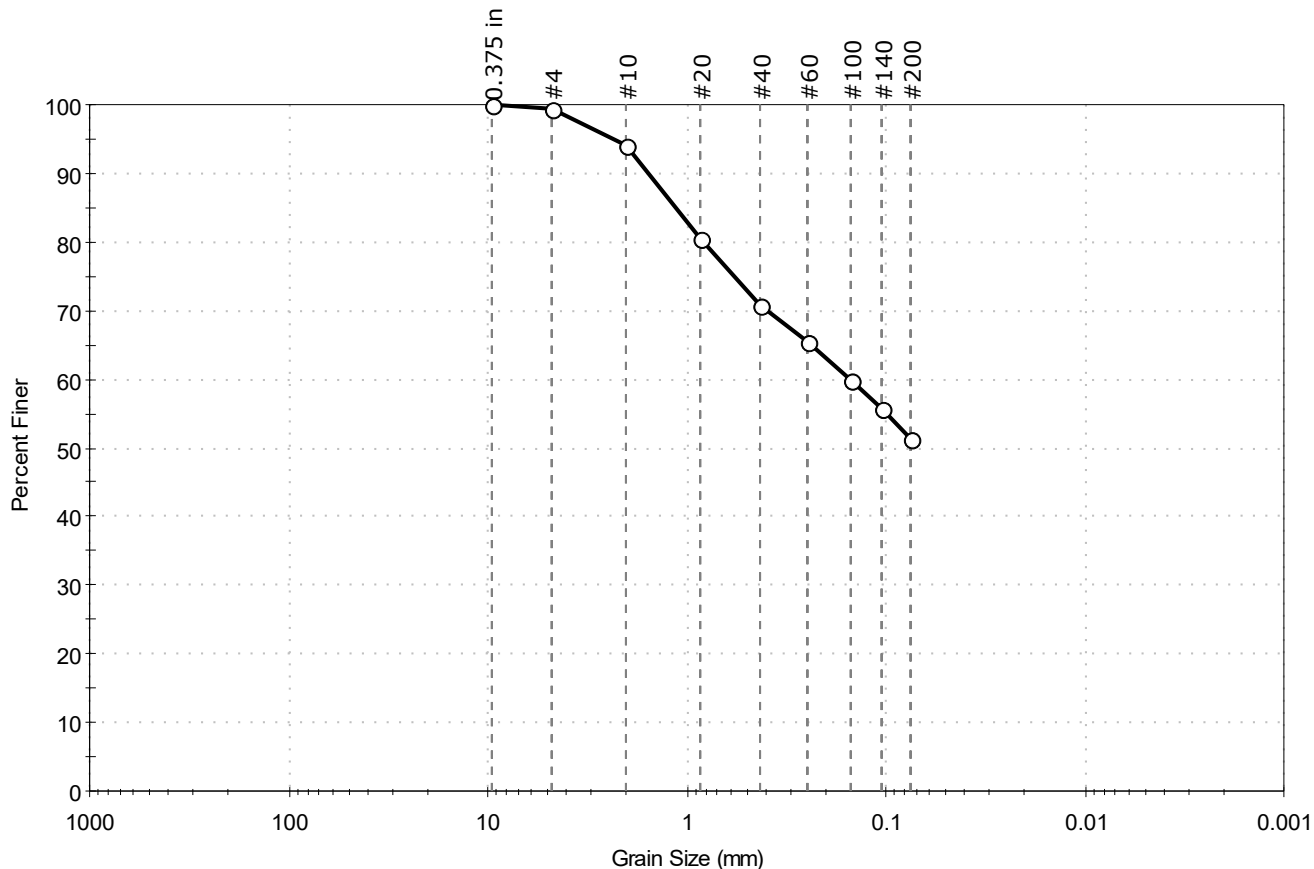
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-232	Sample Type:	jar
Sample ID:	3D	Test Date:	03/29/21
Depth :	10-12	Test Id:	613883
Test Comment:	---		
Visual Description:	Moist, grayish brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.7	47.9	51.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	94		
#20	0.85	80		
#40	0.42	71		
#60	0.25	65		
#100	0.15	60		
#140	0.11	56		
#200	0.075	51		

Coefficients

$D_{85} = 1.1290 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.1521 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

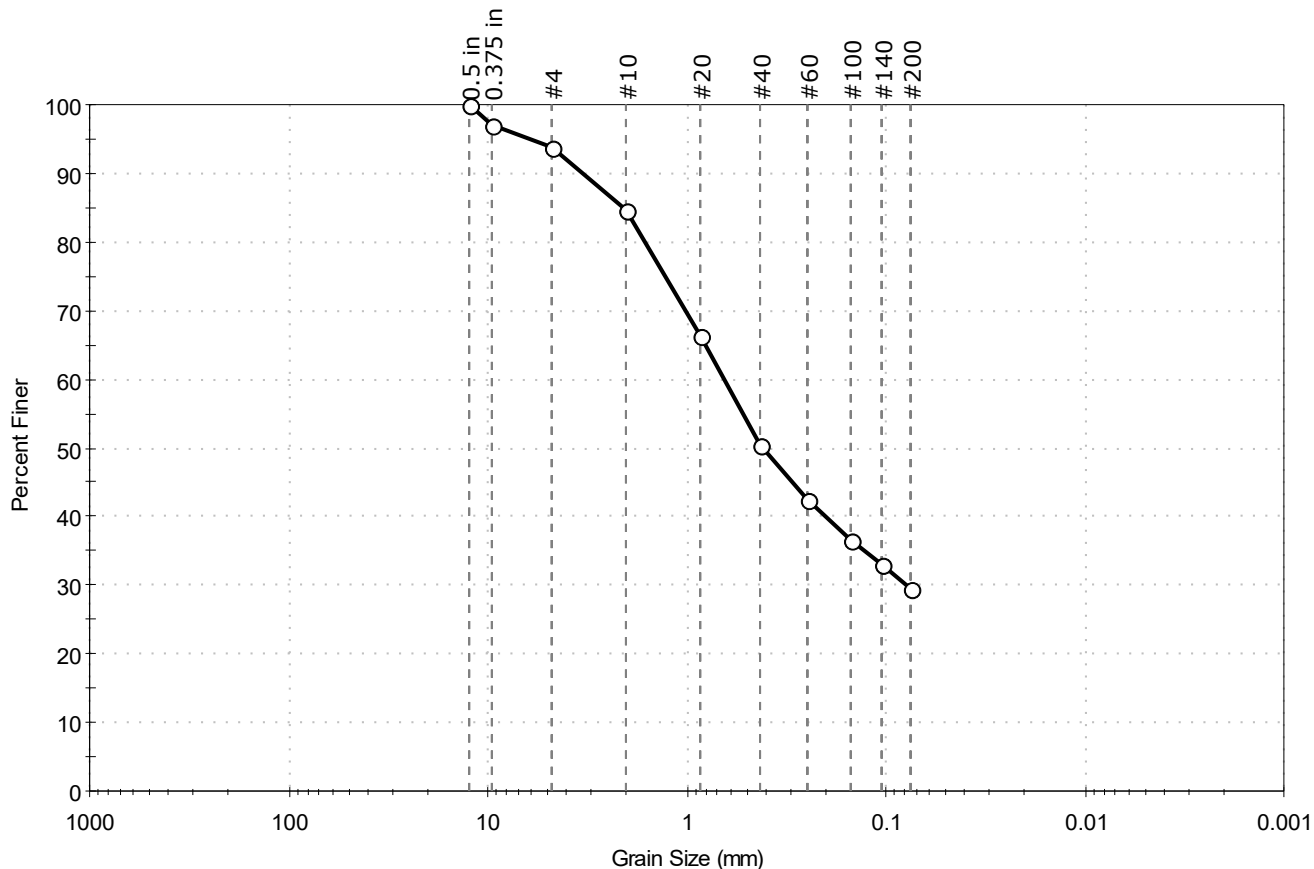
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-234	Sample Type:	jar
Sample ID:	4D	Test Date:	03/29/21
Depth :	15-17	Test Id:	613884
Test Comment:	---		
Visual Description:	Moist, olive brown clayey sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	6.2	64.2	29.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	97		
#4	4.75	94		
#10	2.00	85		
#20	0.85	66		
#40	0.42	50		
#60	0.25	43		
#100	0.15	36		
#140	0.11	33		
#200	0.075	30		

Coefficients

$D_{85} = 2.0389$ mm $D_{30} = 0.0781$ mm
 $D_{60} = 0.6448$ mm $D_{15} = \text{N/A}$
 $D_{50} = 0.4146$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

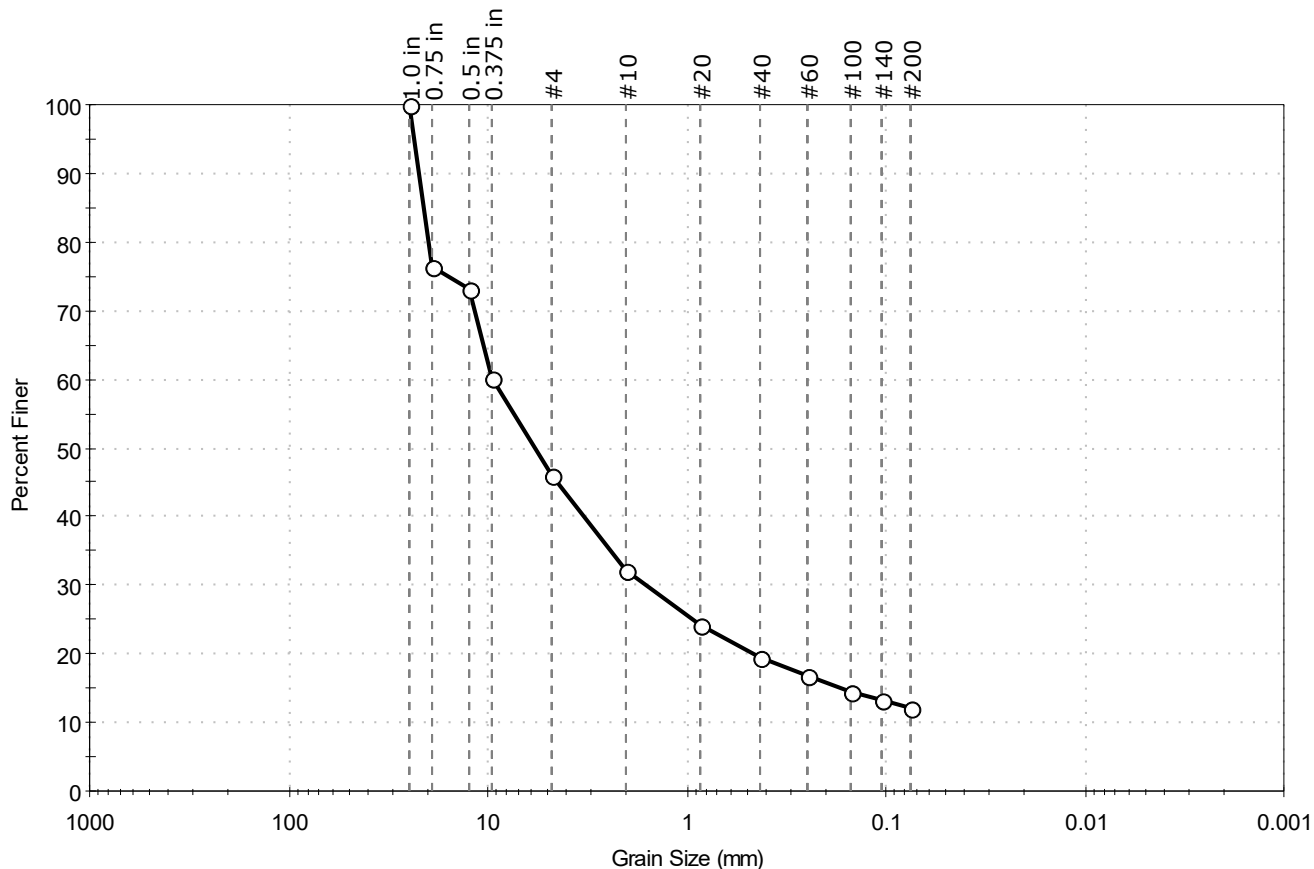
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-234	Sample Type:	jar
Sample ID:	5D	Test Date:	03/29/21
Depth :	20-20.4	Test Id:	613885
Test Comment:	---		
Visual Description:	Moist, gray gravel with clay and sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	54.0	34.1	11.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.0 in	25.00	100		
0.75 in	19.00	76		
0.5 in	12.50	73		
0.375 in	9.50	60		
#4	4.75	46		
#10	2.00	32		
#20	0.85	24		
#40	0.42	19		
#60	0.25	17		
#100	0.15	14		
#140	0.11	13		
#200	0.075	12		

Coefficients

$D_{85} = 21.0027$ mm $D_{30} = 1.6008$ mm
 $D_{60} = 9.4169$ mm $D_{15} = 0.1687$ mm
 $D_{50} = 5.7571$ mm $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

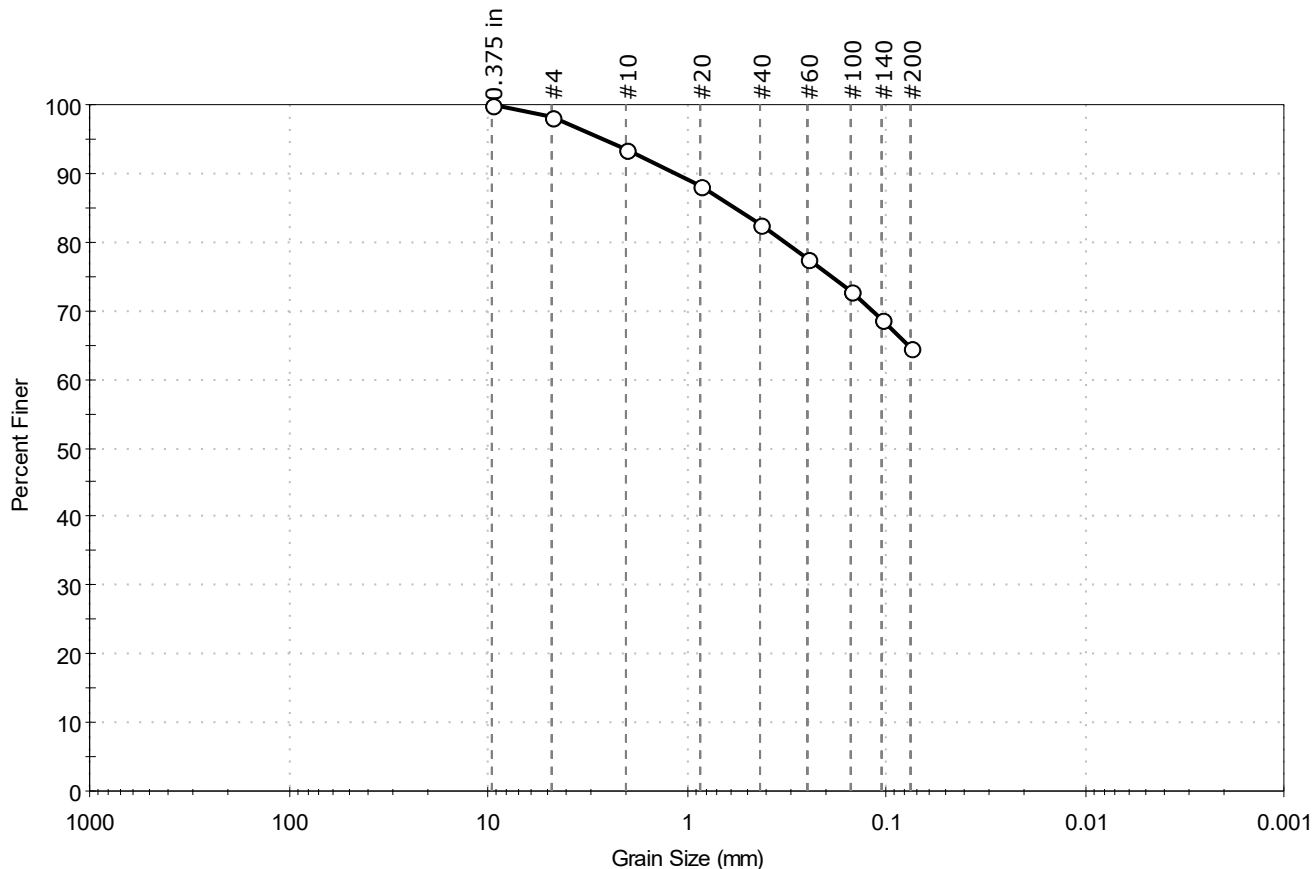
AASHTO Stone Fragments, Gravel and Sand (A-1-a (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-235	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-7	Test Id:	613886
Test Comment:	---		
Visual Description:	Moist, olive brown sandy clay		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	1.7	33.7	64.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	98		
#10	2.00	94		
#20	0.85	88		
#40	0.42	83		
#60	0.25	78		
#100	0.15	73		
#140	0.11	69		
#200	0.075	65		

Coefficients

$D_{85} = 0.5704$ mm $D_{30} = \text{N/A}$
 $D_{60} = \text{N/A}$ $D_{15} = \text{N/A}$
 $D_{50} = \text{N/A}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

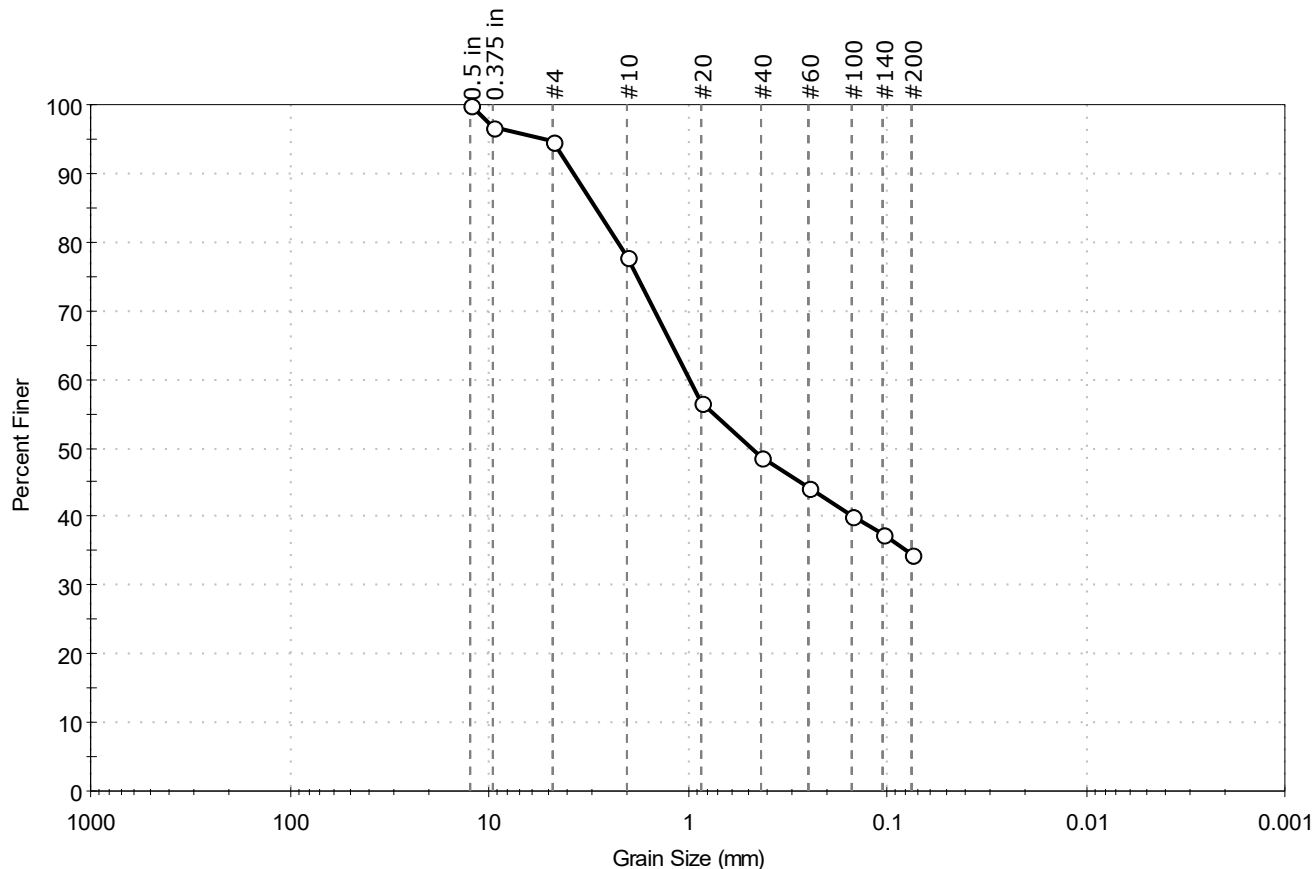
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-236	Sample Type:	jar
Sample ID:	3D	Test Date:	03/29/21
Depth :	10-12	Test Id:	613887
Test Comment:	---		
Visual Description:	Moist, grayish brown clayey sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	5.2	60.4	34.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.5 in	12.50	100		
0.375 in	9.50	97		
#4	4.75	95		
#10	2.00	78		
#20	0.85	57		
#40	0.42	49		
#60	0.25	44		
#100	0.15	40		
#140	0.11	37		
#200	0.075	34		

Coefficients

$D_{85} = 2.8702 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.9749 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.4778 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

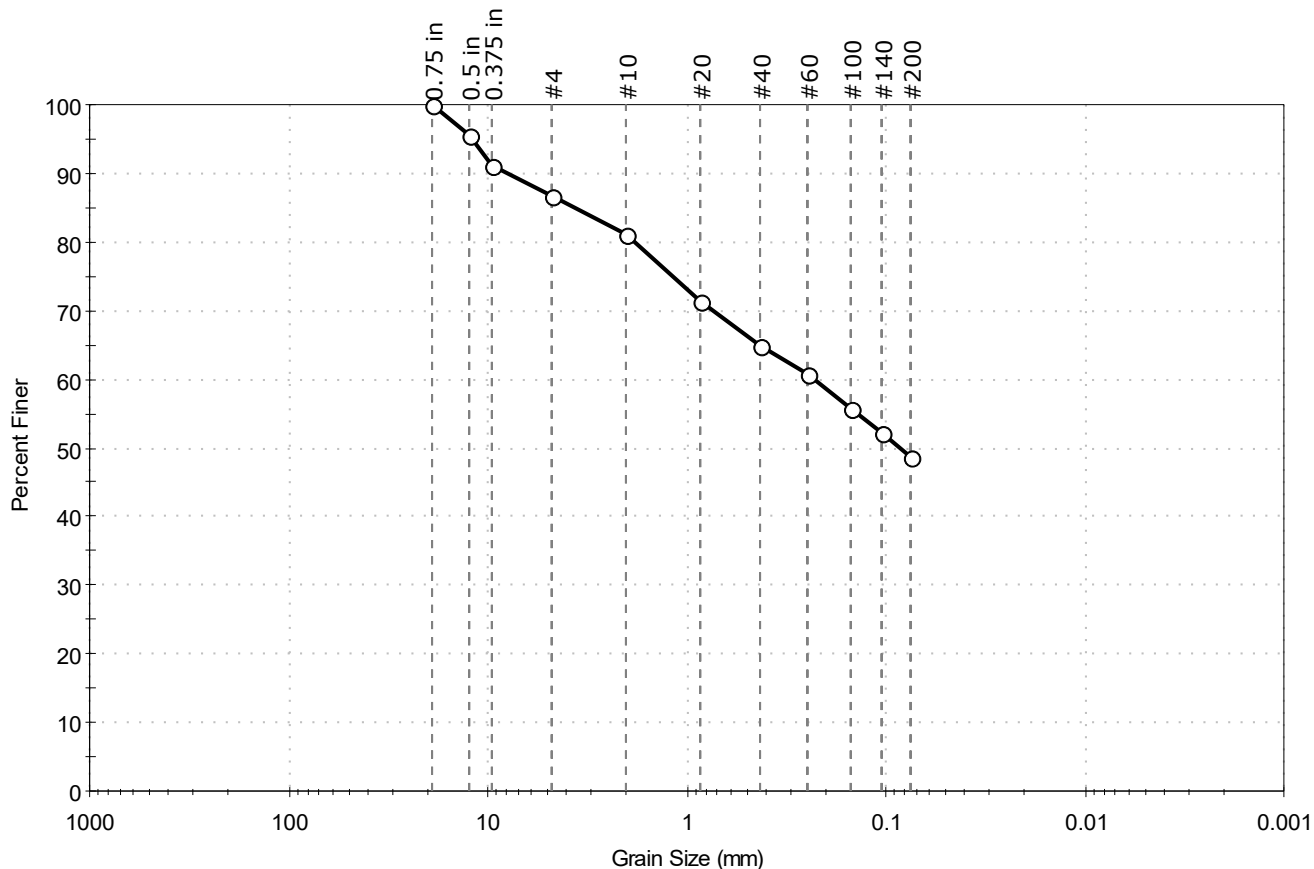
AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-237	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-7	Test Id:	613888
Test Comment:	---		
Visual Description:	Moist, grayish brown clayey sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	13.3	38.0	48.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	96		
0.375 in	9.50	91		
#4	4.75	87		
#10	2.00	81		
#20	0.85	71		
#40	0.42	65		
#60	0.25	61		
#100	0.15	56		
#140	0.11	52		
#200	0.075	49		

Coefficients

$D_{85} = 3.6512 \text{ mm}$ $D_{30} = \text{N/A}$
 $D_{60} = 0.2307 \text{ mm}$ $D_{15} = \text{N/A}$
 $D_{50} = 0.0857 \text{ mm}$ $D_{10} = \text{N/A}$
 $C_u = \text{N/A}$ $C_c = \text{N/A}$

Classification

ASTM N/A

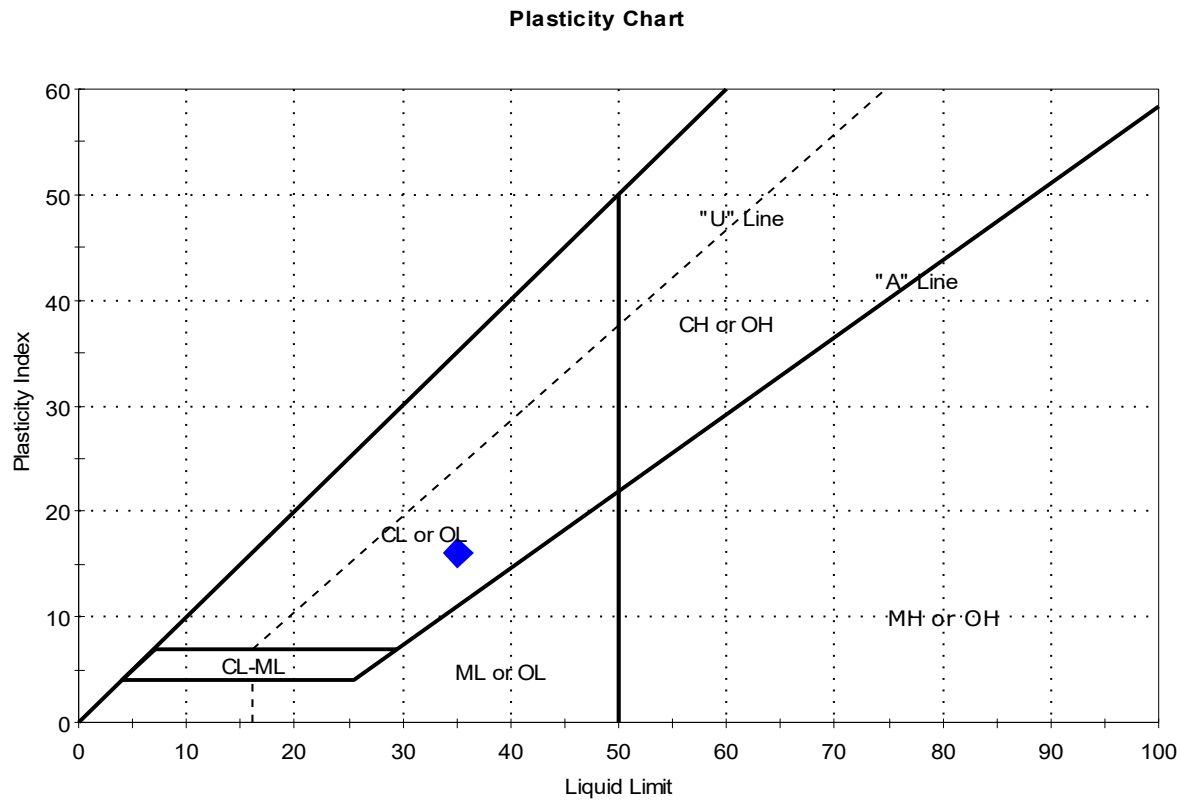
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	BB-BEB-101	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/29/19	Checked By:	bfs
Depth :	5-7 ft	Test Id:	513660		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	B-BEB-10	5-7 ft	31	35	19	16	0.8	

Sample Prepared using the WET method

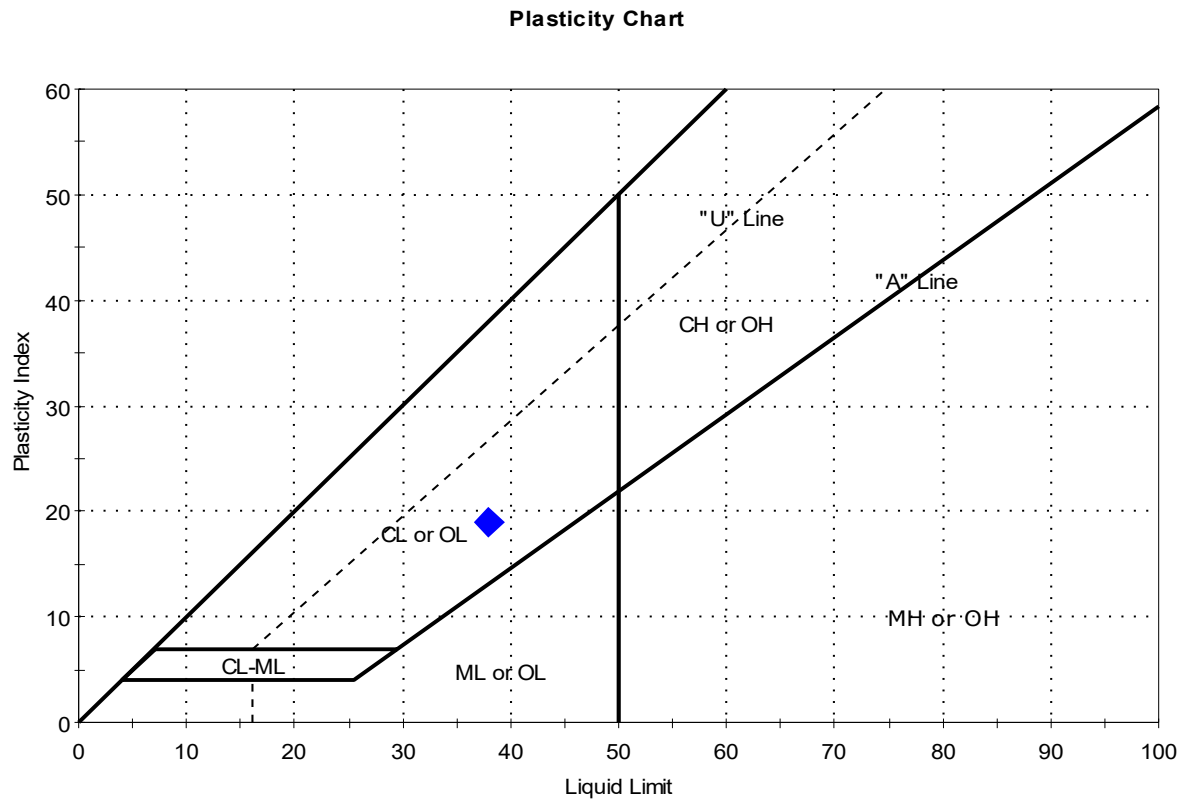
Dry Strength: VERY HIGH

Dilatancy: n/a

Toughness: n/a

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	BB-BEB-103	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/30/19	Checked By:	bfs
Depth :	10-12 ft	Test Id:	513654		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	B-BEB-10	10-12 ft	36	38	19	19	0.9	

Sample Prepared using the WET method

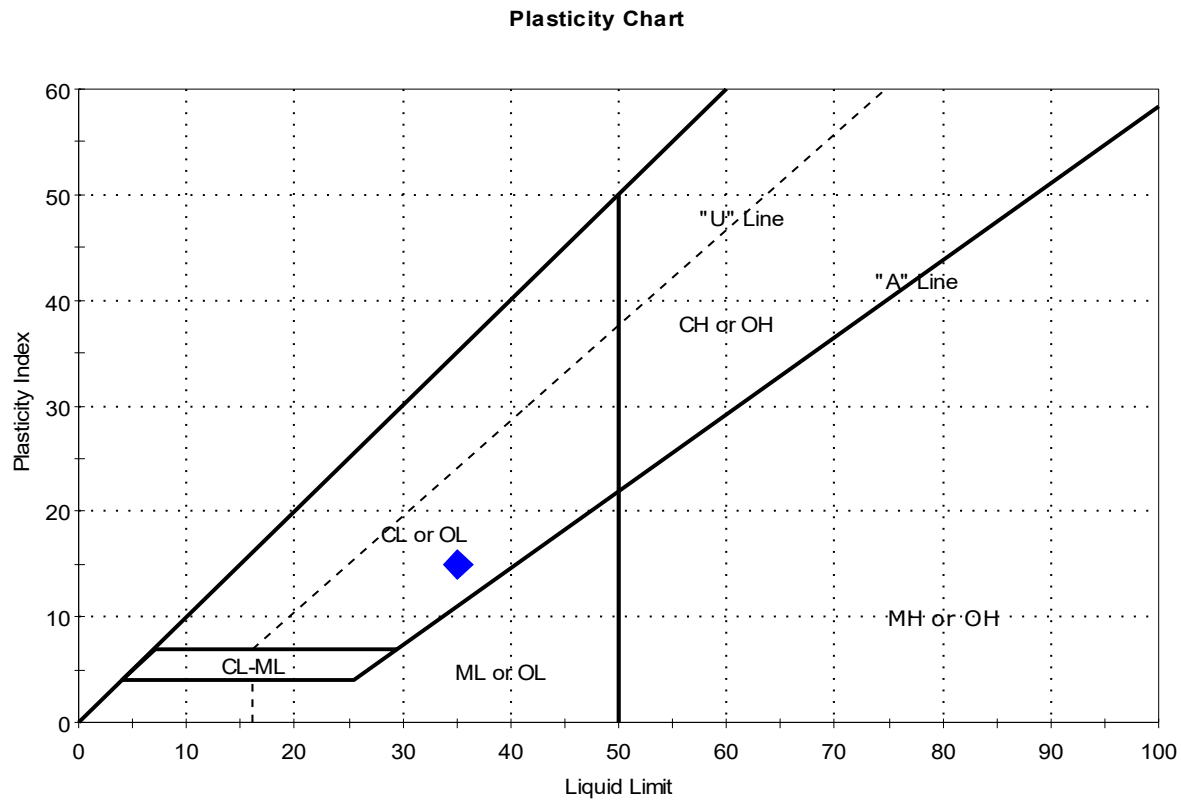
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	BB-BEB-104	Sample Type:	tube	Tested By:	cam
Sample ID:	3U	Test Date:	07/26/19	Checked By:	bfs
Depth :	15-17 ft	Test Id:	513664		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	3U	B-BEB-10	15-17 ft	34	35	20	15	0.9	

Sample Prepared using the WET method

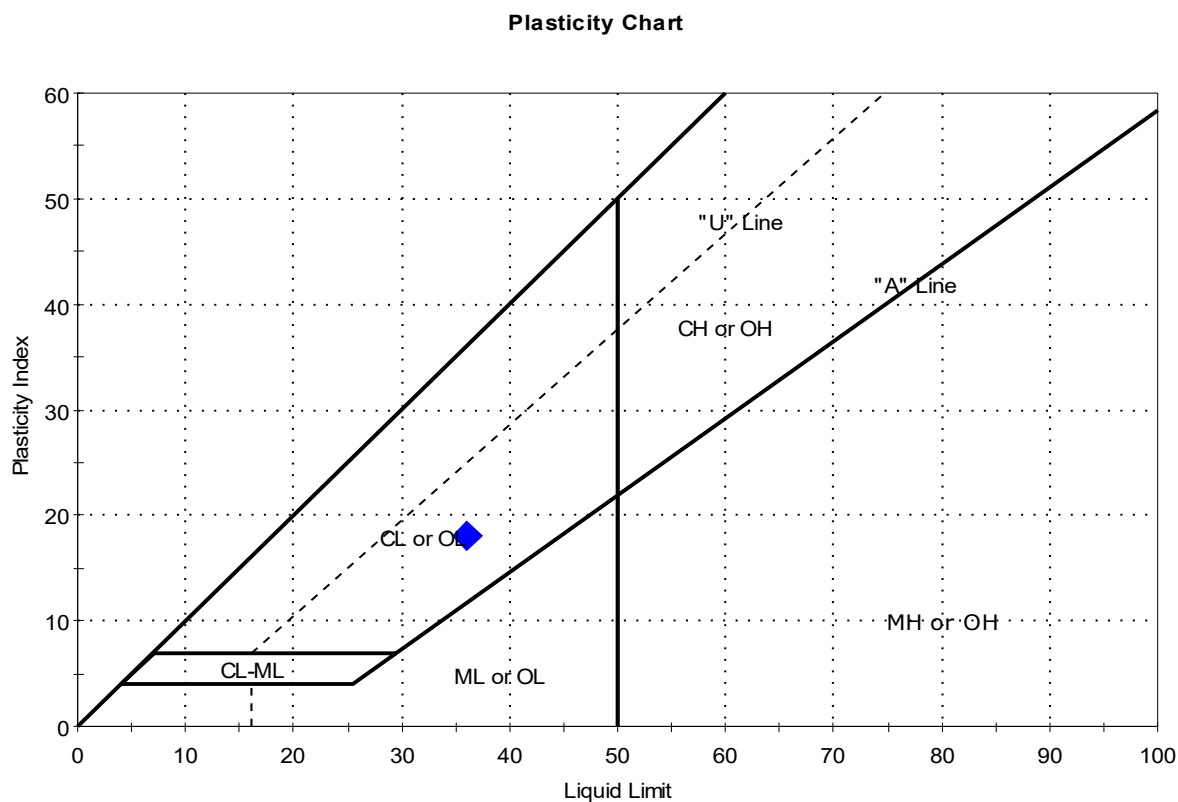
Dry Strength: HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 2)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313196
Boring ID:	BB-BEB-202	Sample Type:	tube
Sample ID:	U1	Test Date:	03/22/21
Depth :	5-7 ft	Test Id:	611802
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	B-BEB-20	5-7 ft	33	36	18	18	0.8	

Sample Prepared using the WET method

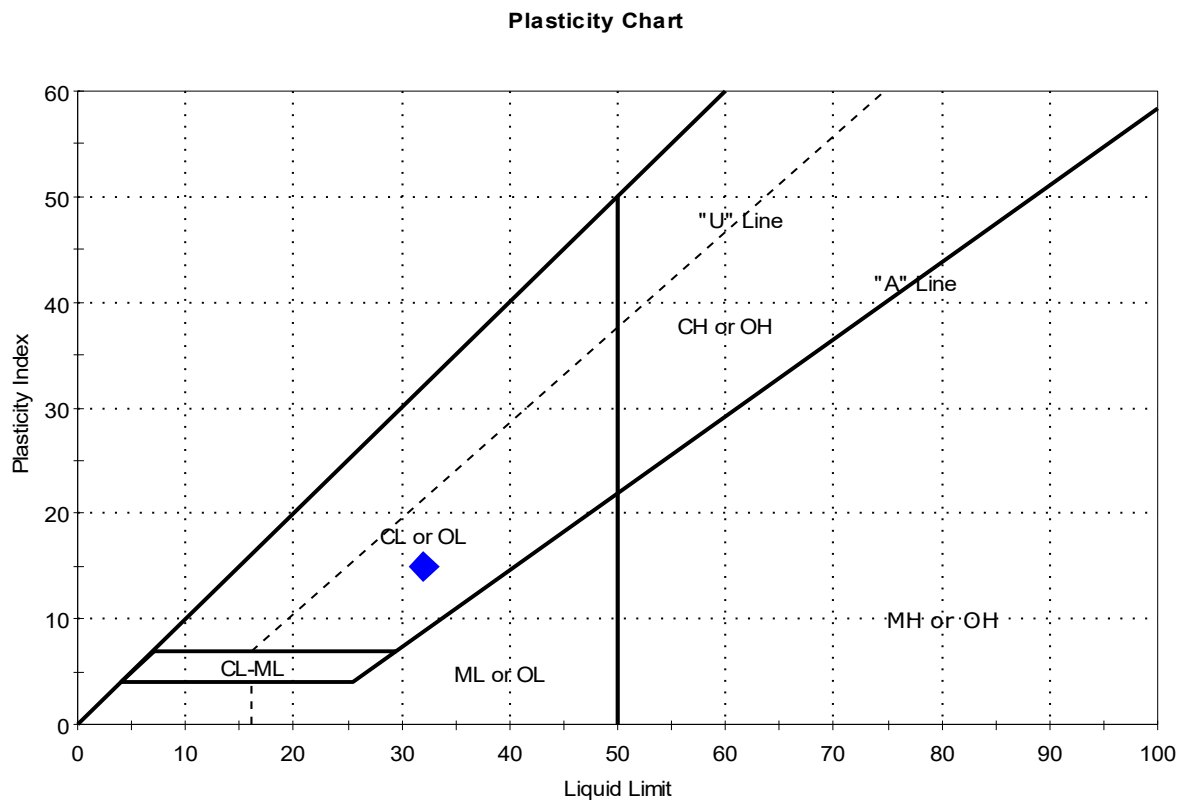
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 2)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313196
Boring ID:	BB-BEB-202	Sample Type:	tube
Sample ID:	U2	Test Date:	03/22/21
Depth :	15-17 ft	Test Id:	611803
Test Comment:	---		
Visual Description:	Wet, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	B-BEB-20	15-17 ft	36	32	17	15	1.3	

Sample Prepared using the WET method

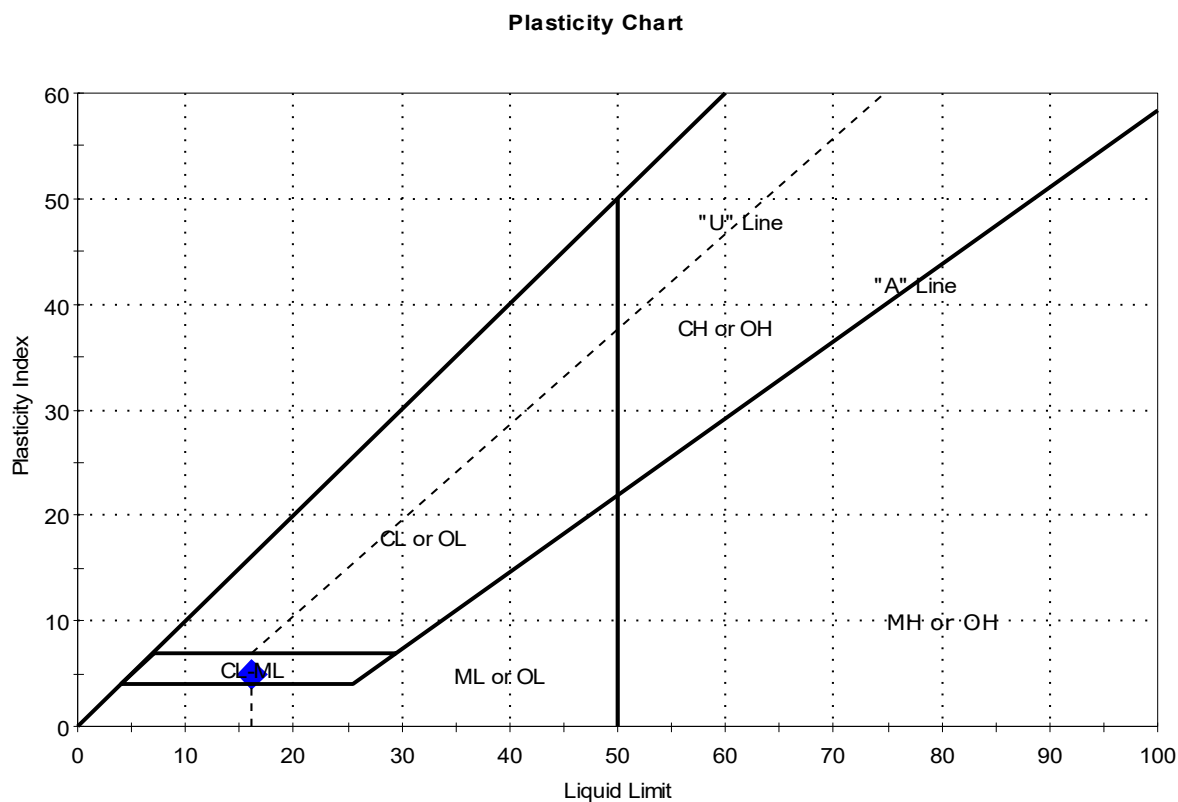
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	BB-BEB-202	Sample Type:	jar
Sample ID:	4D	Test Date:	03/29/21
Depth :	20-22	Test Id:	613863
Test Comment:	---		
Visual Description:	Moist, gray silty clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	4D	B-BEB-20	20-22	13	16	11	5	0.4	

Sample Prepared using the WET method

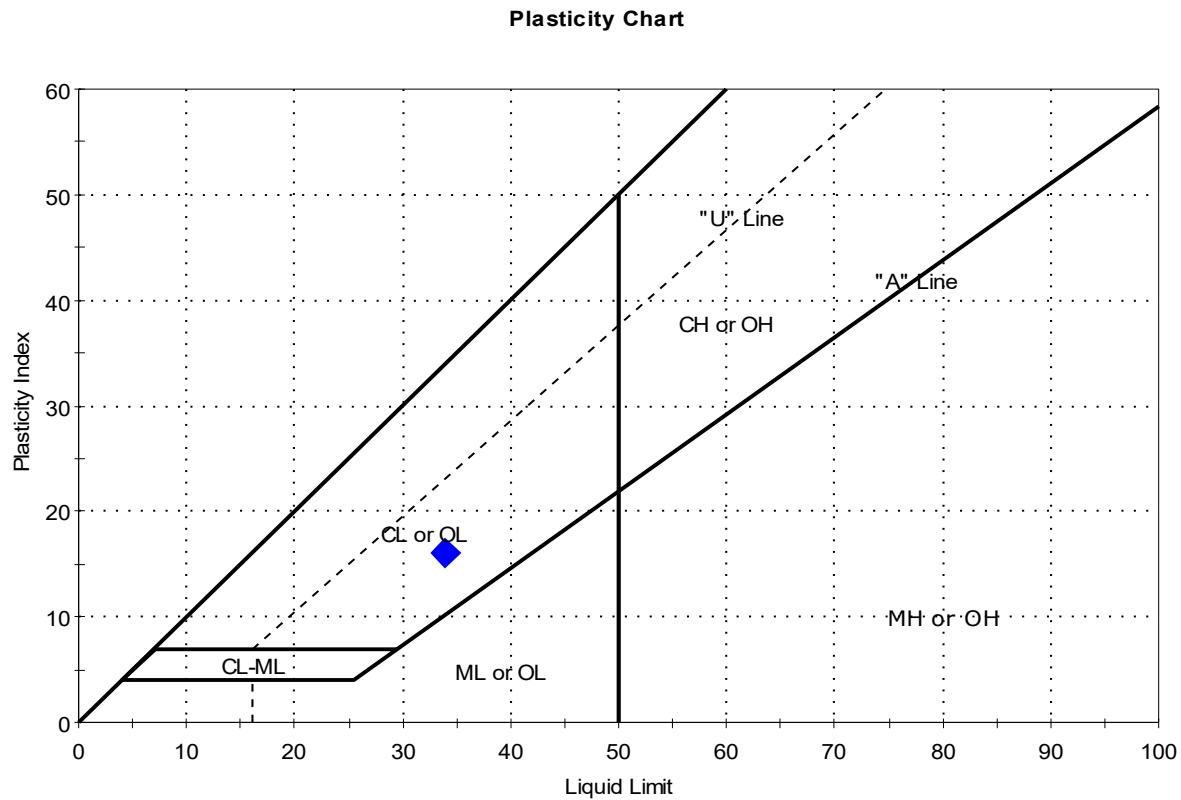
Dry Strength: VERY HIGH

Dilatancy: RAPID

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 2)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313196
Boring ID:	BB-BEB-204	Sample Type:	tube
Sample ID:	U1	Test Date:	03/22/21
Depth :	5-7 ft	Test Id:	611804
Test Comment:	---		
Visual Description:	Moist, light yellowish brown clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	B-BEB-20	5-7 ft	29	34	18	16	0.7	

Sample Prepared using the WET method

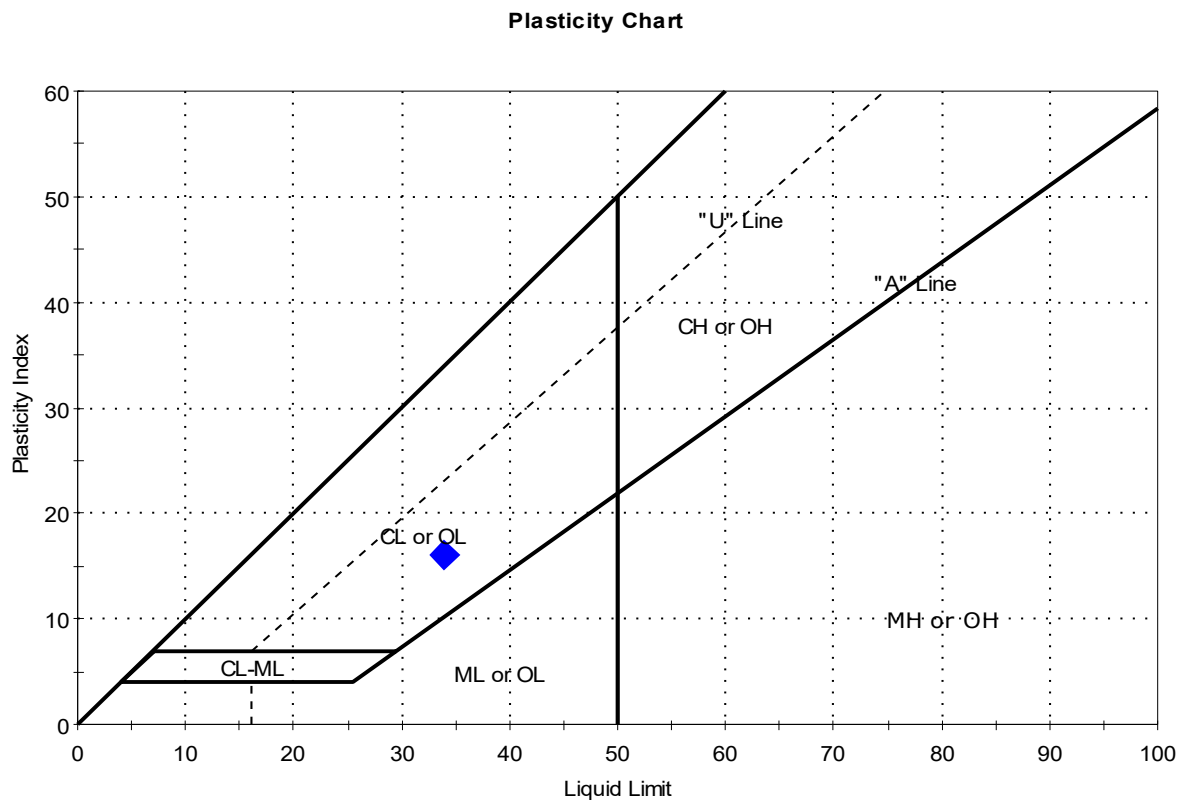
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 2)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313196
Boring ID:	BB-BEB-205	Sample Type:	tube
Sample ID:	U1	Test Date:	03/23/21
Depth :	10-12 ft	Test Id:	611805
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	B-BEB-20	10-12 ft	37	34	18	16	1.2	

Sample Prepared using the WET method

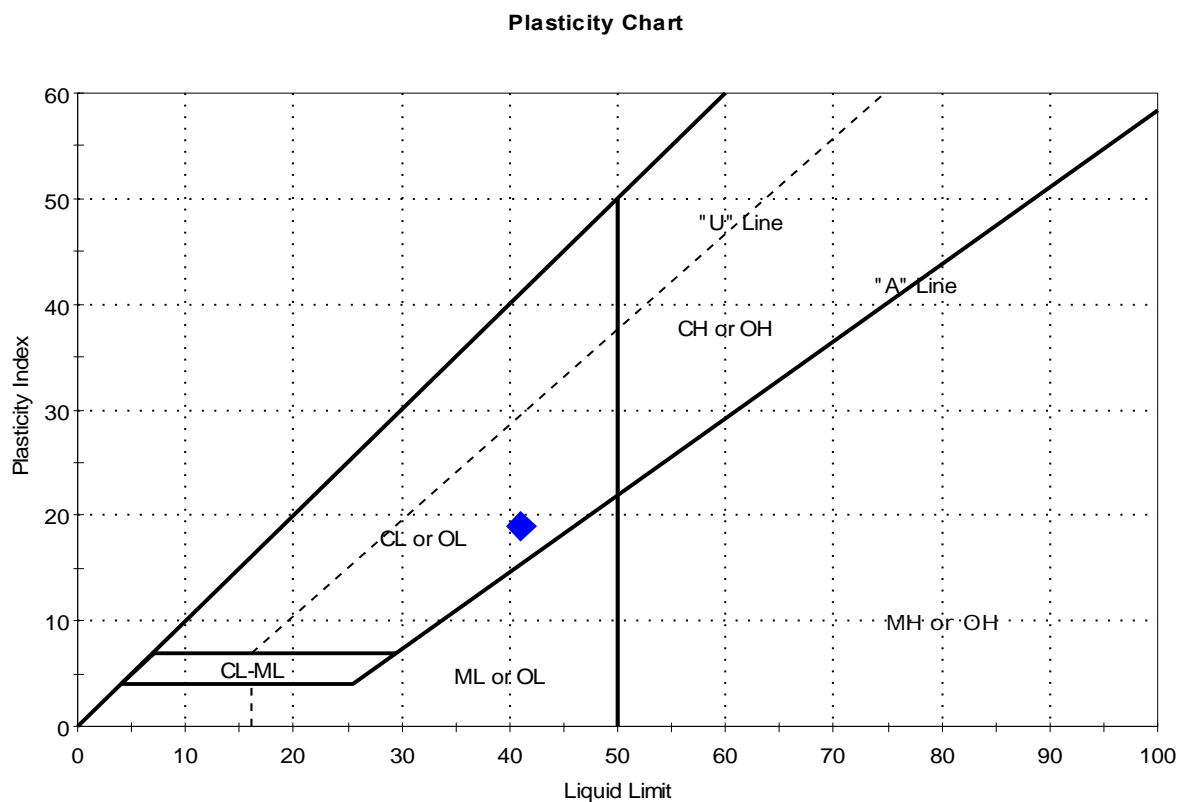
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BFB-101 BB-BFB-101	Sample Type:	tube
Sample ID:	1U	Test Date:	10/17/18
Depth :	5-7 ft	Test Id:	474307
Test Comment:	---		
Visual Description:	Moist, very dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	B-BFB-10	5-7 ft	36	41	22	19	0.7	

Sample Prepared using the WET method

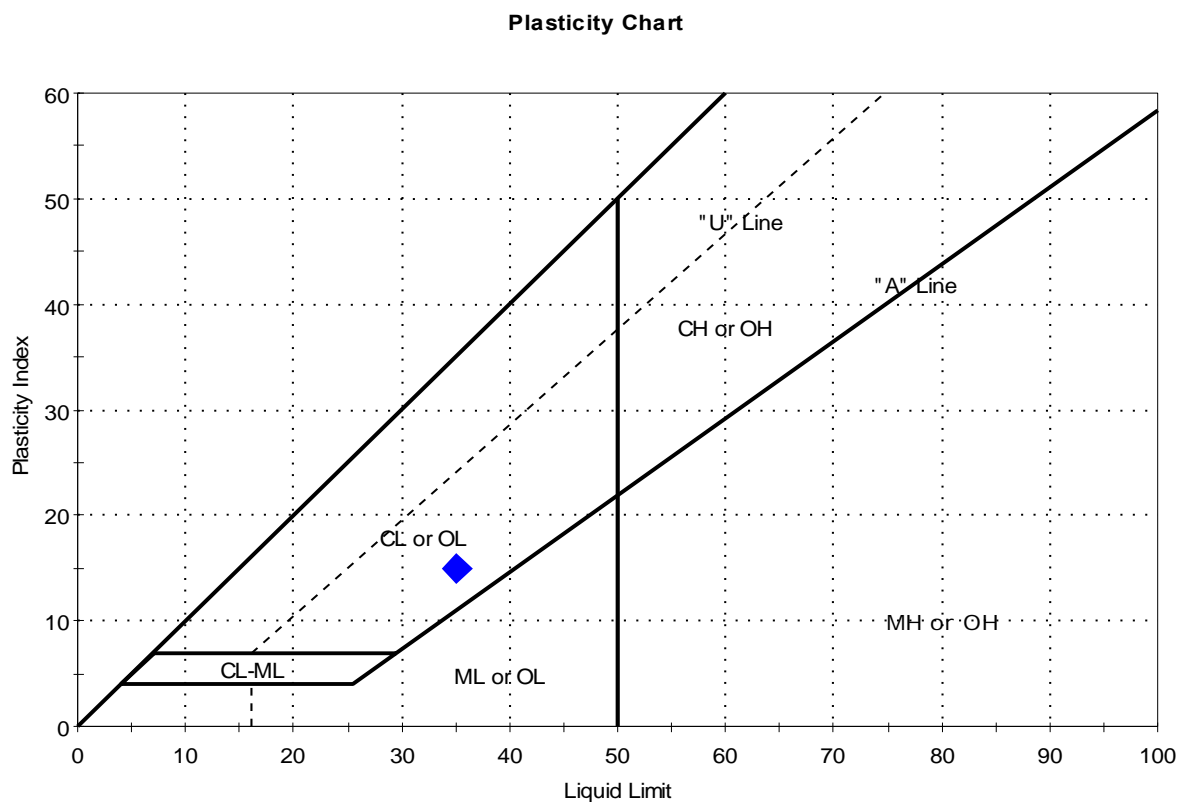
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BFB-101 BB-BFB-101	Sample Type:	tube
Sample ID:	2U	Test Date:	10/17/18
Depth :	12-14 ft	Test Id:	474308
Test Comment:	---		
Visual Description:	Wet, very dark greenish gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	2U	B-BFB-10	12-14 ft	40	35	20	15	1.4	

Sample Prepared using the WET method

Dry Strength: VERY HIGH

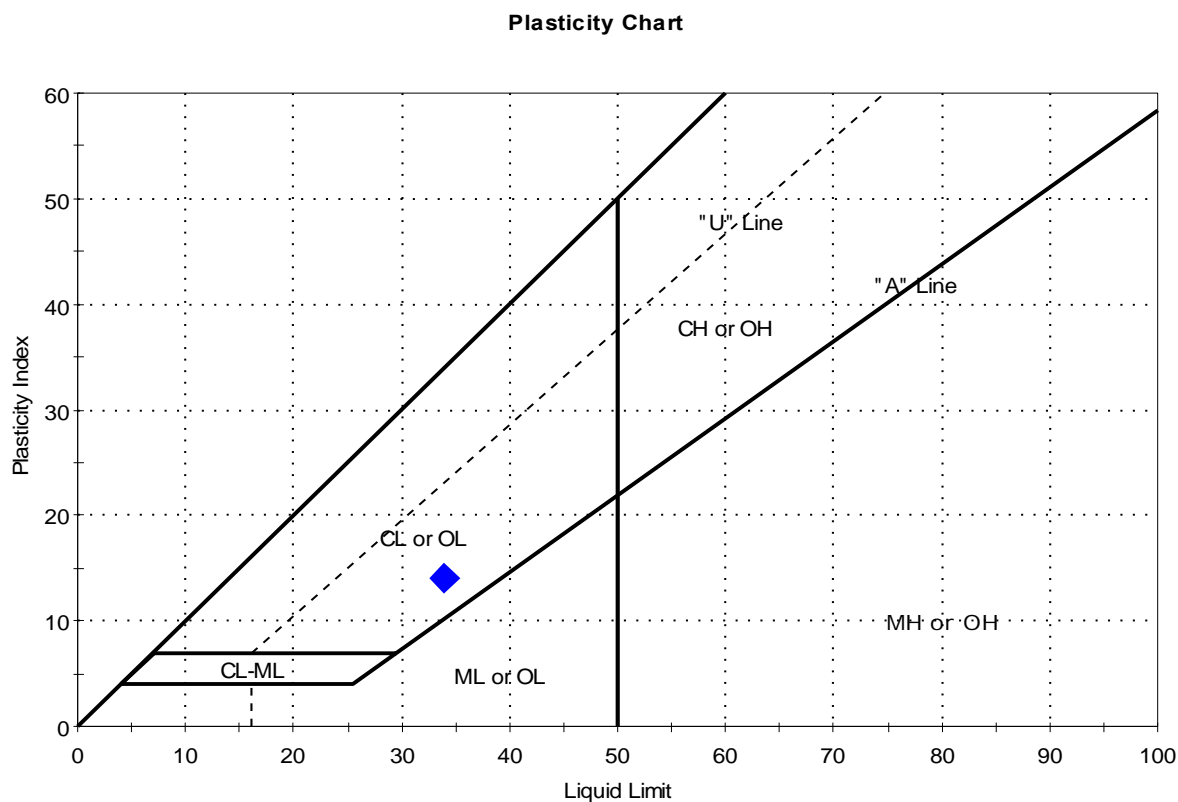
Dilatancy: SLOW

Toughness: LOW



Client:	Haley & Aldrich, Inc.			Project No:	GTX-308853
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME				
Boring ID:	HB-BFB-101 BB-BFB-101	Sample Type:	tube	Tested By:	cam
Sample ID:	4U	Test Date:	10/17/18	Checked By:	emm
Depth :	30-32 ft	Test Id:	474309		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	4U	B-BFB-101	30-32 ft	35	34	20	14	1.1	

Sample Prepared using the WET method

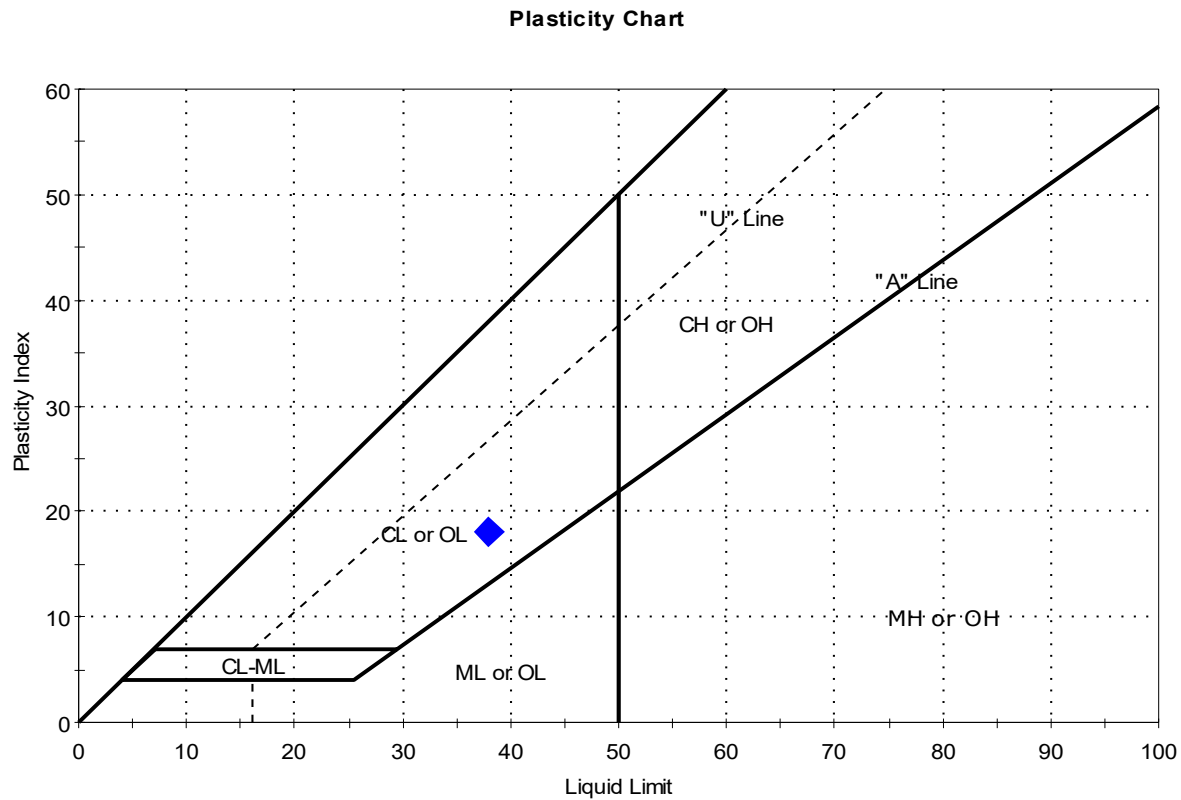
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	BB-BFB1-101	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/30/19	Checked By:	bfs
Depth :	15-16.4 ft	Test Id:	513657		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	BB-BFB1-101	15-16.4 ft	38	38	20	18	1	

Sample Prepared using the WET method

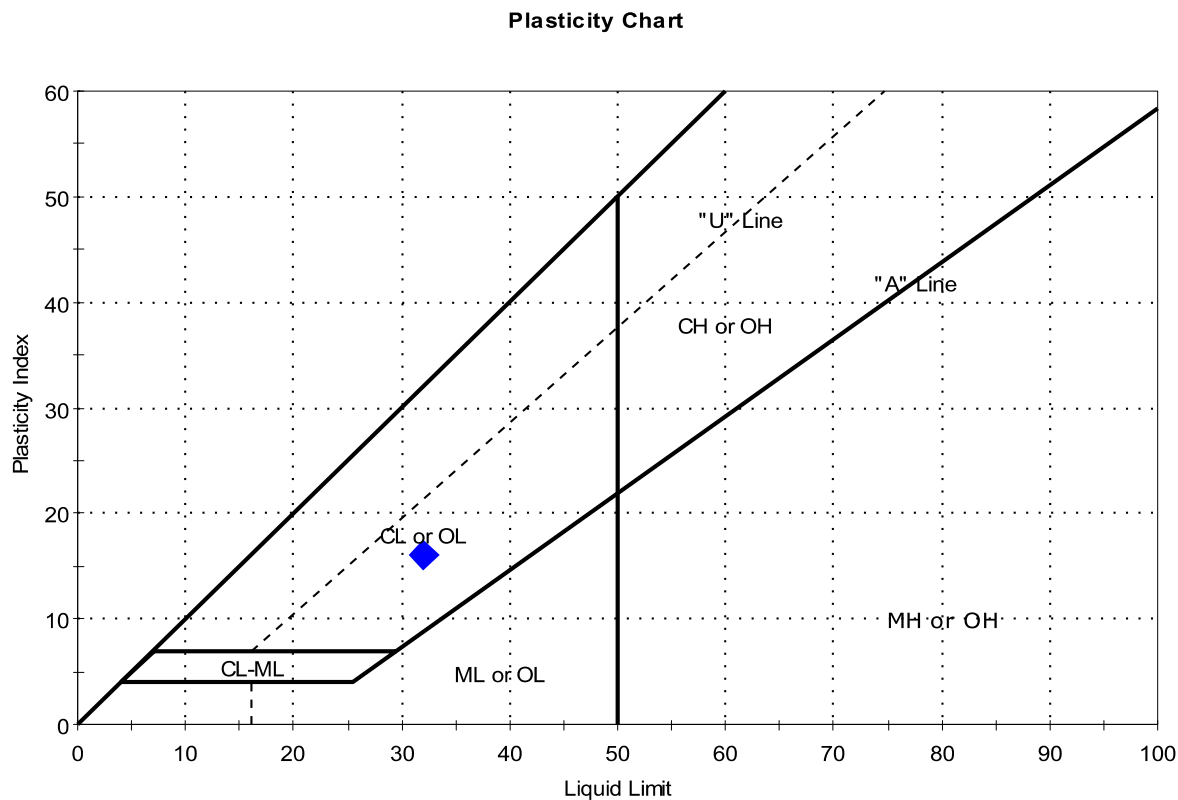
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB-201	Sample Type:	tube
Sample ID:	U2	Test Date:	03/10/21
Depth :	23-25 ft	Test Id:	611430
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	B-BFB-20	23-25 ft	34	32	16	16	1.1	

Sample Prepared using the WET method

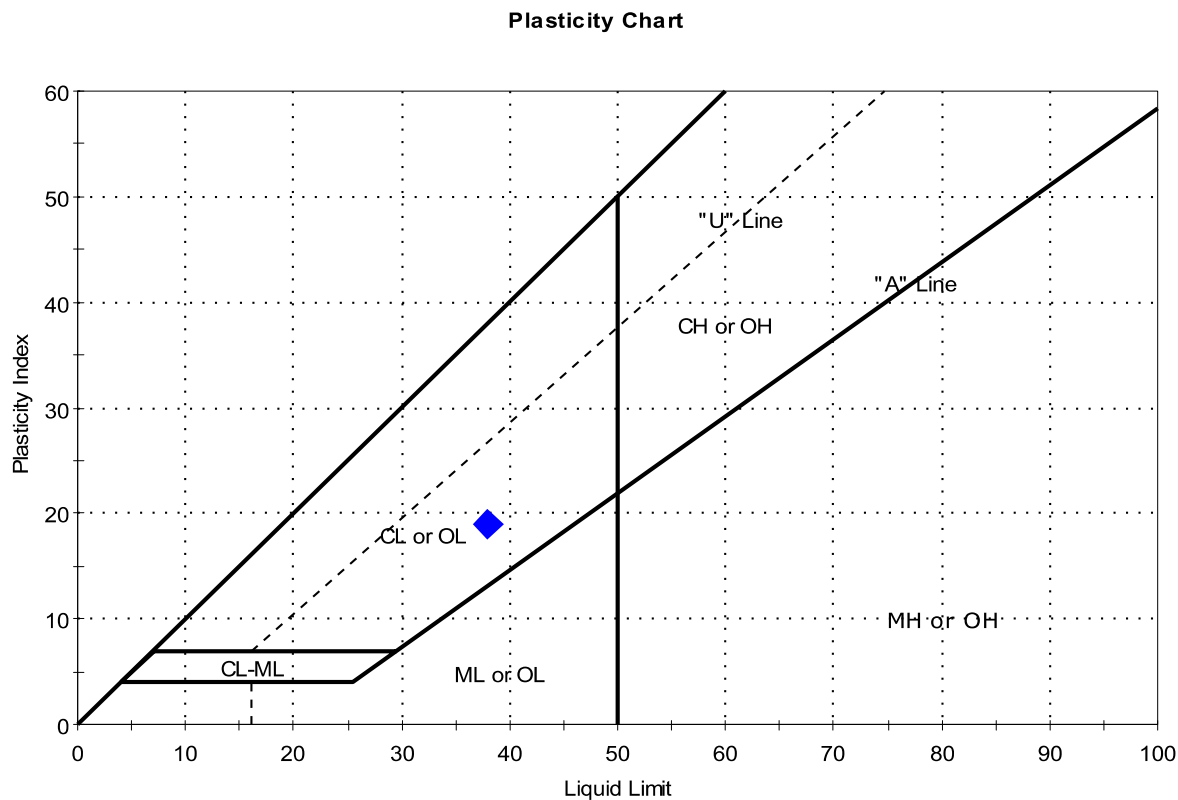
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB-202	Sample Type:	tube
Sample ID:	U1	Test Date:	03/11/21
Depth :	18-20 ft	Test Id:	611431
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	B-BFB-20	18-20 ft	42	38	19	19	1.2	

Sample Prepared using the WET method

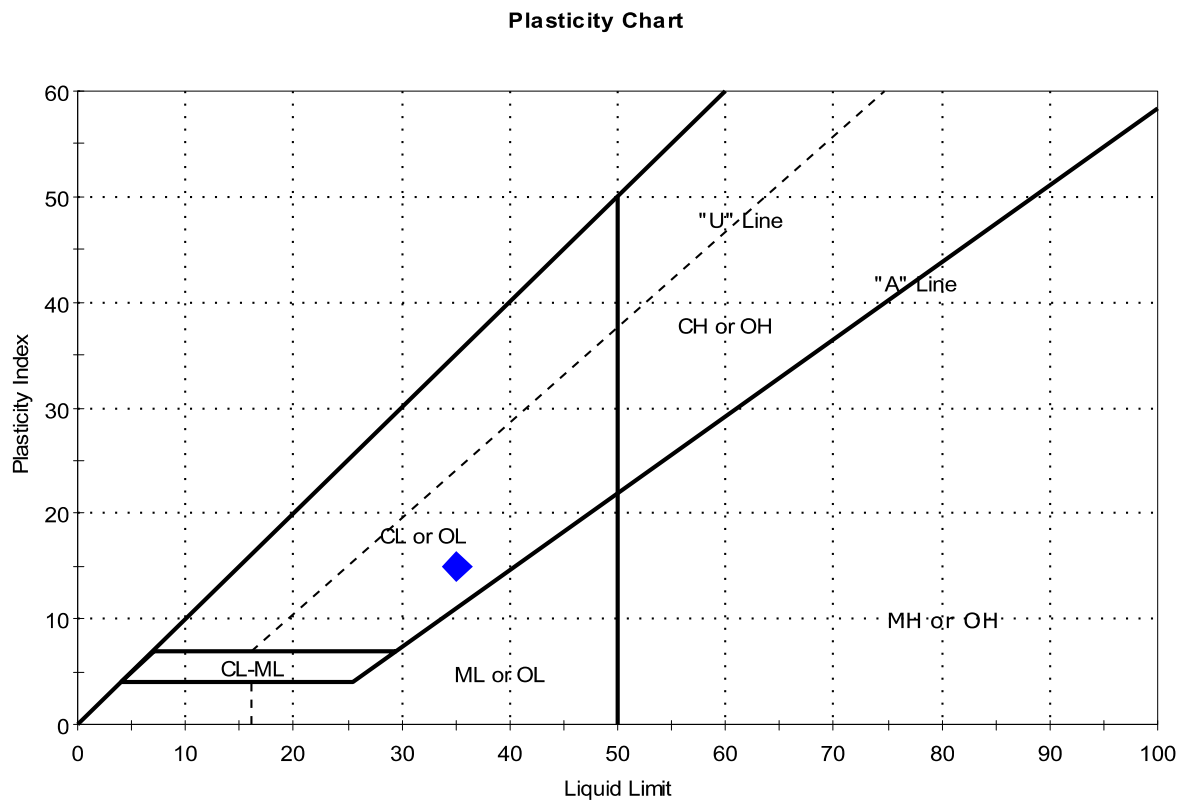
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.			Project No:	GTX-312665
Project:	I-395/Rte 9 Connector (Area 1)				
Location:	Brewer-Eddington, ME				
Boring ID:	BB-BFB-202	Sample Type:	tube	Tested By:	cam
Sample ID:	U2	Test Date:	03/08/21	Checked By:	emm
Depth :	25-27 ft	Test Id:	611432		
Test Comment:	---				
Visual Description:	Moist, gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	B-BFB-20	25-27 ft	36	35	20	15	1.1	

Sample Prepared using the WET method

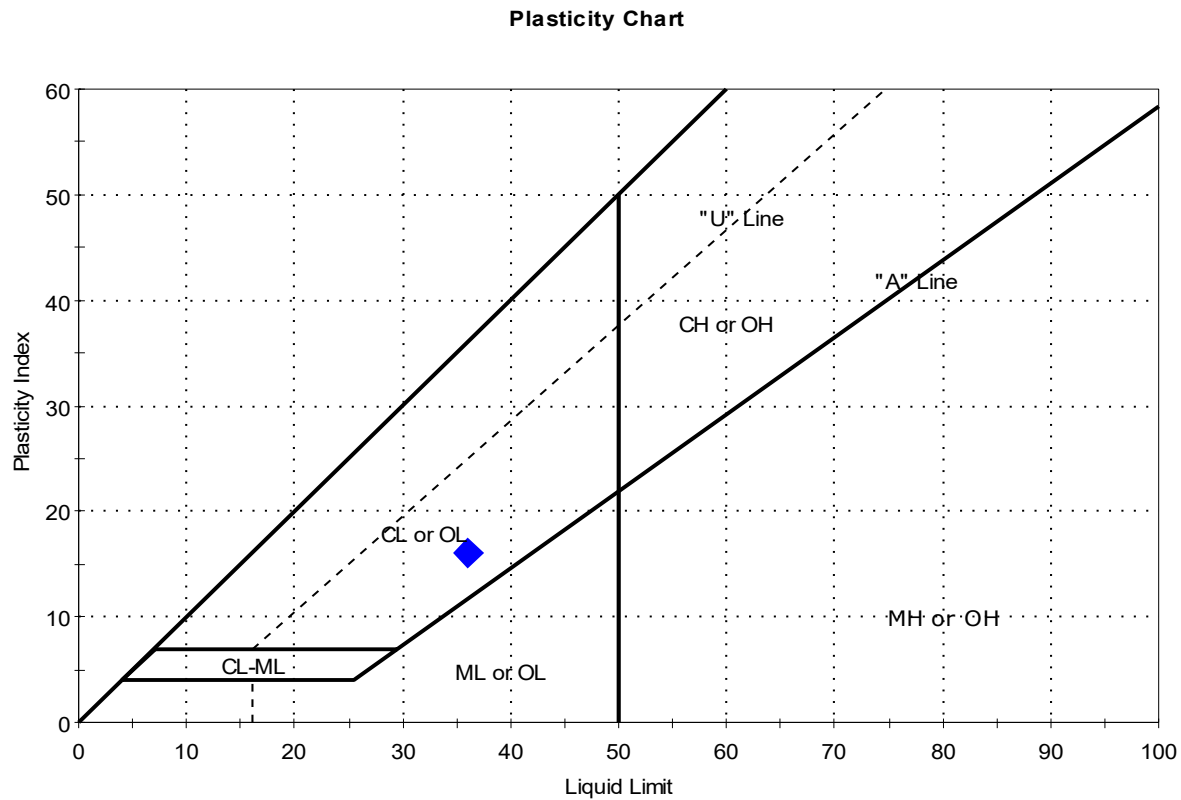
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB1-202	Sample Type:	tube
Sample ID:	U1	Test Date:	03/05/21
Depth :	10-12 ft	Test Id:	611433
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	BB-BFB1-202	10-12 ft	35	36	20	16	1	

Sample Prepared using the WET method

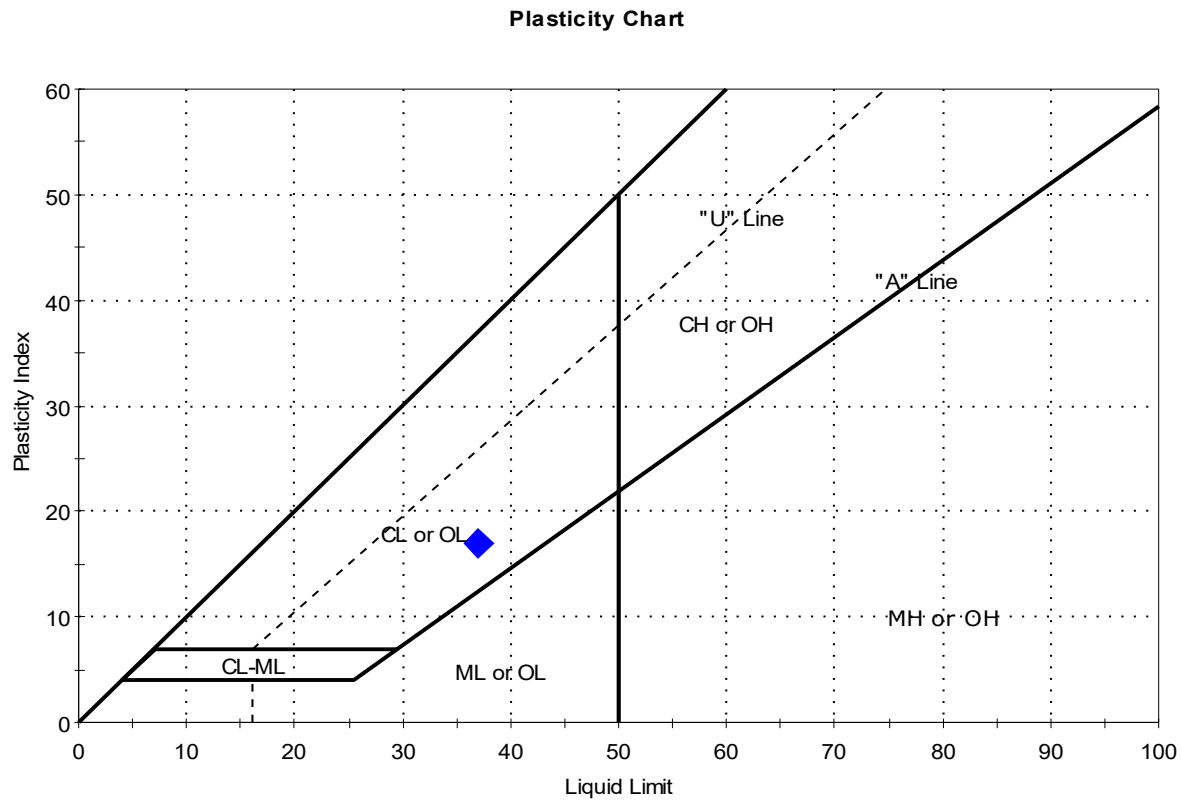
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB1-202	Sample Type:	tube
Sample ID:	U2	Test Date:	03/09/21
Depth :	15-17 ft	Test Id:	611434
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	BB-BFB1-202	15-17 ft	39	37	20	17	1.1	

Sample Prepared using the WET method

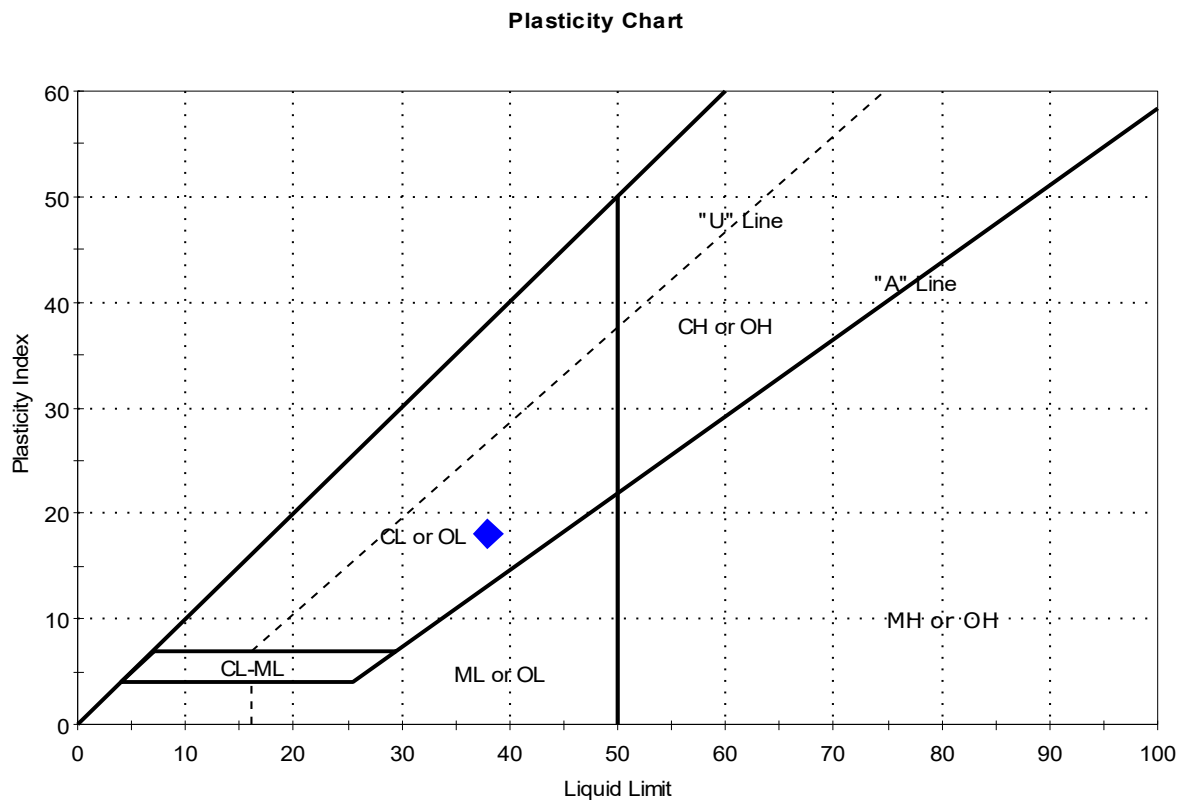
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB1-204	Sample Type:	tube
Sample ID:	U1	Test Date:	03/05/21
Depth :	10-12 ft	Test Id:	611435
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	BB-BFB1-204	10-12 ft	35	38	20	18	0.9	

Sample Prepared using the WET method

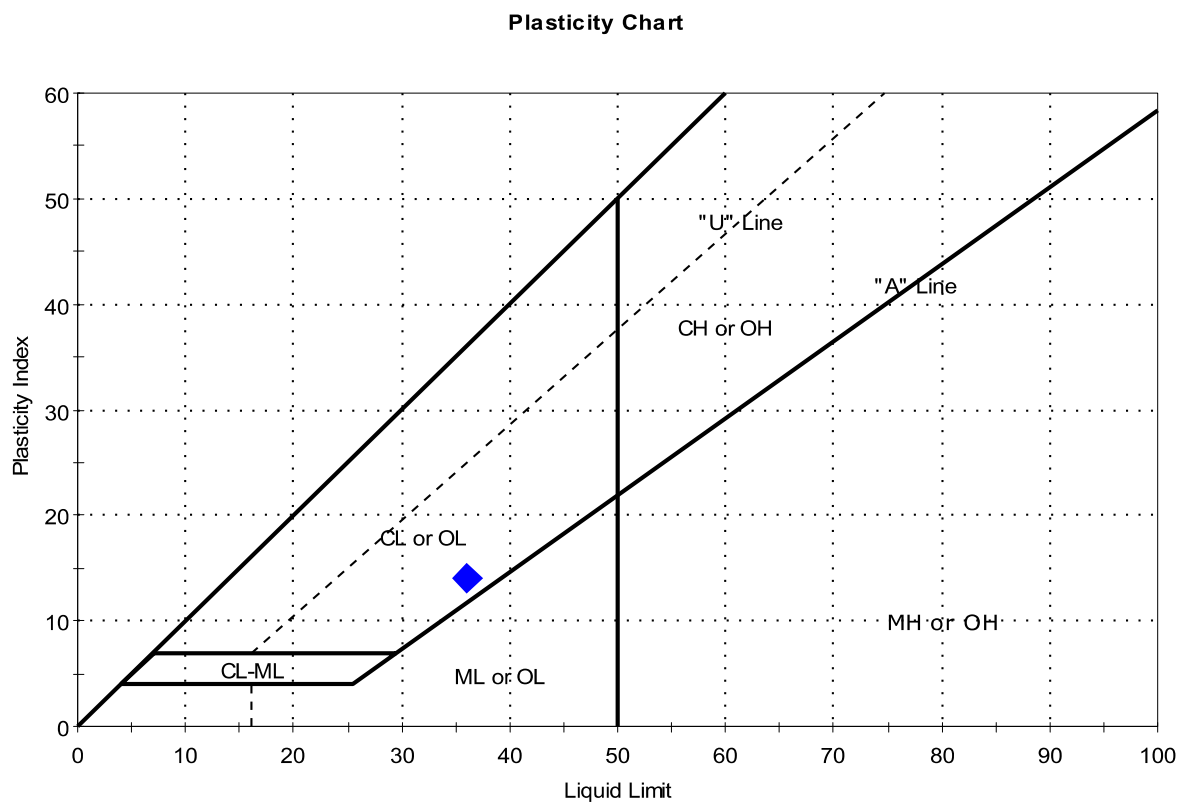
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB2-201	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	5-7 ft	Test Id:	611436
Test Comment:	---		
Visual Description:	Moist, olive gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	BB-BFB2-201	5-7 ft	34	36	22	14	0.8	

Sample Prepared using the WET method

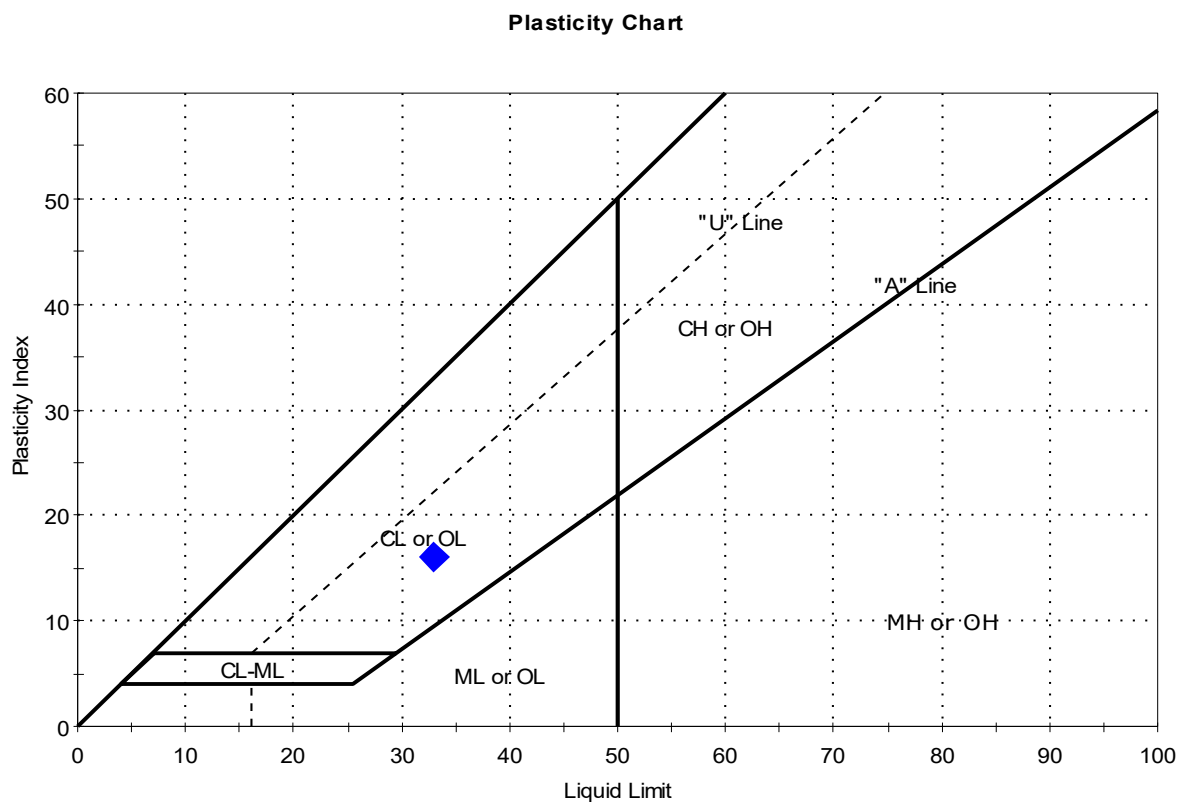
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BFB2-202	Sample Type:	tube
Sample ID:	U1	Test Date:	03/26/21
Depth :	10-12 ft	Test Id:	611437
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	BB-BFB2-202	10-12 ft	36	33	17	16	1.2	

Sample Prepared using the WET method

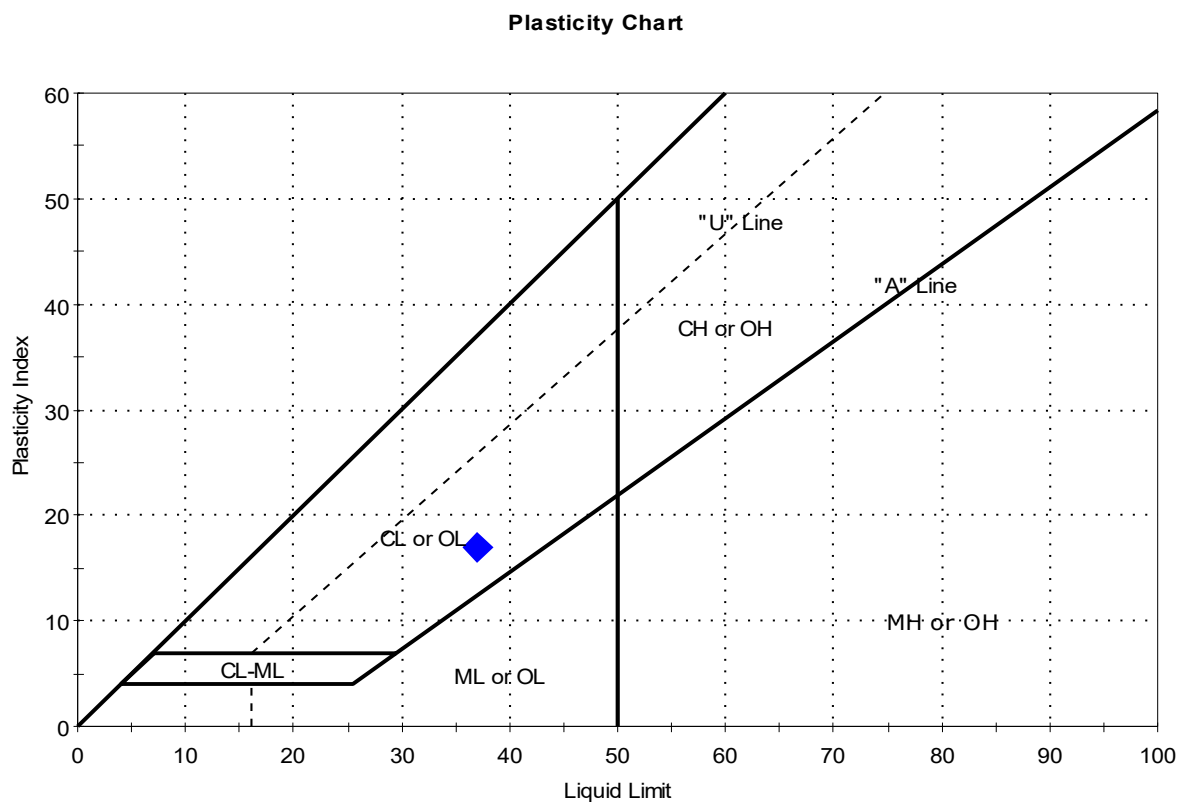
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	BB-BST1-101	Sample Type:	tube
Sample ID:	1U	Test Date:	07/26/19
Depth :	10-12 ft	Test Id:	513655
Test Comment:	---		
Visual Description:	Moist, dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	BB-BST1-101	10-12 ft	34	37	20	17	0.8	

Sample Prepared using the WET method

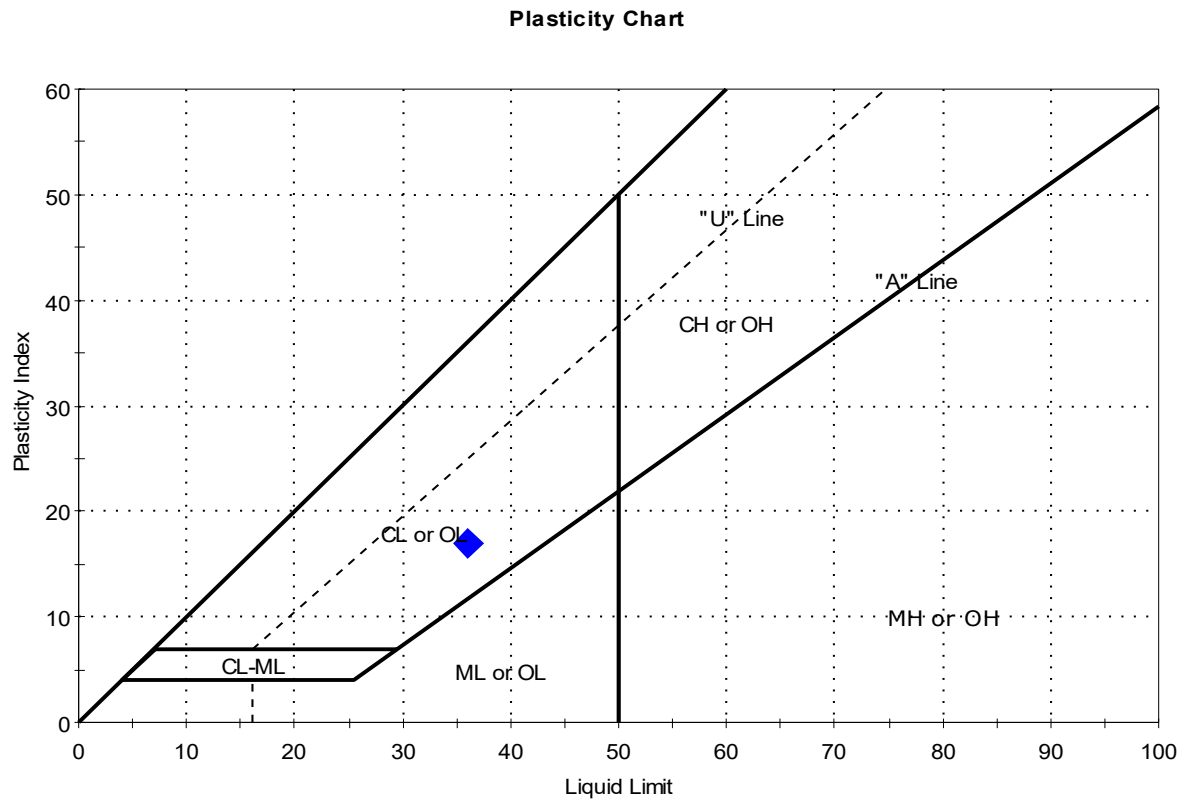
Dry Strength: HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	BB-BST1-201	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	15-17 ft	Test Id:	611438
Test Comment:	---		
Visual Description:	Moist, olive gray		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	BB-BST1-201	15-17 ft	38	36	19	17	1.1	

Sample Prepared using the WET method

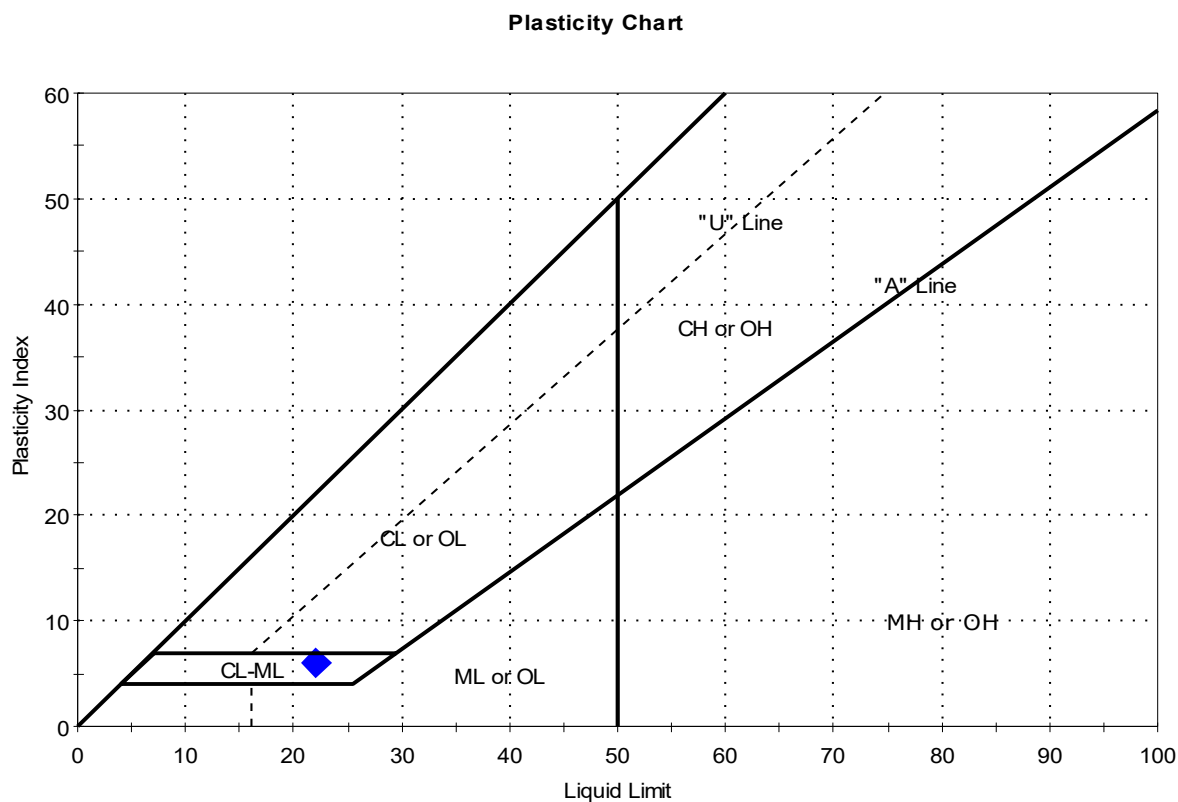
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar
Sample ID:	3D	Test Date:	10/03/18
Depth :	5-7 ft	Test Id:	474538
Test Comment:	---		
Visual Description:	Moist, olive sandy silty clay with gravel		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

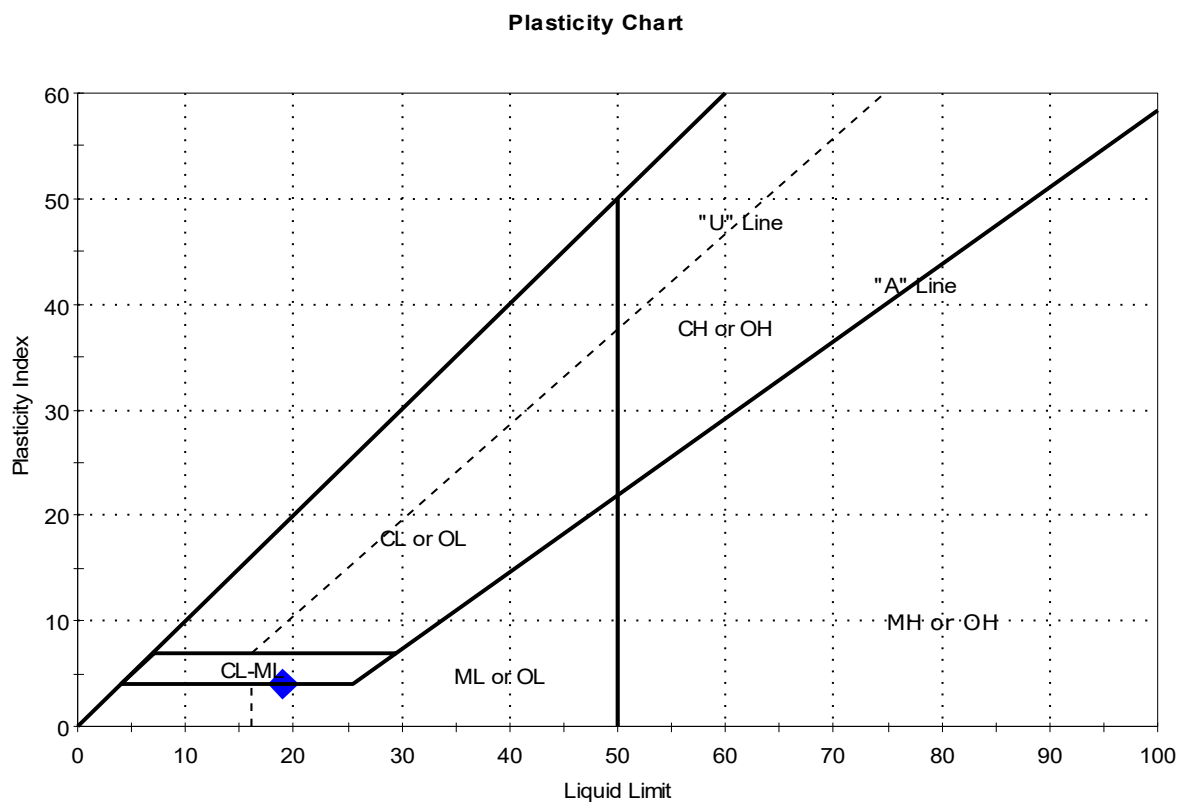


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	3D	B-BWS-10	5-7 ft	13	22	16	6	-0.5	Sandy Silty CLAY with Gravel (CL-ML)

Sample Prepared using the WET method
 30% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Wilson St Bridge				
Location:	Brewer and Eddington, ME			Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar	Tested By:	GA
Sample ID:	4D	Test Date:	10/02/18	Checked By:	emm
Depth :	10-12 ft	Test Id:	474539		
Test Comment:	---				
Visual Description:	Moist, olive silty clay with sand				
Sample Comment:	---				

Atterberg Limits - ASTM D4318

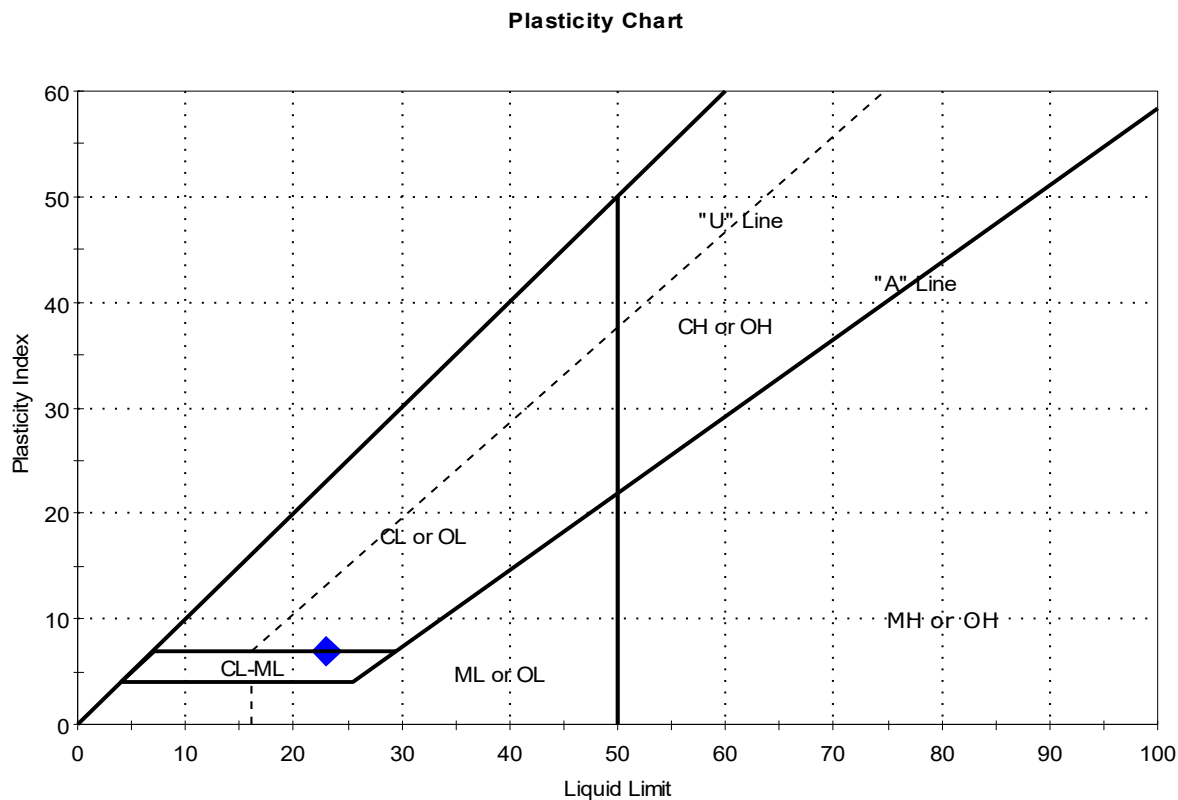


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	4D	B-BWS-103	10-12 ft	16	19	15	4	0.2	Silty CLAY with Sand (CL-ML)

Sample Prepared using the WET method
 10% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-103	Sample Type:	jar
Sample ID:	5D	Test Date:	10/02/18
Depth :	15-17 ft	Test Id:	474540
Test Comment:	---		
Visual Description:	Moist, olive gray silty clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

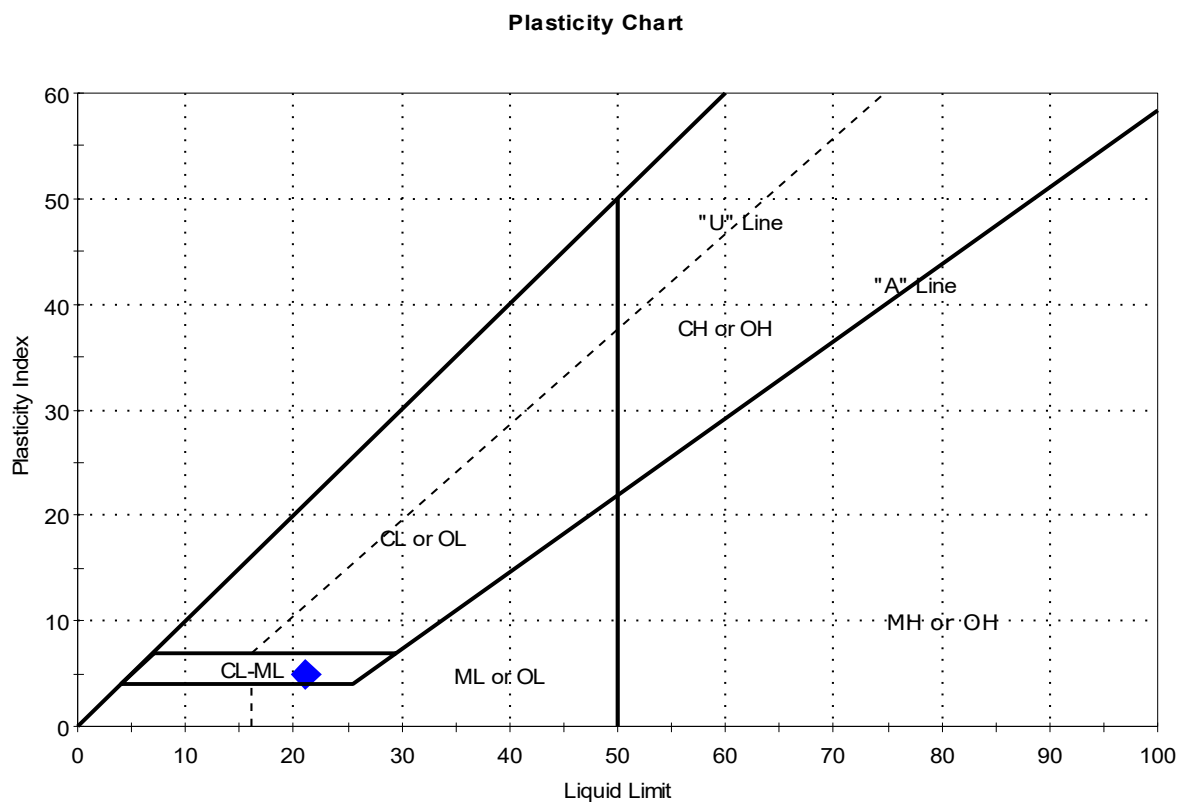


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	B-BWS-103	15-17 ft	15	23	16	7	-0.1	Silty CLAY (CL-ML)

Sample Prepared using the WET method
 7% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Wilson St Bridge				
Location:	Brewer and Eddington, ME			Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar	Tested By:	GA
Sample ID:	4D	Test Date:	10/02/18	Checked By:	emm
Depth :	6-8 ft	Test Id:	474541		
Test Comment:	---				
Visual Description:	Moist, olive silty clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	4D	B-BWS-10	6-8 ft	17	21	16	5	0.3	Silty CLAY (CL-ML)

Sample Prepared using the WET method

7% Retained on #40 Sieve

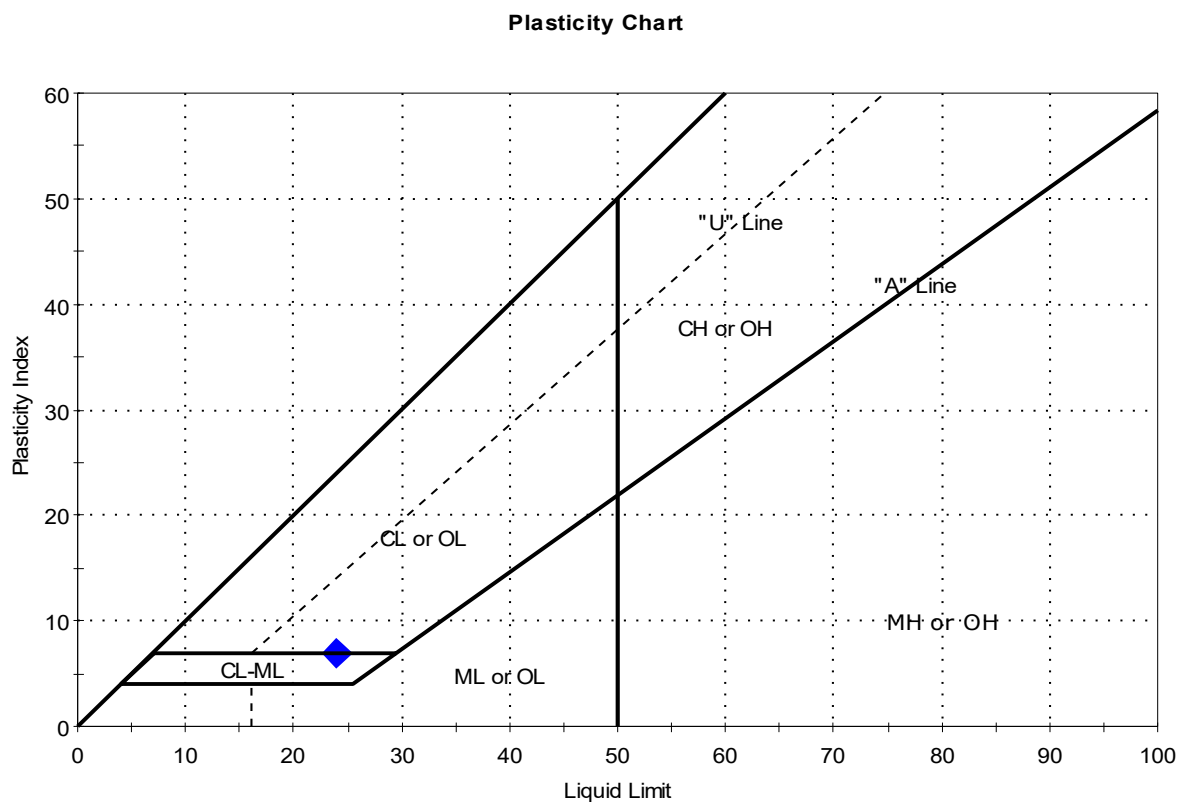
Dry Strength: LOW

Dilatancy: SLOW

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.	Project No:	GTX-308858
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME		
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	5D	Test Date:	10/02/18
Depth :	10-12 ft	Test Id:	474542
Test Comment:	---	Tested By:	GA
Visual Description:	Moist, olive gray silty clay	Checked By:	emm
Sample Comment:	---		

Atterberg Limits - ASTM D4318

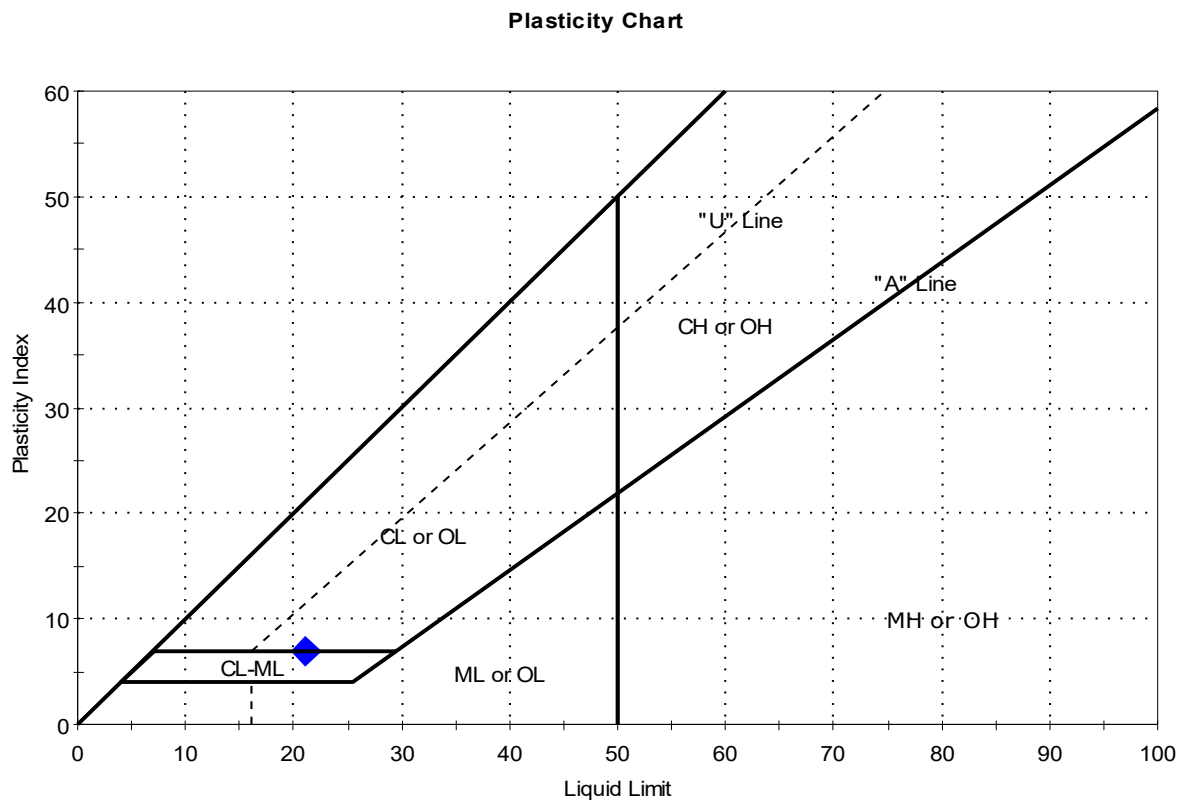


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	B-BWS-104	10-12 ft	15	24	17	7	-0.2	Silty CLAY (CL-ML)

Sample Prepared using the WET method
 5% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Wilson St Bridge				
Location:	Brewer and Eddington, ME			Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar	Tested By:	GA
Sample ID:	6D	Test Date:	10/03/18	Checked By:	emm
Depth :	15-17 ft	Test Id:	474543		
Test Comment:	---				
Visual Description:	Moist, olive gray silty clay with sand				
Sample Comment:	---				

Atterberg Limits - ASTM D4318

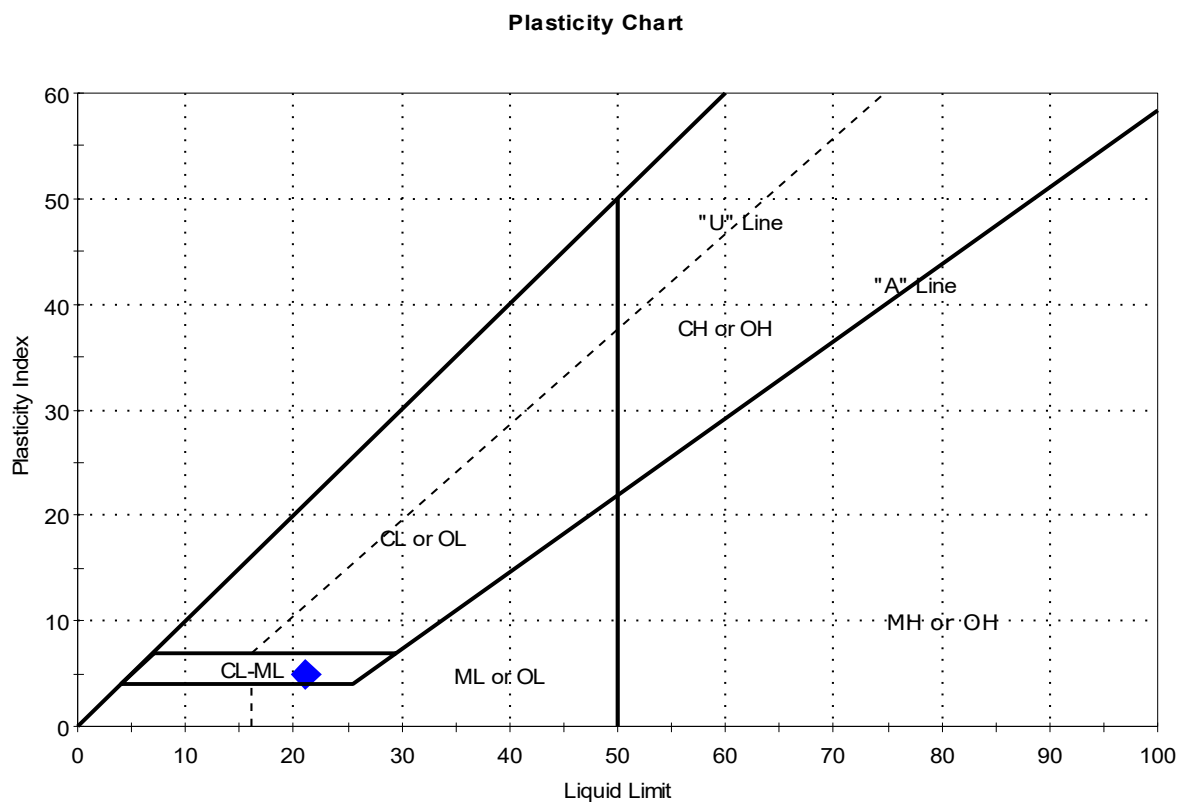


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	6D	B-BWS-104	15-17 ft	12	21	14	7	-0.3	Silty CLAY with Sand (CL-ML)

Sample Prepared using the WET method
 12% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Wilson St Bridge		
Location:	Brewer and Eddington, ME	Project No:	GTX-308858
Boring ID:	BB-BWS-104	Sample Type:	jar
Sample ID:	7D	Test Date:	10/03/18
Depth :	20-22 ft	Test Id:	474544
Test Comment:	---		
Visual Description:	Moist, dark gray silty clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

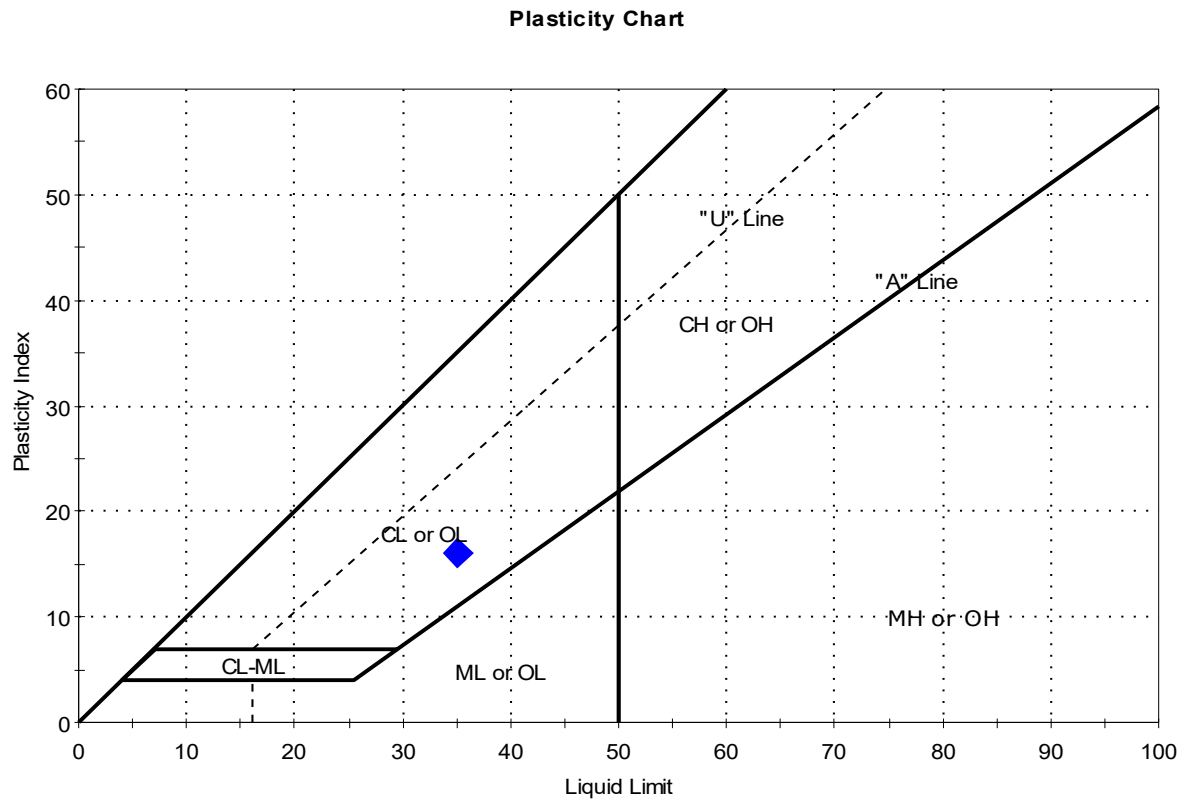


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	7D	B-BWS-10	20-22 ft	15	21	16	5	-0.3	Silty CLAY (CL-ML)

Sample Prepared using the WET method
 4% Retained on #40 Sieve
 Dry Strength: LOW
 Dilatancy: SLOW
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	tube
Sample ID:	3U	Test Date:	03/02/20
Depth :	29-31 ft	Test Id:	545426
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	3U	B-BWS-301	29-31 ft	32	35	19	16	0.8	

Sample Prepared using the WET method

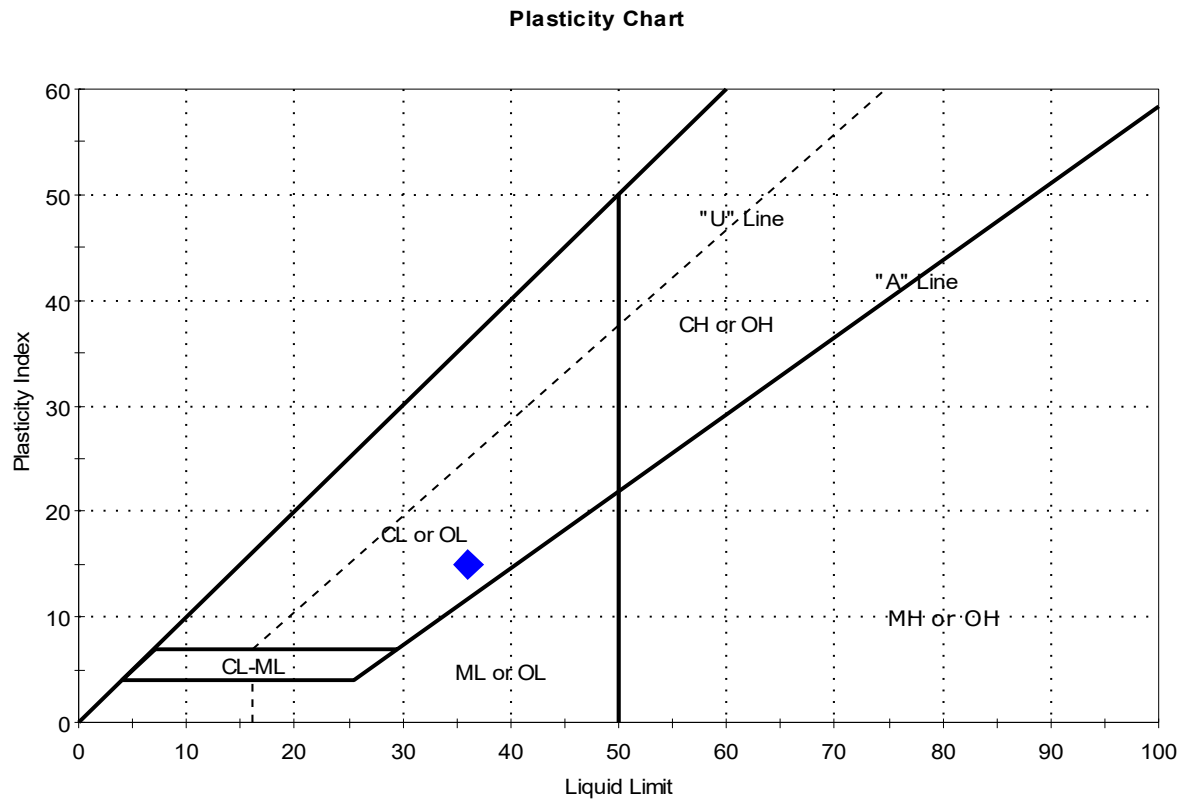
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	jar
Sample ID:	5D	Test Date:	03/03/20
Depth :	19-21 ft	Test Id:	545427
Test Comment:	---		
Visual Description:	Moist, olive clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	B-BWS-301	19-21 ft	24	36	21	15	0.2	

Sample Prepared using the WET method

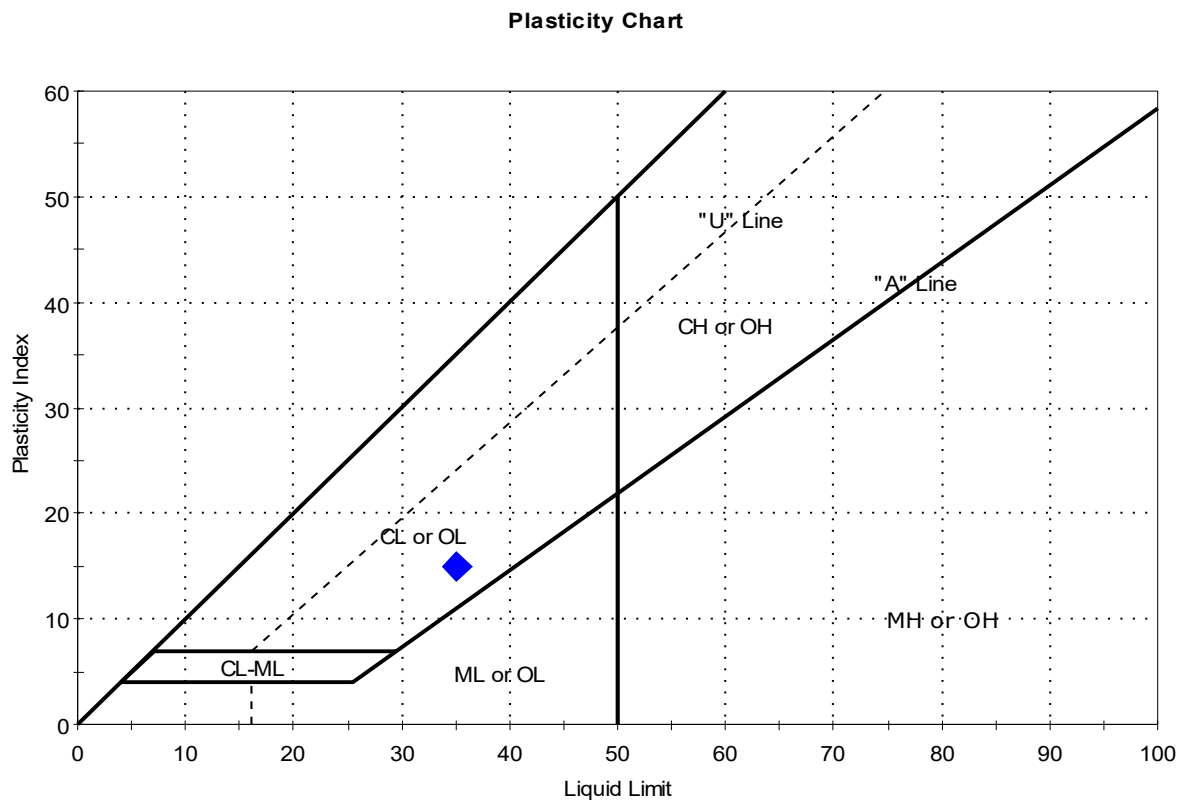
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	jar
Sample ID:	6D	Test Date:	03/03/20
Depth :	24-26 ft	Test Id:	545428
Test Comment:	---		
Visual Description:	Moist, olive clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	6D	B-BWS-301	24-26 ft	29	35	20	15	0.6	

Sample Prepared using the WET method

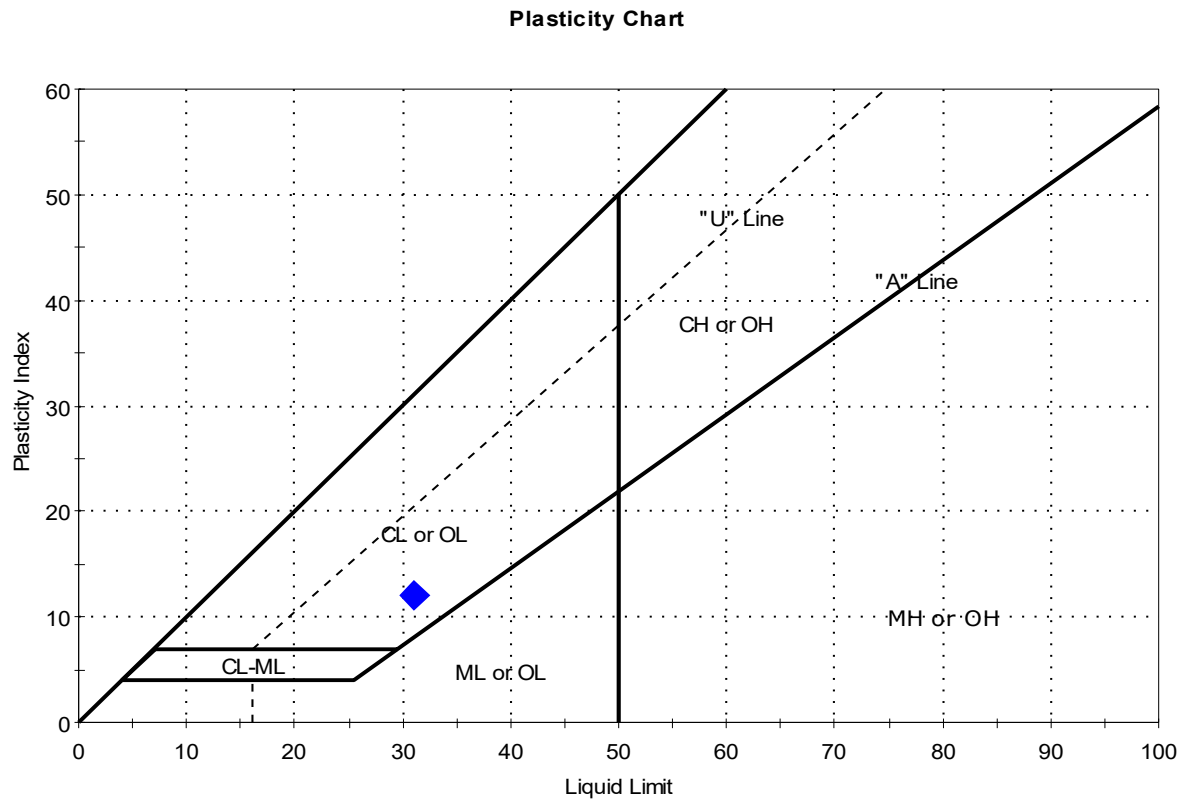
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rte 9/I-395 Conn. - Wilson St Bridge		
Location:	Brewer & Eddington, ME	Project No:	GTX-311345
Boring ID:	BB-BWS-301	Sample Type:	jar
Sample ID:	7D	Test Date:	03/03/20
Depth :	34-36 ft	Test Id:	545429
Test Comment:	---		
Visual Description:	Moist, olive gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	7D	B-BWS-301	34-36 ft	33	31	19	12	1.2	

Sample Prepared using the WET method

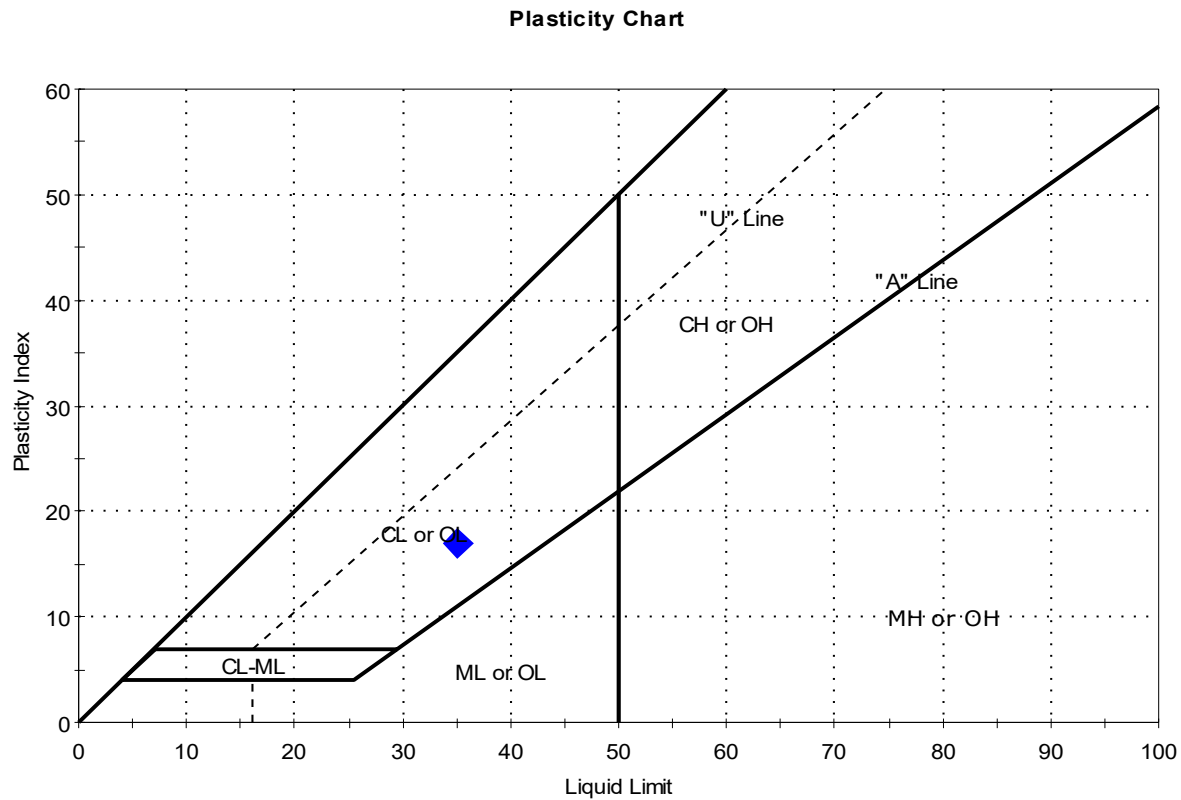
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	BB-EEBT2-101	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/29/19	Checked By:	bfs
Depth :	5-7 ft	Test Id:	513659		
Test Comment:	---				
Visual Description:	Moist, olive gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	-EEBT2-1	5-7 ft	29	35	18	17	0.7	

Sample Prepared using the WET method

Dry Strength: VERY HIGH

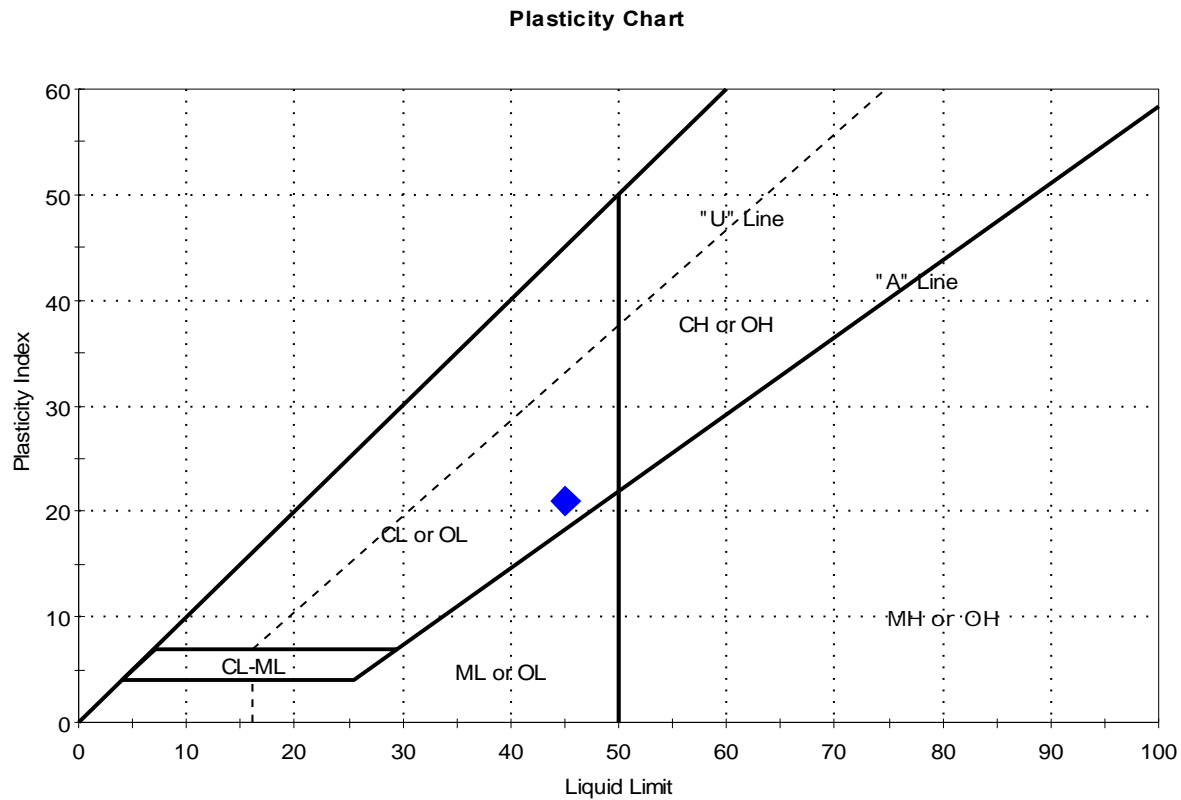
Dilatancy: SLOW

Toughness: LOW



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-101	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	10/17/18	Checked By:	emm
Depth :	5-7 ft	Test Id:	474306		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-101	5-7 ft	35	45	24	21	0.5	

Sample Prepared using the WET method

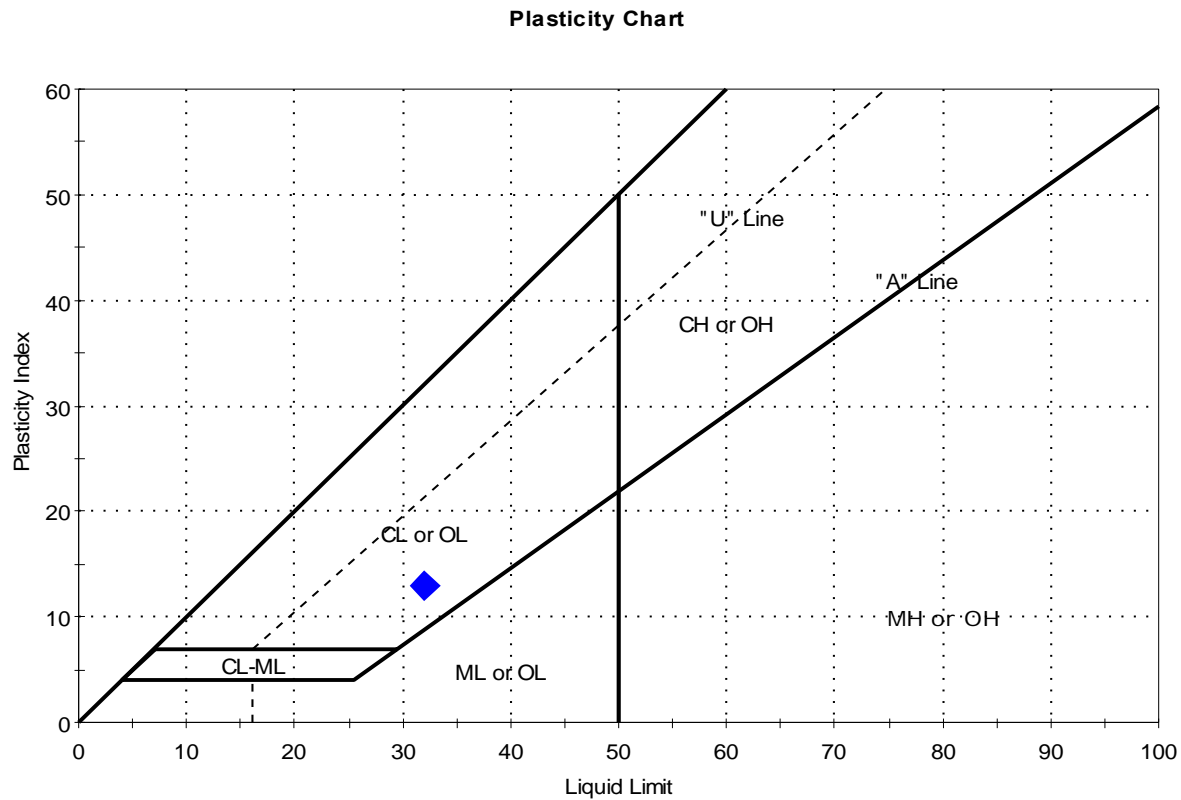
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-102	Sample Type:	tube
Sample ID:	1U	Test Date:	10/17/18
Depth :	10-11.3 ft	Test Id:	474310
Test Comment:	---		
Visual Description:	Moist, dark olive gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-102	10-11.3 ft	28	32	19	13	0.7	

Sample Prepared using the WET method

Dry Strength: VERY HIGH

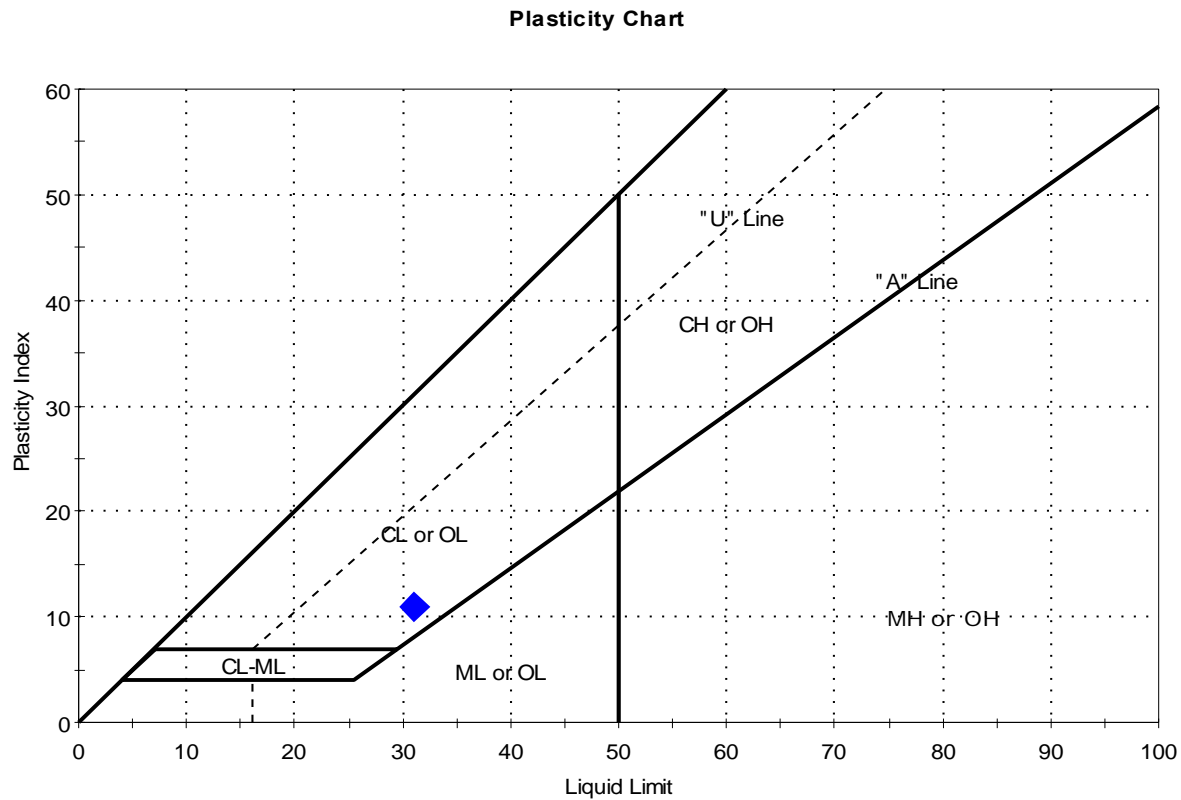
Dilatancy: SLOW

Toughness: LOW



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-105	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	10/10/18	Checked By:	emm
Depth :	10-12 ft	Test Id:	474311		
Test Comment:	---				
Visual Description:	Moist, olive gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-105	10-12 ft	30	31	20	11	0.9	

Sample Prepared using the WET method

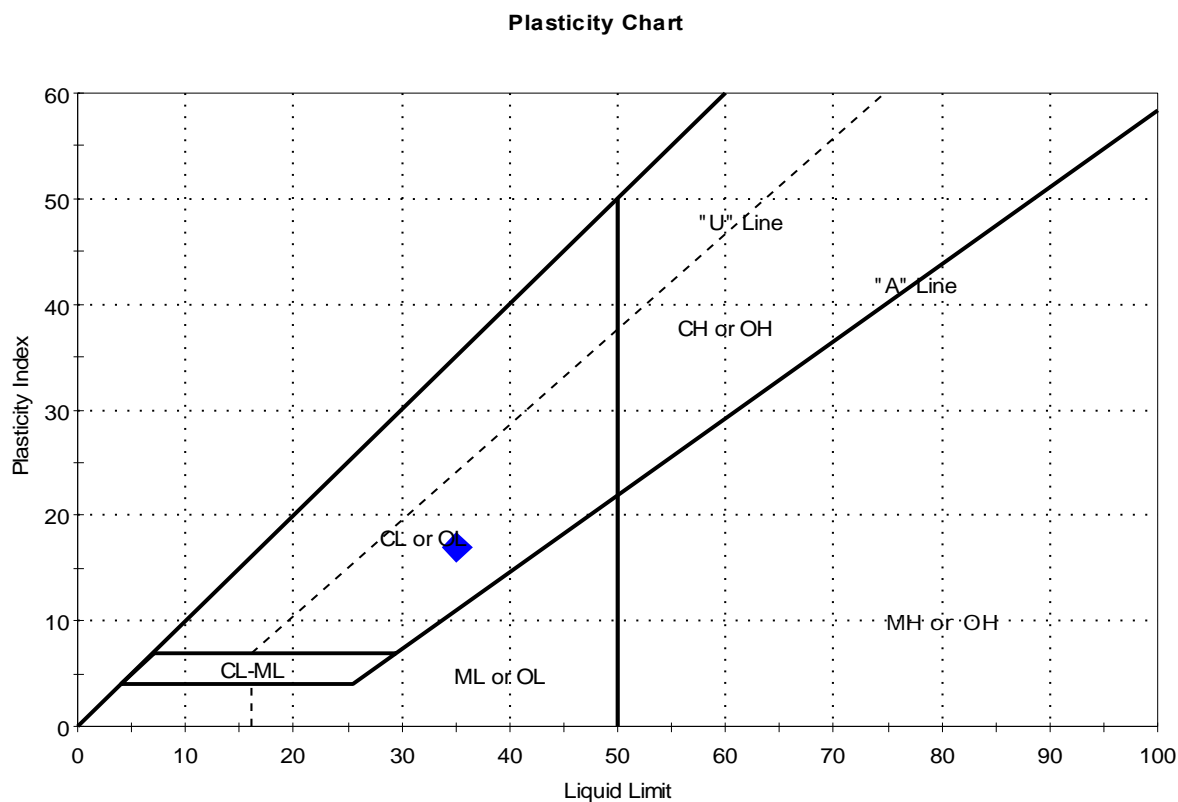
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-105	Sample Type:	tube
Sample ID:	2U	Test Date:	10/17/18
Depth :	14-16 ft	Test Id:	474312
Test Comment:	---		
Visual Description:	Moist, dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	2U	HB-BE-105	14-16 ft	37	35	18	17	1.1	

Sample Prepared using the WET method

Dry Strength: VERY HIGH

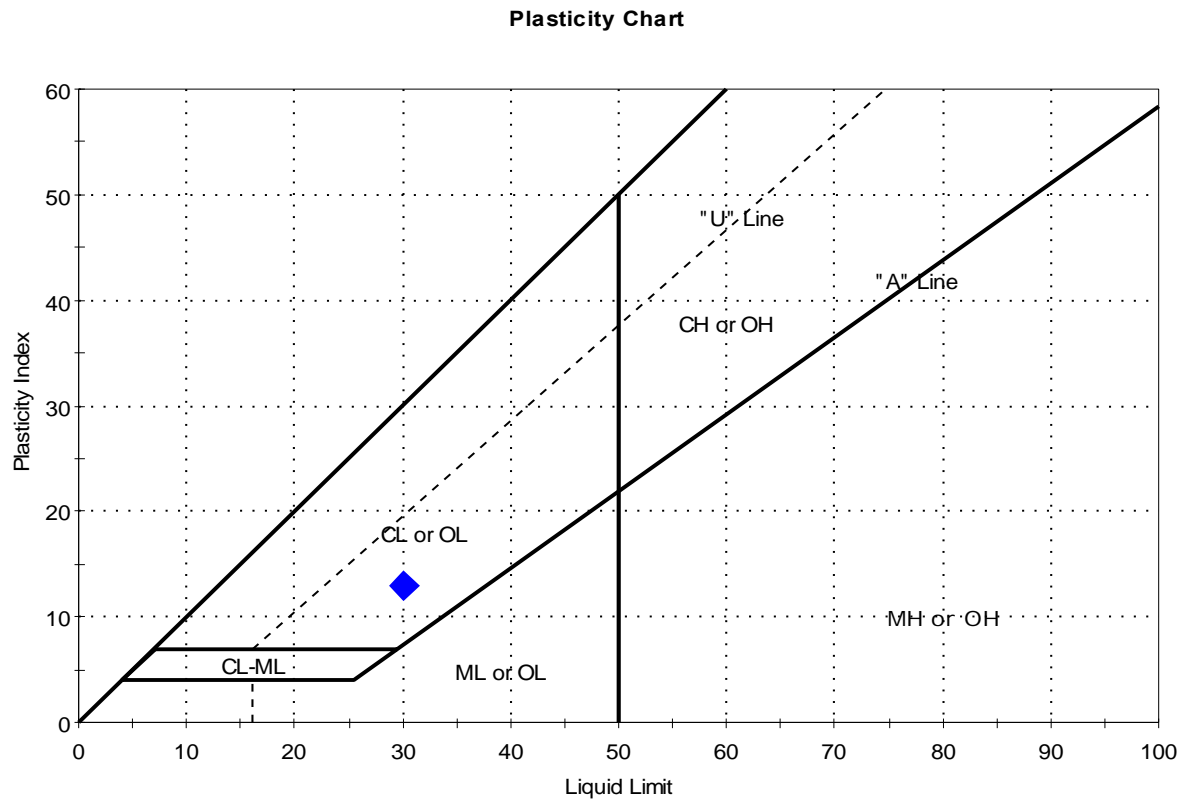
Dilatancy: SLOW

Toughness: LOW



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-107A	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	10/17/18	Checked By:	emm
Depth :	10-12 ft	Test Id:	474313		
Test Comment:	---				
Visual Description:	Wet, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	B-BE-107	10-12 ft	39	30	17	13	1.7	

Sample Prepared using the WET method

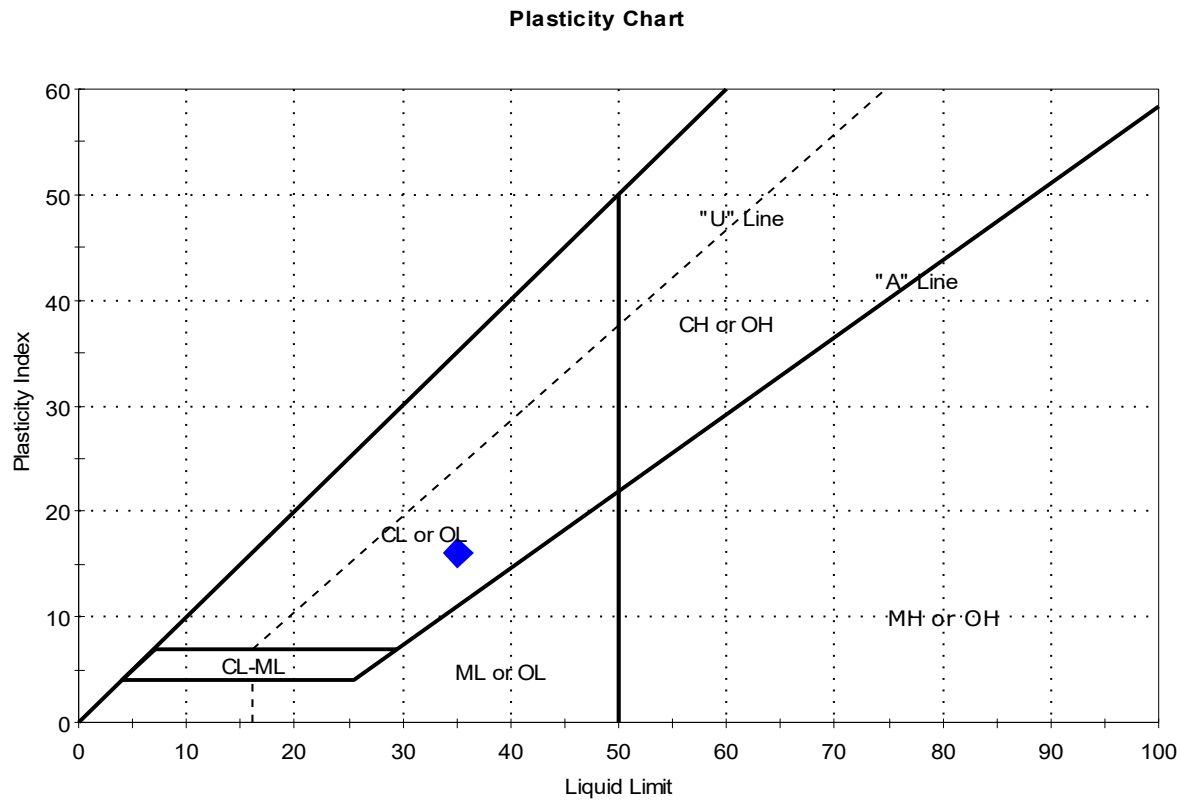
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-108	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/29/19	Checked By:	bfs
Depth :	12-14 ft	Test Id:	514936		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-108	12-14 ft	36	35	19	16	1	

Sample Prepared using the WET method

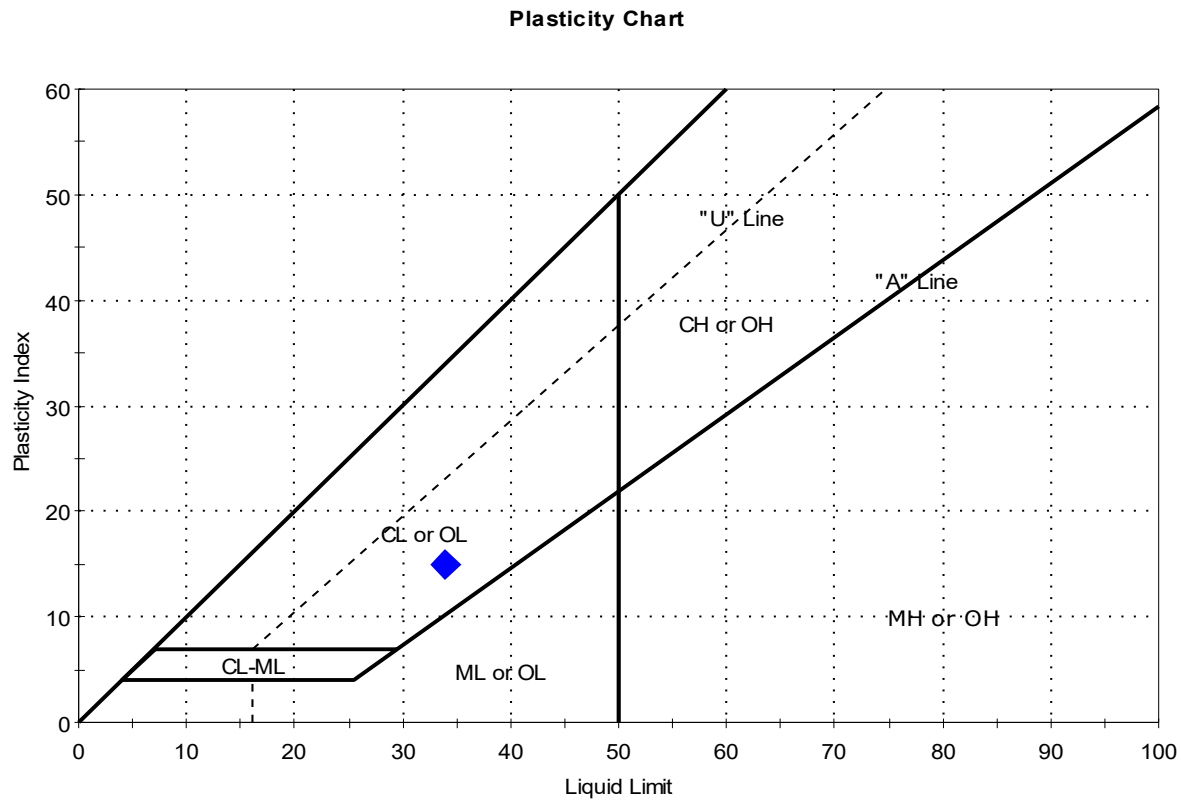
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-111	Sample Type:	tube
Sample ID:	1U	Test Date:	07/26/19
Depth :	10-12 ft	Test Id:	513656
Test Comment:	---		
Visual Description:	Moist, dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-111	10-12 ft	35	34	19	15	1	

Sample Prepared using the WET method

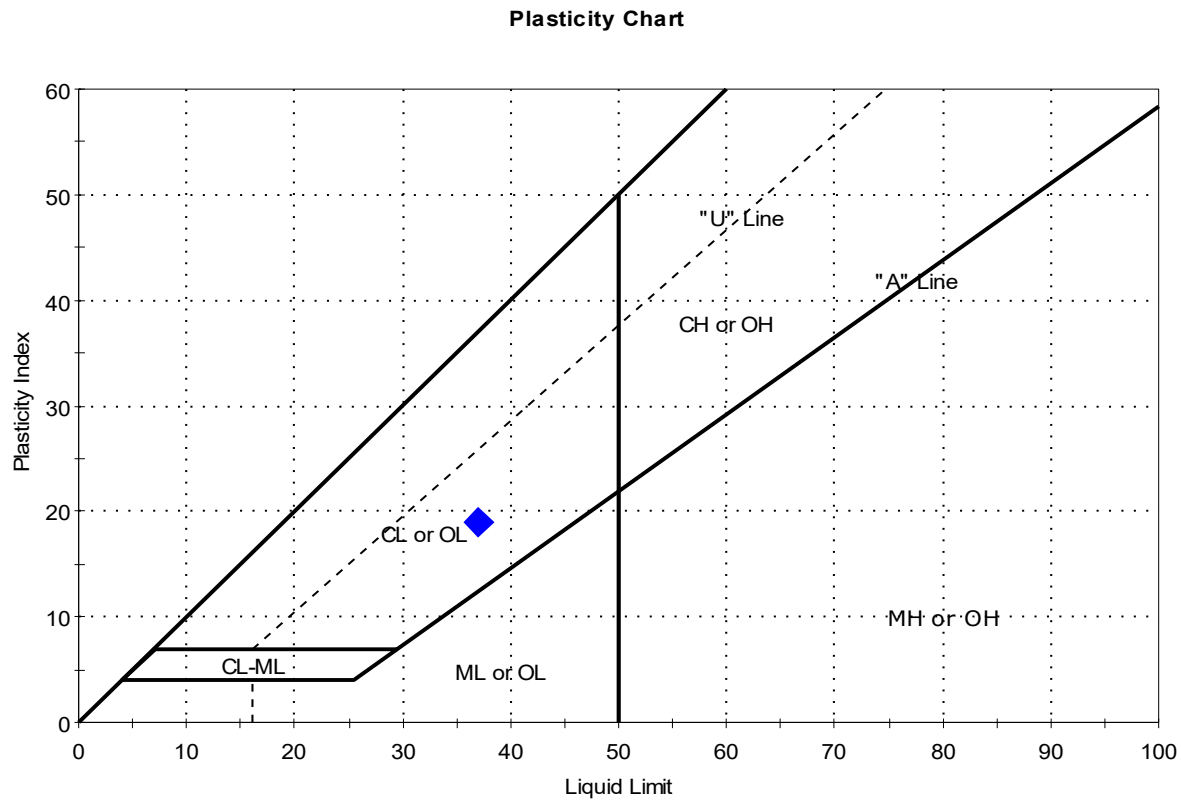
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-135	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/26/19	Checked By:	bfs
Depth :	5-7 ft	Test Id:	513661		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-135	5-7 ft	36	37	18	19	1	

Sample Prepared using the WET method

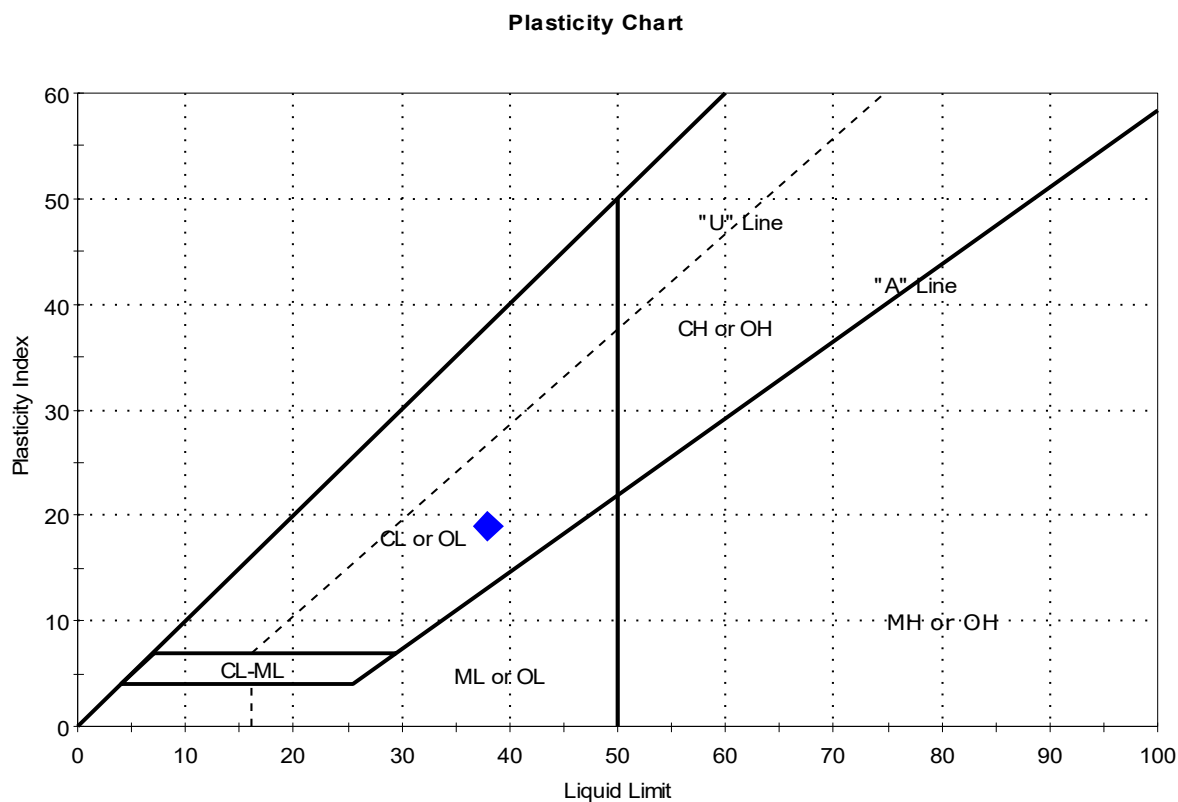
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	Rt 9/I-395 Connector		
Location:	Brewer and Eddington, ME	Project No:	GTX-308853
Boring ID:	HB-BE-136	Sample Type:	tube
Sample ID:	1U	Test Date:	07/31/19
Depth :	10-12 ft	Test Id:	513658
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-136	10-12 ft	40	38	19	19	1.1	

Sample Prepared using the WET method

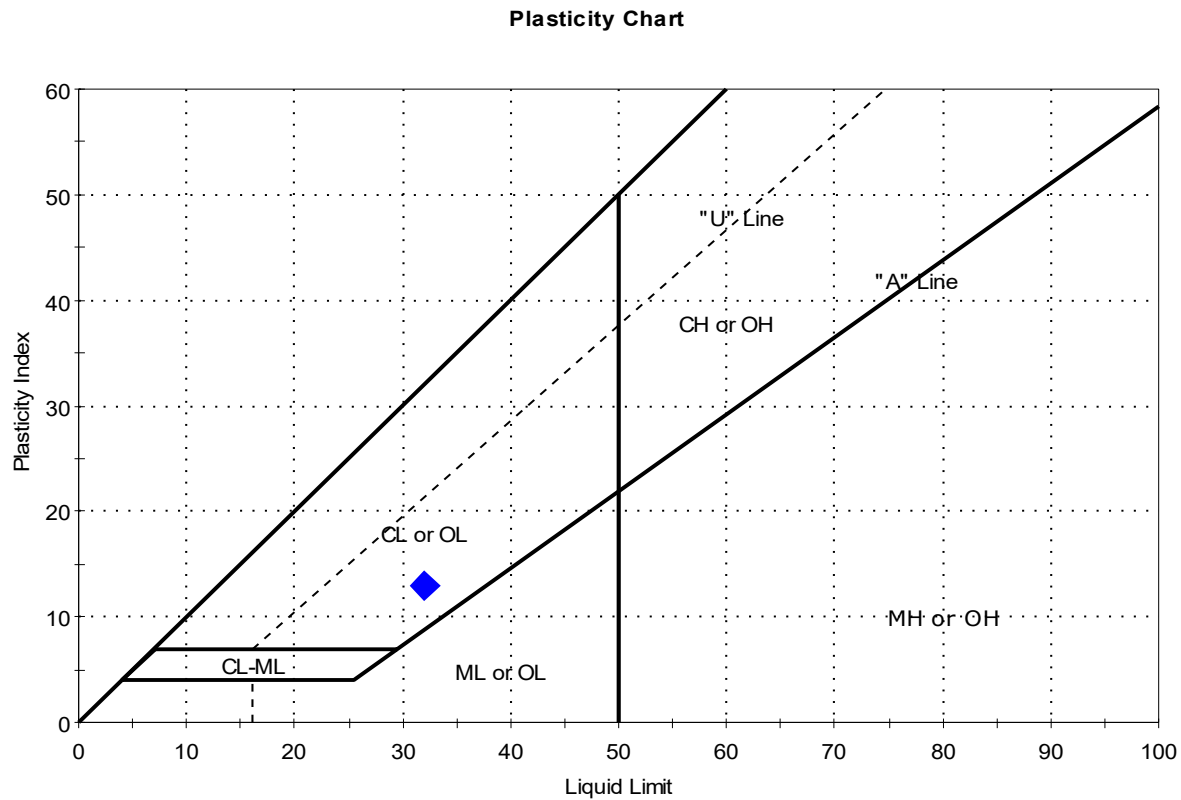
Dry Strength: HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-137	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/26/19	Checked By:	bfs
Depth :	20-22 ft	Test Id:	513663		
Test Comment:	---				
Visual Description:	Moist, dark gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-137	20-22 ft	42	32	19	13	1.7	

Sample Prepared using the WET method

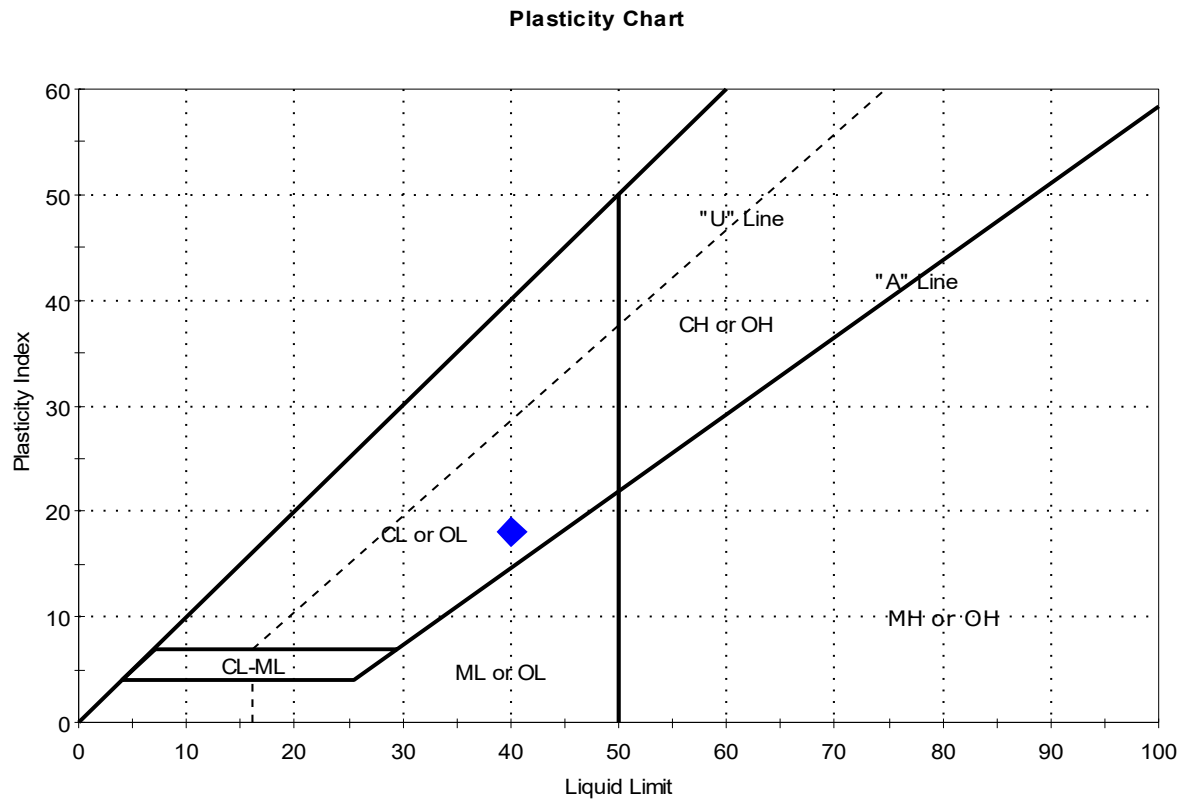
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-138	Sample Type:	tube	Tested By:	cam
Sample ID:	1U	Test Date:	07/30/19	Checked By:	bfs
Depth :	8-10 ft	Test Id:	513662		
Test Comment:	---				
Visual Description:	Moist, dark grayish olive clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	1U	HB-BE-138	8-10 ft	33	40	22	18	0.6	

Sample Prepared using the WET method

Dry Strength: VERY HIGH

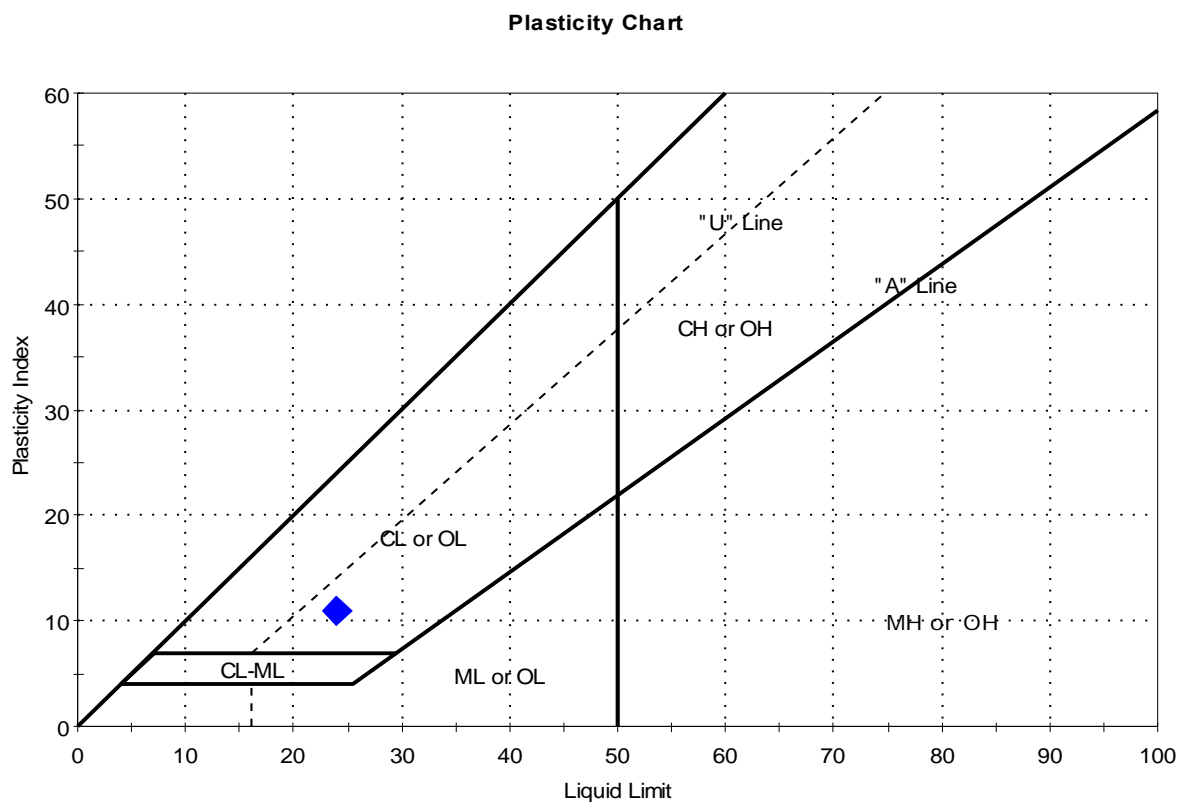
Dilatancy: SLOW

Toughness: LOW



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME			Project No:	GTX-308853
Boring ID:	HB-BE-148	Sample Type:	jar	Tested By:	GA
Sample ID:	5D	Test Date:	10/12/18	Checked By:	emm
Depth :	12-14 ft	Test Id:	474390		
Test Comment:	---				
Visual Description:	Moist, olive sandy clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	HB-BE-148	12-14 ft	14	24	13	11	0.1	Sandy Lean CLAY (CL)

Sample Prepared using the WET method

22% Retained on #40 Sieve

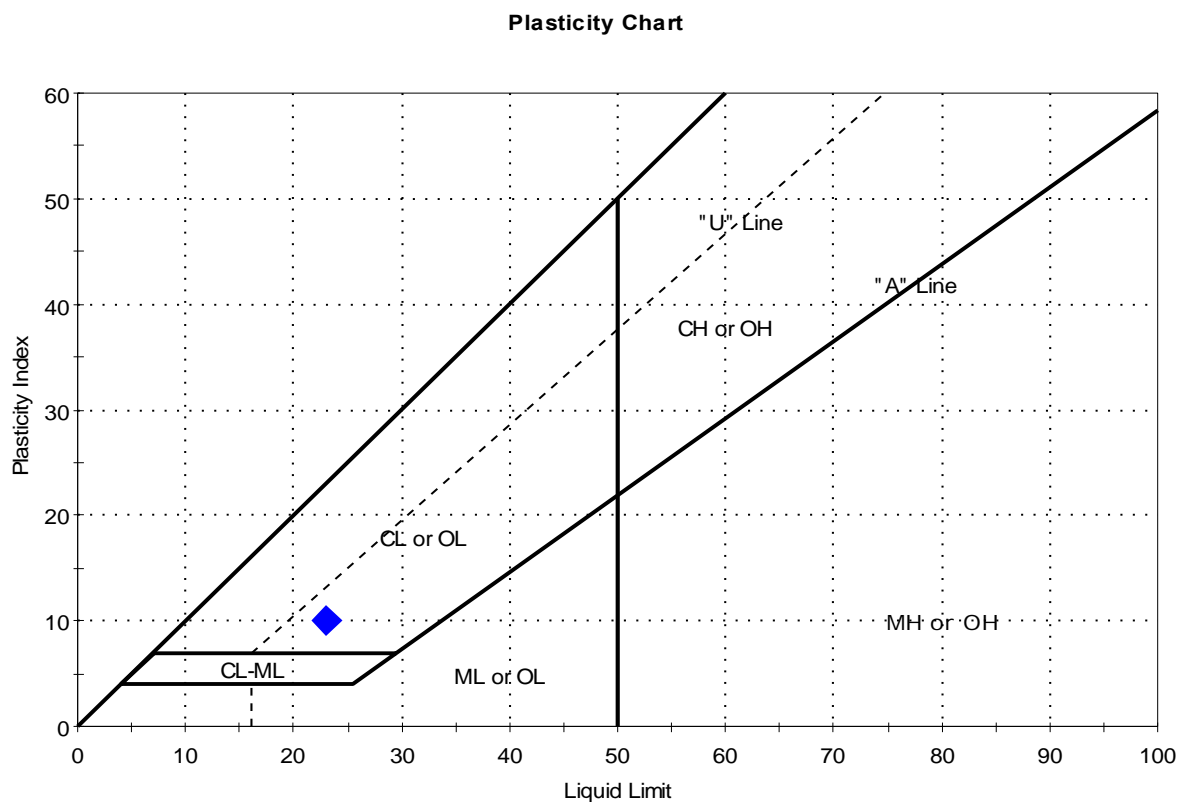
Dry Strength: HIGH

Dilatancy: NONE

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.			
Project:	Rt 9/I-395 Connector			
Location:	Brewer and Eddington, ME		Project No:	GTX-308853
Boring ID:	HB-BE-148	Sample Type:	jar	Tested By: GA
Sample ID:	6D	Test Date:	10/12/18	Checked By: emm
Depth :	14-15.3 ft	Test Id:	474391	
Test Comment:	---			
Visual Description:	Moist, olive sandy clay			
Sample Comment:	---			

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	6D	HB-BE-148	14-15.3 ft	16	23	13	10	0.3	Sandy Lean CLAY (CL)

Sample Prepared using the WET method

22% Retained on #40 Sieve

Dry Strength: HIGH

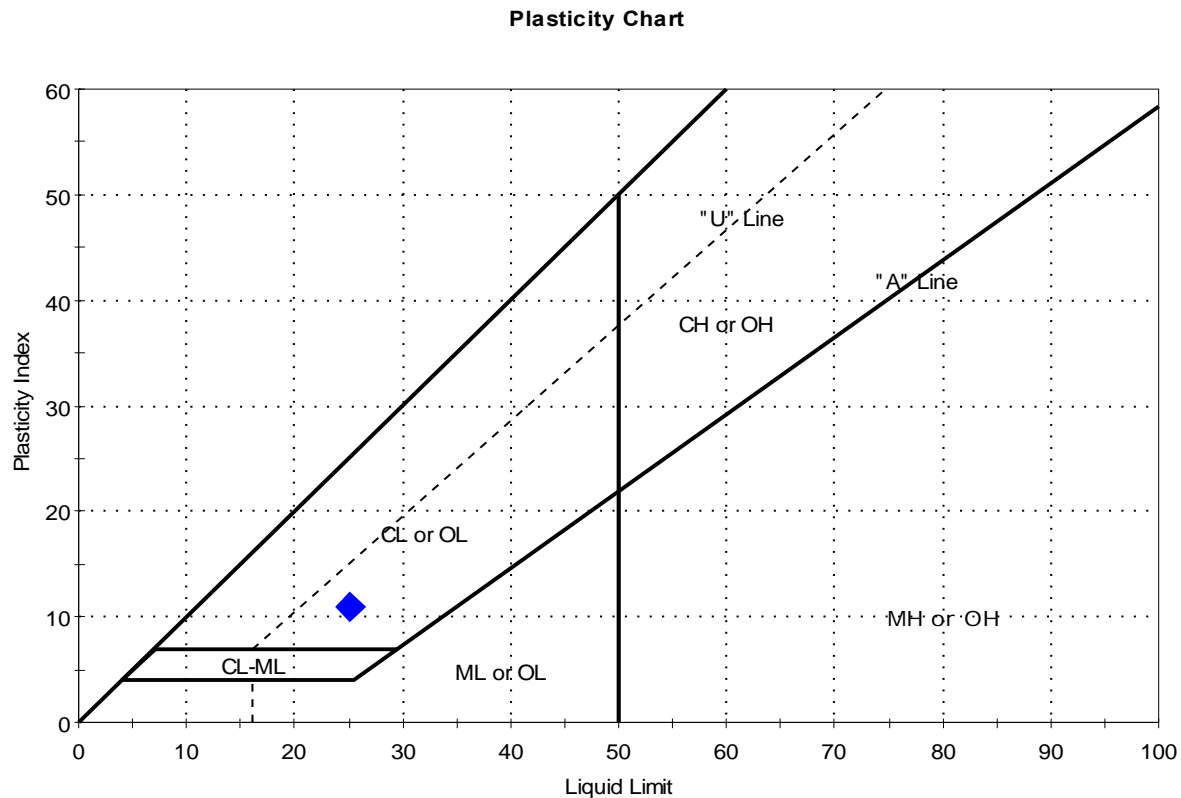
Dilatancy: NONE

Toughness: MEDIUM



Client:	Haley & Aldrich, Inc.				
Project:	Rt 9/I-395 Connector				
Location:	Brewer and Eddington, ME		Project No:	GTX-308853	
Boring ID:	HB-BE-151	Sample Type:	jar	Tested By:	GA
Sample ID:	5D	Test Date:	10/12/18	Checked By:	emm
Depth :	15-16.2 ft	Test Id:	474392		
Test Comment:	---				
Visual Description:	Moist, olive sandy clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318

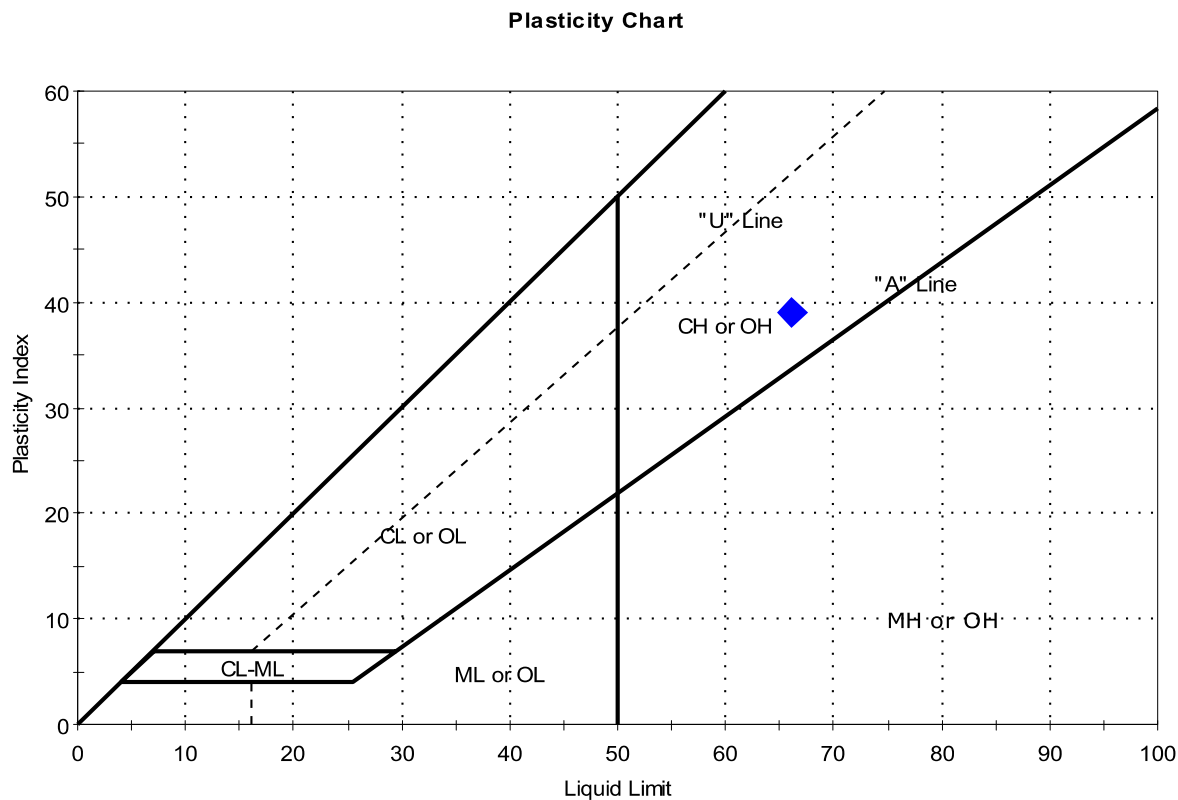


Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	HB-BE-151	15-16.2 ft	11	25	14	11	-0.2	Sandy Lean CLAY (CL)

Sample Prepared using the WET method
 30% Retained on #40 Sieve
 Dry Strength: HIGH
 Dilatancy: NONE
 Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-201	Sample Type:	tube
Sample ID:	U1	Test Date:	03/08/21
Depth :	30-32 ft	Test Id:	611422
Test Comment:	---		
Visual Description:	Moist, olive gray and very dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-201	30-32 ft	54	66	27	39	0.7	

Sample Prepared using the WET method

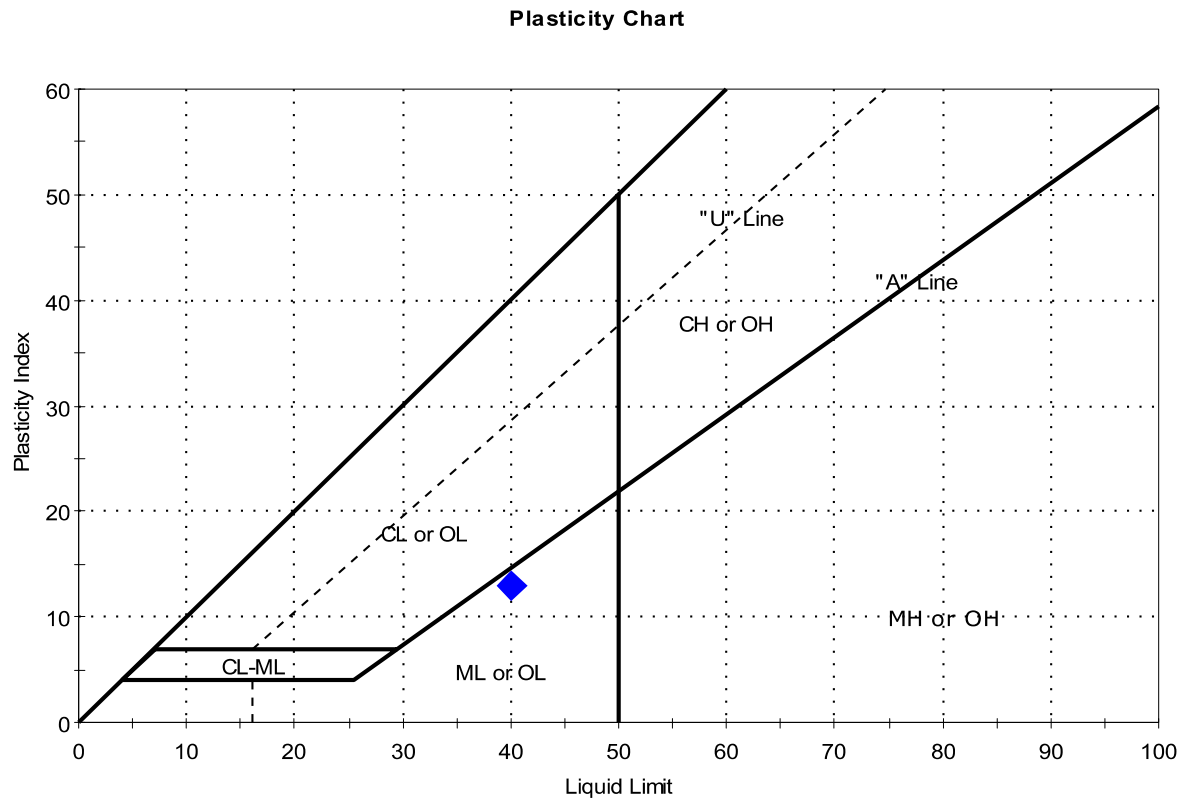
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-201	Sample Type:	tube
Sample ID:	U2	Test Date:	03/08/21
Depth :	46-48 ft	Test Id:	611423
Test Comment:	---		
Visual Description:	Moist, black and light olive silt		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	HB-BE-201	46-48 ft	36	40	27	13	0.7	

Sample Prepared using the WET method

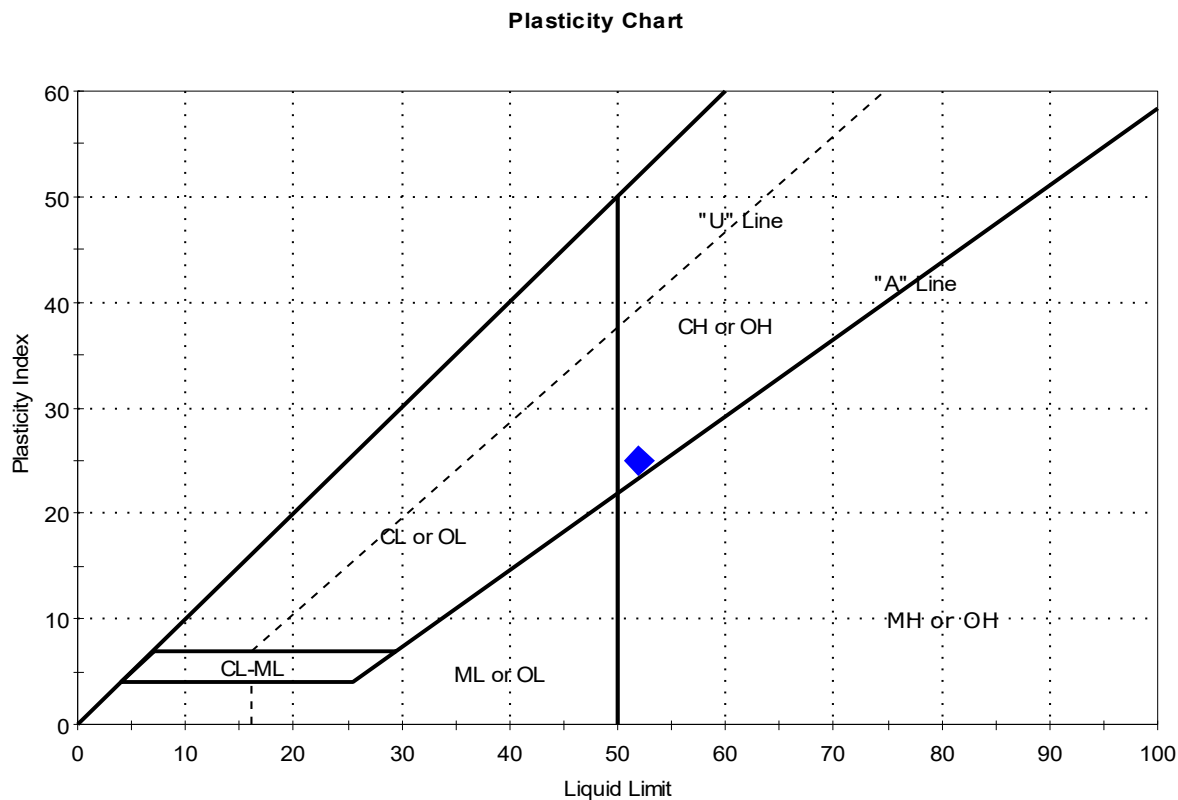
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.			Project No:	GTX-313370
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington				
Location:	Brewer, ME				
Boring ID:	HB-BE-201	Sample Type:	jar	Tested By:	GA
Sample ID:	6D	Test Date:	03/29/21	Checked By:	emm
Depth :	25-27	Test Id:	613864		
Test Comment:	---				
Visual Description:	Moist, gray sandy clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	6D	HB-BE-201	25-27	39	52	27	25	0.5	

Sample Prepared using the WET method

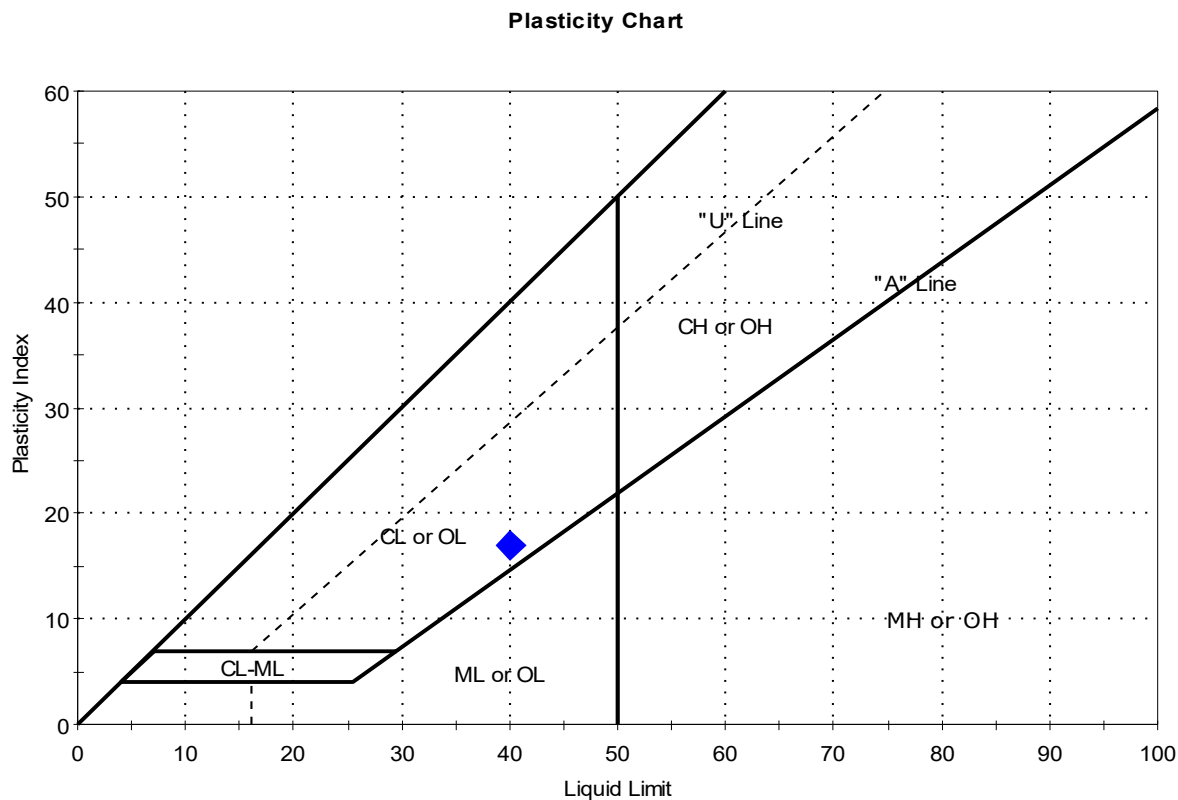
Dry Strength: VERY HIGH

Dilatancy: NONE

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-202	Sample Type:	tube
Sample ID:	U2	Test Date:	03/09/21
Depth :	18-20 ft	Test Id:	611424
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	HB-BE-202	18-20 ft	38	40	23	17	0.9	

Sample Prepared using the WET method

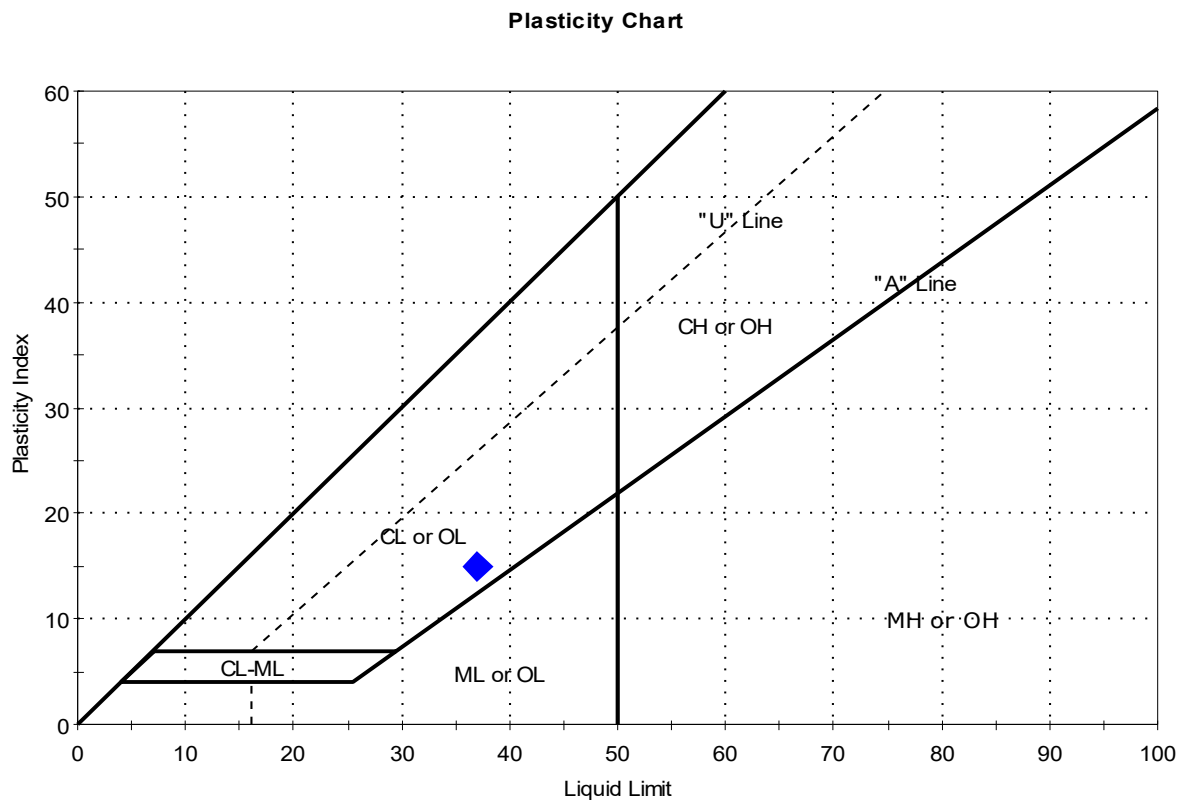
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-204	Sample Type:	tube
Sample ID:	U1	Test Date:	03/10/21
Depth :	13-15 ft	Test Id:	611425
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-204	13-15 ft	34	37	22	15	0.8	

Sample Prepared using the WET method

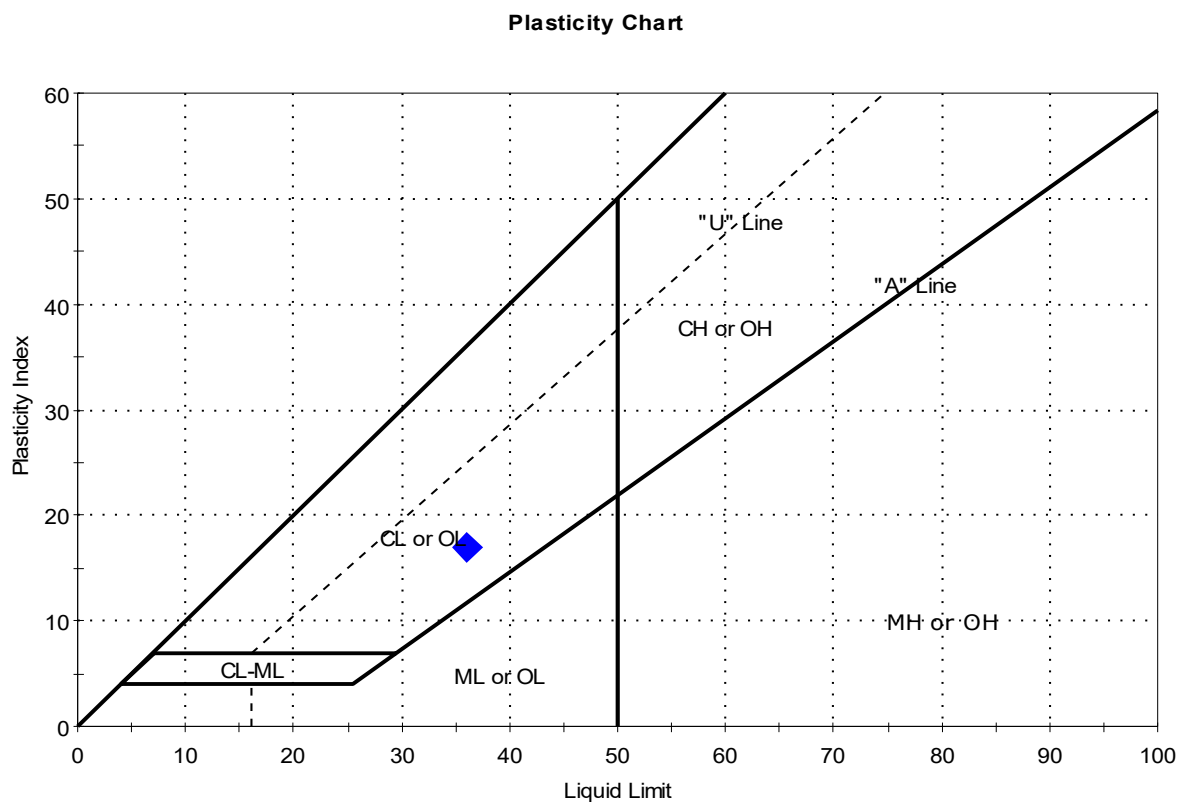
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-205	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	12-14 ft	Test Id:	611426
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-205	12-14 ft	36	36	19	17	1	

Sample Prepared using the WET method

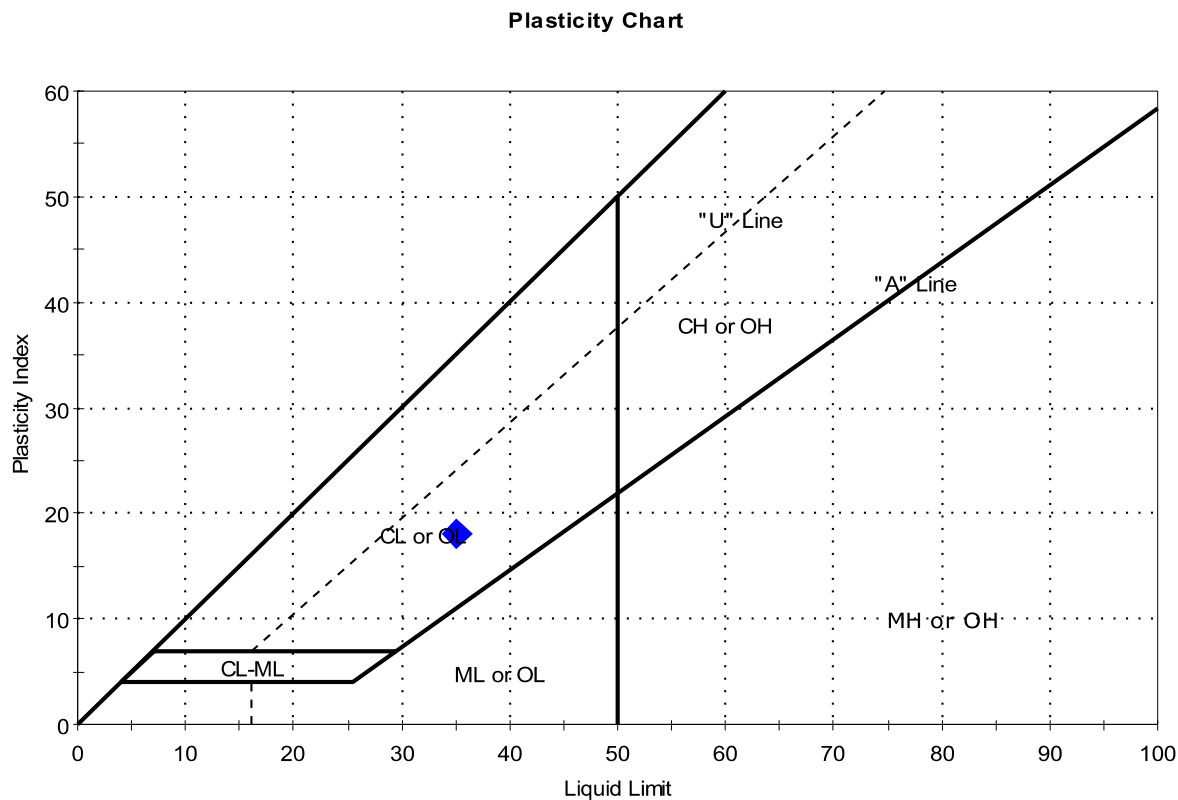
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-206	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	12-14 ft	Test Id:	611427
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-206	12-14 ft	38	35	17	18	1.1	

Sample Prepared using the WET method

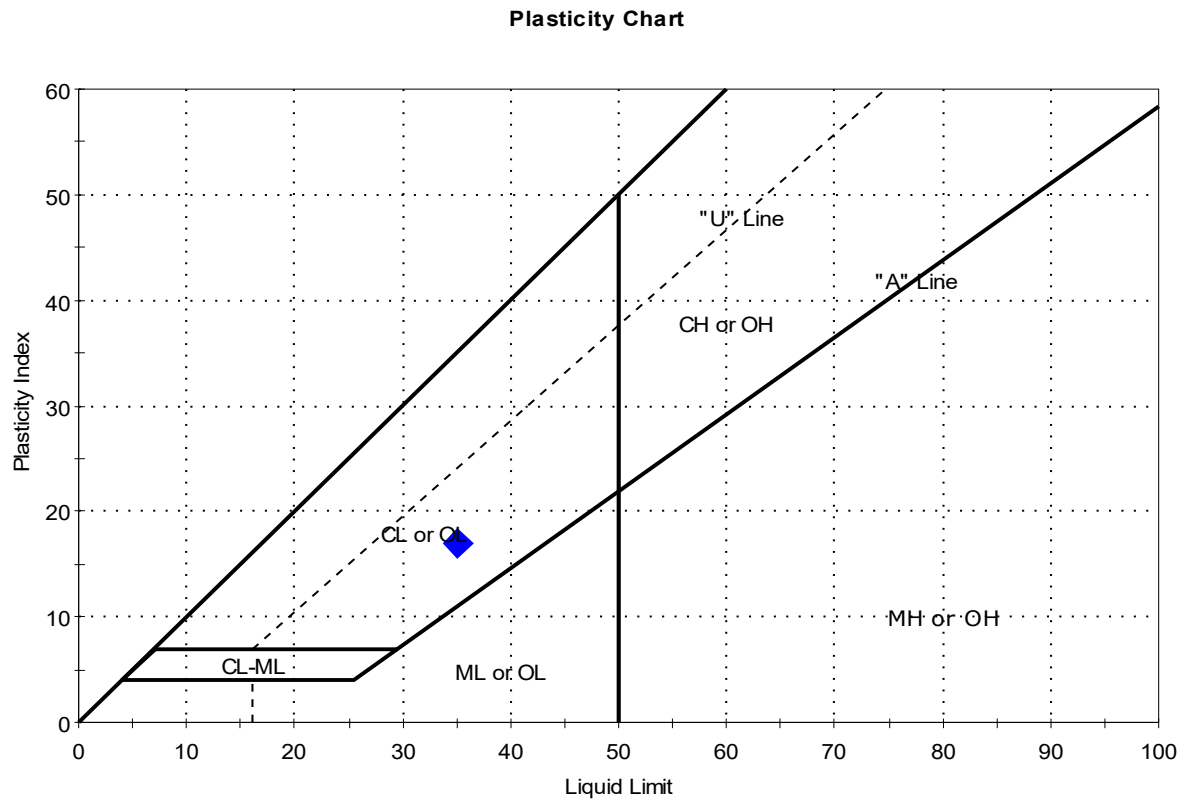
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-207	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	5-7 ft	Test Id:	611428
Test Comment:	---		
Visual Description:	Moist, olive gray and brownish yellow clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-207	5-7 ft	27	35	18	17	0.5	

Sample Prepared using the WET method

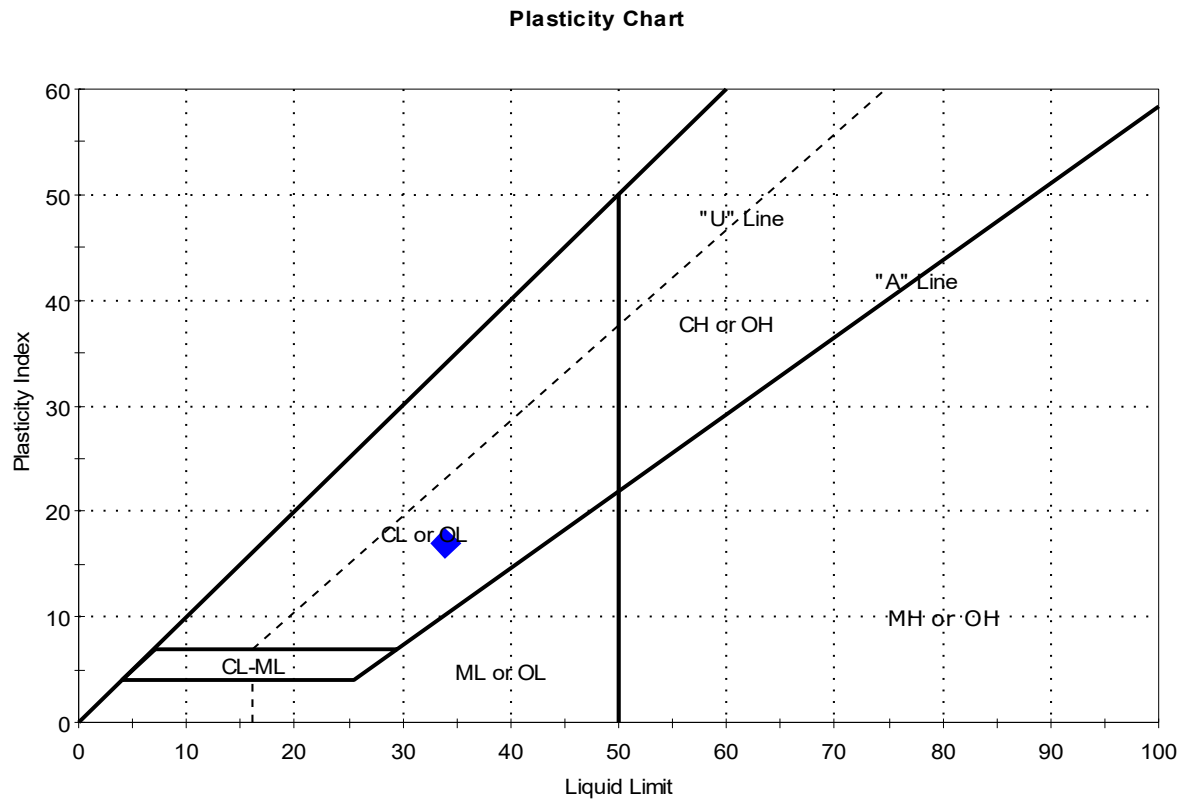
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-208	Sample Type:	tube
Sample ID:	U1	Test Date:	03/08/21
Depth :	10-12 ft	Test Id:	611429
Test Comment:	---		
Visual Description:	Moist, gray and olive yellow clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-208	10-12 ft	32	34	17	17	0.9	

Sample Prepared using the WET method

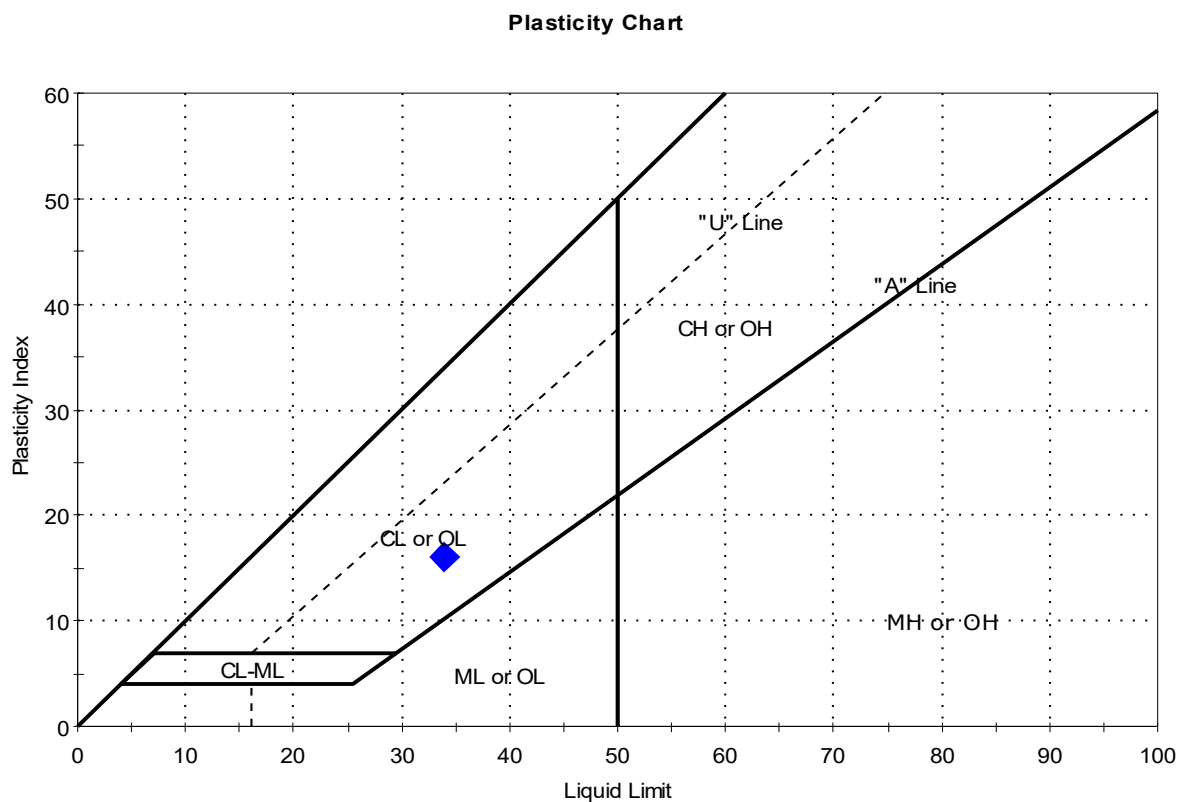
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-210	Sample Type:	tube
Sample ID:	U1	Test Date:	03/17/21
Depth :	15-17 ft	Test Id:	611443
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-210	15-17 ft	33	34	18	16	1	

Sample Prepared using the WET method

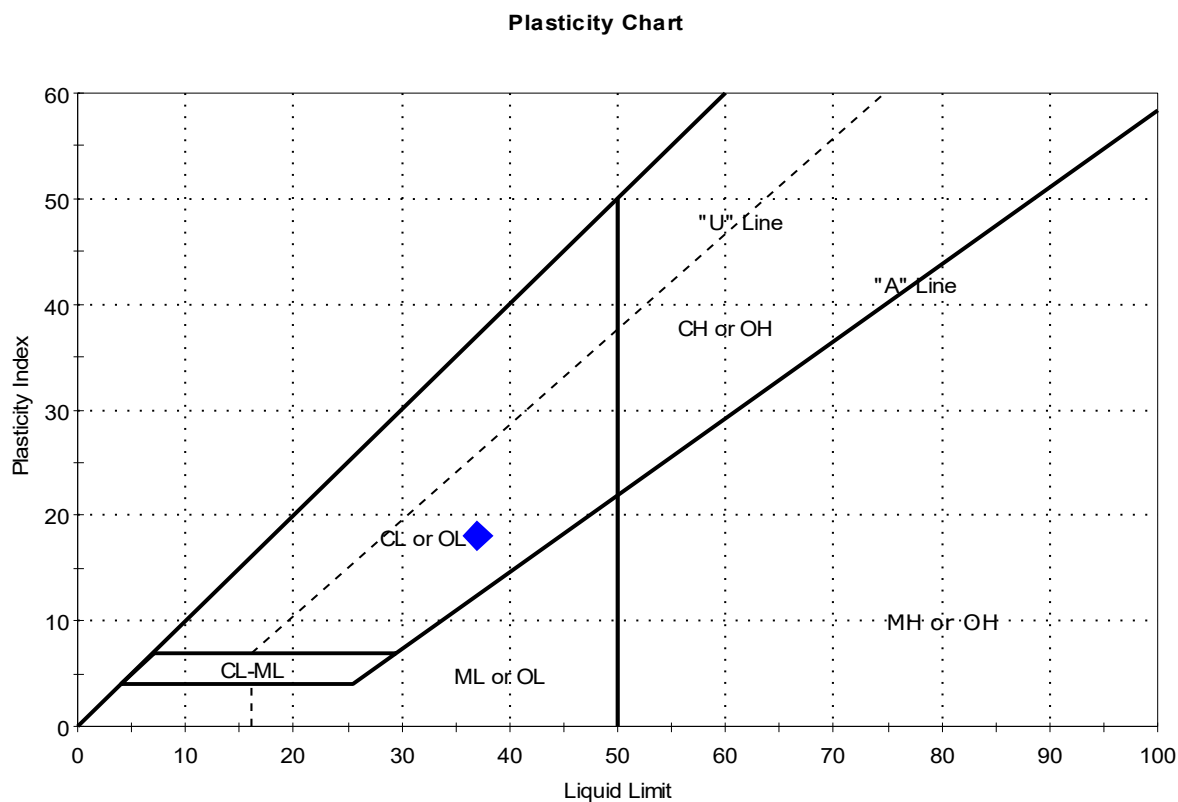
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-215	Sample Type:	tube
Sample ID:	U1	Test Date:	03/09/21
Depth :	10-12 ft	Test Id:	611444
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-215	10-12 ft	36	37	19	18	0.9	

Sample Prepared using the WET method

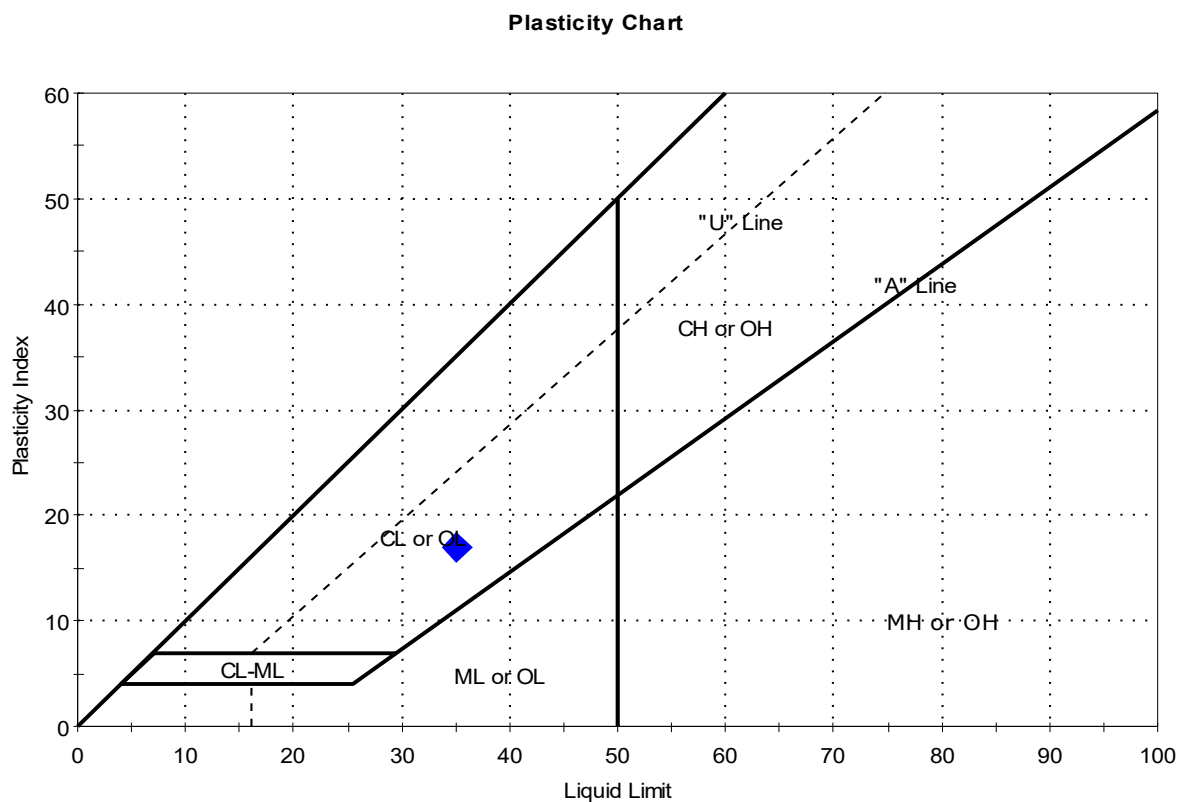
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-216	Sample Type:	tube
Sample ID:	U1	Test Date:	03/17/21
Depth :	8-10 ft	Test Id:	611445
Test Comment:	---		
Visual Description:	Moist, olive gray clay		
Sample Comment:	----		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-216	8-10 ft	31	35	18	17	0.7	

Sample Prepared using the WET method

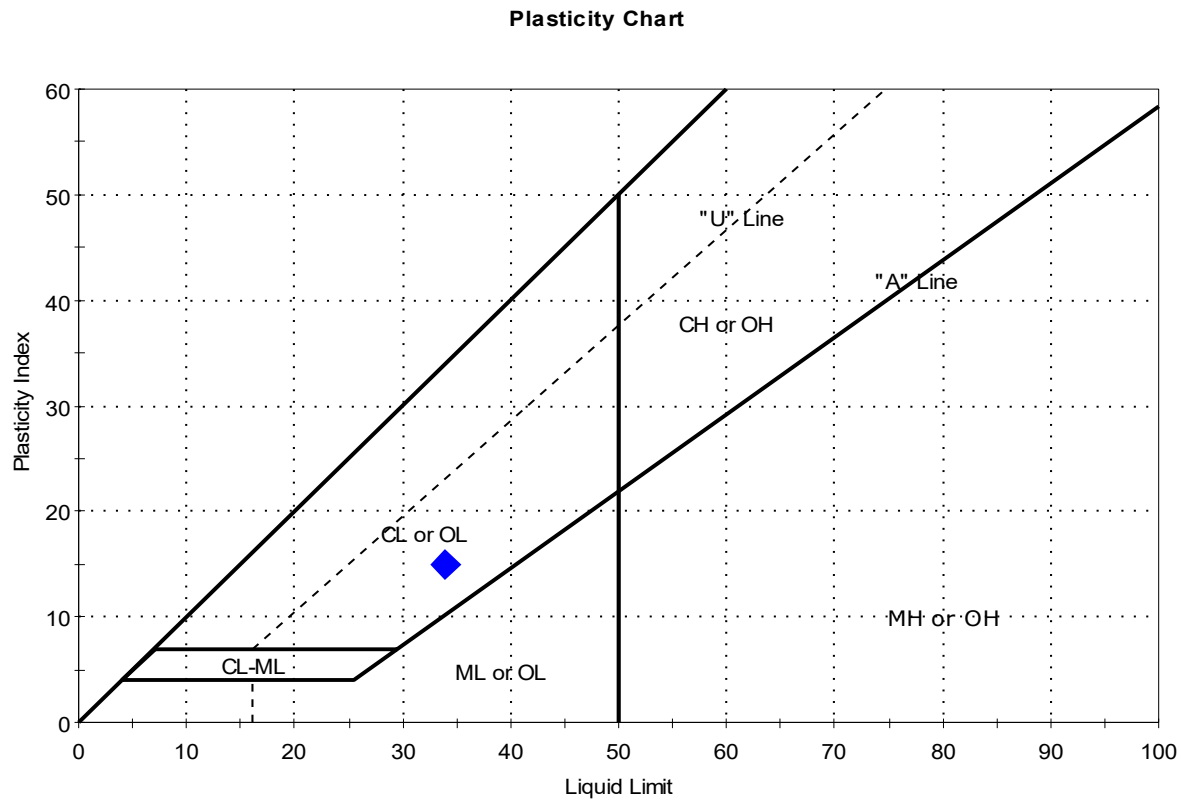
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 3)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313197
Boring ID:	HB-BE-223	Sample Type:	tube
Sample ID:	U1(19.2")	Test Date:	03/23/21
Depth :	12-14	Test Id:	613068
Test Comment:	---		
Visual Description:	Wet, dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1(19.2")	HB-BE-223	12-14	42	34	19	15	1.5	

Sample Prepared using the WET method

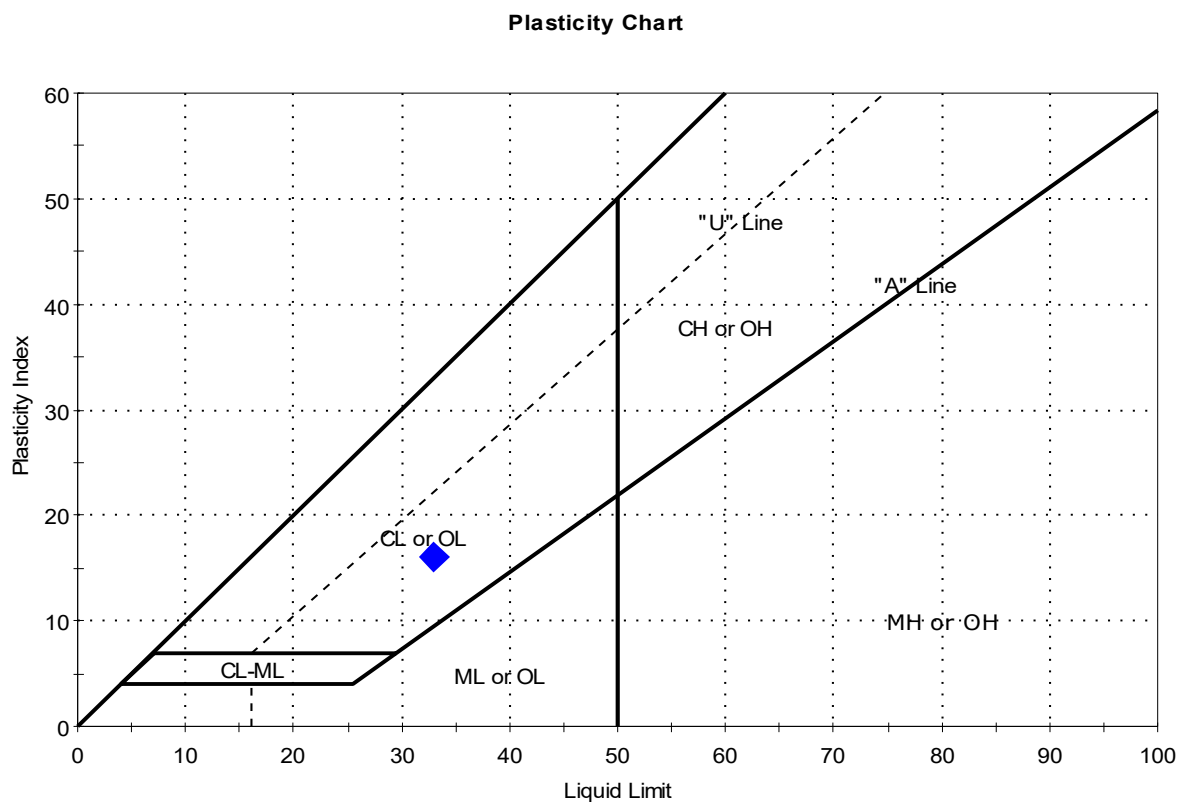
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 3)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313197
Boring ID:	HB-BE-223A	Sample Type:	tube
Sample ID:	U1(20.4")	Test Date:	03/23/21
Depth :	8-10	Test Id:	613069
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1(20.4")	B-BE-223	8-10	36	33	17	16	1.2	

Sample Prepared using the WET method

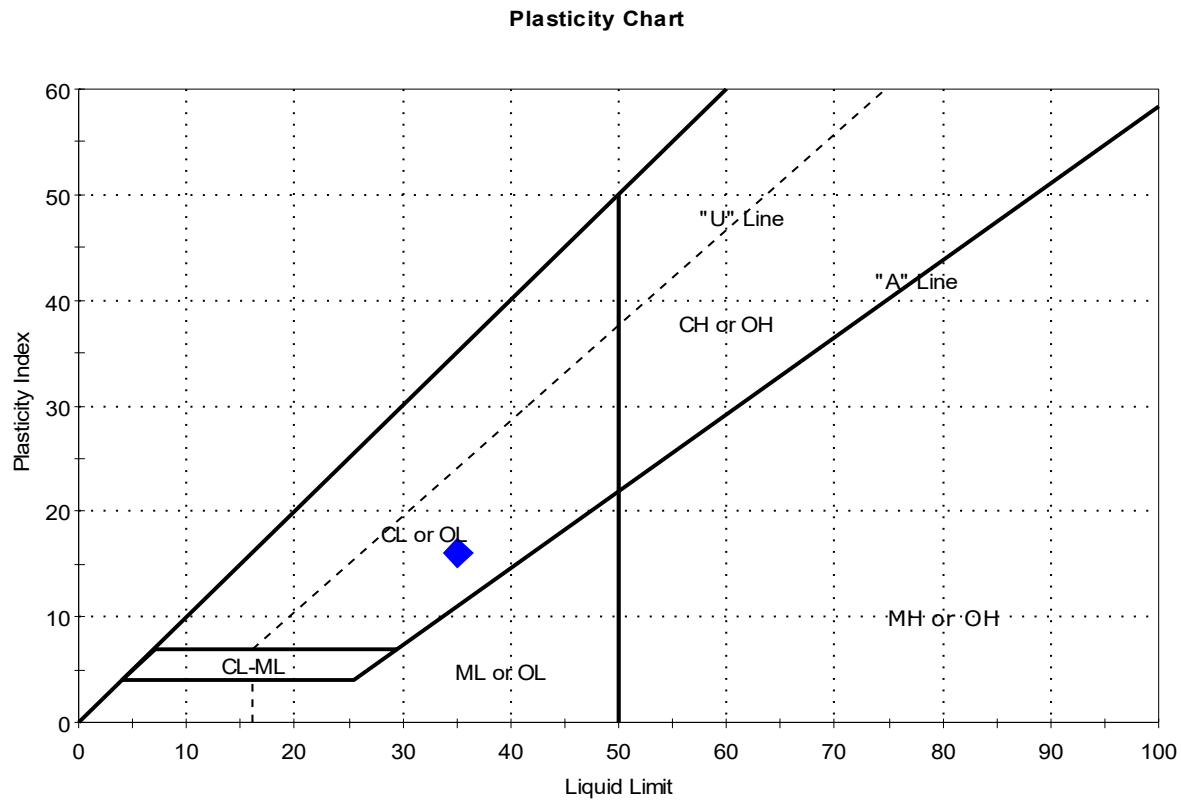
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.			Project No:	GTX-313370
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington				
Location:	Brewer, ME				
Boring ID:	HB-BE-223A	Sample Type:	jar	Tested By:	GA
Sample ID:	2D	Test Date:	03/29/21	Checked By:	emm
Depth :	5-7	Test Id:	613865		
Test Comment:	---				
Visual Description:	Moist, light gray sandy clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	2D	B-BE-223	5-7	29	35	19	16	0.6	

Sample Prepared using the WET method

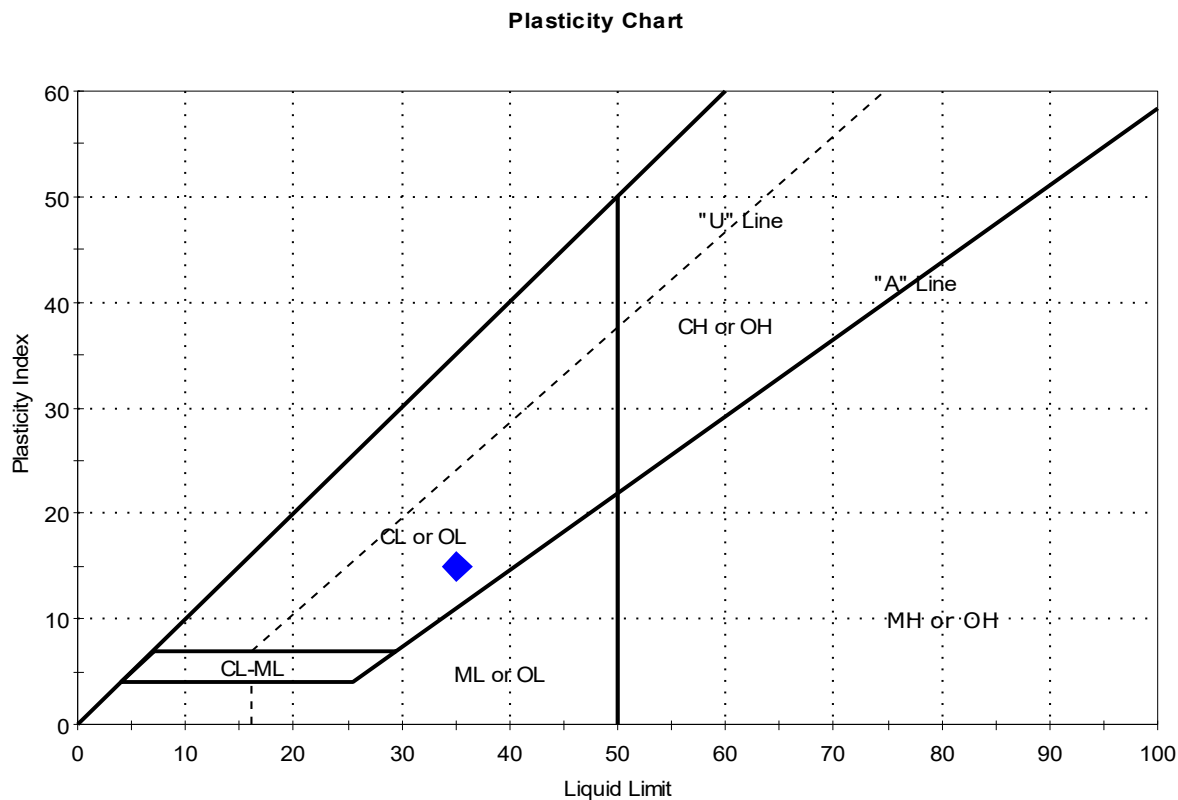
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 3)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313197
Boring ID:	HB-BE-224	Sample Type:	tube
Sample ID:	U1(21.6")	Test Date:	03/26/21
Depth :	8-10	Test Id:	613070
Test Comment:	---		
Visual Description:	Moist, dark gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1(21.6")	HB-BE-224	8-10	34	35	20	15	0.9	

Sample Prepared using the WET method

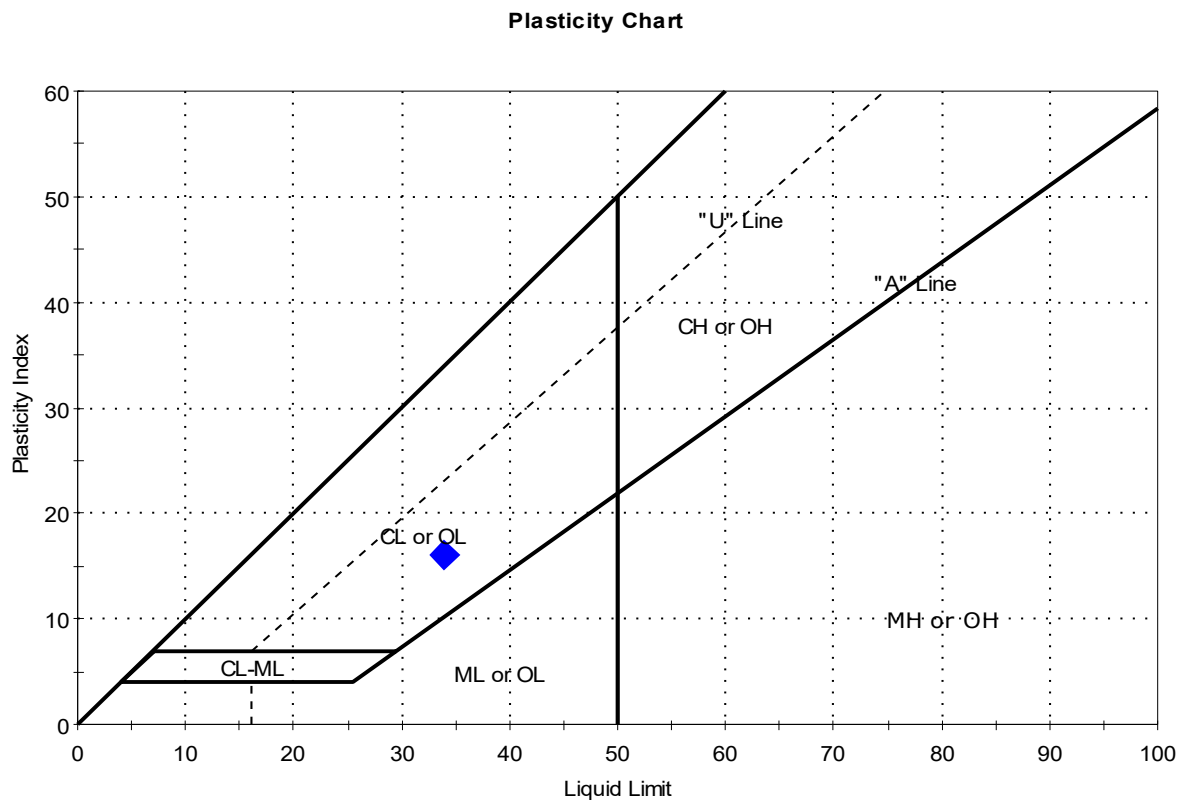
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.			Project No:	GTX-313370
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington				
Location:	Brewer, ME				
Boring ID:	HB-BE-224	Sample Type:	jar	Tested By:	GA
Sample ID:	2D	Test Date:	03/29/21	Checked By:	emm
Depth :	5-7	Test Id:	613866		
Test Comment:	---				
Visual Description:	Moist, olive gray clay				
Sample Comment:	---				

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	2D	HB-BE-224	5-7	29	34	18	16	0.7	

Sample Prepared using the WET method

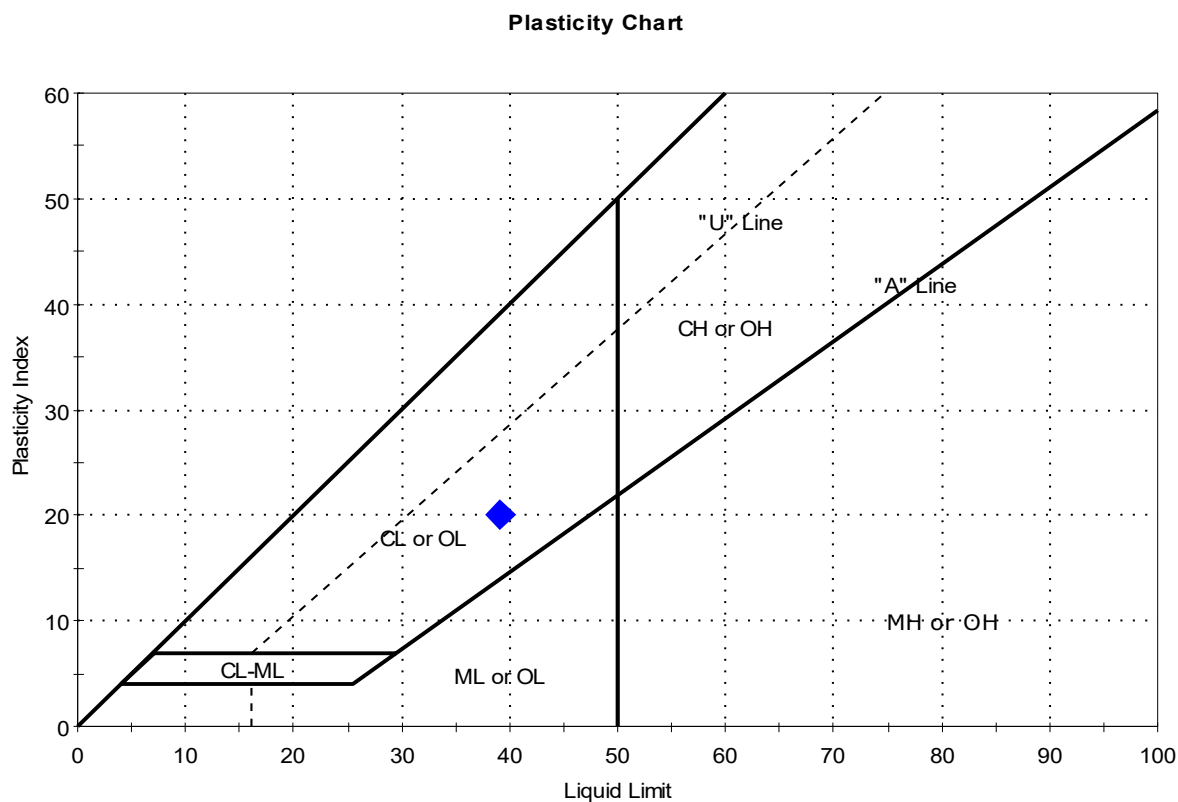
Dry Strength: VERY HIGH

Dilatancy: NONE

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-224A	Sample Type:	jar
Sample ID:	2D	Test Date:	03/29/21
Depth :	5-7	Test Id:	613867
Test Comment:	---		
Visual Description:	Moist, gray sandy clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	2D	B-BE-224	5-7	27	39	19	20	0.4	

Sample Prepared using the WET method

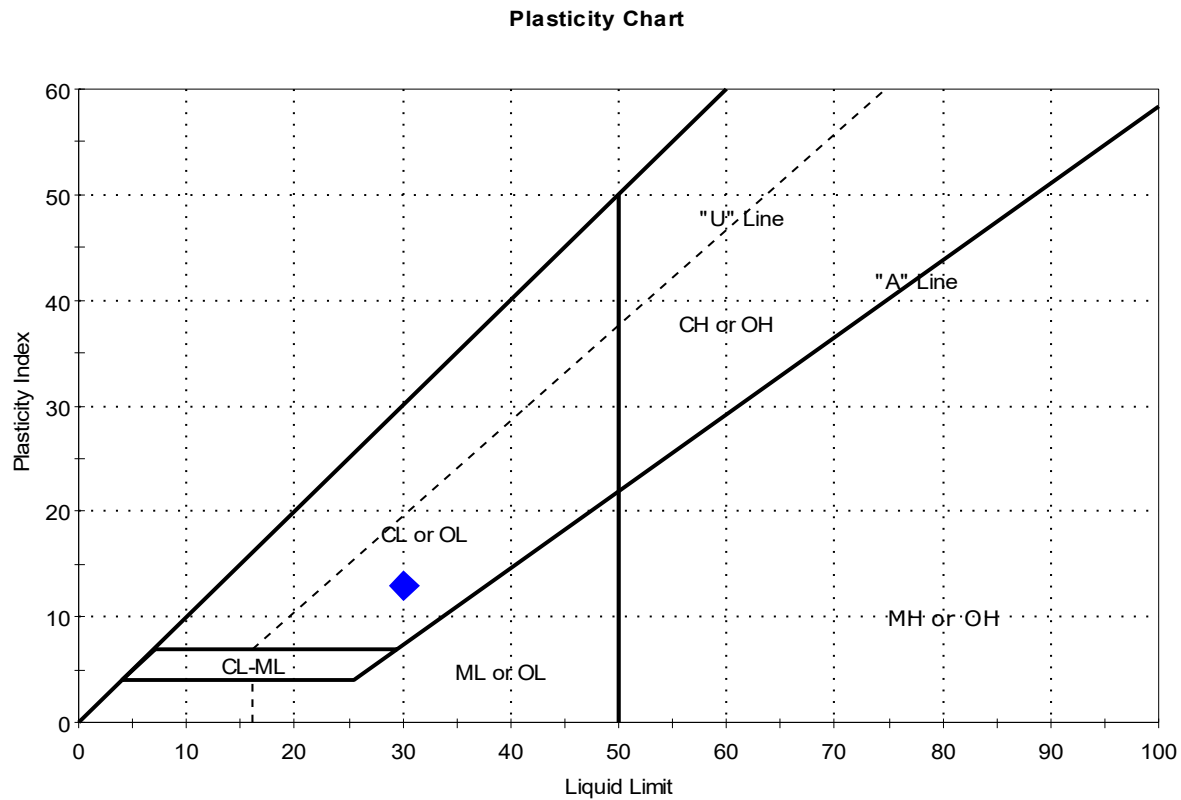
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: MEDIUM

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 4)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313198
Boring ID:	HB-BE-225	Sample Type:	tube
Sample ID:	U1	Test Date:	03/23/21
Depth :	8-10 ft	Test Id:	611817
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-225	8-10 ft	32	30	17	13	1.2	

Sample Prepared using the WET method

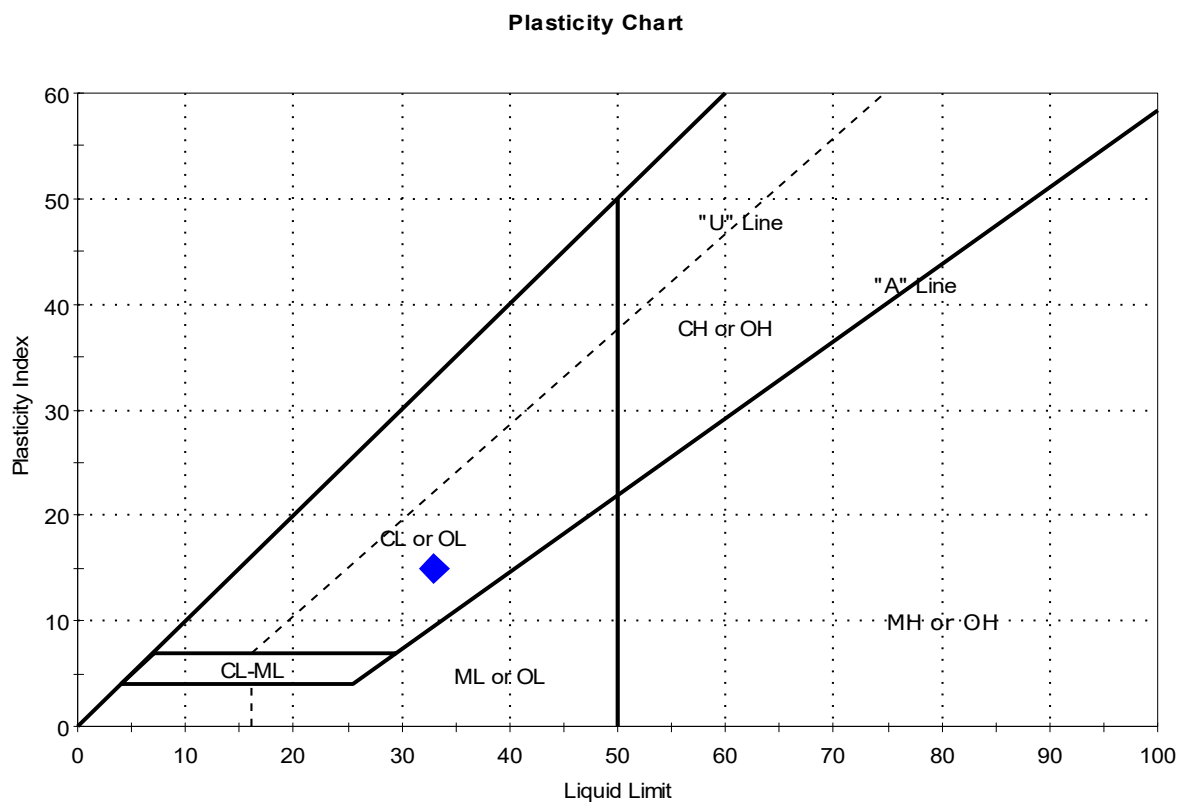
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 4)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313198
Boring ID:	HB-BB-226	Sample Type:	tube
Sample ID:	U1	Test Date:	03/22/21
Depth :	6-8 ft	Test Id:	611818
Test Comment:	---		
Visual Description:	Moist, olive gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BB-22	6-8 ft	31	33	18	15	0.9	

Sample Prepared using the WET method

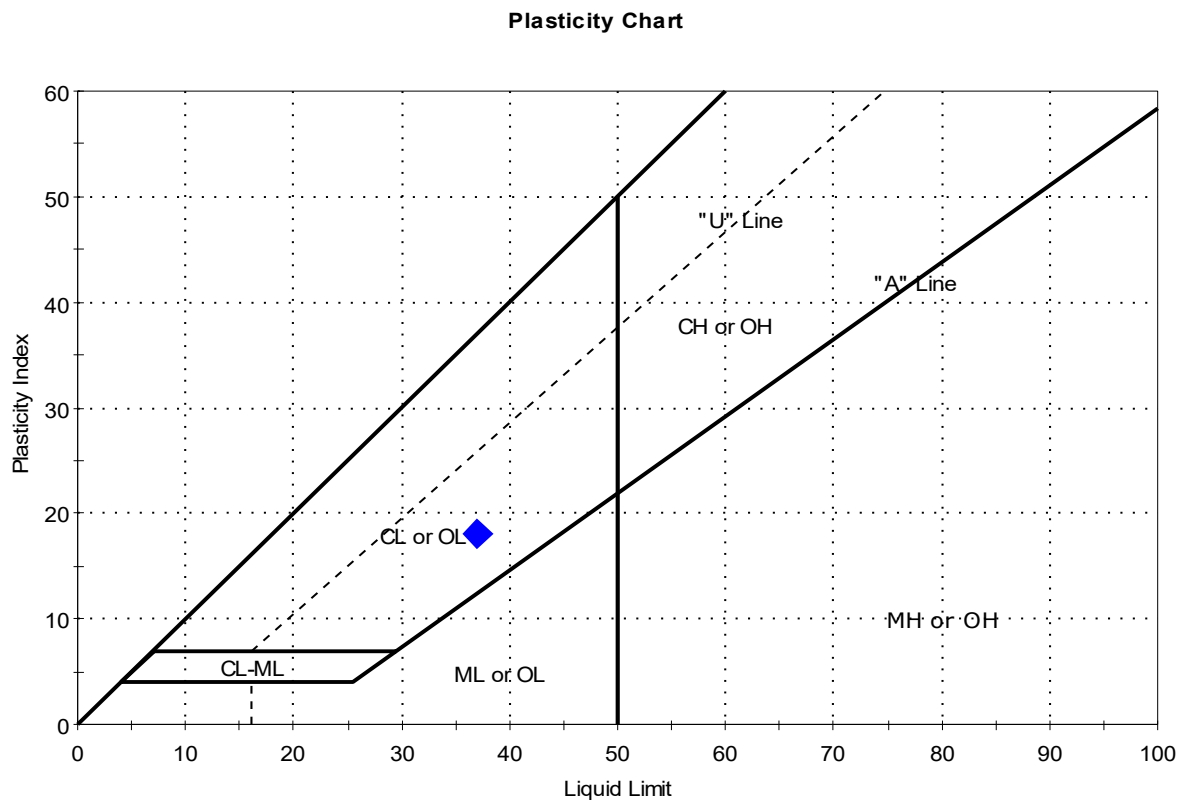
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 4)		
Location:	Brewer-Eddington, ME	Project No:	GTX-313198
Boring ID:	HB-BB-227	Sample Type:	tube
Sample ID:	U1	Test Date:	03/23/21
Depth :	5-7 ft	Test Id:	611819
Test Comment:	---		
Visual Description:	Moist, olive gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BB-22	5-7 ft	31	37	19	18	0.7	

Sample Prepared using the WET method

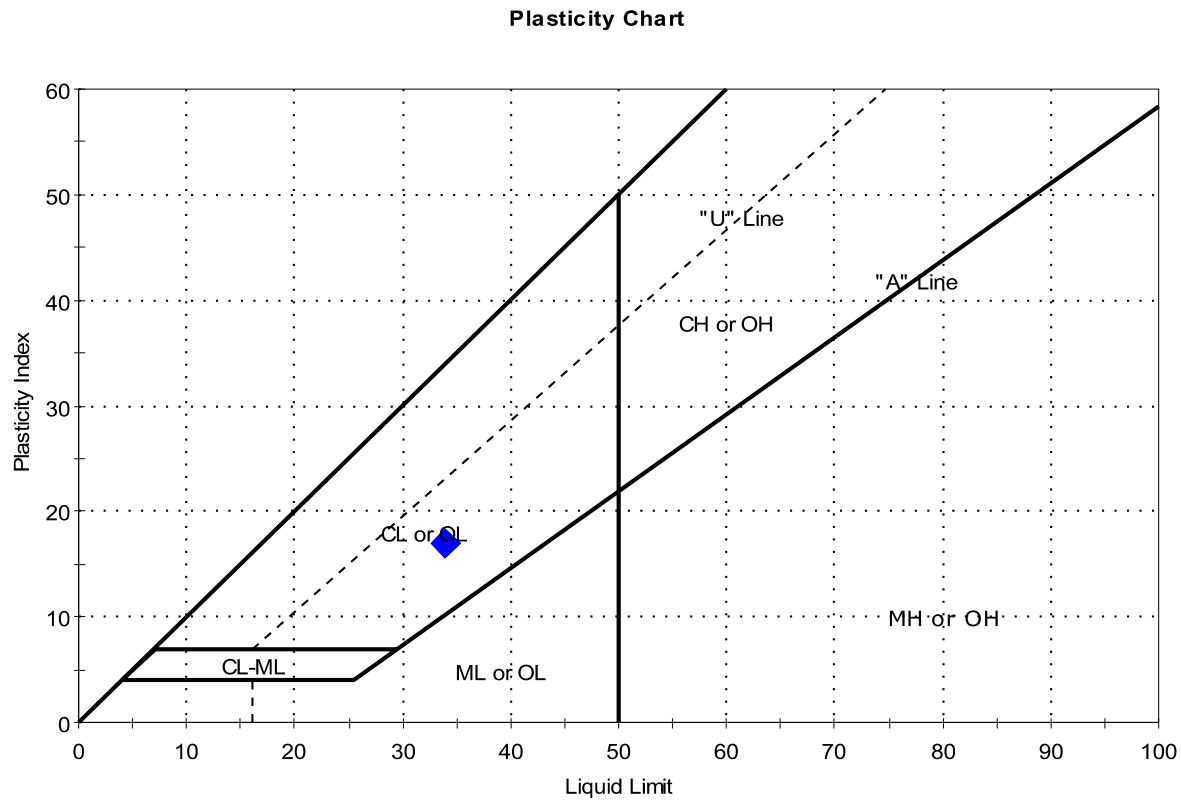
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-239	Sample Type:	tube
Sample ID:	U1	Test Date:	03/10/21
Depth :	13-15 ft	Test Id:	611439
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-239	13-15 ft	36	34	17	17	1.1	

Sample Prepared using the WET method

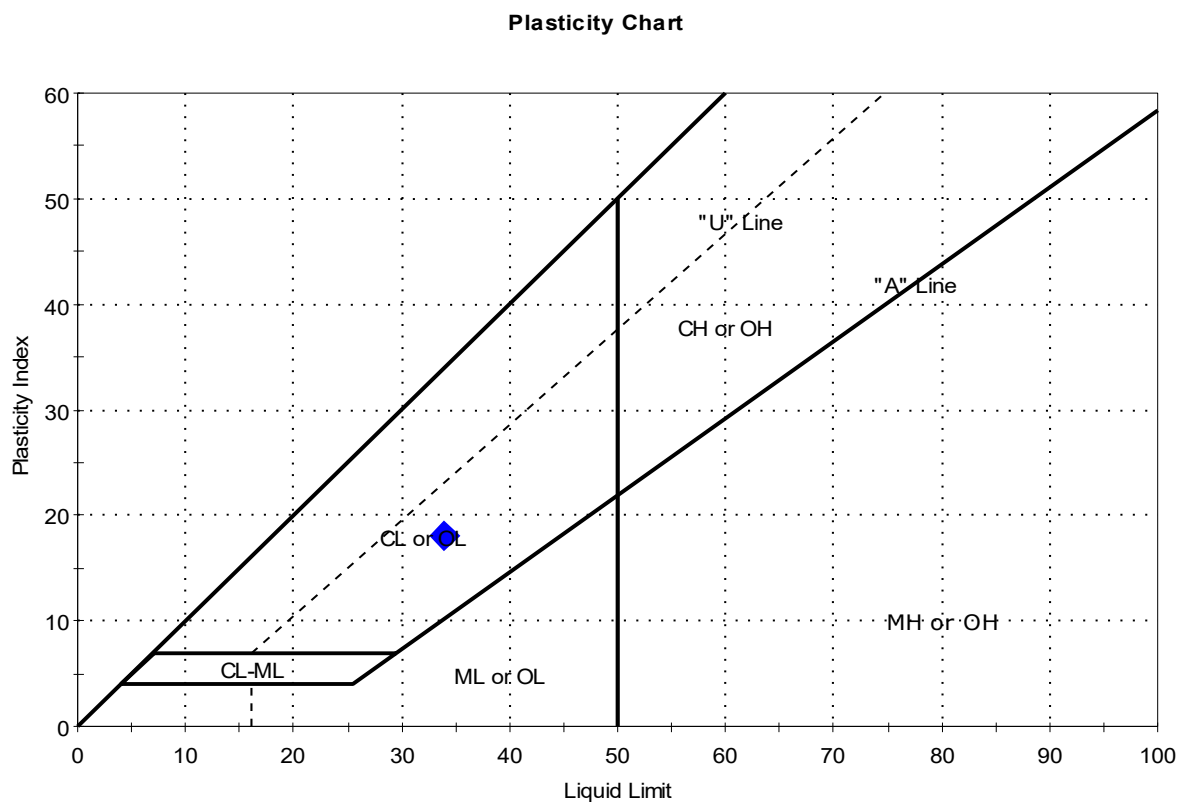
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-239	Sample Type:	tube
Sample ID:	U2	Test Date:	03/09/21
Depth :	23-25 ft	Test Id:	611440
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	HB-BE-239	23-25 ft	37	34	16	18	1.2	

Sample Prepared using the WET method

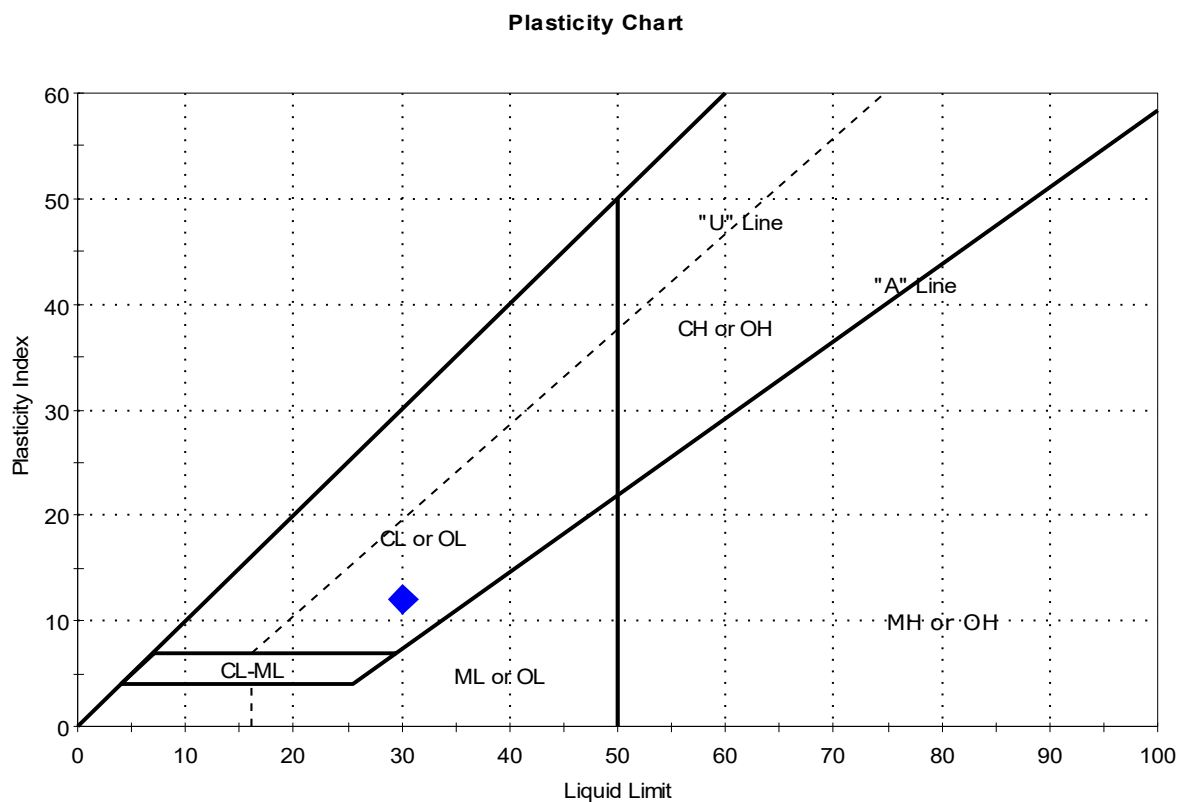
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-240	Sample Type:	tube
Sample ID:	U1	Test Date:	03/08/21
Depth :	10-12 ft	Test Id:	611441
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	----		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	HB-BE-240	10-12 ft	27	30	18	12	0.8	

Sample Prepared using the WET method

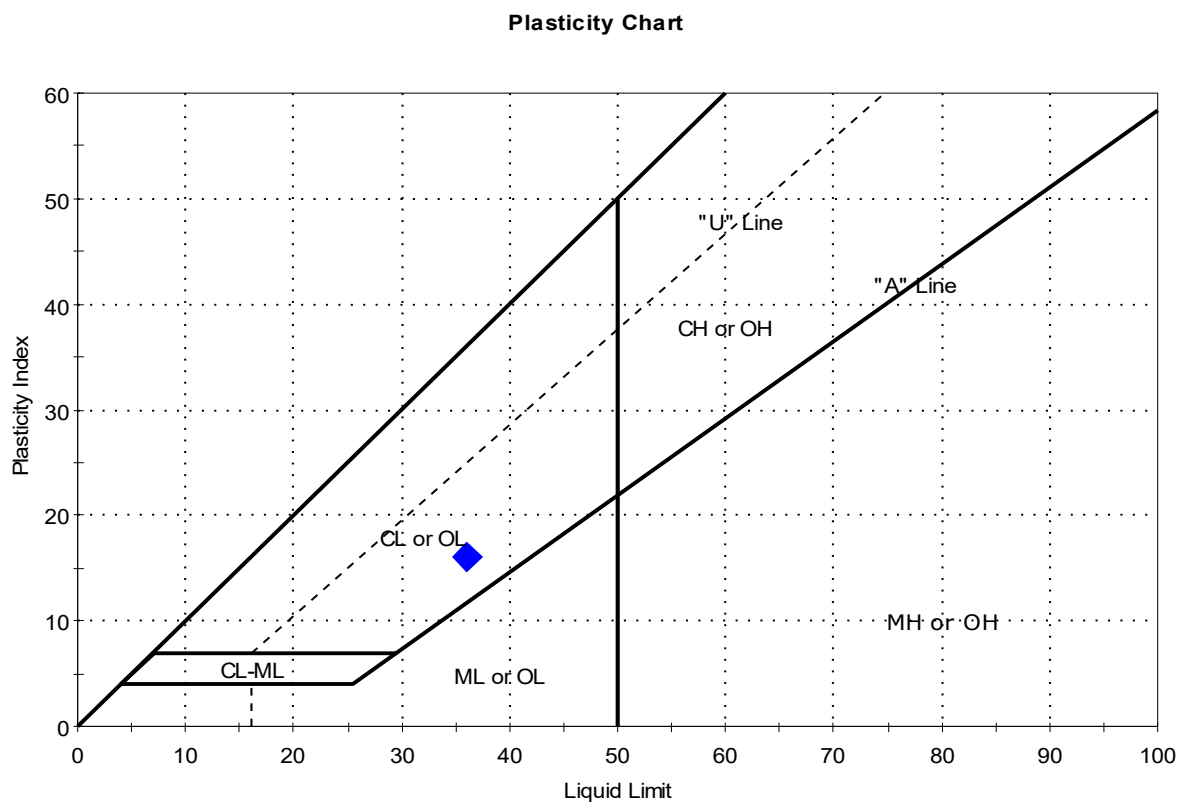
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-240	Sample Type:	tube
Sample ID:	U2	Test Date:	03/08/21
Depth :	20-22 ft	Test Id:	611442
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U2	HB-BE-240	20-22 ft	38	36	20	16	1.1	

Sample Prepared using the WET method

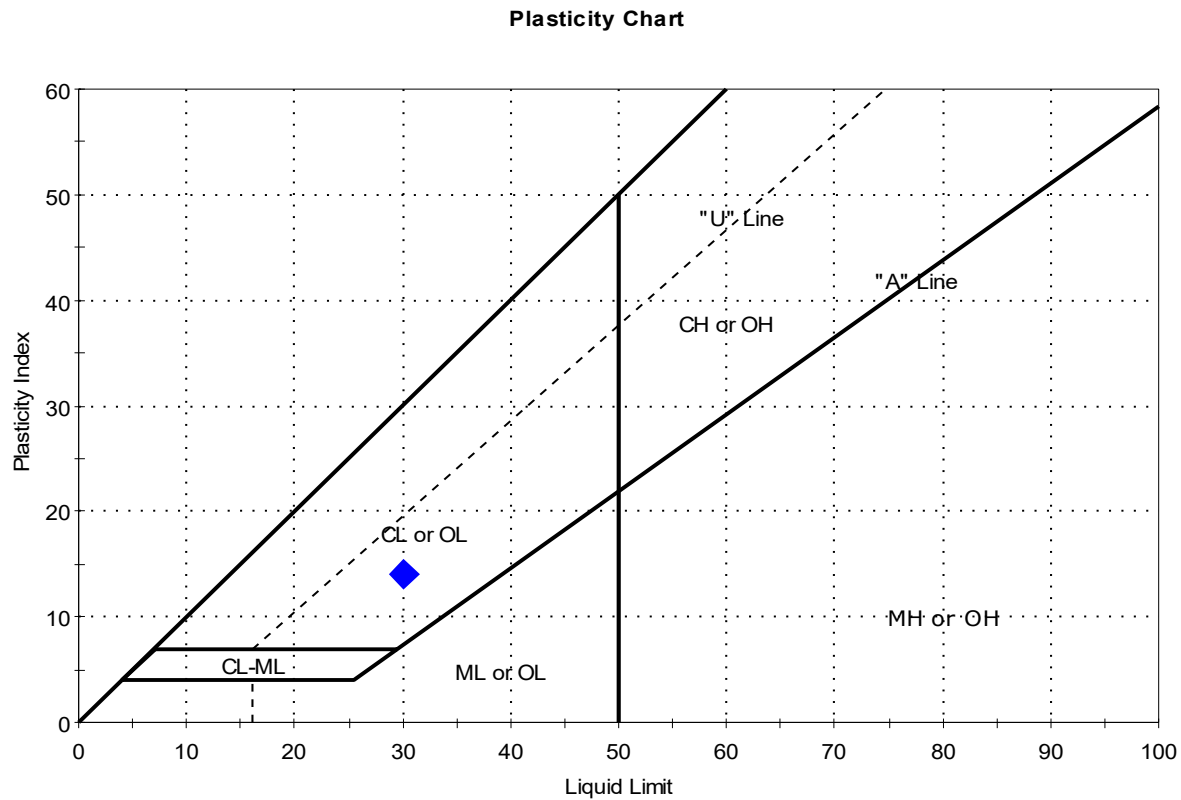
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector (Area 1)		
Location:	Brewer-Eddington, ME	Project No:	GTX-312665
Boring ID:	HB-BE-242A	Sample Type:	tube
Sample ID:	U1	Test Date:	03/17/21
Depth :	9-11 ft	Test Id:	611446
Test Comment:	---		
Visual Description:	Moist, gray clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	U1	B-BE-242	9-11 ft	31	30	16	14	1.1	

Sample Prepared using the WET method

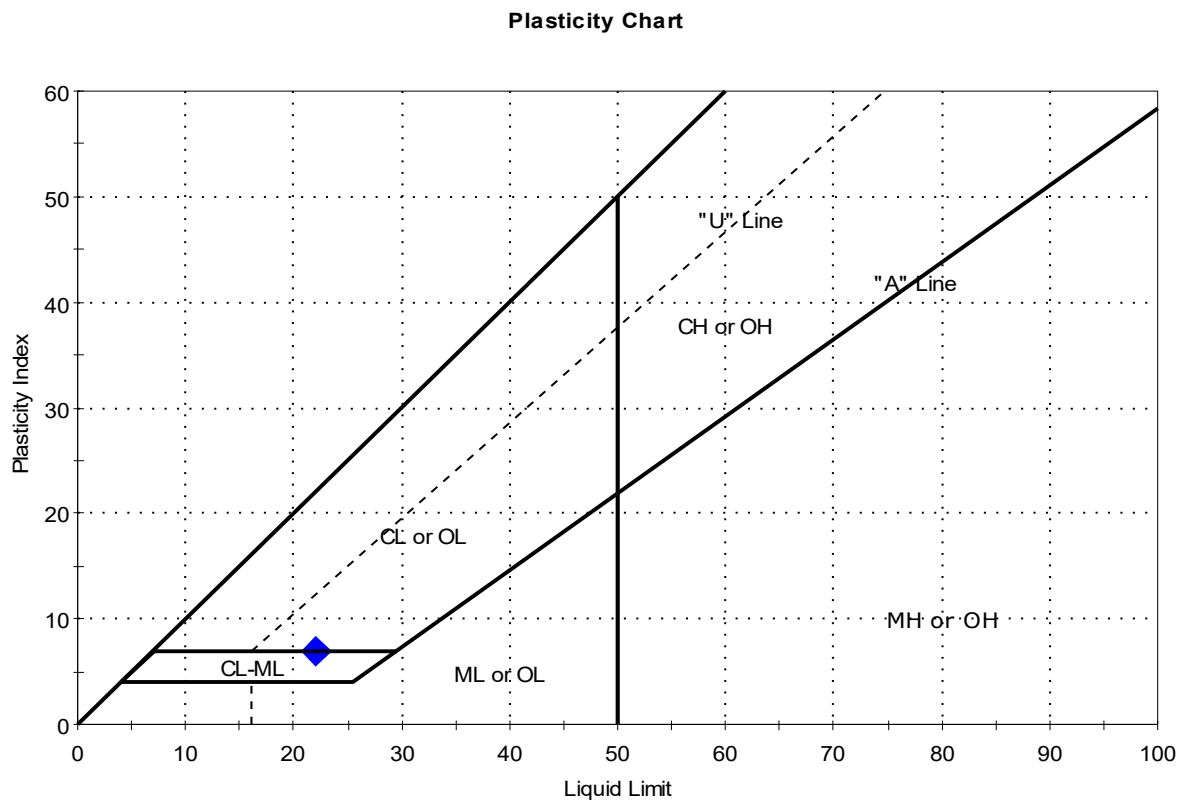
Dry Strength: VERY HIGH

Dilatancy: SLOW

Toughness: LOW

Client:	Haley & Aldrich, Inc.		
Project:	I-395/Rte 9 Connector Hwy, Brewer-Eddington		
Location:	Brewer, ME	Project No:	GTX-313370
Boring ID:	HB-BE-328-E1	Sample Type:	jar
Sample ID:	5D	Test Date:	03/29/21
Depth :	20-22	Test Id:	613868
Test Comment:	---		
Visual Description:	Moist, greenish gray silty clay		
Sample Comment:	---		

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	5D	BE-328	20-22	18	22	15	7	0.4	

Sample Prepared using the WET method

Dry Strength: HIGH
Dilatancy: SLOW
Toughness: MEDIUM



Client: Haley & Aldrich, Inc.

Project Name: Rt 9/ I-395 Connector

Project Location: Brewer and Eddington, ME

Project Number: GTX-308853

Tested By: trm

Checked By: mcm

Boring ID: BB-BEB-101

Preparation: Intact

Description: Moist, dark gray clay

Classification: ---

Group Symbol: ---

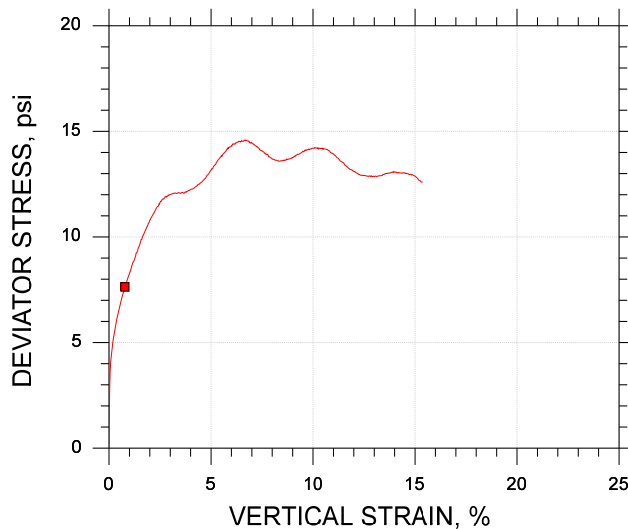
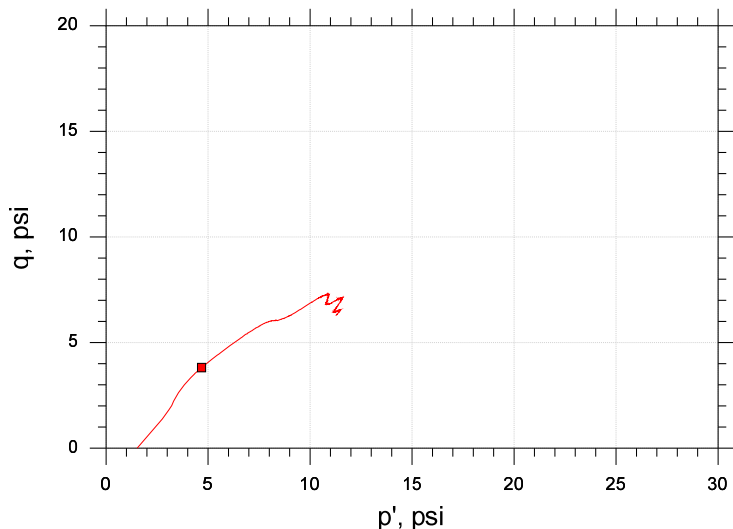
Liquid Limit: 35

Plastic Limit: 19

Plasticity Index: 16

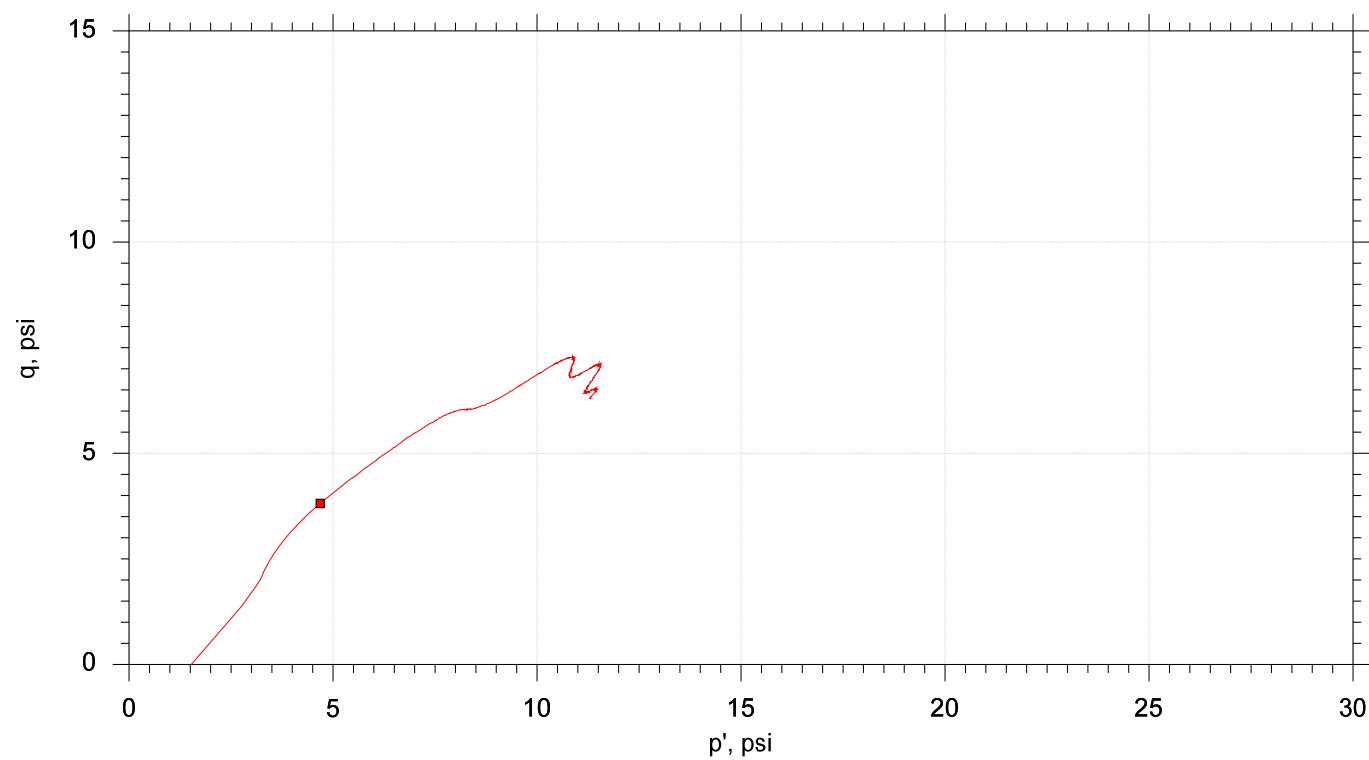
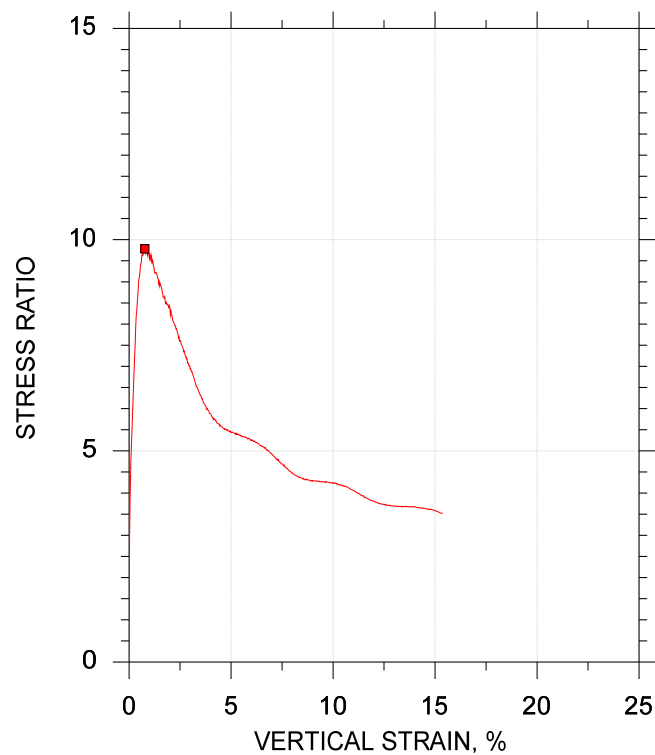
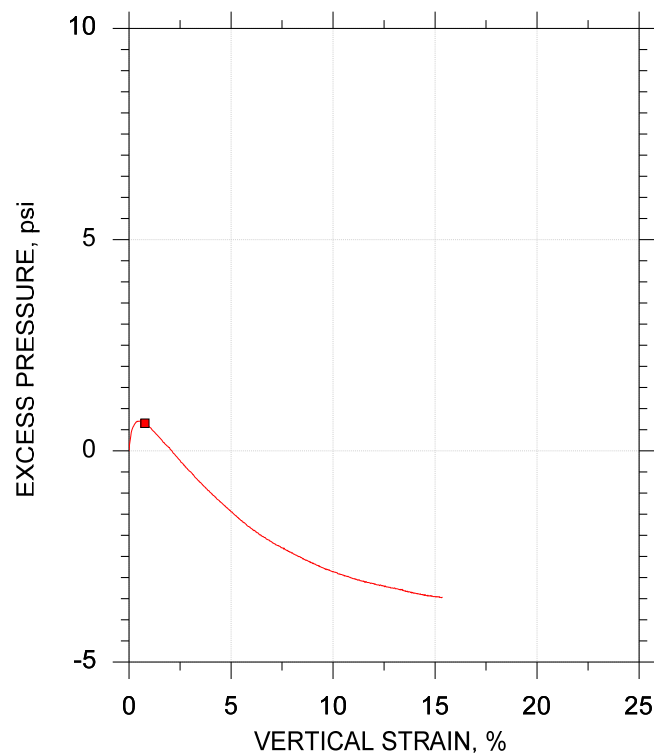
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		5-7 ft		
Test Number		CU-12-1		
Initial	Height, in	4.910		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	31.0		
	Dry Density, pcf	91.0		
	Saturation (Wet Method), %	98.3		
	Void Ratio	0.852		
Before Shear	Moisture Content, %	31.7		
	Dry Density, pcf	90.8		
	Cross-sectional Area (Method A), in ²	3.272		
	Saturation, %	100.0		
	Void Ratio	0.857		
	Back Pressure, psi	122.4		
Vertical Effective Consolidation Stress, psi		1.524		
Horizontal Effective Consolidation Stress, psi		1.523		
Vertical Strain after Consolidation, %		0.01118		
Volumetric Strain after Consolidation, %		0.2342		
Time to 50% Consolidation, min		56.25		
Shear Strength, psi		3.815		
Strain at Failure, %		0.775		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		7.629		
Effective Minor Principal Stress at Failure, psi		0.8686		
Effective Major Principal Stress at Failure, psi		8.498		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System S				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



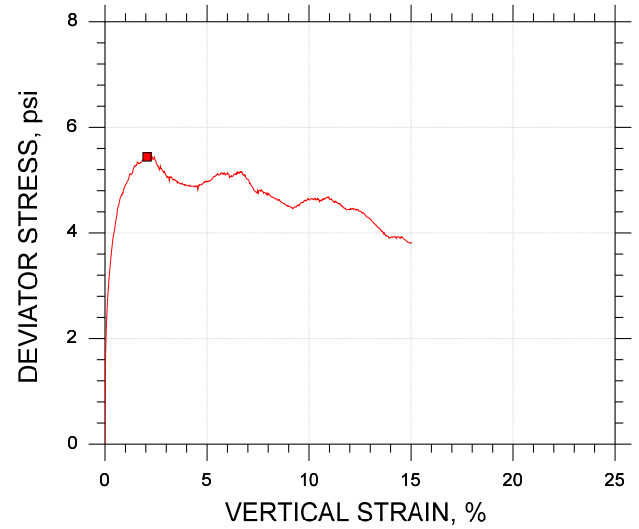
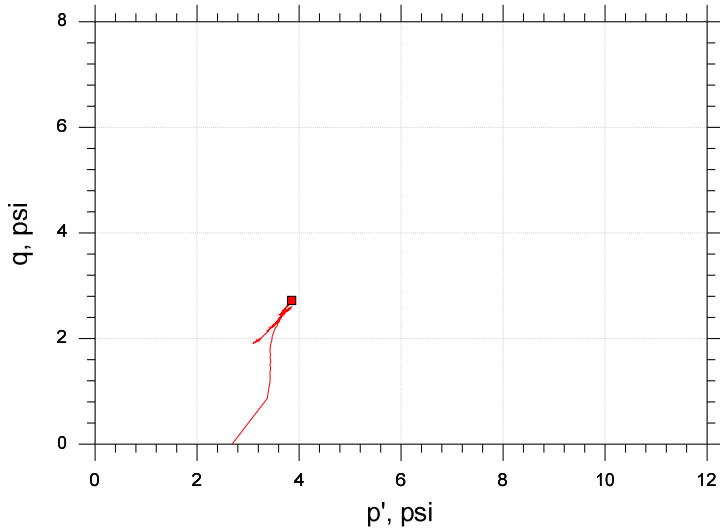
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■	1U	CU-12-1	5-7 ft	trm	7/23/19	mcm	8/2/19	308853-CU-12-1m.dat

			
	Project: Rt 9/ I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System S		



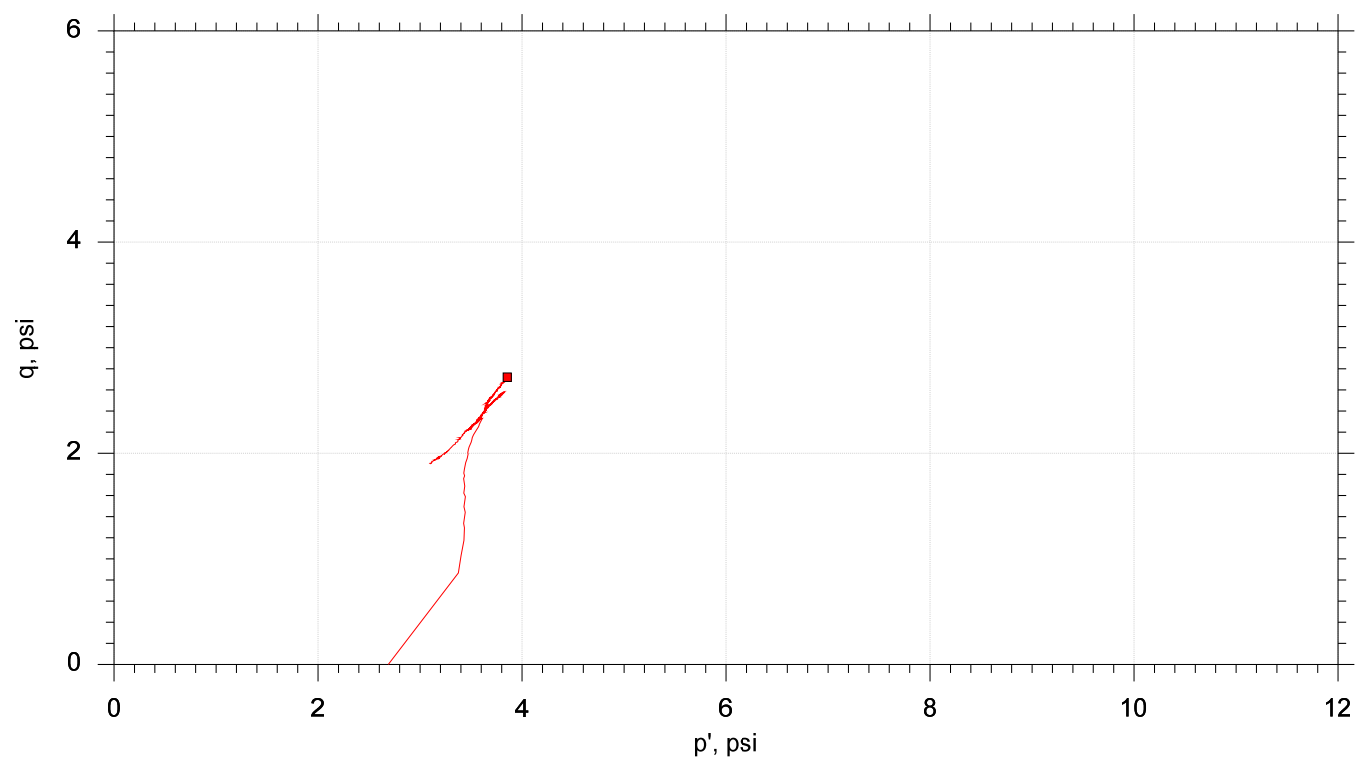
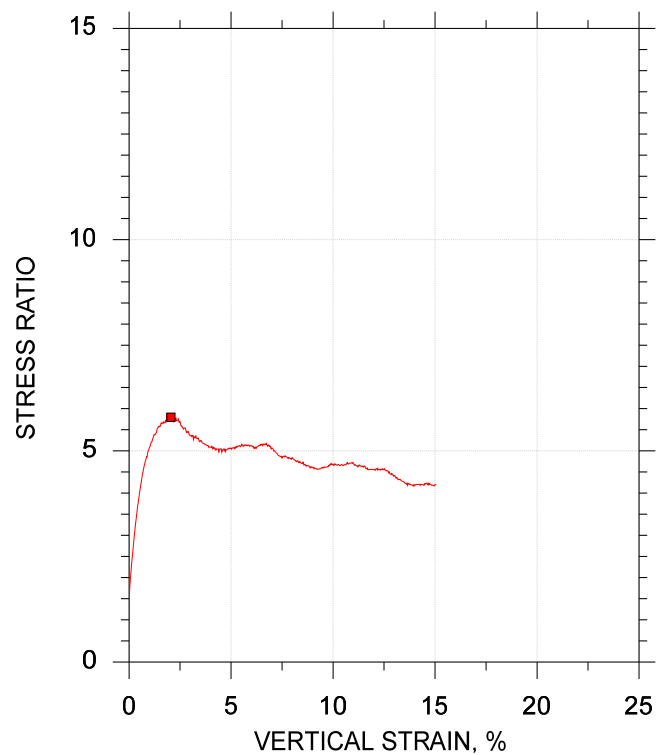
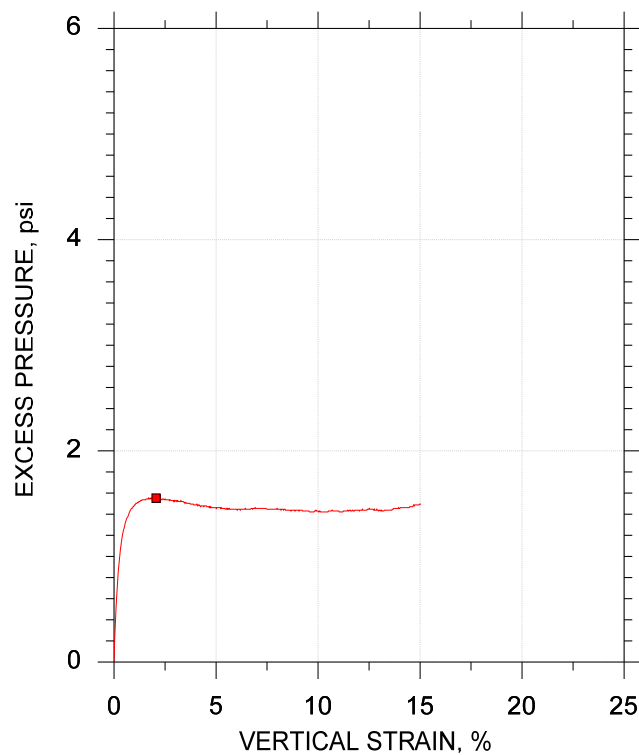
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: BB-BEB-103	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 38	Plastic Limit: 19
Plasticity Index: 19	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-12 ft		
Test Number		CU-15-1		
Initial	Height, in	4.500		
	Diameter, in	1.950		
	Moisture Content (from Cuttings), %	37.9		
	Dry Density, pcf	81.2		
	Saturation (Wet Method), %	95.1		
	Void Ratio	1.08		
Before Shear	Moisture Content, %	35.0		
	Dry Density, pcf	86.7		
	Cross-sectional Area (Method A), in ²	2.842		
	Saturation, %	100.0		
	Void Ratio	0.945		
	Back Pressure, psi	48.99		
Vertical Effective Consolidation Stress, psi		2.689		
Horizontal Effective Consolidation Stress, psi		2.688		
Vertical Strain after Consolidation, %		0.006770		
Volumetric Strain after Consolidation, %		1.699		
Time to 50% Consolidation, min		110.3		
Shear Strength, psi		2.721		
Strain at Failure, %		2.05		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.441		
Effective Minor Principal Stress at Failure, psi		1.134		
Effective Major Principal Stress at Failure, psi		6.576		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System S				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



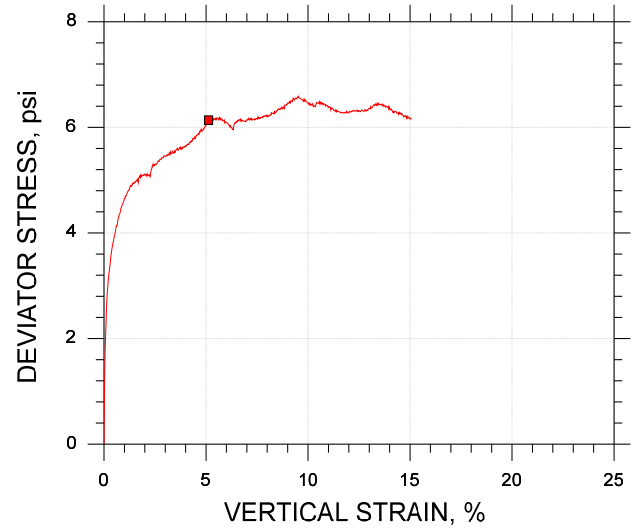
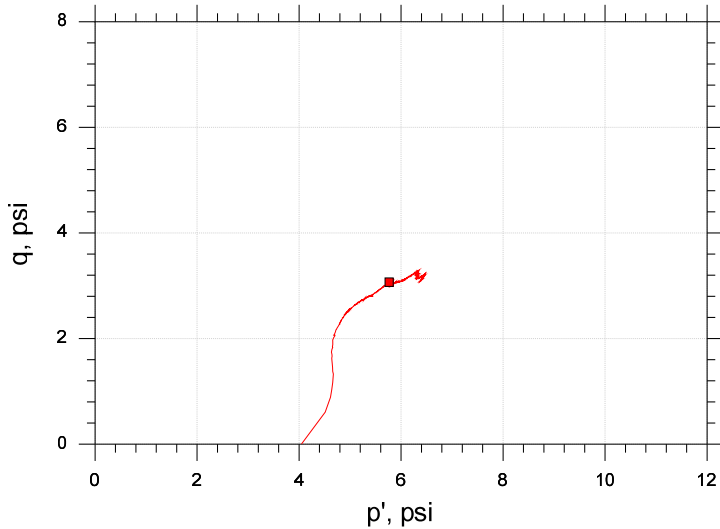
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■	1U	CU-15-1	10-12 ft	trm	7/25/19	mcm	8/2/19	308853-CU-15-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System S		



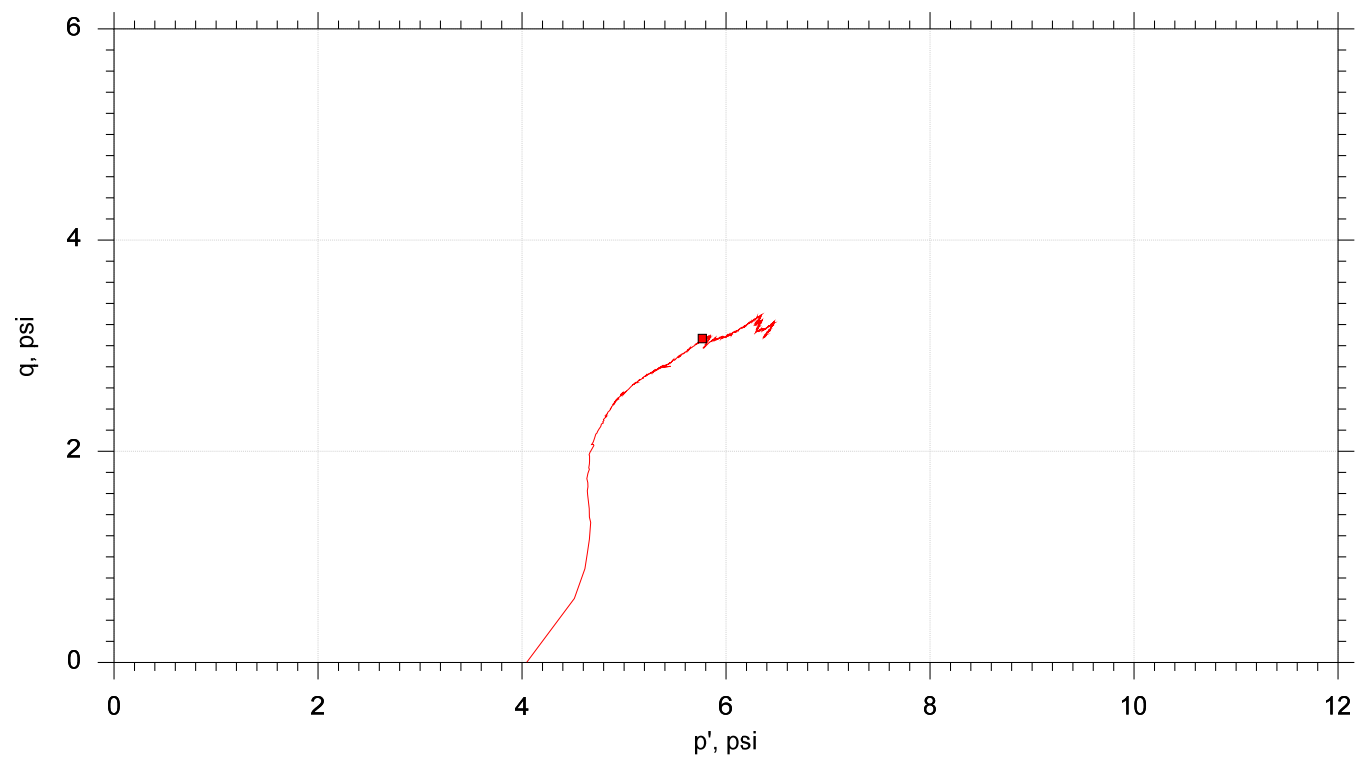
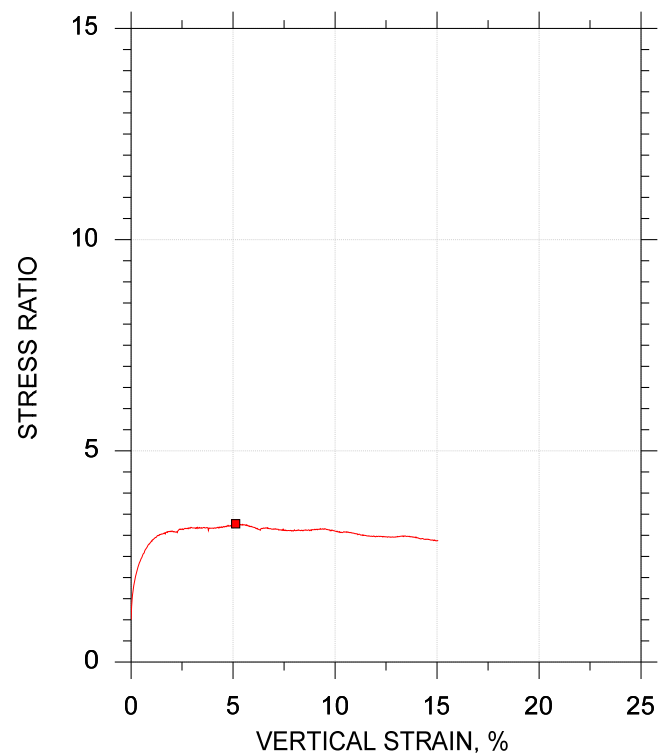
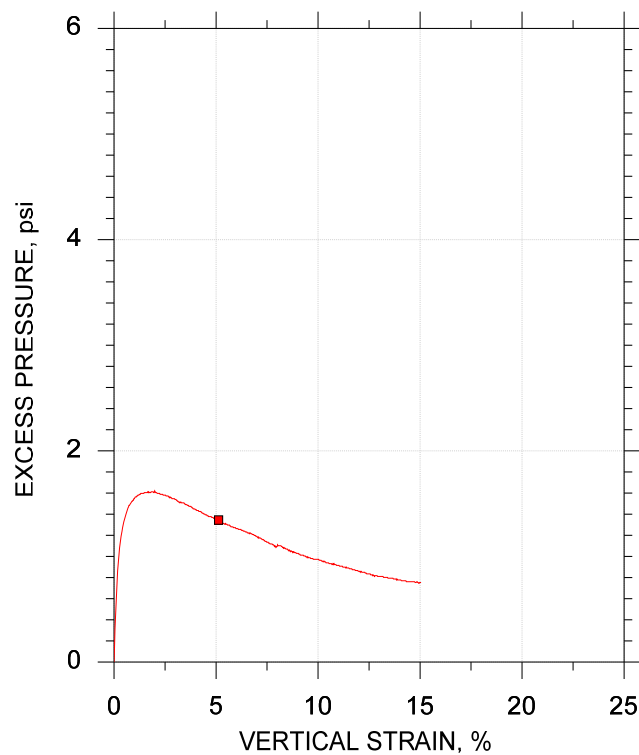
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: BB-BEB-104	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 20
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	3U			
Depth, ft	15-17 ft			
Test Number	CU-13-1			
Initial	Height, in	4.840		
	Diameter, in	1.930		
	Moisture Content (from Cuttings), %	34.7		
	Dry Density, pcf	86.6		
	Saturation (Wet Method), %	99.0		
	Void Ratio	0.945		
Before Shear	Moisture Content, %	33.8		
	Dry Density, pcf	88.1		
	Cross-sectional Area (Method A), in ²	2.874		
	Saturation, %	100.0		
	Void Ratio	0.912		
	Back Pressure, psi	69.99		
Vertical Effective Consolidation Stress, psi		4.052		
Horizontal Effective Consolidation Stress, psi		4.044		
Vertical Strain after Consolidation, %		-0.01880		
Volumetric Strain after Consolidation, %		1.842		
Time to 50% Consolidation, min		72.25		
Shear Strength, psi		3.069		
Strain at Failure, %		5.12		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		6.137		
Effective Minor Principal Stress at Failure, psi		2.698		
Effective Major Principal Stress at Failure, psi		8.835		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System O				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



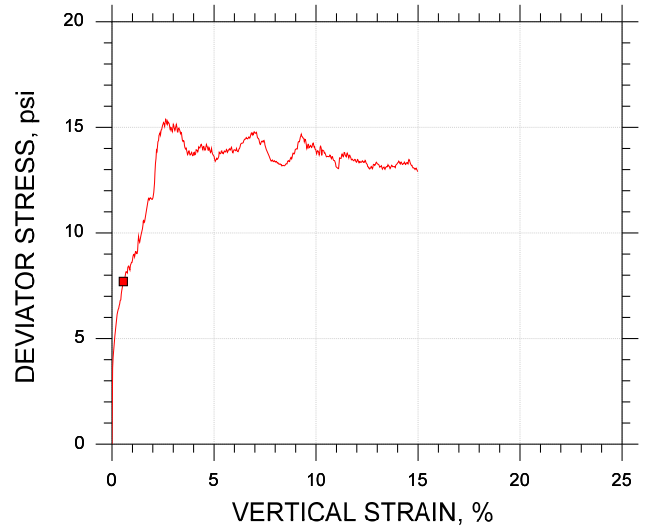
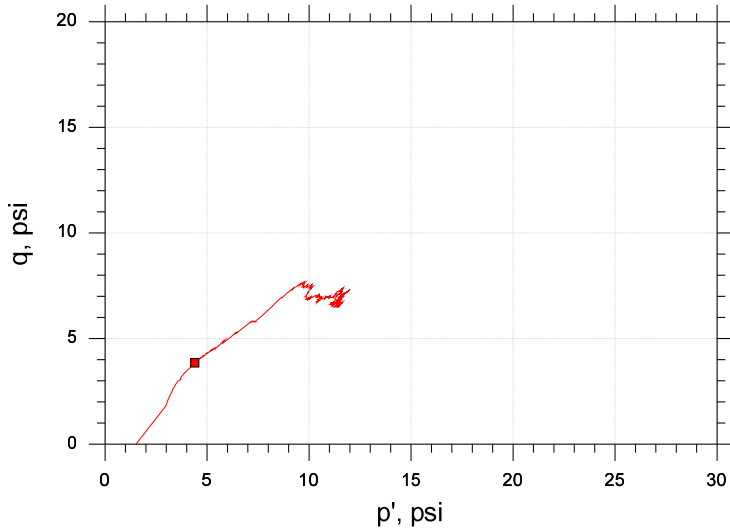
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■	3U	CU-13-1	15-17 ft	trm	7/24/19	mcm	8/2/19	308853-CU-13-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System O		



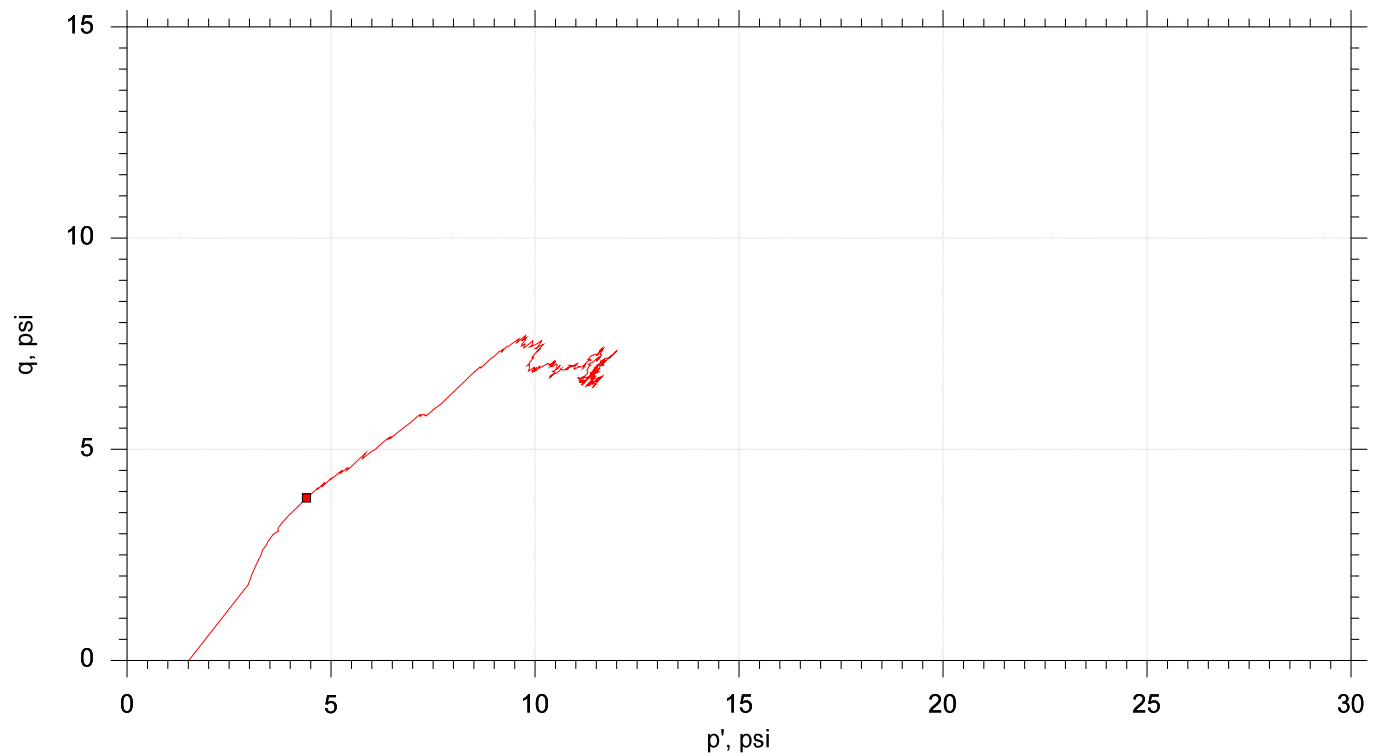
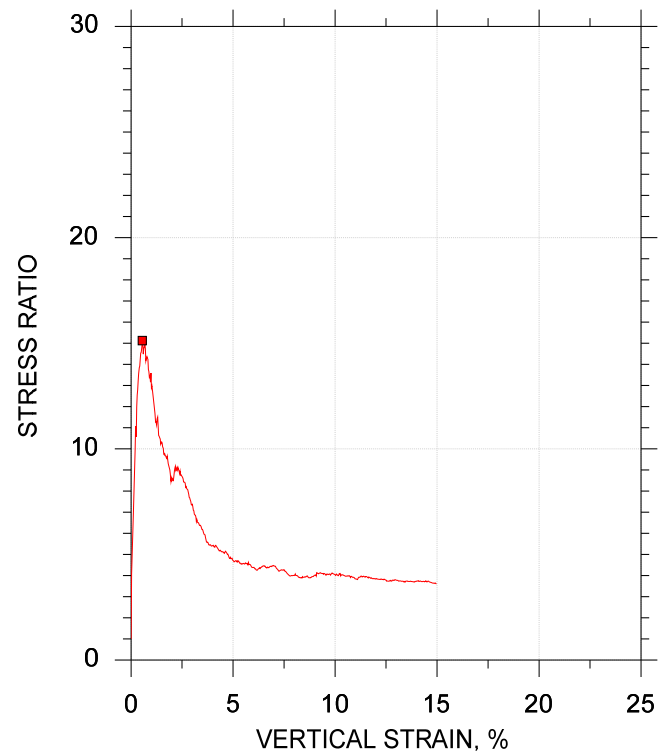
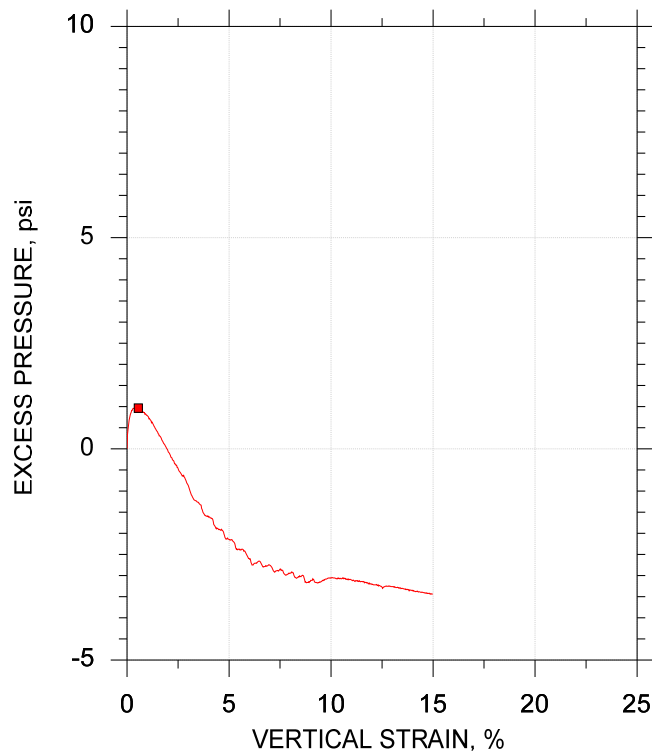
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 2)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313196	
Tested By: trm	Checked By: njh
Boring ID: BB-BEB-202	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 36	Plastic Limit: 18
Plasticity Index: 18	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		5-7		
Test Number		CU-2		
Initial	Height, in	4.510		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	31.8		
	Dry Density, pcf	90.4		
	Saturation (Wet Method), %	99.3		
	Void Ratio	0.865		
Before Shear	Moisture Content, %	31.5		
	Dry Density, pcf	91.1		
	Cross-sectional Area (Method A), in ²	3.253		
	Saturation, %	100.0		
	Void Ratio	0.851		
	Back Pressure, psi	153.0		
Vertical Effective Consolidation Stress, psi		1.514		
Horizontal Effective Consolidation Stress, psi		1.512		
Vertical Strain after Consolidation, %		0.02496		
Volumetric Strain after Consolidation, %		0.05722		
Time to 50% Consolidation, min		64.00		
Shear Strength, psi		3.850		
Strain at Failure, %		0.549		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		7.701		
Effective Minor Principal Stress at Failure, psi		0.5450		
Effective Major Principal Stress at Failure, psi		8.246		
B-Value		0.94		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



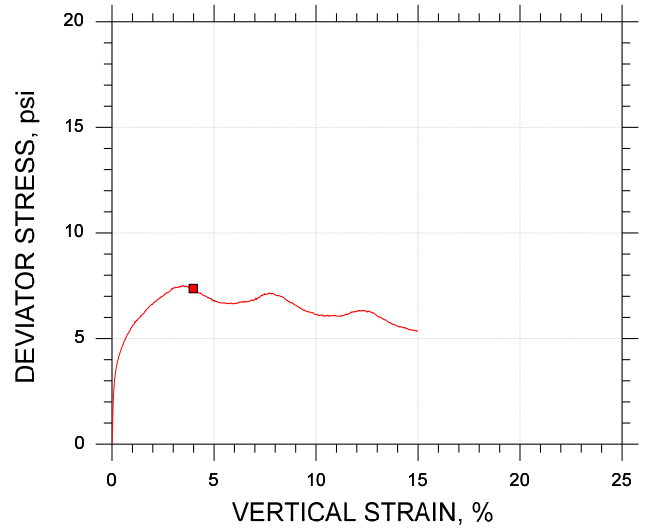
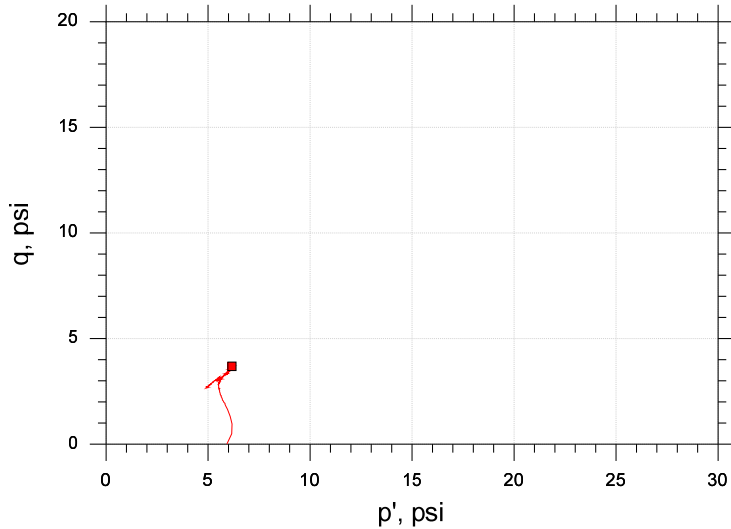
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■	U1	CU-2	5-7	trm	4/1/21	njh	4/9/21	313196-CU-2n.dat

			
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System F		



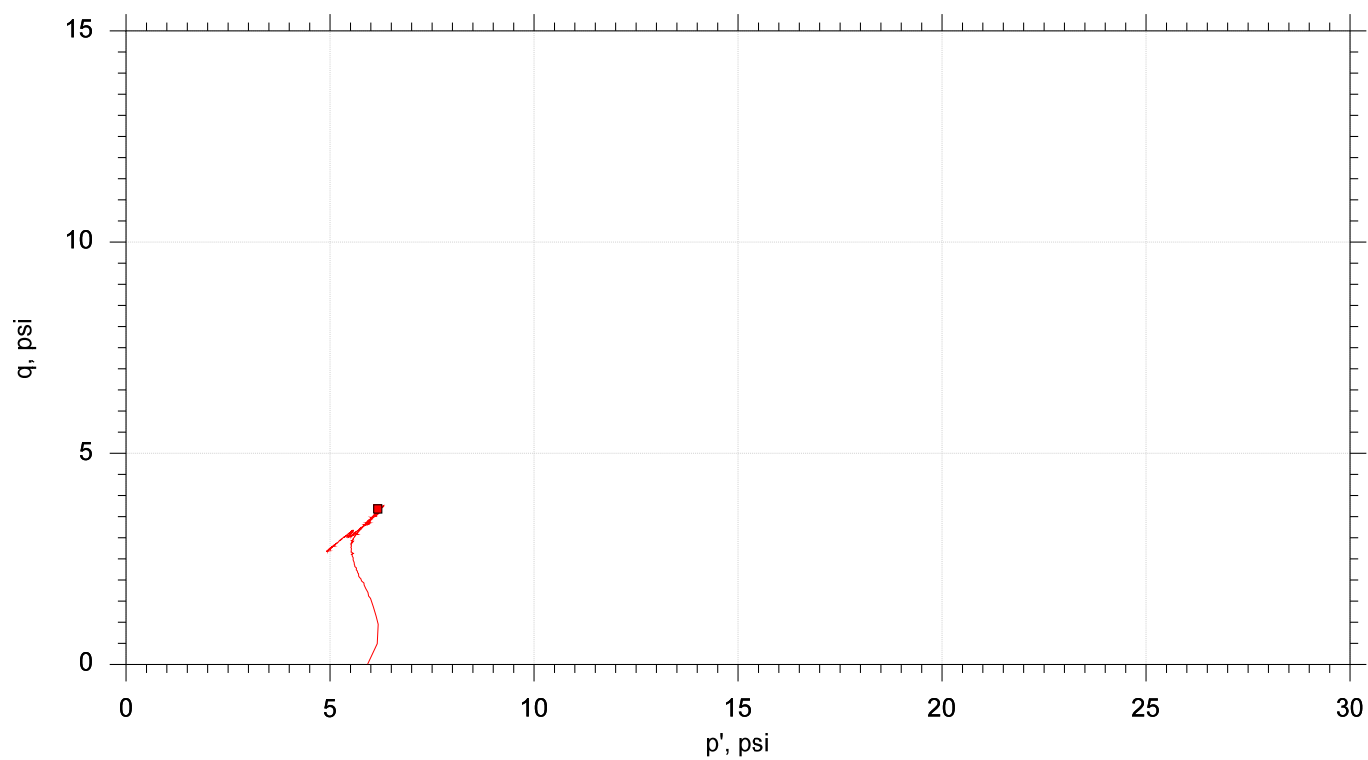
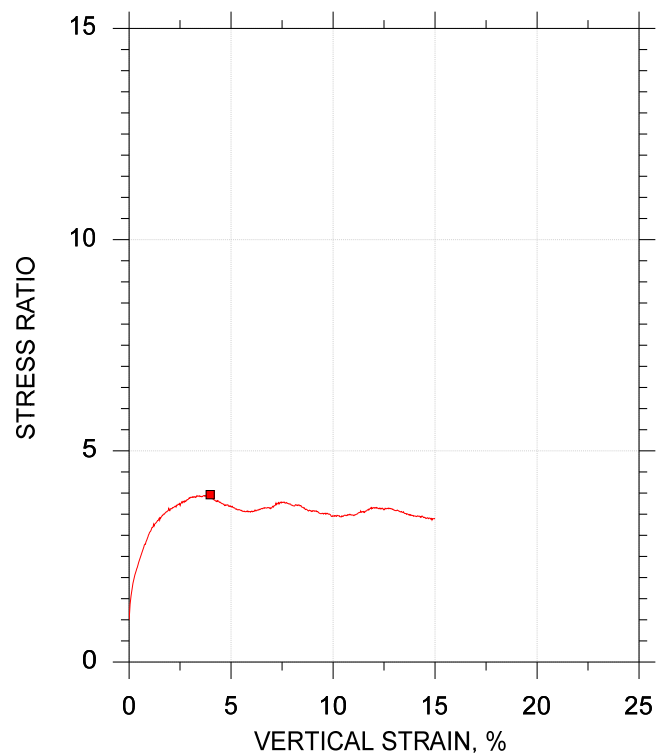
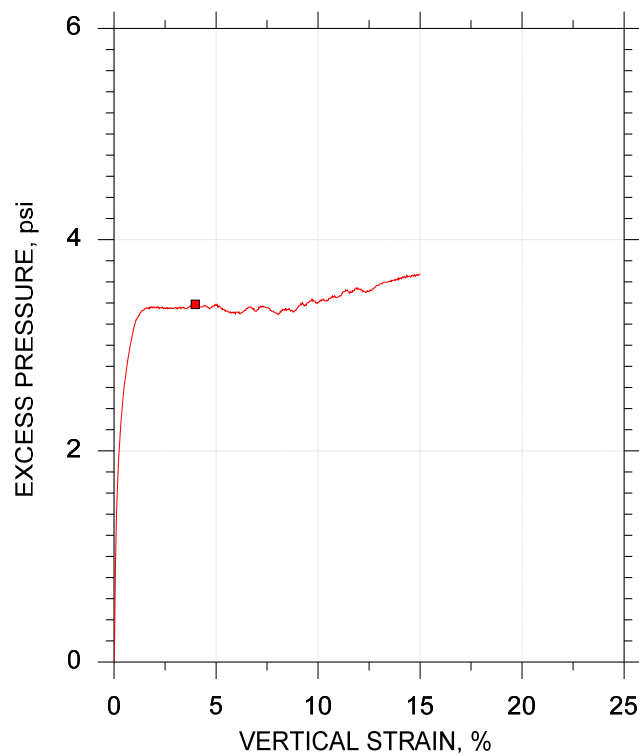
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 2)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313196	
Tested By: trm	Checked By: njh
Boring ID: BB-BEB-202	
Preparation: intact	
Description: Wet, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 32	Plastic Limit: 17
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U2		
Depth, ft		15-17		
Test Number		CU-3-1		
Initial	Height, in	4.510		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	38.2		
	Dry Density, pcf	79.7		
	Saturation (Wet Method), %	92.5		
	Void Ratio	1.11		
Before Shear	Moisture Content, %	40.3		
	Dry Density, pcf	80.8		
	Cross-sectional Area (Method A), in ²	3.208		
	Saturation, %	100.0		
	Void Ratio	1.09		
	Back Pressure, psi	150.9		
Vertical Effective Consolidation Stress, psi		5.902		
Horizontal Effective Consolidation Stress, psi		5.914		
Vertical Strain after Consolidation, %		0.3311		
Volumetric Strain after Consolidation, %		1.042		
Time to 50% Consolidation, min		64.00		
Shear Strength, psi		3.684		
Strain at Failure, %		3.98		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		7.367		
Effective Minor Principal Stress at Failure, psi		2.486		
Effective Major Principal Stress at Failure, psi		9.853		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



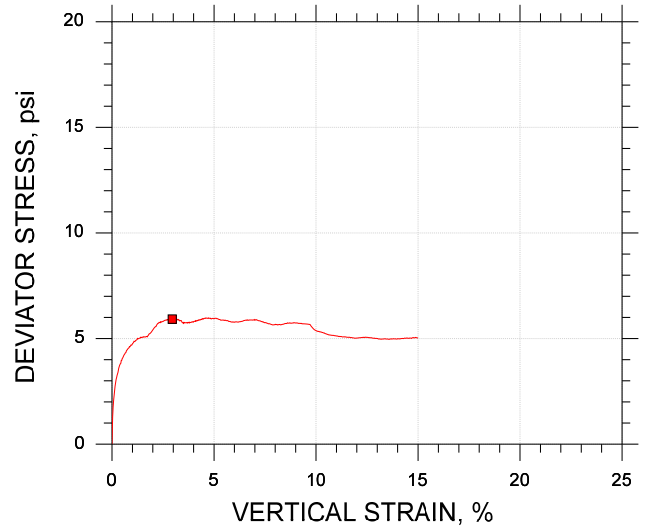
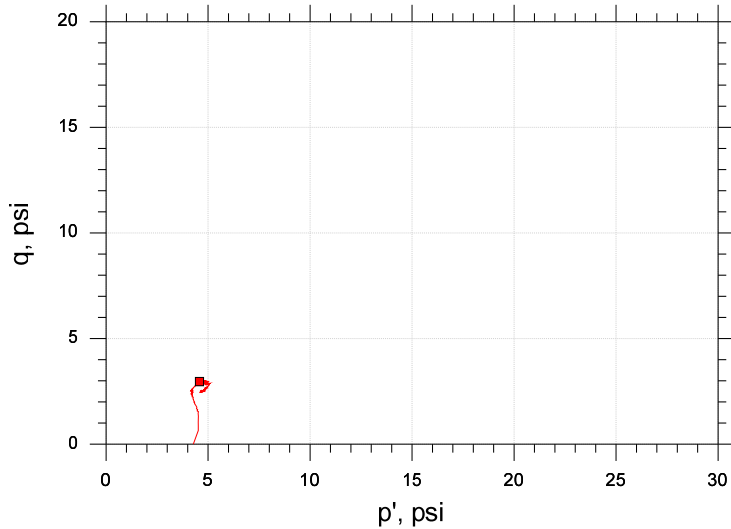
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■	U2	CU-3-1	15-17	trm	4/26/21	njh	5/5/21	313196-CU-3n.dat

			
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Sample Type: intact	
	Description: Wet, gray clay		
	Remarks: System JJ		



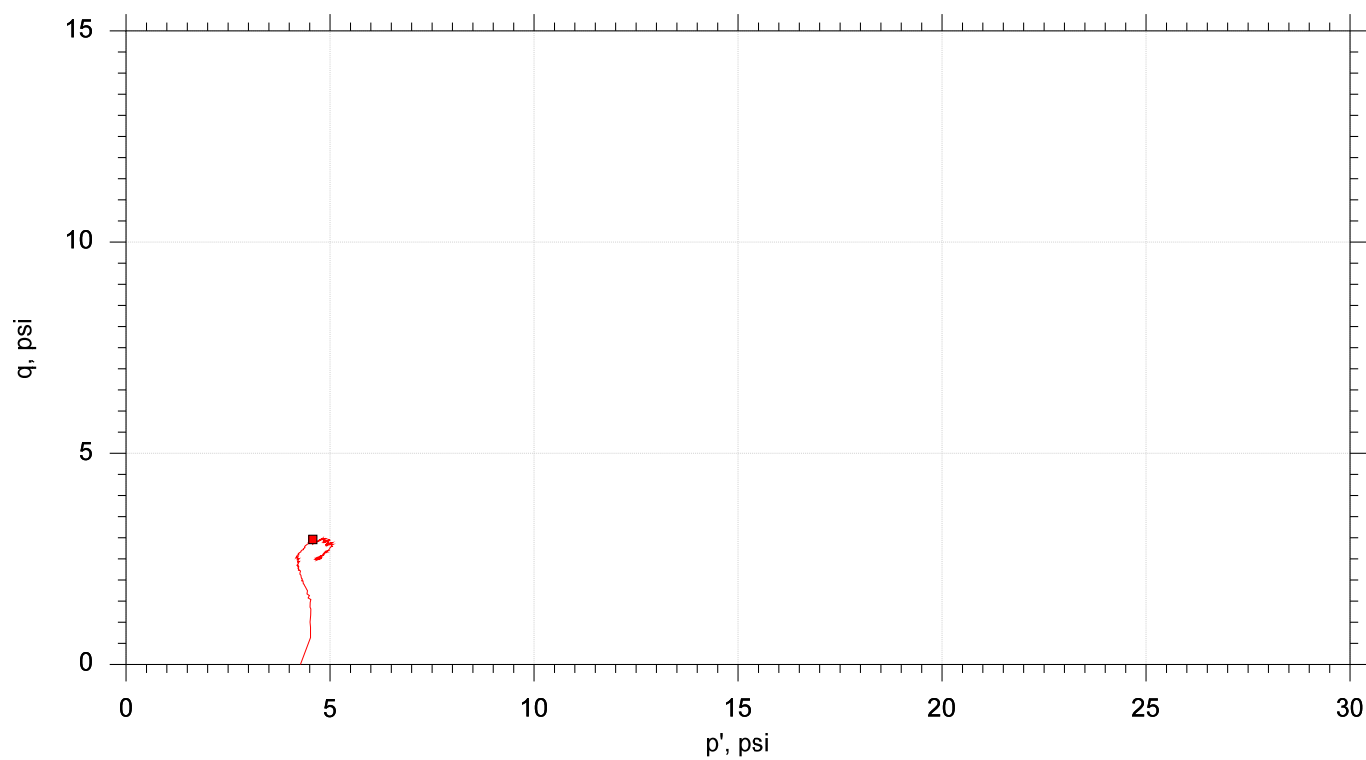
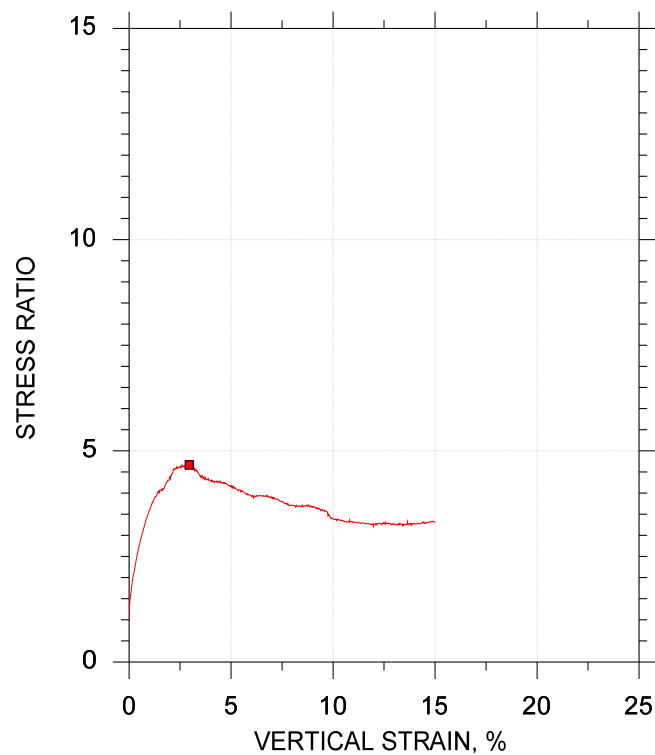
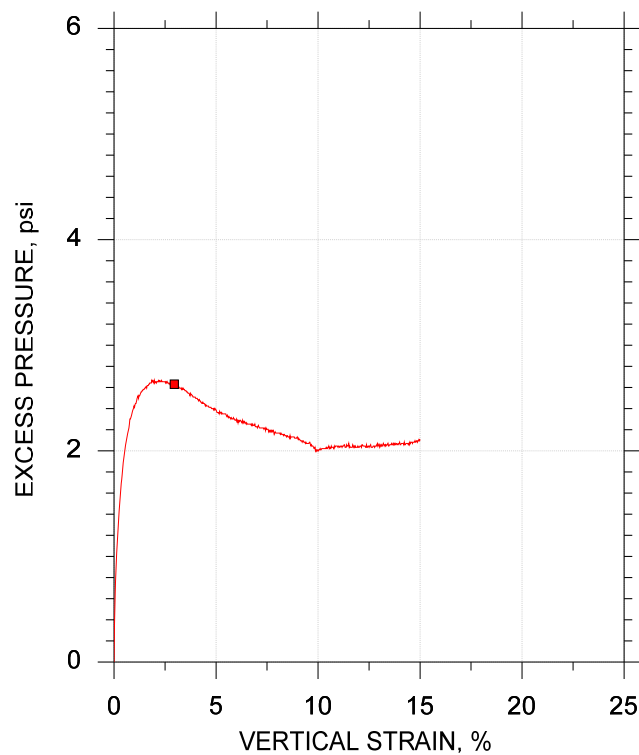
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 2)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313196	
Tested By: trm	Checked By: njh
Boring ID: BB-BEB-205	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 18
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		10-12		
Test Number		CU-1		
Initial	Height, in	4.550		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	37.0		
	Dry Density, pcf	80.2		
	Saturation (Wet Method), %	90.7		
	Void Ratio	1.10		
Before Shear	Moisture Content, %	40.6		
	Dry Density, pcf	80.4		
	Cross-sectional Area (Method A), in ²	3.254		
	Saturation, %	100.0		
	Void Ratio	1.10		
	Back Pressure, psi	147.0		
Vertical Effective Consolidation Stress, psi		4.275		
Horizontal Effective Consolidation Stress, psi		4.271		
Vertical Strain after Consolidation, %		0.0000		
Volumetric Strain after Consolidation, %		0.7371		
Time to 50% Consolidation, min		51.84		
Shear Strength, psi		2.961		
Strain at Failure, %		2.95		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.922		
Effective Minor Principal Stress at Failure, psi		1.616		
Effective Major Principal Stress at Failure, psi		7.538		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System E				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U1	CU-1	10-12	trm	3/16/21	njh	4/9/21	313196-CU-1n.dat

			
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System E		



Client: Haley & Aldrich Inc.

Project Name: Rt 9/ I-395 Connector

Project Location: Brewer and Eddington, ME

Project Number: GTX-308853

Tested By: md/trm

Checked By: mcm

Boring ID: ~~HB-BFB-101~~ BB-BFB-101

Preparation: intact

Description: Moist, very dark gray clay

Classification: ---

Group Symbol: ---

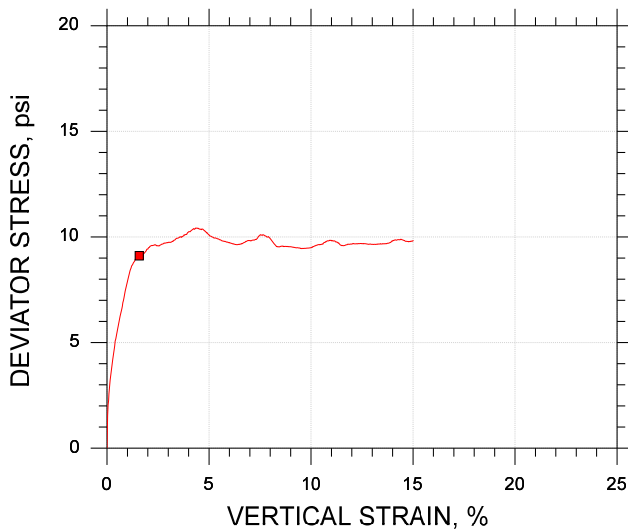
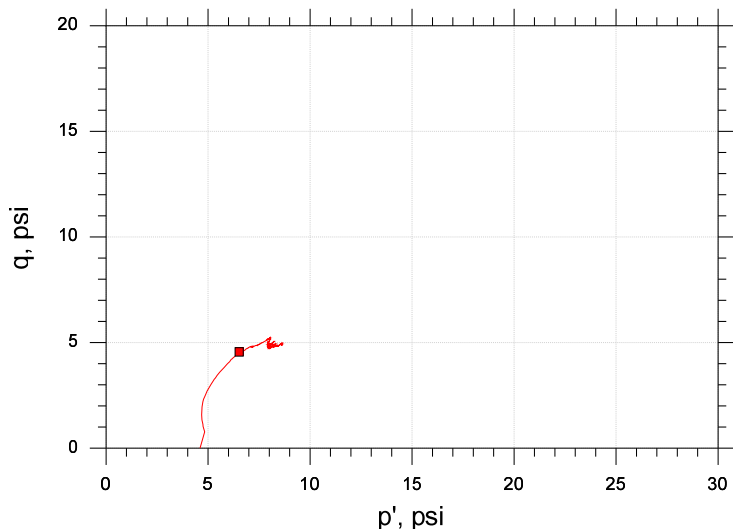
Liquid Limit: 41

Plastic Limit: 22

Plasticity Index: 19

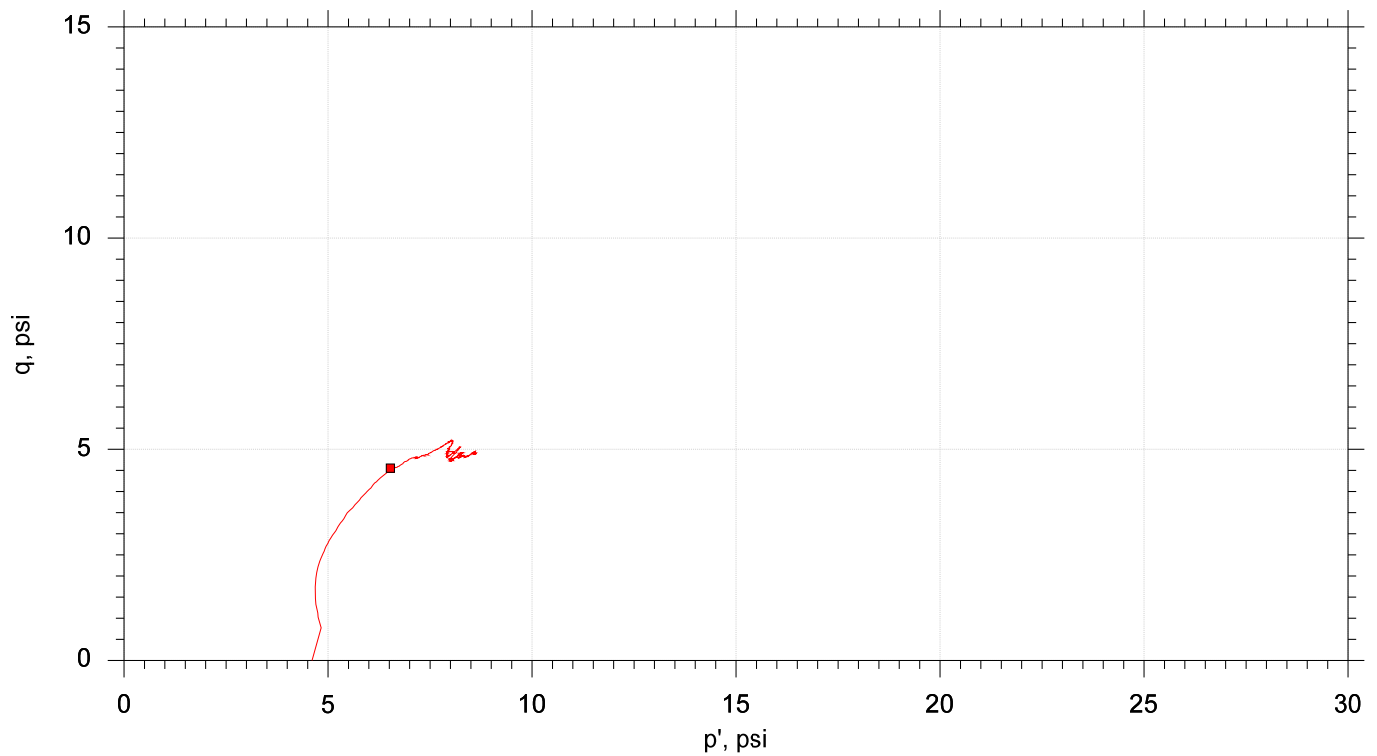
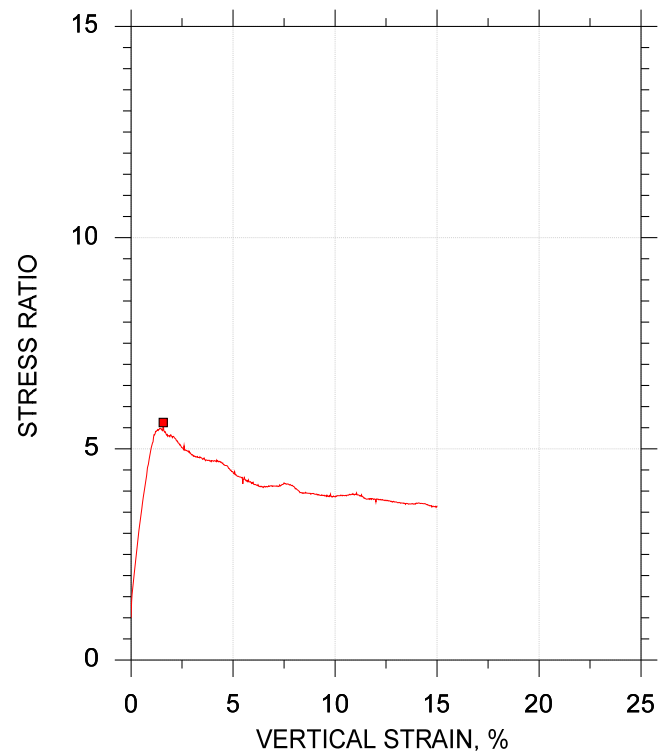
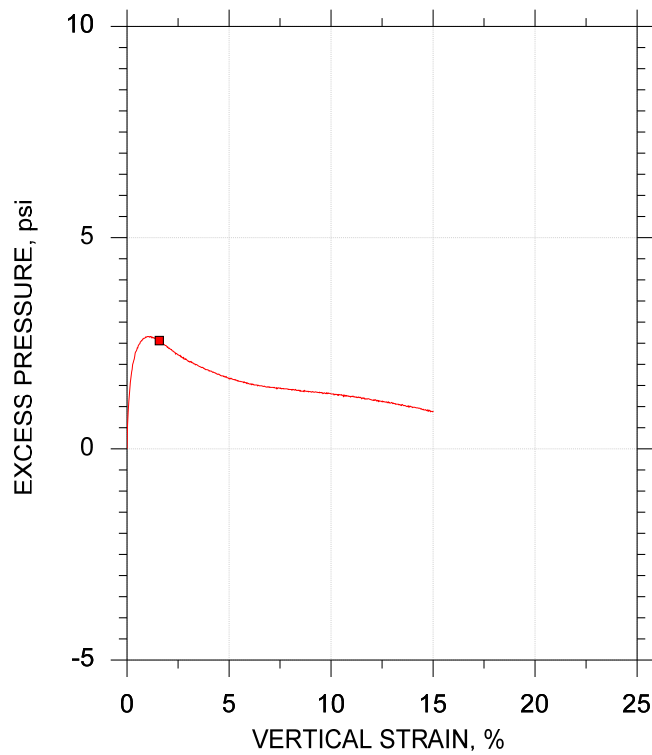
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		5-7 ft		
Test Number		CU-1-1		
Initial	Height, in	6.110		
	Diameter, in	2.850		
	Moisture Content (from Cuttings), %	36.0		
	Dry Density, pcf	85.4		
	Saturation (Wet Method), %	99.8		
	Void Ratio	0.974		
Before Shear	Moisture Content, %	36.1		
	Dry Density, pcf	85.4		
	Cross-sectional Area (Method A), in ²	6.374		
	Saturation, %	100.0		
	Void Ratio	0.974		
	Back Pressure, psi	147.0		
Vertical Effective Consolidation Stress, psi		4.606		
Horizontal Effective Consolidation Stress, psi		4.607		
Vertical Strain after Consolidation, %		0.04999		
Volumetric Strain after Consolidation, %		0.4042		
Time to 50% Consolidation, min		4.000		
Shear Strength, psi		4.555		
Strain at Failure, %		1.58		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		9.110		
Effective Minor Principal Stress at Failure, psi		1.970		
Effective Major Principal Stress at Failure, psi		11.08		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



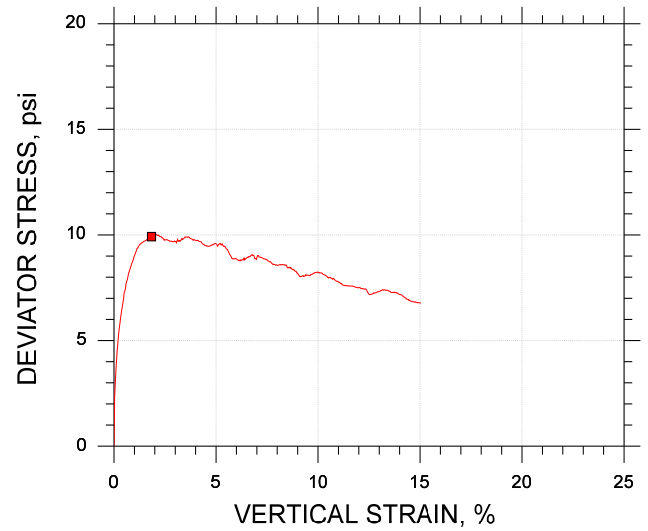
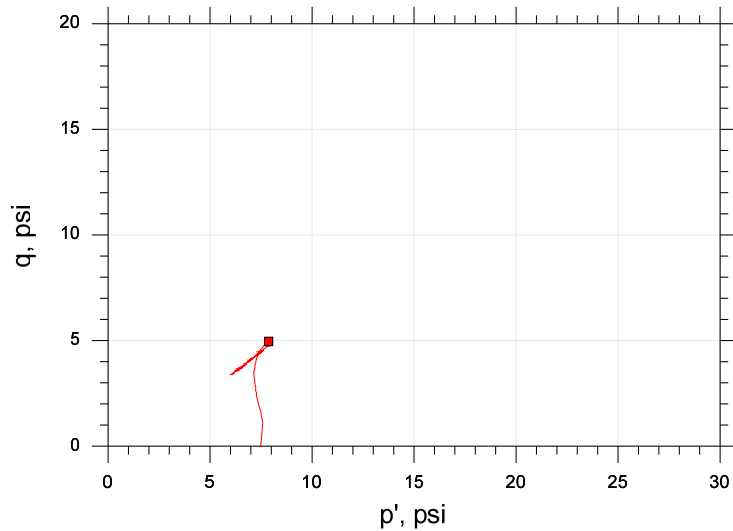
	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	1U	CU-1-1	5-7 ft	md/trm	9/27/18	mcm	10/17/18	308853-CU-1-1m.dat

			
	Project: Rt 9/ I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Sample Type: intact	
	Description: Moist, very dark gray clay		
	Remarks: System JJ		



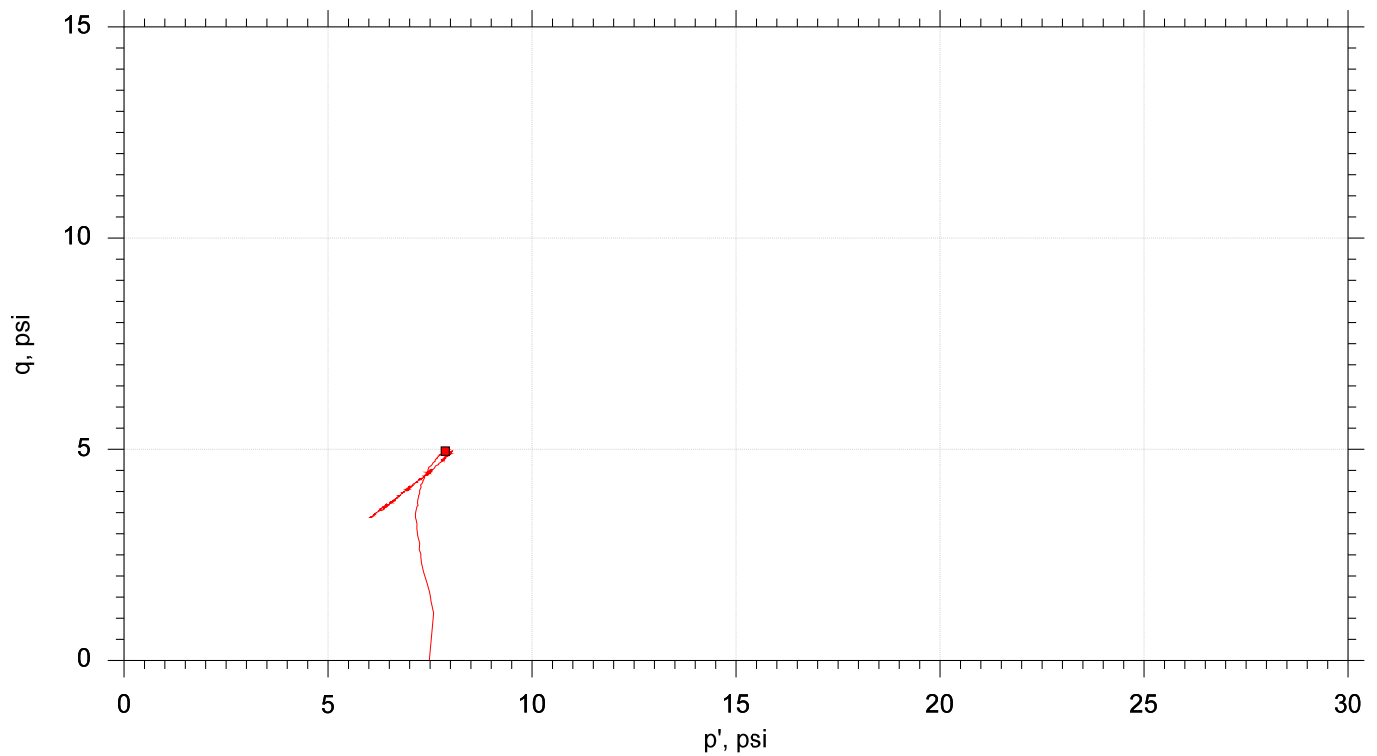
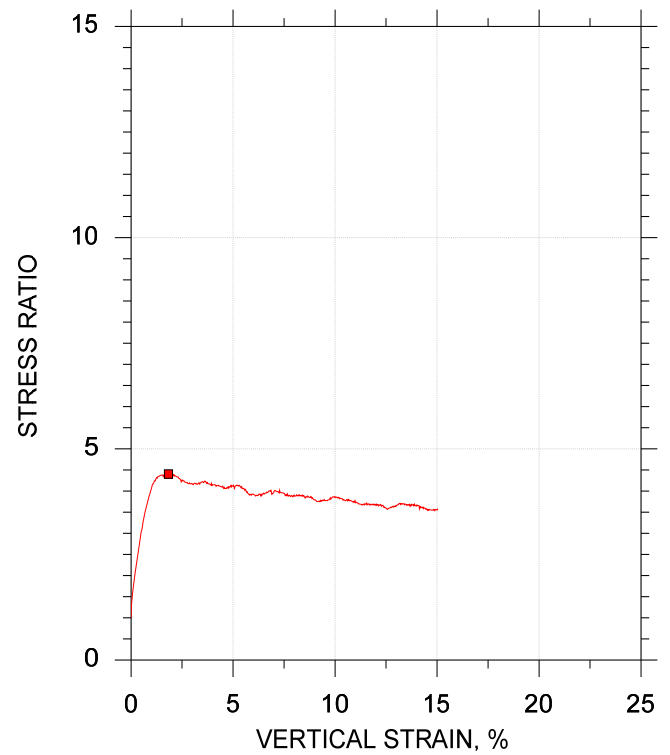
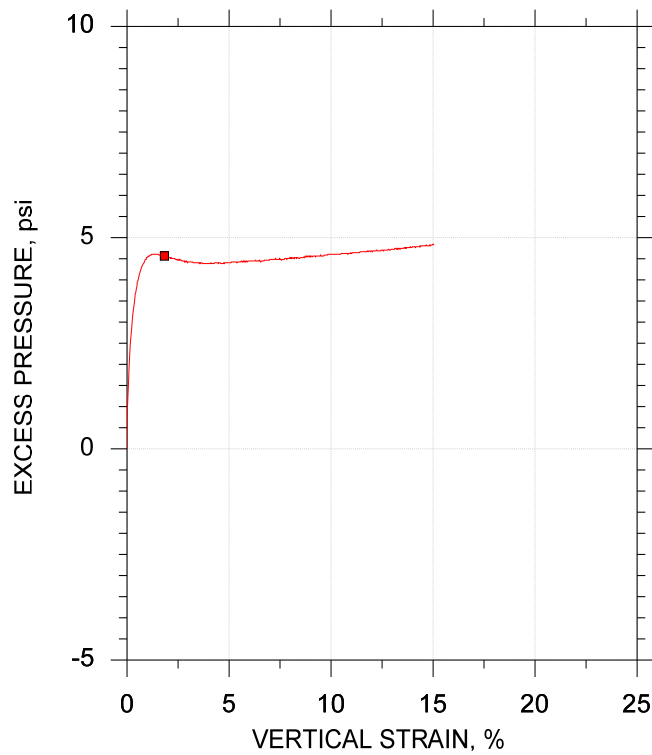
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/ I-395	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: md/trm	Checked By: mcm
Boring ID: HB-BFB-101 BB-BFB-101	
Preparation: Intact	
Description: Wet, very dark greenish gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 20
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		2U		
Depth, ft		12-14 ft		
Test Number		CU-2-1		
Initial	Height, in	6.060		
	Diameter, in	2.870		
	Moisture Content (from Cuttings), %	38.7		
	Dry Density, pcf	82.3		
	Saturation (Wet Method), %	99.8		
	Void Ratio	1.05		
Before Shear	Moisture Content, %	37.6		
	Dry Density, pcf	83.7		
	Cross-sectional Area (Method A), in ²	6.393		
	Saturation, %	100.0		
	Void Ratio	1.01		
	Back Pressure, psi	105.0		
Vertical Effective Consolidation Stress, psi		7.462		
Horizontal Effective Consolidation Stress, psi		7.479		
Vertical Strain after Consolidation, %		0.4240		
Volumetric Strain after Consolidation, %		1.534		
Time to 50% Consolidation, min		90.25		
Shear Strength, psi		4.959		
Strain at Failure, %		1.83		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		9.917		
Effective Minor Principal Stress at Failure, psi		2.912		
Effective Major Principal Stress at Failure, psi		12.83		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



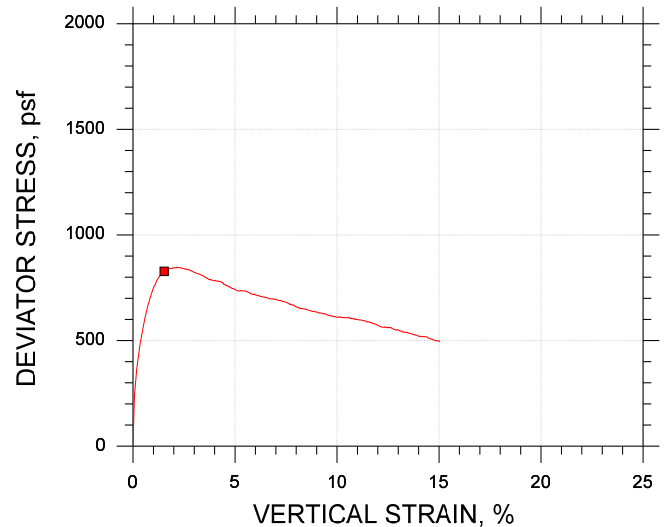
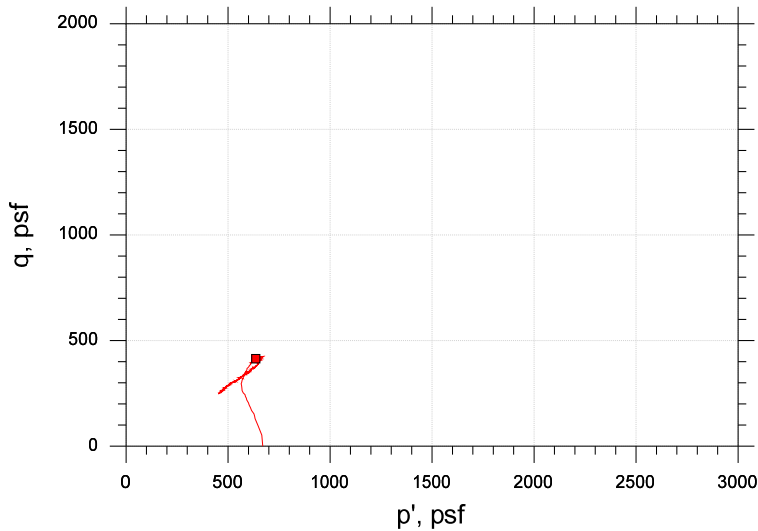
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■	2U	CU-2-1	12-14 ft	md/trm	9/27/18	mcm	10/17/18	308853-CU-2-1m.dat

			
	Project: Rt 9/ I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Sample Type: Intact	
	Description: Wet, very dark greenish gray clay		
	Remarks: System X		



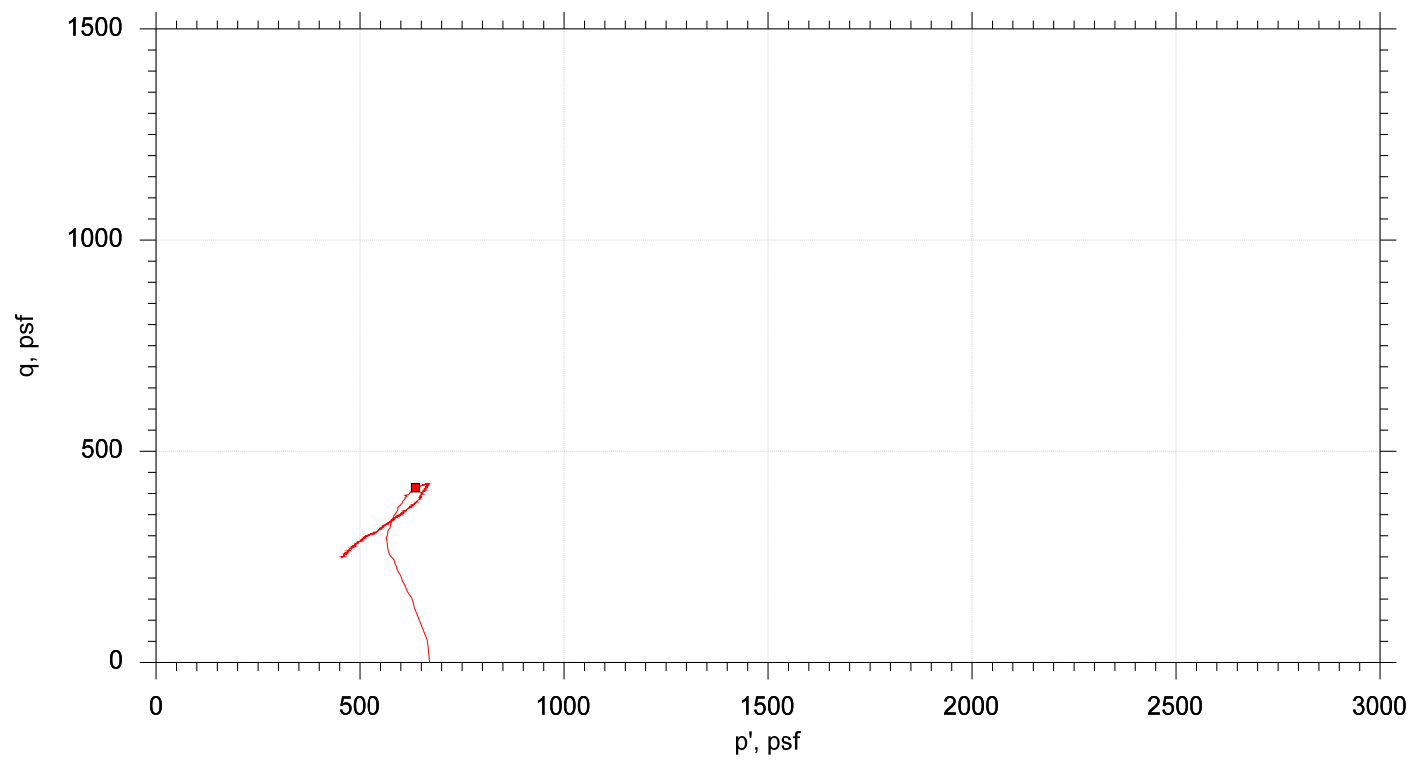
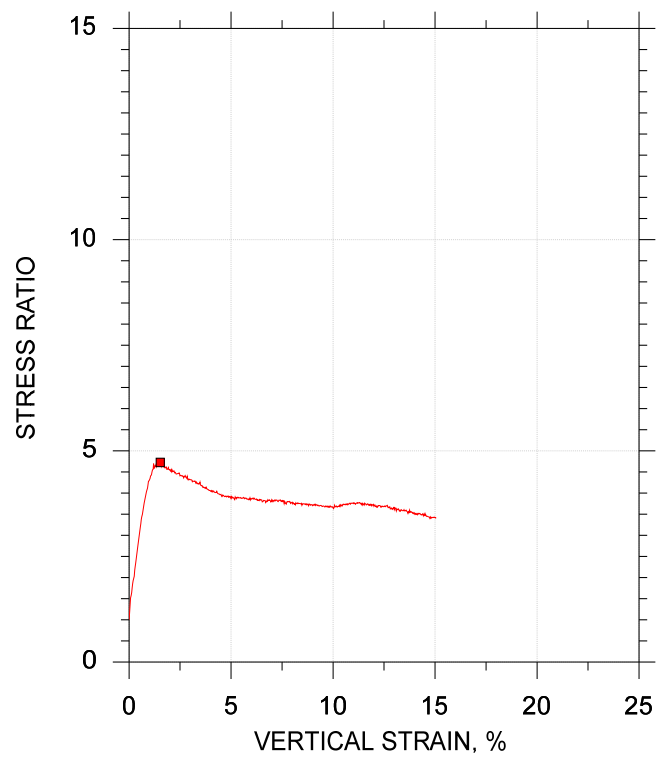
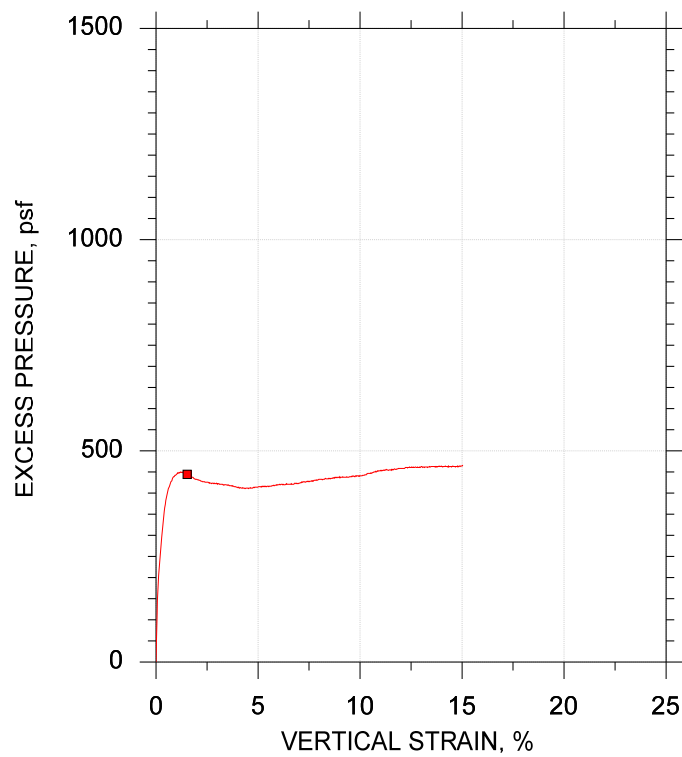
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: md	Checked By: mcm
Boring ID: BB-BFB-202	
Preparation: intact	
Description: Wet, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 38	Plastic Limit: 19
Plasticity Index: 19	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		18-20 ft		
Test Number		CU-17-1		
Initial	Height, in	4.600		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	42.9		
	Dry Density, pcf	74.8		
	Saturation (Wet Method), %	92.4		
	Void Ratio	1.25		
Before Shear	Moisture Content, %	45.3		
	Dry Density, pcf	75.8		
	Cross-sectional Area (Method A), in ²	3.212		
	Saturation, %	100.0		
	Void Ratio	1.22		
	Back Pressure, psf	2.173e+004		
Vertical Effective Consolidation Stress, psf		667.7		
Horizontal Effective Consolidation Stress, psf		670.1		
Vertical Strain after Consolidation, %		0.2371		
Volumetric Strain after Consolidation, %		0.1463		
Time to 50% Consolidation, min		17.64		
Shear Strength, psf		413.8		
Strain at Failure, %		1.53		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		827.7		
Effective Minor Principal Stress at Failure, psf		222.1		
Effective Major Principal Stress at Failure, psf		1050.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System Y				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U1	CU-17-1	18-20 ft	md	03/09/21	mcm	4/1/21	312665-CU-17-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brwre-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Sample Type: intact	
	Description: Wet, gray clay		
	Remarks: System Y		



Client: Haley & Aldrich, Inc.

Project Name: Rt 9/I-395 Connector

Project Location: Brewer and Eddington, ME

Project Number: GTX-308853

Tested By: md

Checked By: mcm

Boring ID: ~~HB-BFB1-101~~ BB-BFB1-101

Preparation: Intact

Description: Moist, dark gray clay

Classification: ---

Group Symbol: ---

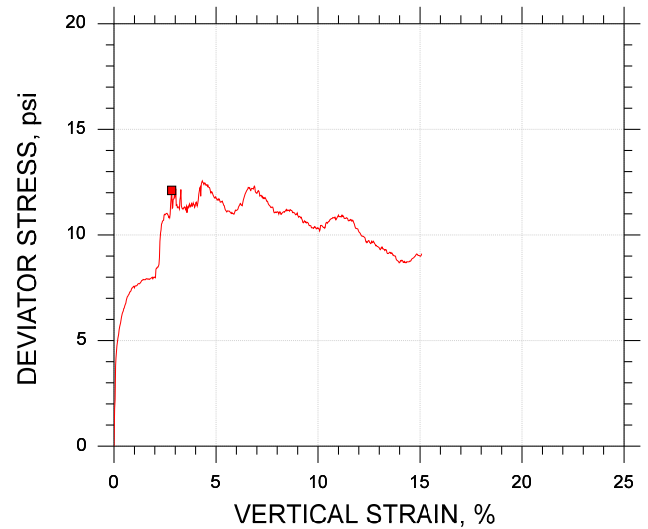
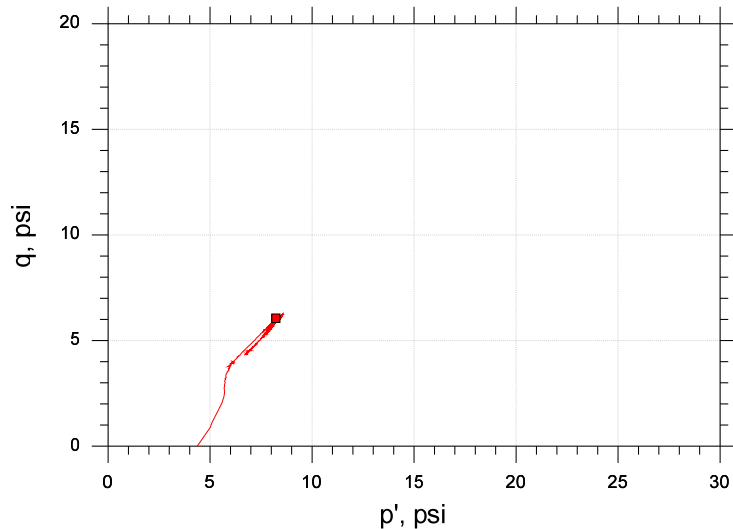
Liquid Limit: 38

Plastic Limit: 20

Plasticity Index: 18

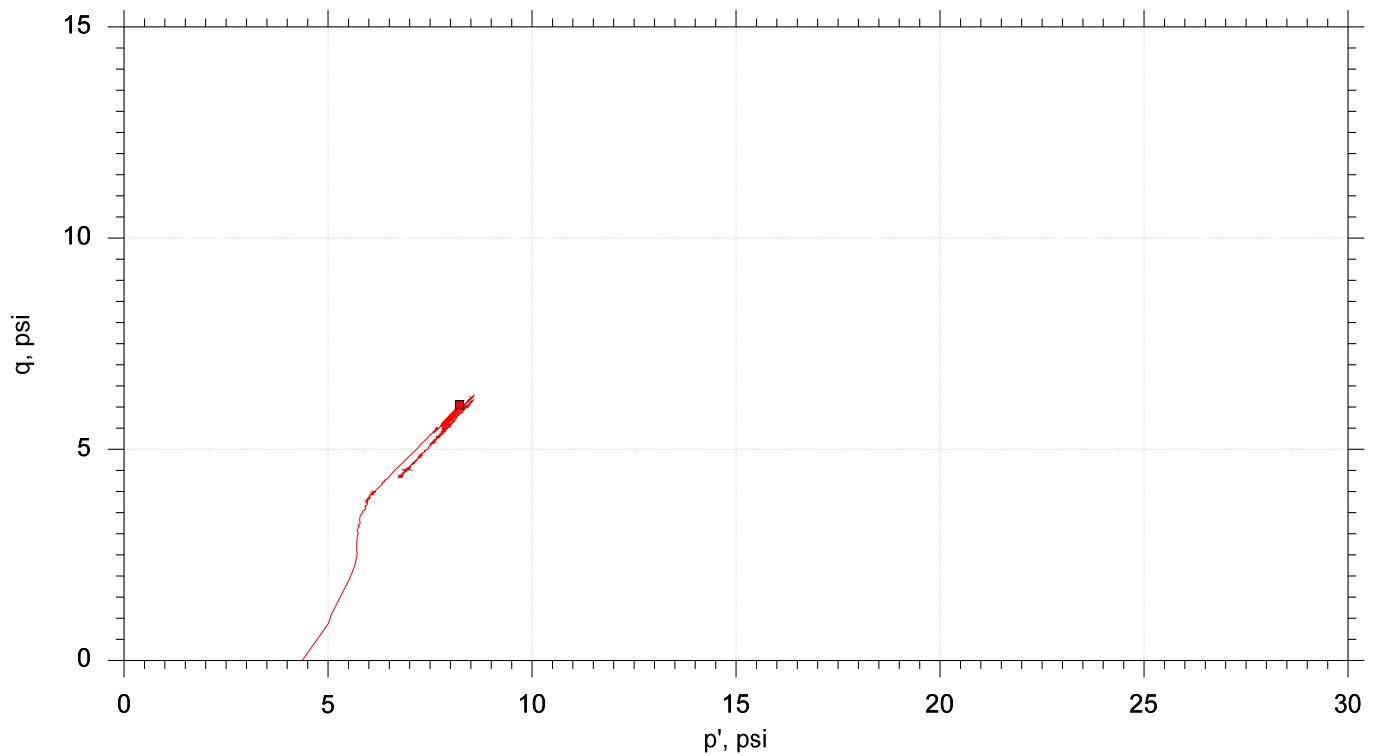
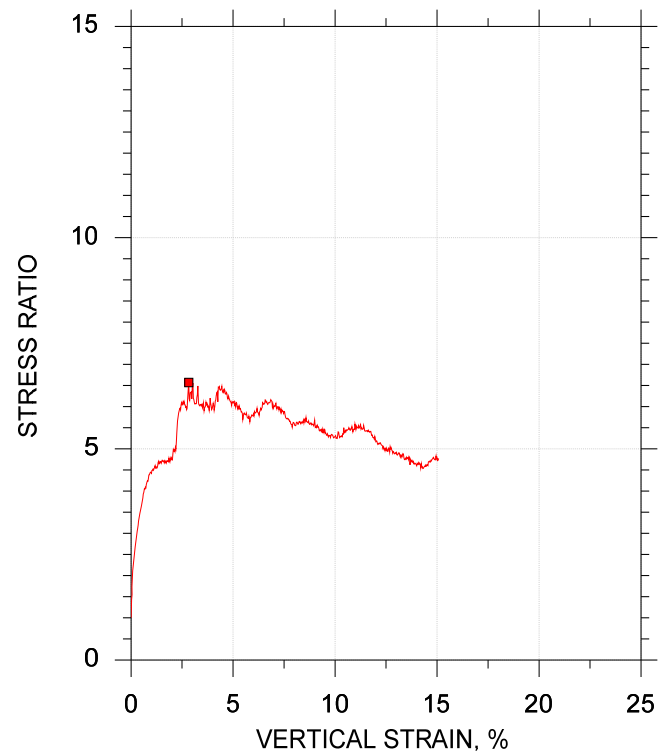
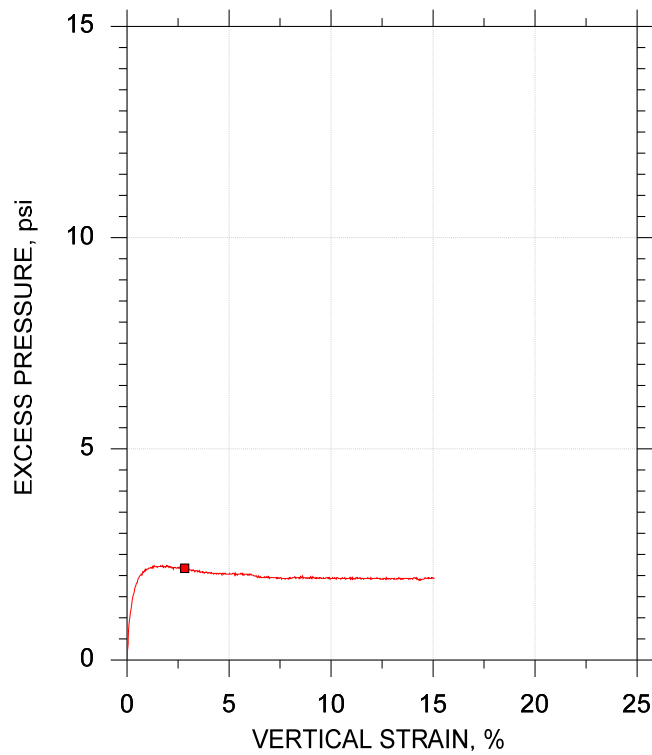
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		15-16.4 ft		
Test Number		CU-17-1		
Initial	Height, in	4.450		
	Diameter, in	1.950		
	Moisture Content (from Cuttings), %	38.0		
	Dry Density, pcf	82.5		
	Saturation (Wet Method), %	98.2		
	Void Ratio	1.04		
Before Shear	Moisture Content, %	38.5		
	Dry Density, pcf	82.7		
	Cross-sectional Area (Method A), in ²	2.977		
	Saturation, %	100.0		
	Void Ratio	1.04		
	Back Pressure, psi	160.6		
Vertical Effective Consolidation Stress, psi		4.370		
Horizontal Effective Consolidation Stress, psi		4.368		
Vertical Strain after Consolidation, %		0.004397		
Volumetric Strain after Consolidation, %		0.3734		
Time to 50% Consolidation, min		46.24		
Shear Strength, psi		6.054		
Strain at Failure, %		2.83		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		12.11		
Effective Minor Principal Stress at Failure, psi		2.173		
Effective Major Principal Stress at Failure, psi		14.28		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



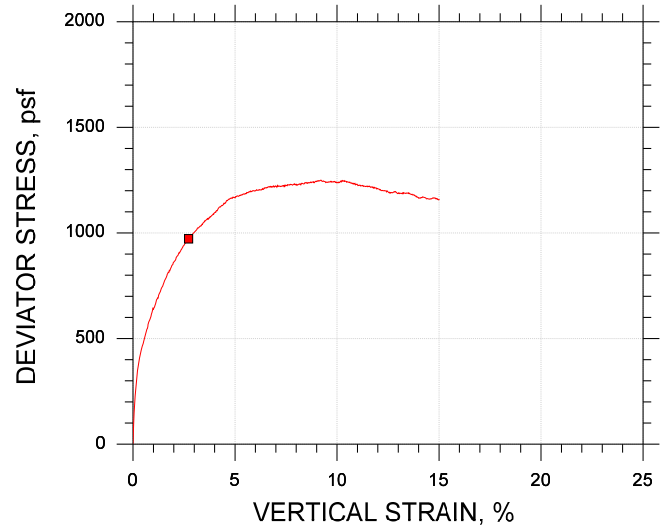
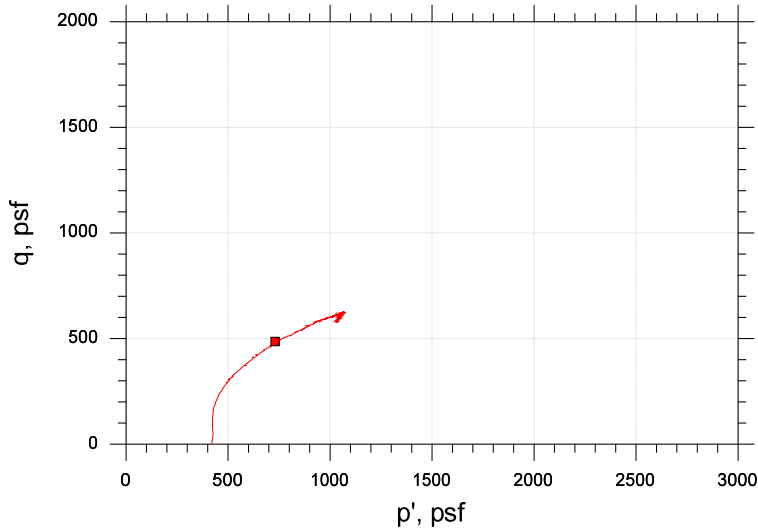
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■	1U	CU-17-1	15-16.4 ft	md	07/24/19	mcm	8/2/19	308853-CU-17-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BFB1-101 BB-BFB1-101	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System LL		



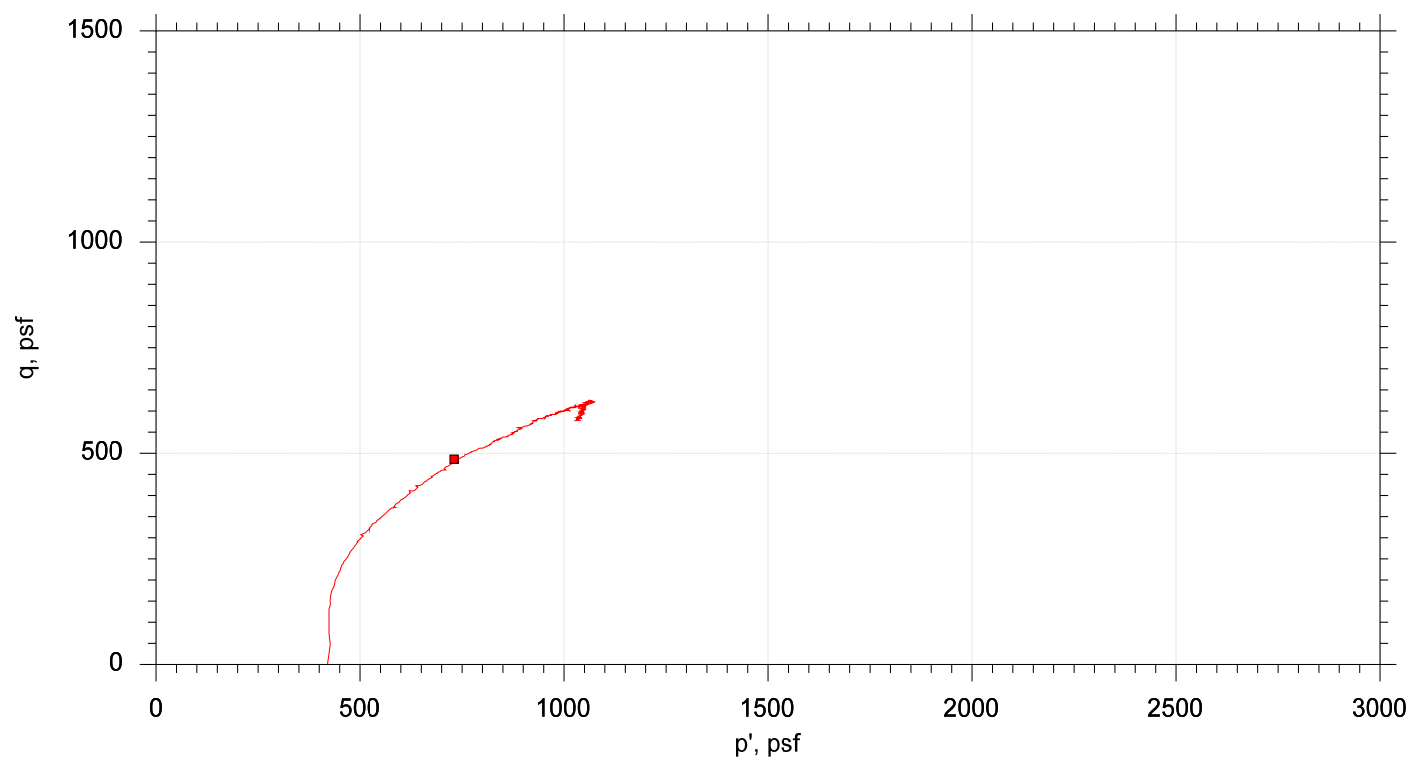
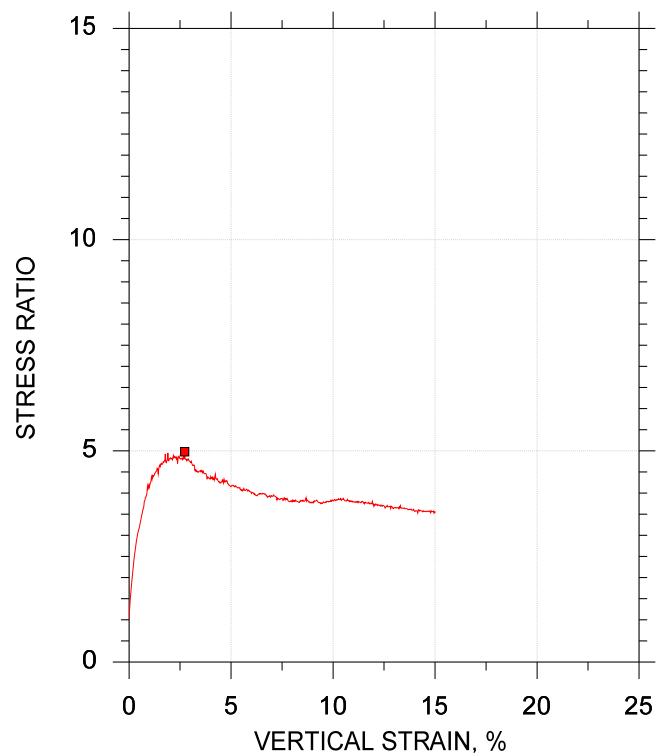
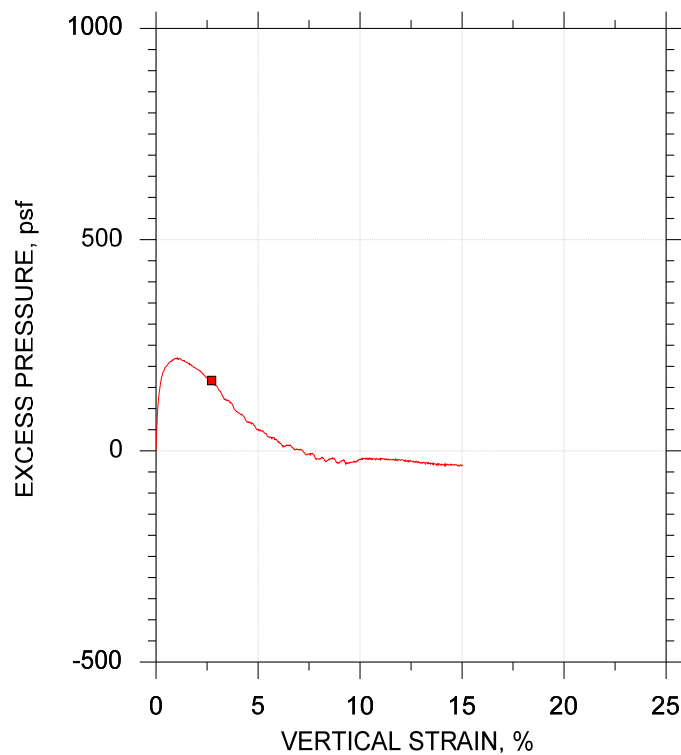
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brrewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: BB-BFB1-202	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: ---	Plastic Limit: ---
Plasticity Index: ---	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		10-12ft		
Test Number		CU-5-1A		
Initial	Height, in	4.450		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	32.8		
	Dry Density, pcf	86.2		
	Saturation (Wet Method), %	92.8		
	Void Ratio	0.955		
Before Shear	Moisture Content, %	34.2		
	Dry Density, pcf	87.6		
	Cross-sectional Area (Method A), in ²	3.213		
	Saturation, %	100.0		
	Void Ratio	0.924		
	Back Pressure, psf	2.027e+004		
Vertical Effective Consolidation Stress, psf		421.4		
Horizontal Effective Consolidation Stress, psf		419.0		
Vertical Strain after Consolidation, %		0.006370		
Volumetric Strain after Consolidation, %		1.941		
Time to 50% Consolidation, min		9.000		
Shear Strength, psf		486.2		
Strain at Failure, %		2.73		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		972.3		
Effective Minor Principal Stress at Failure, psf		244.2		
Effective Major Principal Stress at Failure, psf		1217.		
B-Value		0.94		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System RR				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



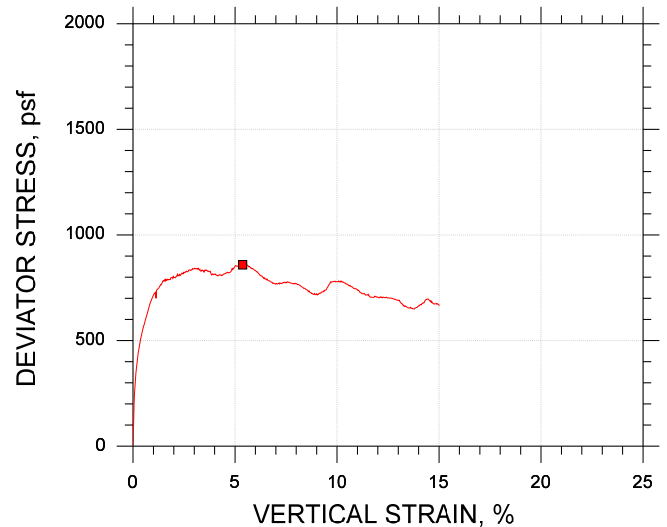
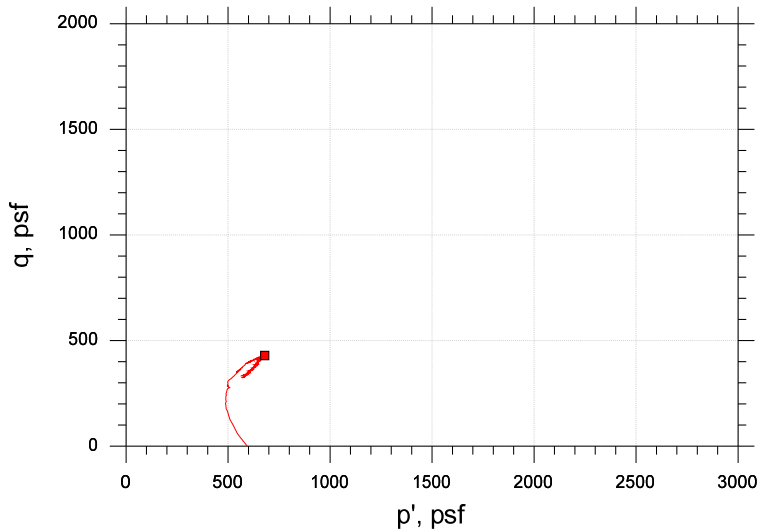
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■	U1	CU-5-1A	10-12ft	trm	4/1/21	mcm	4/9/21	312665-CU-5-1Am.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System RR		



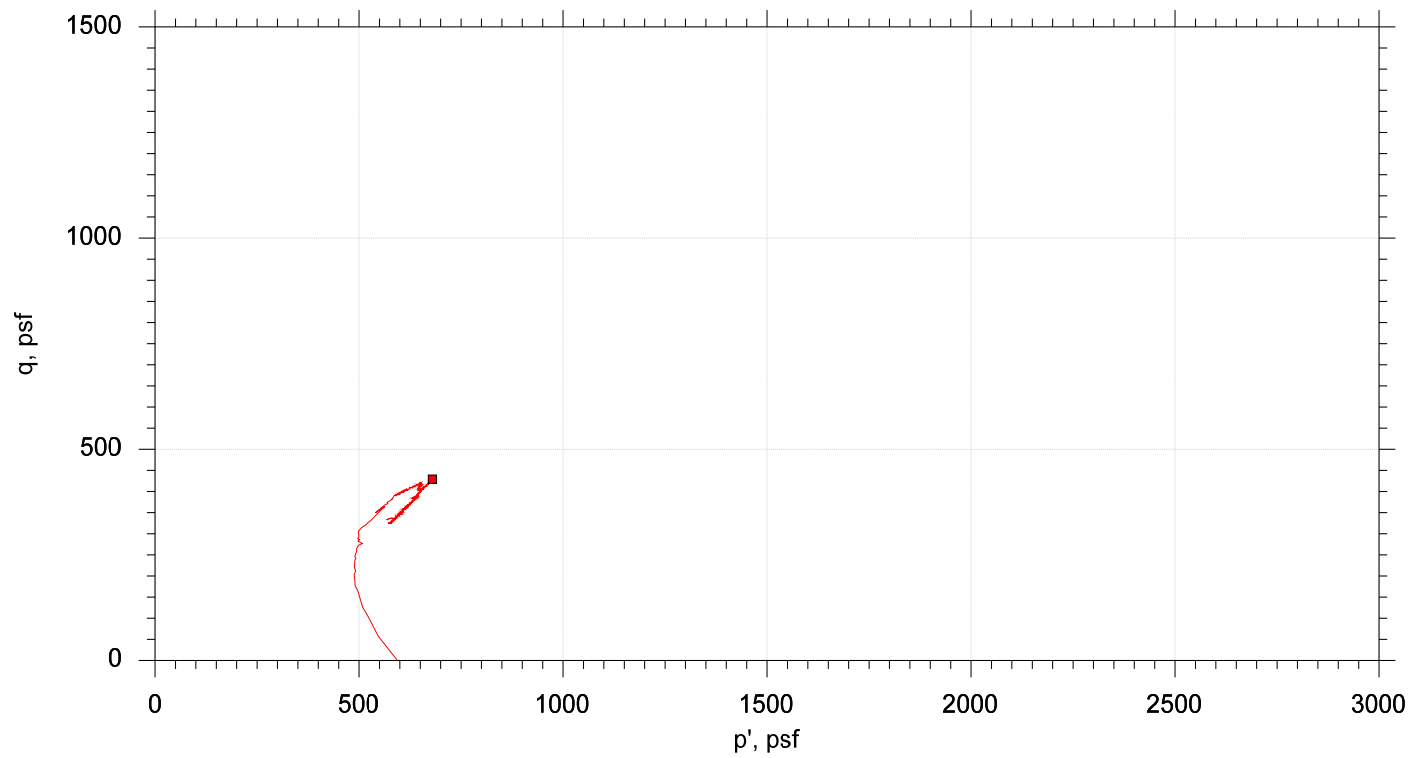
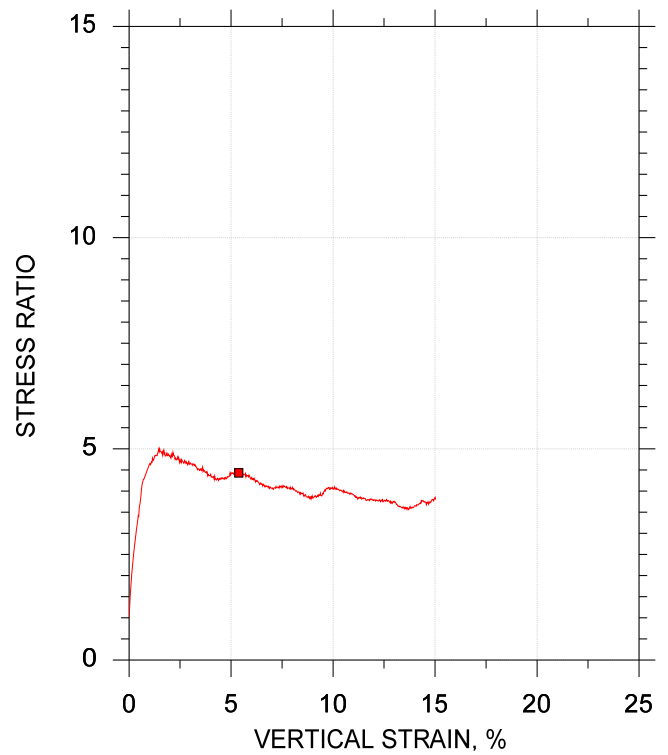
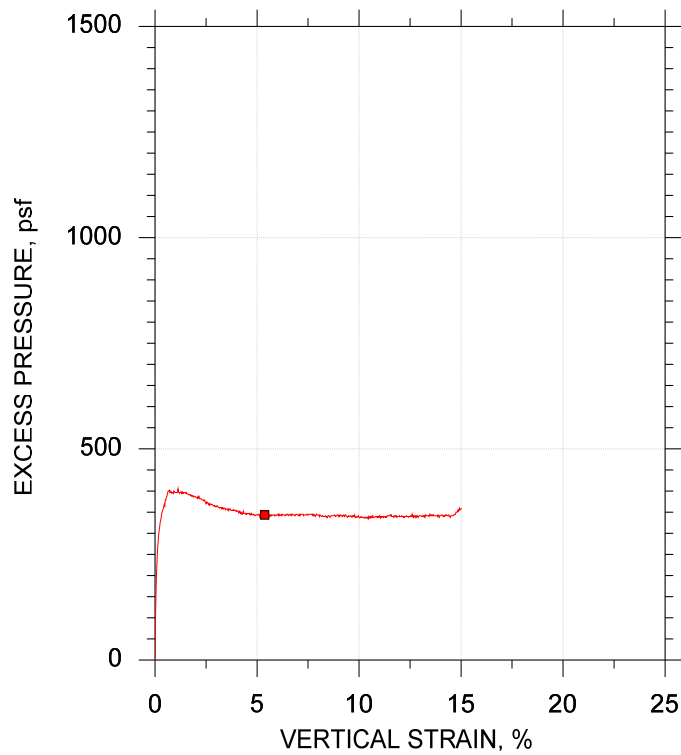
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: BB-BFB-1-202	
Preparation: Intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 20
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED DRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U2		
Depth, ft		15-17 ft		
Test Number		CU-4-1		
Initial	Height, in	4.130		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	39.1		
	Dry Density, pcf	79.8		
	Saturation (Wet Method), %	95.0		
	Void Ratio	1.11		
Before Shear	Moisture Content, %	38.4		
	Dry Density, pcf	82.8		
	Cross-sectional Area (Method A), in ²	3.164		
	Saturation, %	100.0		
	Void Ratio	1.04		
	Back Pressure, psf	2.316e+004		
Vertical Effective Consolidation Stress, psf		590.3		
Horizontal Effective Consolidation Stress, psf		593.9		
Vertical Strain after Consolidation, %		0.3365		
Volumetric Strain after Consolidation, %		0.5055		
Time to 50% Consolidation, min		25.00		
Shear Strength, psf		429.4		
Strain at Failure, %		5.38		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		858.7		
Effective Minor Principal Stress at Failure, psf		250.3		
Effective Major Principal Stress at Failure, psf		1109.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED DRAINED TRIAXIAL TEST by ASTM D4767



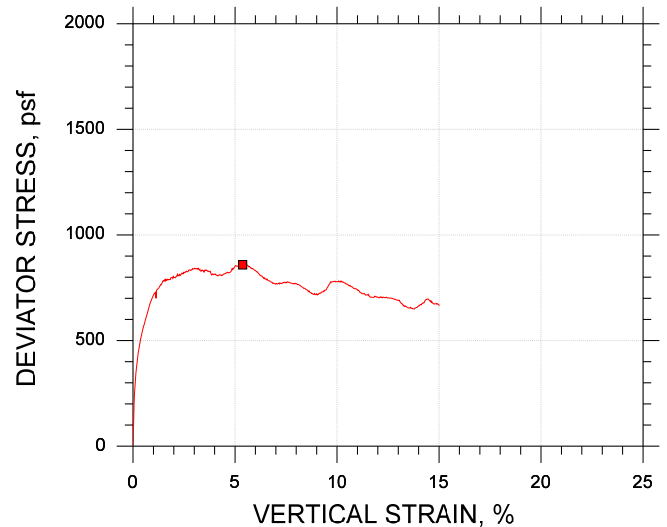
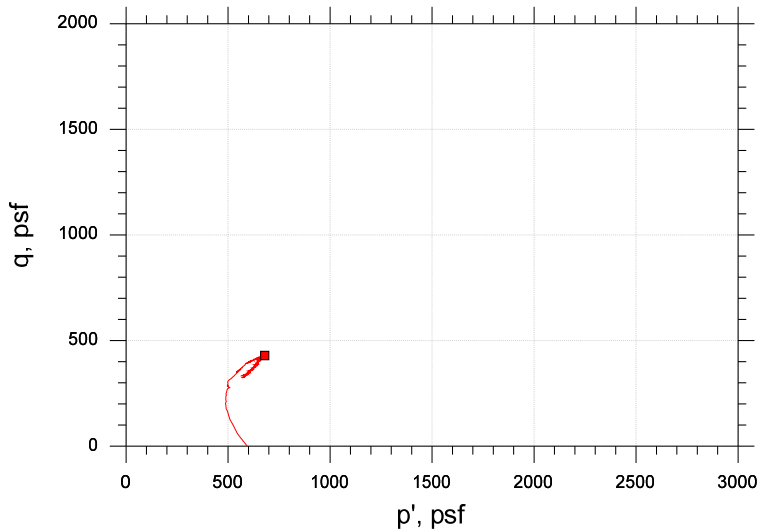
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■	U2	CU-4-1	15-17 ft	trm	2/26/21	mcm	3/16/21	312665-CU-4-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-1-202	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System OO		



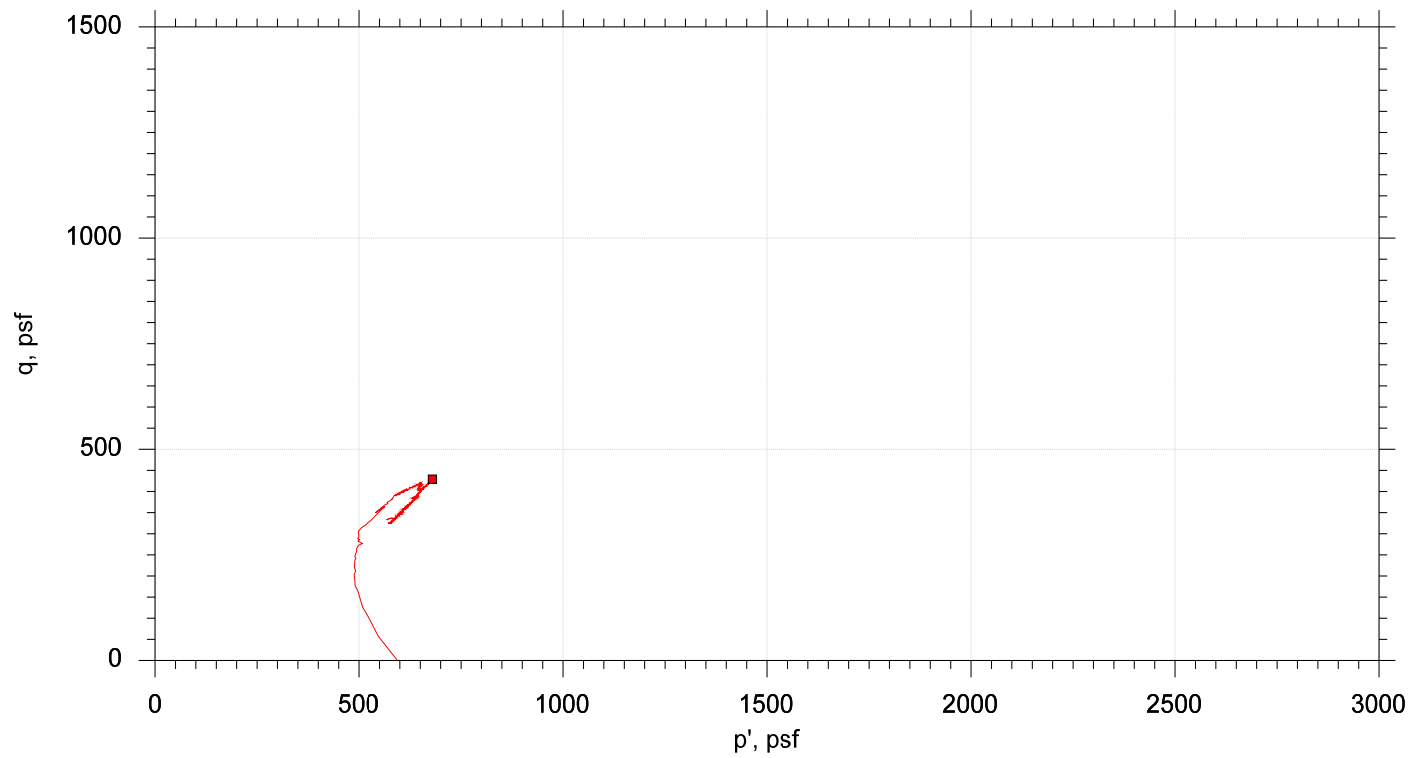
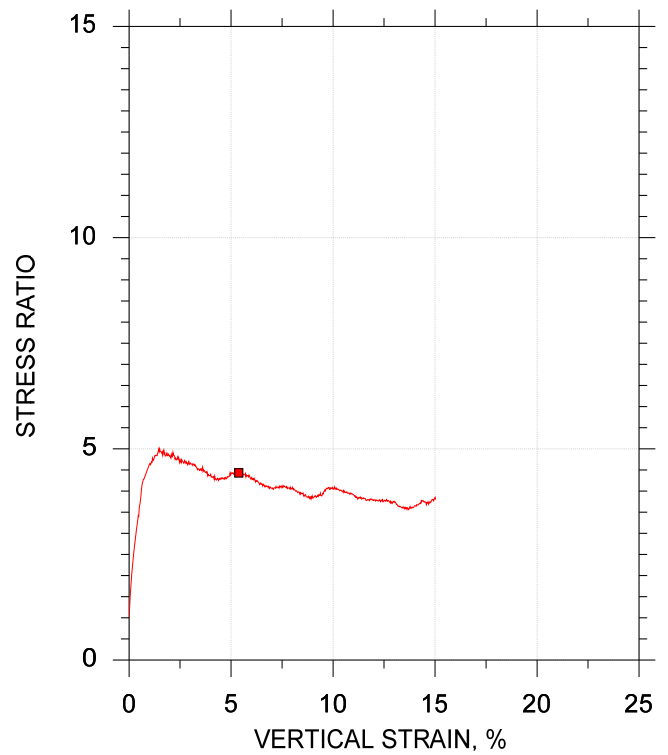
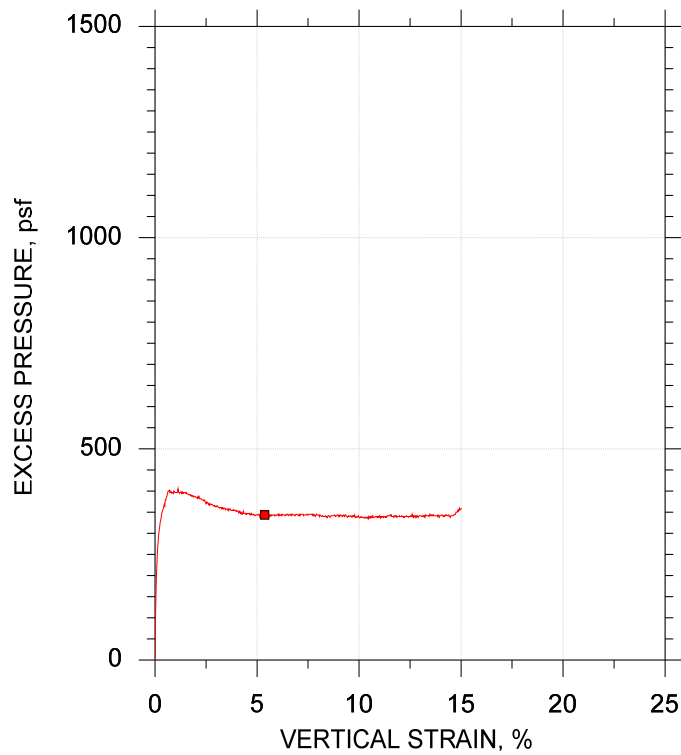
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: BB-BFB-1-202	
Preparation: Intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 20
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U2		
Depth, ft		15-17 ft		
Test Number		CU-4-1		
Initial	Height, in	4.130		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	39.1		
	Dry Density, pcf	79.8		
	Saturation (Wet Method), %	95.0		
	Void Ratio	1.11		
Before Shear	Moisture Content, %	38.4		
	Dry Density, pcf	82.8		
	Cross-sectional Area (Method A), in ²	3.164		
	Saturation, %	100.0		
	Void Ratio	1.04		
	Back Pressure, psf	2.316e+004		
Vertical Effective Consolidation Stress, psf		590.3		
Horizontal Effective Consolidation Stress, psf		593.9		
Vertical Strain after Consolidation, %		0.3365		
Volumetric Strain after Consolidation, %		0.5055		
Time to 50% Consolidation, min		25.00		
Shear Strength, psf		429.4		
Strain at Failure, %		5.38		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		858.7		
Effective Minor Principal Stress at Failure, psf		250.3		
Effective Major Principal Stress at Failure, psf		1109.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED DRAINED TRIAXIAL TEST by ASTM D4767



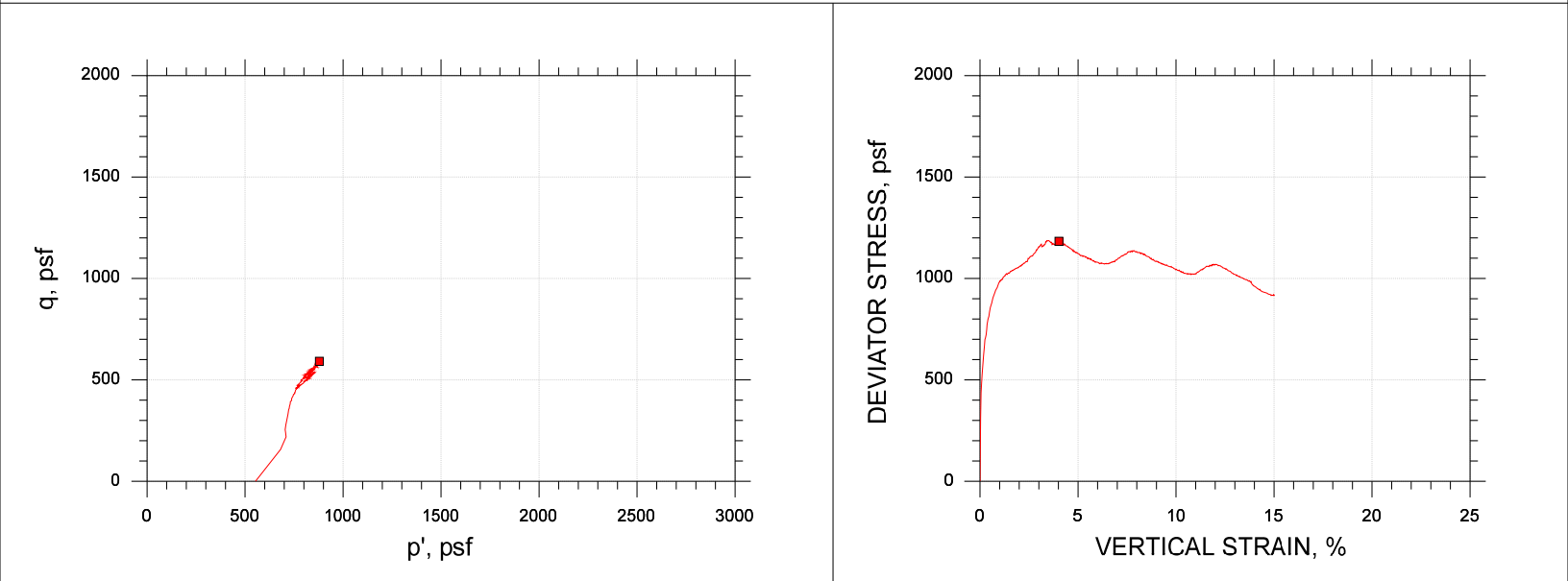
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■	U2	CU-4-1	15-17 ft	trm	2/26/21	mcm	3/16/21	312665-CU-4-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-1-202	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System OO		



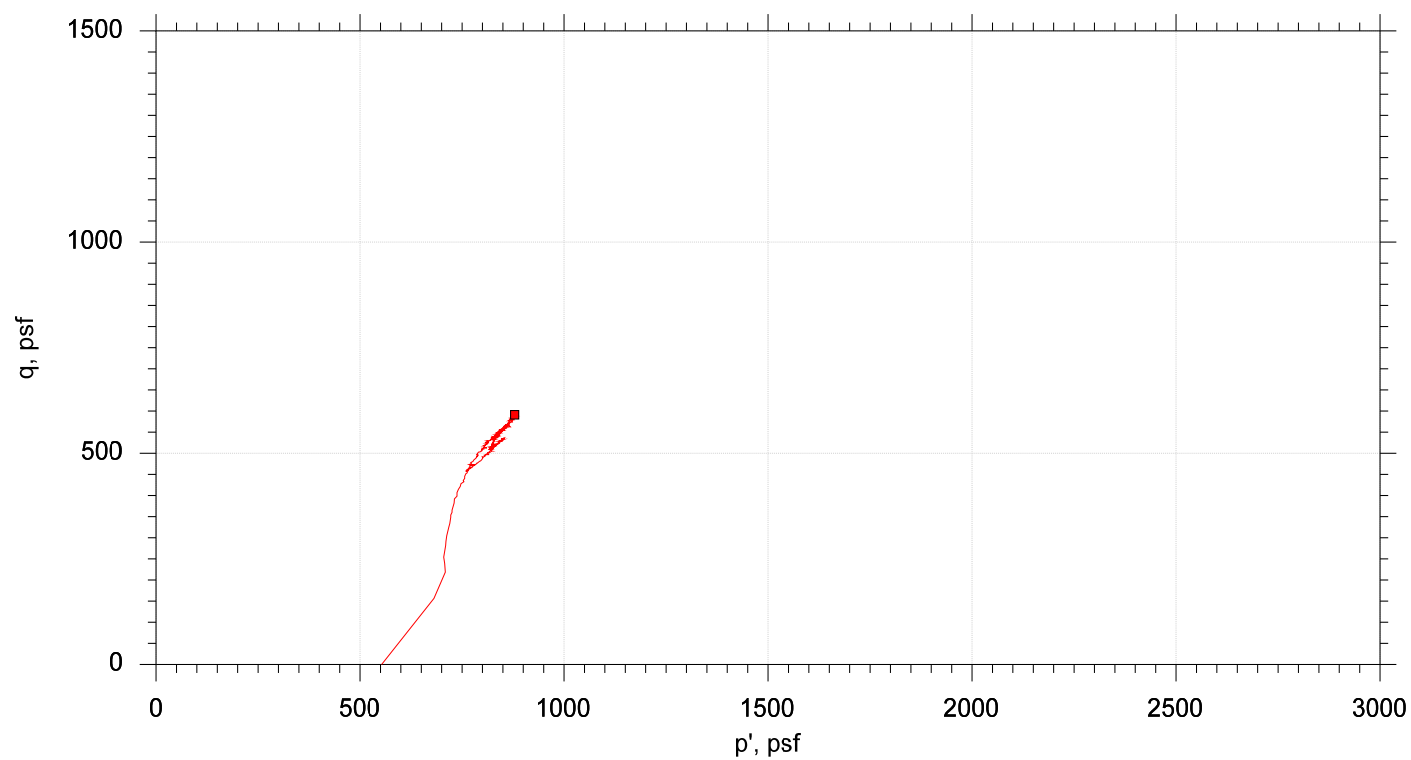
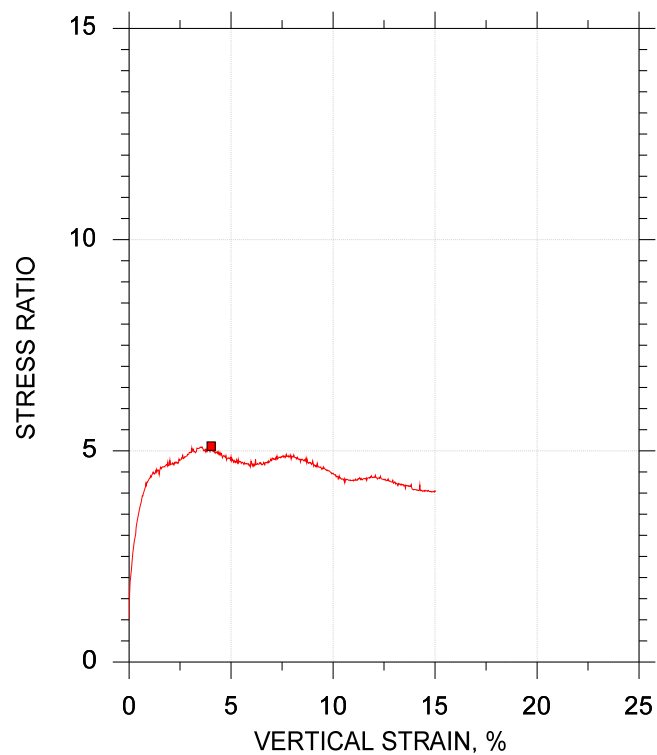
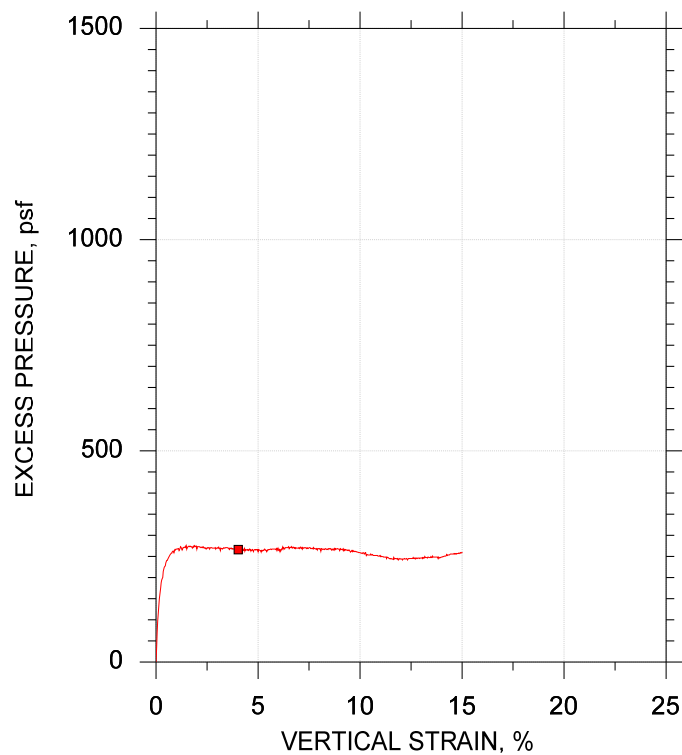
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME.	
Project Number: GTX-312665	
Tested By: md	Checked By: mcm
Boring ID: BB-BFB2-202	
Preparation: intact	
Description: Wet, gray clay	
Classification:	
Group Symbol:	
Liquid Limit: 33	Plastic Limit: 17
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	U1			
Depth, ft	10-12 ft			
Test Number	CU-7-1			
Initial	Height, in	4.450		
	Diameter, in	2.020		
	Moisture Content (from Cuttings), %	35.6		
	Dry Density, pcf	84.1		
	Saturation (Wet Method), %	95.6		
	Void Ratio	1.00		
Before Shear	Moisture Content, %	37.8		
	Dry Density, pcf	83.4		
	Cross-sectional Area (Method A), in²	3.224		
	Saturation, %	100.0		
	Void Ratio	1.02		
	Back Pressure, psf	2.165e+004		
Vertical Effective Consolidation Stress, psf		553.0		
Horizontal Effective Consolidation Stress, psf		552.6		
Vertical Strain after Consolidation, %		0.002077		
Volumetric Strain after Consolidation, %		0.2669		
Time to 50% Consolidation, min		6.800		
Shear Strength, psf		591.3		
Strain at Failure, %		4.03		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1183.		
Effective Minor Principal Stress at Failure, psf		287.4		
Effective Major Principal Stress at Failure, psf		1470.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



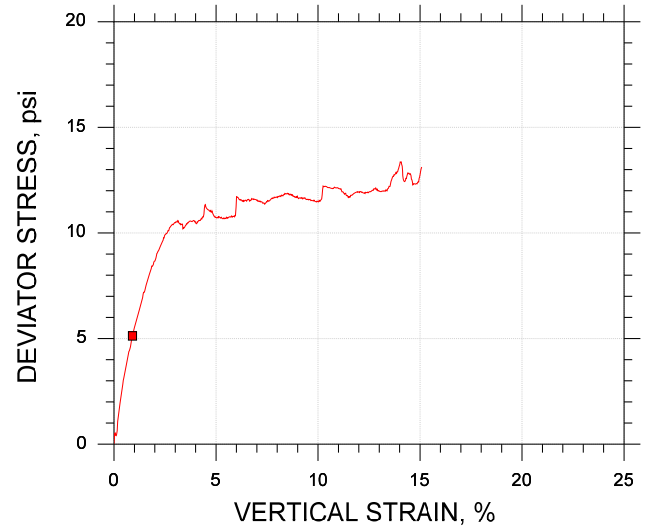
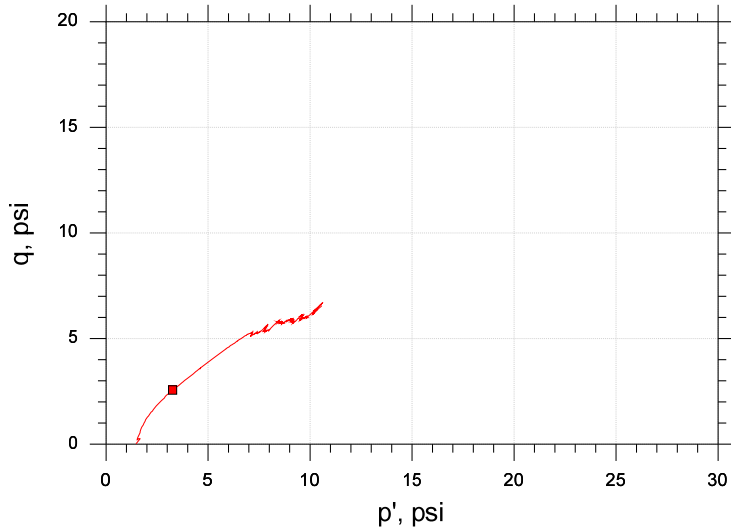
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■	U1	CU-7-1	10-12 ft	md	02/27/21	mcm	3/26/21	312665-CU-7-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME.	Project No.: GTX-312665
	Boring No.: BB-BFB2-202	Sample Type: intact	
	Description: Wet, gray clay		
	Remarks: System V		



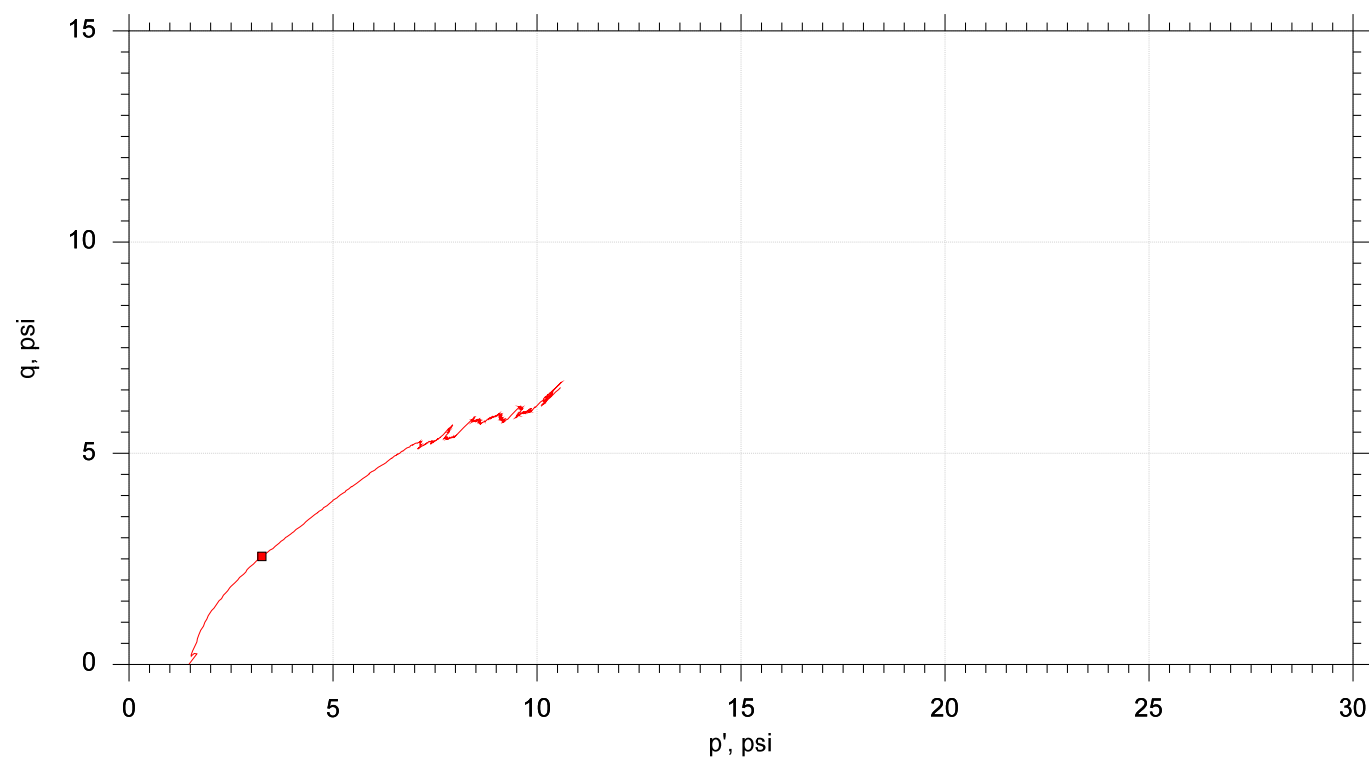
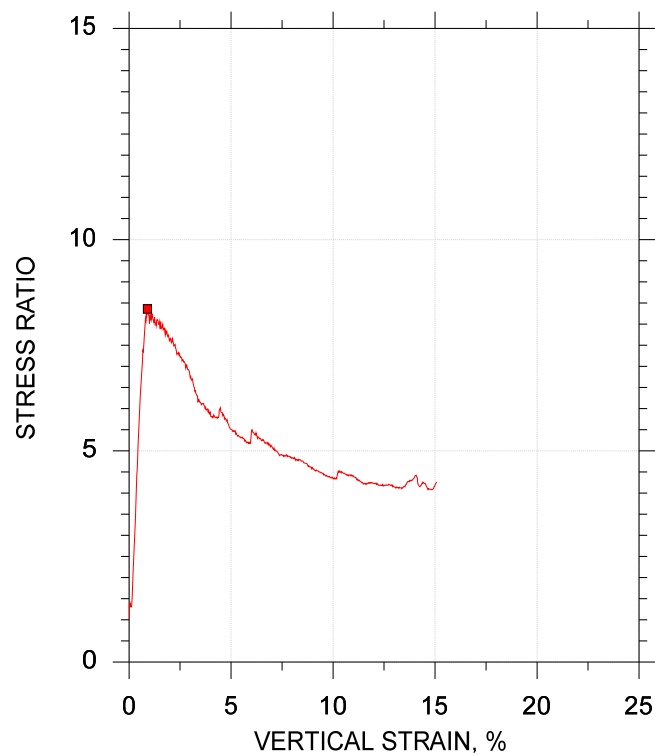
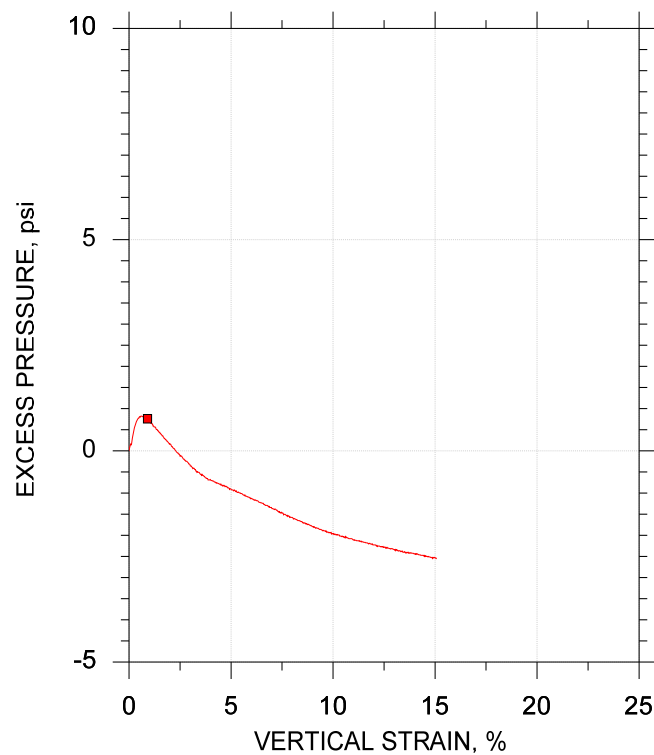
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: BB-EEBT2-101	
Preparation: Intact	
Description: Moist, olive gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 18
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		5-7 ft		
Test Number		CU-19-1		
Initial	Height, in	4.800		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	29.1		
	Dry Density, pcf	89.3		
	Saturation (Wet Method), %	88.7		
	Void Ratio	0.888		
Before Shear	Moisture Content, %	32.5		
	Dry Density, pcf	89.8		
	Cross-sectional Area (Method A), in ²	3.220		
	Saturation, %	100.0		
	Void Ratio	0.876		
	Back Pressure, psi	72.99		
Vertical Effective Consolidation Stress, psi		1.480		
Horizontal Effective Consolidation Stress, psi		1.459		
Vertical Strain after Consolidation, %		0.0009822		
Volumetric Strain after Consolidation, %		0.3388		
Time to 50% Consolidation, min		0.4900		
Shear Strength, psi		2.562		
Strain at Failure, %		0.901		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.123		
Effective Minor Principal Stress at Failure, psi		0.6961		
Effective Major Principal Stress at Failure, psi		5.819		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



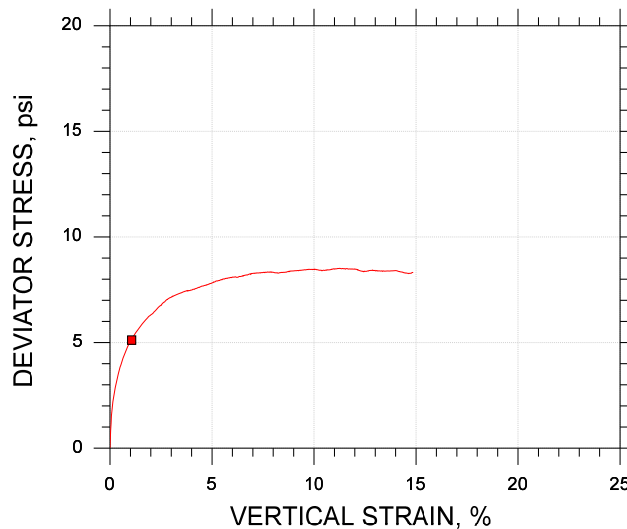
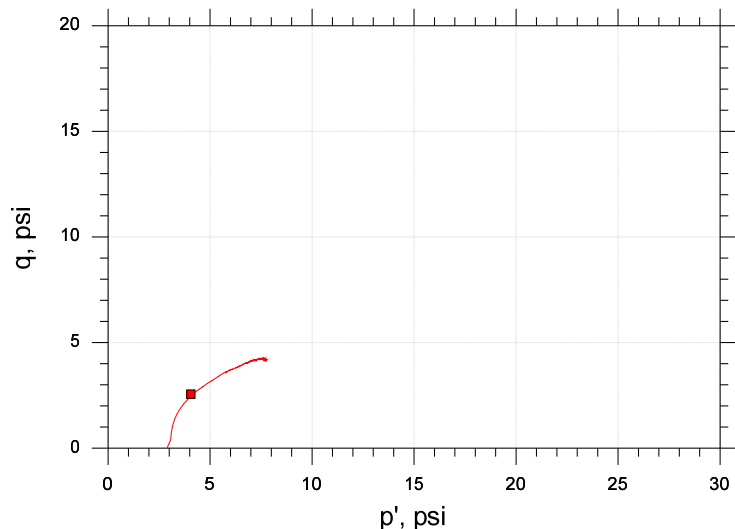
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■	1U	CU-19-1	5-7 ft	trm	7/25/19	mcm	8/2/19	308853-CU-19-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Sample Type: Intact	
	Description: Moist, olive gray clay		
	Remarks: System OO		



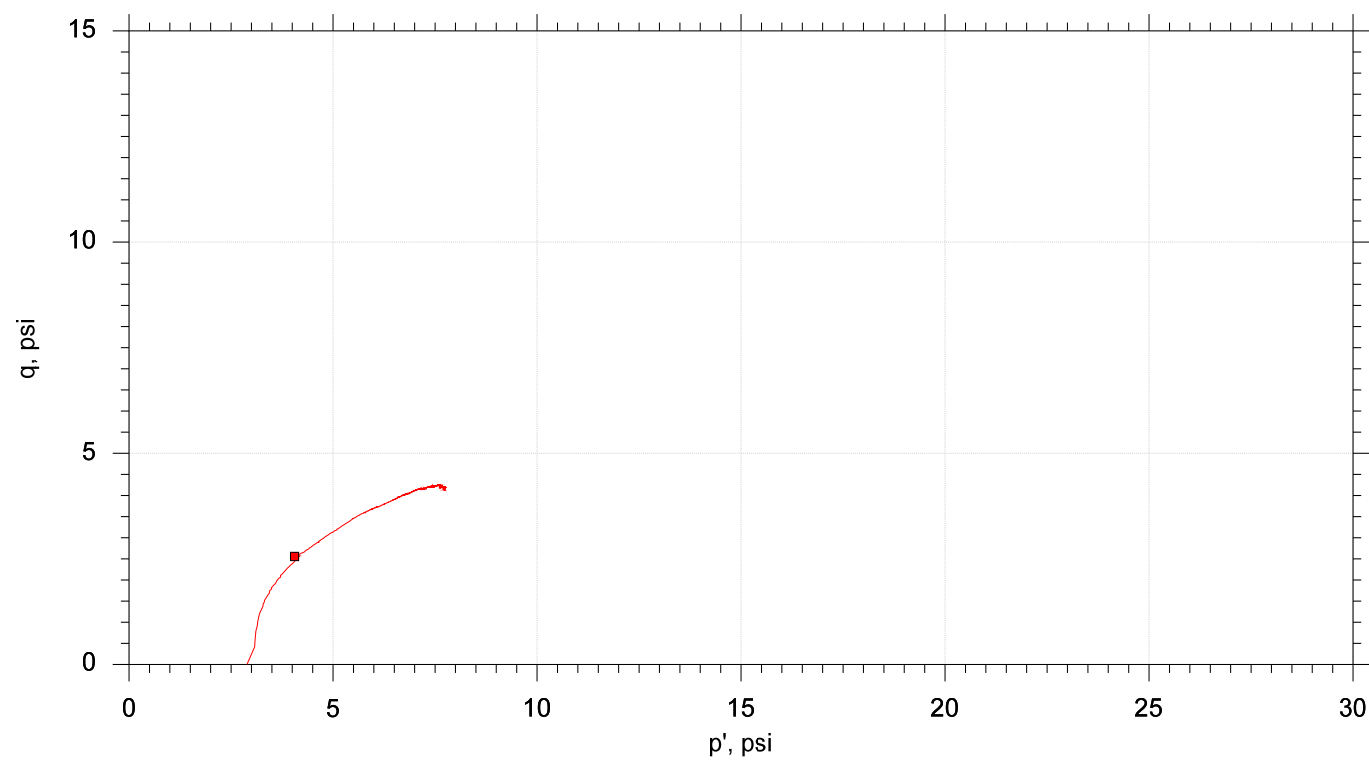
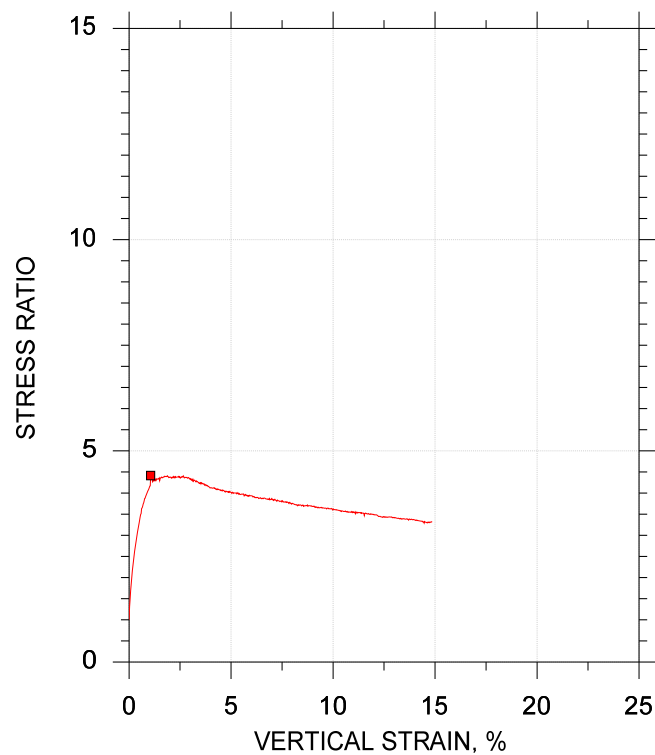
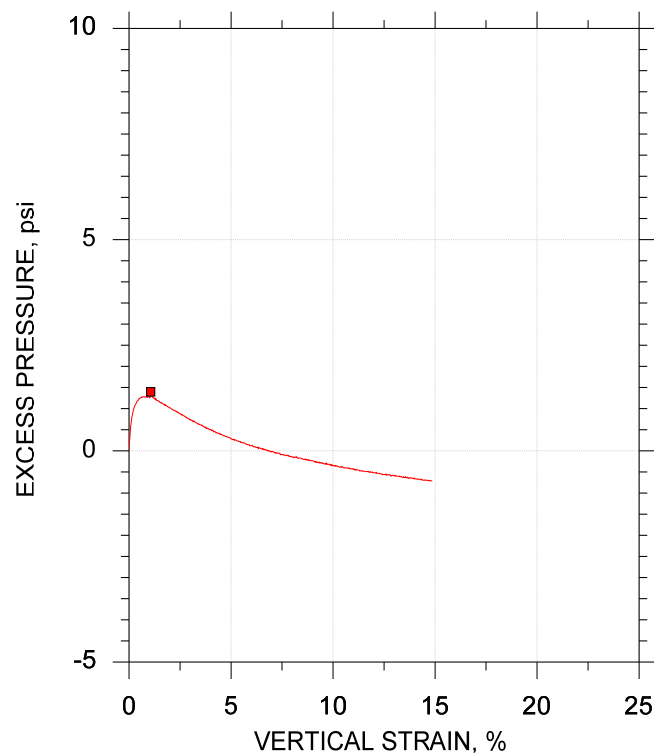
Client: Hakey & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: BB-BST1-101	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 20
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-12 ft		
Test Number		CU-11-1		
Initial	Height, in	4.880		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	34.1		
	Dry Density, pcf	87.0		
	Saturation (Wet Method), %	98.3		
	Void Ratio	0.937		
Before Shear	Moisture Content, %	34.9		
	Dry Density, pcf	86.7		
	Cross-sectional Area (Method A), in ²	3.276		
	Saturation, %	100.0		
	Void Ratio	0.943		
	Back Pressure, psi	142.8		
Vertical Effective Consolidation Stress, psi		2.880		
Horizontal Effective Consolidation Stress, psi		2.887		
Vertical Strain after Consolidation, %		0.1294		
Volumetric Strain after Consolidation, %		0.3651		
Time to 50% Consolidation, min		39.69		
Shear Strength, psi		2.559		
Strain at Failure, %		1.06		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.118		
Effective Minor Principal Stress at Failure, psi		1.498		
Effective Major Principal Stress at Failure, psi		6.616		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



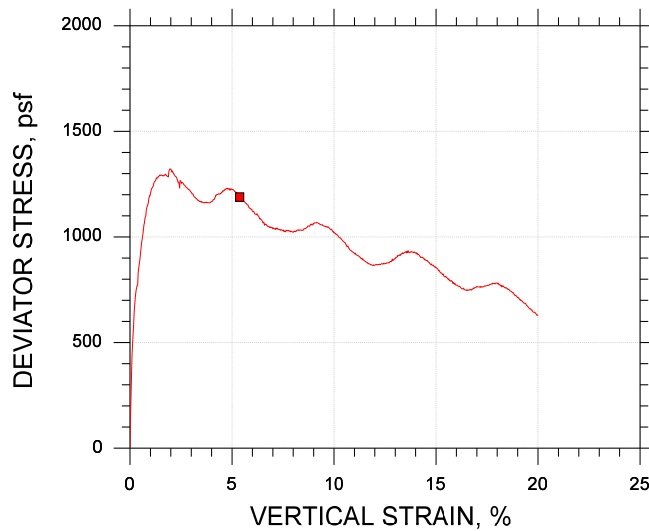
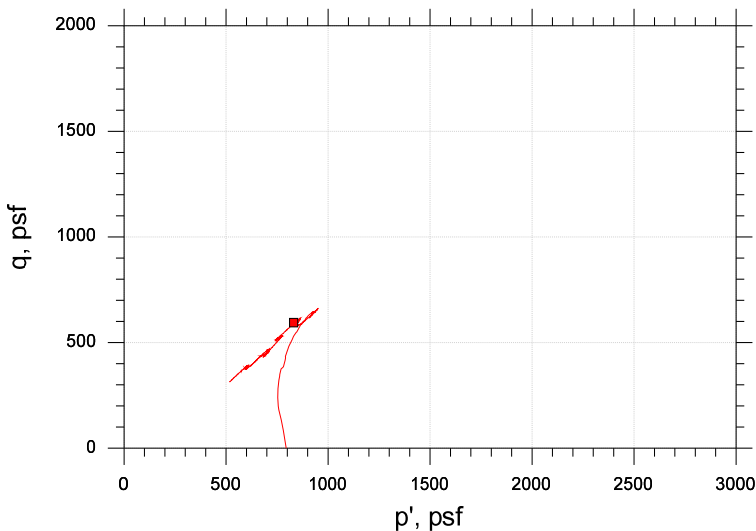
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■	1U	CU-11-1	10-12 ft	trm	7/23/19	mcm	8/2/19	308853-CU-11-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System Q		



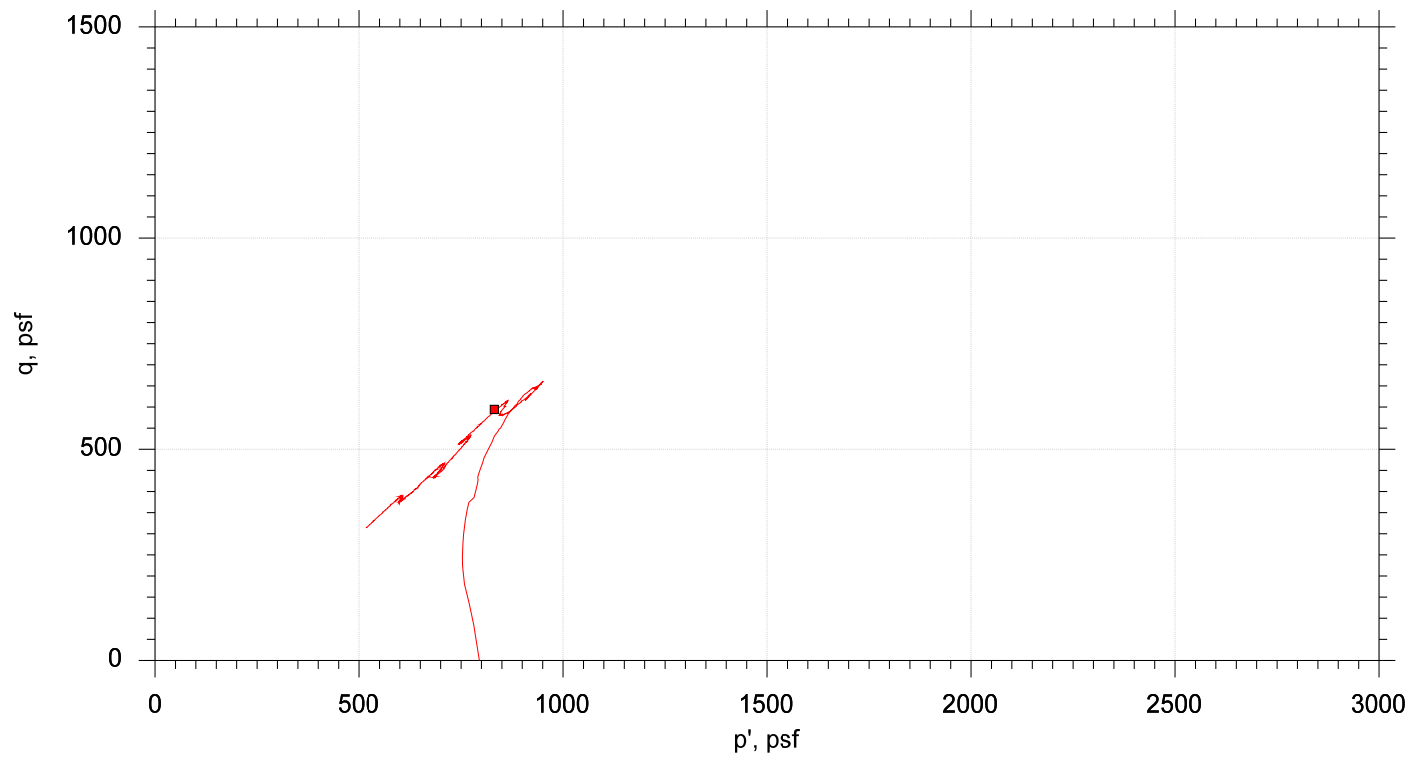
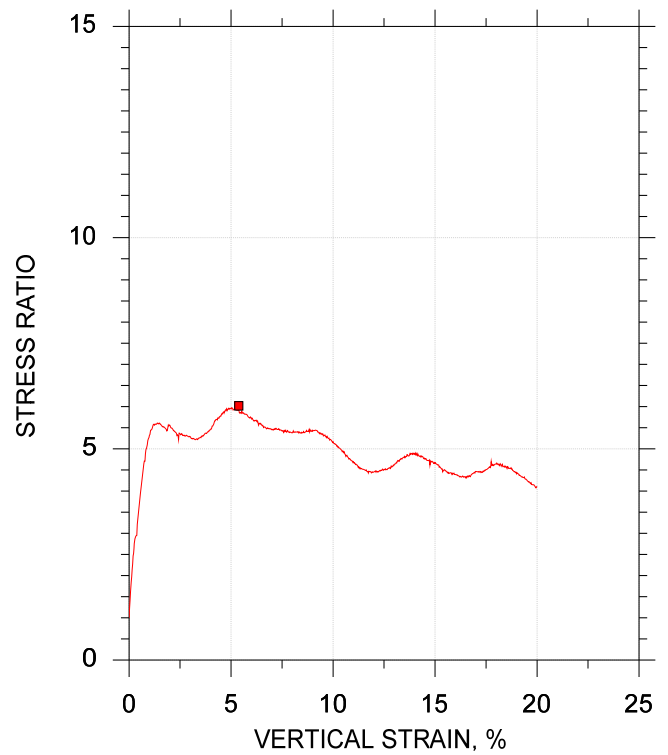
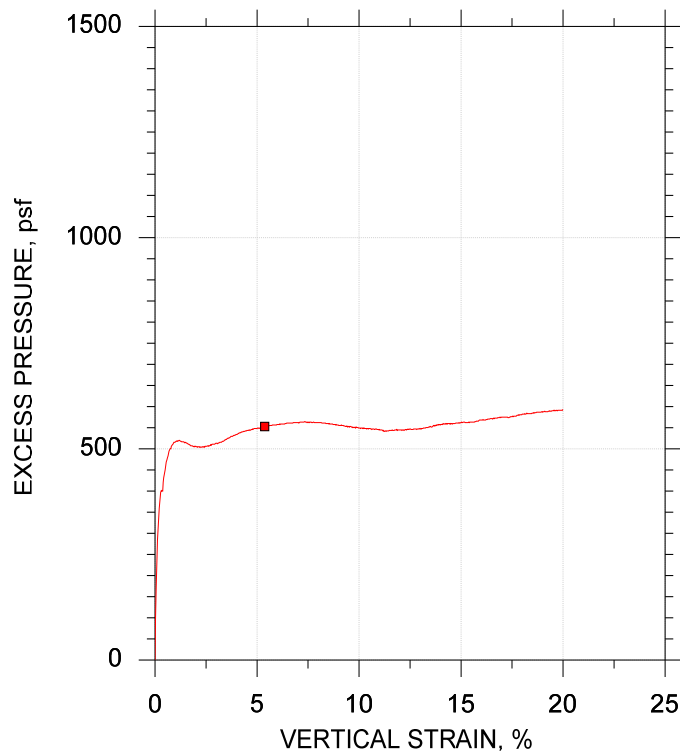
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME.	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: BB-BSTI-201	
Preparation: intact	
Description: Moist, olive gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 36	Plastic Limit: 19
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U-1		
Depth, ft		15-17 ft		
Test Number		CU-2-1		
Initial	Height, in	4.510		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	37.3		
	Dry Density, pcf	81.6		
	Saturation (Wet Method), %	94.5		
	Void Ratio	1.07		
Before Shear	Moisture Content, %	39.2		
	Dry Density, pcf	81.9		
	Cross-sectional Area (Method A), in ²	3.228		
	Saturation, %	100.0		
	Void Ratio	1.06		
	Back Pressure, psf	2.170e+004		
Vertical Effective Consolidation Stress, psf		794.6		
Horizontal Effective Consolidation Stress, psf		794.6		
Vertical Strain after Consolidation, %		0.02864		
Volumetric Strain after Consolidation, %		0.2779		
Time to 50% Consolidation, min		25.00		
Shear Strength, psf		594.5		
Strain at Failure, %		5.38		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1189.		
Effective Minor Principal Stress at Failure, psf		236.9		
Effective Major Principal Stress at Failure, psf		1426.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System Y				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



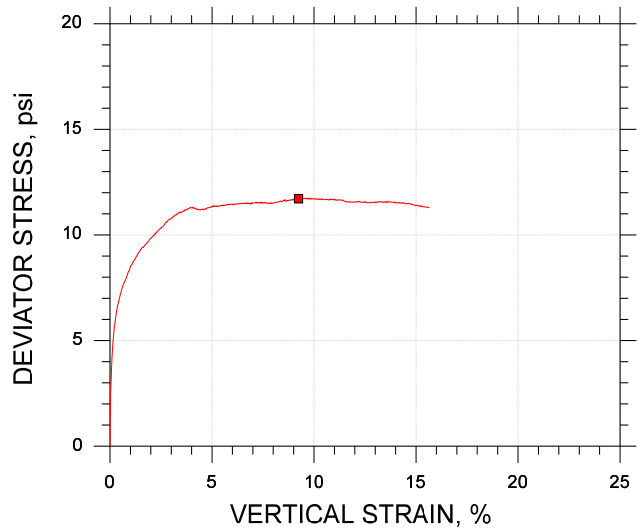
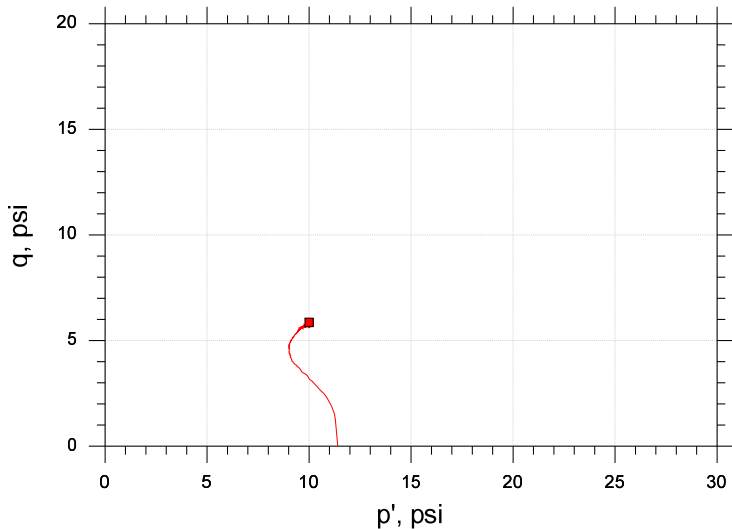
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■	U-1	CU-2-1	15-17 ft	trm	2/26/21	mcm	3/15/21	312665-CU-2-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME.	Project No.: GTX-312665
	Boring No.: BB-BSTI-201	Sample Type: intact	
	Description: Moist, olive gray clay		
	Remarks: System Y		



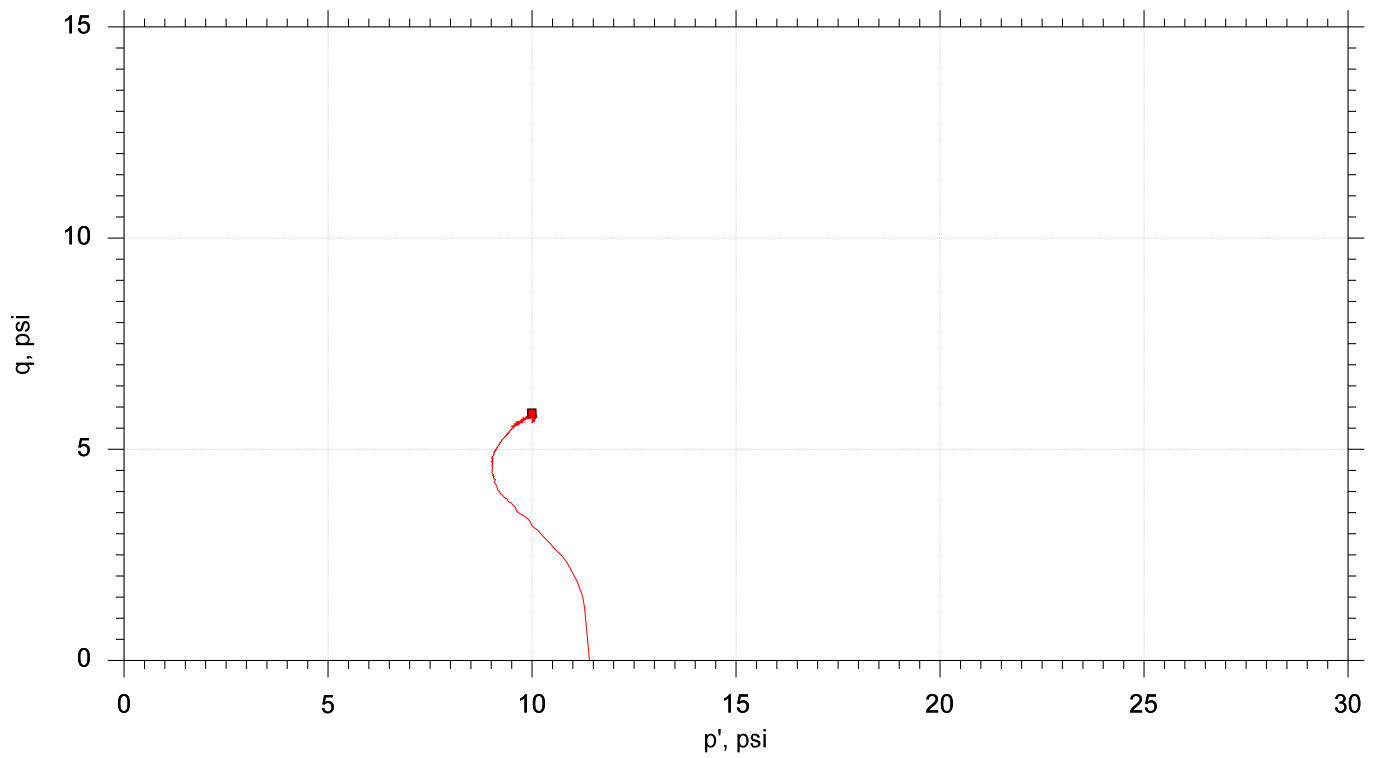
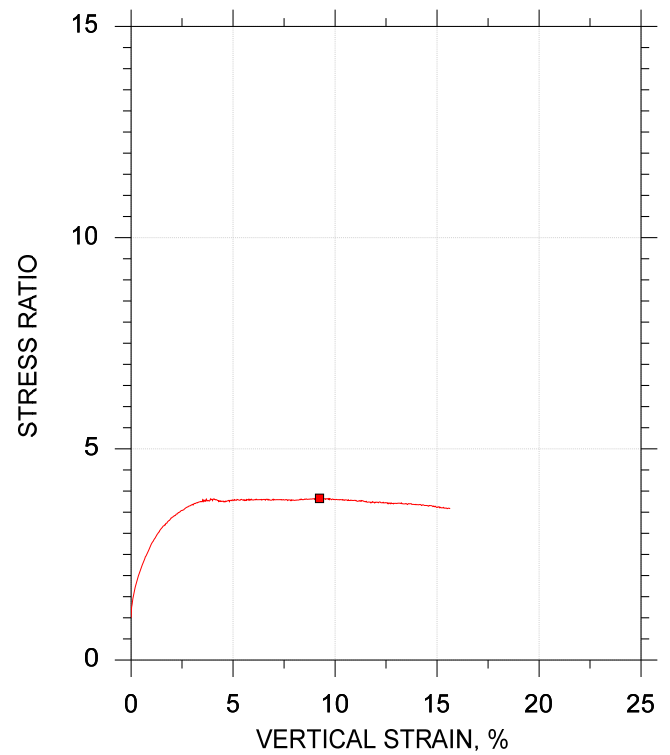
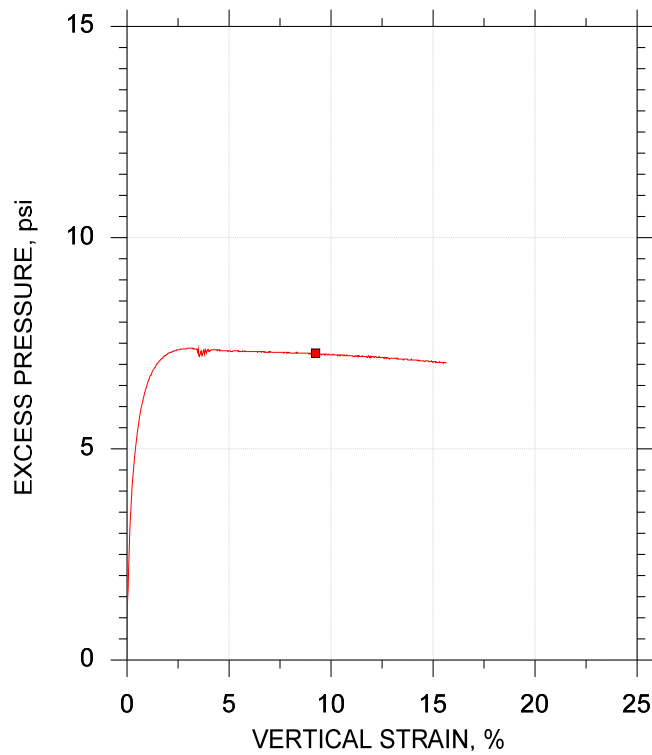
Client: Haley & Aldrich, Inc.	
Project Name: Rte-9/I-395 Conn. - Wilson St	
Project Location: Brewer & Eddington, ME	
Project Number: GTX-311345	
Tested By: md	Checked By: njh
Boring ID: BB-BWS-301	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 19
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		3U		
Depth, ft		29-31		
Test Number		CU-1-1		
Initial	Height, in	4.530		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	32.4		
	Dry Density, pcf	88.7		
	Saturation (Wet Method), %	97.2		
	Void Ratio	0.900		
Before Shear	Moisture Content, %	32.0		
	Dry Density, pcf	90.4		
	Cross-sectional Area (Method A), in ²	3.189		
	Saturation, %	100.0		
	Void Ratio	0.864		
	Back Pressure, psi	154.9		
Vertical Effective Consolidation Stress, psi		11.38		
Horizontal Effective Consolidation Stress, psi		11.41		
Vertical Strain after Consolidation, %		0.4404		
Volumetric Strain after Consolidation, %		1.952		
Time to 50% Consolidation, min		49.00		
Shear Strength, psi		5.856		
Strain at Failure, %		9.24		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		11.71		
Effective Minor Principal Stress at Failure, psi		4.140		
Effective Major Principal Stress at Failure, psi		15.85		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System RR				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



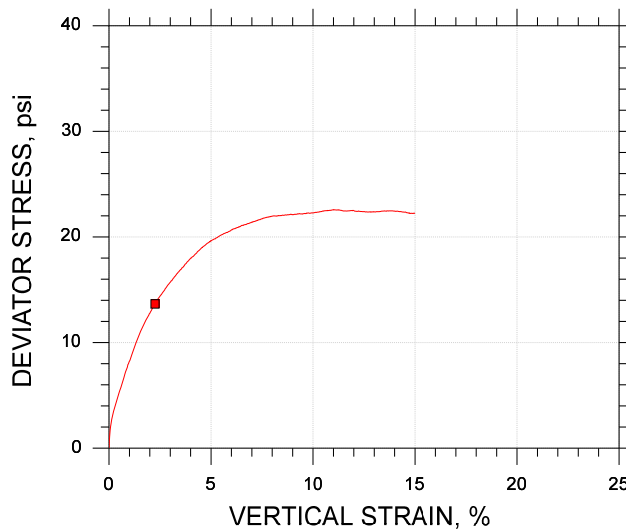
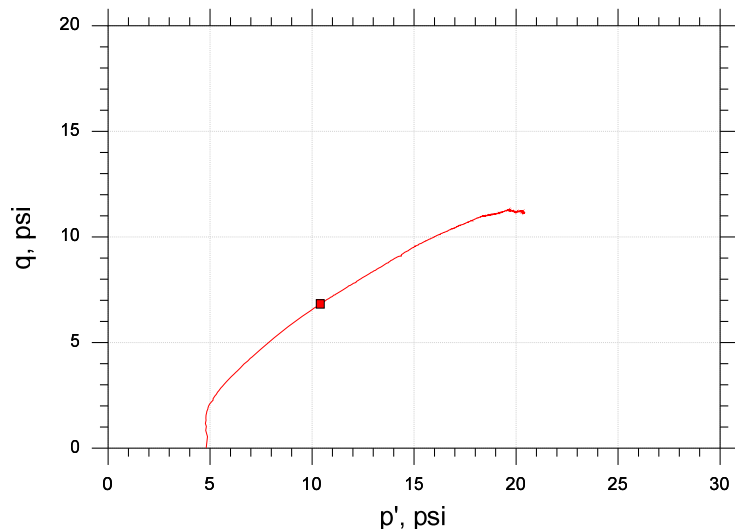
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	Project: Rte-9/I-395 Conn. - Wilson St	Location: Brewer & Eddington, ME	Project No.: GTX-311345
	Boring No.: BB-BWS-301	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System RR		



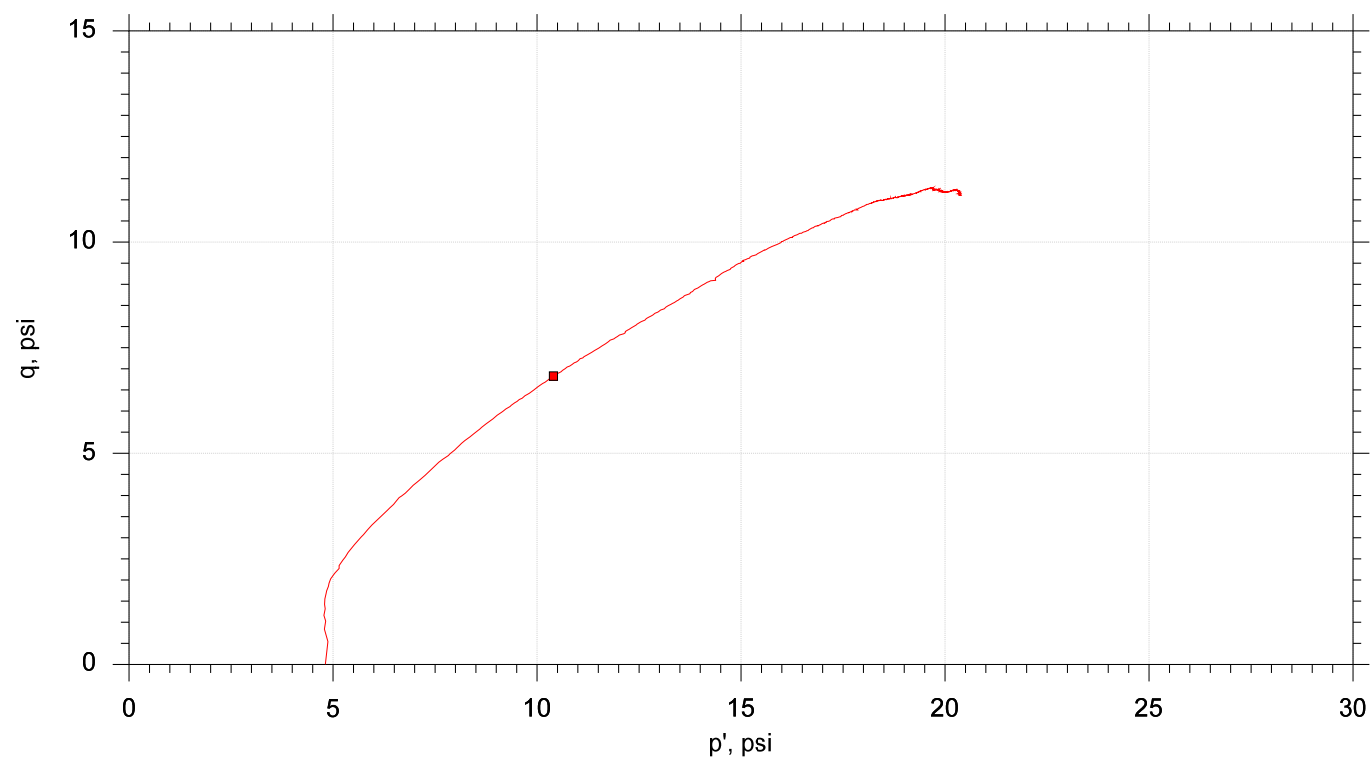
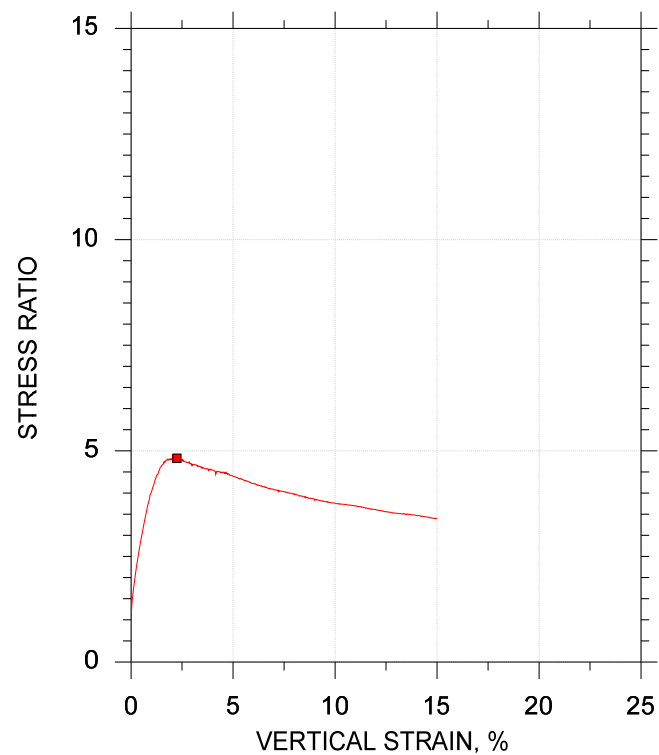
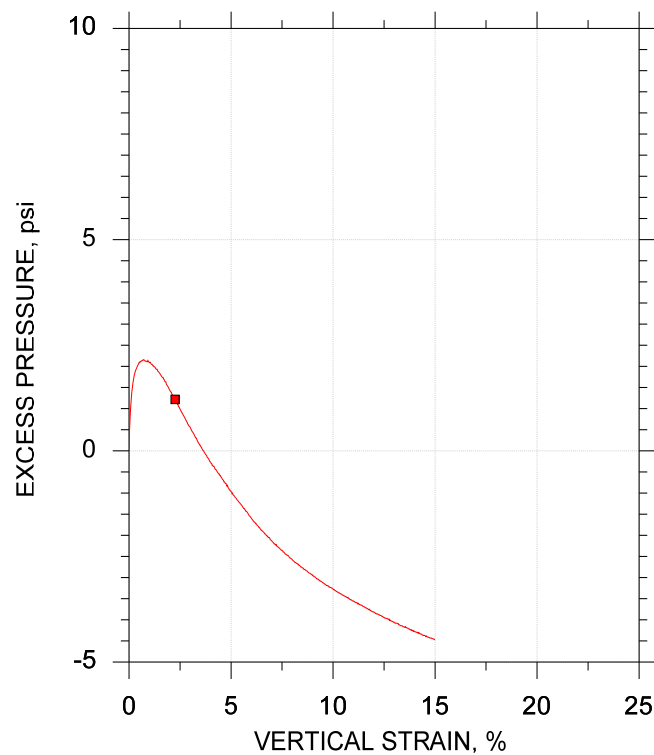
Client: Haley& Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308553	
Tested By: md/trm	Checked By: mcm
Boring ID: HB-BE-101	
Preparation: intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 45	Plastic Limit: 24
Plasticity Index: 21	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	1U			
Depth, ft	5-7 ft			
Test Number	CU-3-1			
Initial	Height, in	6.340		
	Diameter, in	2.860		
	Moisture Content (from Cuttings), %	33.0		
	Dry Density, pcf	88.8		
	Saturation (Wet Method), %	99.1		
	Void Ratio	0.899		
Before Shear	Moisture Content, %	32.4		
	Dry Density, pcf	89.9		
	Cross-sectional Area (Method A), in ²	6.372		
	Saturation, %	100.0		
	Void Ratio	0.876		
	Back Pressure, psi	157.0		
Vertical Effective Consolidation Stress, psi		4.805		
Horizontal Effective Consolidation Stress, psi		4.805		
Vertical Strain after Consolidation, %		0.1753		
Volumetric Strain after Consolidation, %		0.4779		
Time to 50% Consolidation, min		4.000		
Shear Strength, psi		6.830		
Strain at Failure, %		2.25		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		13.66		
Effective Minor Principal Stress at Failure, psi		3.572		
Effective Major Principal Stress at Failure, psi		17.23		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



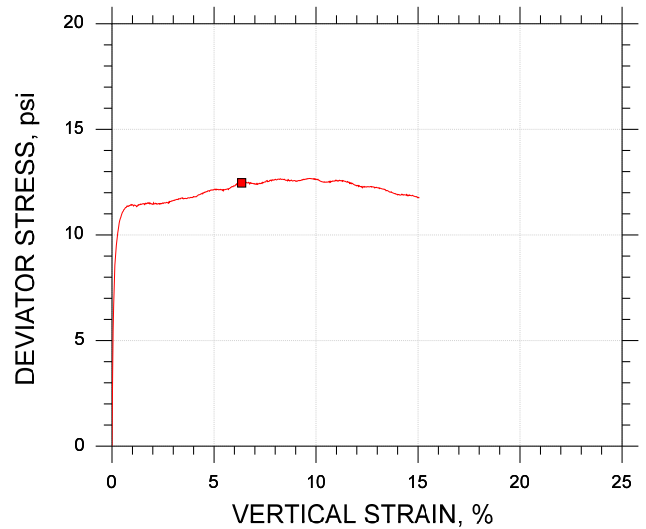
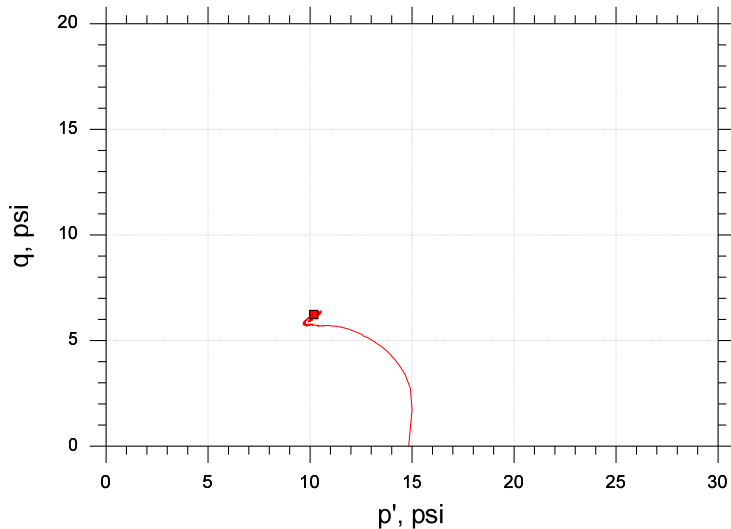
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■	1U	CU-3-1	5-7 ft	md/trm	9/27/18	mcm	10/17/18	308853-CU-3-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308553
	Boring No.: HB-BE-101	Sample Type: intact	
	Description: Moist, dark gray clay		
	Remarks: System F		



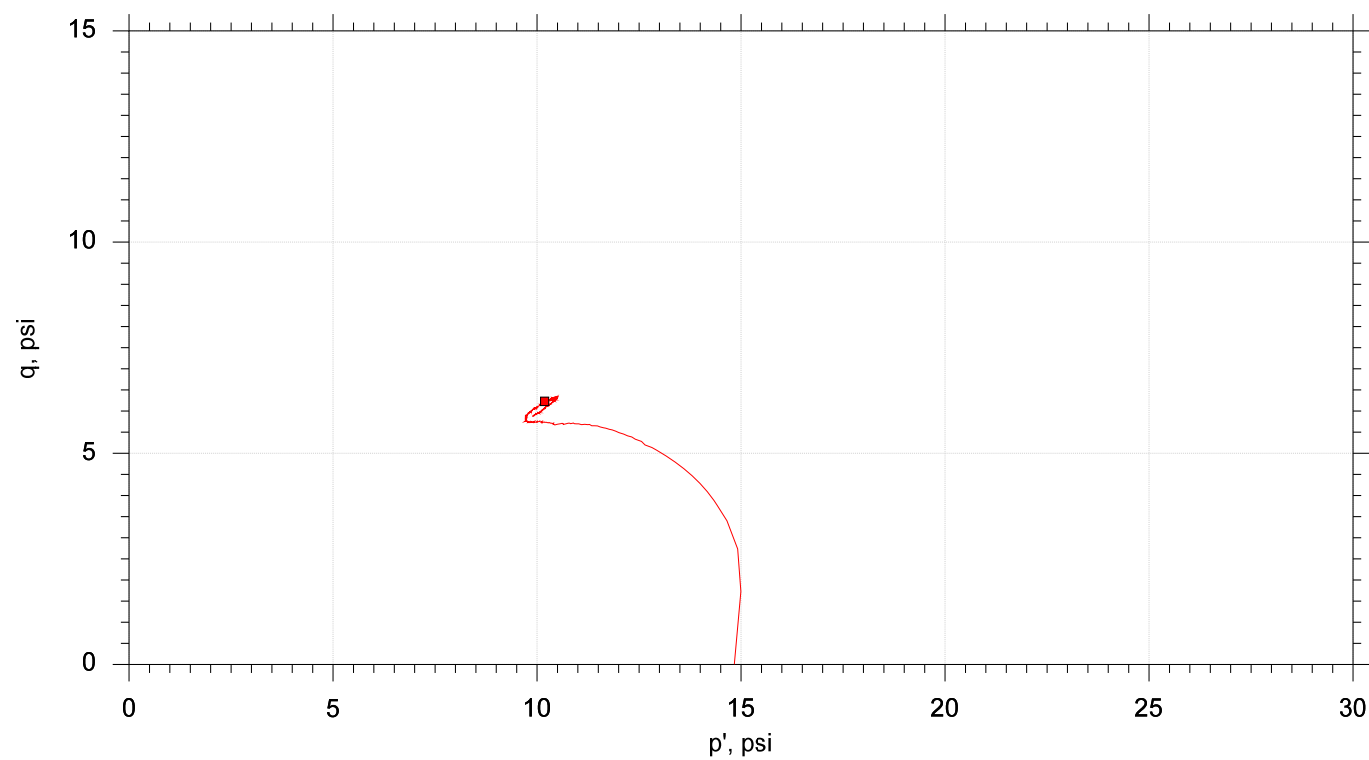
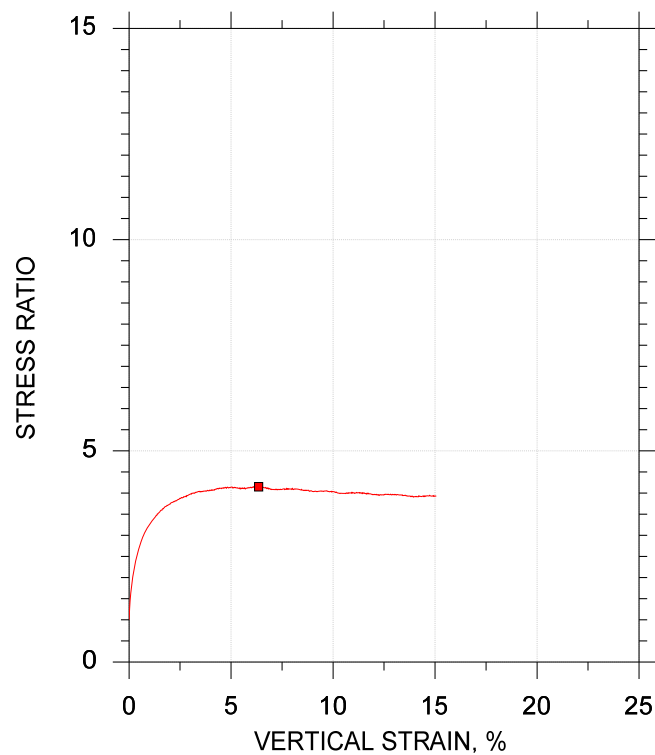
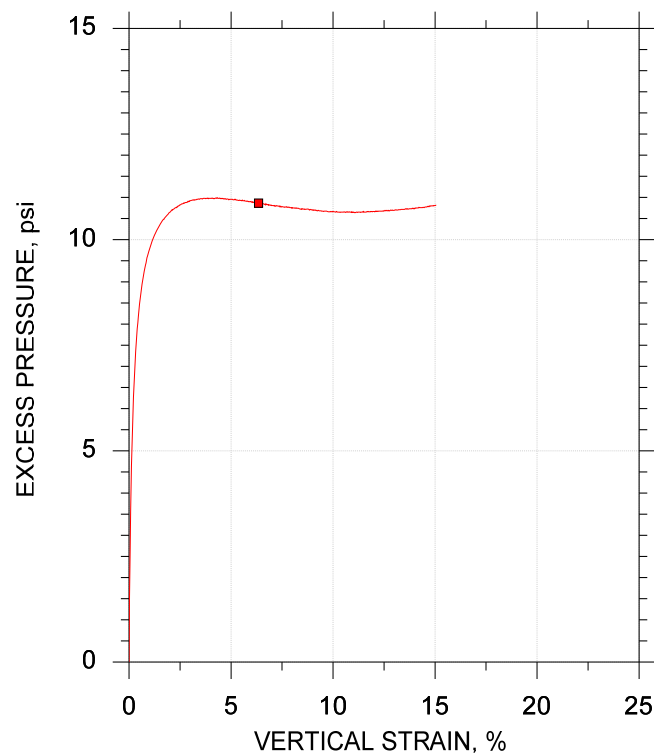
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: md/trm	Checked By: mcm
Boring ID: HB-BE-101	
Preparation: intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 20
Plasticity Index: 14	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		4U		
Depth, ft		30-32 ft		
Test Number		CU-5-1		
Initial	Height, in	6.000		
	Diameter, in	2.860		
	Moisture Content (from Cuttings), %	34.7		
	Dry Density, pcf	86.8		
	Saturation (Wet Method), %	99.5		
	Void Ratio	0.942		
Before Shear	Moisture Content, %	32.1		
	Dry Density, pcf	90.3		
	Cross-sectional Area (Method A), in ²	6.196		
	Saturation, %	100.0		
	Void Ratio	0.866		
	Back Pressure, psi	167.0		
Vertical Effective Consolidation Stress, psi		14.81		
Horizontal Effective Consolidation Stress, psi		14.83		
Vertical Strain after Consolidation, %		0.4010		
Volumetric Strain after Consolidation, %		4.010		
Time to 50% Consolidation, min		110.3		
Shear Strength, psi		6.232		
Strain at Failure, %		6.35		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		12.46		
Effective Minor Principal Stress at Failure, psi		3.954		
Effective Major Principal Stress at Failure, psi		16.42		
B-Value		0.91		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System S				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	4U	CU-5-1	30-32 ft	md/trm	9/28/2018	mcm	10/17/18	308853-CU-5-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Sample Type: intact	
	Description: Moist, dark gray clay		
	Remarks: System S		



Client: Haley & Aldrich, Inc.

Project Name: Rt 9/I-395

Project Location: Brewer and Eddington, ME

Project Number: GTX-308853

Tested By: md/trm

Checked By: mcm

Boring ID: HB-BE-102

Preparation: md

Description: Moist, dark olive gray clay

Classification: ---

Group Symbol: ---

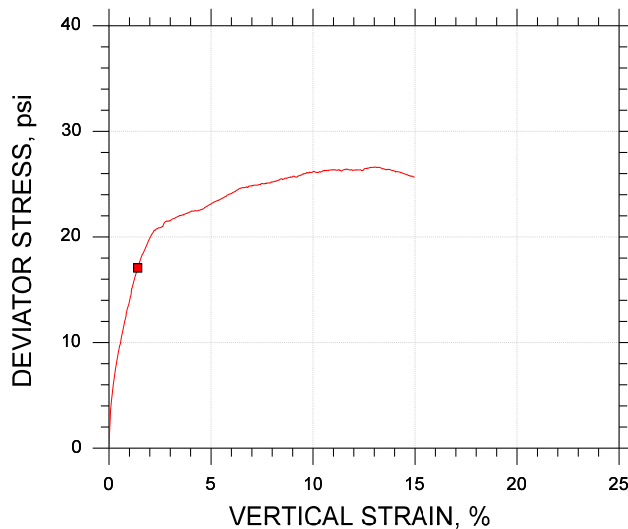
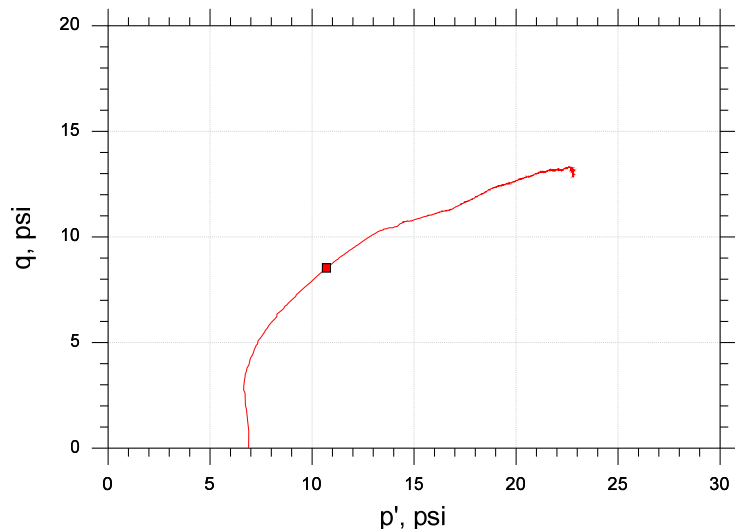
Liquid Limit: 32

Plastic Limit: 19

Plasticity Index: 13

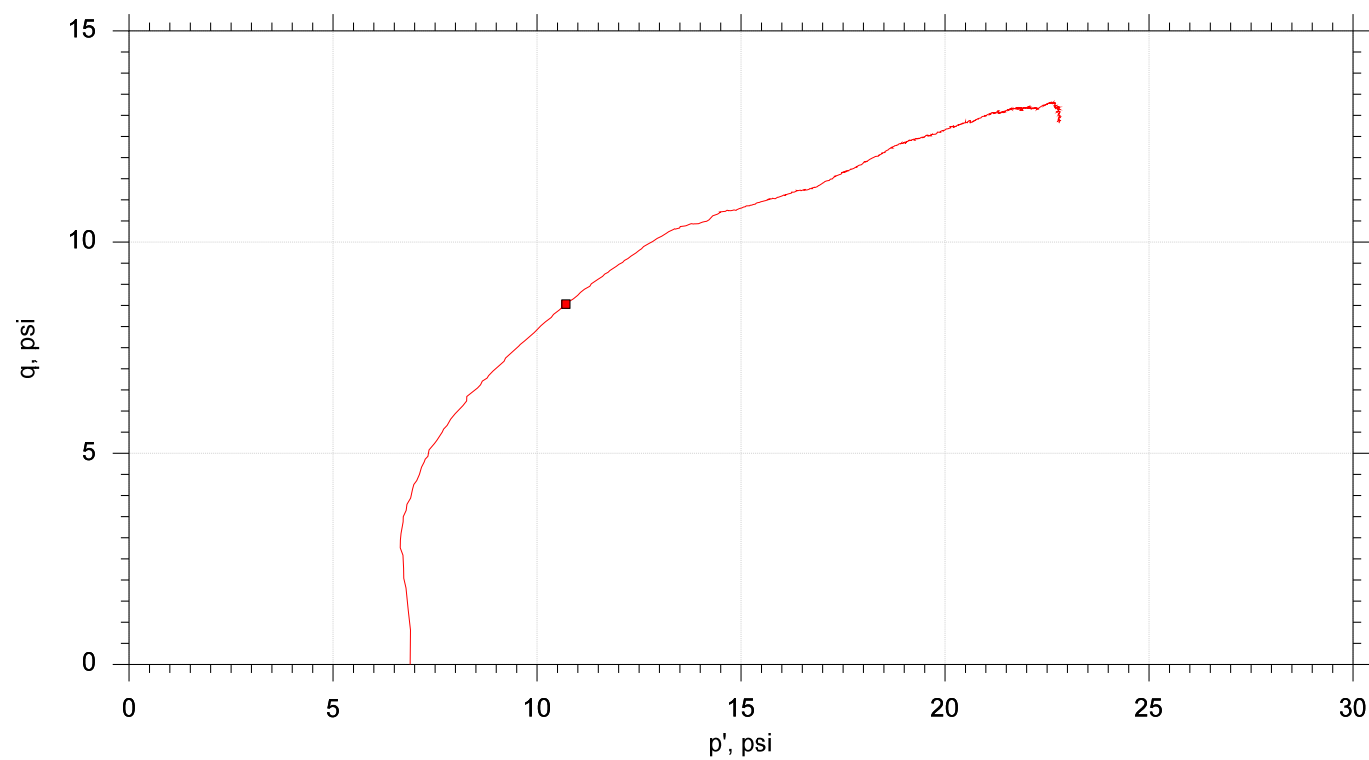
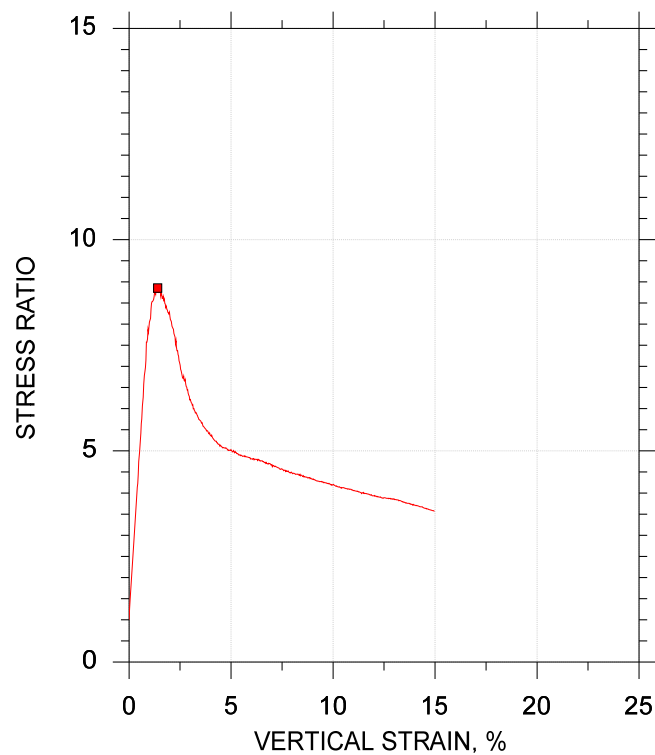
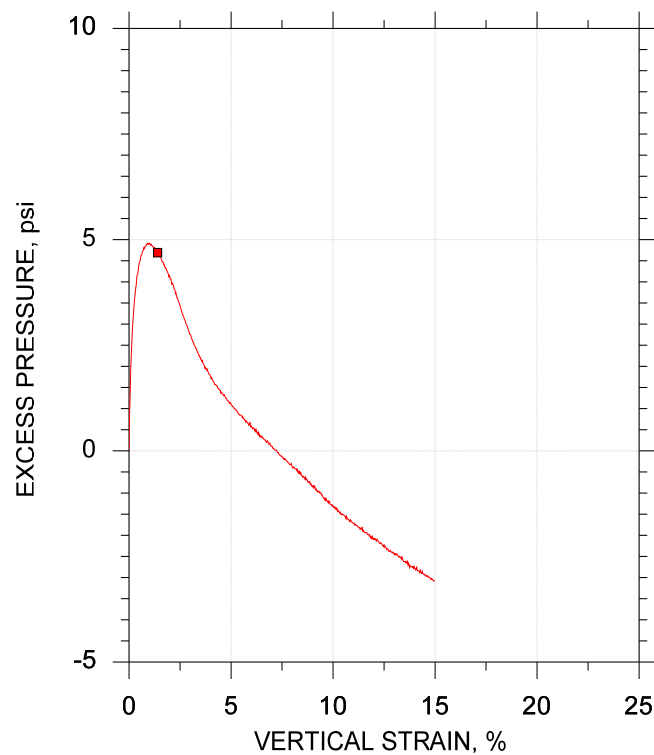
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-11.3 ft		
Test Number		CU-8-1		
Initial	Height, in	6.000		
	Diameter, in	2.860		
	Moisture Content (from Cuttings), %	30.2		
	Dry Density, pcf	92.8		
	Saturation (Wet Method), %	99.8		
	Void Ratio	0.817		
Before Shear	Moisture Content, %	29.6		
	Dry Density, pcf	93.7		
	Cross-sectional Area (Method A), in ²	6.385		
	Saturation, %	100.0		
	Void Ratio	0.799		
	Back Pressure, psi	186.9		
Vertical Effective Consolidation Stress, psi		6.877		
Horizontal Effective Consolidation Stress, psi		6.885		
Vertical Strain after Consolidation, %		0.2189		
Volumetric Strain after Consolidation, %		0.5240		
Time to 50% Consolidation, min		3.240		
Shear Strength, psi		8.534		
Strain at Failure, %		1.40		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		17.07		
Effective Minor Principal Stress at Failure, psi		2.173		
Effective Major Principal Stress at Failure, psi		19.24		
B-Value		0.94		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



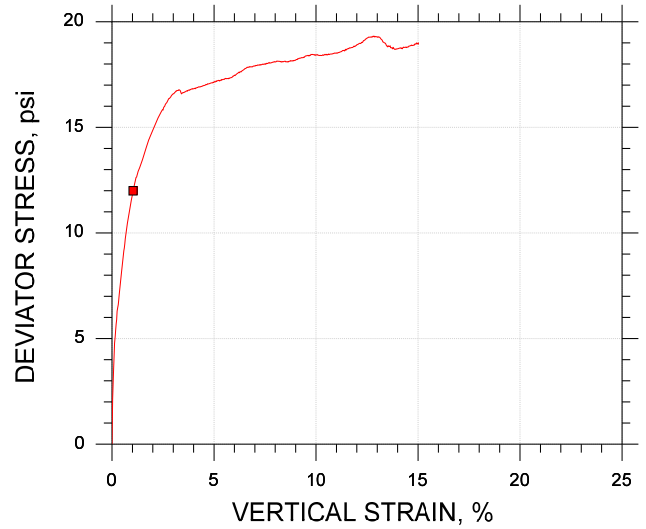
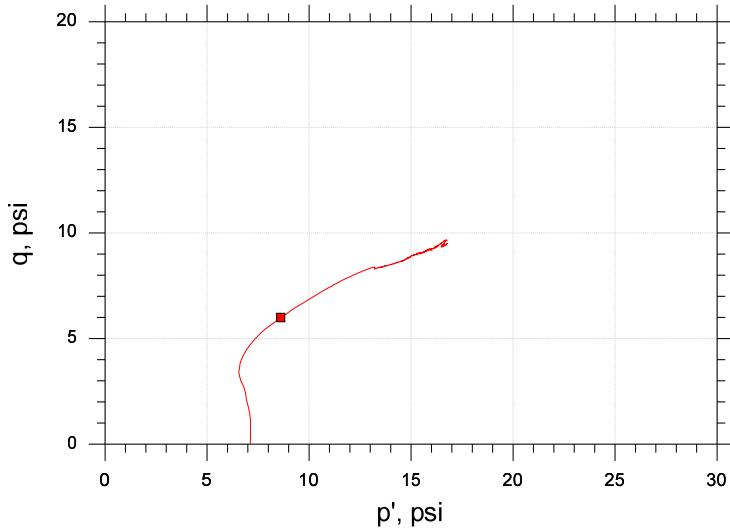
	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	1U	CU-8-1	10-11.3 ft	md/trm	9/28/18	mcm	10/17/18	308853-CU-8-1m.dat

			
	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Sample Type: md	
	Description: Moist, dark olive gray clay		
	Remarks: System LL		



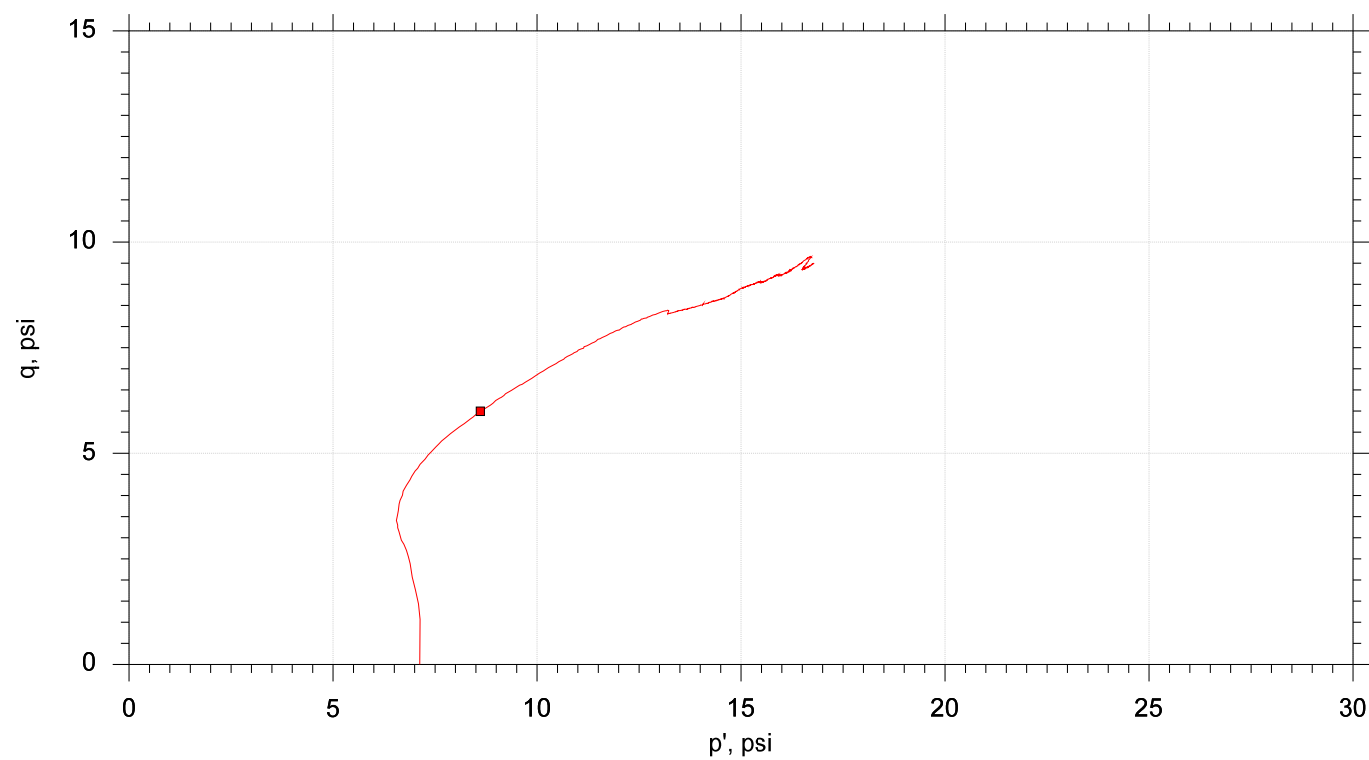
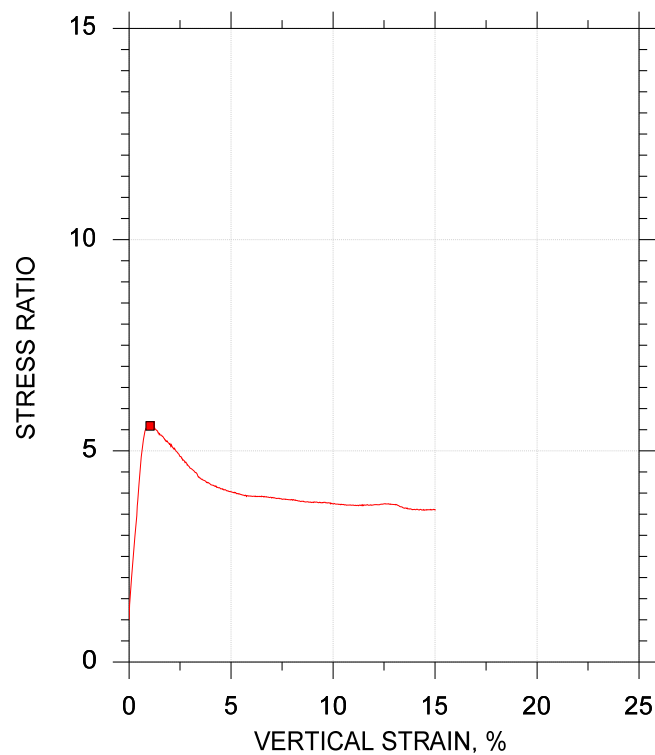
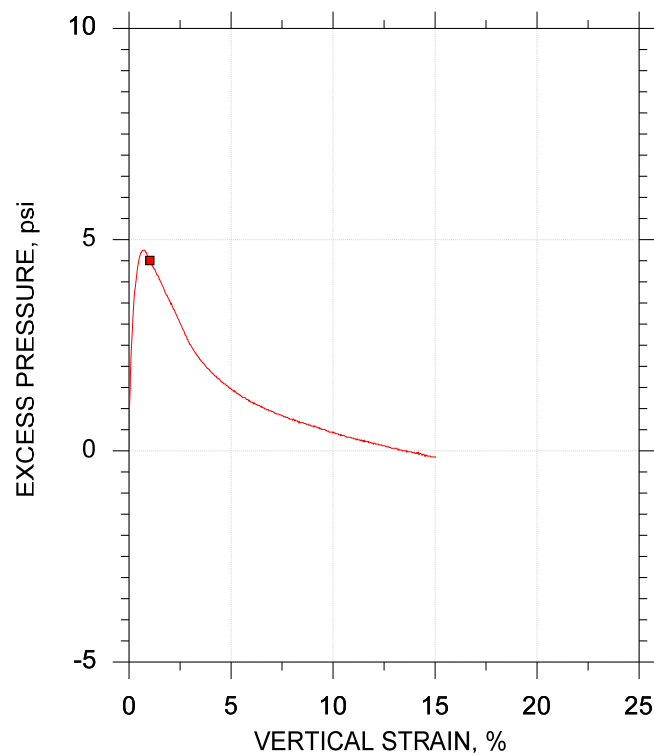
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: md/trm	Checked By: mcm
Boring ID: HB-BE-105	
Preparation: intact	
Description: Moist, olive gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 31	Plastic Limit: 20
Plasticity Index: 11	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-12 ft		
Test Number		CU-6-1		
Initial	Height, in	5.900		
	Diameter, in	2.830		
	Moisture Content (from Cuttings), %	30.5		
	Dry Density, pcf	90.1		
	Saturation (Wet Method), %	94.4		
	Void Ratio	0.871		
Before Shear	Moisture Content, %	31.3		
	Dry Density, pcf	91.4		
	Cross-sectional Area (Method A), in ²	6.224		
	Saturation, %	100.0		
	Void Ratio	0.844		
	Back Pressure, psi	121.0		
Vertical Effective Consolidation Stress, psi		7.118		
Horizontal Effective Consolidation Stress, psi		7.120		
Vertical Strain after Consolidation, %		0.1817		
Volumetric Strain after Consolidation, %		0.8334		
Time to 50% Consolidation, min		6.250		
Shear Strength, psi		5.998		
Strain at Failure, %		1.03		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		12.00		
Effective Minor Principal Stress at Failure, psi		2.613		
Effective Major Principal Stress at Failure, psi		14.61		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System KK				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



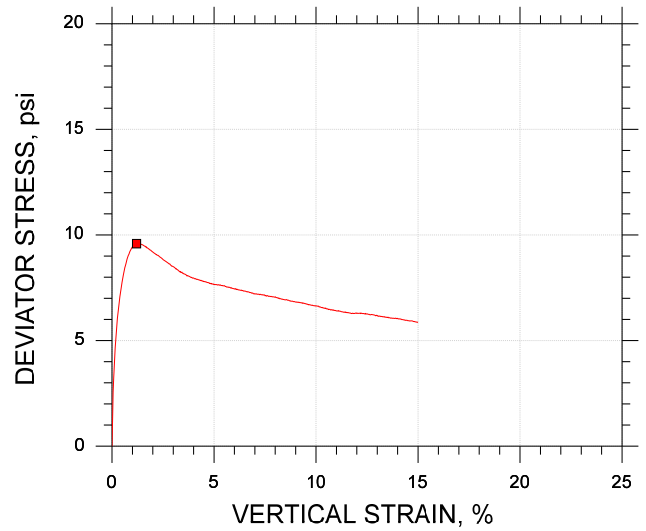
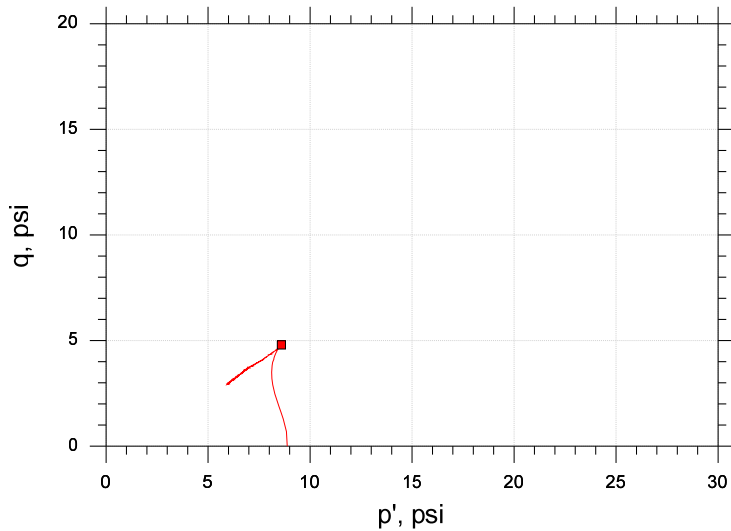
	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	1U	CU-6-1	10-12 ft	md/trm	9/28/2018	mcm	10/17/18	308853-CU-6-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Sample Type: intact	
	Description: Moist, olive gray clay		
	Remarks: System KK		



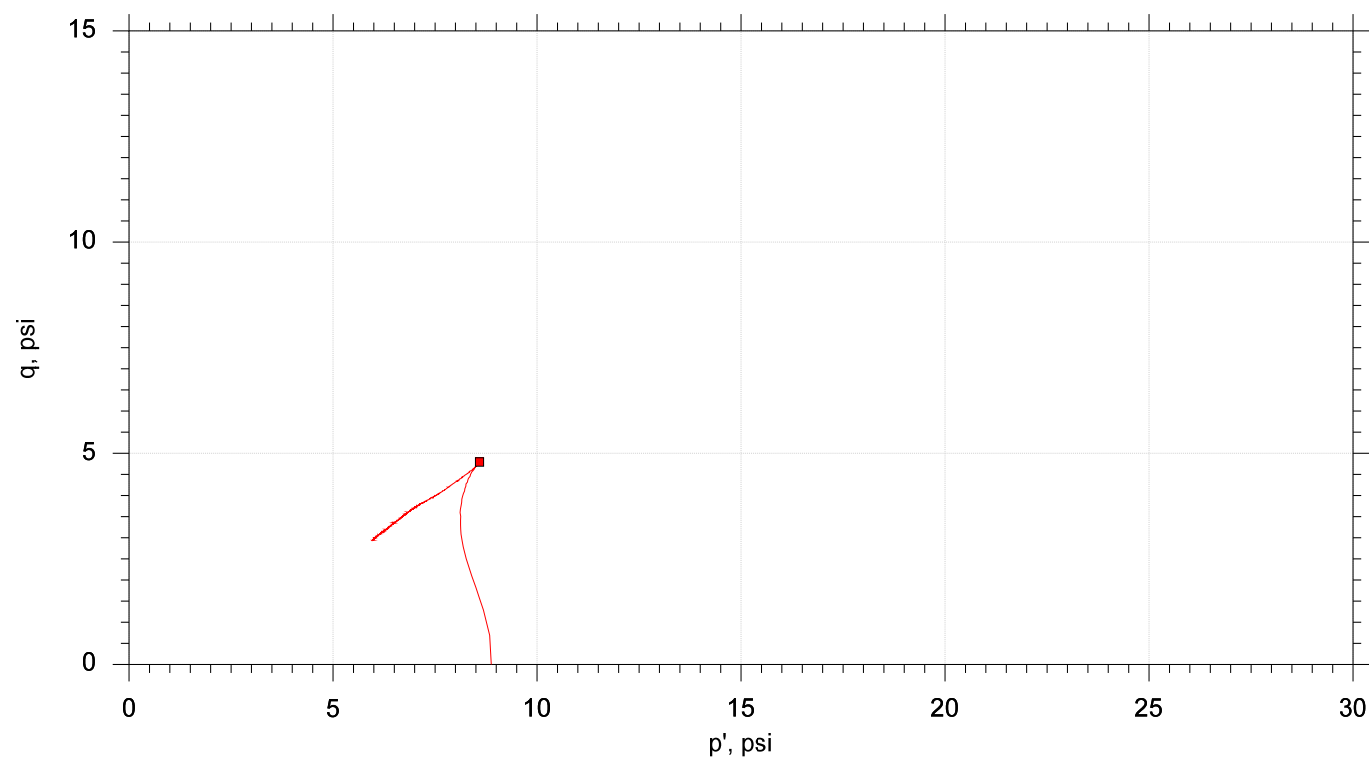
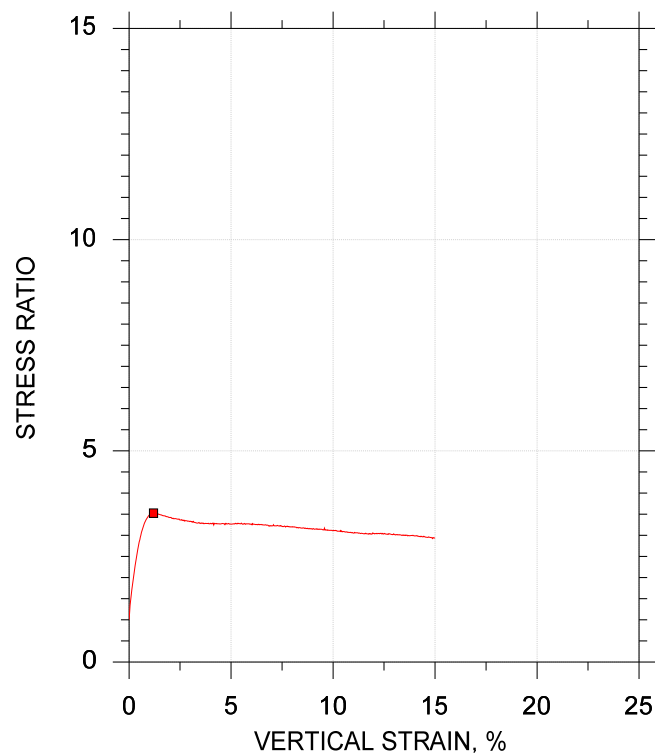
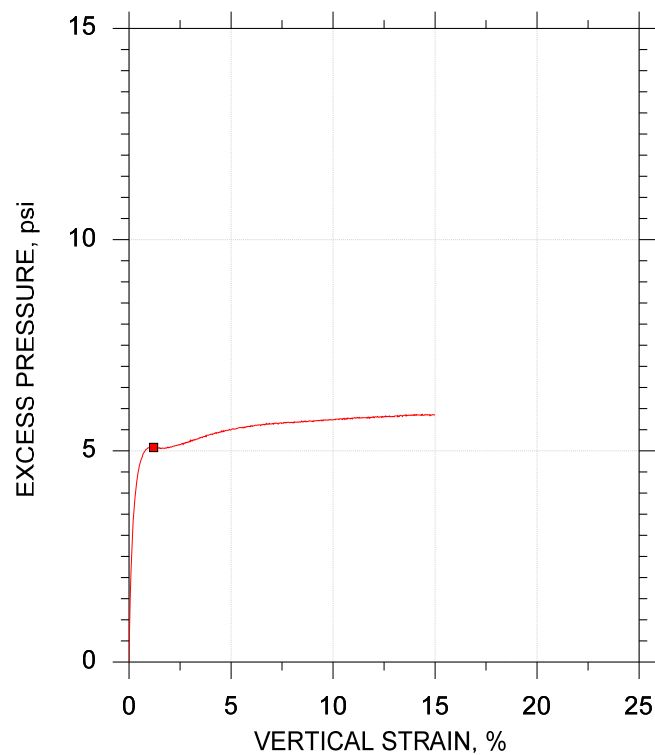
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and eddington, ME	
Project Number: GTX-308853	
Tested By: md/trm	Checked By: mcm
Boring ID: HB-BE-105	
Preparation: Intact	
Description: Wet, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 18
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		2U		
Depth, ft		14-16 ft		
Test Number		CU-7-1		
Initial	Height, in	6.050		
	Diameter, in	2.860		
	Moisture Content (from Cuttings), %	34.7		
	Dry Density, pcf	86.8		
	Saturation (Wet Method), %	99.6		
	Void Ratio	0.942		
Before Shear	Moisture Content, %	34.1		
	Dry Density, pcf	87.8		
	Cross-sectional Area (Method A), in ²	6.371		
	Saturation, %	100.0		
	Void Ratio	0.920		
	Back Pressure, psi	169.0		
Vertical Effective Consolidation Stress, psi		8.863		
Horizontal Effective Consolidation Stress, psi		8.878		
Vertical Strain after Consolidation, %		0.2852		
Volumetric Strain after Consolidation, %		1.047		
Time to 50% Consolidation, min		27.56		
Shear Strength, psi		4.794		
Strain at Failure, %		1.20		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		9.588		
Effective Minor Principal Stress at Failure, psi		3.797		
Effective Major Principal Stress at Failure, psi		13.39		
B-Value		0.91		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System R				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	2U	CU-7-1	14-16 ft	md/trm	9/28/2018	mcm	10/17/18	308853-CU-7-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Sample Type: Intact	
	Description: Wet, dark gray clay		
	Remarks: System R		



Client: Haley & Aldrich, Inc.

Project Name: Rt 9/I-395 Connector

Project Location: Brewer and Eddington, ME

Project Number: GTX-308853

Tested By: md/trm

Checked By: mcm

Boring ID: HB-BE-107A

Preparation: intact

Description: Wet, dark gray clay

Classification: ---

Group Symbol: ---

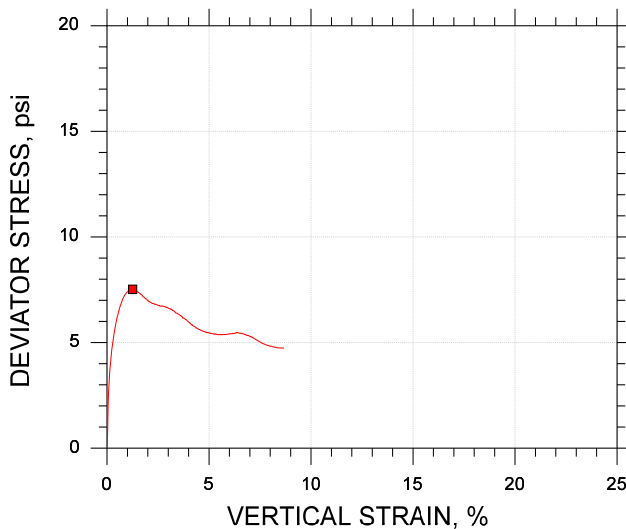
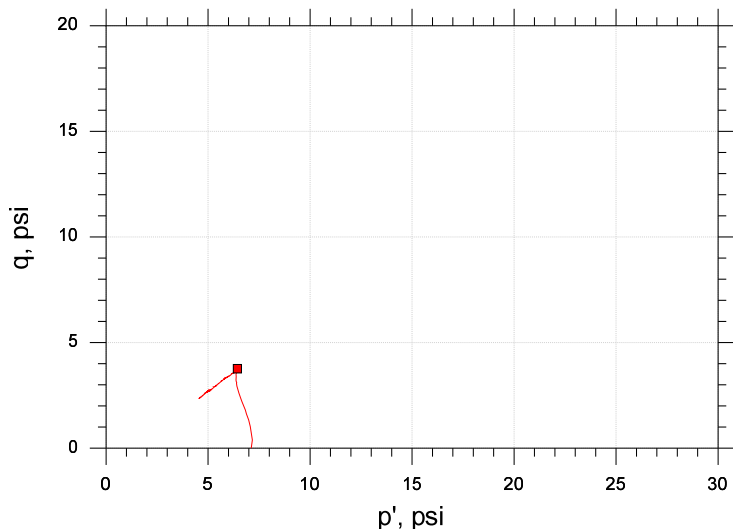
Liquid Limit: 30

Plastic Limit: 17

Plasticity Index: 13

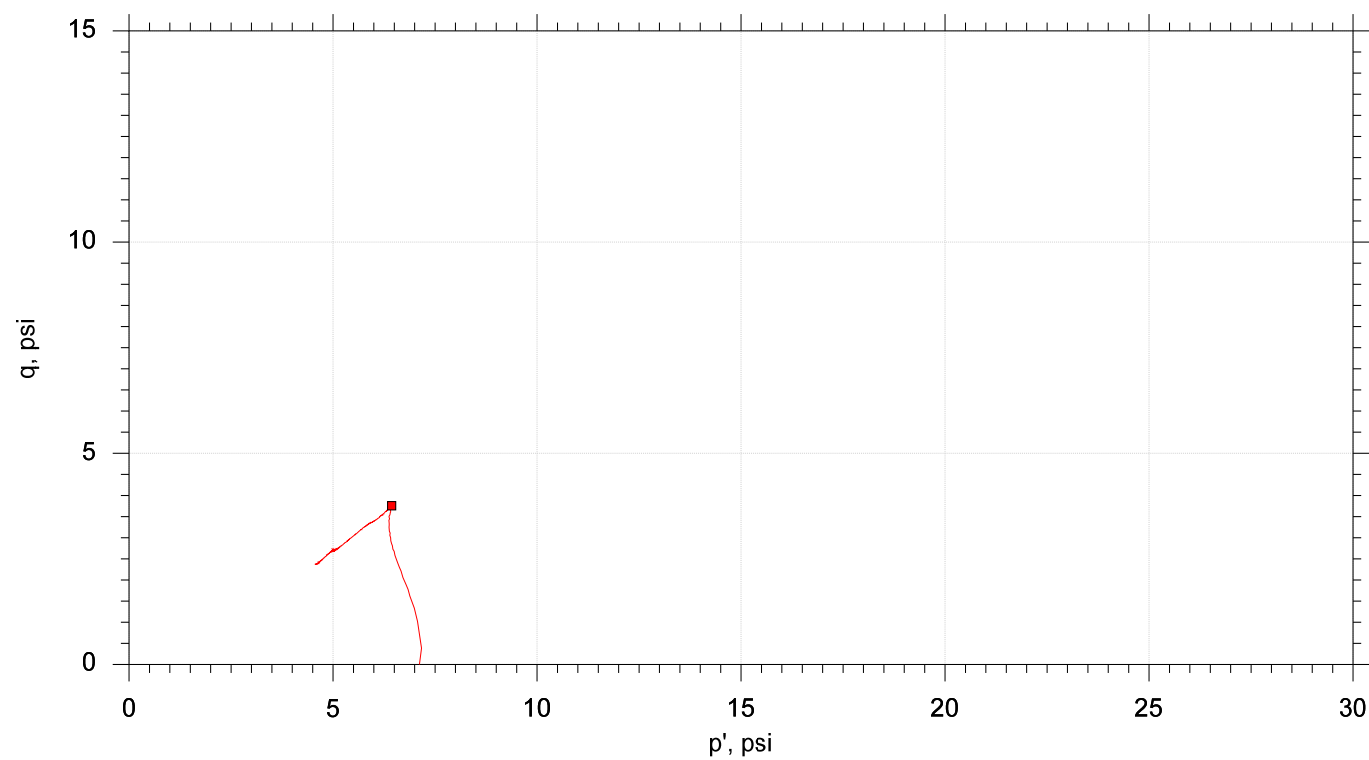
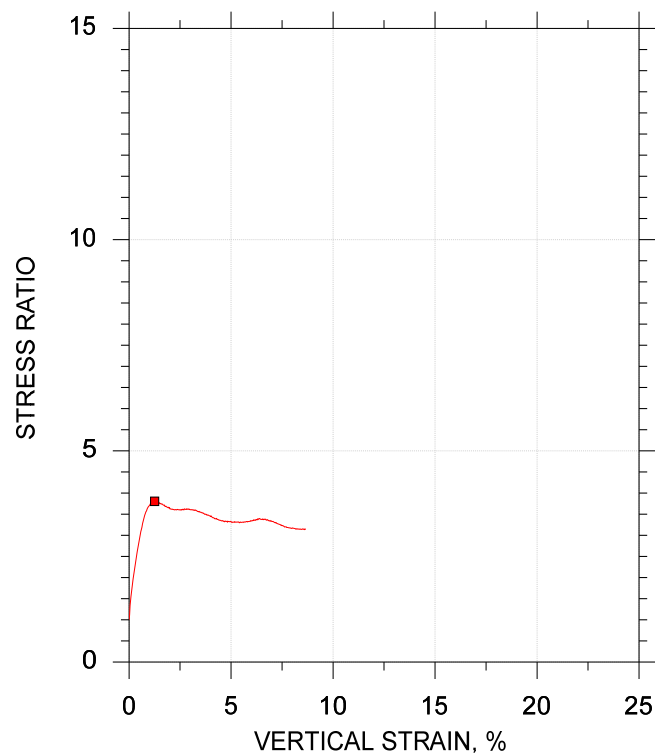
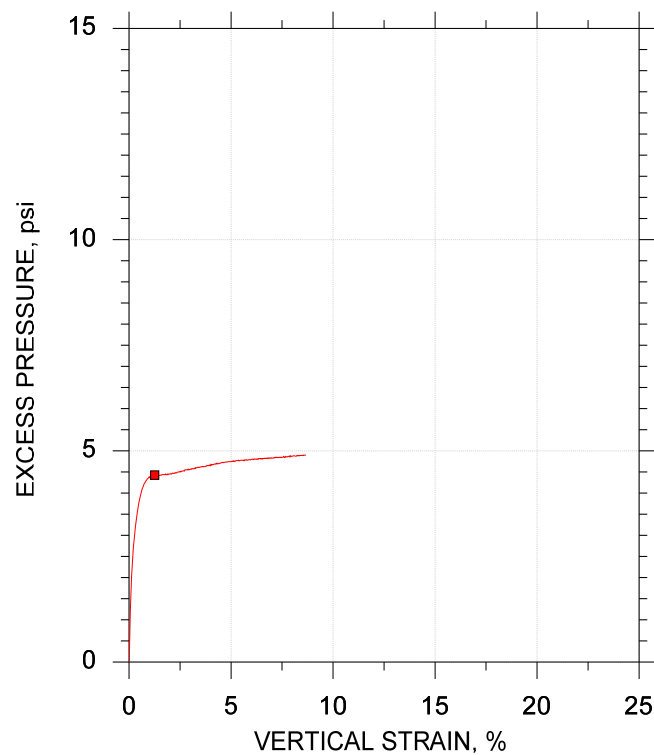
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-12 ft		
Test Number		CU-4-1		
Initial	Height, in	6.100		
	Diameter, in	2.860		
	Moisture Content (from Cuttings), %	34.7		
	Dry Density, pcf	86.6		
	Saturation (Wet Method), %	99.0		
	Void Ratio	0.946		
Before Shear	Moisture Content, %	33.9		
	Dry Density, pcf	88.0		
	Cross-sectional Area (Method A), in ²	6.353		
	Saturation, %	100.0		
	Void Ratio	0.915		
	Back Pressure, psi	170.9		
Vertical Effective Consolidation Stress, psi		7.094		
Horizontal Effective Consolidation Stress, psi		7.114		
Vertical Strain after Consolidation, %		0.3695		
Volumetric Strain after Consolidation, %		1.246		
Time to 50% Consolidation, min		56.25		
Shear Strength, psi		3.759		
Strain at Failure, %		1.25		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		7.518		
Effective Minor Principal Stress at Failure, psi		2.678		
Effective Major Principal Stress at Failure, psi		10.20		
B-Value		0.94		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System Y				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



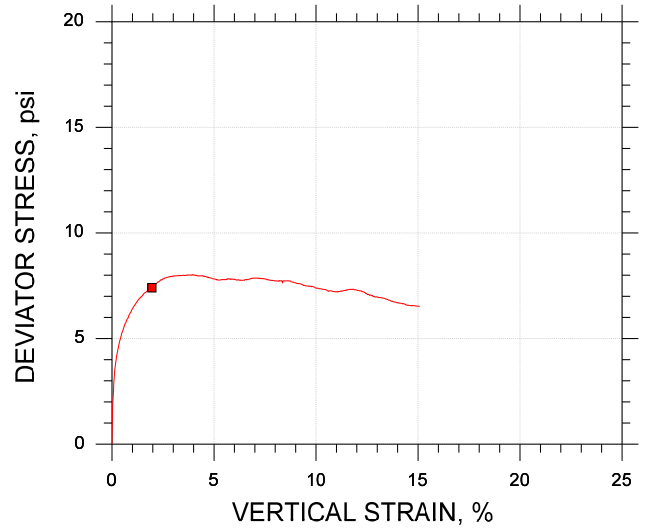
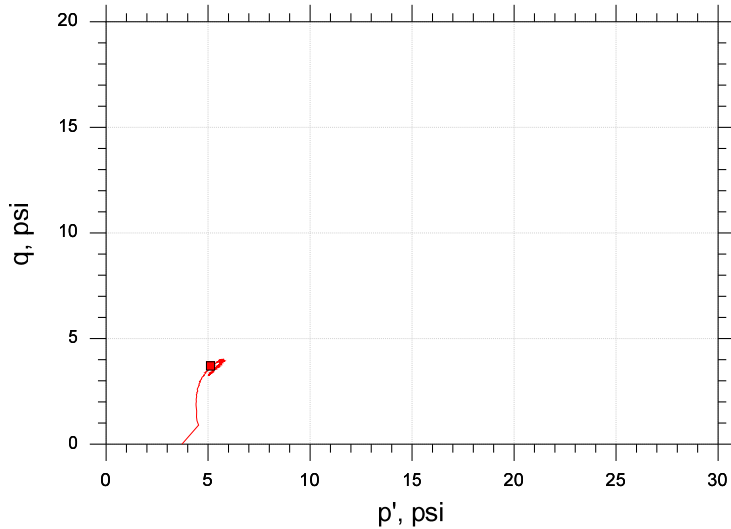
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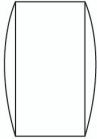
			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Sample Type: intact	
	Description: Wet, dark gray clay		
	Remarks: System Y		



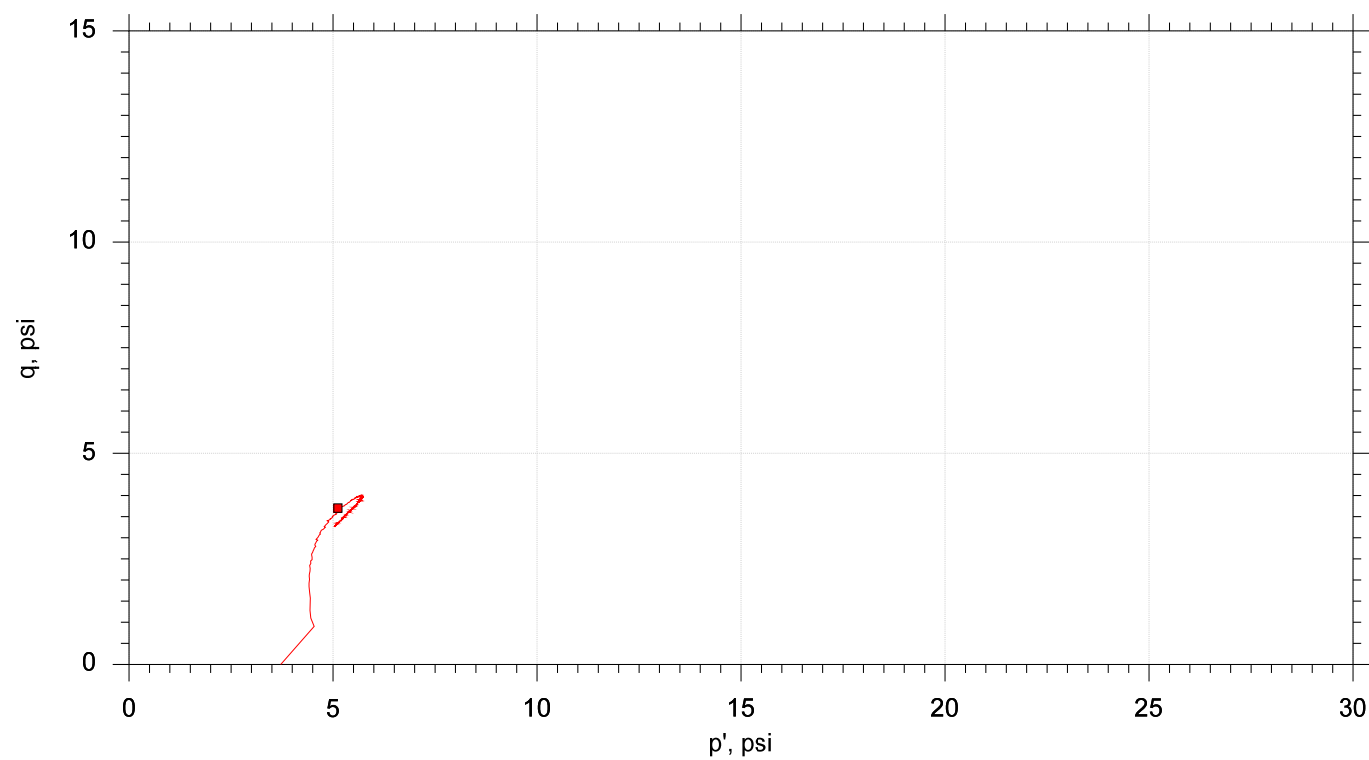
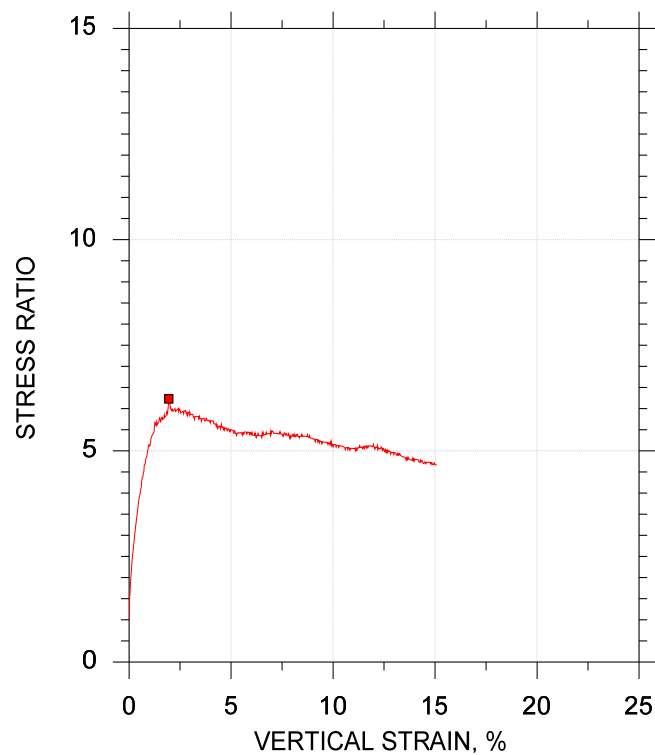
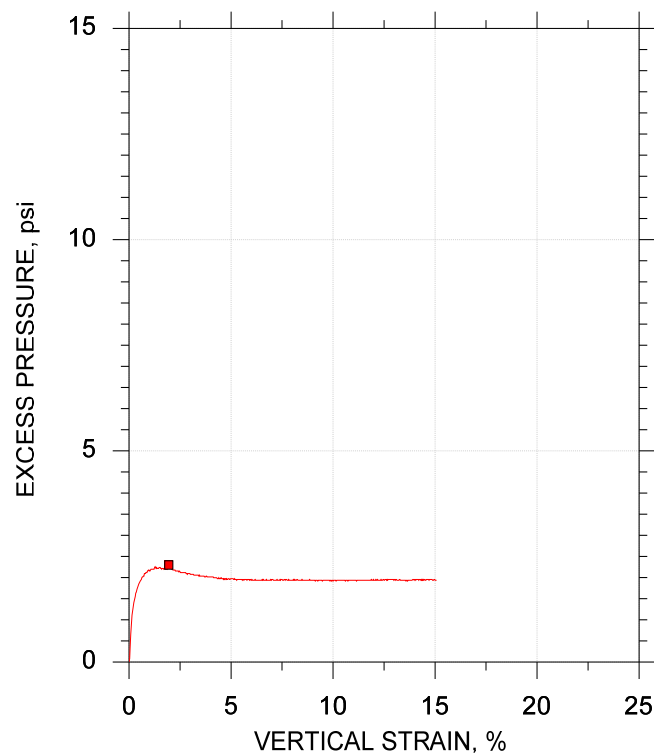
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-108	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 19
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		12-14 ft		
Test Number		CU-9-1		
Initial	Height, in	4.430		
	Diameter, in	1.950		
	Moisture Content (from Cuttings), %	35.6		
	Dry Density, pcf	85.2		
	Saturation (Wet Method), %	98.2		
	Void Ratio	0.979		
Before Shear	Moisture Content, %	36.2		
	Dry Density, pcf	85.2		
	Cross-sectional Area (Method A), in ²	2.981		
	Saturation, %	100.0		
	Void Ratio	0.979		
	Back Pressure, psi	170.4		
Vertical Effective Consolidation Stress, psi		3.720		
Horizontal Effective Consolidation Stress, psi		3.716		
Vertical Strain after Consolidation, %		0.01239		
Volumetric Strain after Consolidation, %		0.5079		
Time to 50% Consolidation, min		70.40		
Shear Strength, psi		3.704		
Strain at Failure, %		1.95		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		7.407		
Effective Minor Principal Stress at Failure, psi		1.416		
Effective Major Principal Stress at Failure, psi		8.823		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



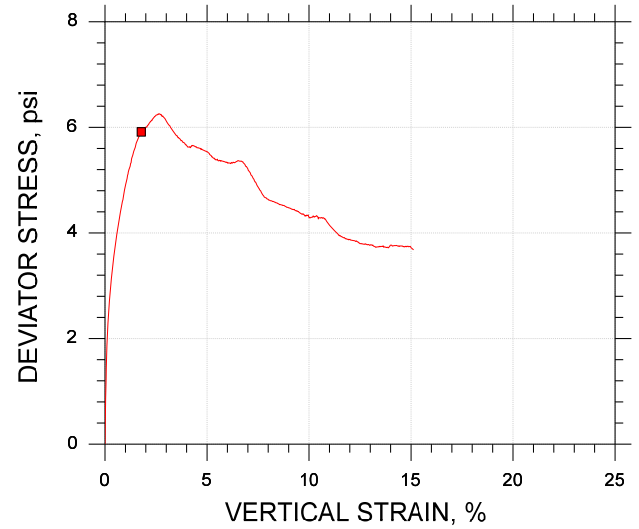
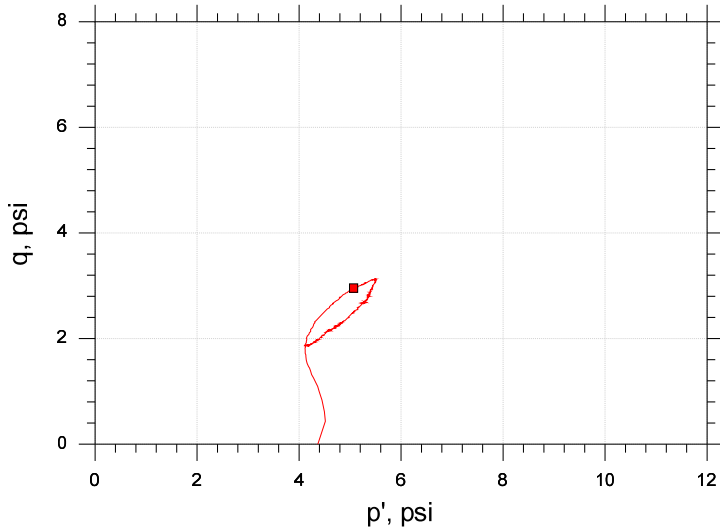
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■	1U	CU-9-1	12-14 ft	trm	7/23/19	mcm	8/1/19	308853-CU-9-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System K		



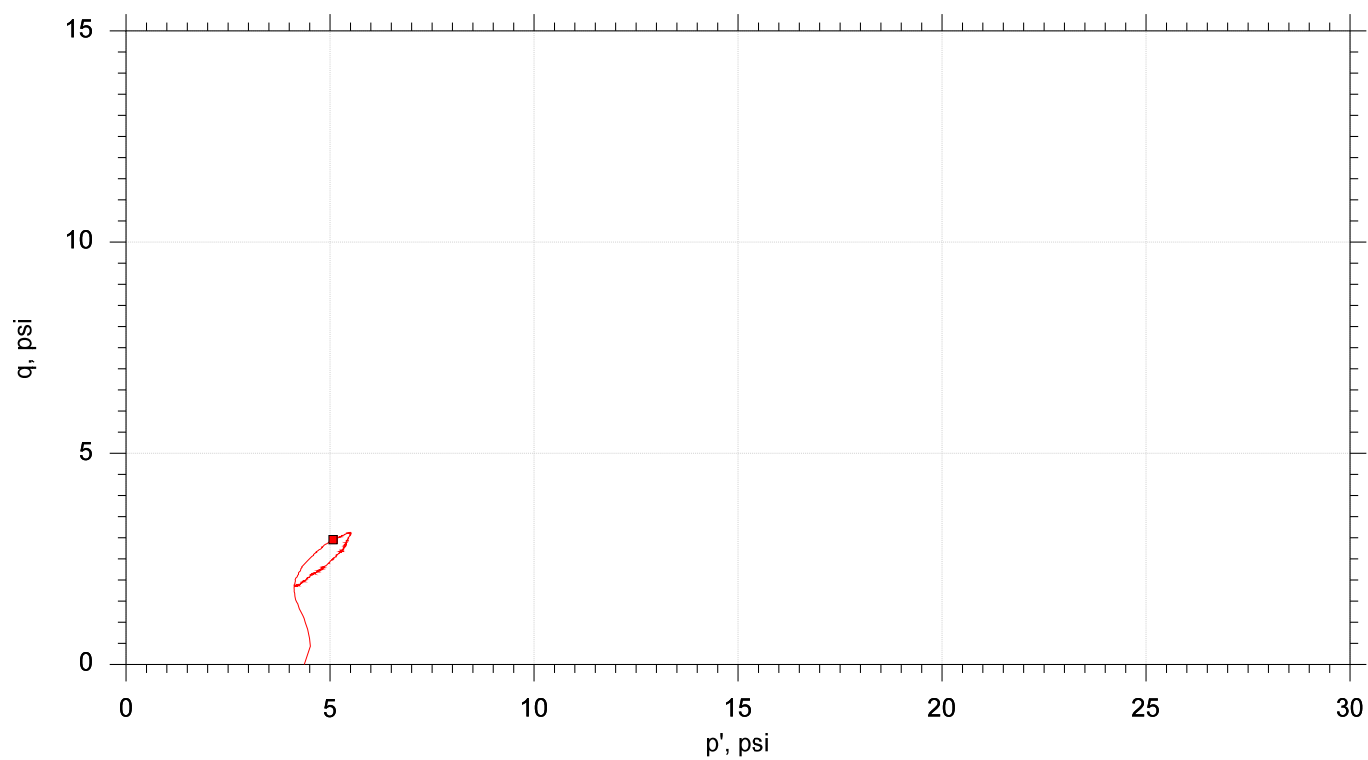
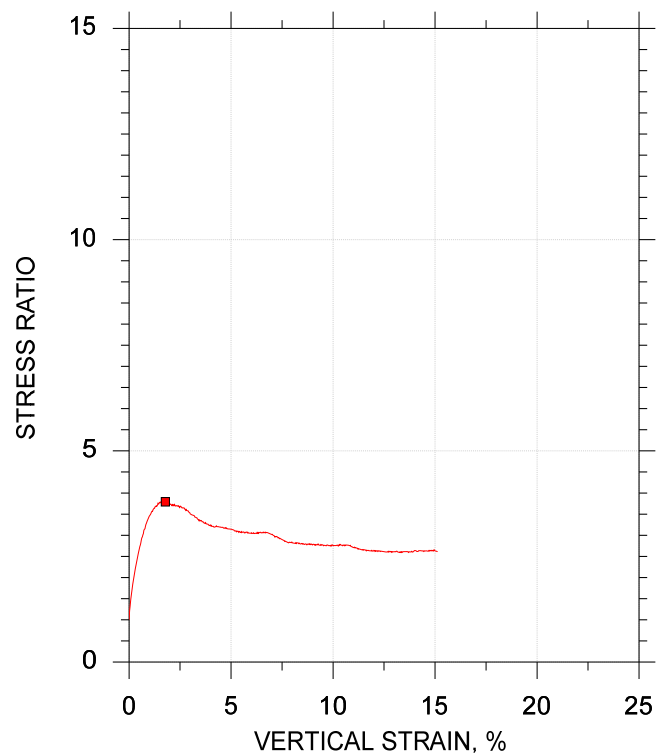
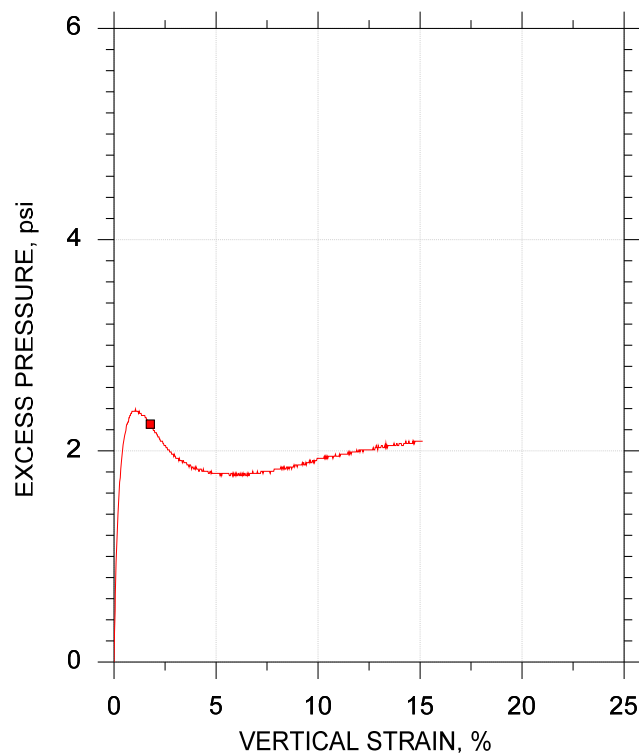
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME.	
Project Number: GTX-308853	
Tested By: md	Checked By: mcm
Boring ID: HB-BE-111	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 19
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	1U			
Depth, ft	10-12 ft			
Test Number	CU-16-1			
Initial	Height, in	4.900		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	34.1		
	Dry Density, pcf	87.6		
	Saturation (Wet Method), %	99.6		
	Void Ratio	0.923		
Before Shear	Moisture Content, %	32.0		
	Dry Density, pcf	90.4		
	Cross-sectional Area (Method A), in ²	3.171		
	Saturation, %	100.0		
	Void Ratio	0.865		
	Back Pressure, psi	162.7		
Vertical Effective Consolidation Stress, psi		4.355		
Horizontal Effective Consolidation Stress, psi		4.368		
Vertical Strain after Consolidation, %		0.1864		
Volumetric Strain after Consolidation, %		0.5215		
Time to 50% Consolidation, min		77.44		
Shear Strength, psi		2.956		
Strain at Failure, %		1.78		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.912		
Effective Minor Principal Stress at Failure, psi		2.114		
Effective Major Principal Stress at Failure, psi		8.027		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System KK				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



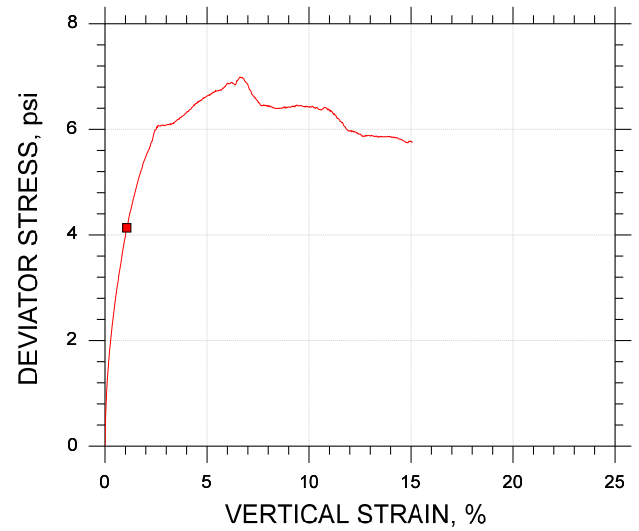
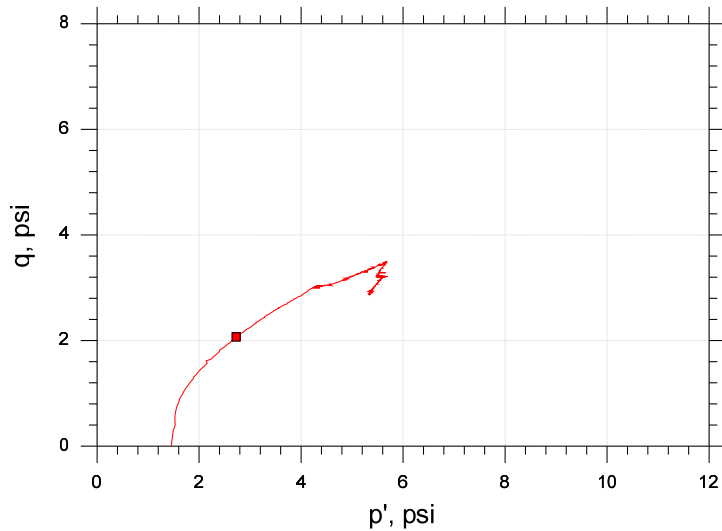
	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	1U	CU-16-1	10-12 ft	md	07/24/19	mcm	8/2/19	308853-CU-16-1m.dat


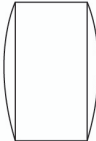
			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME.	Project No.: GTX-308853
	Boring No.: HB-BE-111	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System KK		



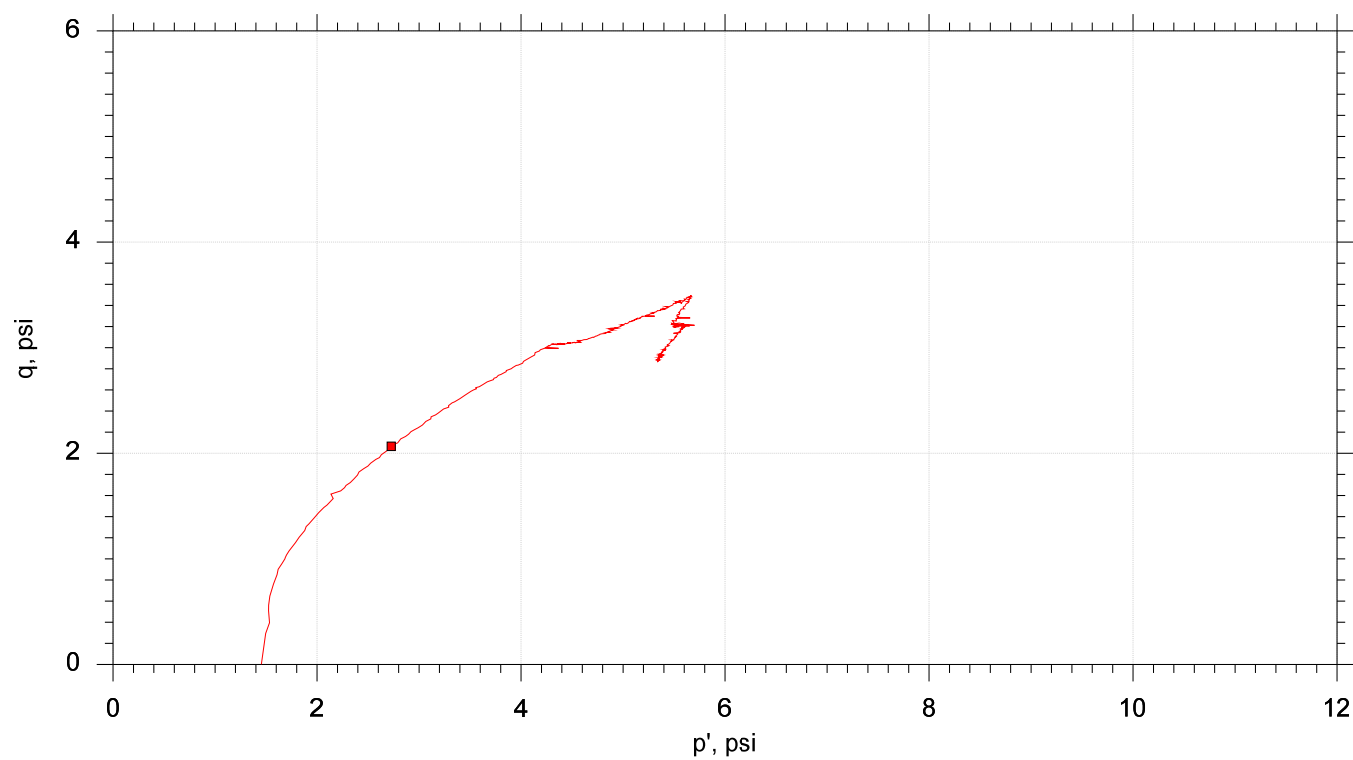
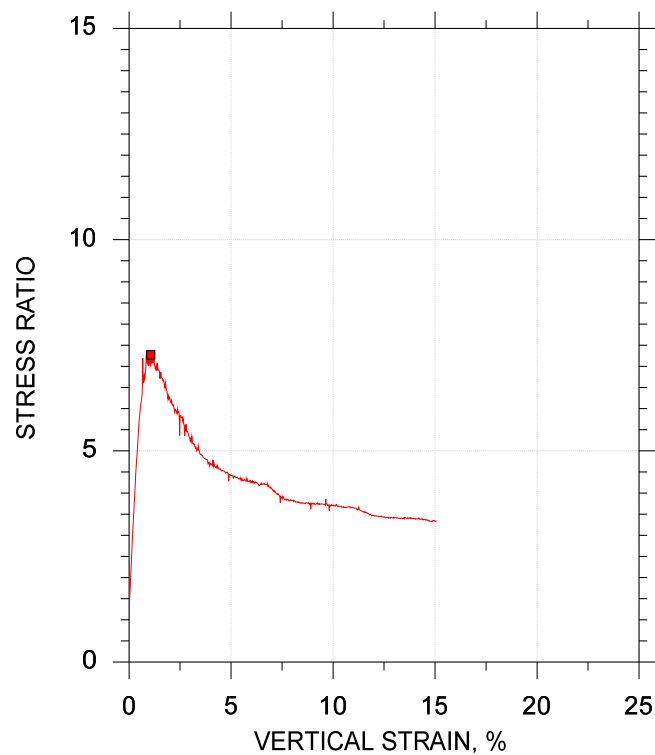
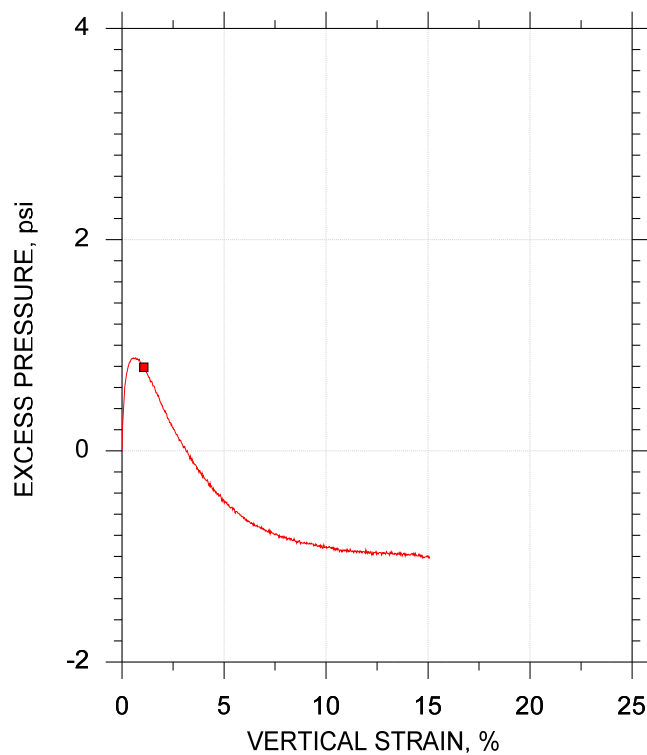
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: md	Checked By: mcm
Boring ID: HB-BE-135	
Preparation: Intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 18
Plasticity Index: 19	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol				
Sample ID		1U		
Depth, ft		5-7 ft		
Test Number		CU-18-1		
Initial	Height, in	4.780		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	36.0		
	Dry Density, pcf	84.9		
	Saturation (Wet Method), %	98.7		
	Void Ratio	0.986		
Before Shear	Moisture Content, %	35.7		
	Dry Density, pcf	85.9		
	Cross-sectional Area (Method A), in²	3.215		
	Saturation, %	100.0		
	Void Ratio	0.963		
	Back Pressure, psi	162.9		
Vertical Effective Consolidation Stress, psi		1.443		
Horizontal Effective Consolidation Stress, psi		1.452		
Vertical Strain after Consolidation, %		0.1467		
Volumetric Strain after Consolidation, %		0.2126		
Time to 50% Consolidation, min		20.25		
Shear Strength, psi		2.067		
Strain at Failure, %		1.07		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		4.134		
Effective Minor Principal Stress at Failure, psi		0.6592		
Effective Major Principal Stress at Failure, psi		4.793		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



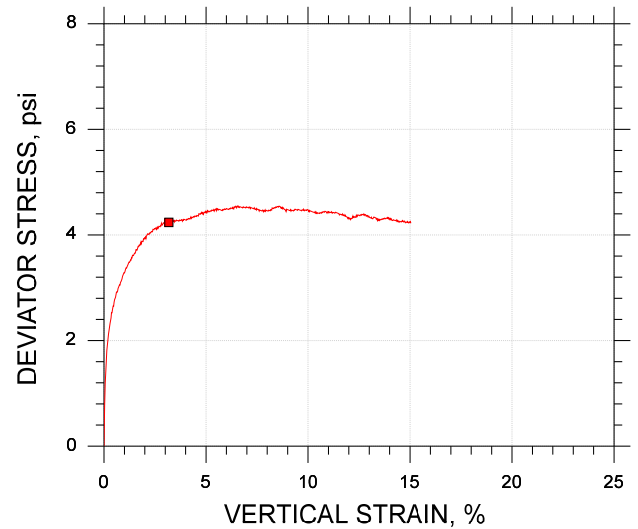
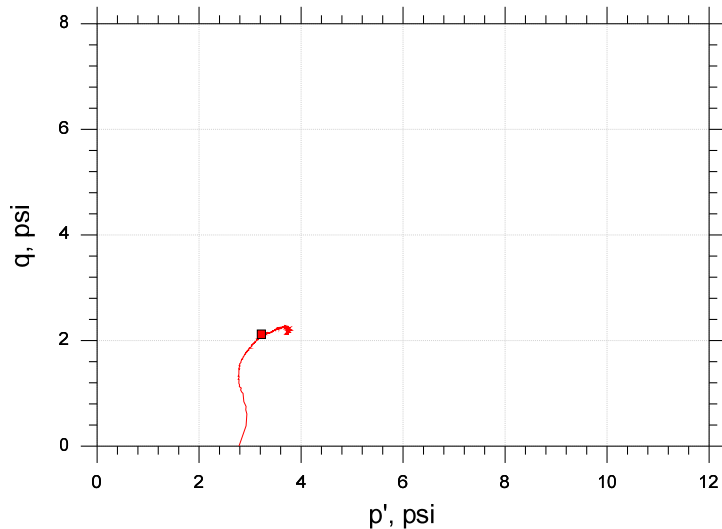
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■	1U	CU-18-1	5-7 ft	md	07/24/19	mcm	8/2/19	308853-CU-18-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Sample Type: Intact	
	Description: Moist, dark gray clay		
	Remarks: System JJ		



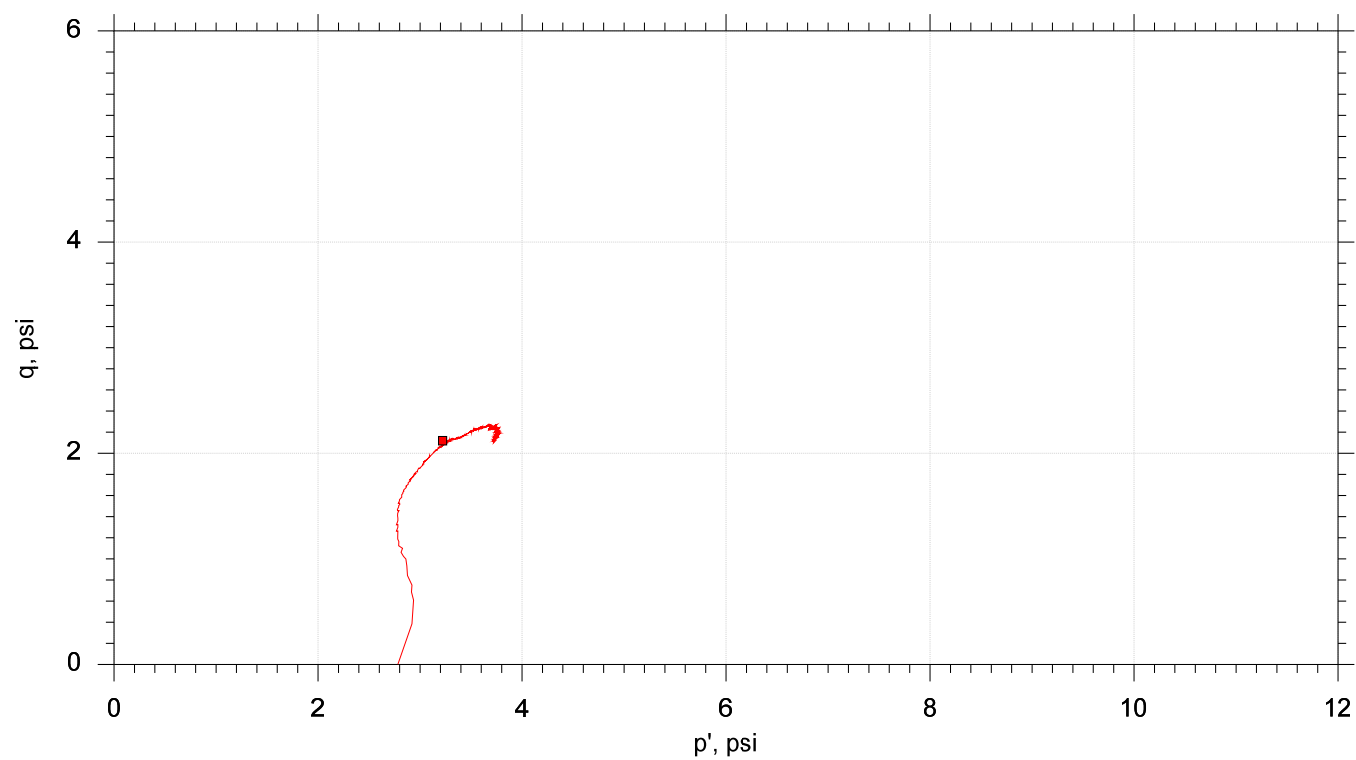
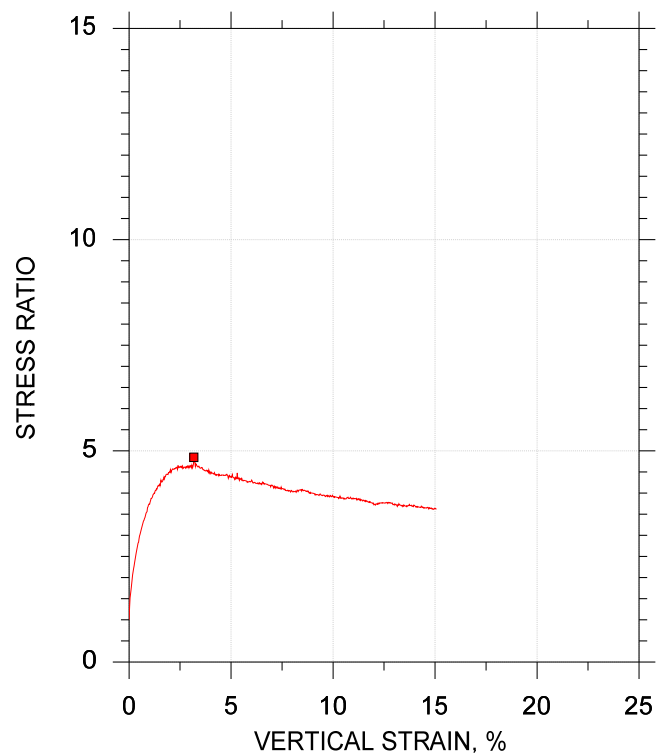
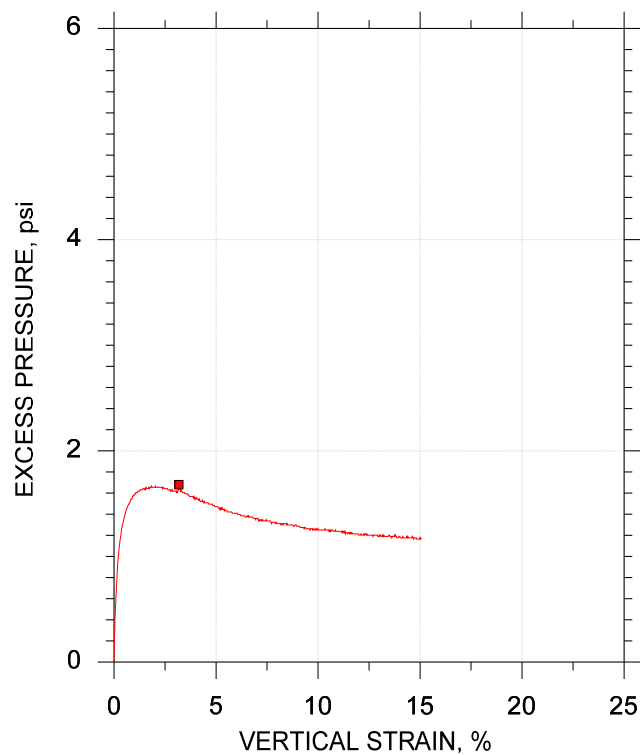
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-136	
Preparation: Intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 38	Plastic Limit: 19
Plasticity Index: 19	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		10-12 ft		
Test Number		CU-14-1		
Initial	Height, in	4.500		
	Diameter, in	1.940		
	Moisture Content (from Cuttings), %	39.0		
	Dry Density, pcf	81.2		
	Saturation (Wet Method), %	97.8		
	Void Ratio	1.08		
Before Shear	Moisture Content, %	39.0		
	Dry Density, pcf	82.1		
	Cross-sectional Area (Method A), in ²	2.930		
	Saturation, %	100.0		
	Void Ratio	1.05		
	Back Pressure, psi	162.9		
Vertical Effective Consolidation Stress, psi		2.760		
Horizontal Effective Consolidation Stress, psi		2.781		
Vertical Strain after Consolidation, %		0.2848		
Volumetric Strain after Consolidation, %		1.138		
Time to 50% Consolidation, min		96.04		
Shear Strength, psi		2.119		
Strain at Failure, %		3.18		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		4.238		
Effective Minor Principal Stress at Failure, psi		1.101		
Effective Major Principal Stress at Failure, psi		5.339		
B-Value		0.94		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



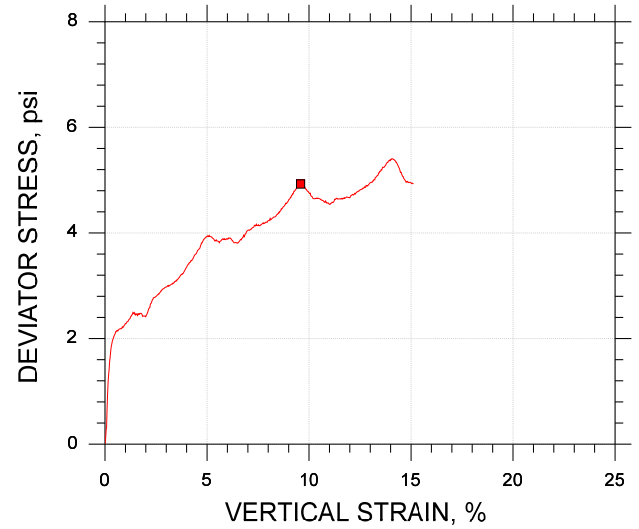
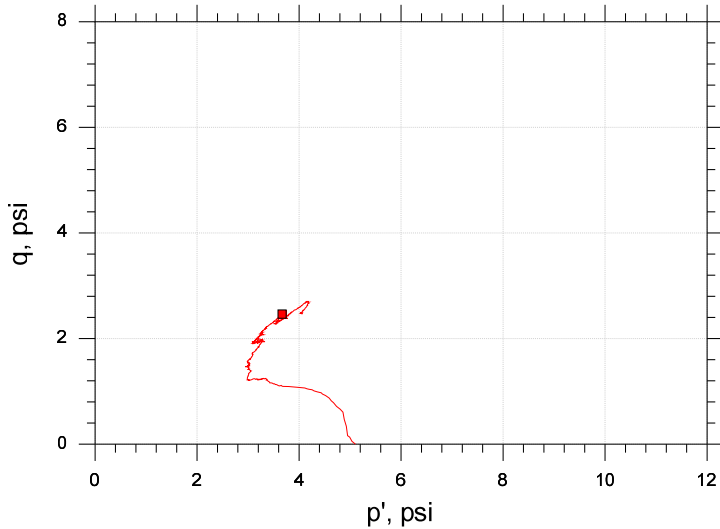
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■	1U	CU-14-1	10-12 ft	trm	7/24/19	mcm	6/18/19	308853-CU-14-1m.dat


			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-136	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System PP		



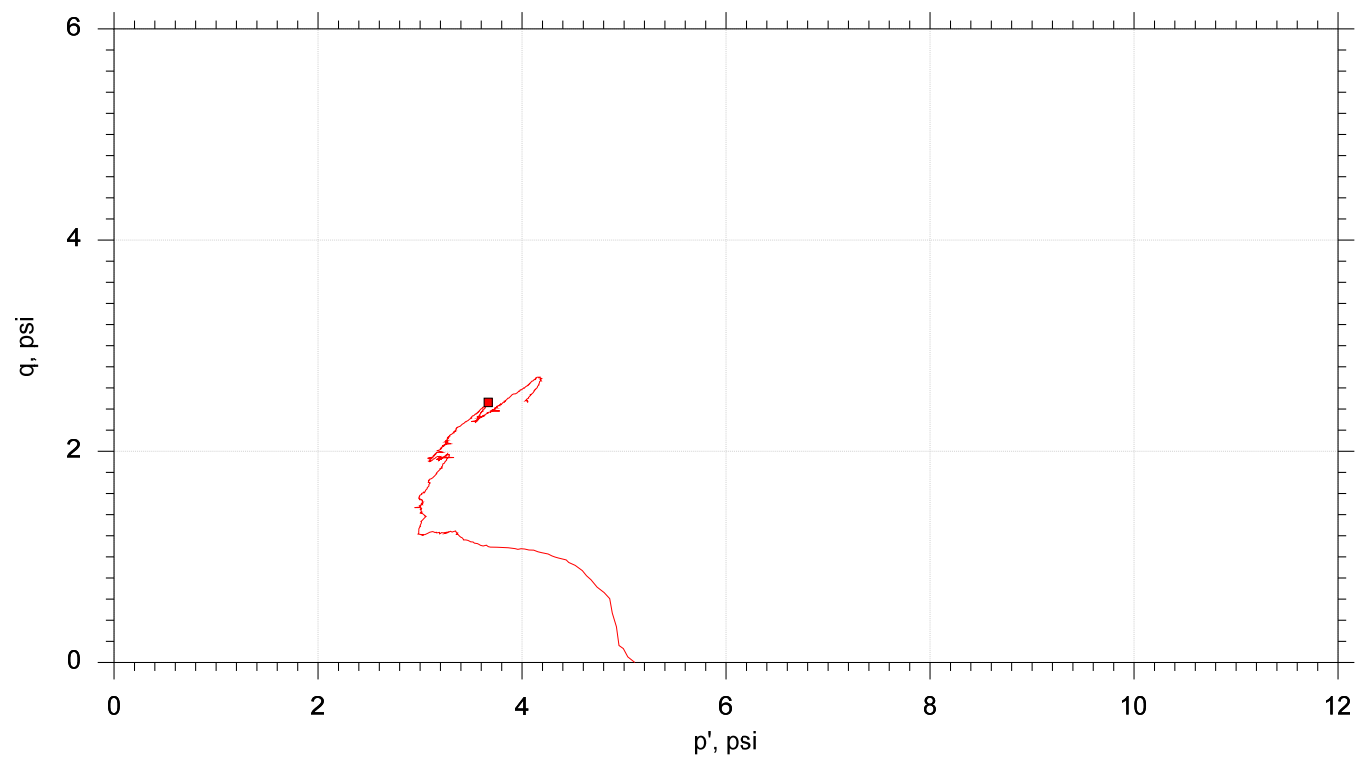
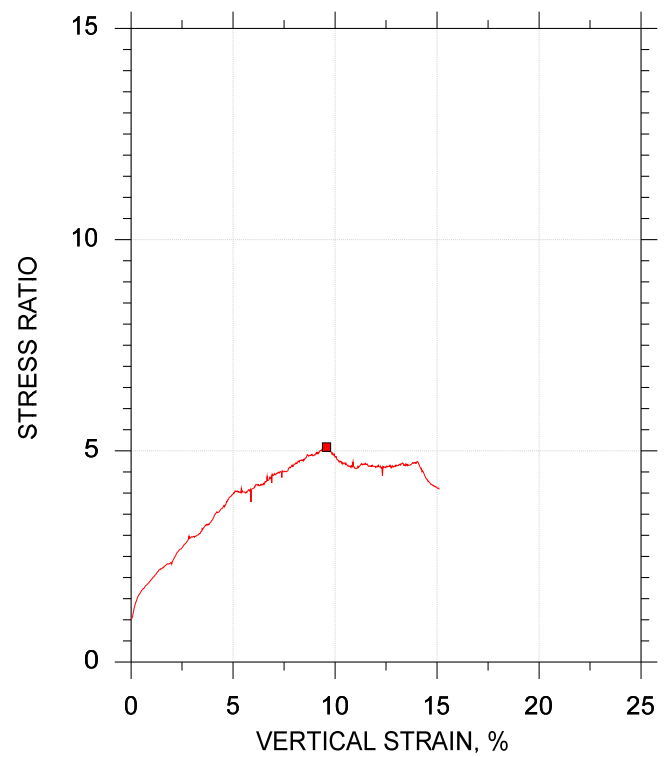
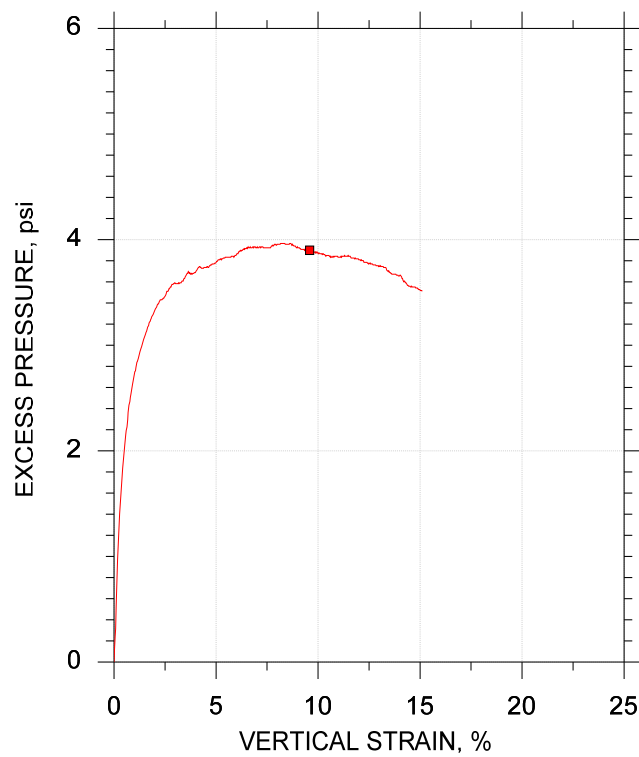
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-137	
Preparation: Intact	
Description: Wet, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 32	Plastic Limit: 19
Plasticity Index: 13	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1-U		
Depth, ft		20-22 ft		
Test Number		CU-10-1B		
Initial	Height, in	4.520		
	Diameter, in	1.950		
	Moisture Content (from Cuttings), %	41.0		
	Dry Density, pcf	79.8		
	Saturation (Wet Method), %	99.5		
	Void Ratio	1.11		
Before Shear	Moisture Content, %	38.5		
	Dry Density, pcf	82.6		
	Cross-sectional Area (Method A), in²	2.906		
	Saturation, %	100.0		
	Void Ratio	1.04		
	Back Pressure, psi	83.00		
Vertical Effective Consolidation Stress, psi		4.959		
Horizontal Effective Consolidation Stress, psi		5.104		
Vertical Strain after Consolidation, %		1.706		
Volumetric Strain after Consolidation, %		6.240		
Time to 50% Consolidation, min		361.0		
Shear Strength, psi		2.463		
Strain at Failure, %		9.58		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		4.927		
Effective Minor Principal Stress at Failure, psi		1.205		
Effective Major Principal Stress at Failure, psi		6.131		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



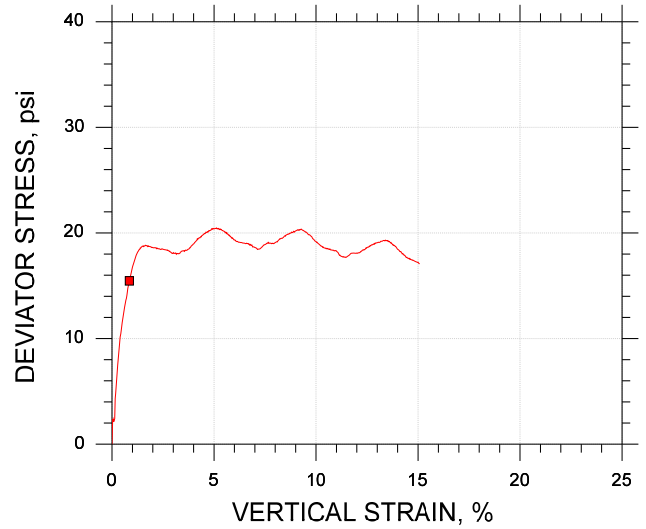
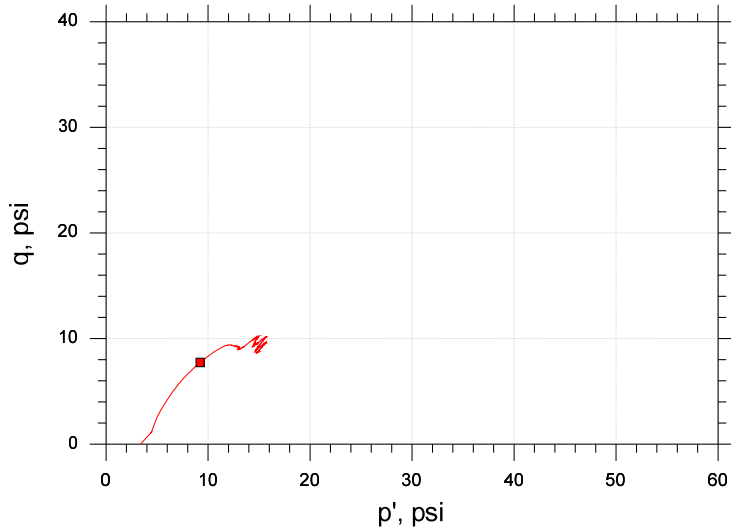
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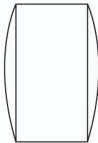
			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Sample Type: Intact	
	Description: Wet, dark gray clay		
	Remarks: System RR		



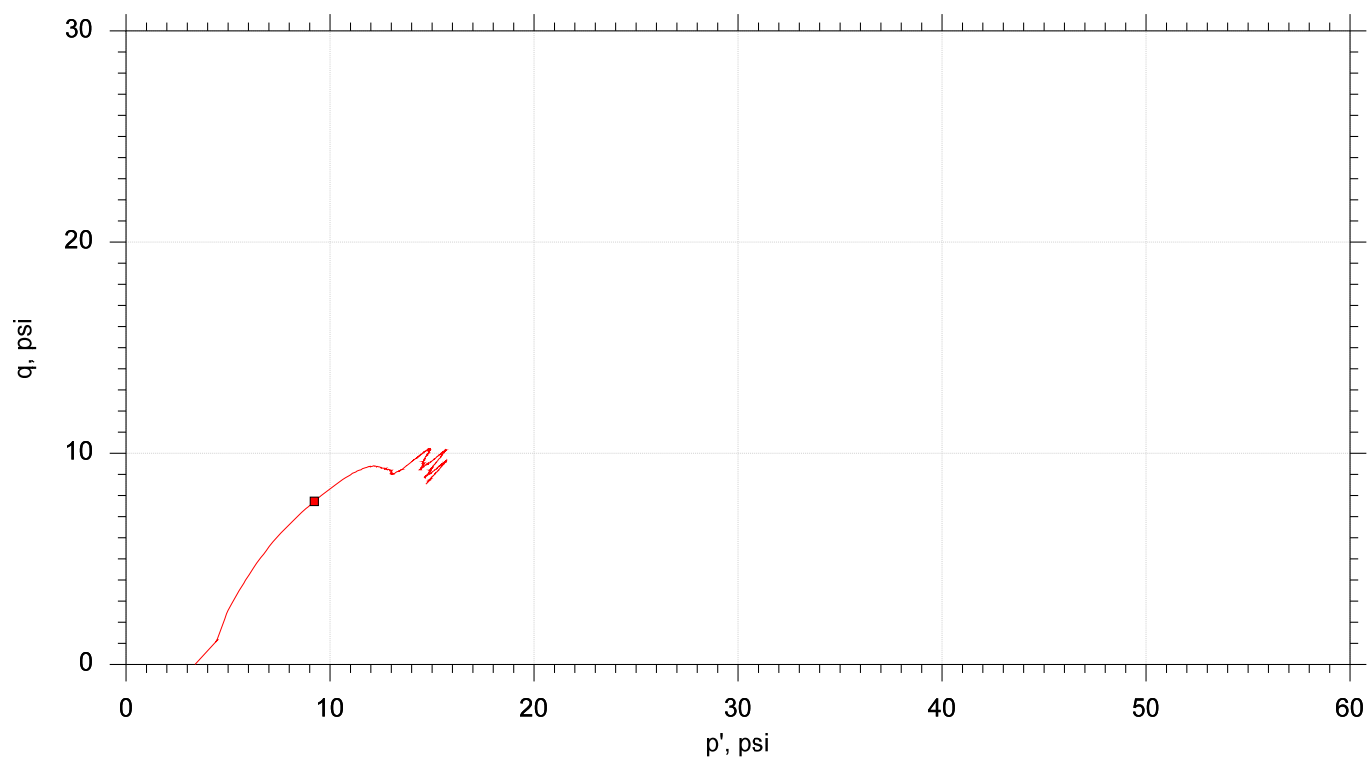
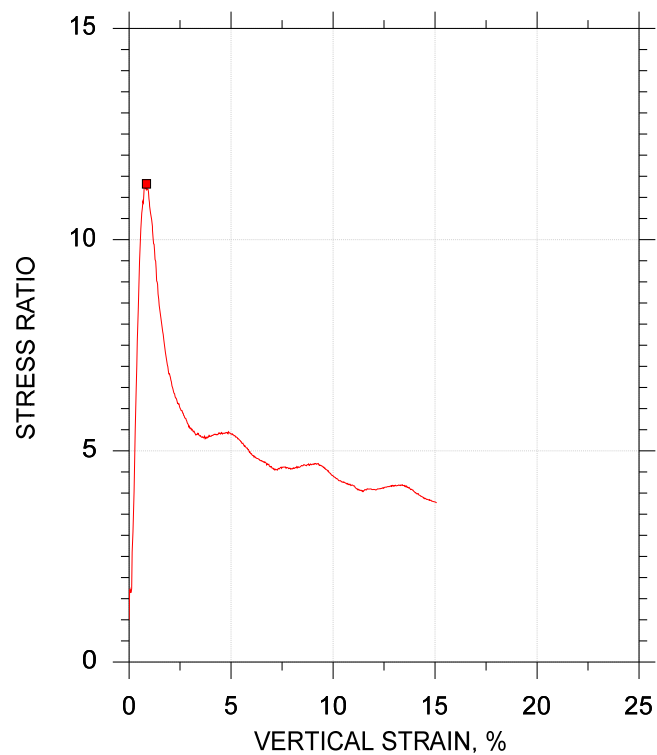
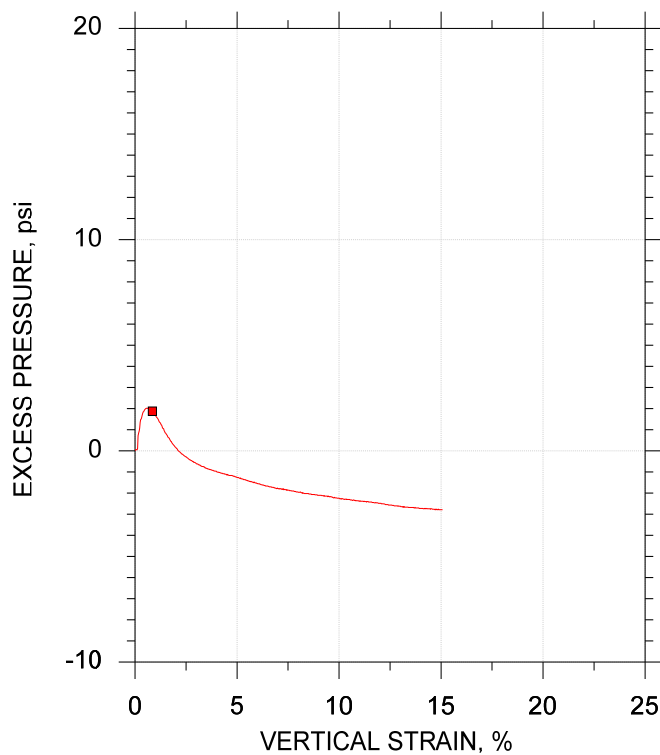
Client: Haley & Aldrich, Inc.	
Project Name: Rt 9/I-395 Connector	
Project Location: Brewer and Eddington, ME	
Project Number: GTX-308853	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-138	
Preparation: Intact	
Description: Moist, dark grayish olive clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 40	Plastic Limit: 22
Plasticity Index: 18	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		1U		
Depth, ft		8-10 ft		
Test Number		CU-20-1		
Initial	Height, in	4.800		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	33.2		
	Dry Density, pcf	86.6		
	Saturation (Wet Method), %	94.7		
	Void Ratio	0.947		
Before Shear	Moisture Content, %	35.3		
	Dry Density, pcf	86.3		
	Cross-sectional Area (Method A), in ²	3.243		
	Saturation, %	100.0		
	Void Ratio	0.954		
	Back Pressure, psi	161.6		
Vertical Effective Consolidation Stress, psi		3.383		
Horizontal Effective Consolidation Stress, psi		3.379		
Vertical Strain after Consolidation, %		0.001952		
Volumetric Strain after Consolidation, %		0.1111		
Time to 50% Consolidation, min		0.0000		
Shear Strength, psi		7.729		
Strain at Failure, %		0.851		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		15.46		
Effective Minor Principal Stress at Failure, psi		1.498		
Effective Major Principal Stress at Failure, psi		16.96		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



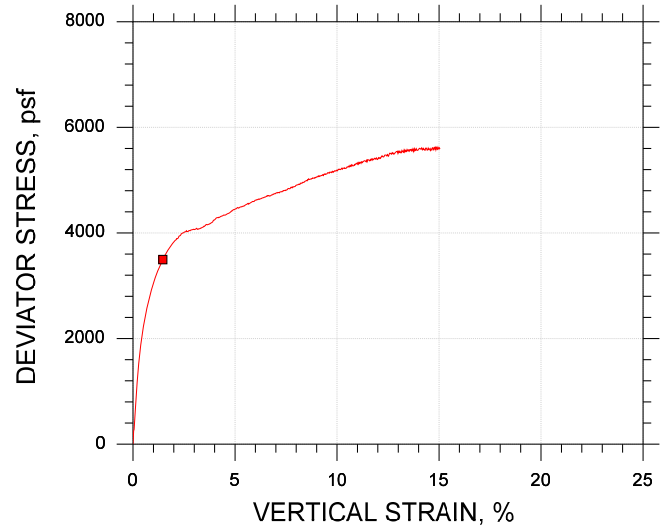
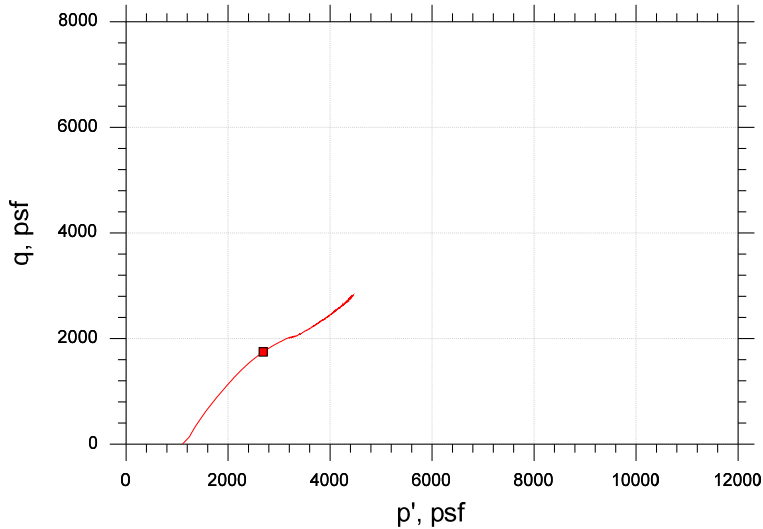
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■	1U	CU-20-1	8-10 ft	trm	7/25/19	mcm	8/2/19	308853-CU-20-1m.dat

			
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Sample Type: Intact	
	Description: Moist, dark grayish olive clay		
	Remarks: System QQ		



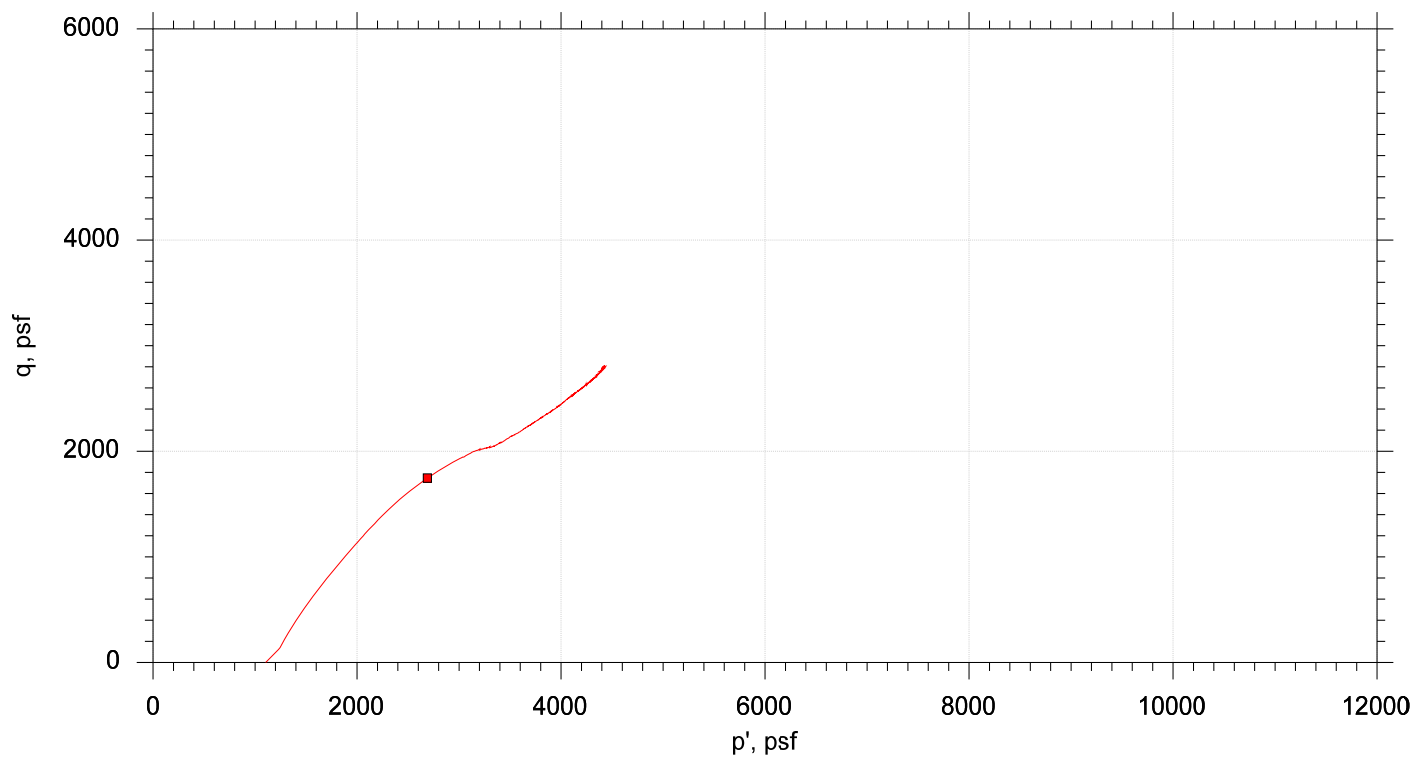
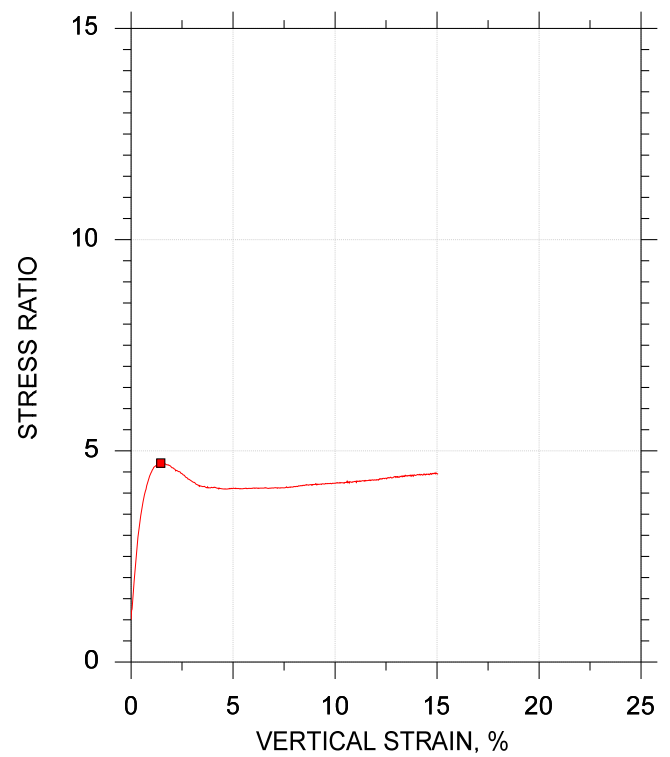
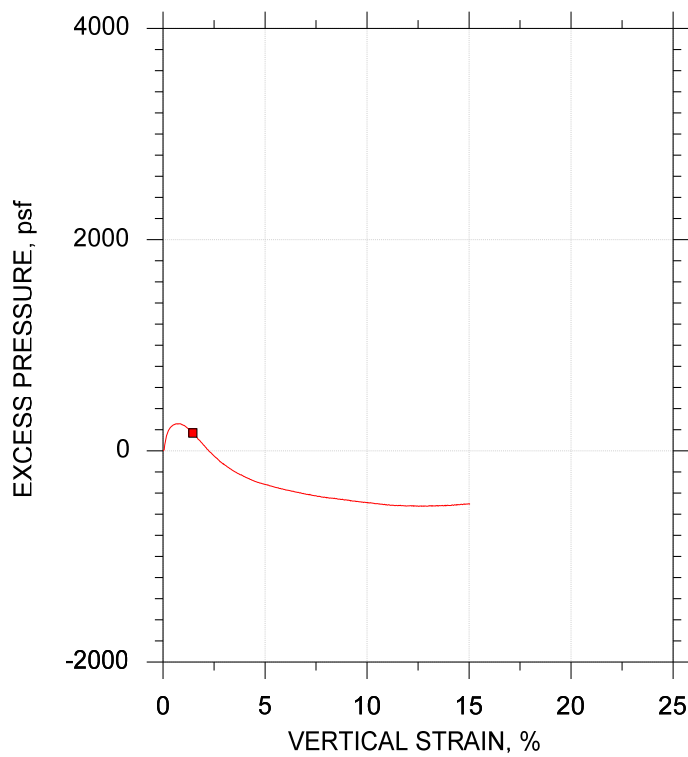
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-202	
Preparation: Intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 40	Plastic Limit: 23
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	U2			
Depth, ft	18-20 ft			
Test Number	CU-15-1			
Initial	Height, in	4.700		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	30.1		
	Dry Density, pcf	88.0		
	Saturation (Wet Method), %	88.8		
	Void Ratio	0.916		
Before Shear	Moisture Content, %	35.4		
	Dry Density, pcf	86.1		
	Cross-sectional Area (Method A), in ²	3.327		
	Saturation, %	100.0		
	Void Ratio	0.957		
	Back Pressure, psf	2.145e+004		
Vertical Effective Consolidation Stress, psf		1095.		
Horizontal Effective Consolidation Stress, psf		1103.		
Vertical Strain after Consolidation, %		0.7610		
Volumetric Strain after Consolidation, %		-1.830		
Time to 50% Consolidation, min		0.4900		
Shear Strength, psf		1747.		
Strain at Failure, %		1.46		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		3494.		
Effective Minor Principal Stress at Failure, psf		941.6		
Effective Major Principal Stress at Failure, psf		4436.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



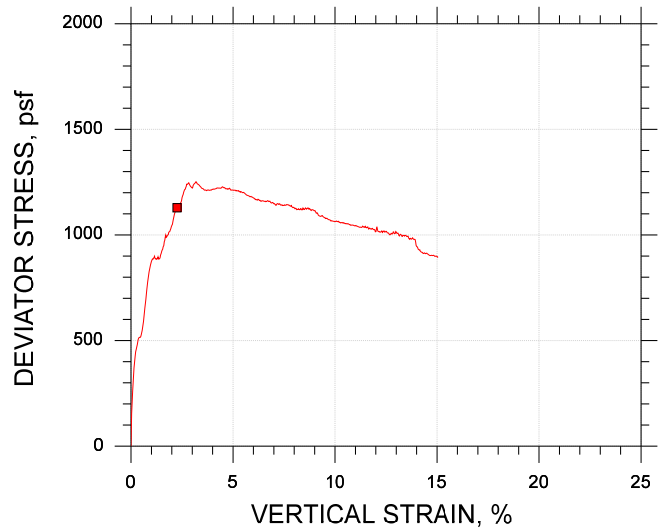
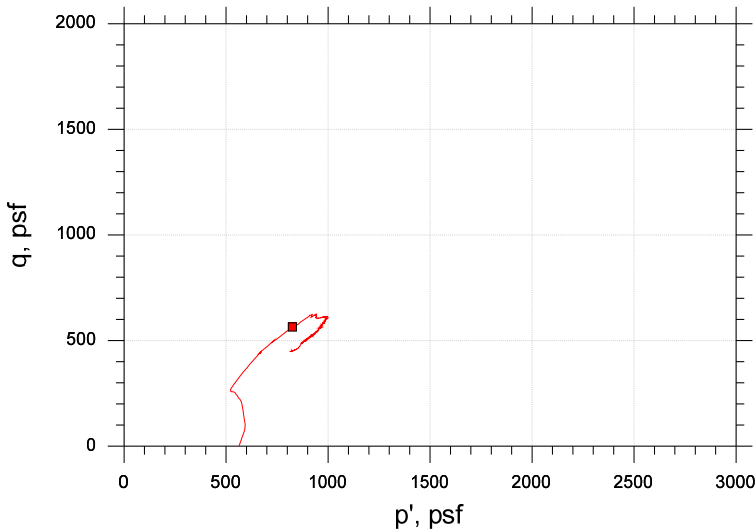
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■	U2	CU-15-1	18-20 ft	trm	3/8/21	mcm	4/1/21	312665-CU-15-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-202	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System F		



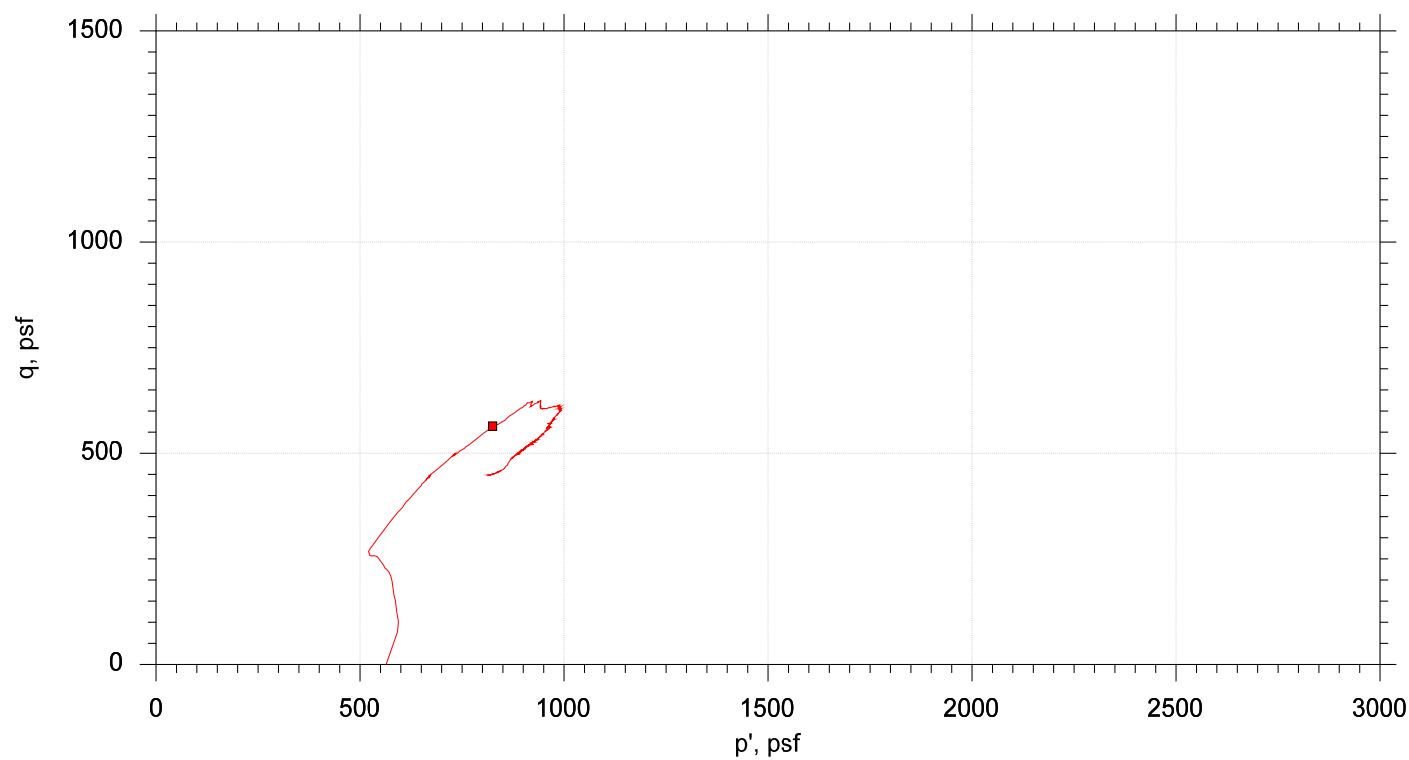
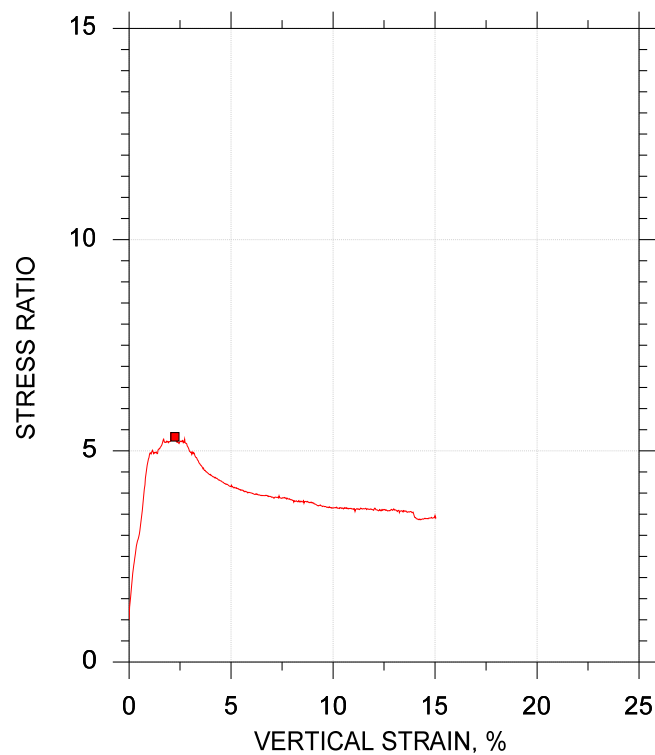
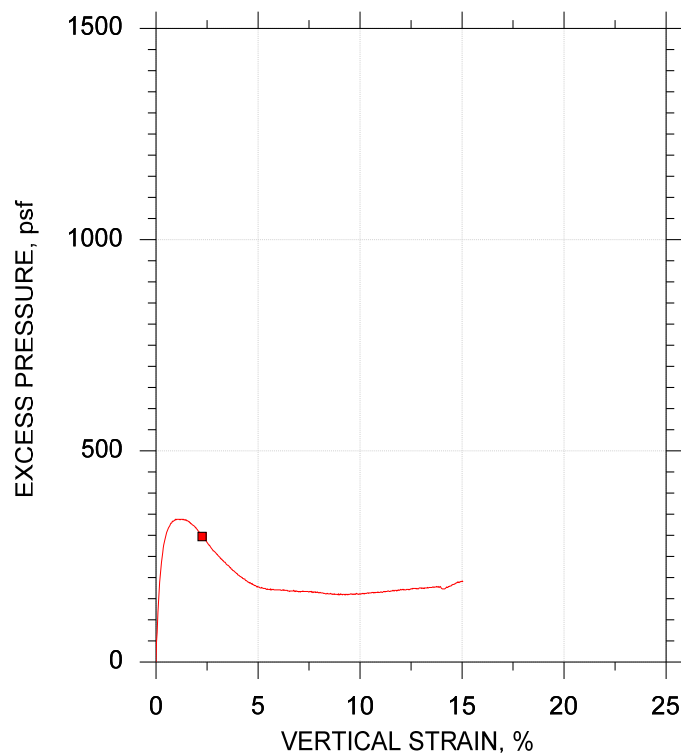
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-204	
Preparation: Intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 22
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	U1			
Depth, ft	13-15 ft			
Test Number	CU-14-1			
Initial	Height, in	4.790		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	33.8		
	Dry Density, pcf	85.9		
	Saturation (Wet Method), %	94.7		
	Void Ratio	0.963		
Before Shear	Moisture Content, %	35.4		
	Dry Density, pcf	86.1		
	Cross-sectional Area (Method A), in ²	3.260		
	Saturation, %	100.0		
	Void Ratio	0.957		
	Back Pressure, psf	2.202e+004		
Vertical Effective Consolidation Stress, psf		563.1		
Horizontal Effective Consolidation Stress, psf		563.5		
Vertical Strain after Consolidation, %		0.06448		
Volumetric Strain after Consolidation, %		0.3073		
Time to 50% Consolidation, min		121.0		
Shear Strength, psf		564.3		
Strain at Failure, %		2.25		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1129.		
Effective Minor Principal Stress at Failure, psf		260.4		
Effective Major Principal Stress at Failure, psf		1389.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



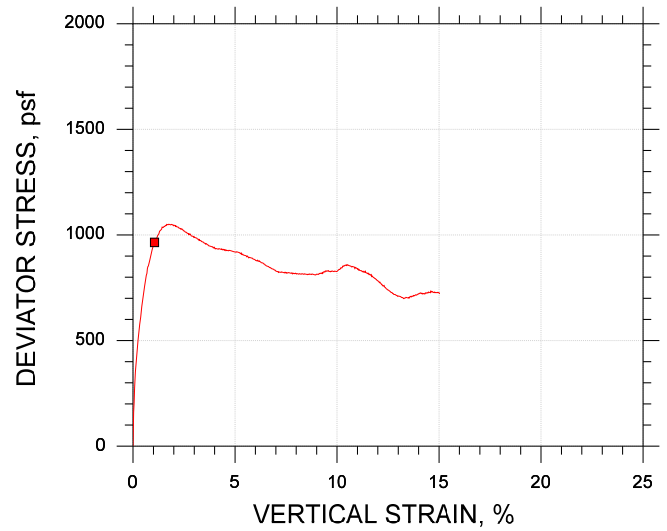
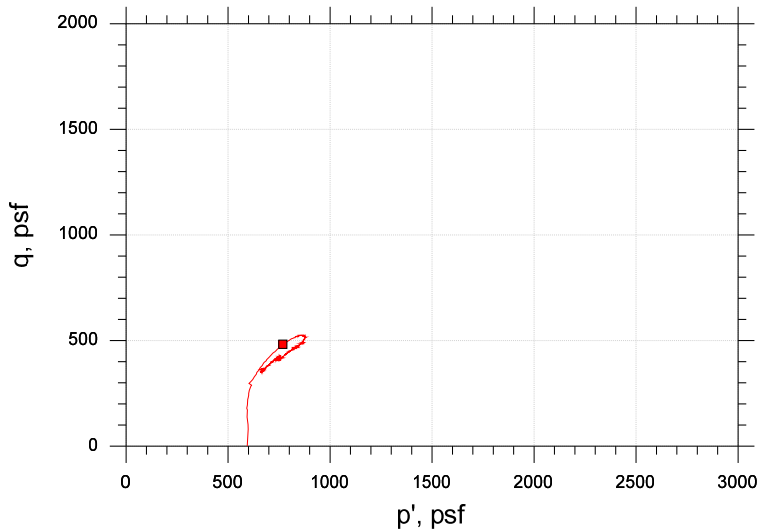
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■	U1	CU-14-1	13-15 ft	trm	3/4/21	mcm	4/1/21	312665-CU-14-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-204	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System F		



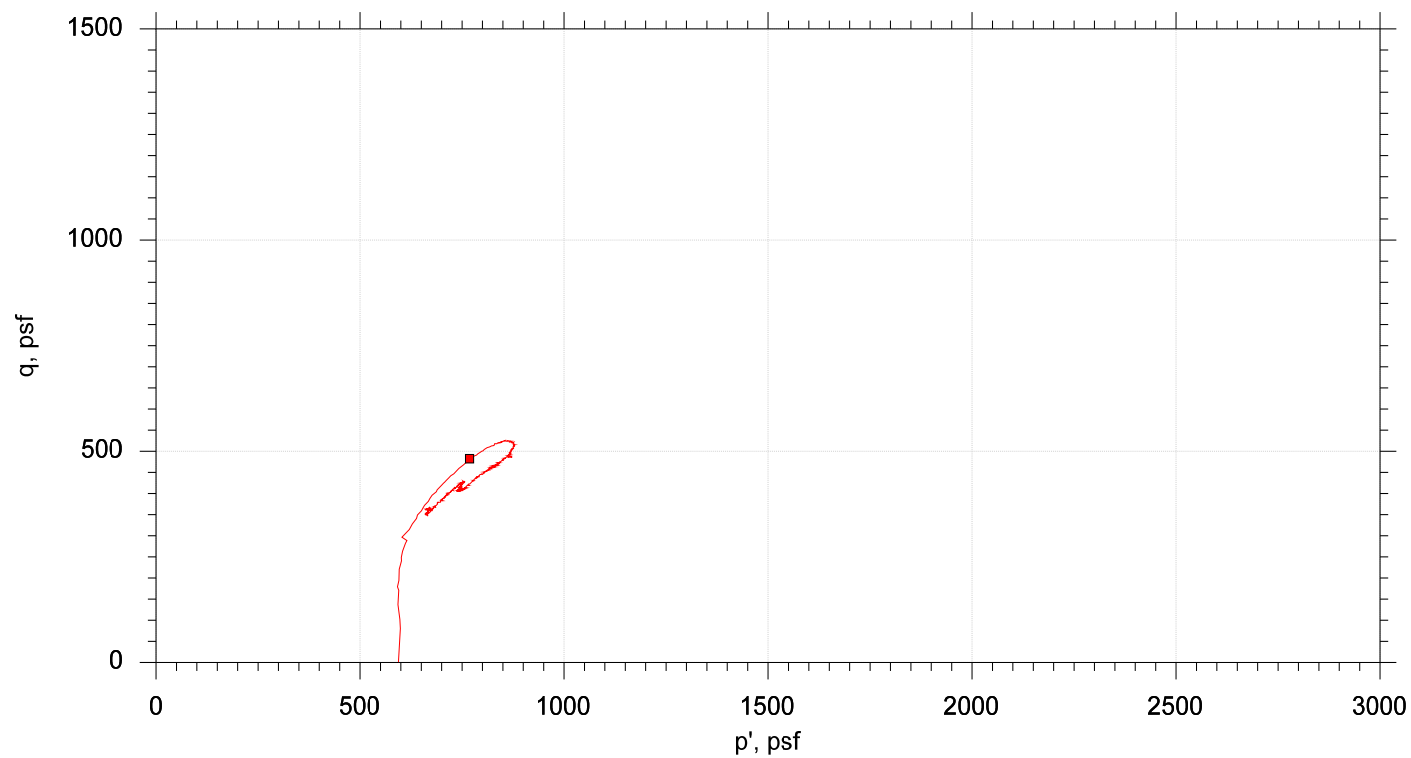
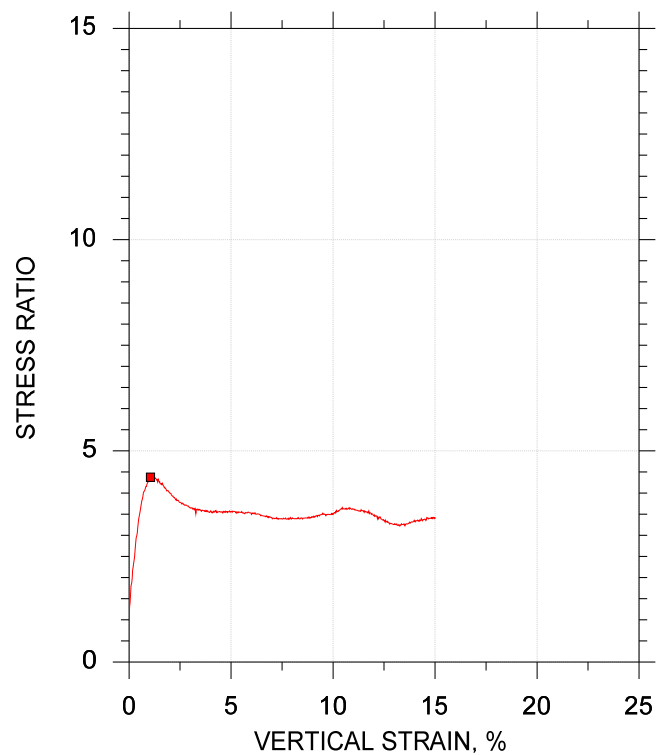
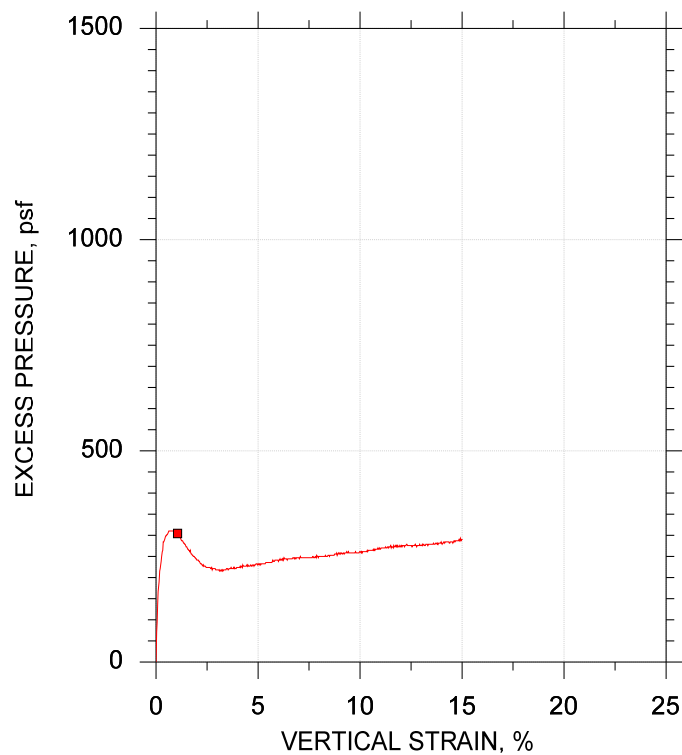
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: md	Checked By: mcm
Boring ID: HB-BE-205	
Preparation: Intact	
Description: Moist gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 36	Plastic Limit: 19
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED DRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		12-14 ft		
Test Number		CU-6-1		
Initial	Height, in	4.500		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	36.1		
	Dry Density, pcf	82.8		
	Saturation (Wet Method), %	94.1		
	Void Ratio	1.04		
Before Shear	Moisture Content, %	37.3		
	Dry Density, pcf	84.0		
	Cross-sectional Area (Method A), in ²	3.240		
	Saturation, %	100.0		
	Void Ratio	1.01		
	Back Pressure, psf	2.199e+004		
Vertical Effective Consolidation Stress, psf		593.3		
Horizontal Effective Consolidation Stress, psf		593.3		
Vertical Strain after Consolidation, %		0.1237		
Volumetric Strain after Consolidation, %		0.1534		
Time to 50% Consolidation, min		0.0000		
Shear Strength, psf		482.4		
Strain at Failure, %		1.05		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		964.9		
Effective Minor Principal Stress at Failure, psf		285.7		
Effective Major Principal Stress at Failure, psf		1251.		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED DRAINED TRIAXIAL TEST by ASTM D4767



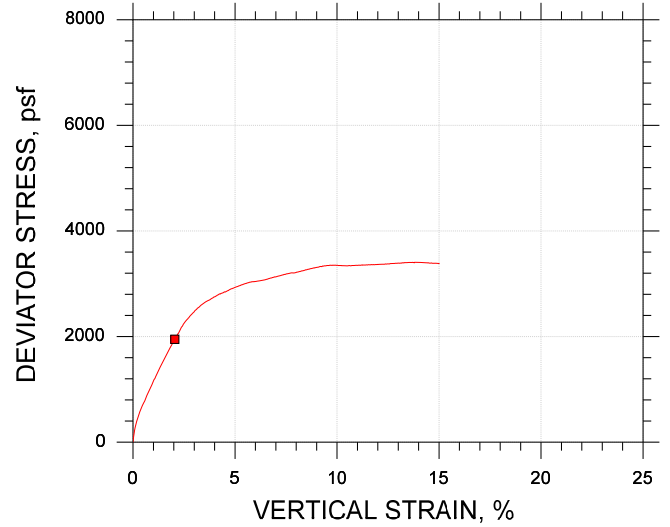
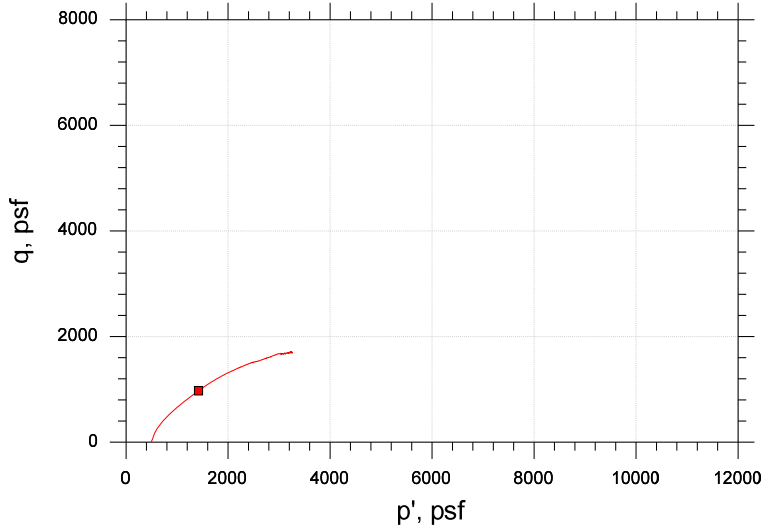
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■	U1	CU-6-1	12-14 ft	md	02/27/21	mcm	3/16/21	312665-CU-6-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-205	Sample Type: Intact	
	Description: Moist gray clay		
	Remarks: System RR		



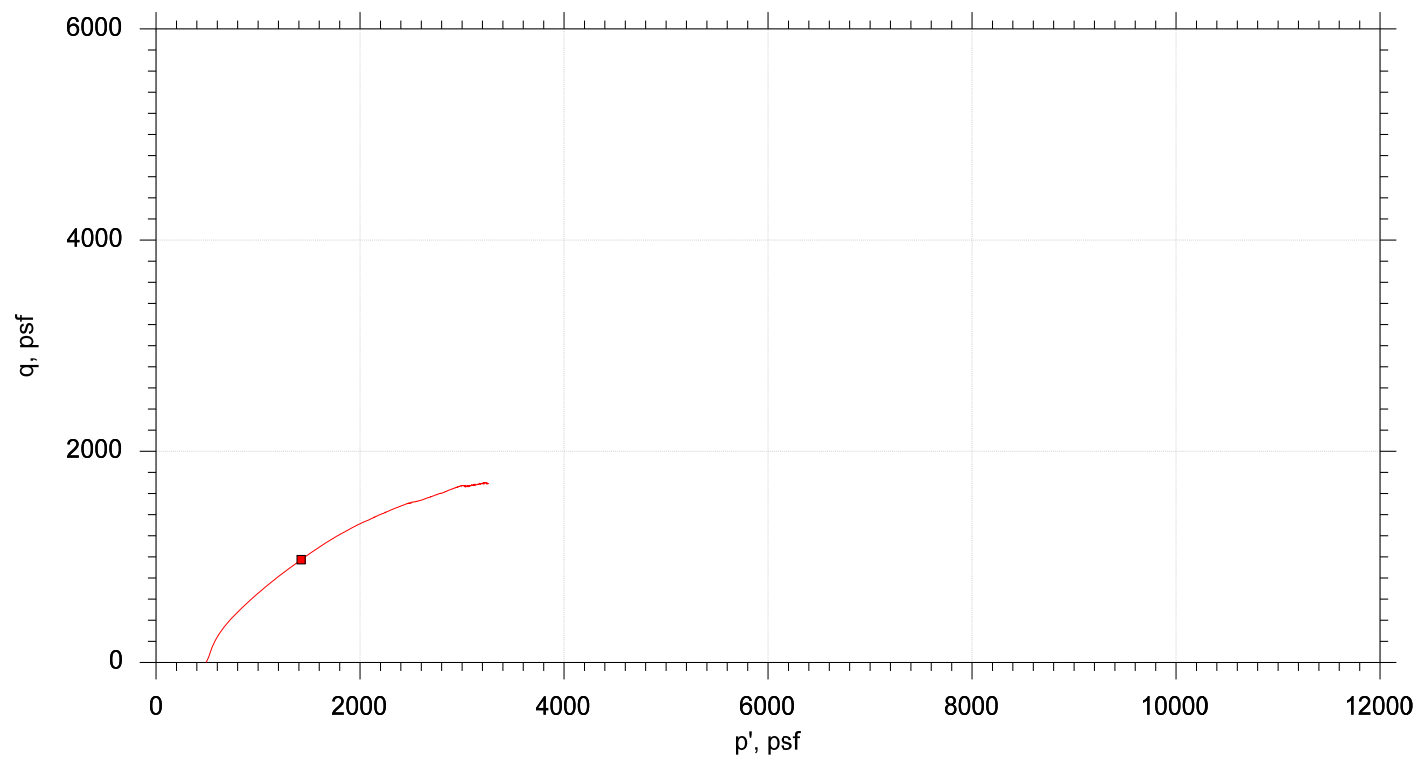
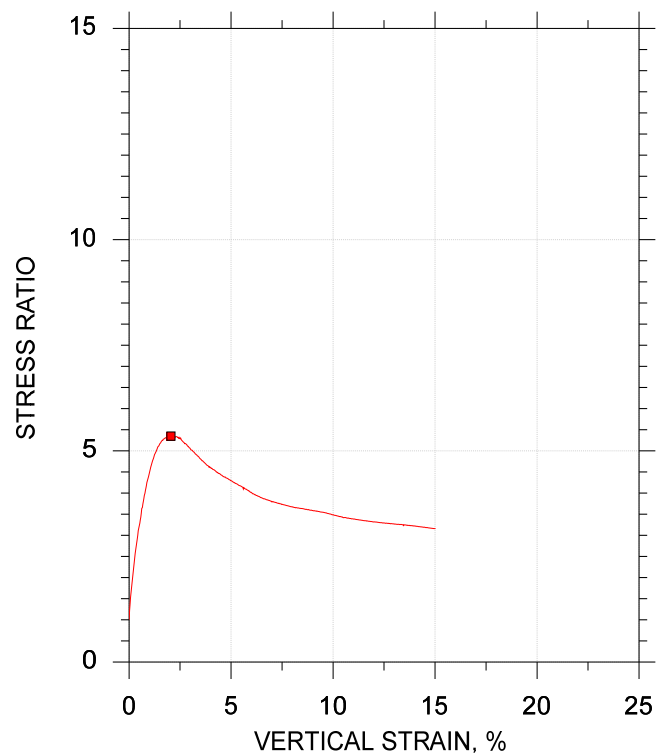
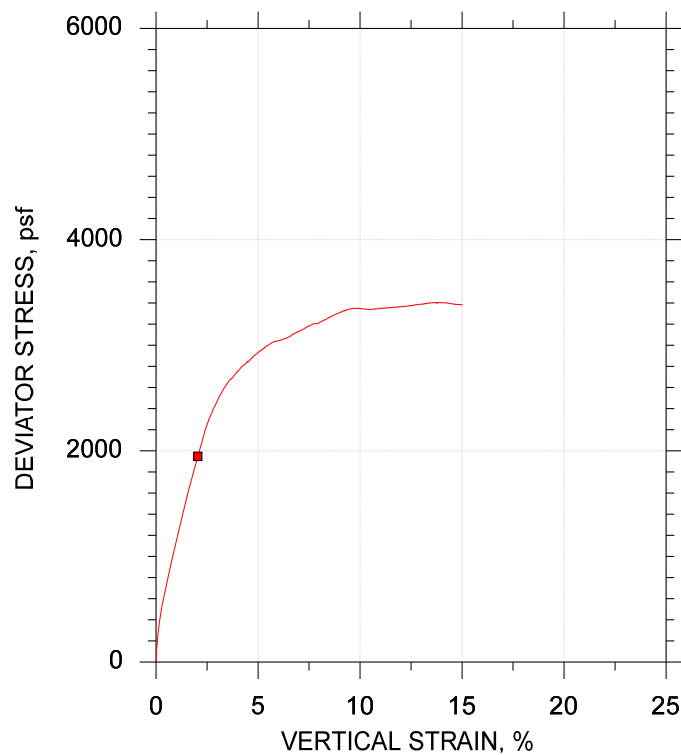
Client: Haley & Aldrich	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HE-BE-207	
Preparation: intact	
Description: Moist, olive gray and brownish yellow clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 18
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol				
Sample ID		U1		
Depth, ft		5-7 ft		
Test Number		CU-10-1		
Initial	Height, in	4.680		
	Diameter, in	2.010		
	Moisture Content (from Cuttings), %	26.5		
	Dry Density, pcf	91.2		
	Saturation (Wet Method), %	84.5		
	Void Ratio	0.847		
Before Shear	Moisture Content, %	31.4		
	Dry Density, pcf	91.2		
	Cross-sectional Area (Method A), in ²	3.171		
	Saturation, %	100.0		
	Void Ratio	0.847		
	Back Pressure, psf	1.238e+004		
Vertical Effective Consolidation Stress, psf		488.4		
Horizontal Effective Consolidation Stress, psf		489.7		
Vertical Strain after Consolidation, %		0.1433		
Volumetric Strain after Consolidation, %		0.6364		
Time to 50% Consolidation, min		1.210		
Shear Strength, psf		974.2		
Strain at Failure, %		2.05		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1948.		
Effective Minor Principal Stress at Failure, psf		447.8		
Effective Major Principal Stress at Failure, psf		2396.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



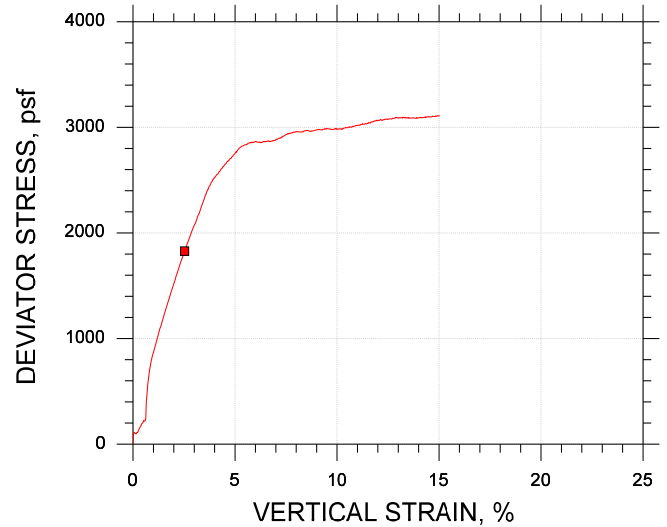
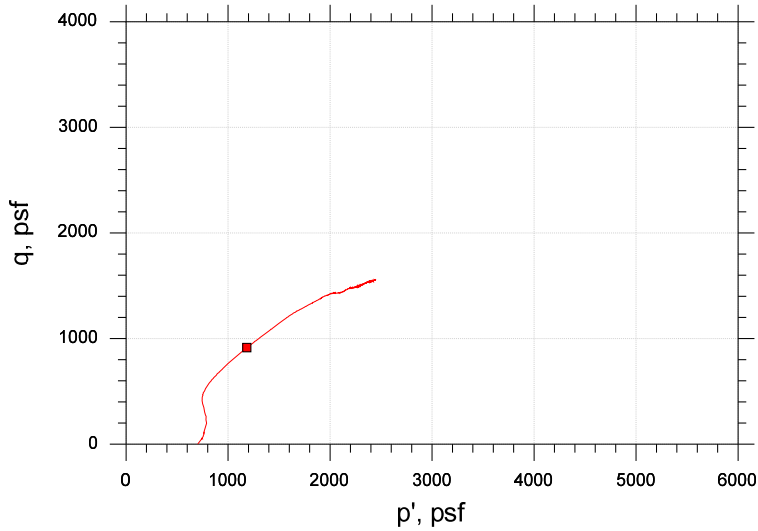
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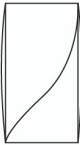
			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HE-BE-207	Sample Type: intact	
	Description: Moist, olive gray and brownish yellow clay		
	Remarks: System F		



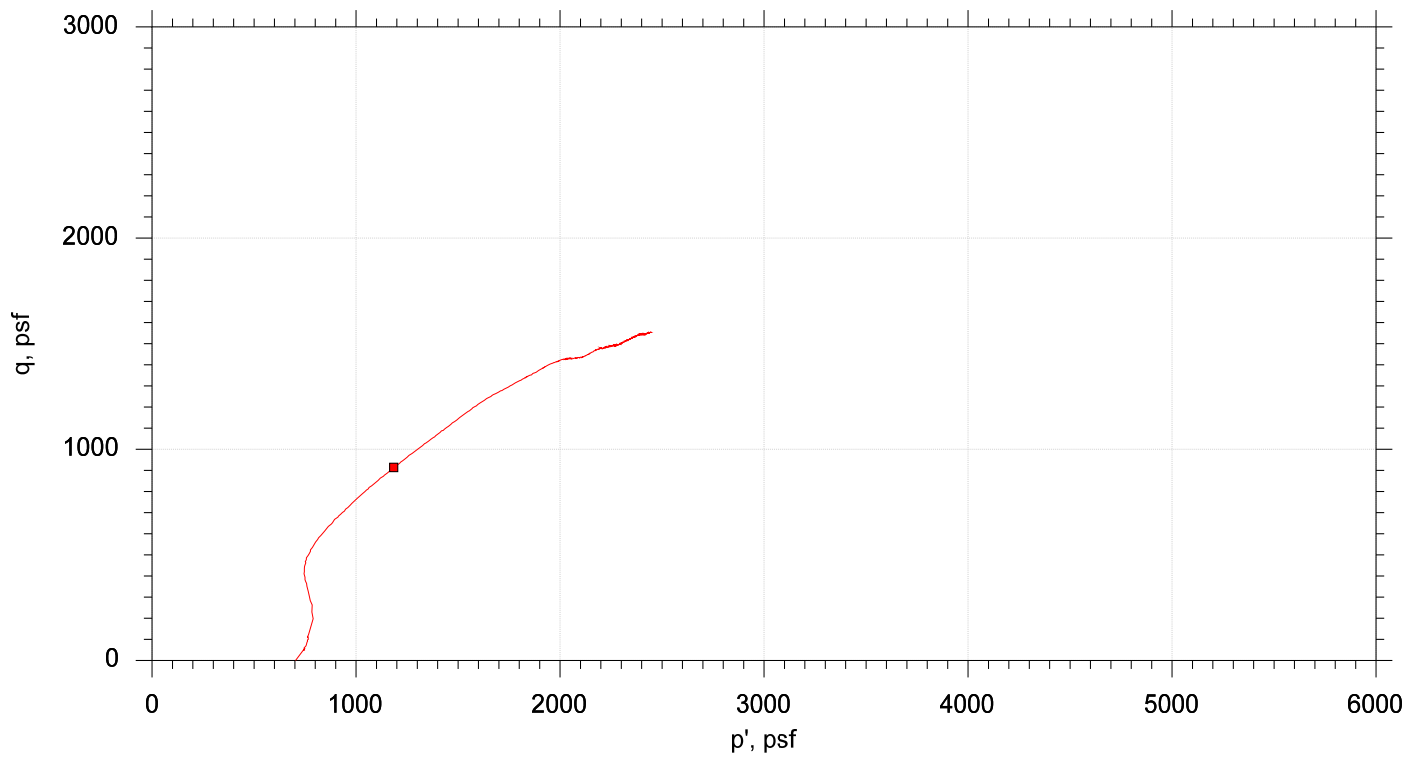
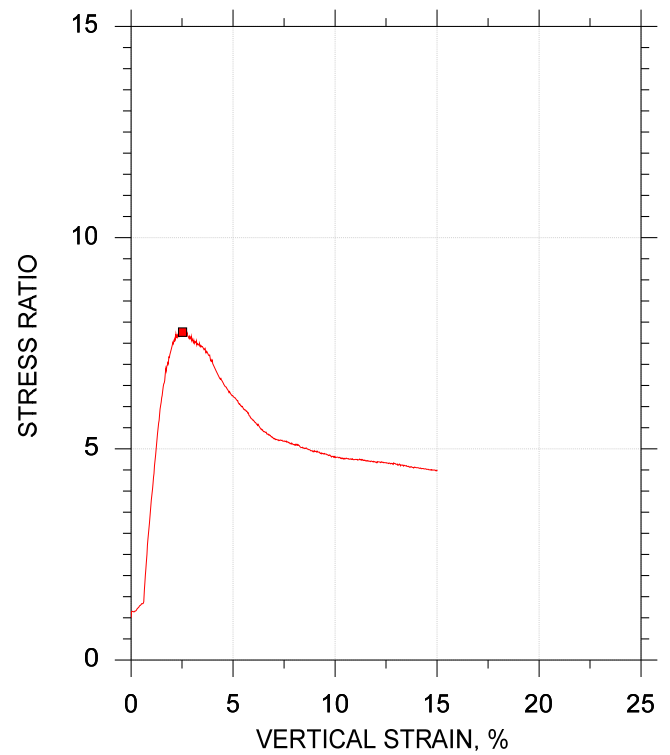
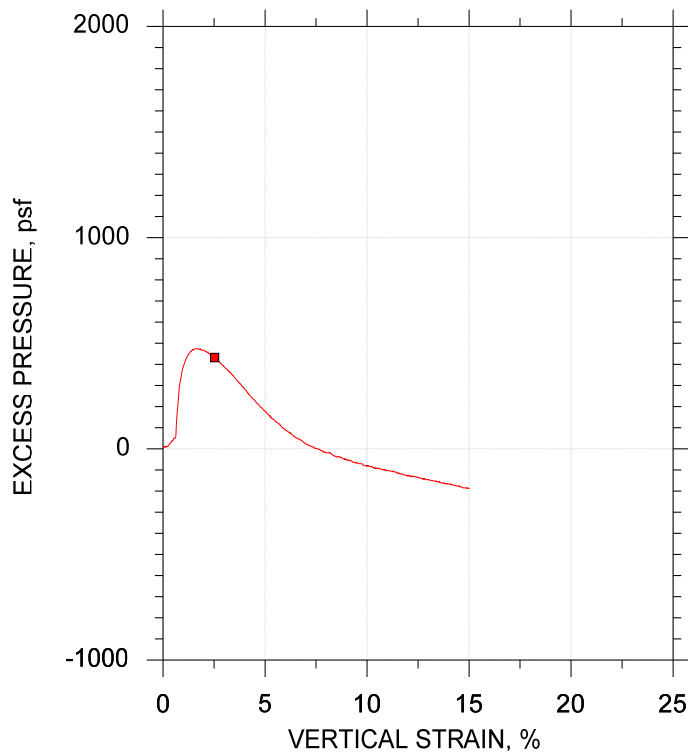
Client: Haley & Aldrich, Inc	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-208	
Preparation: intact	
Description: Moist, gray and olive yellow clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 17
Plasticity Index: 17	Estimated Specific Gravity: 2.65

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol				
Sample ID		U1		
Depth, ft		10-12 ft		
Test Number		CU-22-1		
Initial	Height, in	4.370		
	Diameter, in	2.050		
	Moisture Content (from Cuttings), %	31.1		
	Dry Density, pcf	85.7		
	Saturation (Wet Method), %	88.6		
	Void Ratio	0.931		
Before Shear	Moisture Content, %	34.2		
	Dry Density, pcf	86.8		
	Cross-sectional Area (Method A), in²	3.268		
	Saturation, %	100.0		
	Void Ratio	0.907		
	Back Pressure, psf	2.171e+004		
Vertical Effective Consolidation Stress, psf		704.3		
Horizontal Effective Consolidation Stress, psf		702.9		
Vertical Strain after Consolidation, %		0.008882		
Volumetric Strain after Consolidation, %		0.4789		
Time to 50% Consolidation, min		16.00		
Shear Strength, psf		914.2		
Strain at Failure, %		2.53		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1828.		
Effective Minor Principal Stress at Failure, psf		270.2		
Effective Major Principal Stress at Failure, psf		2099.		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



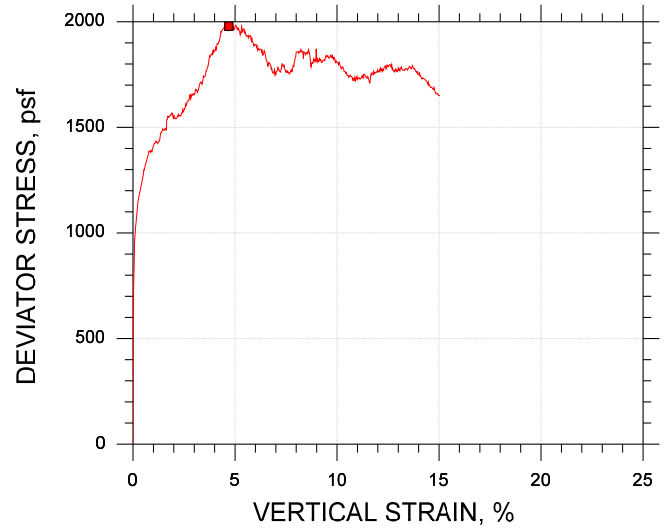
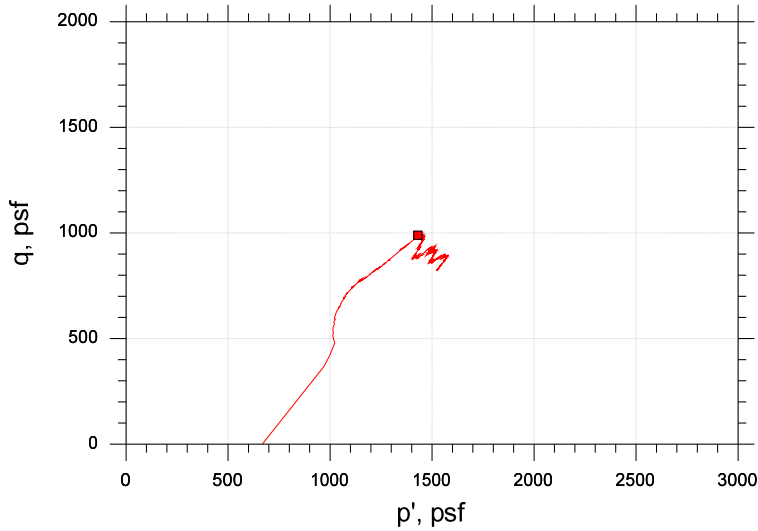
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■	U1	CU-22-1	10-12 ft	trm	4/1/21	mcm	4/9/21	312665-CU-22-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-208	Sample Type: intact	
	Description: Moist, gray and olive yellow clay		
	Remarks: System- Nancy Room Left		



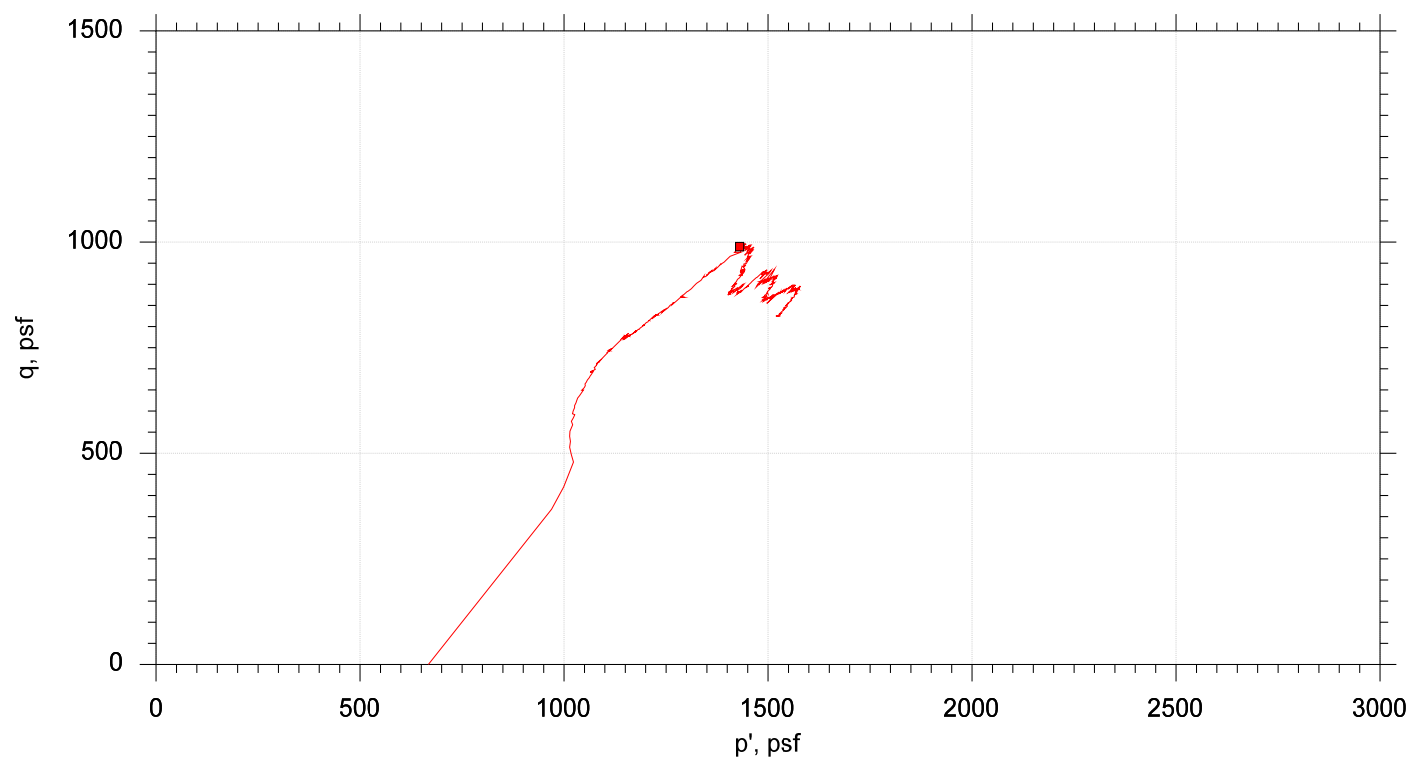
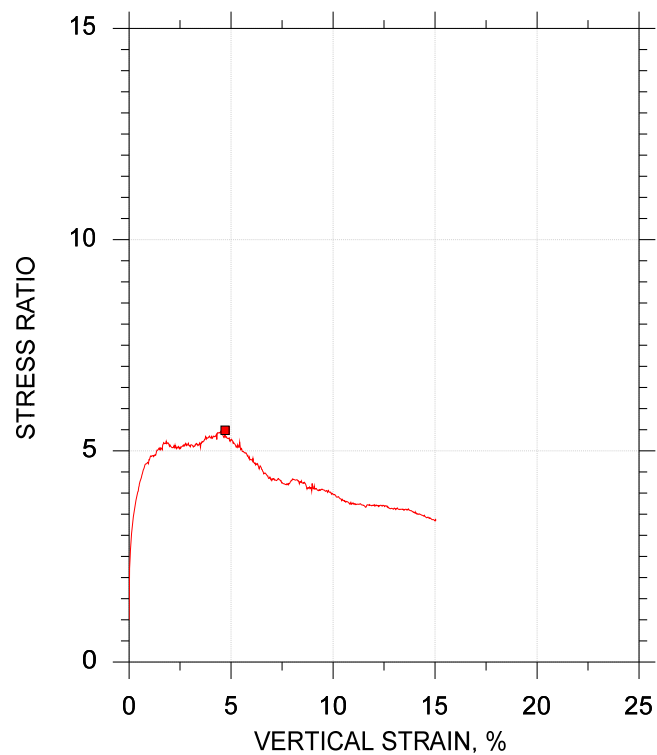
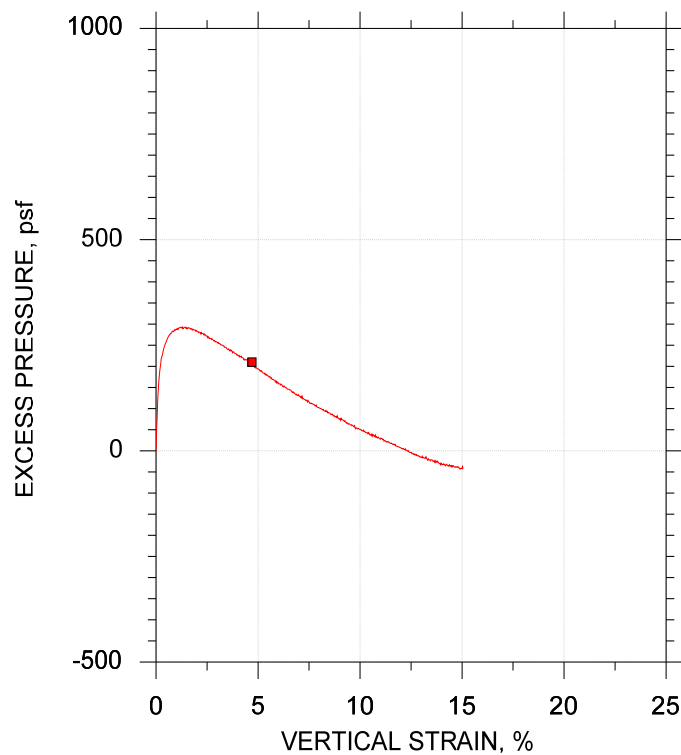
Client: Haley & Aldrich, Inc	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-210	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 18
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		15-17 ft		
Test Number		CU-11-1		
Initial	Height, in	4.510		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	33.3		
	Dry Density, pcf	87.0		
	Saturation (Wet Method), %	95.9		
	Void Ratio	0.938		
Before Shear	Moisture Content, %	34.3		
	Dry Density, pcf	87.5		
	Cross-sectional Area (Method A), in ²	3.251		
	Saturation, %	100.0		
	Void Ratio	0.927		
	Back Pressure, psf	2.319e+004		
Vertical Effective Consolidation Stress, psf		667.8		
Horizontal Effective Consolidation Stress, psf		667.3		
Vertical Strain after Consolidation, %		0.003364		
Volumetric Strain after Consolidation, %		0.5215		
Time to 50% Consolidation, min		67.24		
Shear Strength, psf		989.3		
Strain at Failure, %		4.70		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1979.		
Effective Minor Principal Stress at Failure, psf		440.8		
Effective Major Principal Stress at Failure, psf		2419.		
B-Value		1.0		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



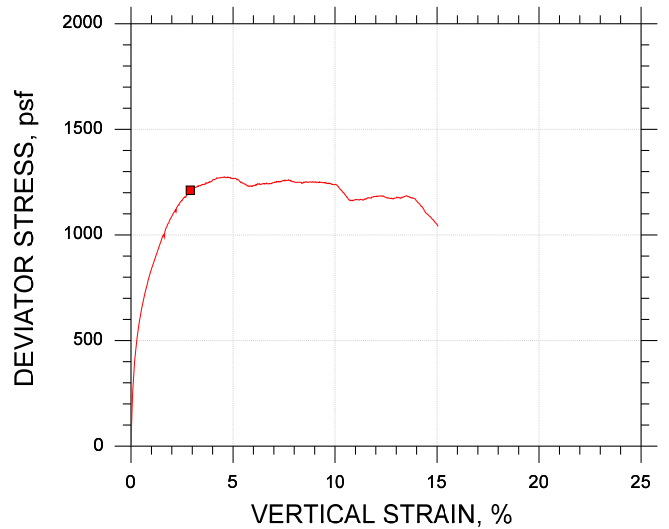
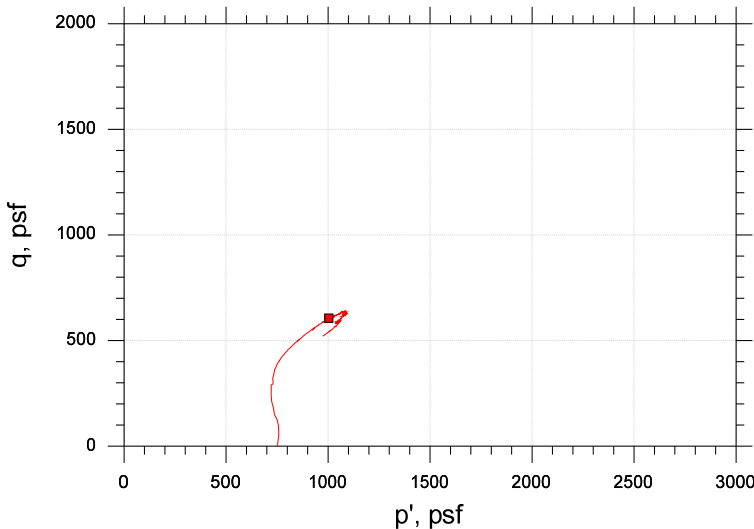
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■	U1	CU-11-1	15-17 ft	trm	3/4/21	mcm	3/31/21	312665-CU-11-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-210	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System O		



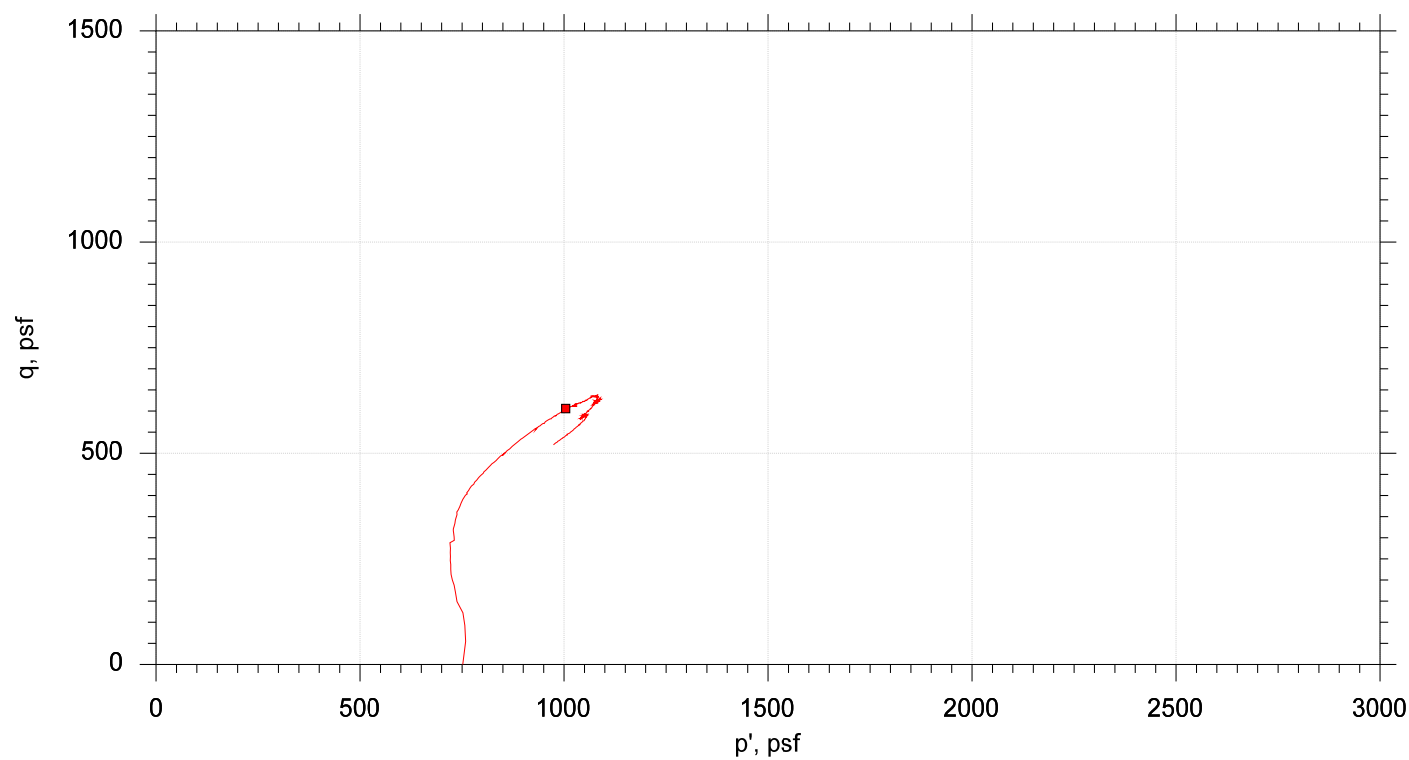
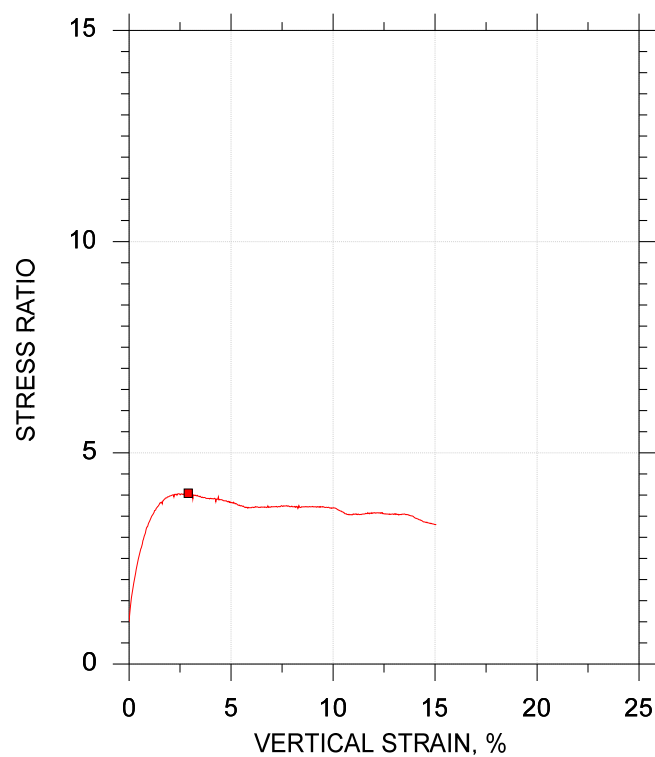
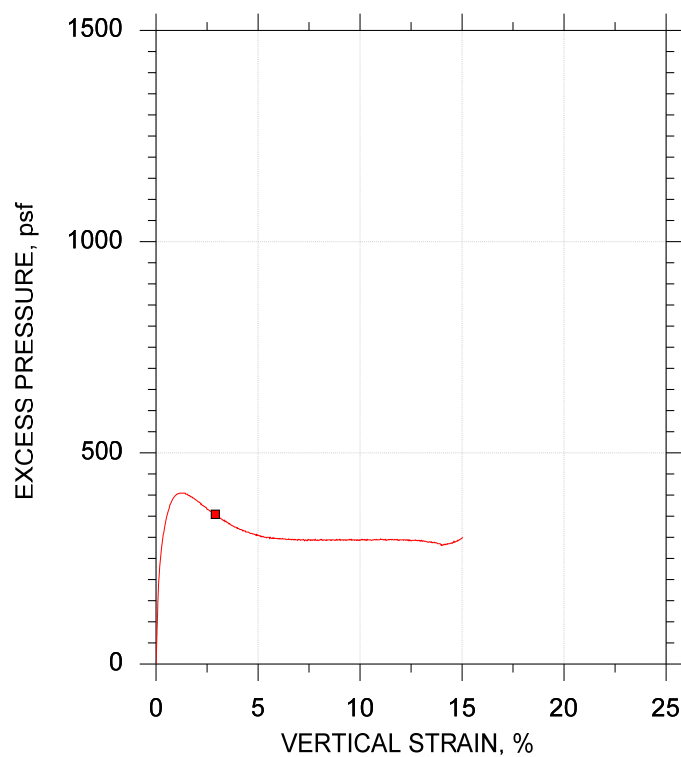
Client: Haley and Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HE-BE-215	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 37	Plastic Limit: 19
Plasticity Index: 18	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		10-12 ft		
Test Number		CU-3-1		
Initial	Height, in	4.300		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	35.3		
	Dry Density, pcf	83.8		
	Saturation (Wet Method), %	94.1		
	Void Ratio	1.01		
Before Shear	Moisture Content, %	36.7		
	Dry Density, pcf	84.7		
	Cross-sectional Area (Method A), in²	3.247		
	Saturation, %	100.0		
	Void Ratio	0.990		
	Back Pressure, psf	2.173e+004		
Vertical Effective Consolidation Stress, psf		749.0		
Horizontal Effective Consolidation Stress, psf		751.6		
Vertical Strain after Consolidation, %		0.2650		
Volumetric Strain after Consolidation, %		0.6506		
Time to 50% Consolidation, min		25.84		
Shear Strength, psf		605.9		
Strain at Failure, %		2.90		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1212.		
Effective Minor Principal Stress at Failure, psf		398.1		
Effective Major Principal Stress at Failure, psf		1610.		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



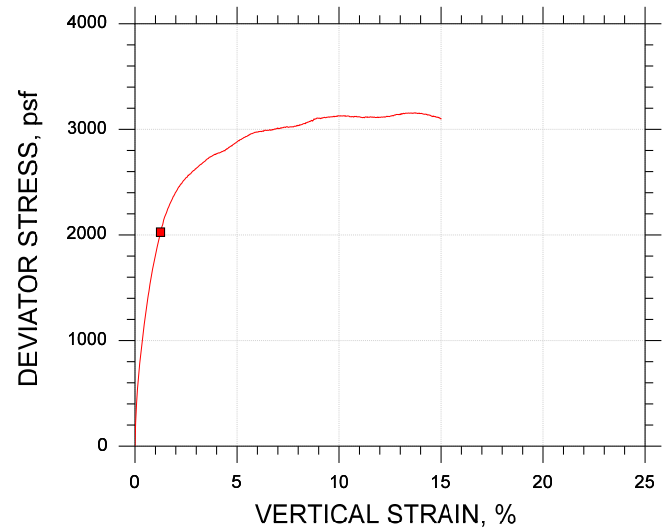
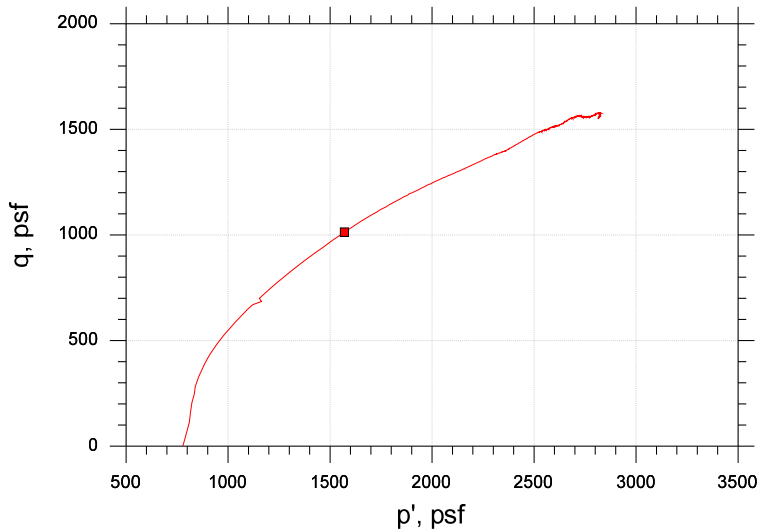
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■	U1	CU-3-1	10-12 ft	trm	2/25/21	mcm	3/16/21	312665-CU-3-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HE-BE-215	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System F		



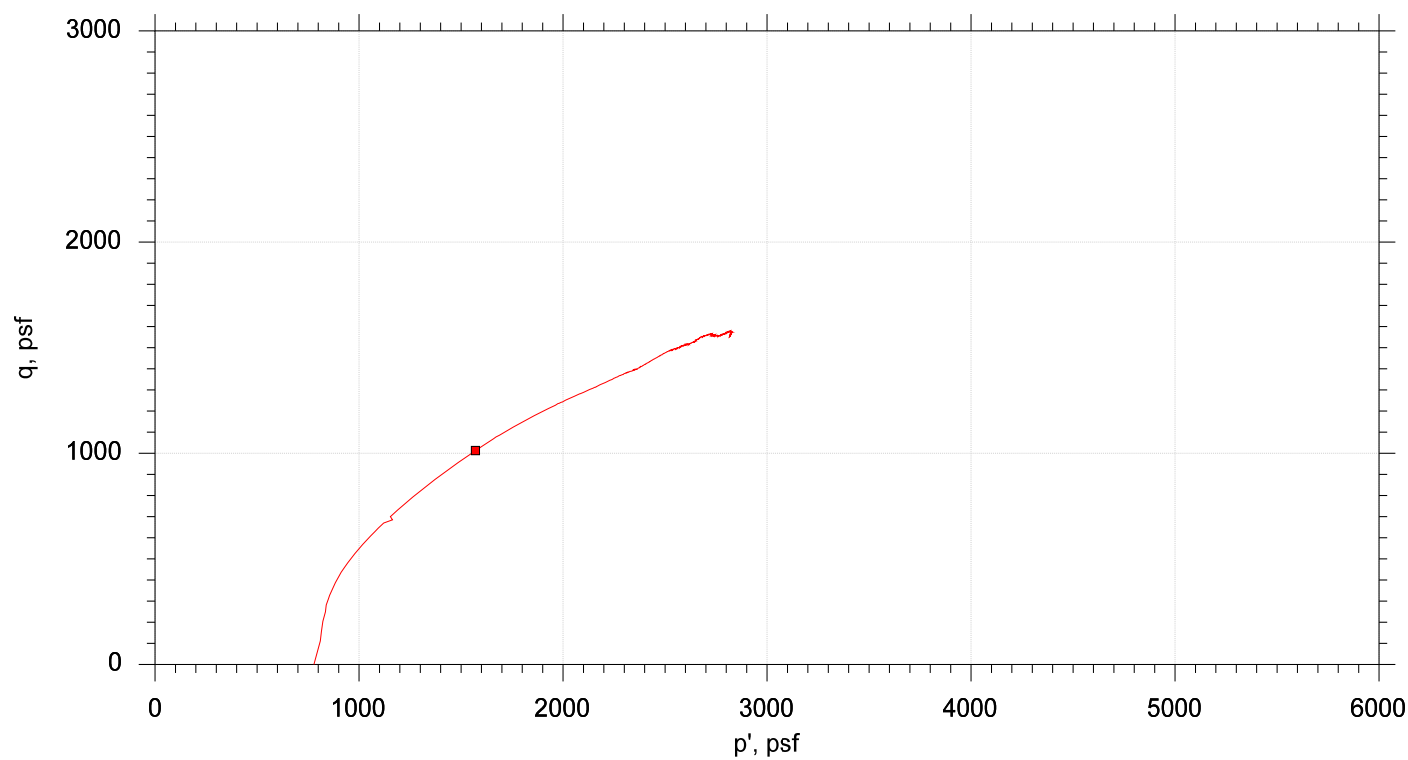
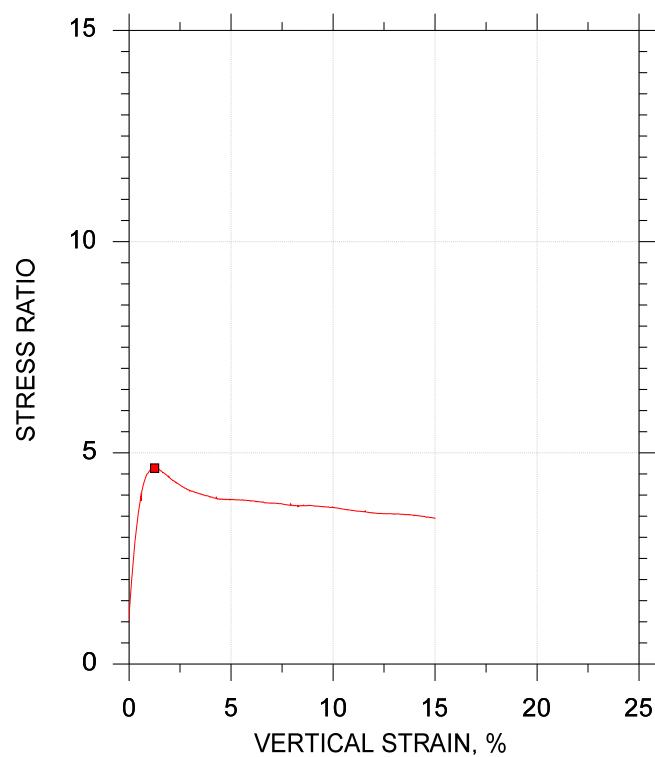
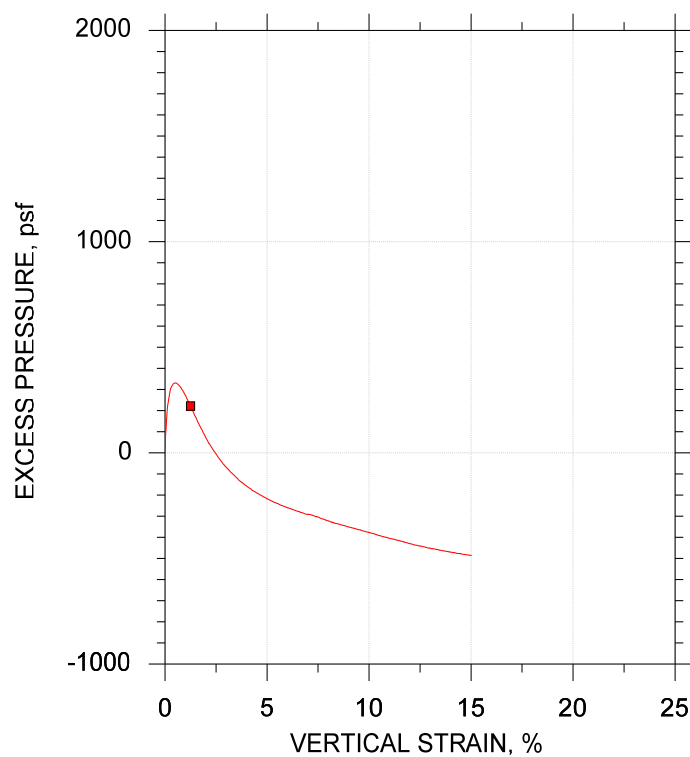
Client: Haley & Aldrich, Inc	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-216	
Preparation: intact	
Description: Moist, olive gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 18
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol	■			
Sample ID	U1			
Depth, ft	8-10 ft			
Test Number	CU-1-1			
Initial	Height, in	4.650		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	0.0		
	Dry Density, pcf	122.		
	Saturation (Wet Method), %	0.0		
	Void Ratio	0.385		
Before Shear	Moisture Content, %	14.0		
	Dry Density, pcf	122.		
	Cross-sectional Area (Method A), in²	3.262		
	Saturation, %	100.0		
	Void Ratio	0.377		
	Back Pressure, psf	2.201e+004		
Vertical Effective Consolidation Stress, psf		778.1		
Horizontal Effective Consolidation Stress, psf		778.6		
Vertical Strain after Consolidation, %		0.09707		
Volumetric Strain after Consolidation, %		0.2736		
Time to 50% Consolidation, min		12.25		
Shear Strength, psf		1013.		
Strain at Failure, %		1.25		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		2027.		
Effective Minor Principal Stress at Failure, psf		556.9		
Effective Major Principal Stress at Failure, psf		2583.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System O				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



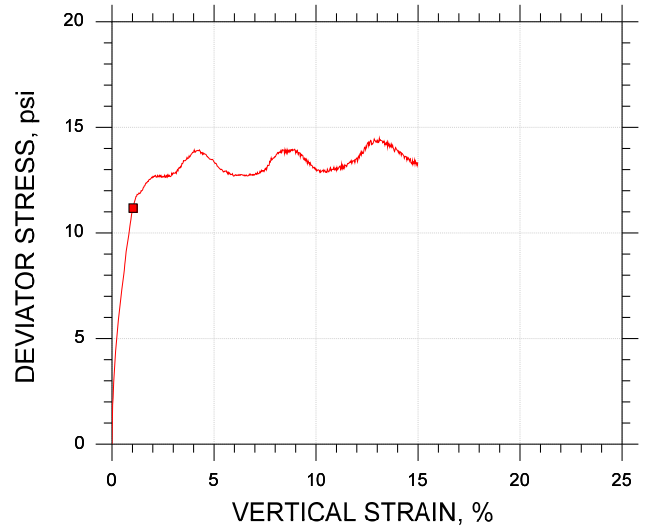
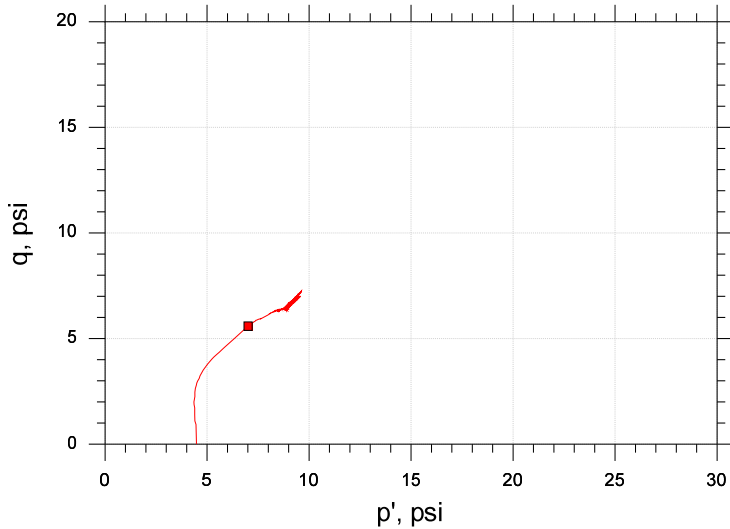
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■	U1	CU-1-1	8-10 ft	trm	2/26/21	mcm	3/31/21	312665-CU-1-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-216	Sample Type: intact	
	Description: Moist, olive gray clay		
	Remarks: System O		



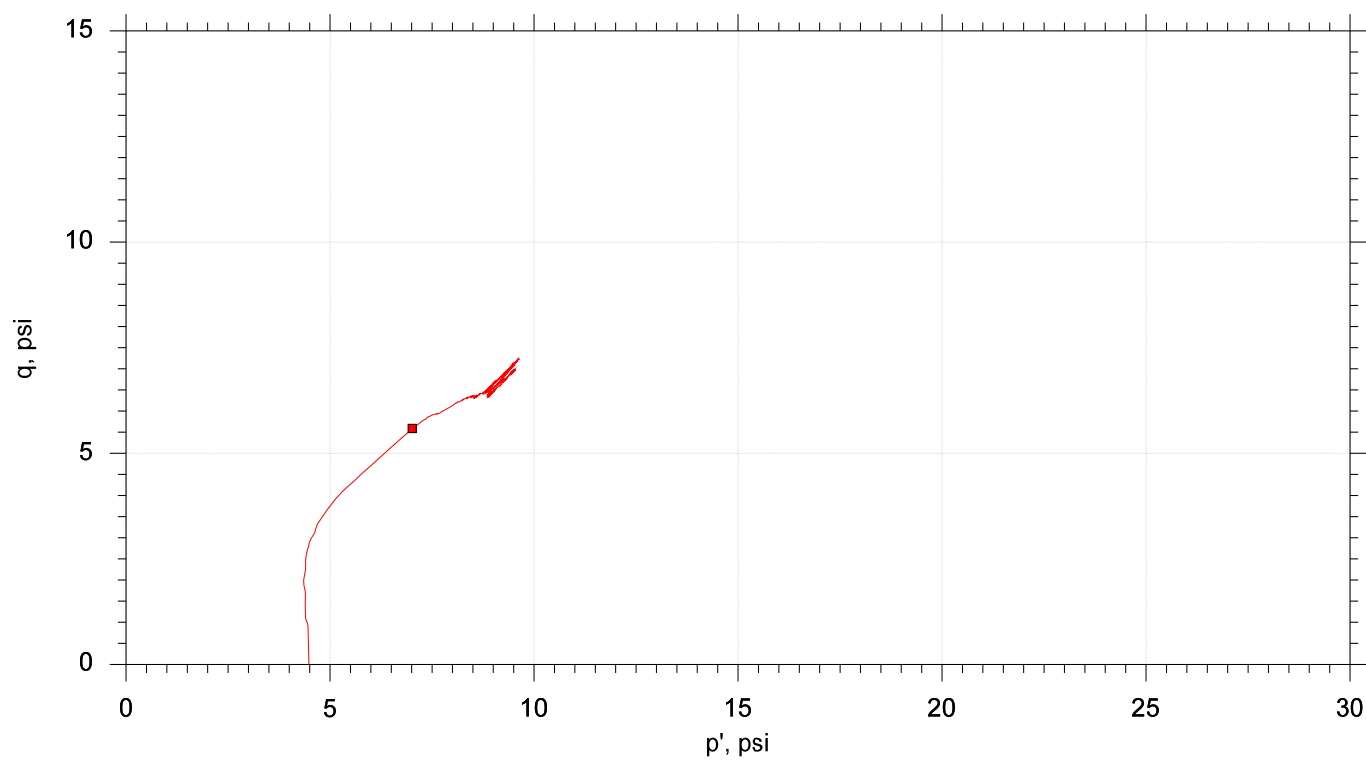
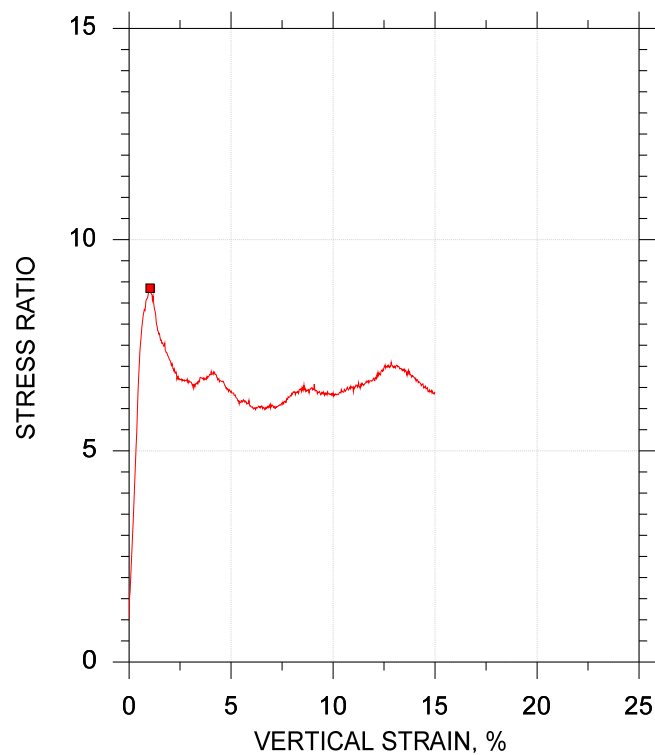
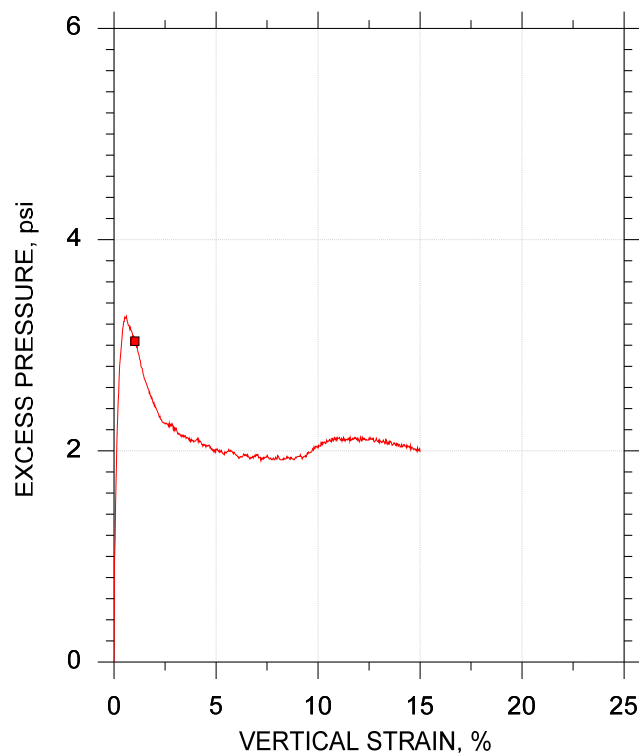
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 3)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313197	
Tested By: trm	Checked By: njh
Boring ID: HB-BE-223	
Preparation: intact	
Description: Wet, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 19
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U-1 (19.2)		
Depth, ft		12-14		
Test Number		CU-2-1		
Initial	Height, in	4.560		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	35.2		
	Dry Density, pcf	86.4		
	Saturation (Wet Method), %	100.0		
	Void Ratio	0.951		
Before Shear	Moisture Content, %	34.9		
	Dry Density, pcf	86.8		
	Cross-sectional Area (Method A), in ²	3.256		
	Saturation, %	100.0		
	Void Ratio	0.943		
	Back Pressure, psi	150.8		
Vertical Effective Consolidation Stress, psi		4.484		
Horizontal Effective Consolidation Stress, psi		4.479		
Vertical Strain after Consolidation, %		0.006397		
Volumetric Strain after Consolidation, %		0.2907		
Time to 50% Consolidation, min		144.0		
Shear Strength, psi		5.589		
Strain at Failure, %		1.03		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		11.18		
Effective Minor Principal Stress at Failure, psi		1.423		
Effective Major Principal Stress at Failure, psi		12.60		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System JJ				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



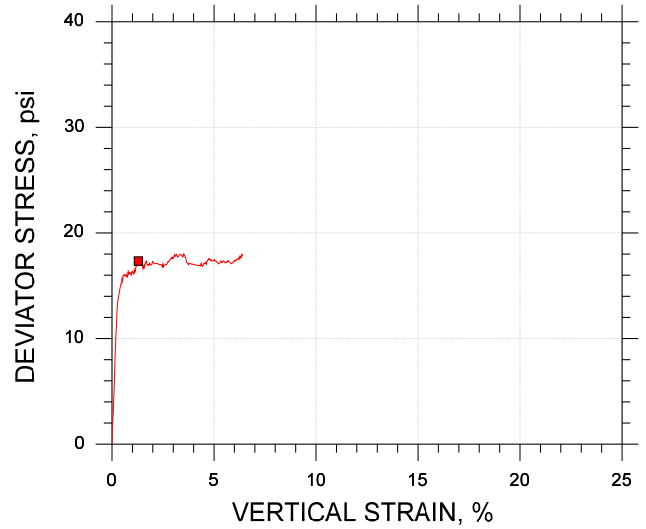
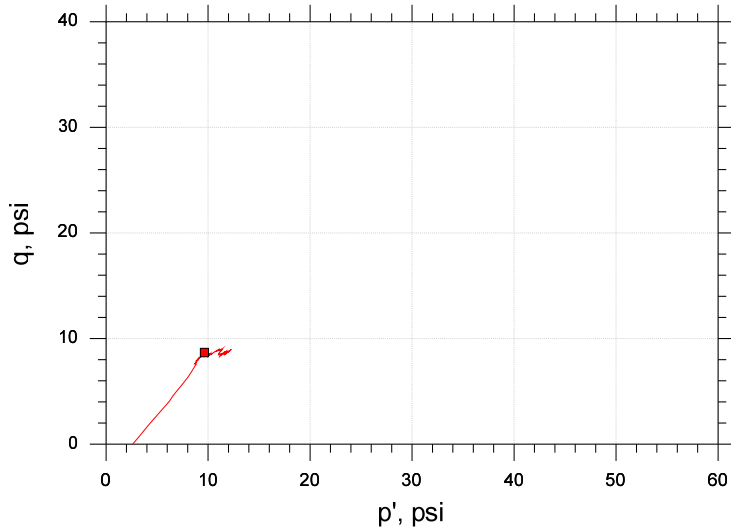
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■	U-1 (19.2)	CU-2-1	12-14	trm	4/1/21	njh	4/9/21	313197-CU-2n.dat

			
	Project: I-395/Rte 9 Connector (Area 3)	Location: Brewer-Eddington, ME	Project No.: GTX-313197
	Boring No.: HB-BE-223	Sample Type: intact	
	Description: Wet, dark gray clay		
	Remarks: System JJ		



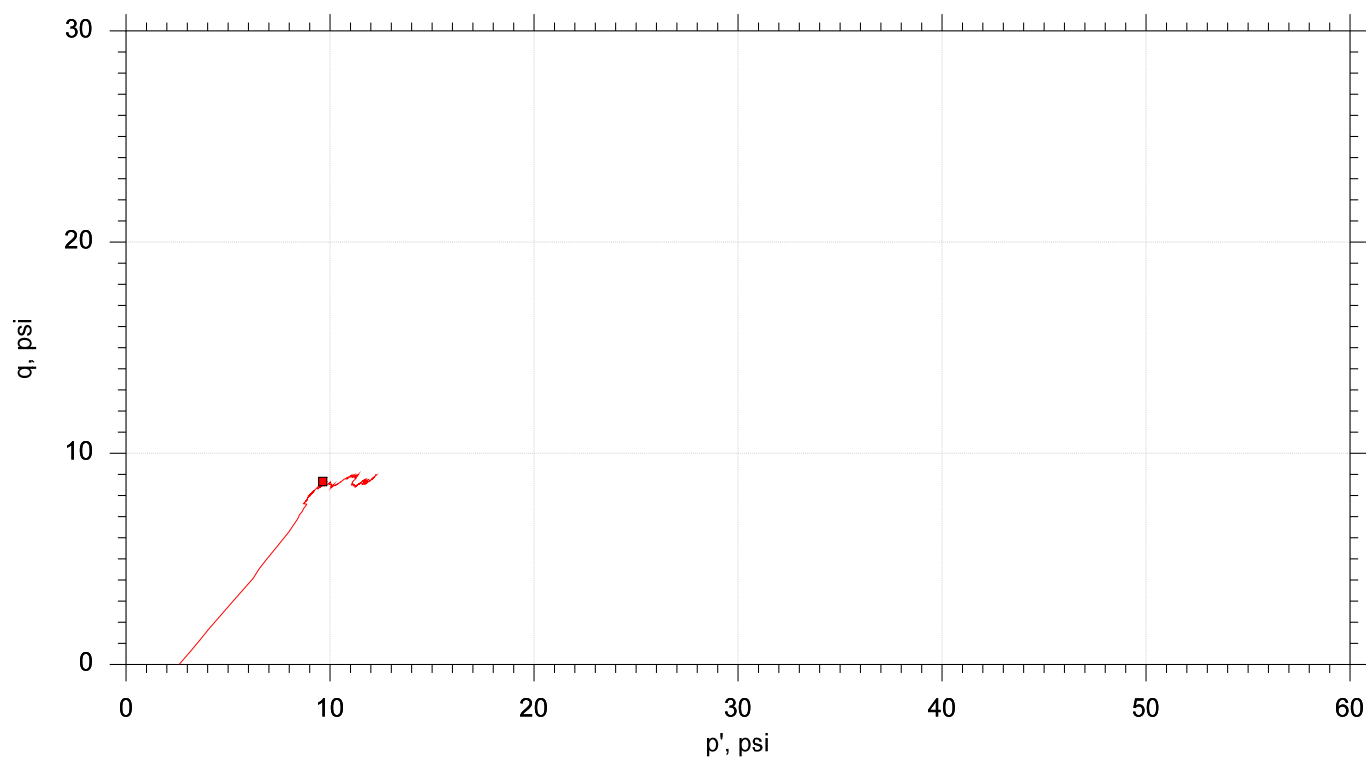
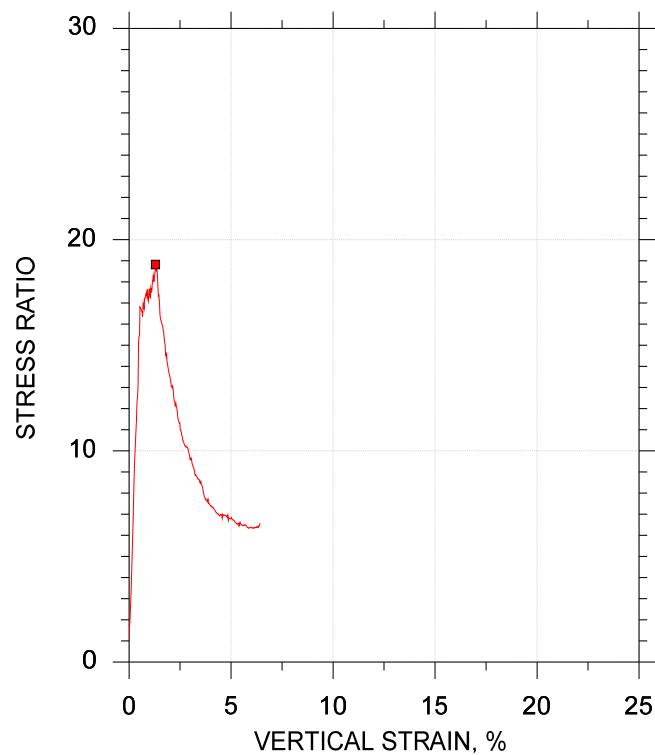
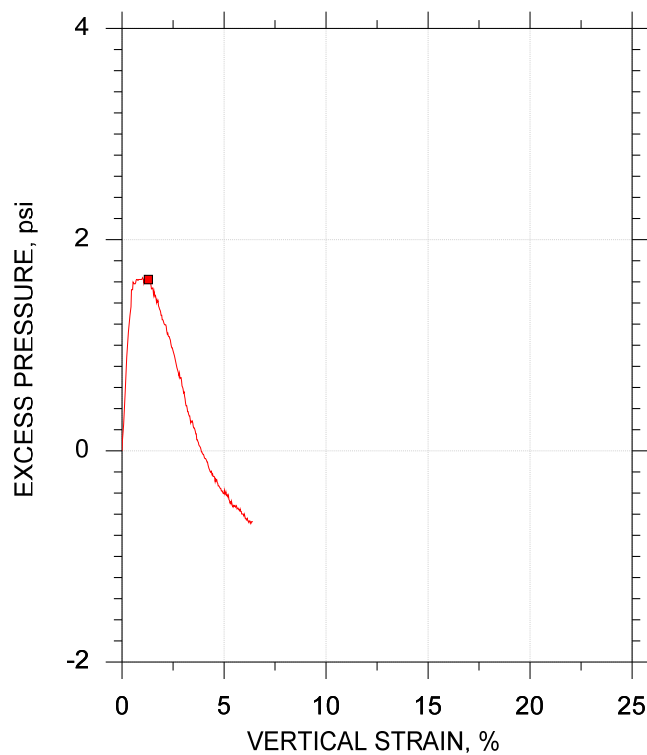
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 3)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313197	
Tested By: trm	Checked By: njh
Boring ID: HE-BE-224	
Preparation: intact	
Description: Moist, dark gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 35	Plastic Limit: 20
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1(21.6")		
Depth, ft		8-10		
Test Number		CU-1-1		
Initial	Height, in	4.410		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	34.2		
	Dry Density, pcf	87.3		
	Saturation (Wet Method), %	99.2		
	Void Ratio	0.931		
Before Shear	Moisture Content, %	34.2		
	Dry Density, pcf	87.7		
	Cross-sectional Area (Method A), in²	3.256		
	Saturation, %	100.0		
	Void Ratio	0.922		
	Back Pressure, psi	150.9		
Vertical Effective Consolidation Stress, psi		2.604		
Horizontal Effective Consolidation Stress, psi		2.604		
Vertical Strain after Consolidation, %		0.005689		
Volumetric Strain after Consolidation, %		0.2378		
Time to 50% Consolidation, min		42.25		
Shear Strength, psi		8.667		
Strain at Failure, %		1.29		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		17.33		
Effective Minor Principal Stress at Failure, psi		0.9725		
Effective Major Principal Stress at Failure, psi		18.31		
B-Value		0.93		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



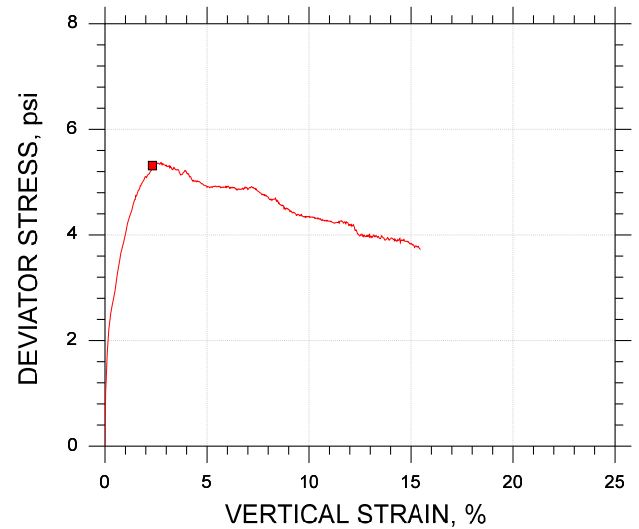
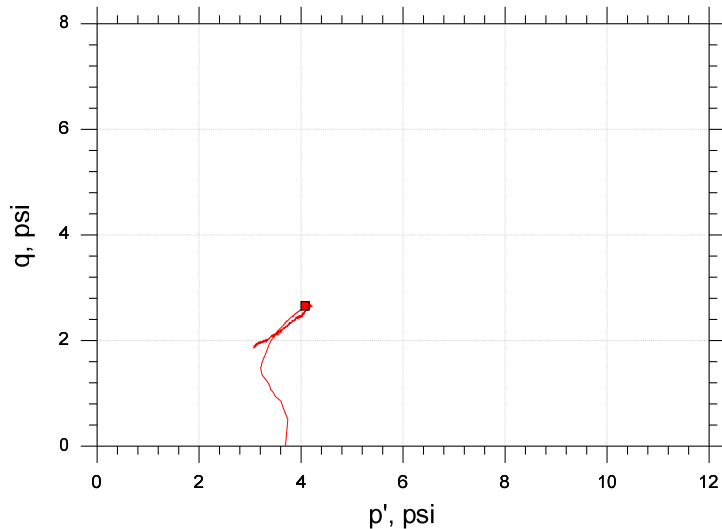
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■	U1(21.6")	CU-1-1	8-10	trm	4/1/21	njh	4/9/21	313197-CU-1n.dat

			
	Project: I-395/Rte 9 Connector (Area 3)	Location: Brewer-Eddington, ME	Project No.: GTX-313197
	Boring No.: HE-BE-224	Sample Type: intact	
	Description: Moist, dark gray clay		
	Remarks: System F		



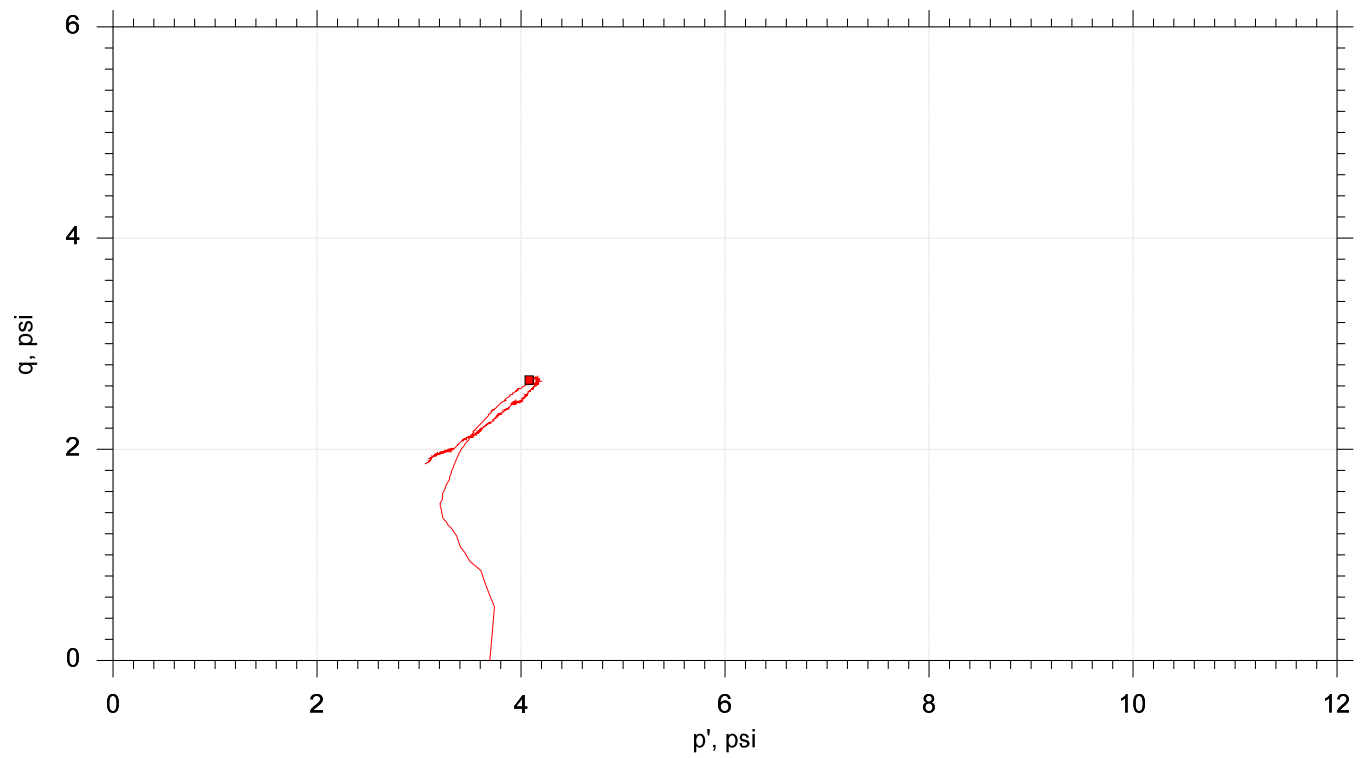
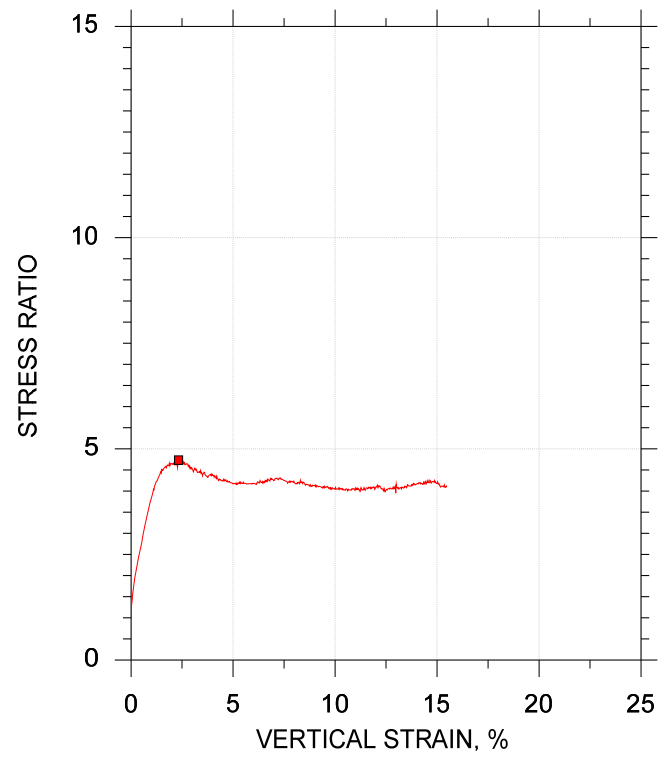
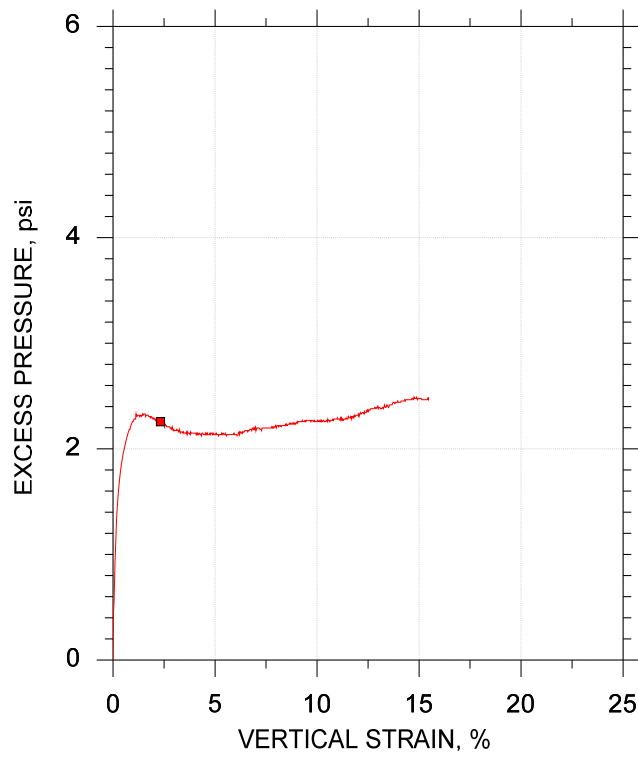
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 4)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313198	
Tested By: md	Checked By: njh
Boring ID: HB-BE-225	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 30	Plastic Limit: 17
Plasticity Index: 13	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		8-10		
Test Number		CU-2		
Initial	Height, in	4.650		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	37.9		
	Dry Density, pcf	80.3		
	Saturation (Wet Method), %	93.3		
	Void Ratio	1.10		
Before Shear	Moisture Content, %	38.5		
	Dry Density, pcf	82.7		
	Cross-sectional Area (Method A), in ²	3.208		
	Saturation, %	100.0		
	Void Ratio	1.04		
	Back Pressure, psi	160.9		
Vertical Effective Consolidation Stress, psi		3.683		
Horizontal Effective Consolidation Stress, psi		3.693		
Vertical Strain after Consolidation, %		0.1417		
Volumetric Strain after Consolidation, %		0.2359		
Time to 50% Consolidation, min		20.25		
Shear Strength, psi		2.657		
Strain at Failure, %		2.33		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		5.313		
Effective Minor Principal Stress at Failure, psi		1.423		
Effective Major Principal Stress at Failure, psi		6.736		
B-Value		0.97		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



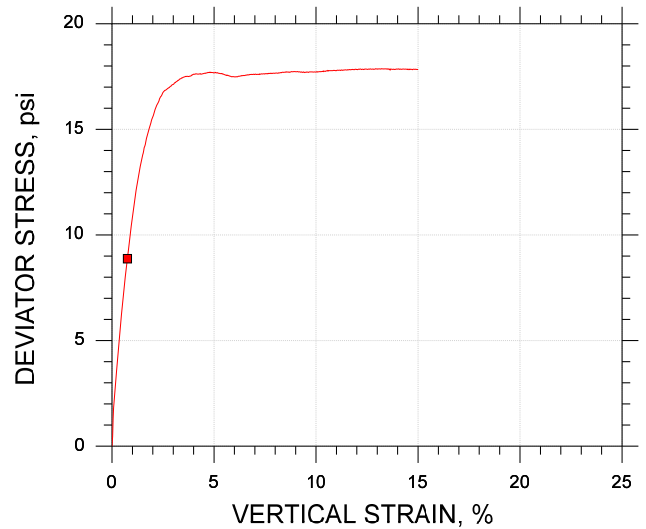
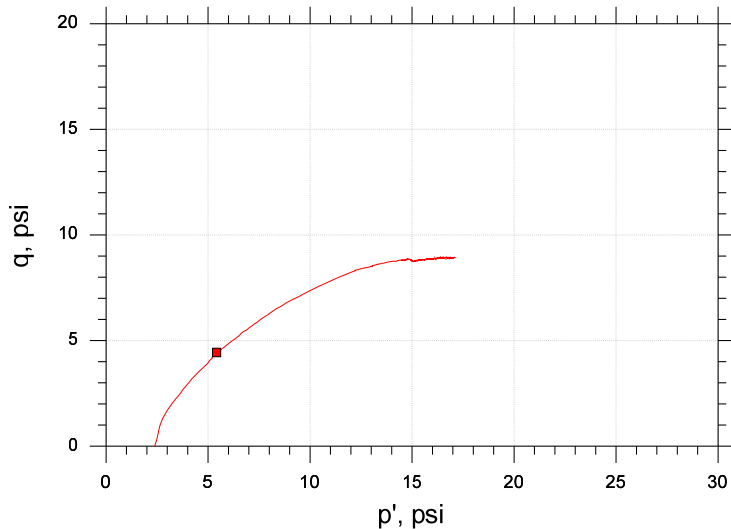
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■	U1	CU-2	8-10	md	03/16/21	njh	4/9/21	313198-CU-2n.dat

			
	Project: I-395/Rte 9 Connector (Area 4)	Location: Brewer-Eddington, ME	Project No.: GTX-313198
	Boring No.: HB-BE-225	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System F		



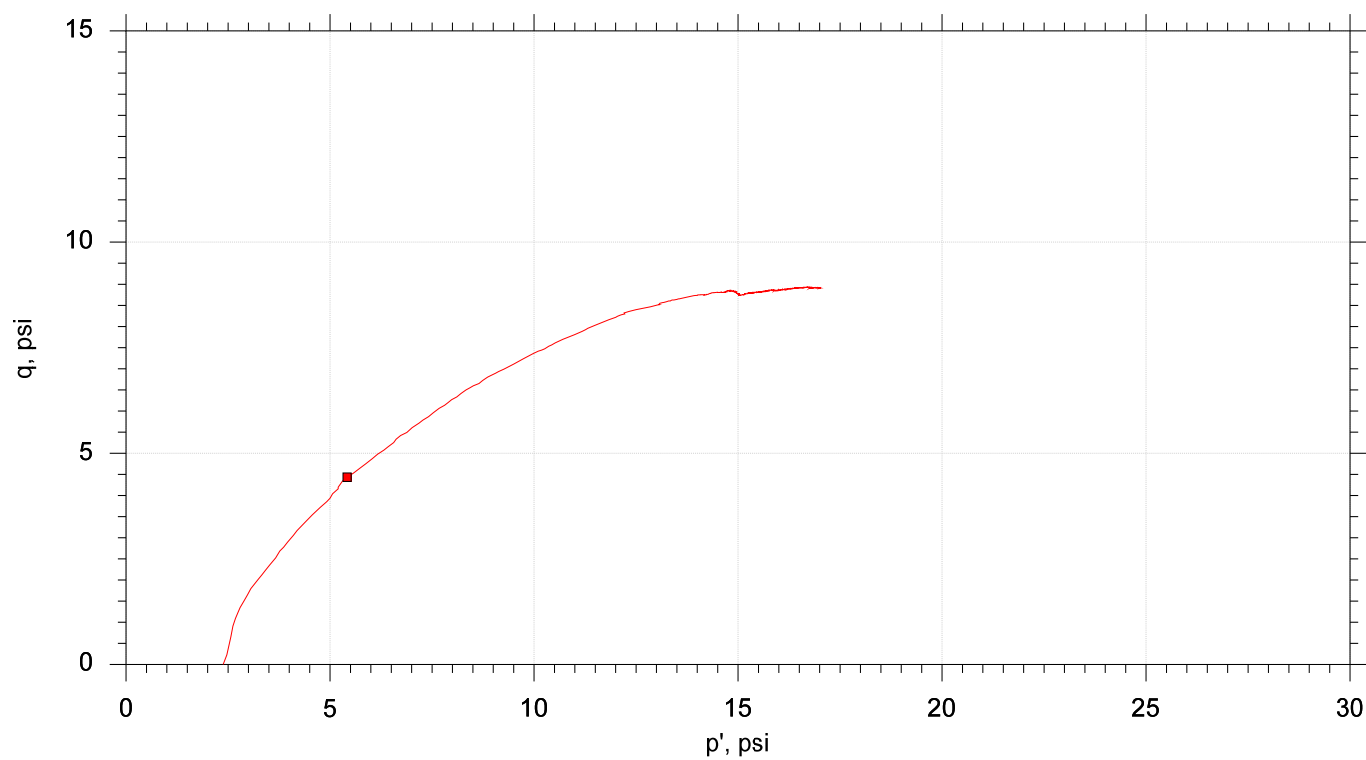
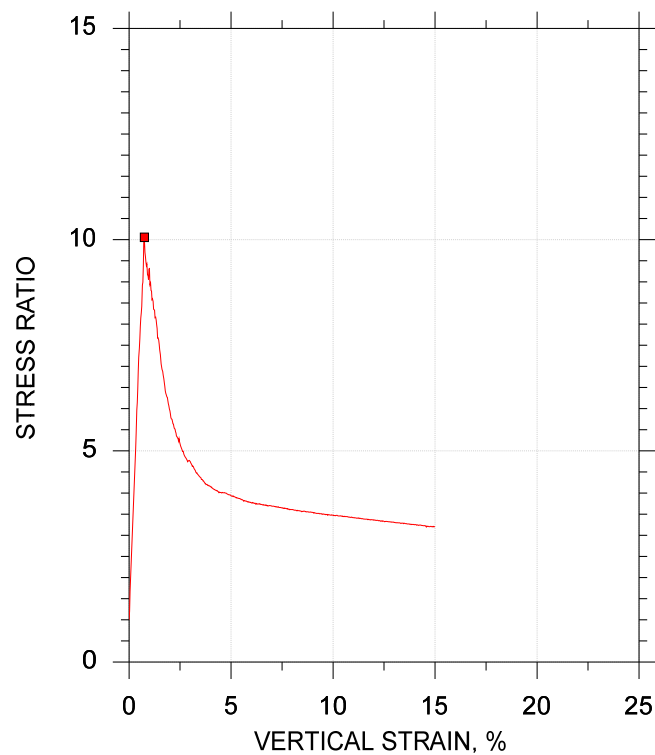
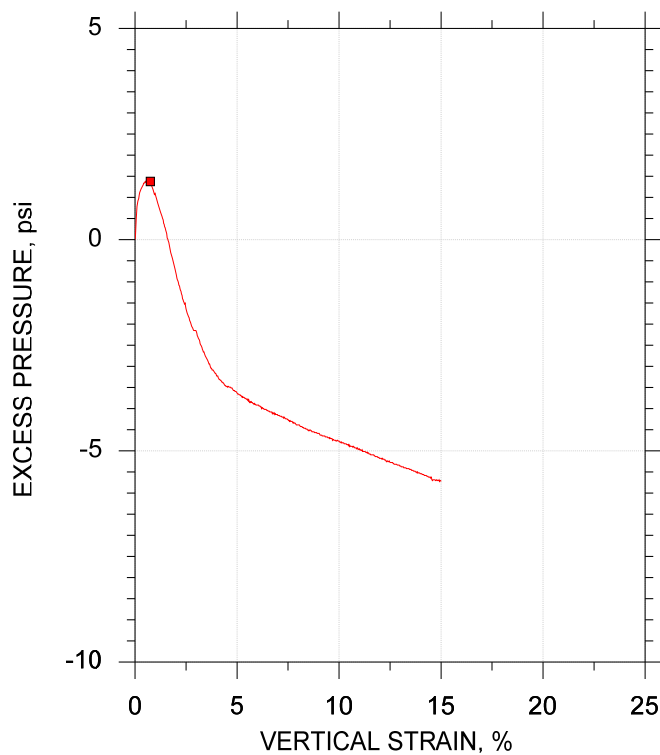
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 4)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-313198	
Tested By: md	Checked By: njh
Boring ID: HB-BB-226	
Preparation: intact	
Description: Moist, olive gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 33	Plastic Limit: 18
Plasticity Index: 15	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		6-8		
Test Number		CU-1-1		
Initial	Height, in	4.450		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	30.7		
	Dry Density, pcf	92.2		
	Saturation (Wet Method), %	100.0		
	Void Ratio	0.829		
Before Shear	Moisture Content, %	29.3		
	Dry Density, pcf	94.1		
	Cross-sectional Area (Method A), in ²	3.225		
	Saturation, %	100.0		
	Void Ratio	0.791		
	Back Pressure, psi	150.9		
Vertical Effective Consolidation Stress, psi		2.381		
Horizontal Effective Consolidation Stress, psi		2.380		
Vertical Strain after Consolidation, %		0.02659		
Volumetric Strain after Consolidation, %		-0.09661		
Time to 50% Consolidation, min		---		
Shear Strength, psi		4.436		
Strain at Failure, %		0.751		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psi		8.873		
Effective Minor Principal Stress at Failure, psi		0.9798		
Effective Major Principal Stress at Failure, psi		9.853		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



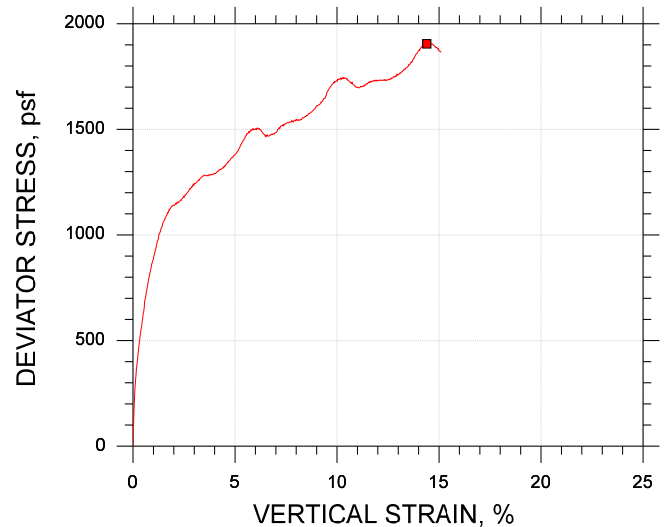
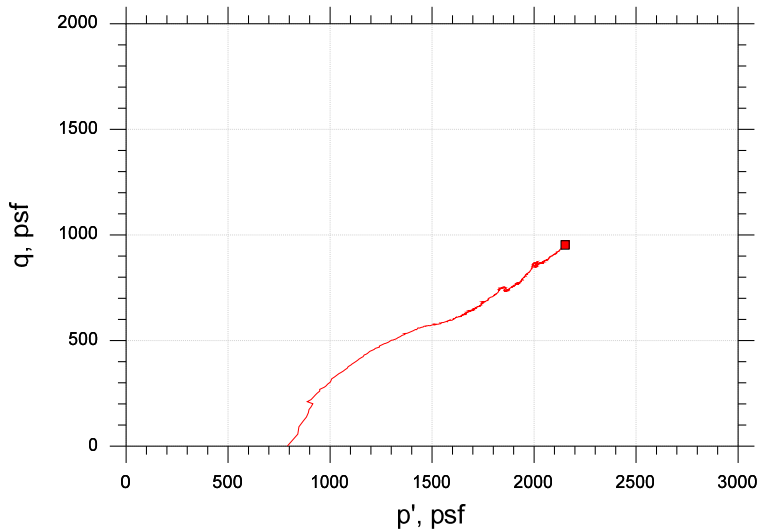
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■	U1	CU-1-1	6-8	md	03/16/21	njh	4/9/21	313198-CU-1.dat

			
	Project: I-395/Rte 9 Connector (Area 4)	Location: Brewer-Eddington, ME	Project No.: GTX-313198
	Boring No.: HB-BB-226	Sample Type: intact	
	Description: Moist, olive gray clay		
	Remarks: System KK		



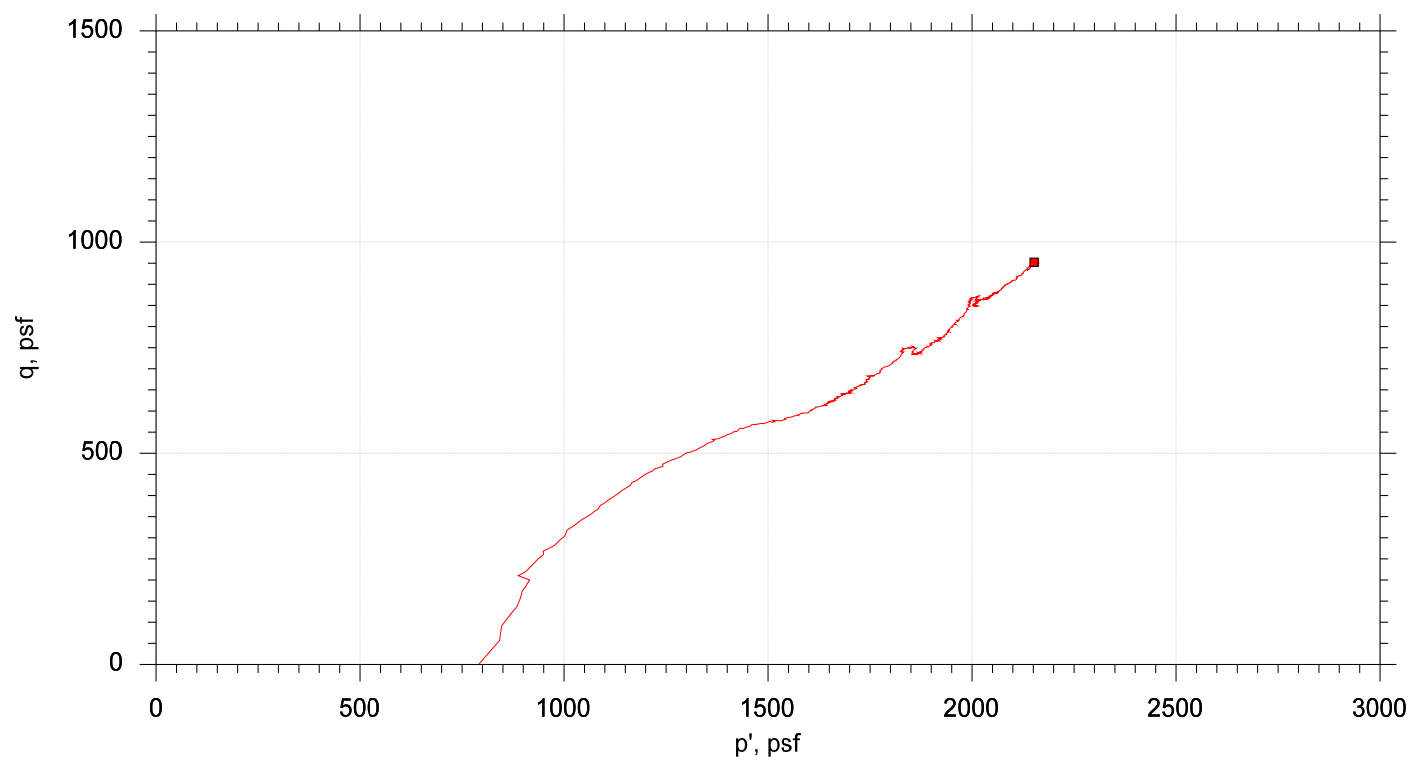
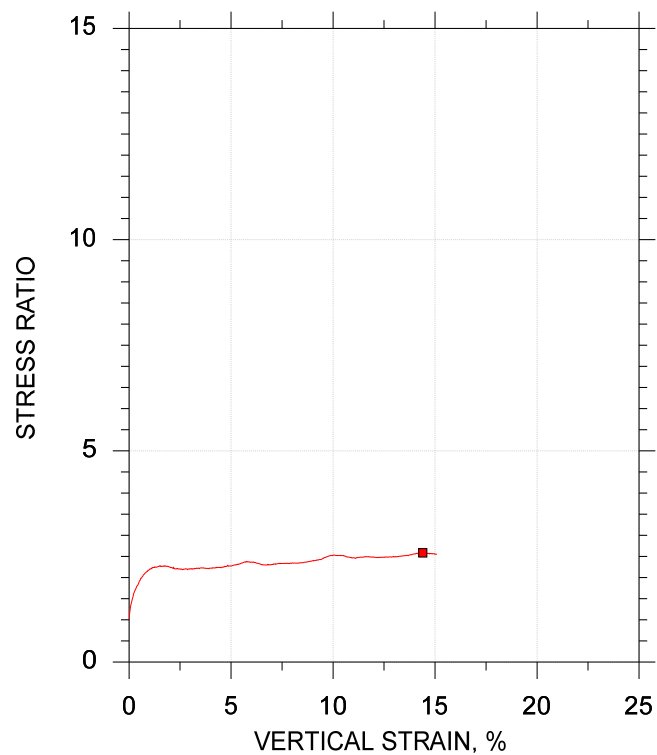
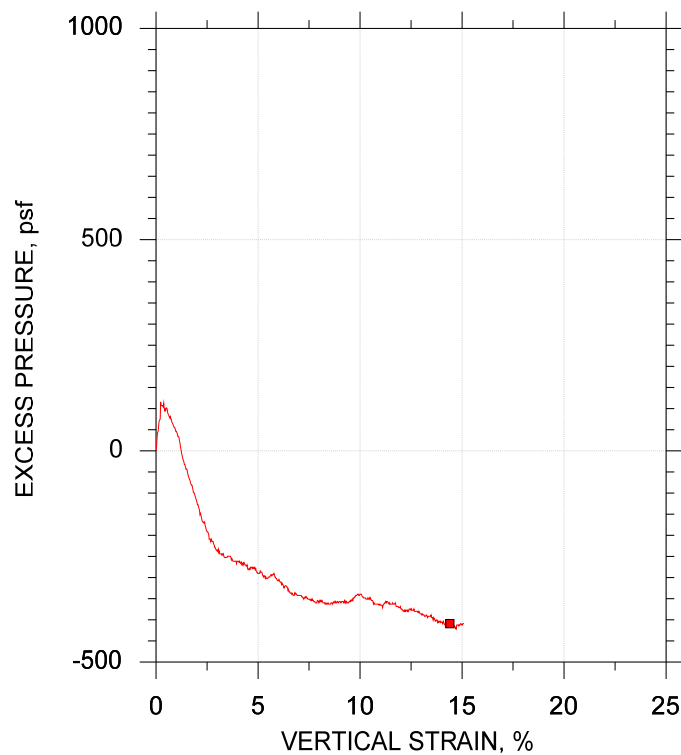
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connecotr (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-239	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 34	Plastic Limit: 17
Plasticity Index: 17	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		13-15 ft		
Test Number		CU-16-1		
Initial	Height, in	4.640		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	37.4		
	Dry Density, pcf	83.4		
	Saturation (Wet Method), %	99.1		
	Void Ratio	1.02		
Before Shear	Moisture Content, %	37.8		
	Dry Density, pcf	83.4		
	Cross-sectional Area (Method A), in ²	3.249		
	Saturation, %	100.0		
	Void Ratio	1.02		
	Back Pressure, psf	2.197e+004		
Vertical Effective Consolidation Stress, psf		789.1		
Horizontal Effective Consolidation Stress, psf		790.5		
Vertical Strain after Consolidation, %		0.1848		
Volumetric Strain after Consolidation, %		0.4517		
Time to 50% Consolidation, min		9.000		
Shear Strength, psf		952.4		
Strain at Failure, %		14.4		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1905.		
Effective Minor Principal Stress at Failure, psf		1199.		
Effective Major Principal Stress at Failure, psf		3104.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U1	CU-16-1	13-15 ft	trm	3/9/21	mcm	4/1/21	312665-CU-16-1m.dat

			
	Project: I-395/Rte 9 Connecotr (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-239	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System LL		



Client: Haley & Aldrich, Inc.

Project Name: I-395/Rte 9 Connector (Area 1)

Project Location: Brewer-Eddington, ME

Project Number: GTX-312665

Tested By: trm

Checked By: mcm

Boring ID: HB-BE-239

Preparation: intact

Description: Moist, gray clay

Classification: ---

Group Symbol: ---

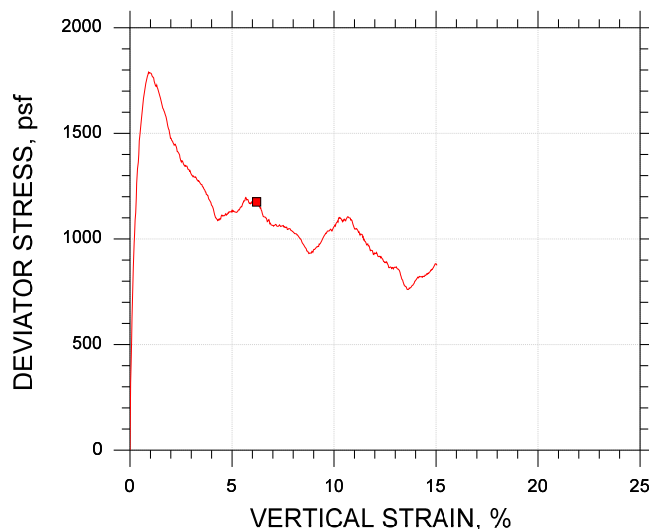
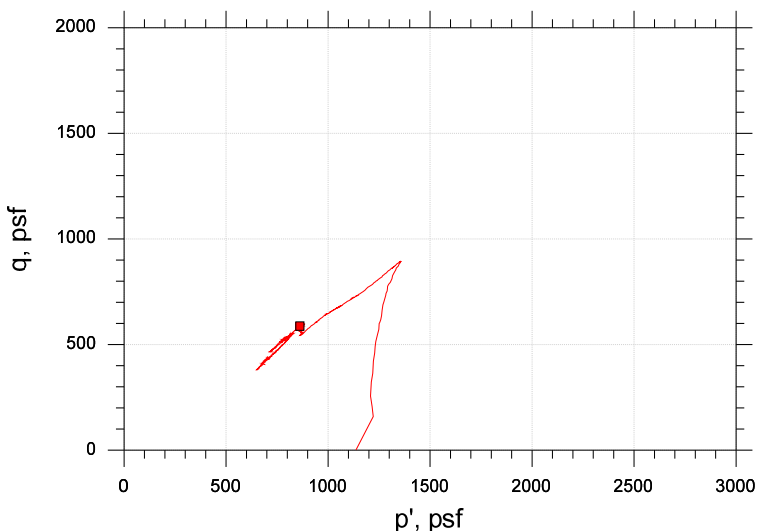
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Plastic Limit: 16

Plasticity Index: 18

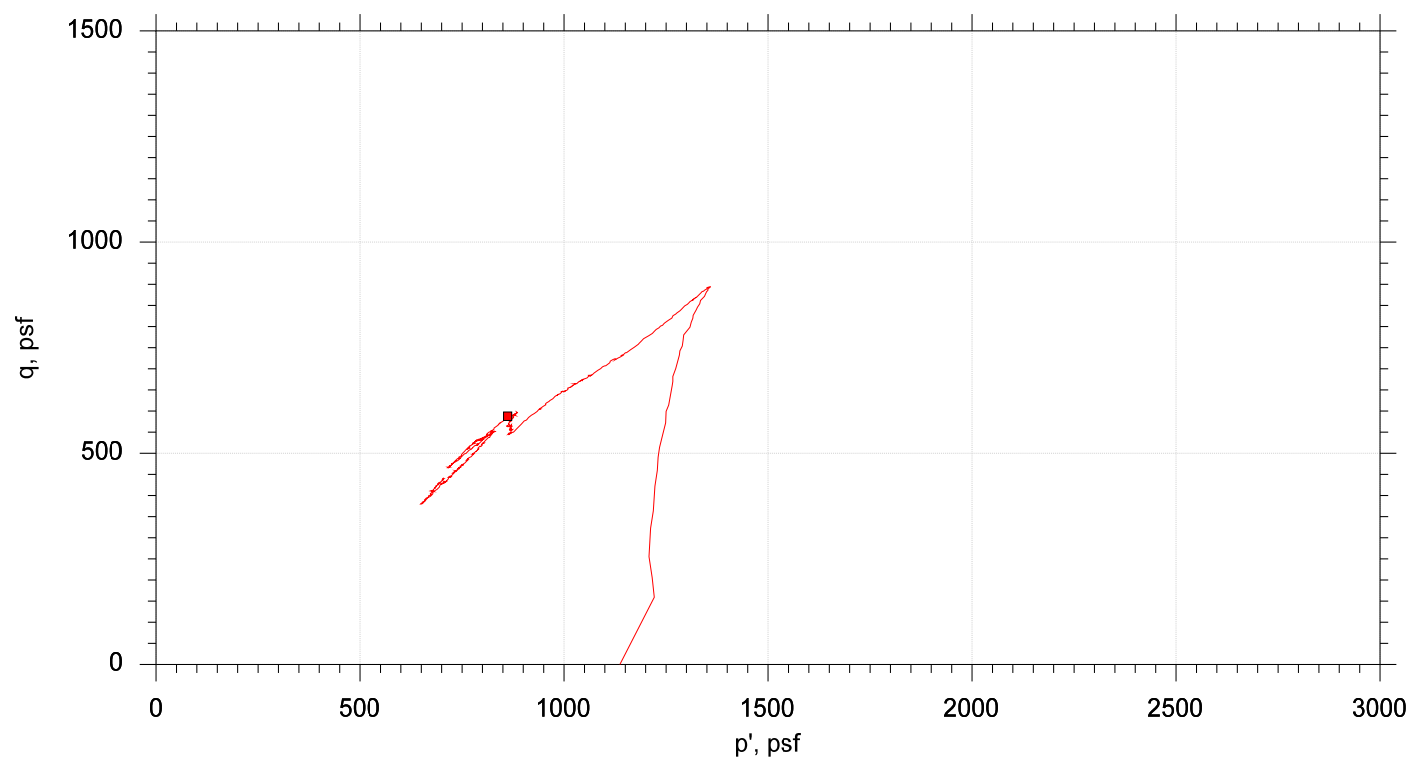
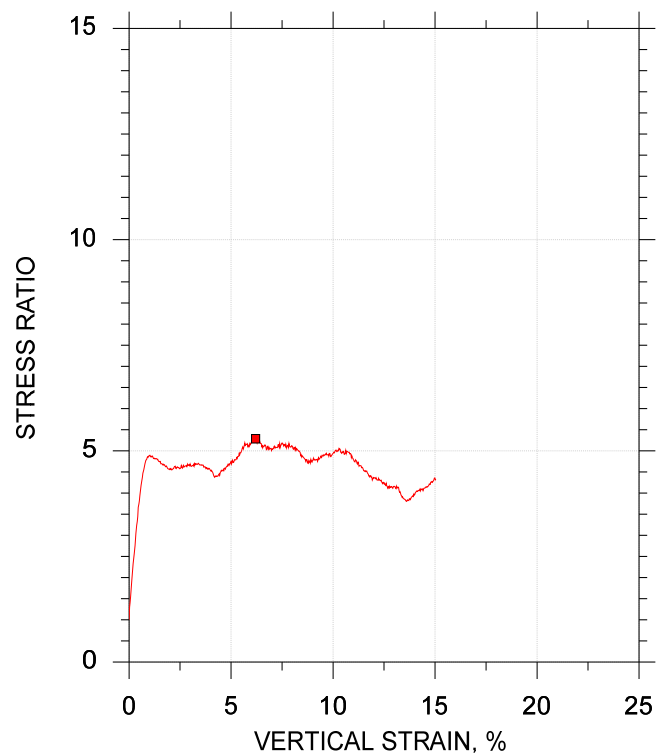
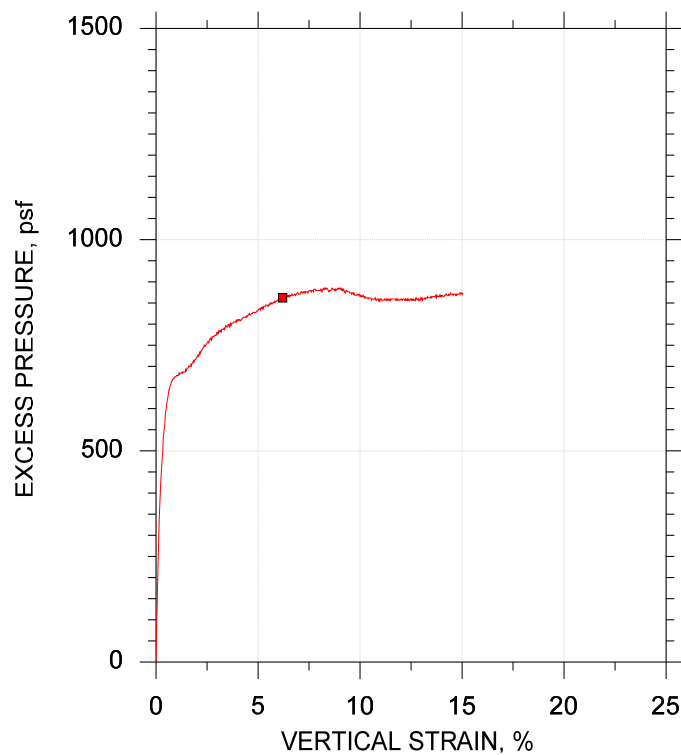
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U2		
Depth, ft		23-25 ft		
Test Number		CU-12-1		
Initial	Height, in	4.300		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	37.1		
	Dry Density, pcf	82.8		
	Saturation (Wet Method), %	96.6		
	Void Ratio	1.04		
Before Shear	Moisture Content, %	37.4		
	Dry Density, pcf	83.8		
	Cross-sectional Area (Method A), in ²	3.245		
	Saturation, %	100.0		
	Void Ratio	1.01		
	Back Pressure, psf	2.315e+004		
Vertical Effective Consolidation Stress, psf		1132.		
Horizontal Effective Consolidation Stress, psf		1137.		
Vertical Strain after Consolidation, %		0.4163		
Volumetric Strain after Consolidation, %		0.8250		
Time to 50% Consolidation, min		7.840		
Shear Strength, psf		587.7		
Strain at Failure, %		6.20		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1175.		
Effective Minor Principal Stress at Failure, psf		274.2		
Effective Major Principal Stress at Failure, psf		1450.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



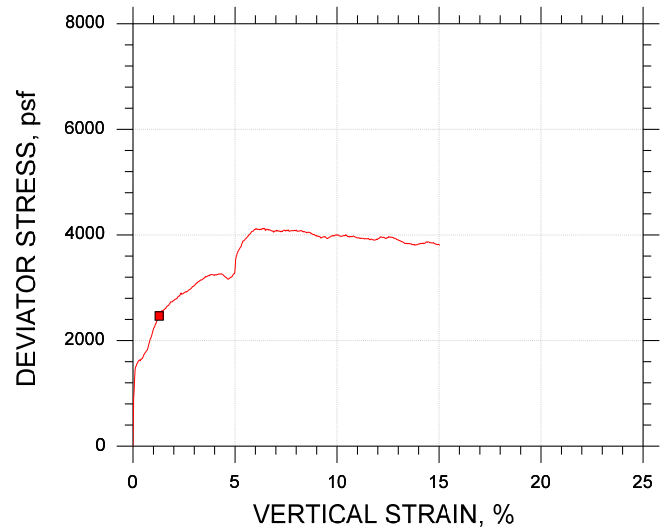
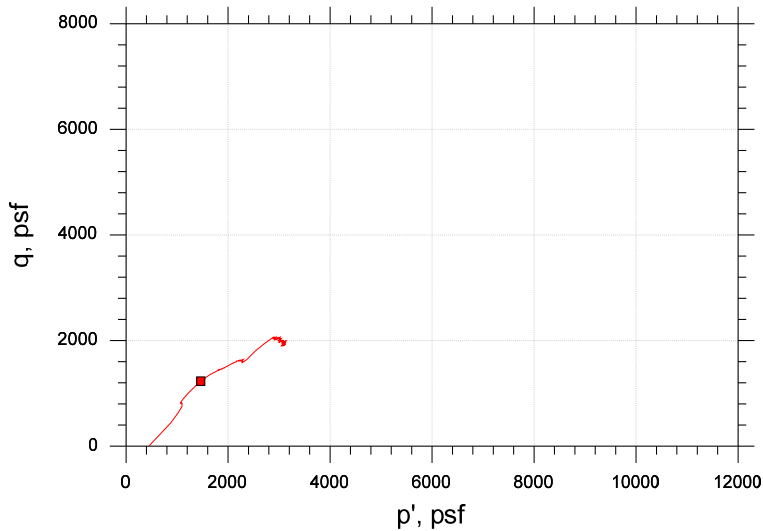
	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U2	CU-12-1	23-25 ft	trm	3/3/21	mcm	3/31/21	312665-CU-12-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-239	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System X		



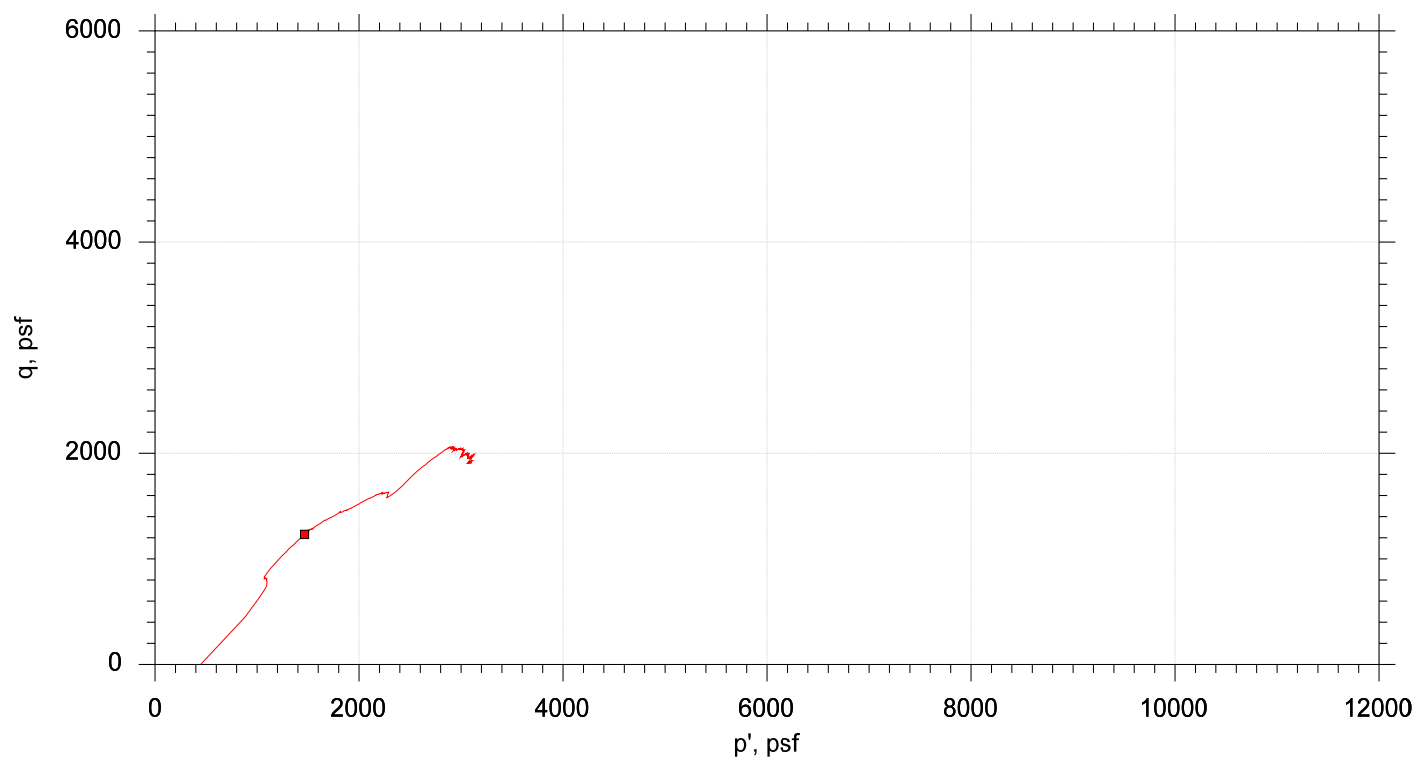
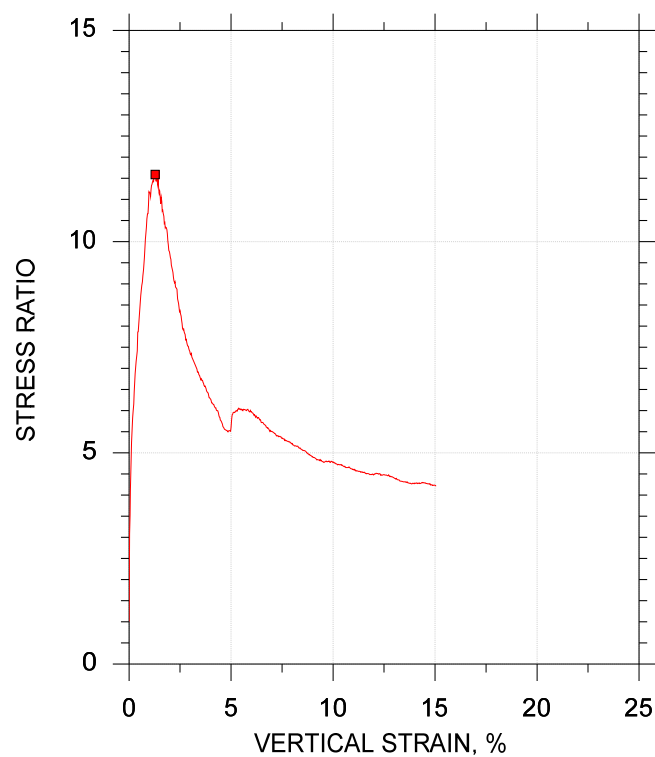
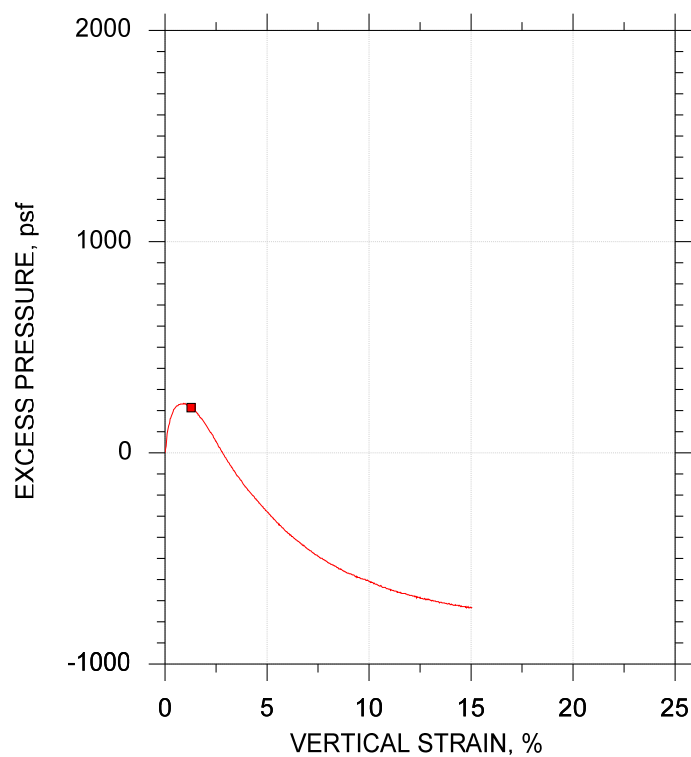
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-240	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 30	Plastic Limit: 18
Plasticity Index: 12	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



Symbol	■			
Sample ID	U1			
Depth, ft	10-12 ft			
Test Number	CU-13-1			
Initial	Height, in	4.580		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	27.4		
	Dry Density, pcf	96.1		
	Saturation (Wet Method), %	98.2		
	Void Ratio	0.753		
Before Shear	Moisture Content, %	27.1		
	Dry Density, pcf	97.3		
	Cross-sectional Area (Method A), in²	3.238		
	Saturation, %	100.0		
	Void Ratio	0.732		
	Back Pressure, psf	2.174e+004		
Vertical Effective Consolidation Stress, psf		450.3		
Horizontal Effective Consolidation Stress, psf		446.9		
Vertical Strain after Consolidation, %		0.001119		
Volumetric Strain after Consolidation, %		0.4213		
Time to 50% Consolidation, min		0.3600		
Shear Strength, psf		1233.		
Strain at Failure, %		1.28		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		2465.		
Effective Minor Principal Stress at Failure, psf		232.8		
Effective Major Principal Stress at Failure, psf		2698.		
B-Value		0.96		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System X				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U1	CU-13-1	10-12 ft	trm	3/3/21	mcm	4/1/21	312665-CU-13-1m.dat

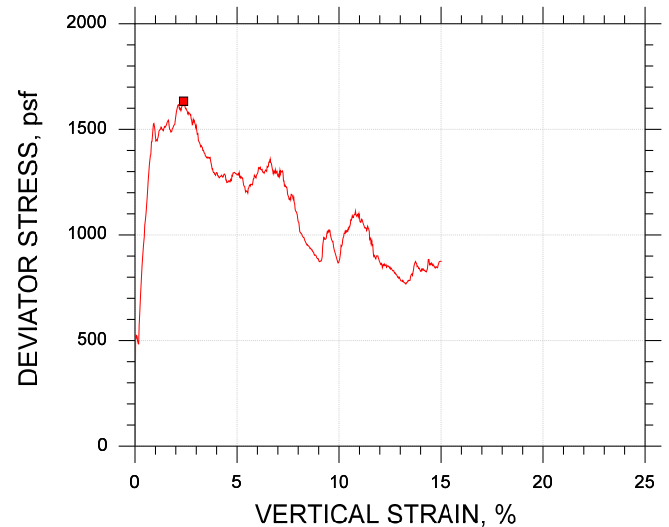
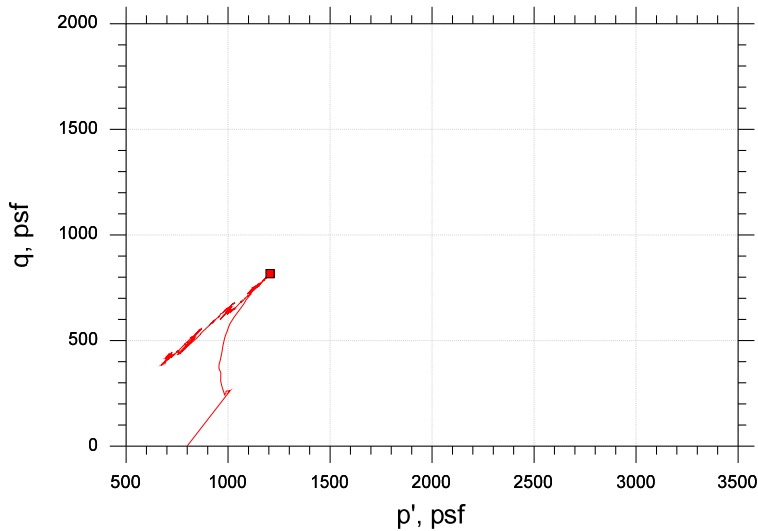


Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
Boring No.: HB-BE-240	Sample Type: intact	
Description: Moist, gray clay		
Remarks: System X		



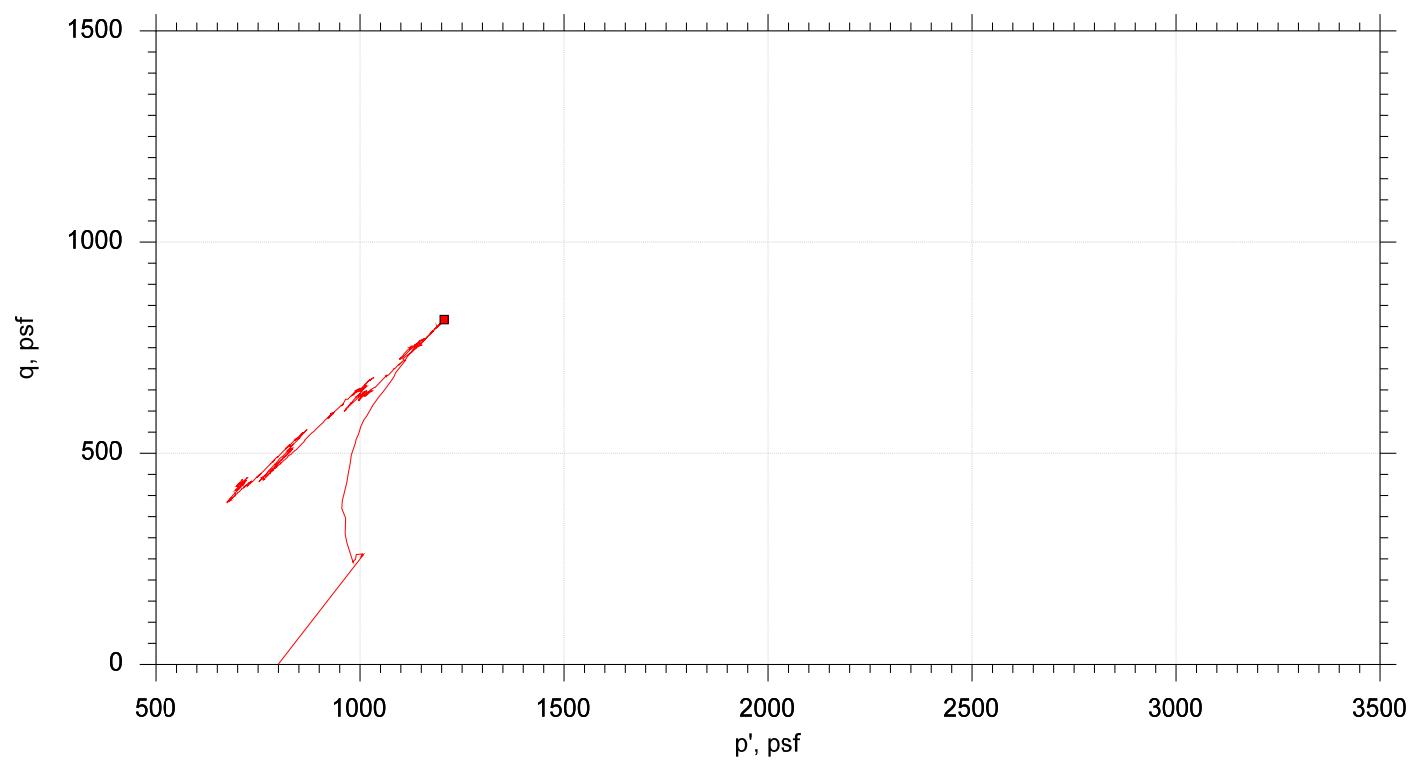
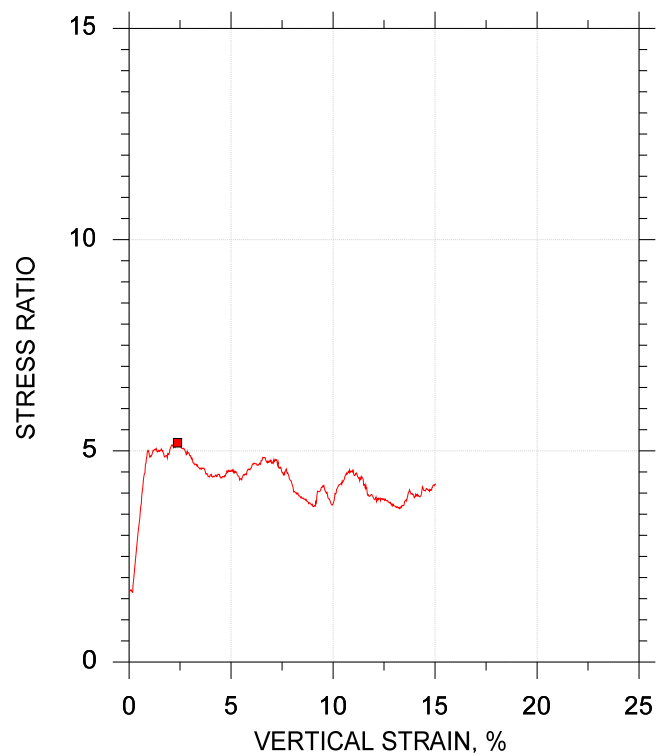
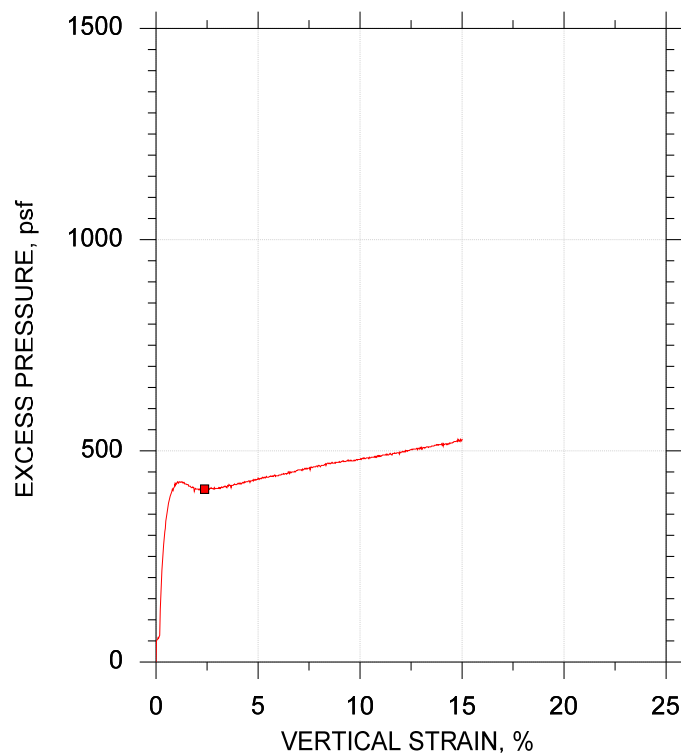
Client: Haley & Aldrich, Inc.	
Project Name: I-395/Rte 9 Connector (Area 1)	
Project Location: Brewer-Eddington, ME	
Project Number: GTX-312665	
Tested By: trm	Checked By: mcm
Boring ID: HB-BE-240	
Preparation: intact	
Description: Moist, gray clay	
Classification: ---	
Group Symbol: ---	
Liquid Limit: 36	Plastic Limit: 20
Plasticity Index: 16	Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U2		
Depth, ft		20-22 ft		
Test Number		CU-8-1		
Initial	Height, in	4.500		
	Diameter, in	2.040		
	Moisture Content (from Cuttings), %	37.9		
	Dry Density, pcf	80.0		
	Saturation (Wet Method), %	92.5		
	Void Ratio	1.11		
Before Shear	Moisture Content, %	40.5		
	Dry Density, pcf	80.5		
	Cross-sectional Area (Method A), in ²	3.250		
	Saturation, %	100.0		
	Void Ratio	1.09		
	Back Pressure, psf	2.317e+004		
Vertical Effective Consolidation Stress, psf		799.5		
Horizontal Effective Consolidation Stress, psf		799.0		
Vertical Strain after Consolidation, %		0.02716		
Volumetric Strain after Consolidation, %		0.4966		
Time to 50% Consolidation, min		17.64		
Shear Strength, psf		816.7		
Strain at Failure, %		2.38		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		1633.		
Effective Minor Principal Stress at Failure, psf		389.4		
Effective Major Principal Stress at Failure, psf		2023.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and ϕ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U2	CU-8-1	20-22 ft	trm	2/26/21	mcm	3/16/21	312665-CU-8-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-240	Sample Type: intact	
	Description: Moist, gray clay		
	Remarks: System F		



Client: Haley & Aldrich, Inc.

Project Name: I-395/Rte 9 Connector (Area 1)

Project Location: Brewer-Eddington, ME

Project Number: GTX-312665

Tested By: trm

Checked By: mcm

Boring ID: HB-BE-242A

Preparation: Intact

Description: Moist, gray clay

Classification: ---

Group Symbol: ---

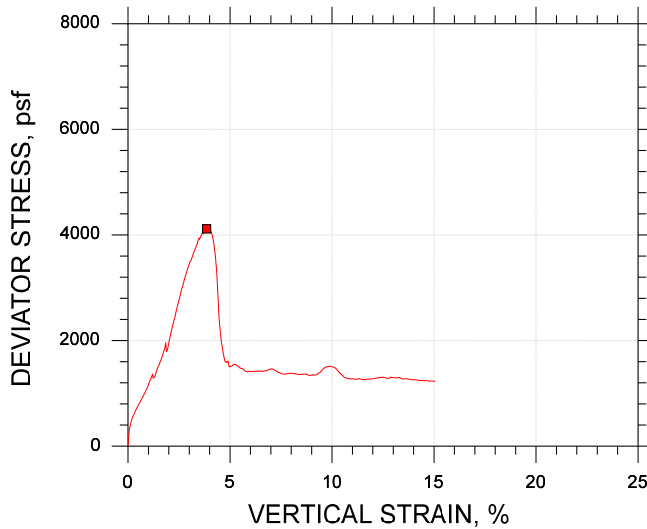
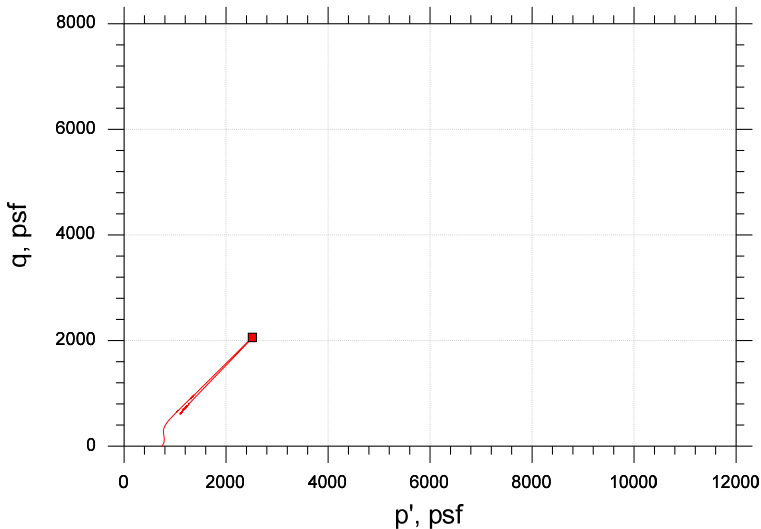
Liquid Limit: 30

Plastic Limit: 16

Plasticity Index: 14

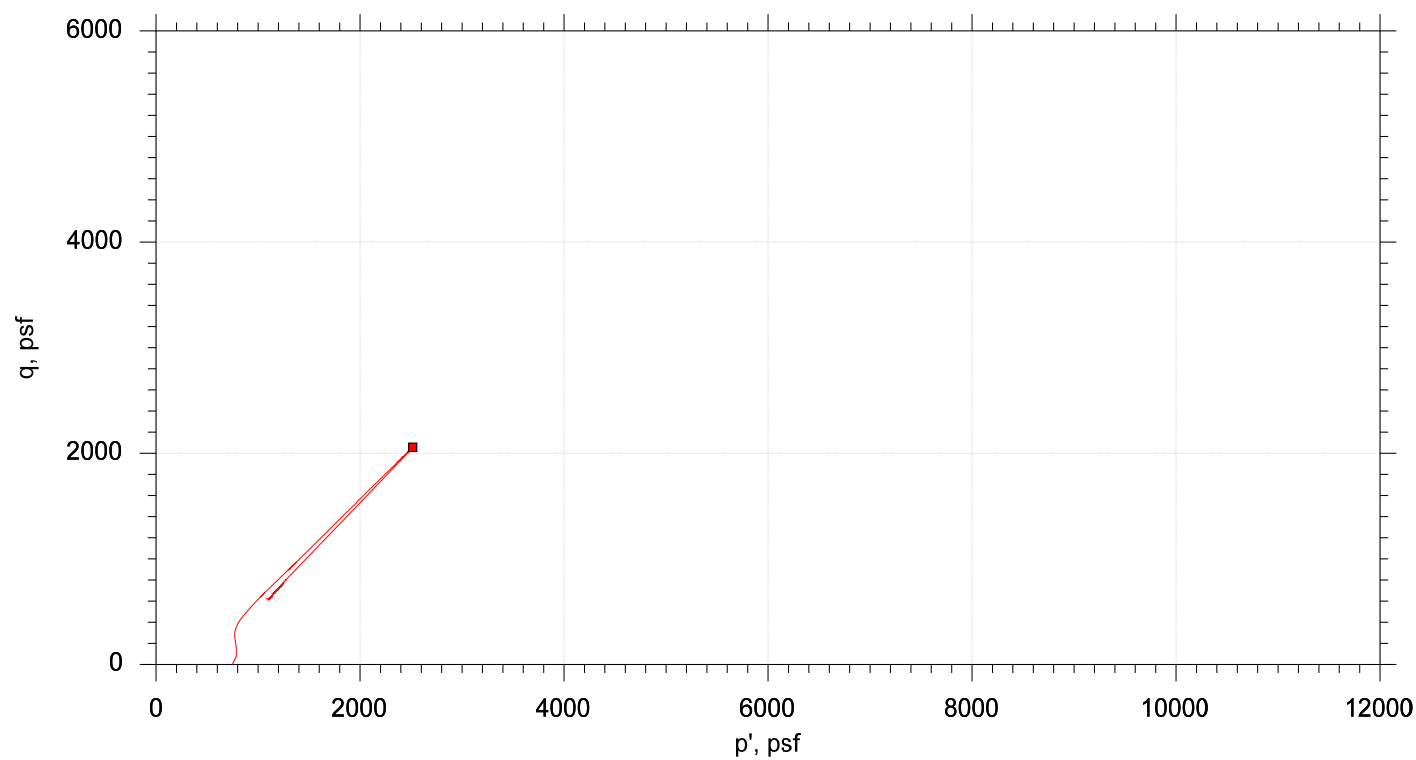
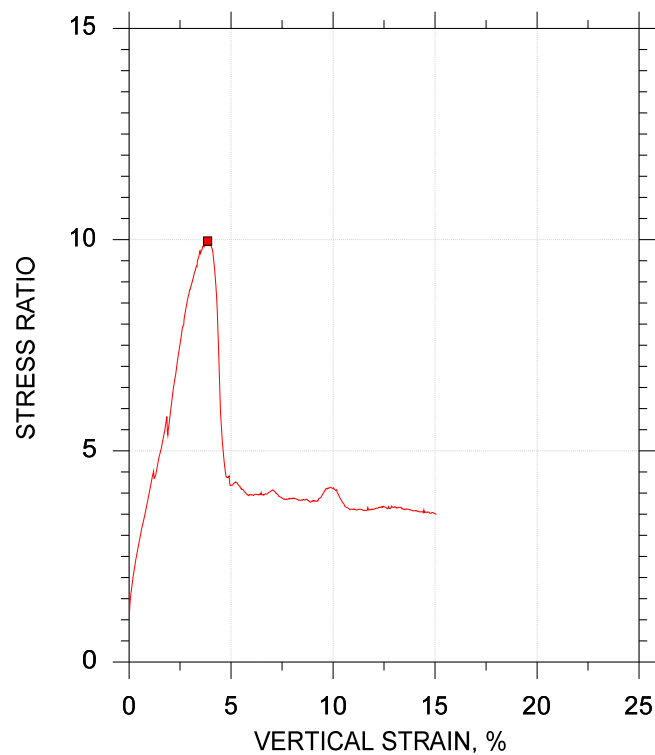
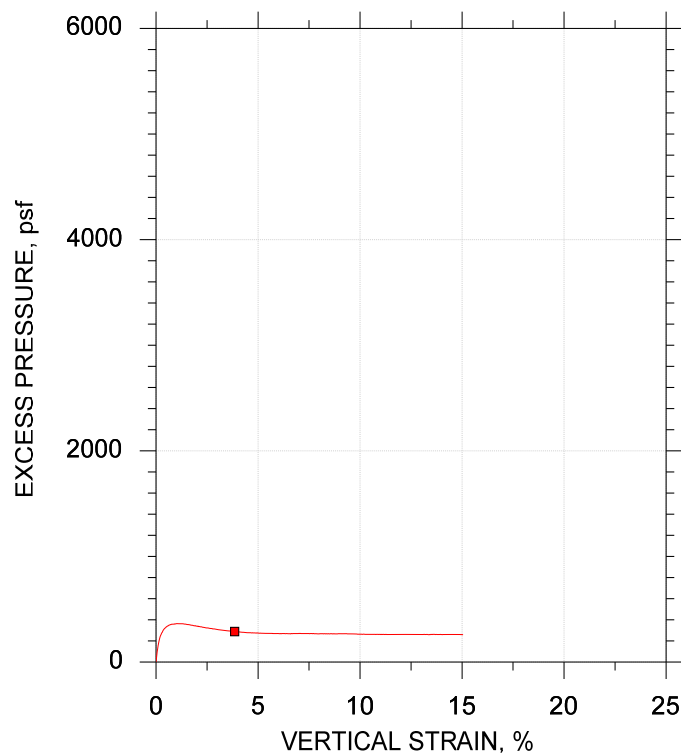
Estimated Specific Gravity: 2.7

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767




Symbol		■		
Sample ID		U1		
Depth, ft		9-11 ft		
Test Number		CU-9-1		
Initial	Height, in	4.230		
	Diameter, in	2.030		
	Moisture Content (from Cuttings), %	31.2		
	Dry Density, pcf	91.4		
	Saturation (Wet Method), %	99.6		
	Void Ratio	0.845		
Before Shear	Moisture Content, %	30.8		
	Dry Density, pcf	92.0		
	Cross-sectional Area (Method A), in ²	3.220		
	Saturation, %	100.0		
	Void Ratio	0.831		
	Back Pressure, psf	2.171e+004		
Vertical Effective Consolidation Stress, psf		746.6		
Horizontal Effective Consolidation Stress, psf		748.0		
Vertical Strain after Consolidation, %		0.1251		
Volumetric Strain after Consolidation, %		0.4475		
Time to 50% Consolidation, min		42.25		
Shear Strength, psf		2058.		
Strain at Failure, %		3.85		
Strain Rate, %/min		0.01600		
Deviator Stress at Failure, psf		4115.		
Effective Minor Principal Stress at Failure, psf		458.9		
Effective Major Principal Stress at Failure, psf		4574.		
B-Value		0.95		
Notes: - Before Shear Saturation set to 100% for phase calculation. - Moisture Content determined by ASTM D2216. - Atterberg Limits determined by ASTM D4318. - Deviator Stress includes membrane correction. - Values for c and φ determined from best-fit straight line for the specific test conditions. Actual strength parameters may vary and should be determined by an engineer for site conditions.				
Remarks:				
System F				

CONSOLIDATED UNDRAINED TRIAXIAL TEST by ASTM D4767

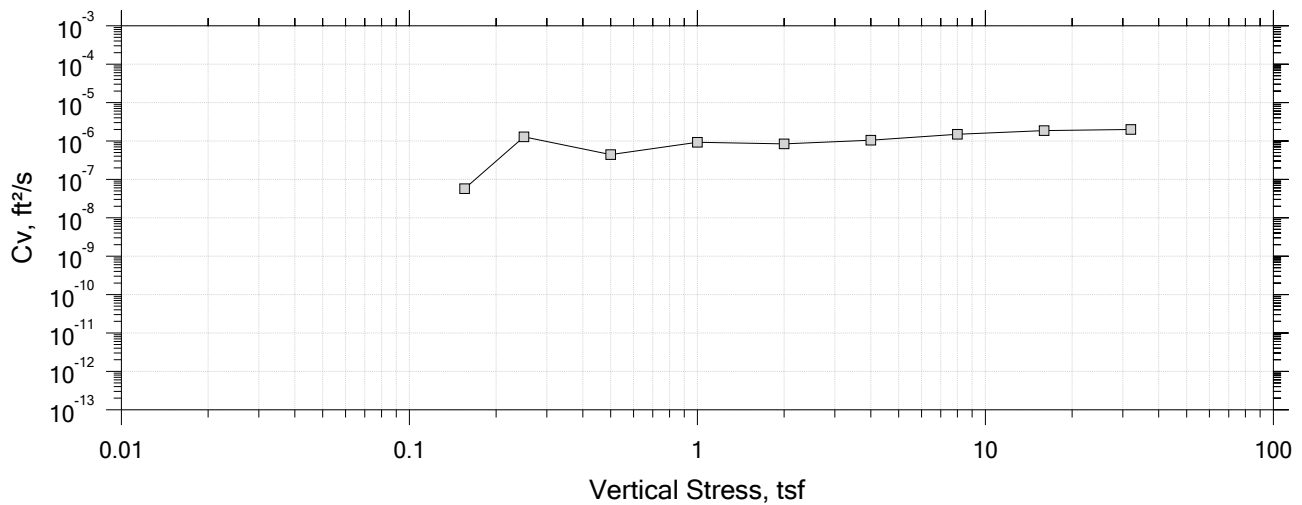
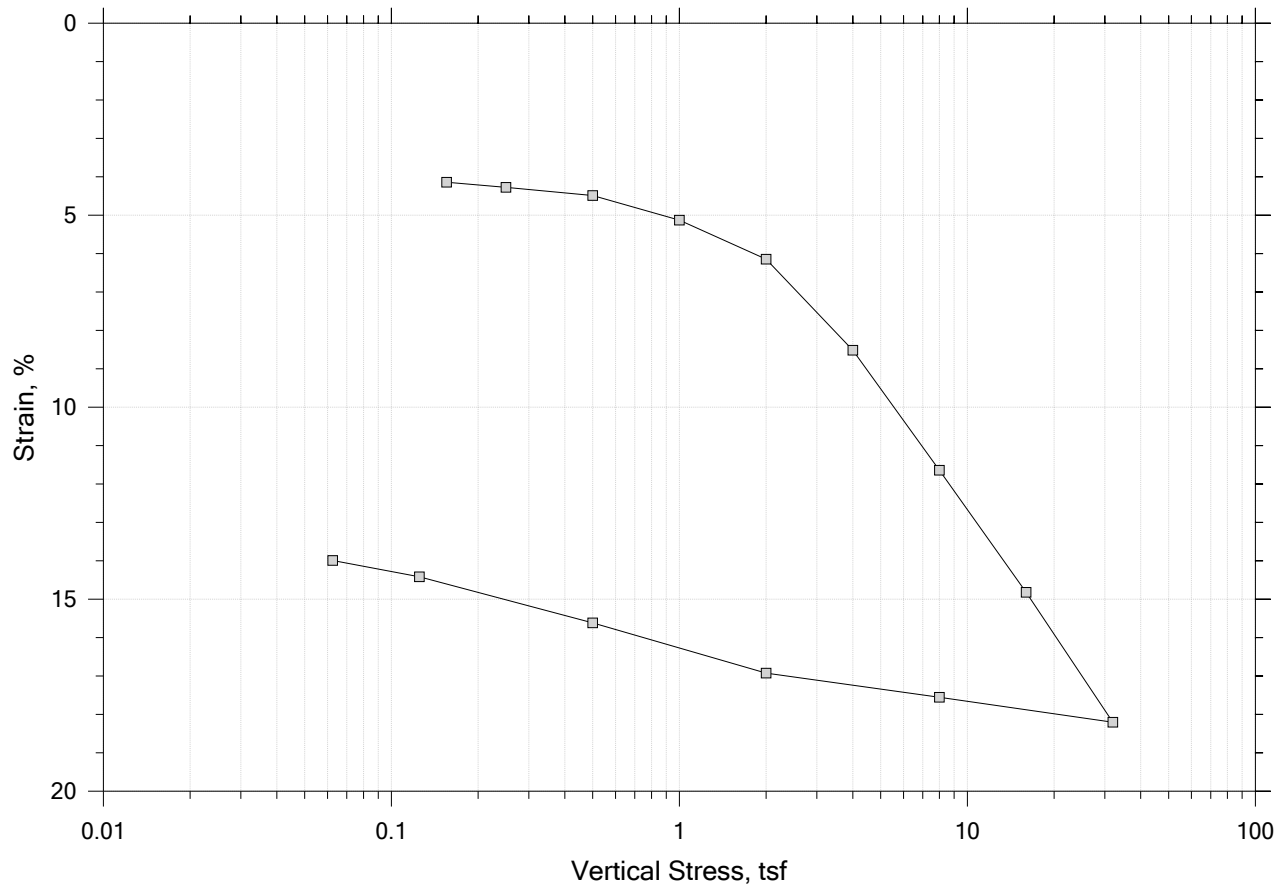



	Sample No.	Test No.	Depth	Tested By	Test Date	Checked By	Check Date	Test File
■	U1	CU-9-1	9-11 ft	trm	2/26/21	mcm	3/16/21	312665-CU-9-1m.dat

			
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: HB-BE-242A	Sample Type: Intact	
	Description: Moist, gray clay		
	Remarks: System F		

One-Dimensional Consolidation by ASTM D2435 - Method B

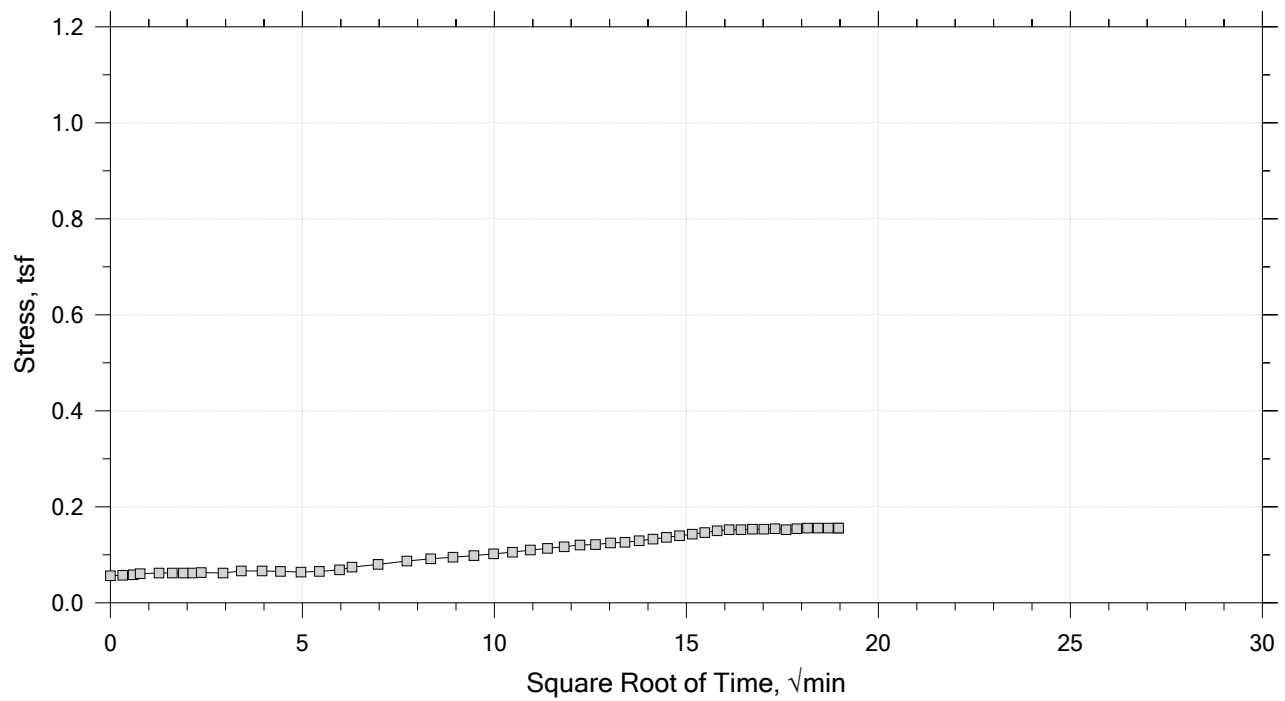
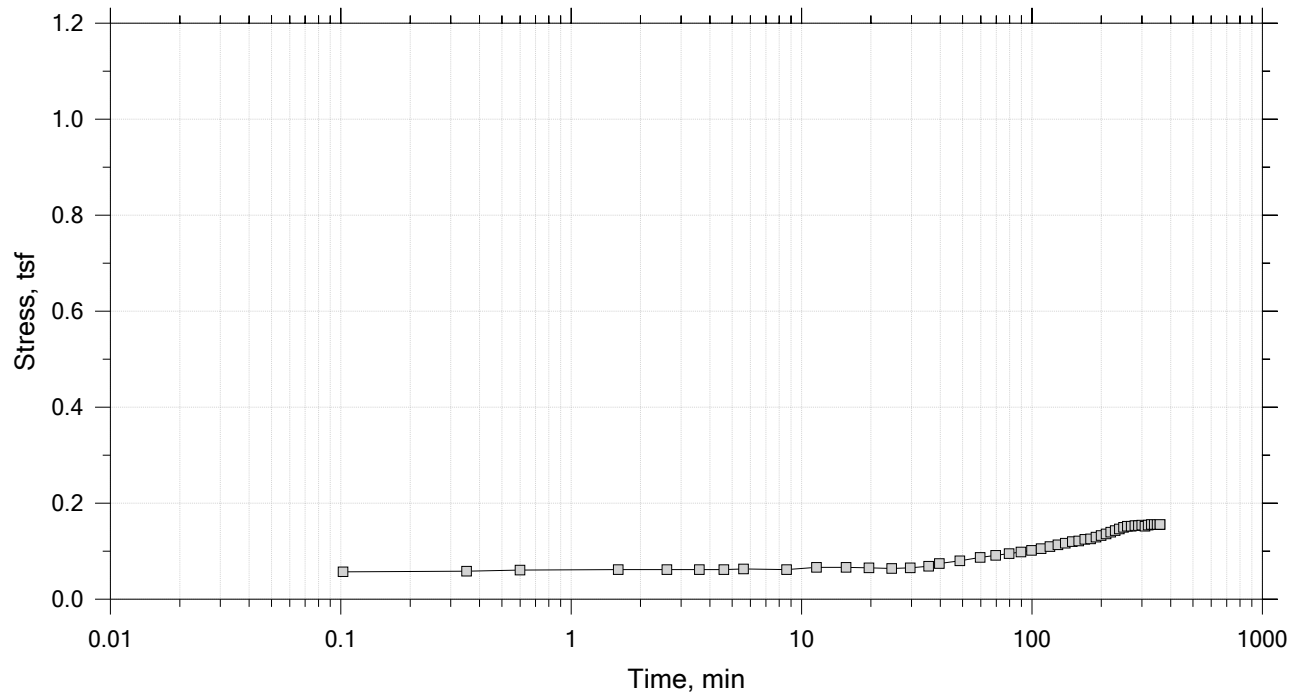
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 14
Constant Volume Step
Stress: 0.156 tsf



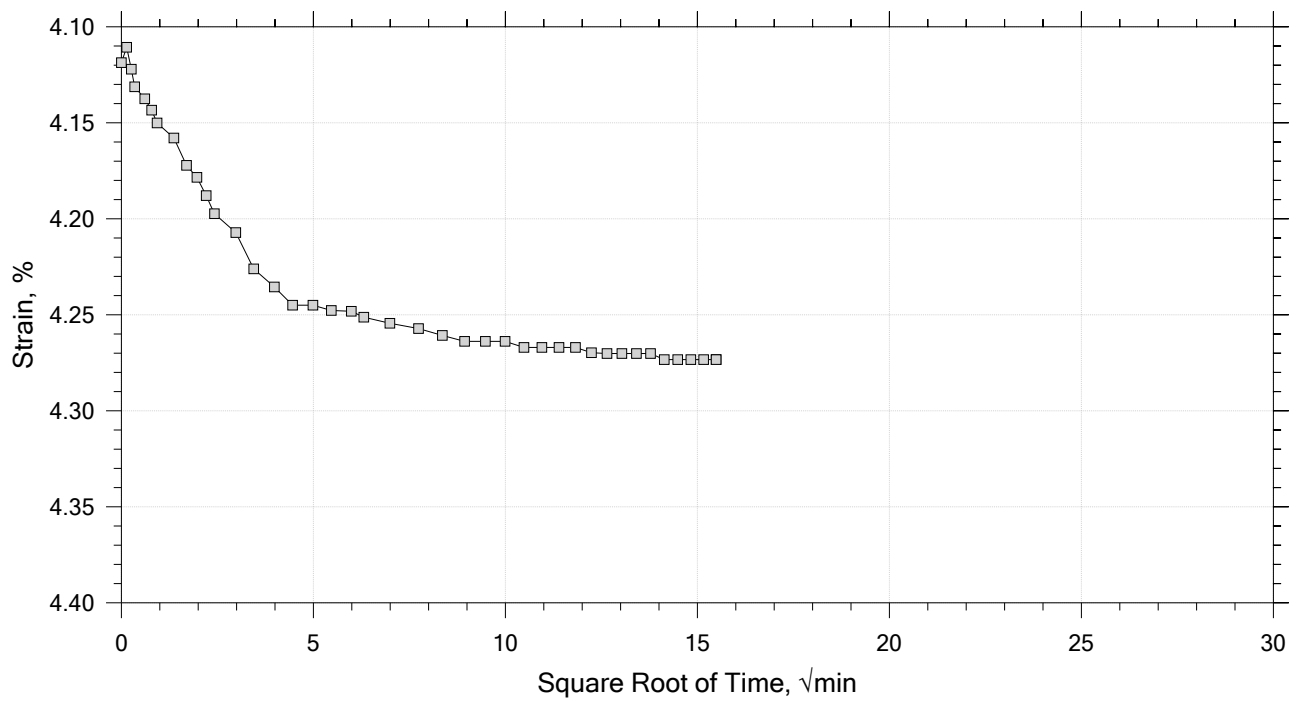
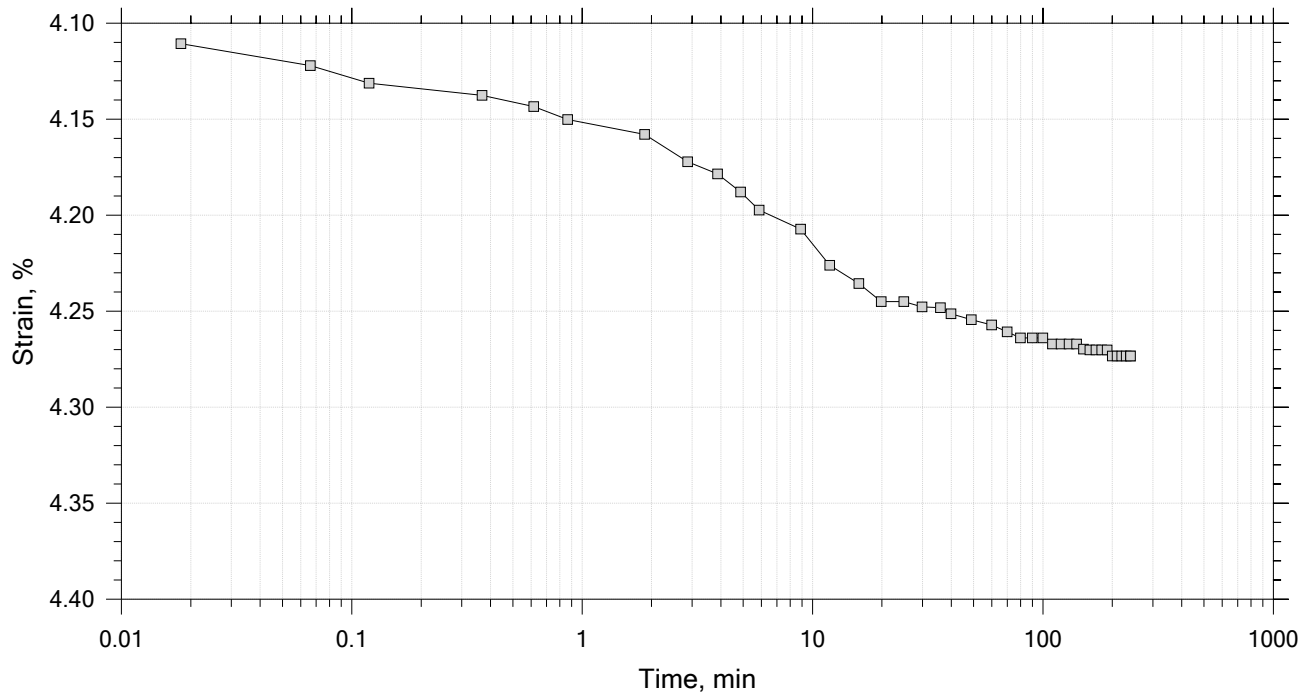
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 14

Constant Load Step

Stress: 0.25 tsf



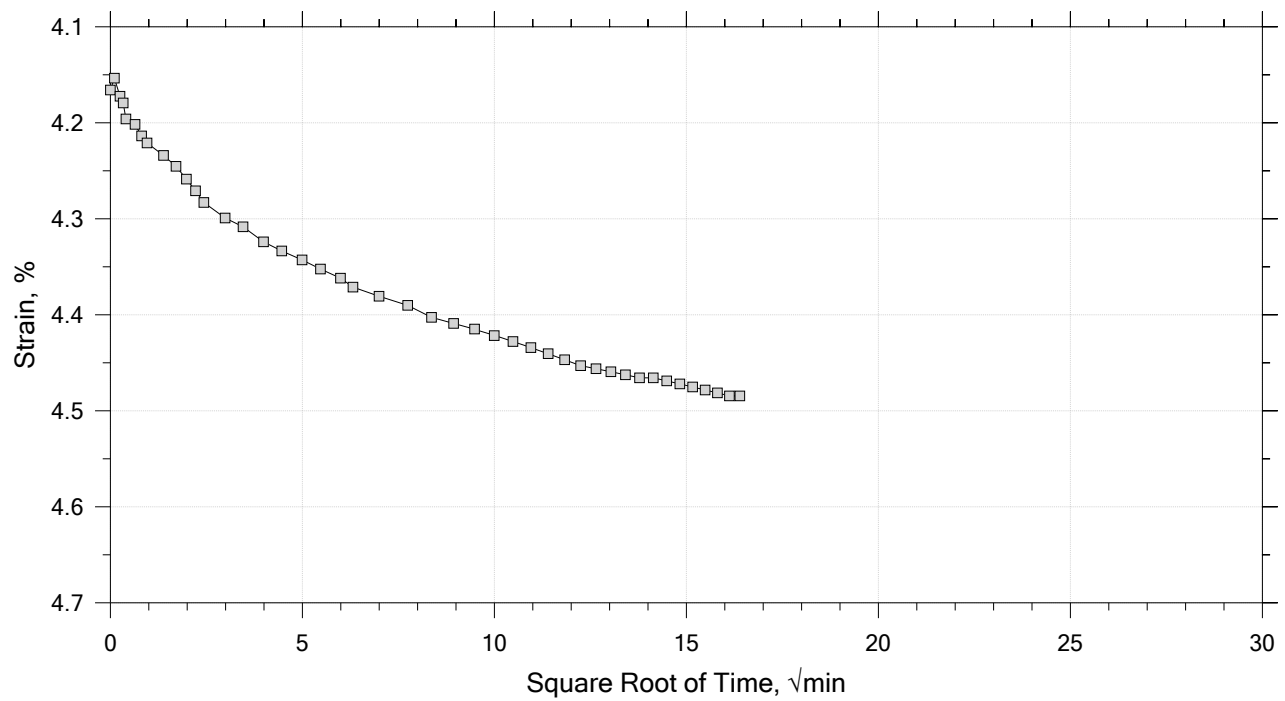
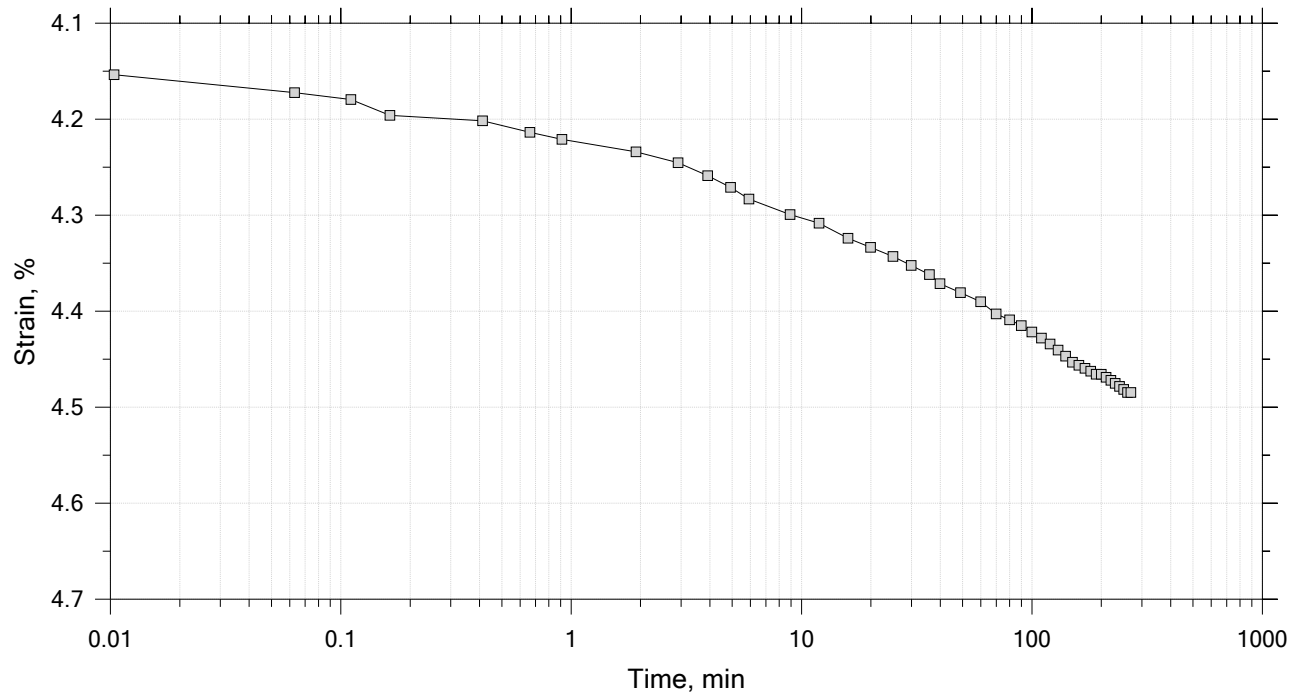
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 14

Constant Load Step

Stress: 0.5 tsf



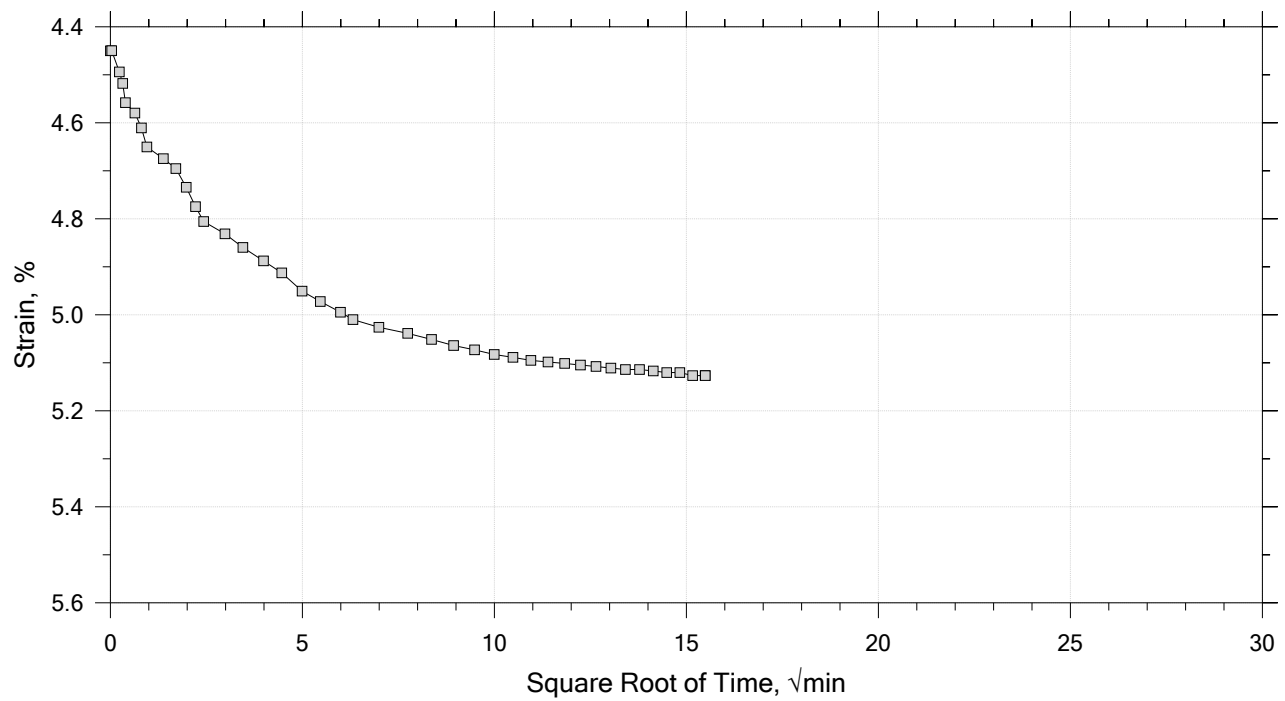
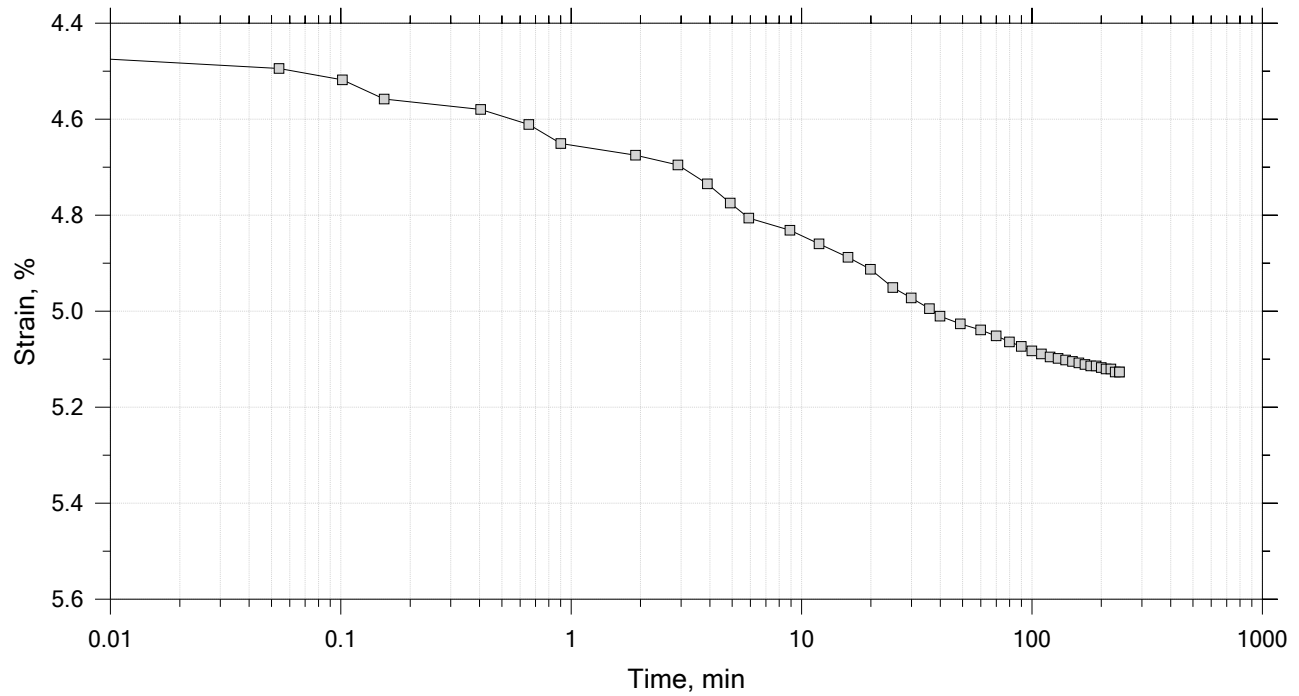
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 14

Constant Load Step

Stress: 1 tsf



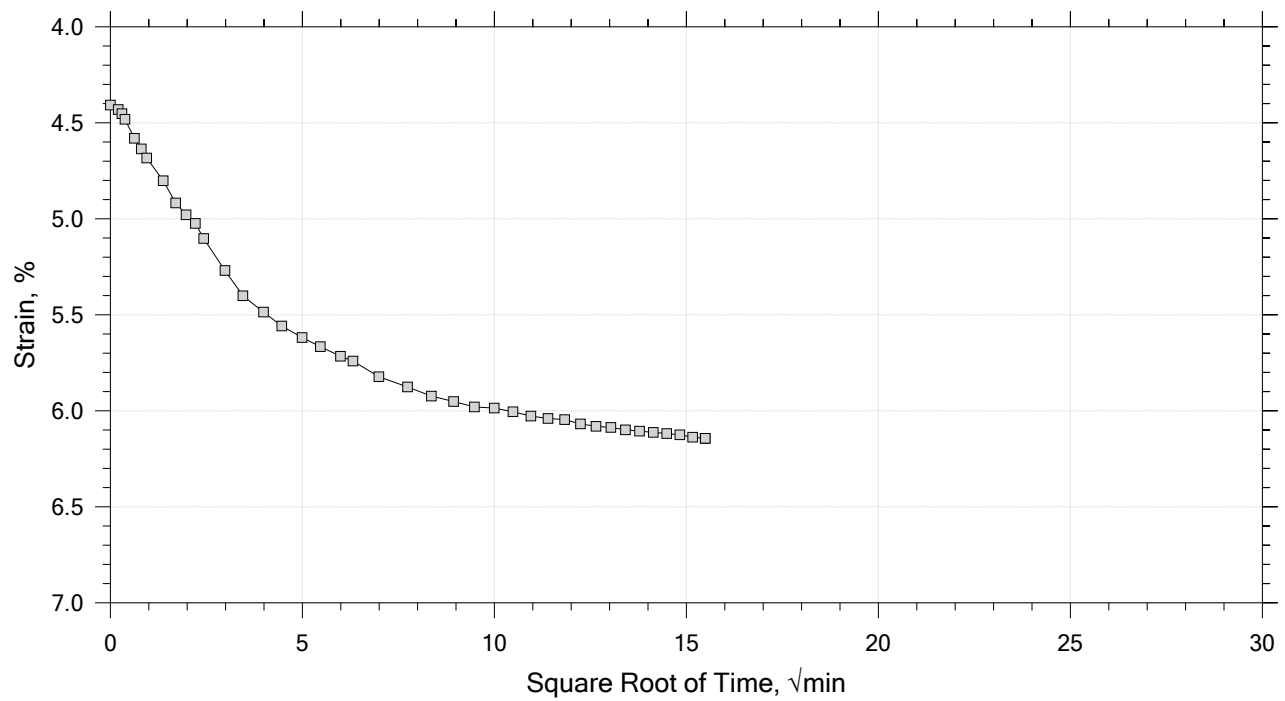
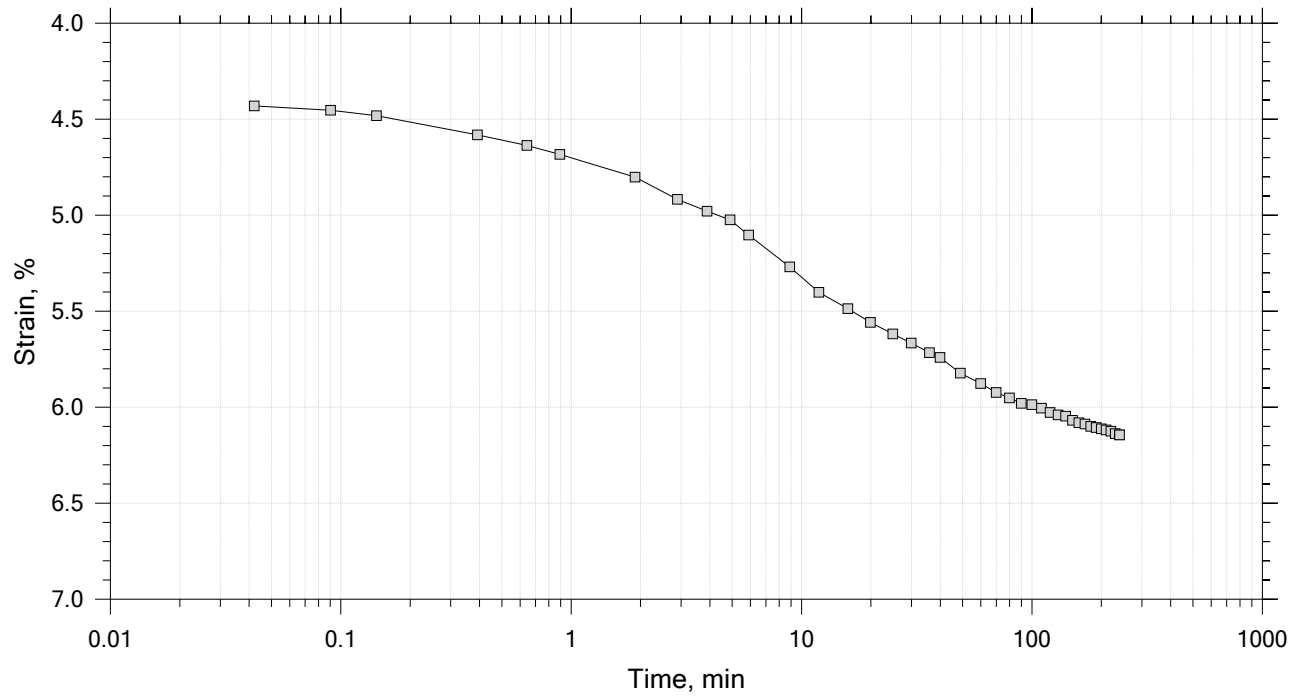
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 14

Constant Load Step

Stress: 2 tsf



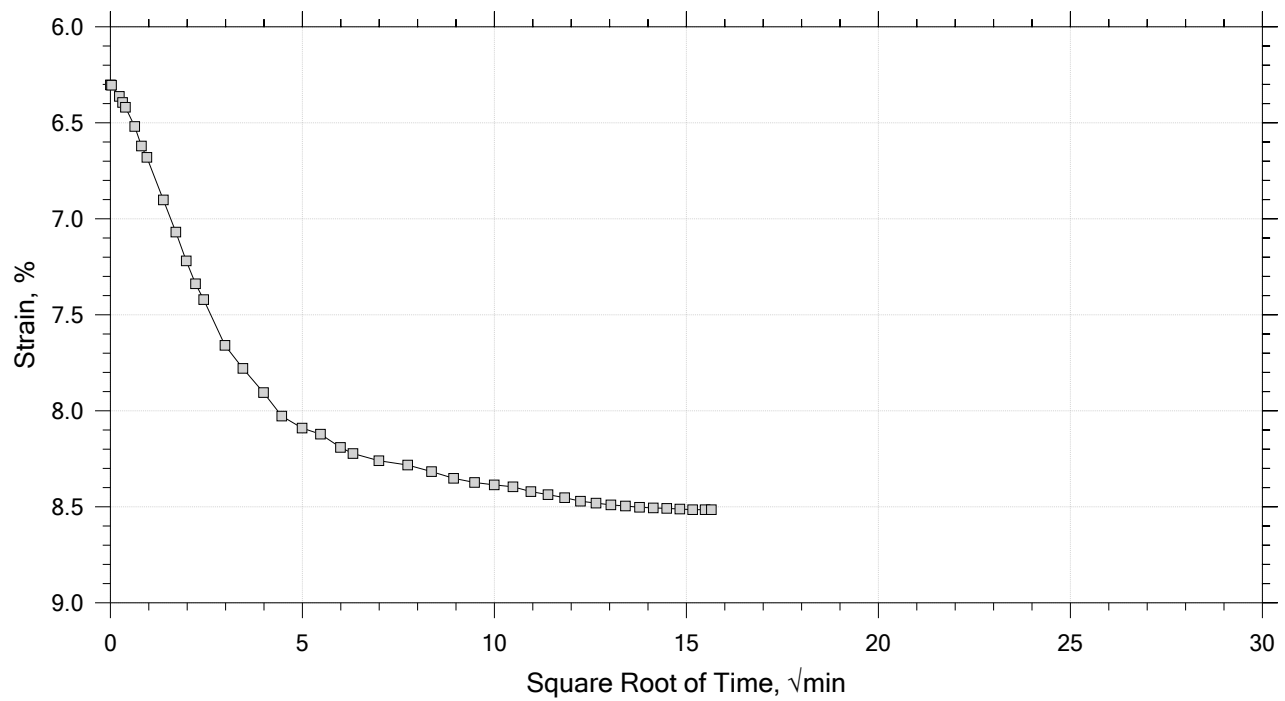
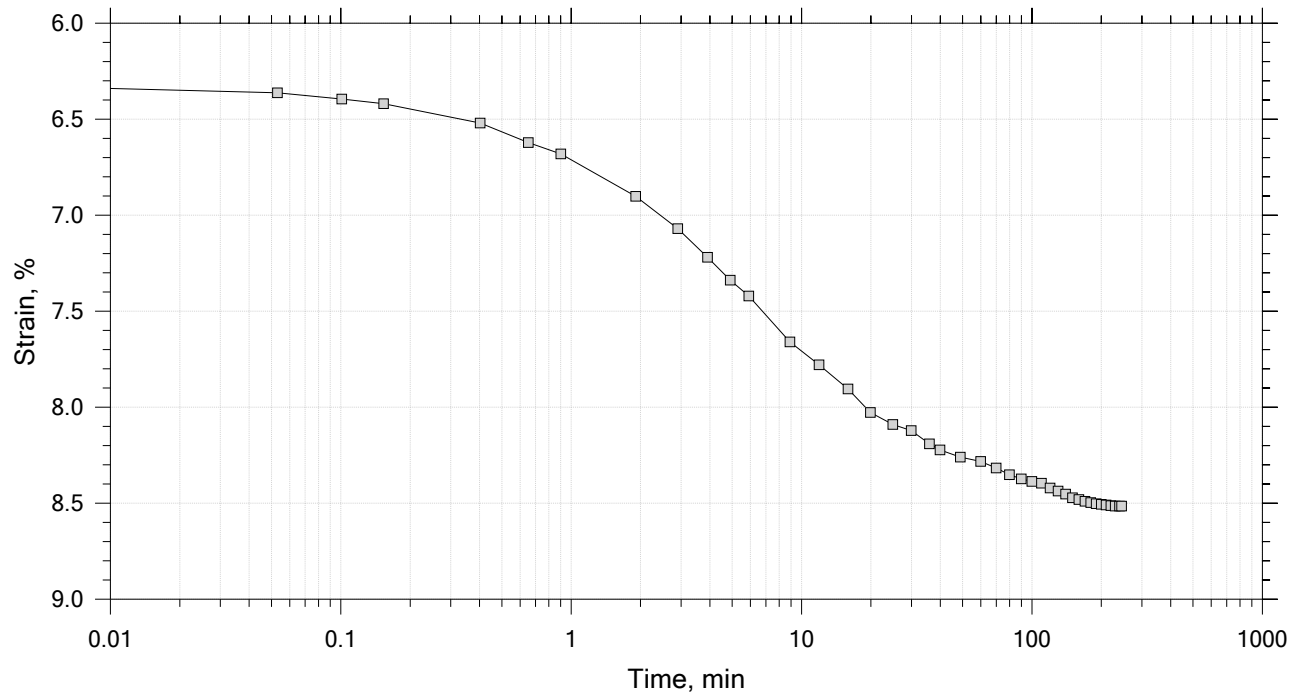
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 14

Constant Load Step

Stress: 4 tsf



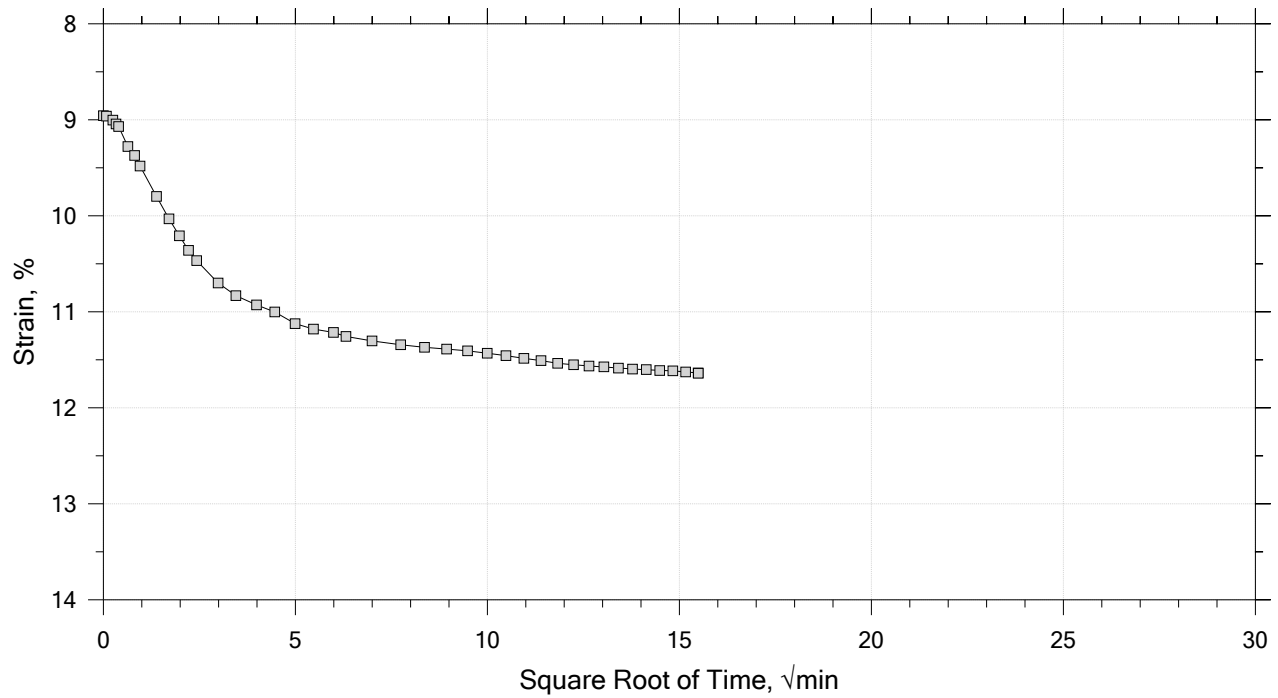
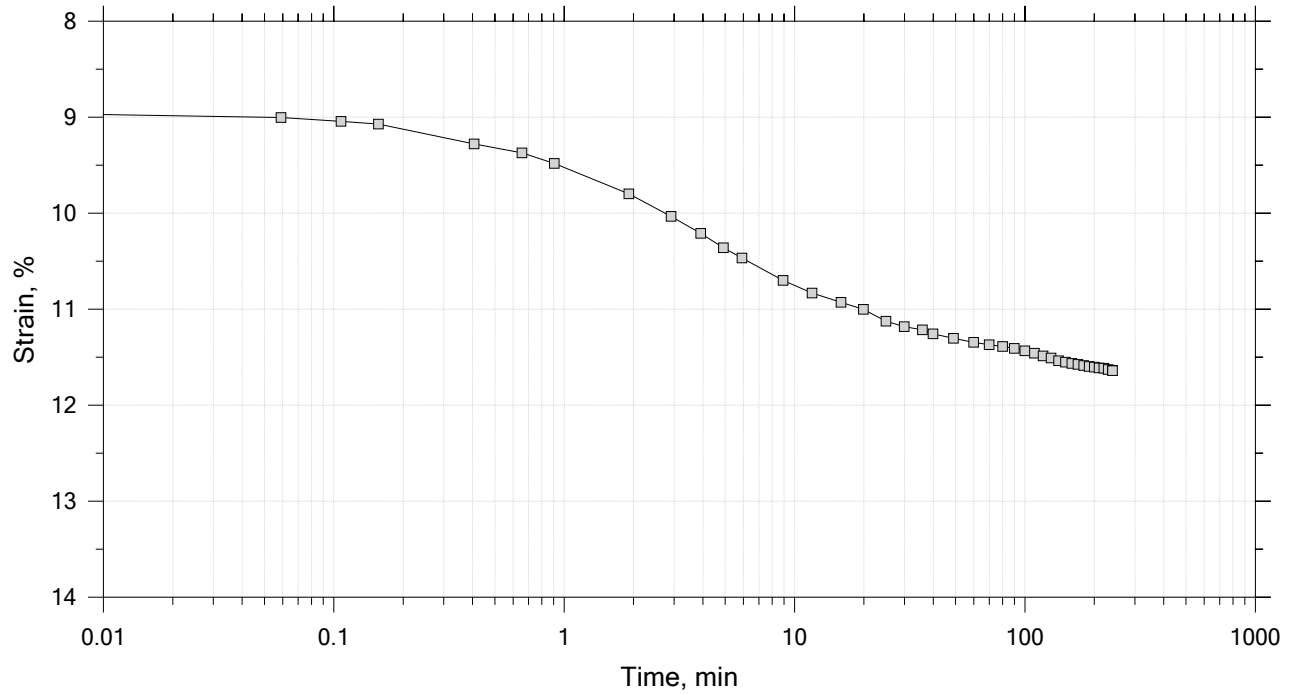
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 14

Constant Load Step

Stress: 8 tsf



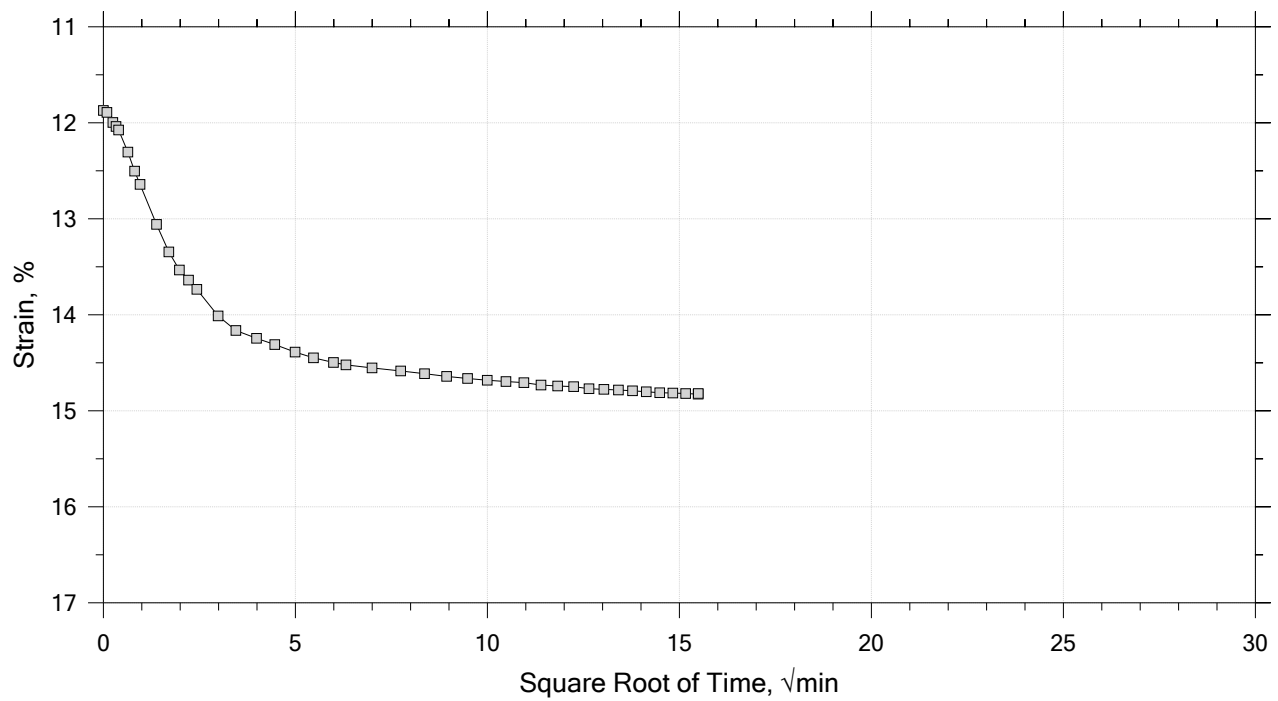
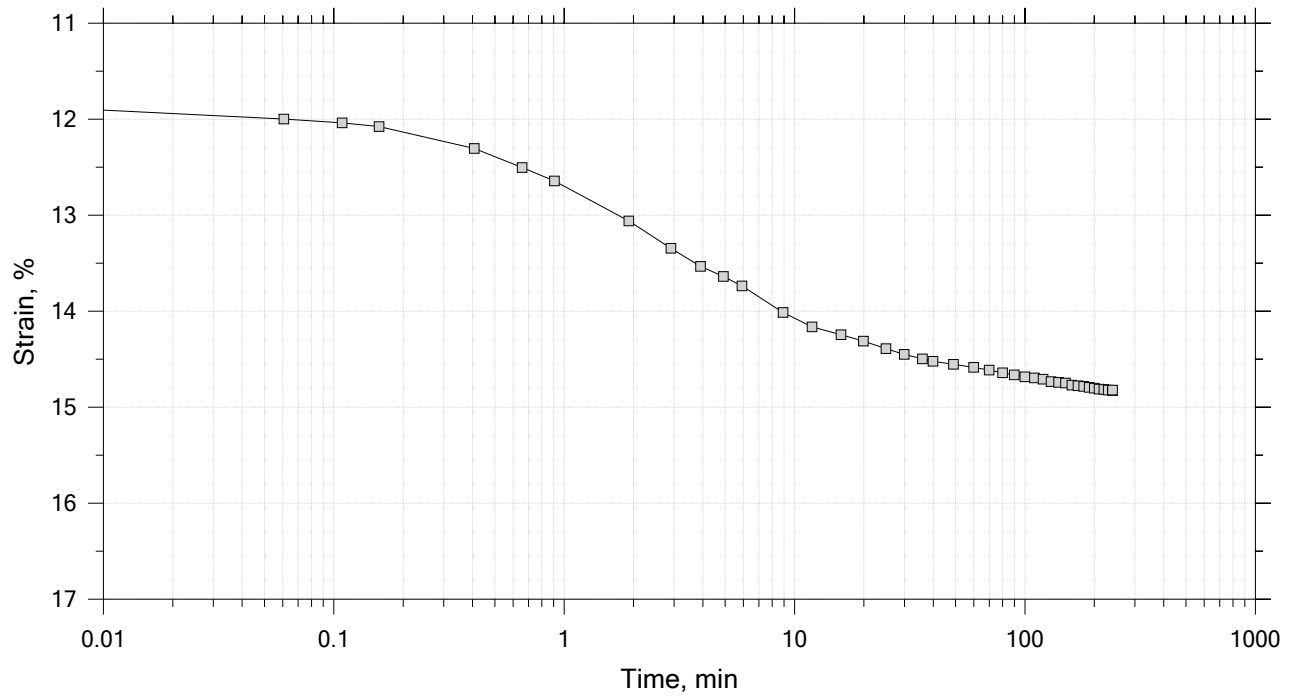
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 14

Constant Load Step

Stress: 16 tsf



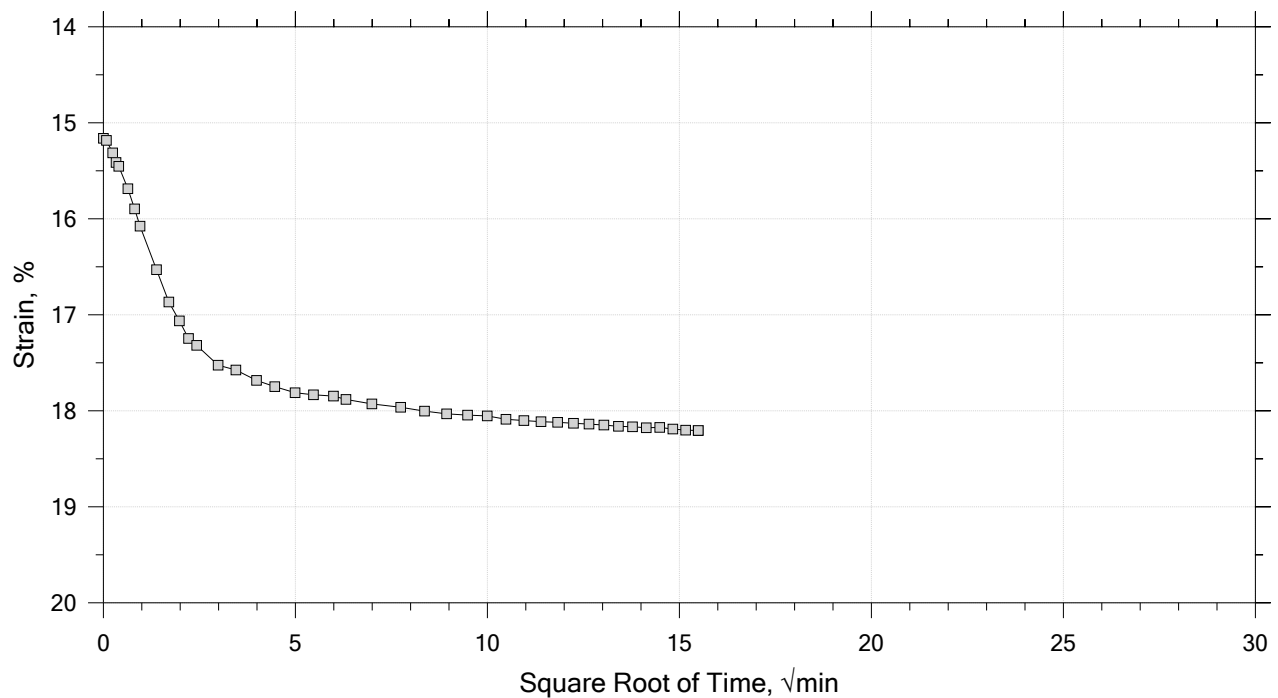
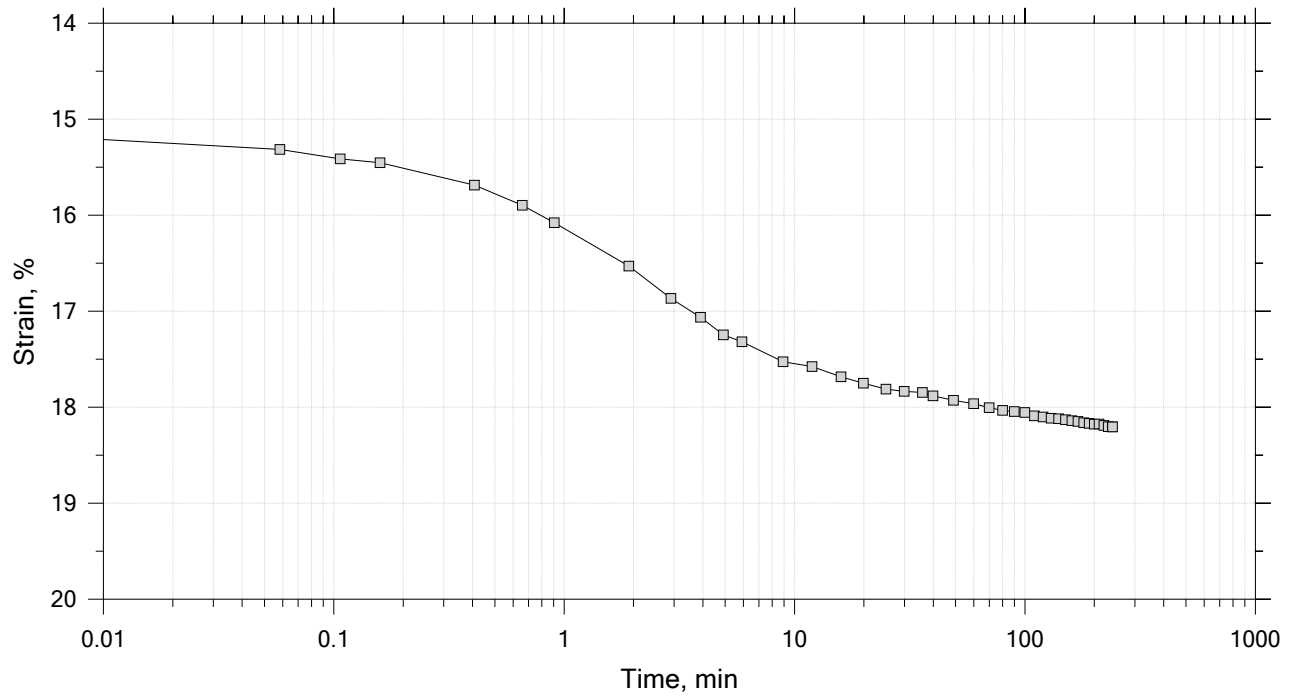
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 14

Constant Load Step

Stress: 32 tsf



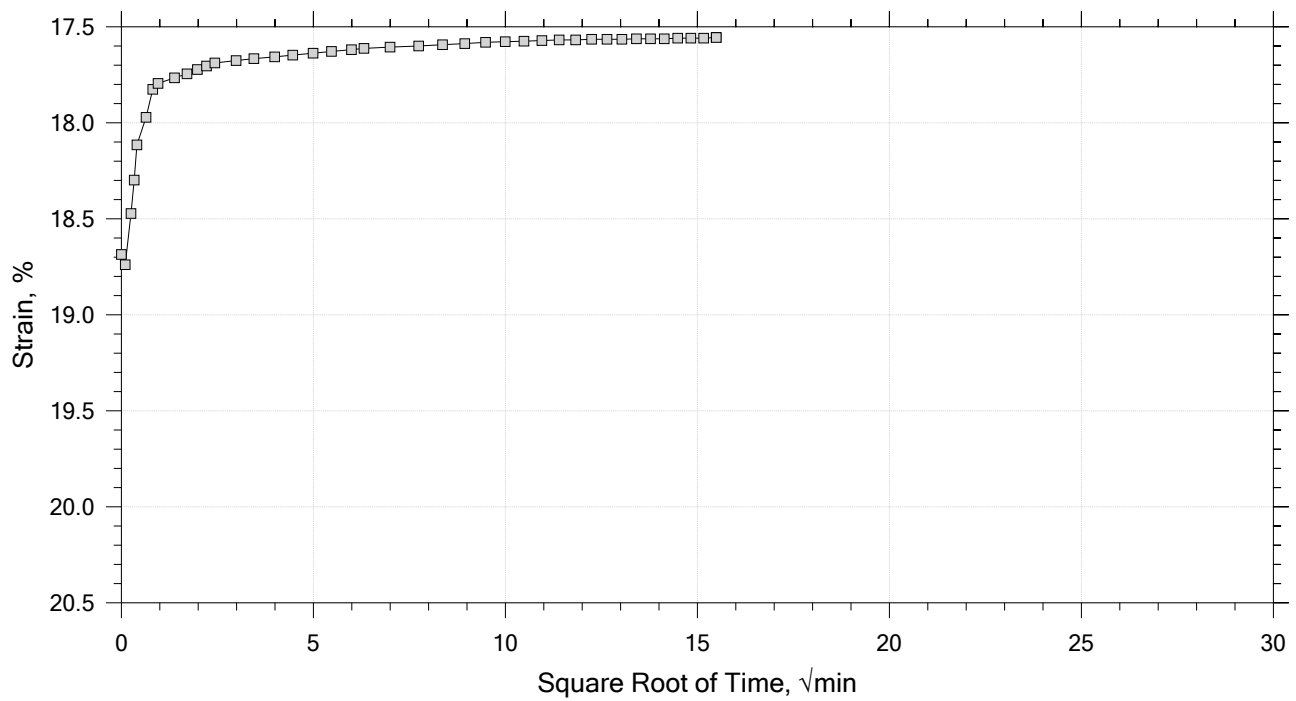
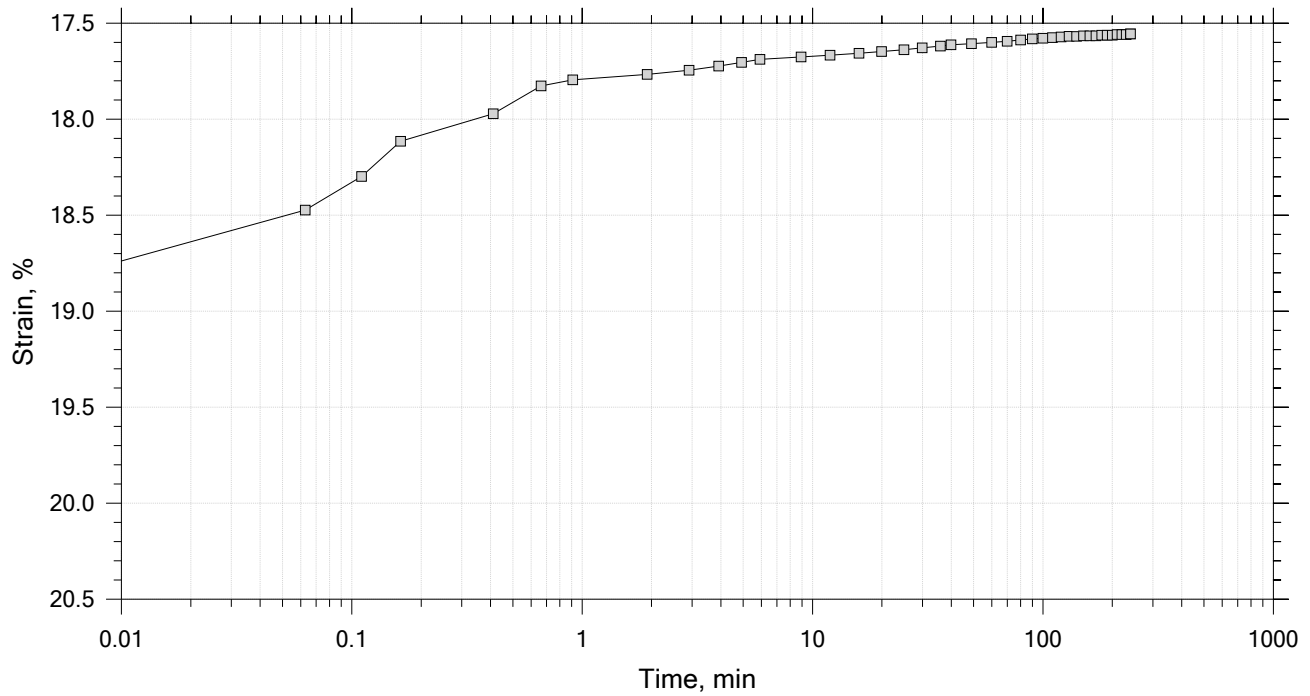
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 14

Constant Load Step

Stress: 8 tsf



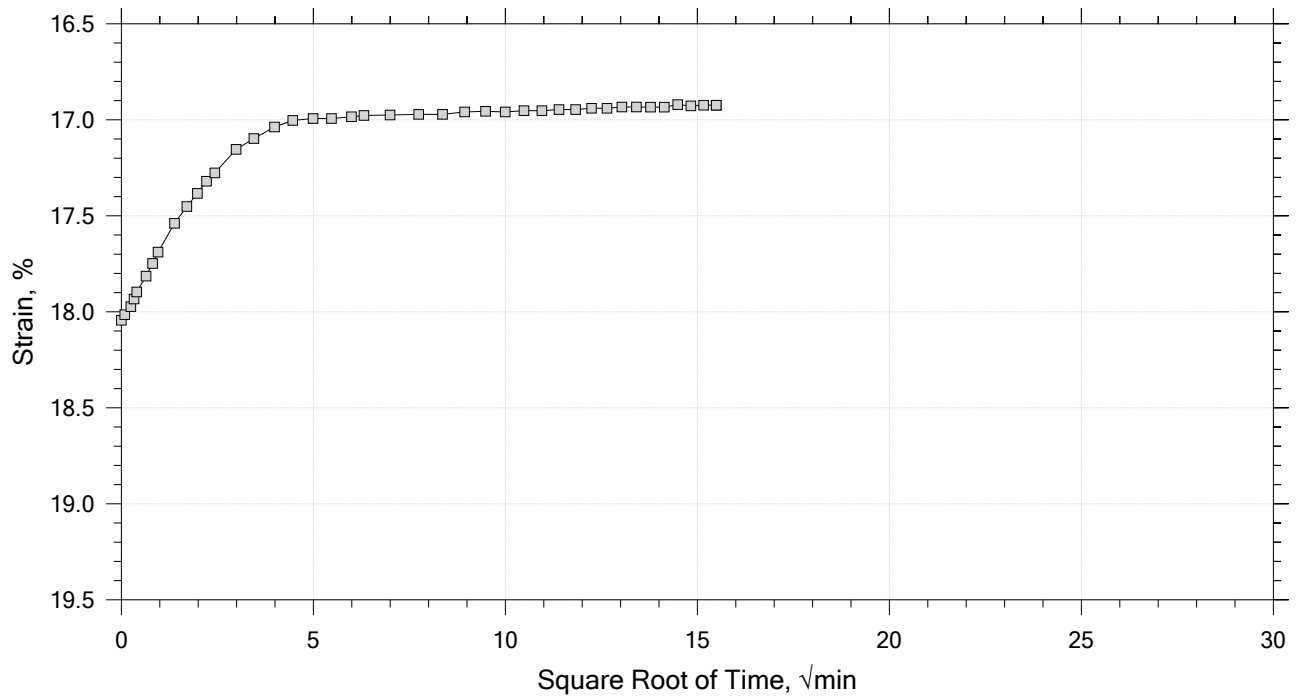
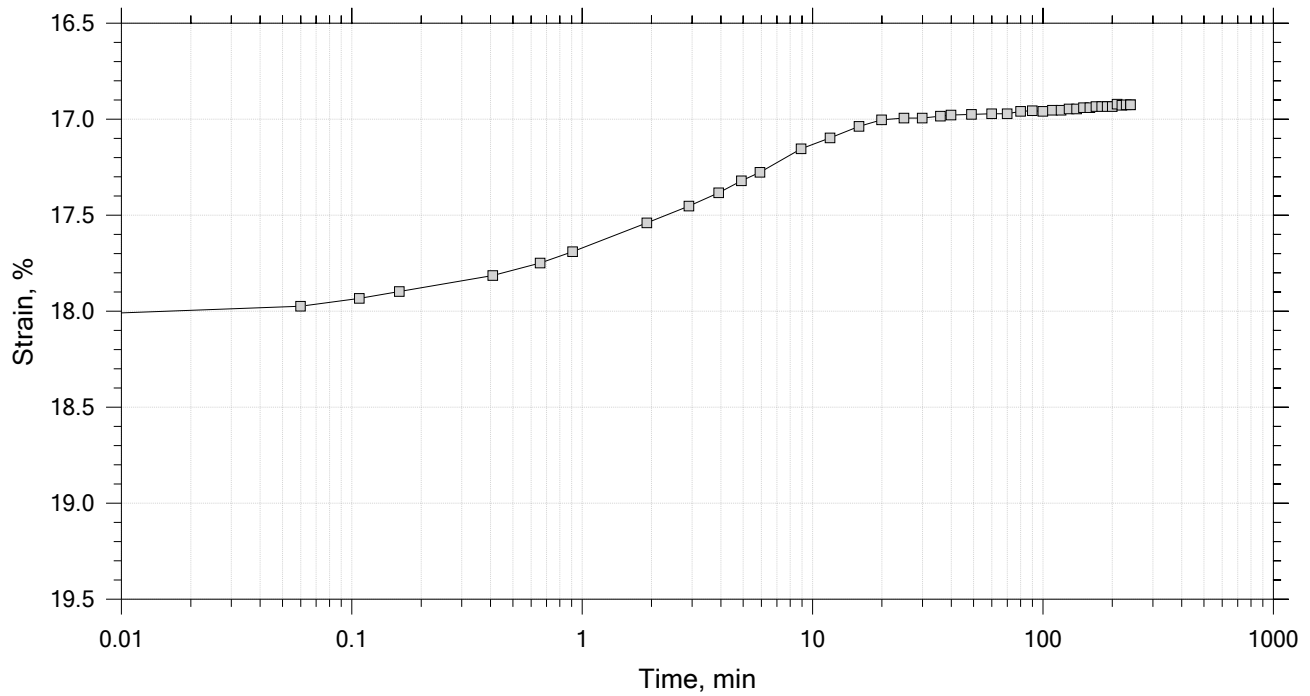
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 14

Constant Load Step

Stress: 2 tsf



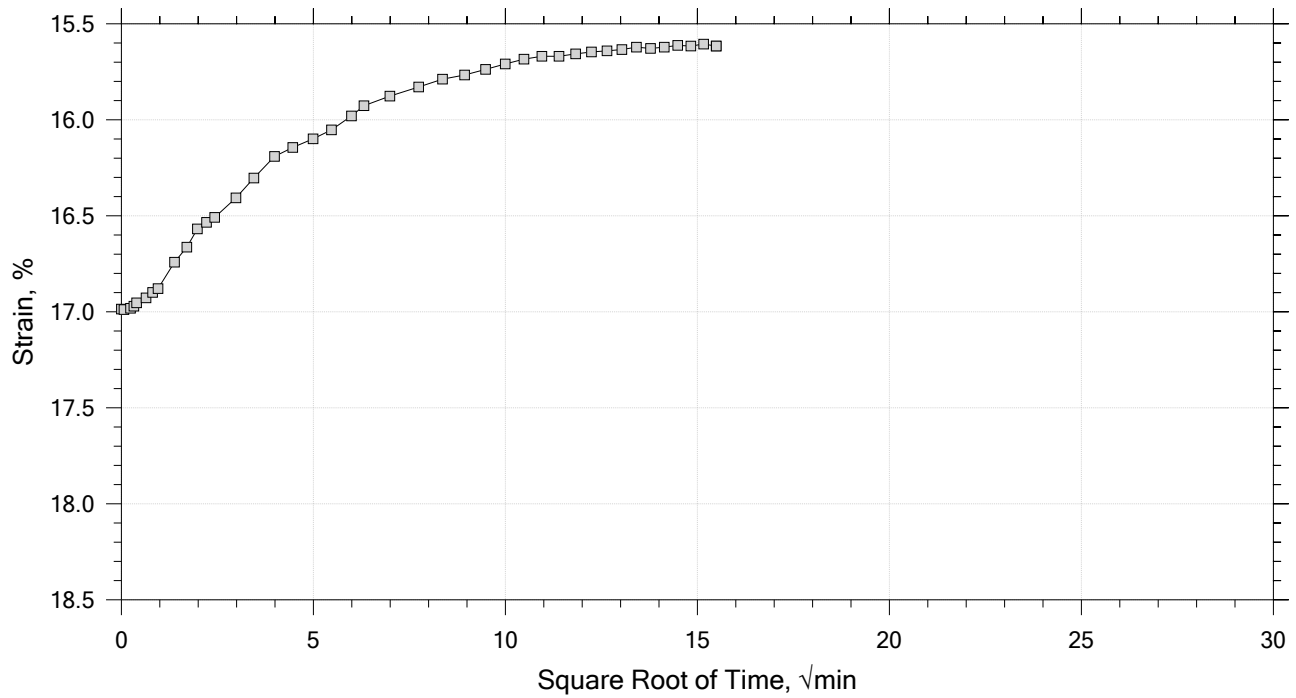
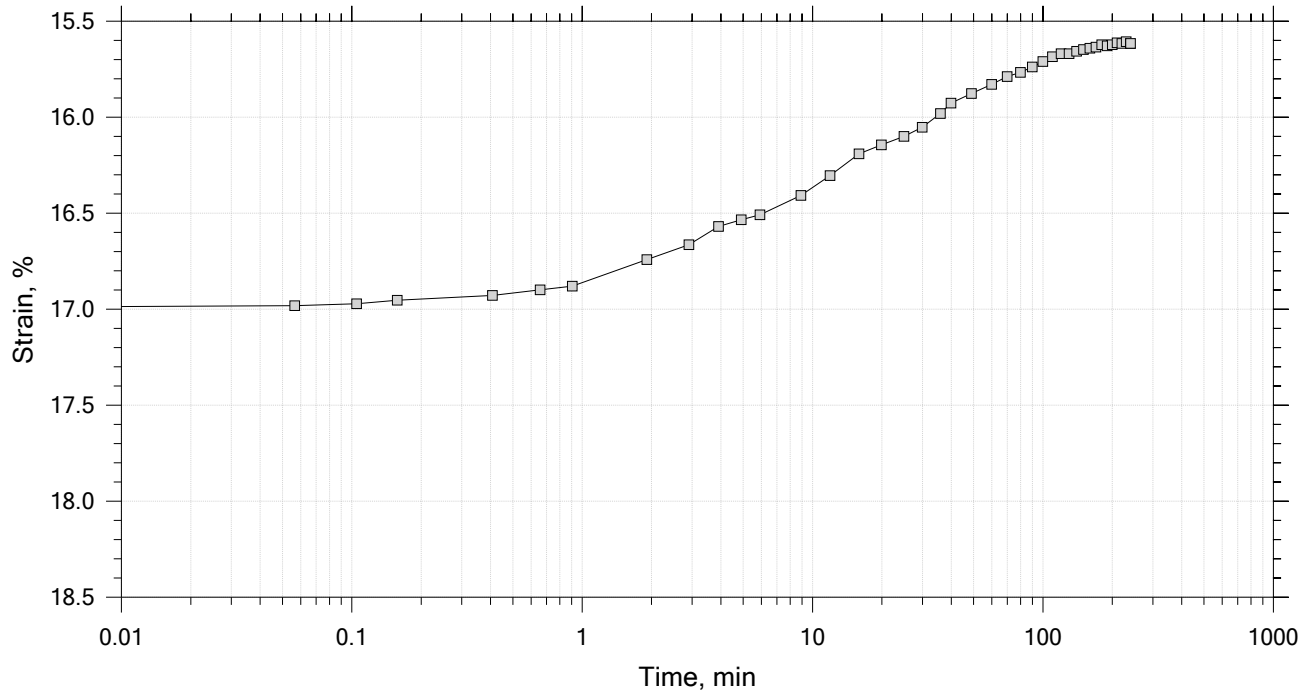
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	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 14

Constant Load Step

Stress: 0.5 tsf



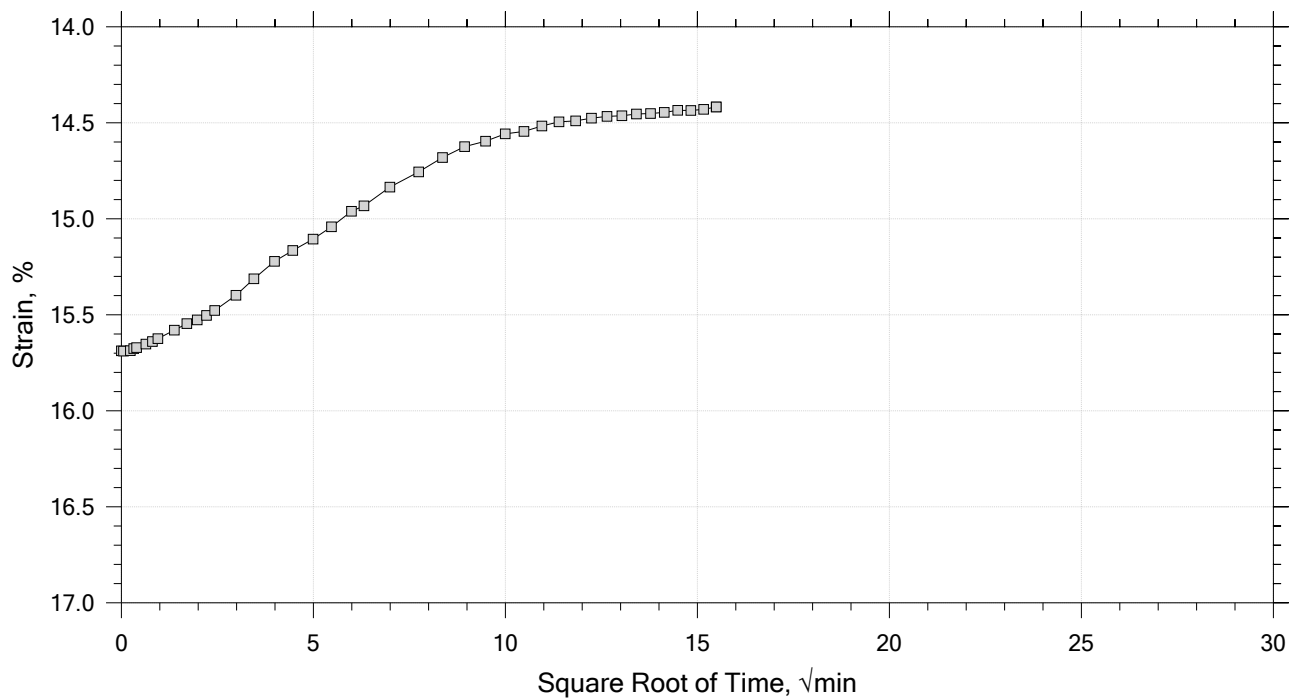
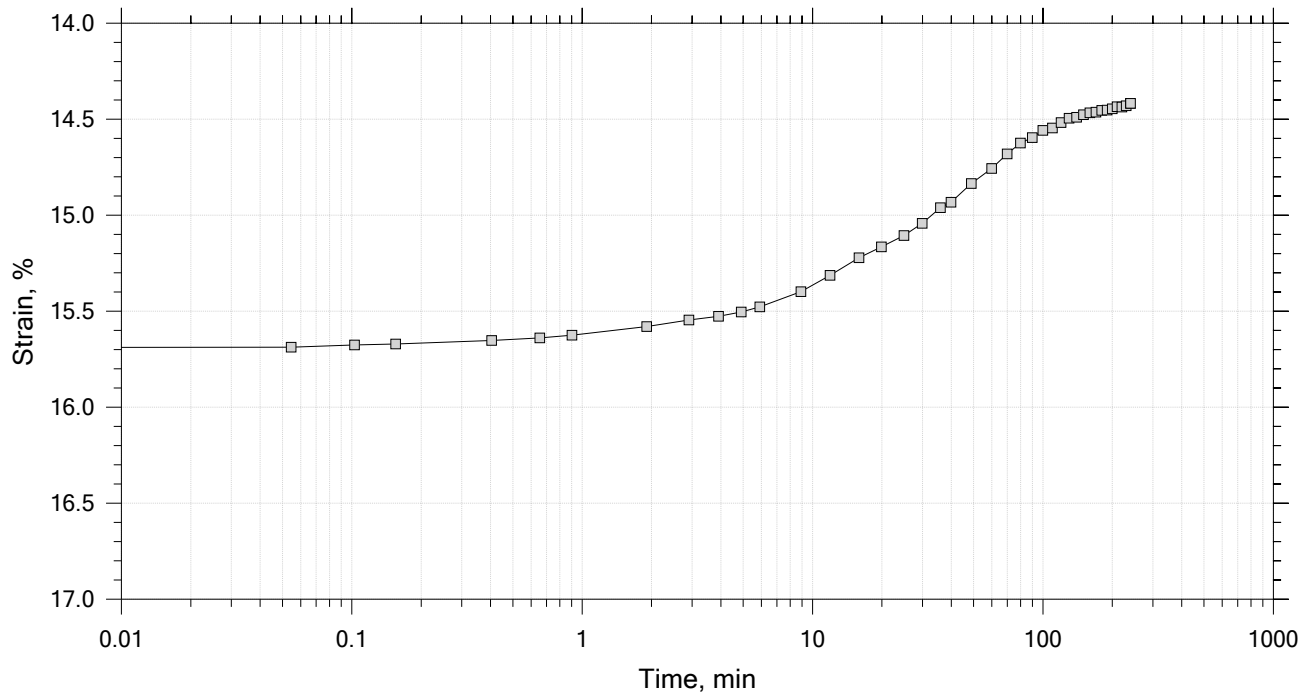
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 14

Constant Load Step

Stress: 0.125 tsf



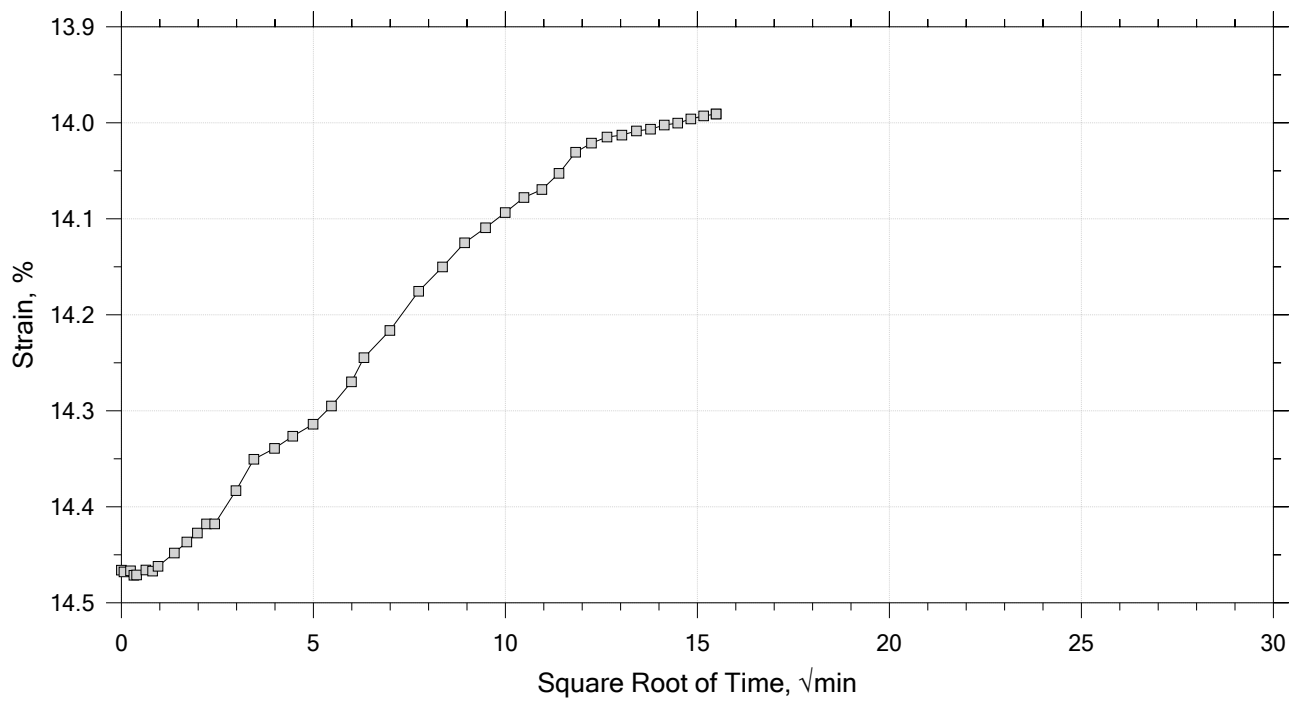
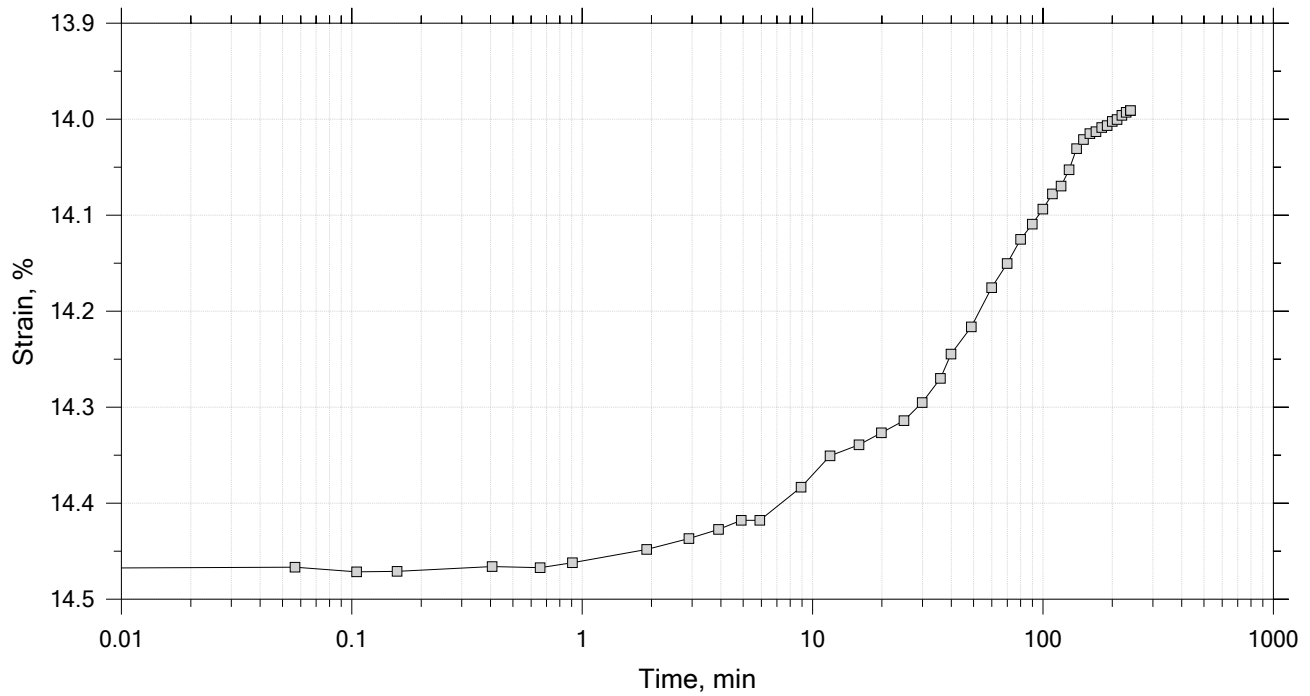
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 14

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 0.836	Plastic Limit: 19
Final Height: 0.86 in	Final Void Ratio: 0.579	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-972	RING		D-2296
Mass Container, gm	8.26	107.93	107.93	8.4
Mass Container + Wet Soil, gm	161.89	264.41	255.27	155.08
Mass Container + Dry Soil, gm	125.33	229.9	229.9	129.82
Mass Dry Soil, gm	117.07	121.97	121.97	121.42
Water Content, %	31.23	28.30	20.80	20.80
Void Ratio	---	0.84	0.58	---
Degree of Saturation, %	---	94.23	100.00	---
Dry Unit Weight, pcf	---	94.656	110.05	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIll-A, Swell Pressure = 0.156 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

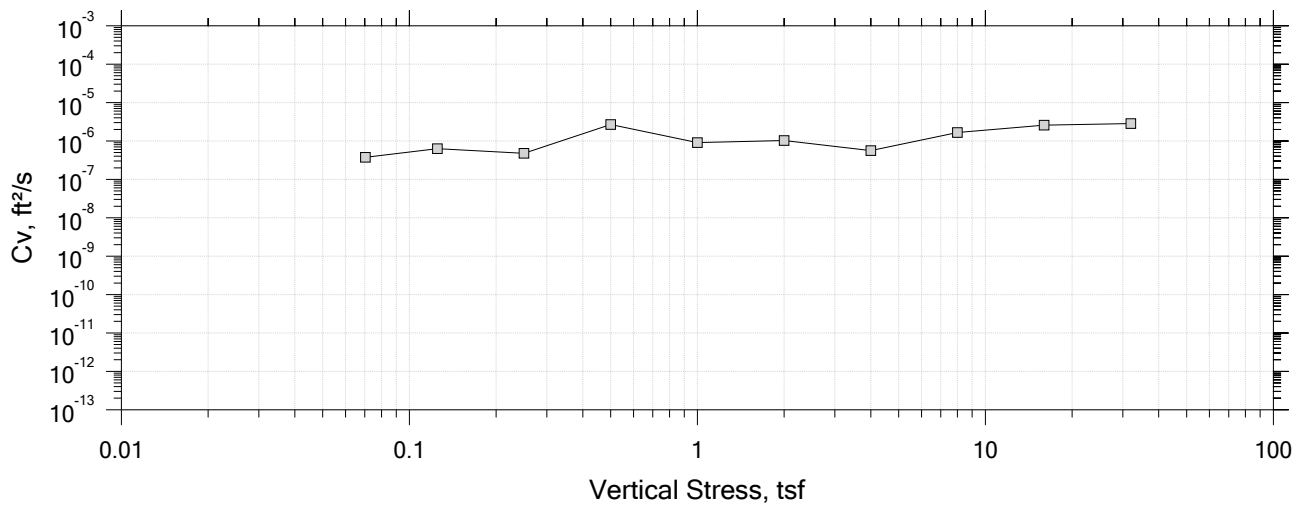
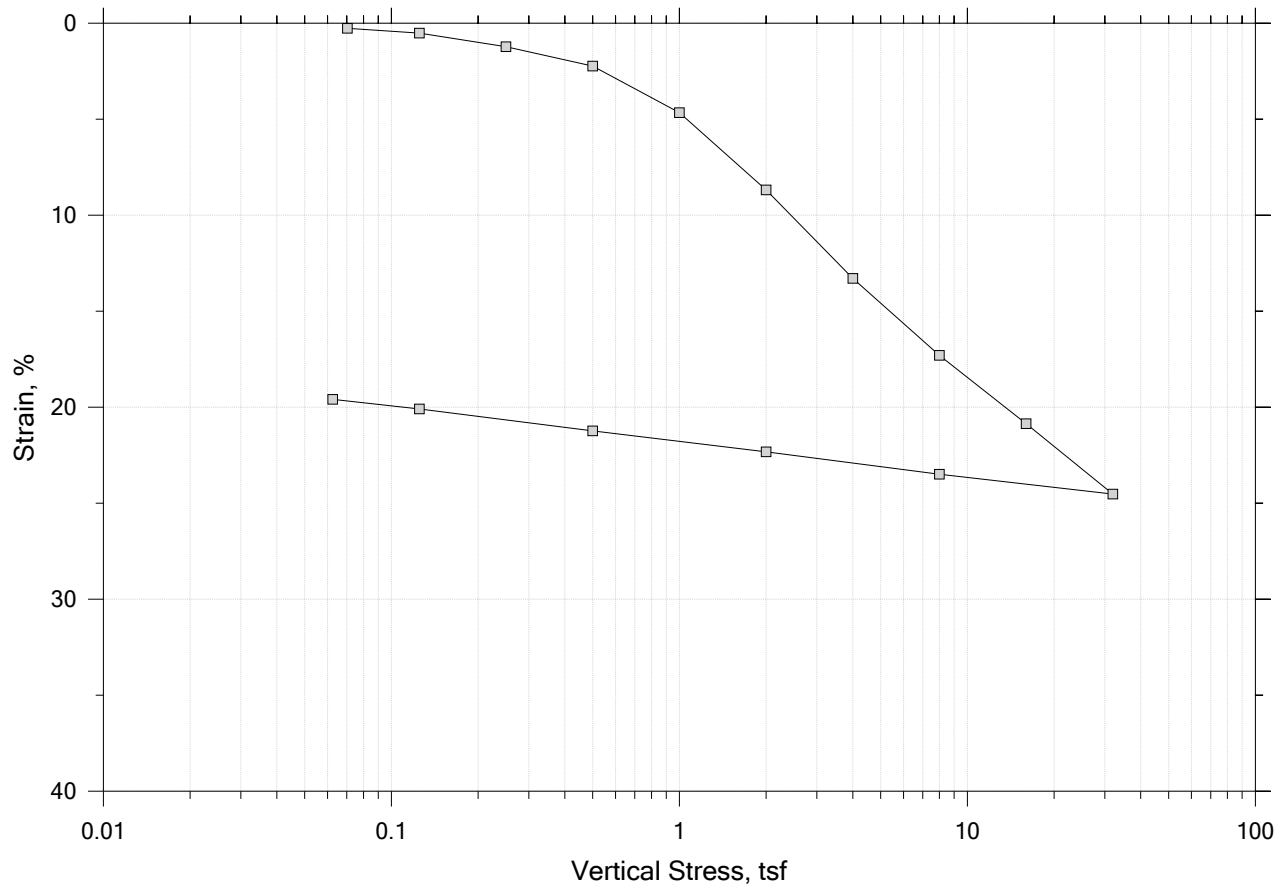
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 5-7 ft
	Test No.: IP-17	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.156 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

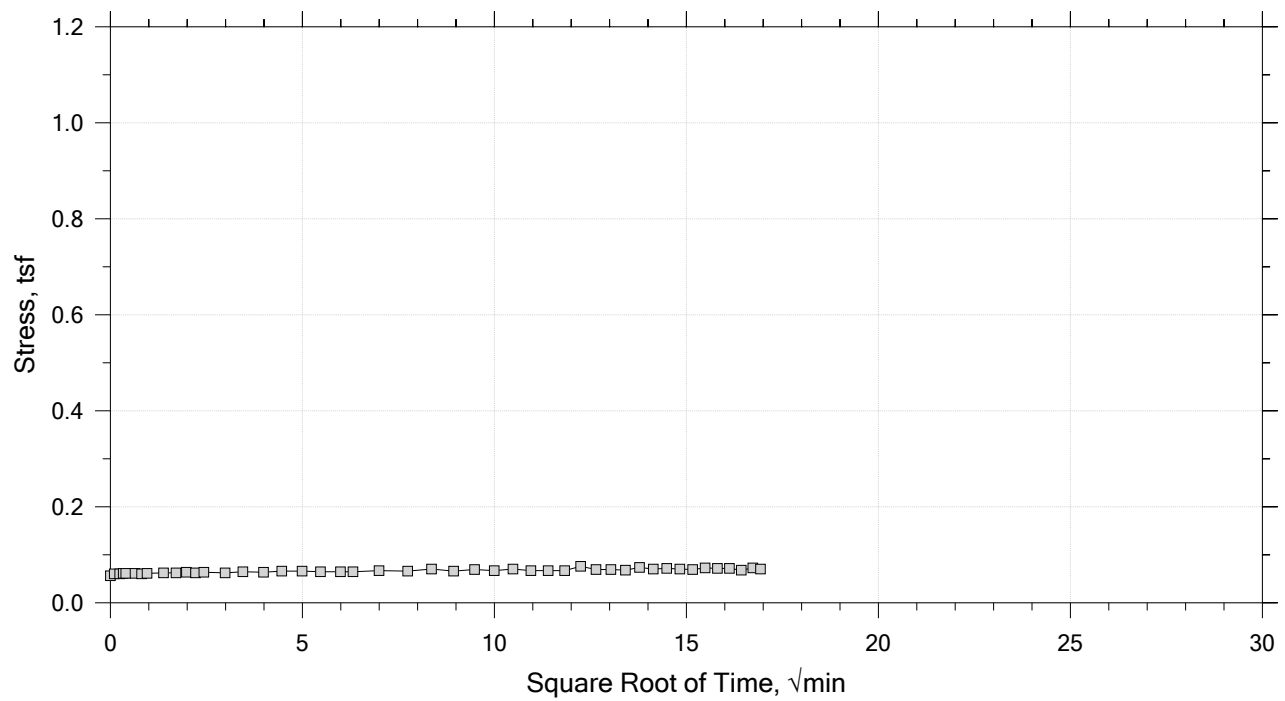
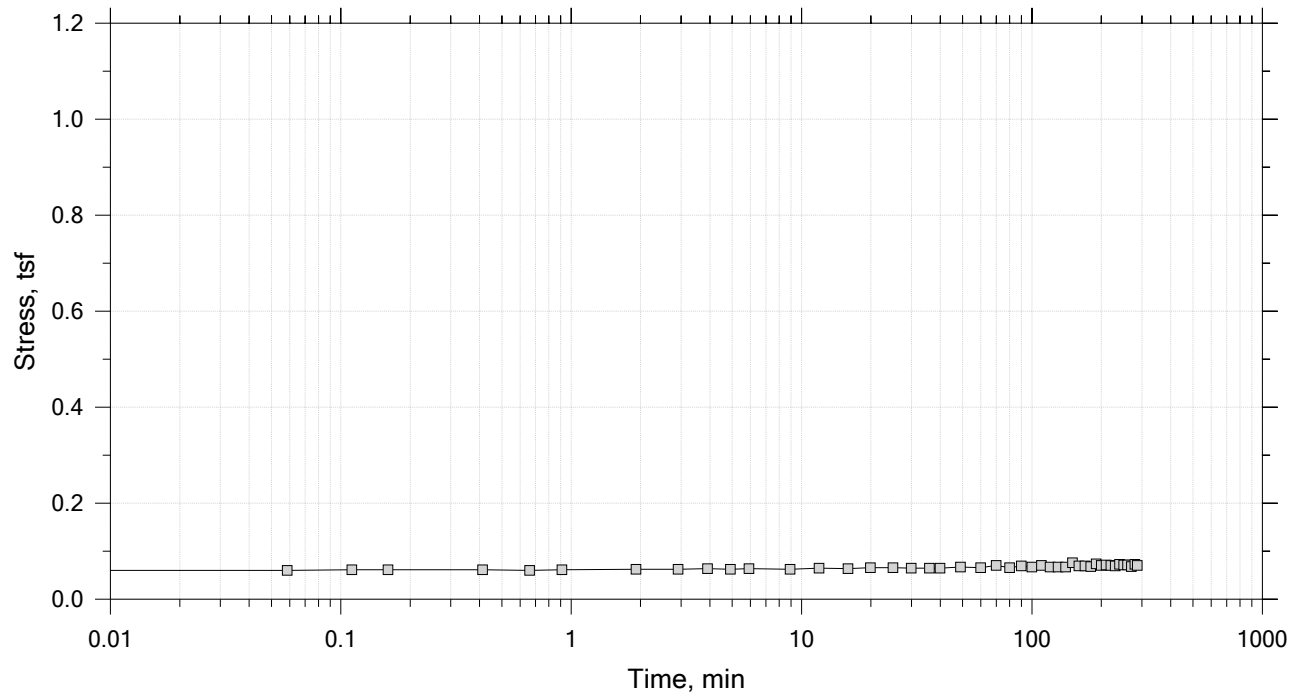
Summary Report




	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0703 tsf



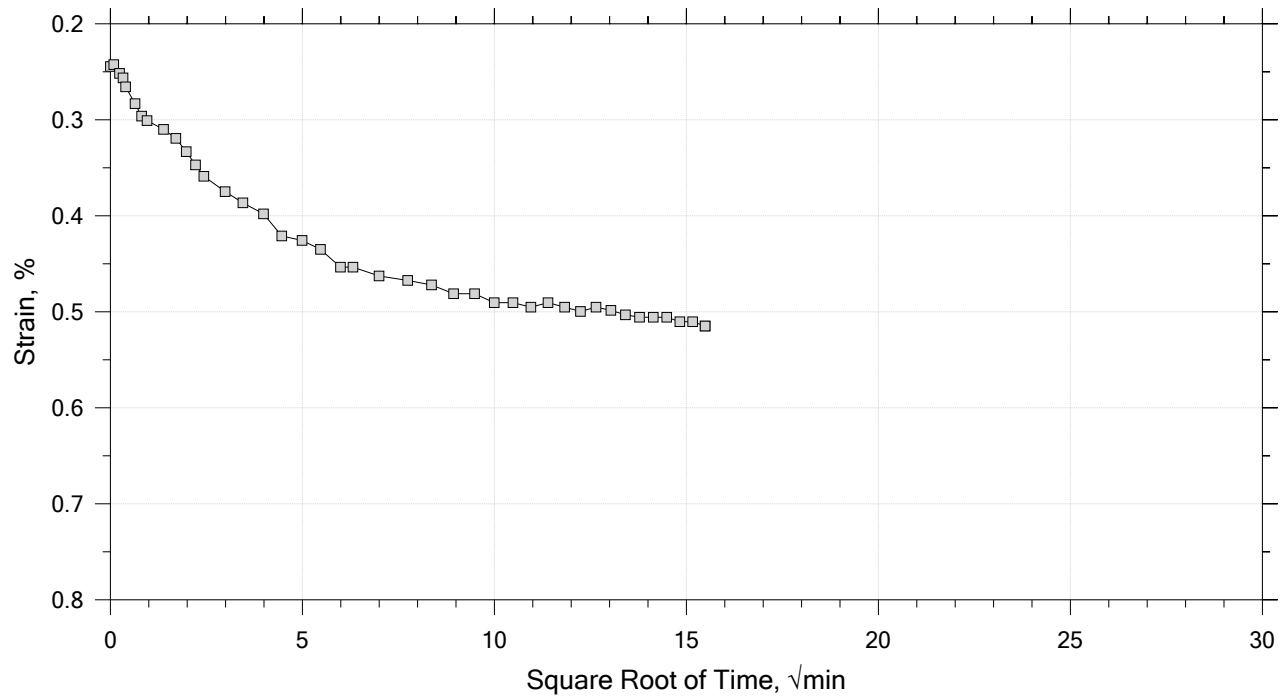
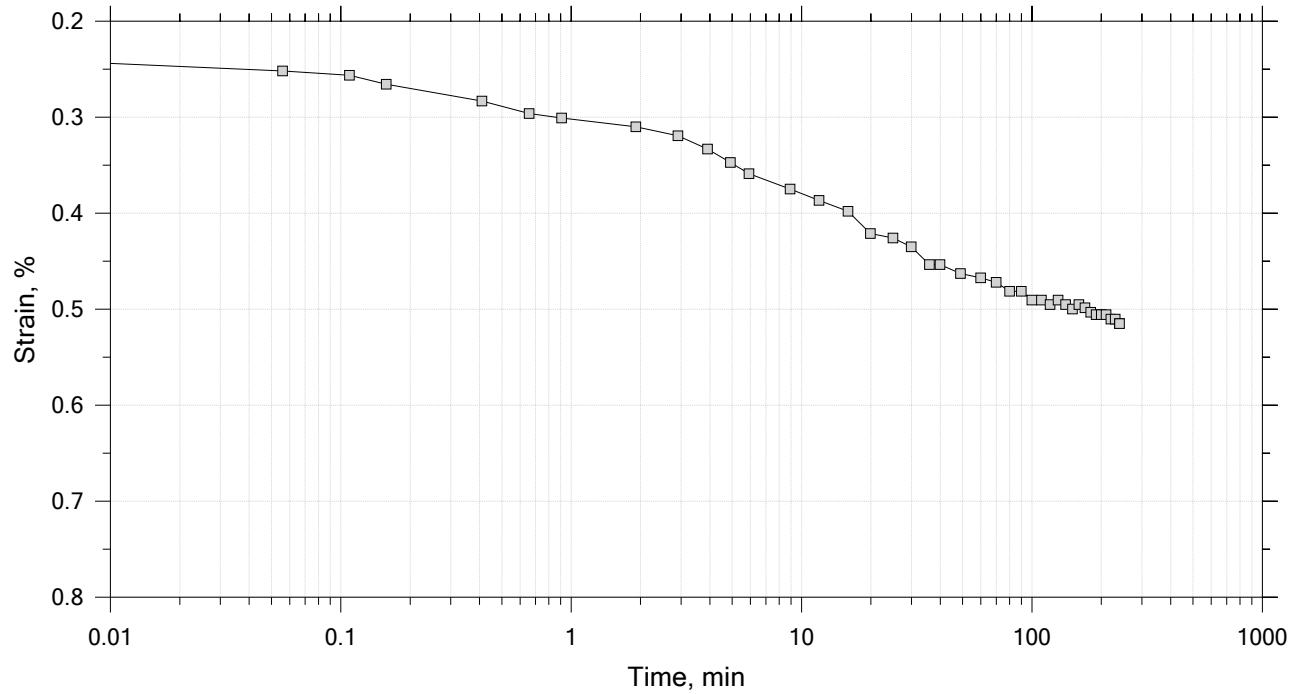
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



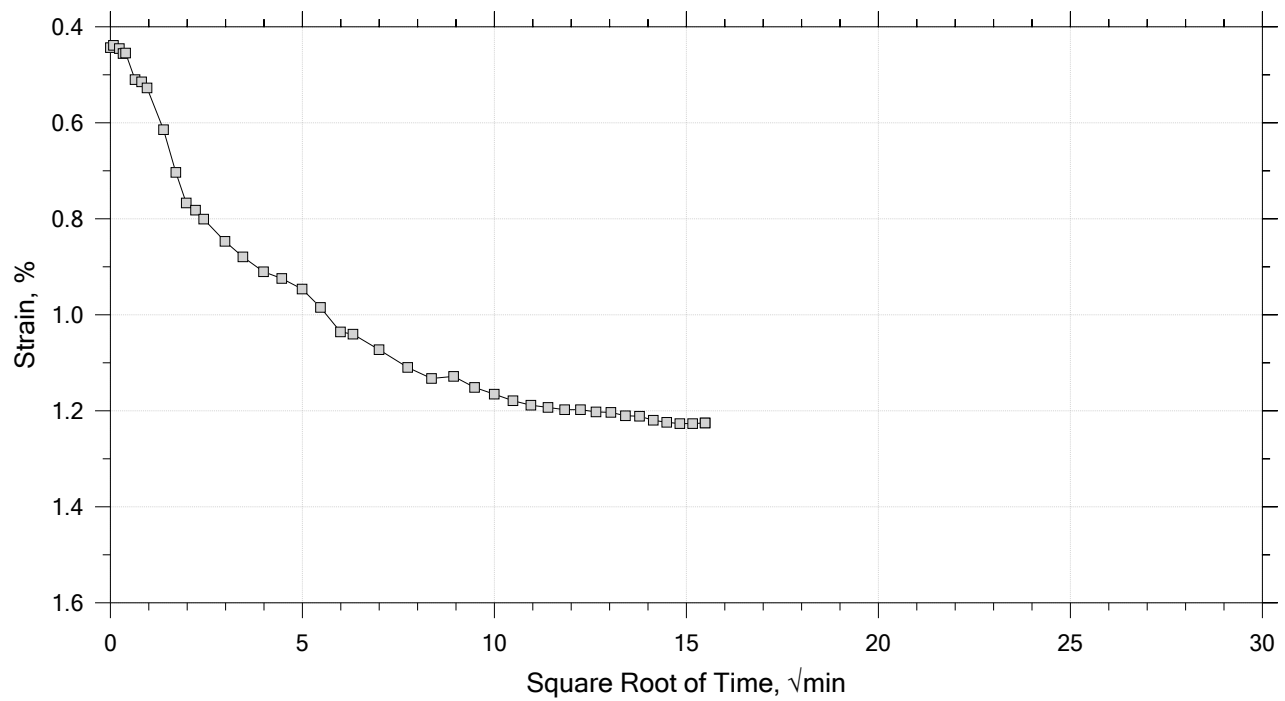
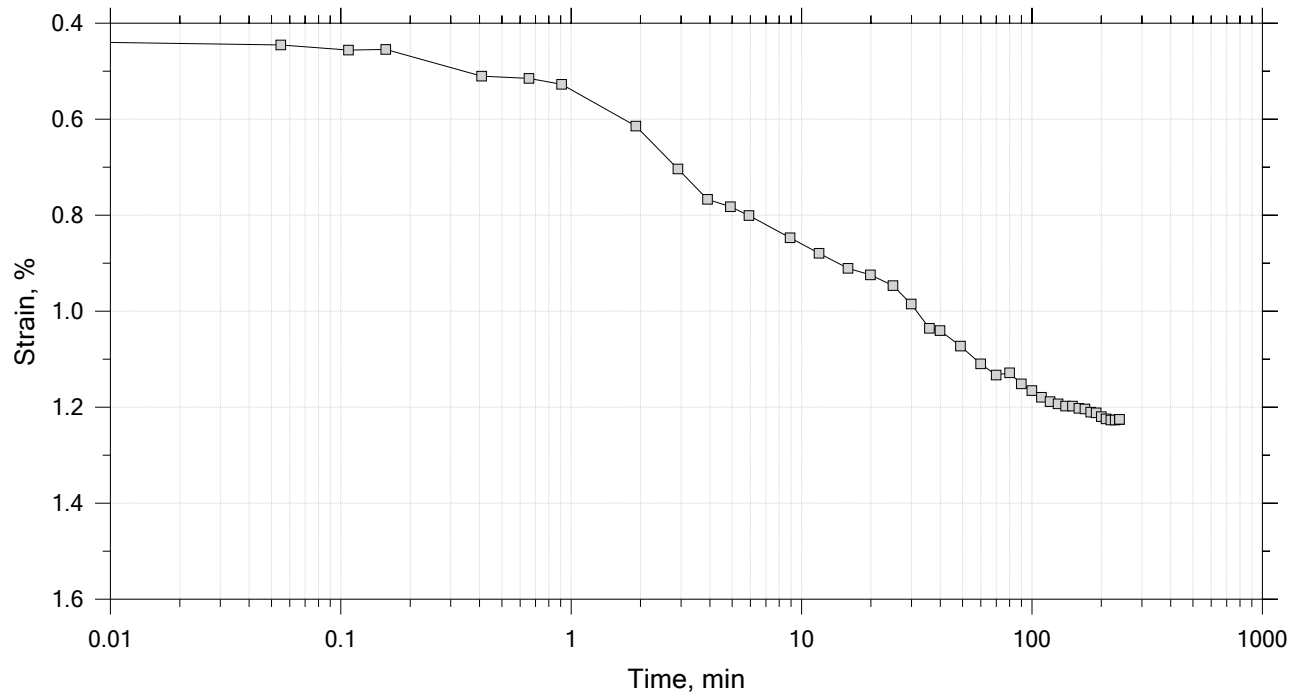
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



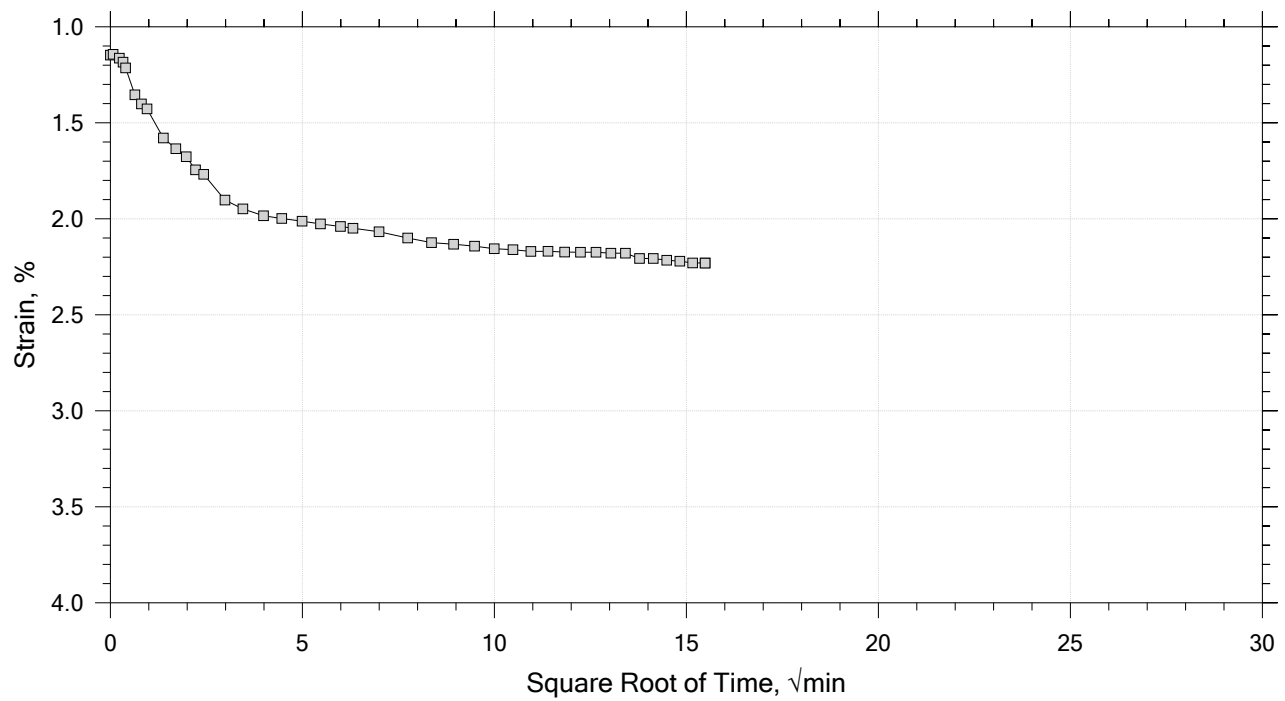
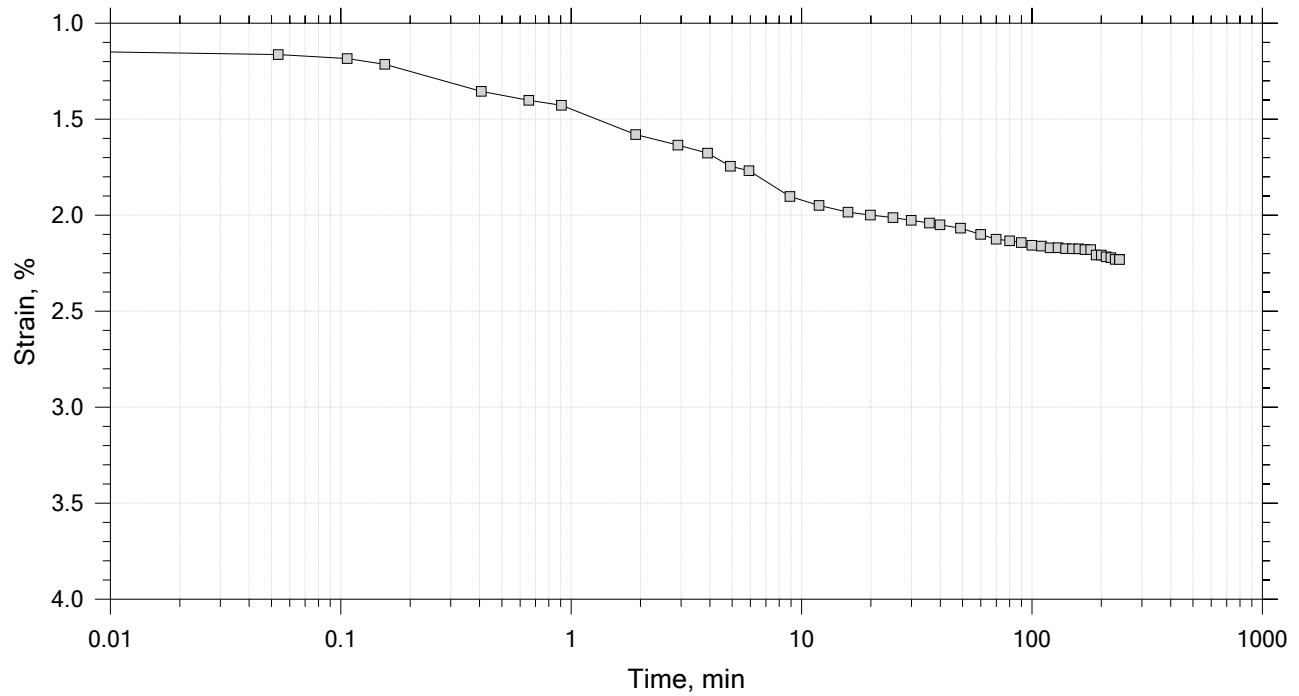
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



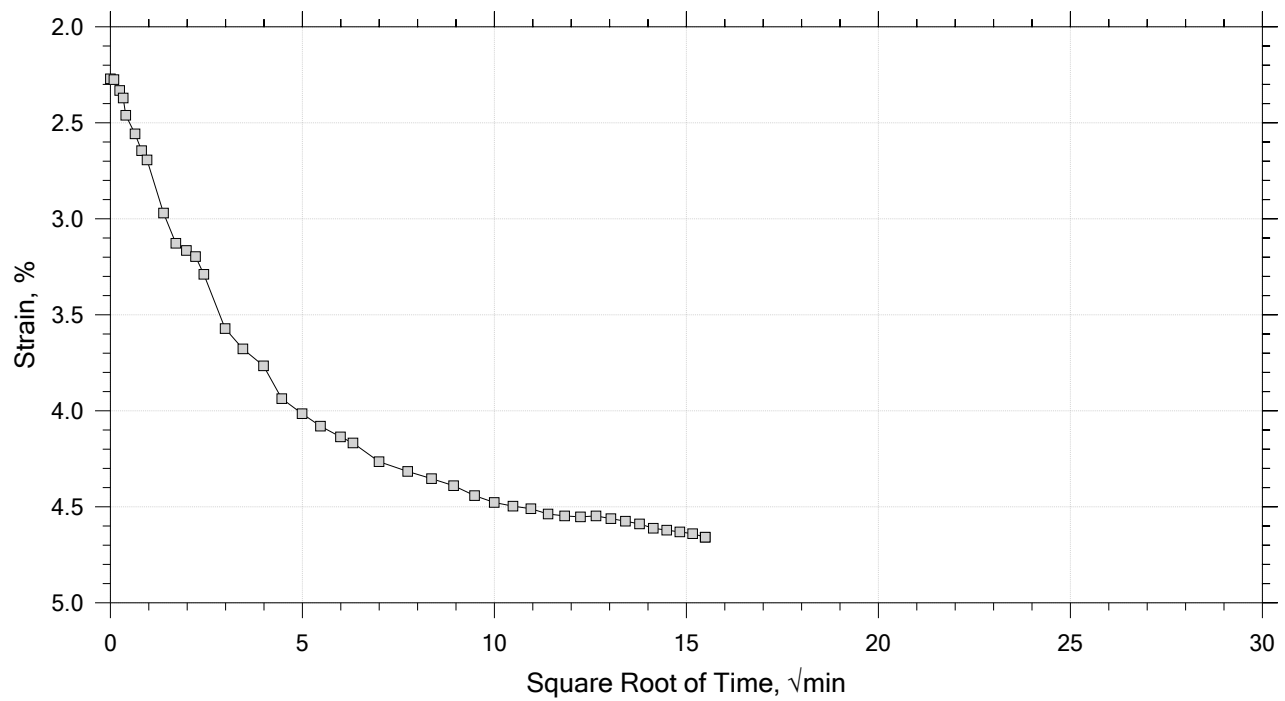
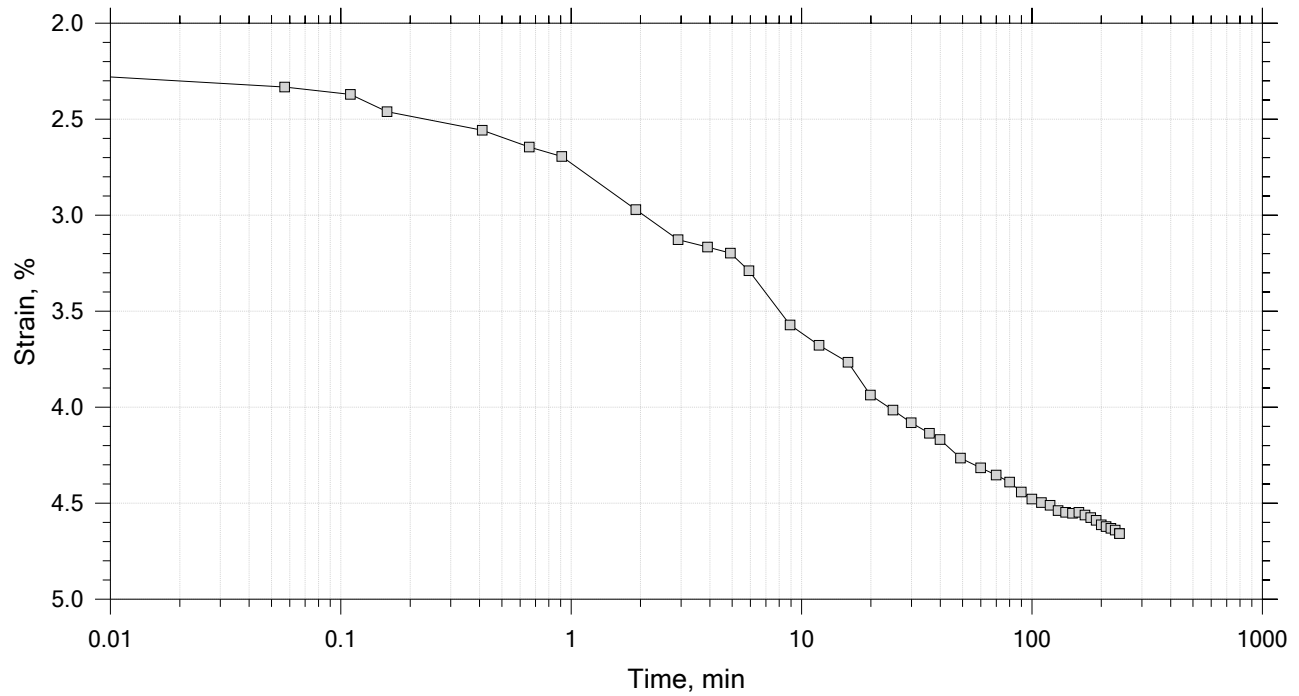
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



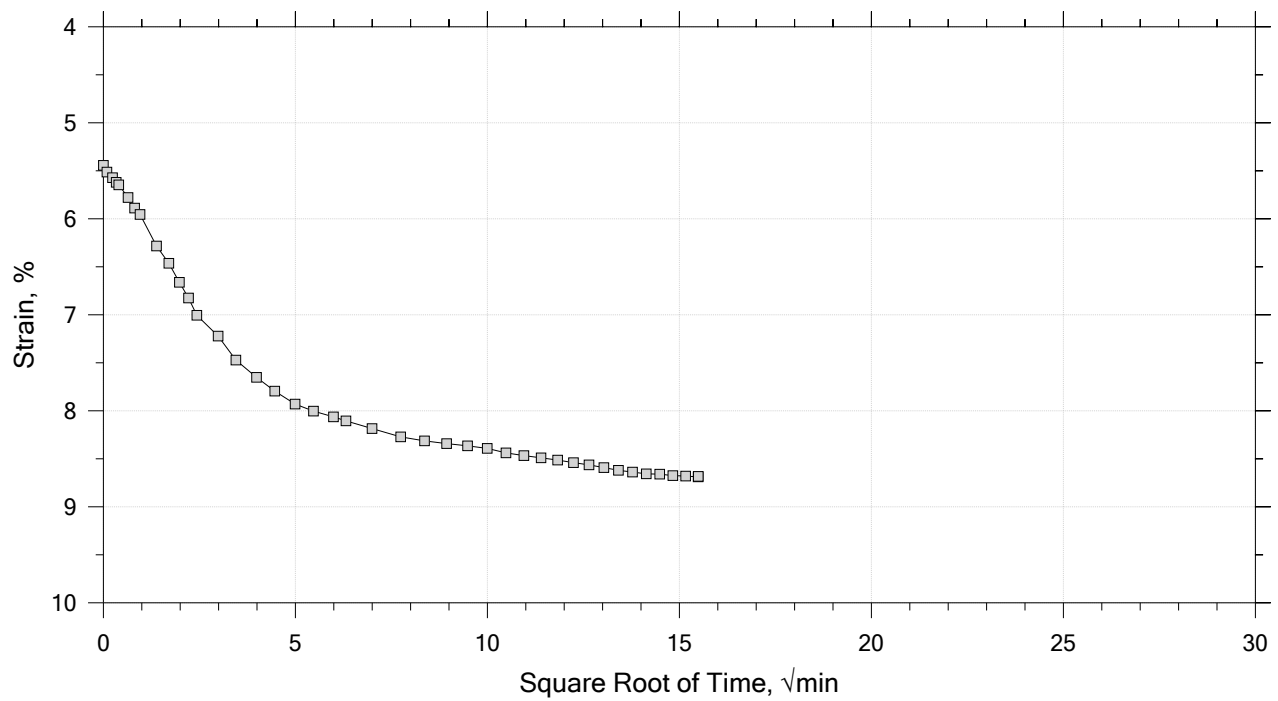
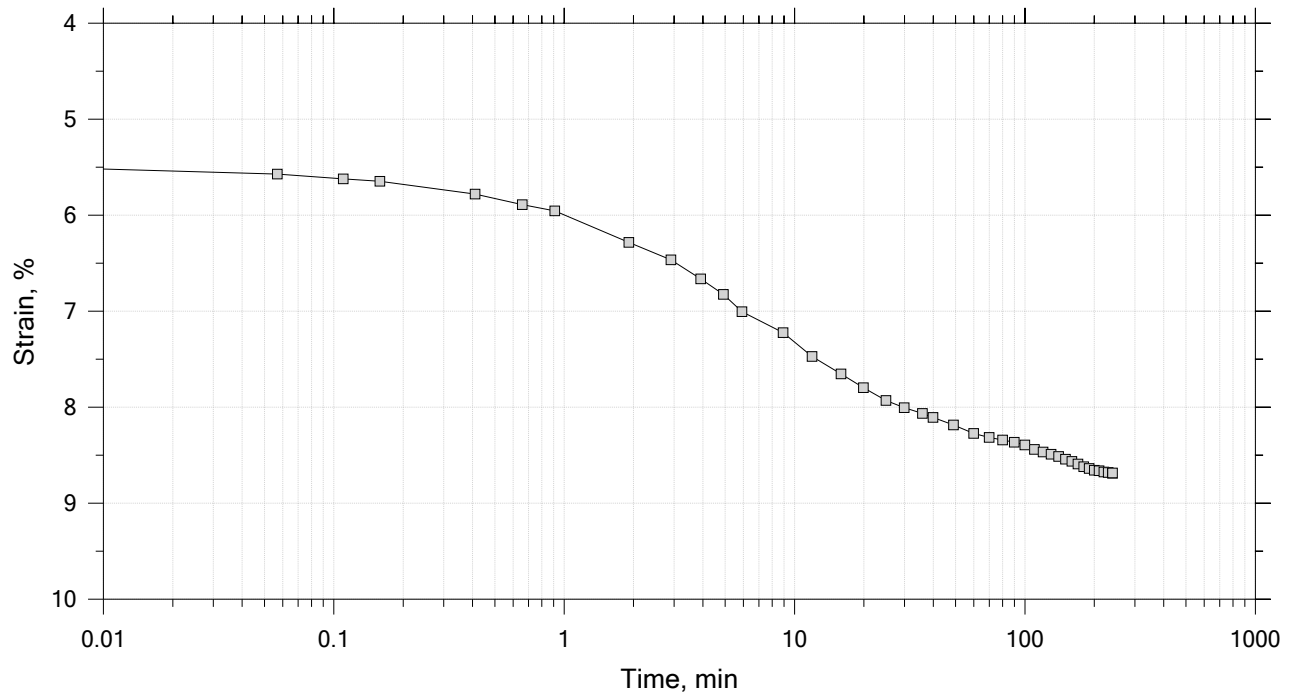
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



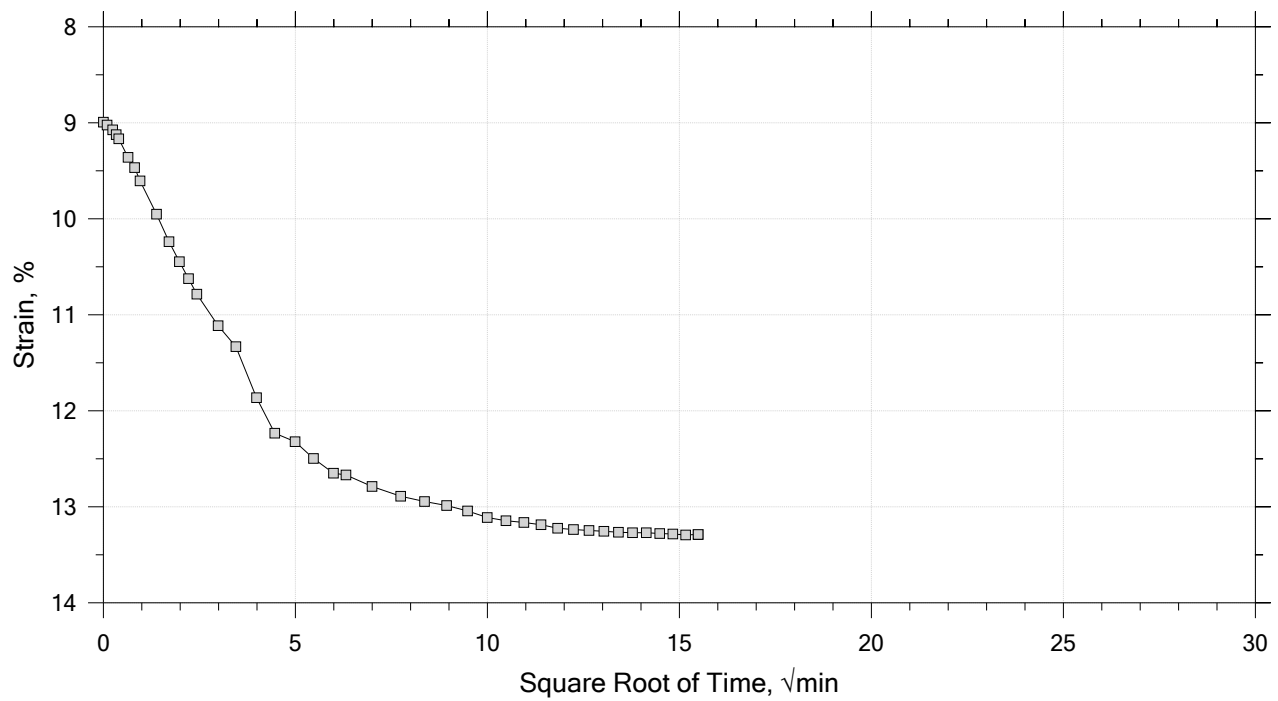
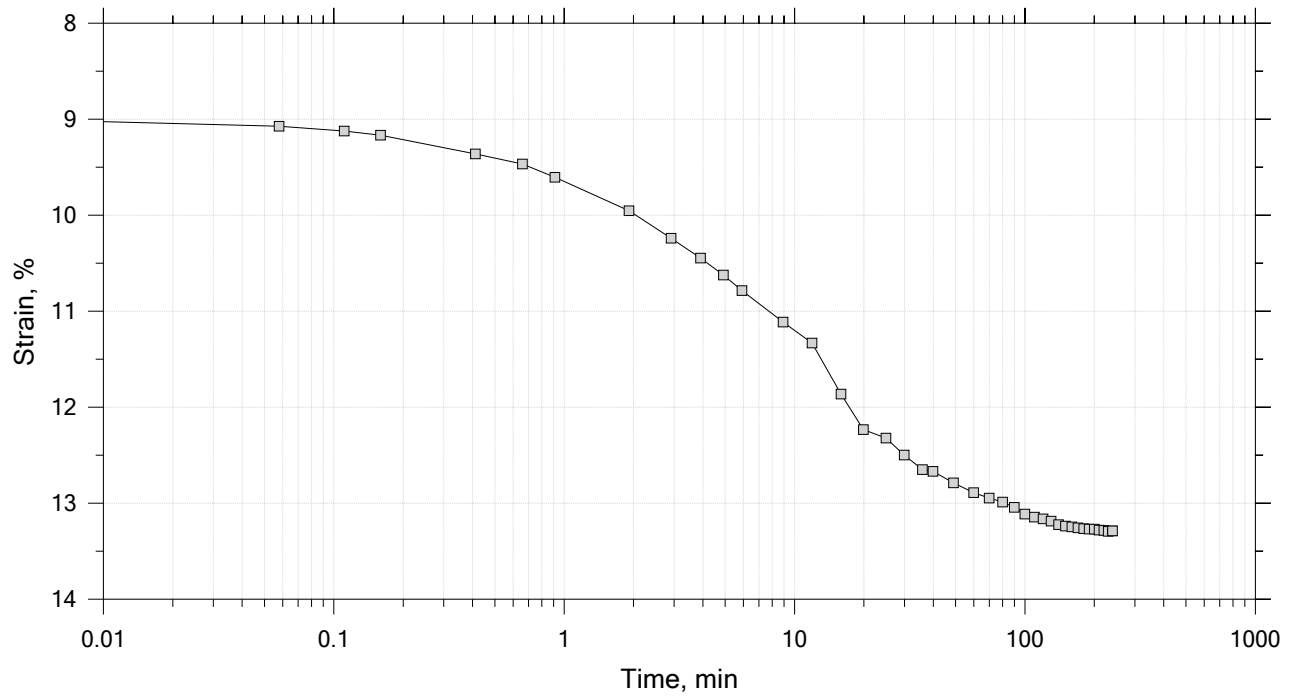
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



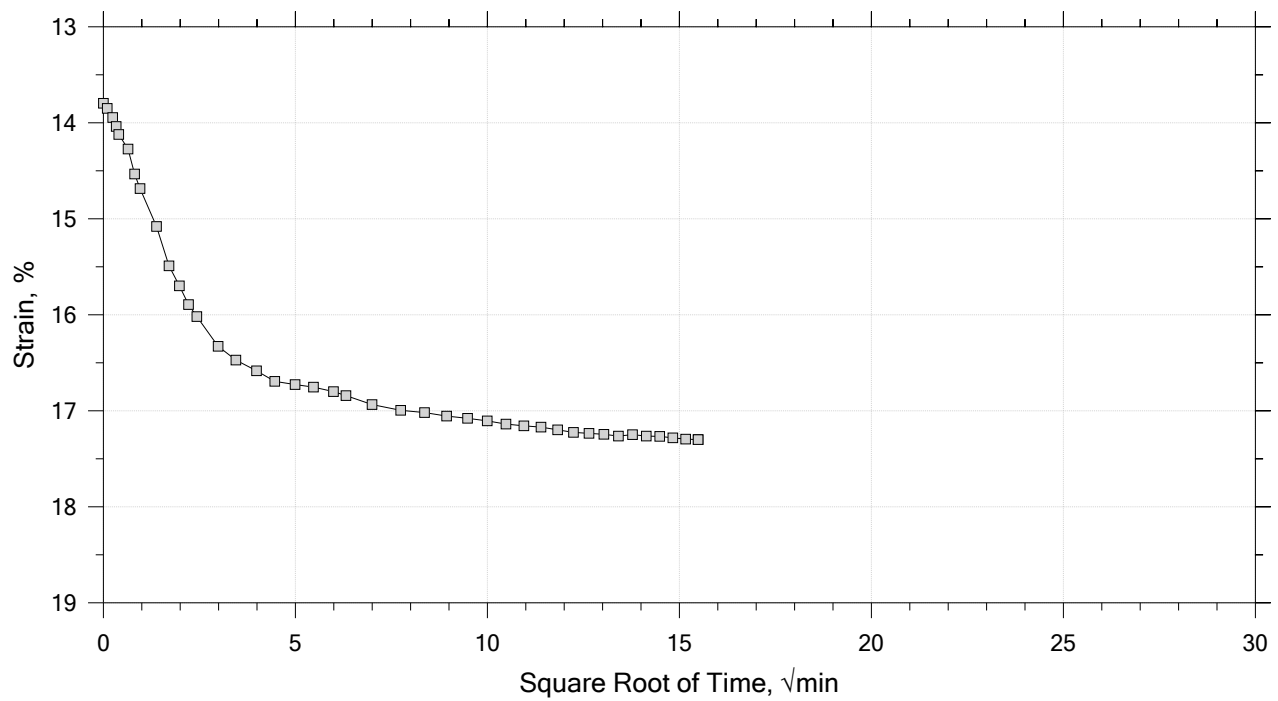
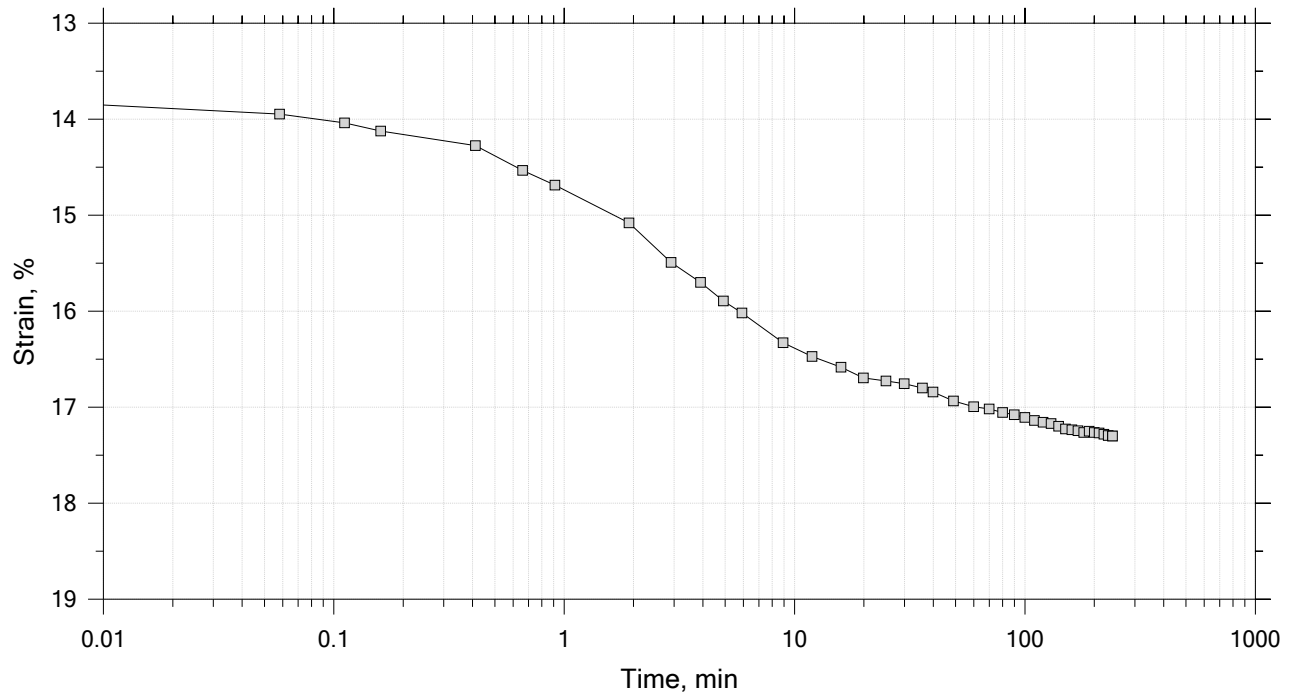
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



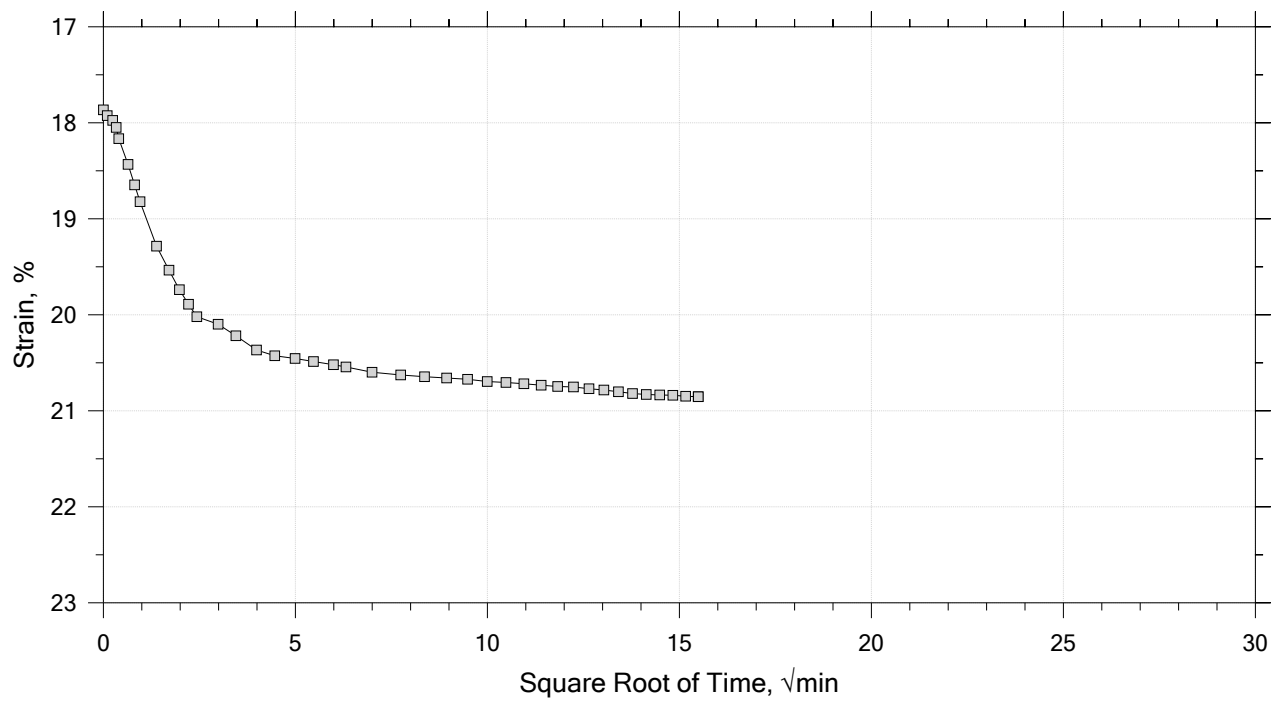
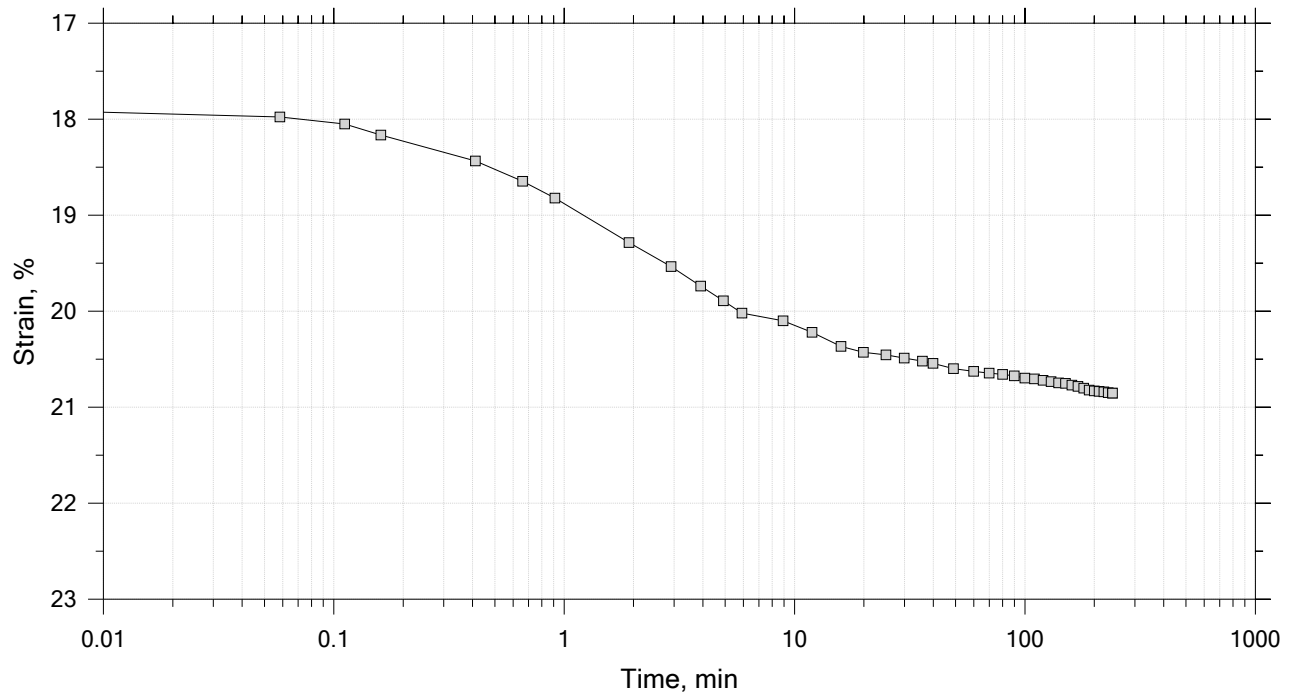
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



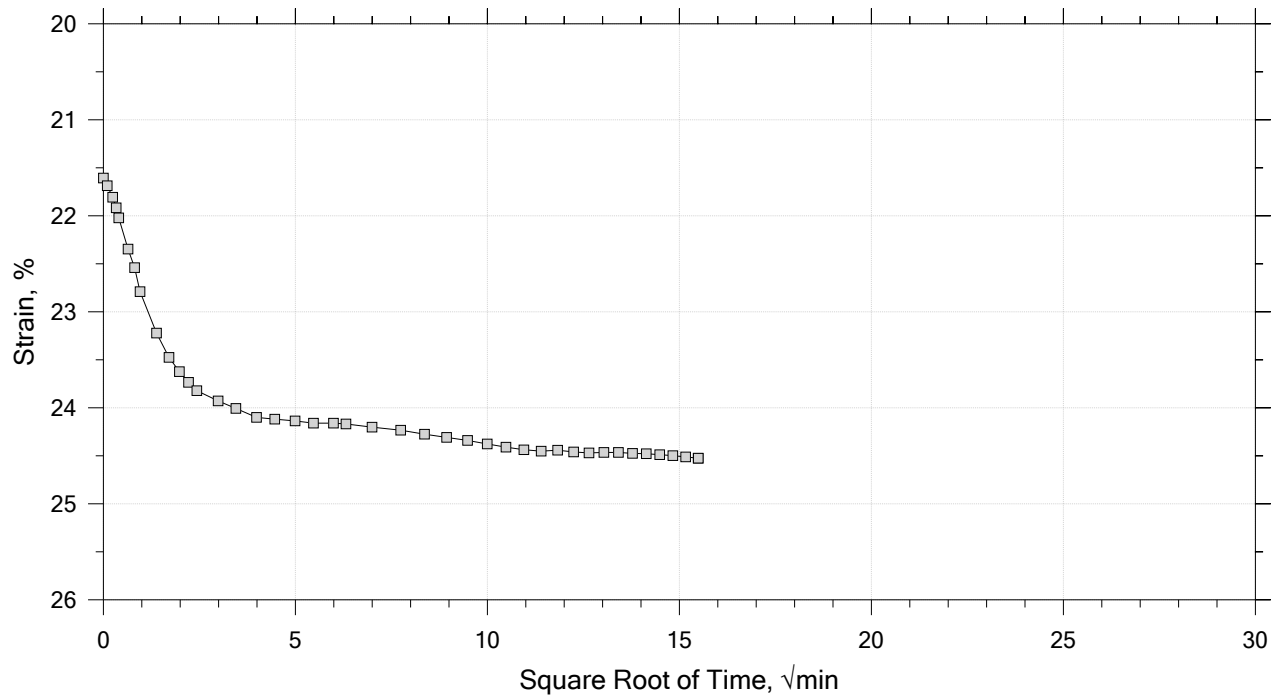
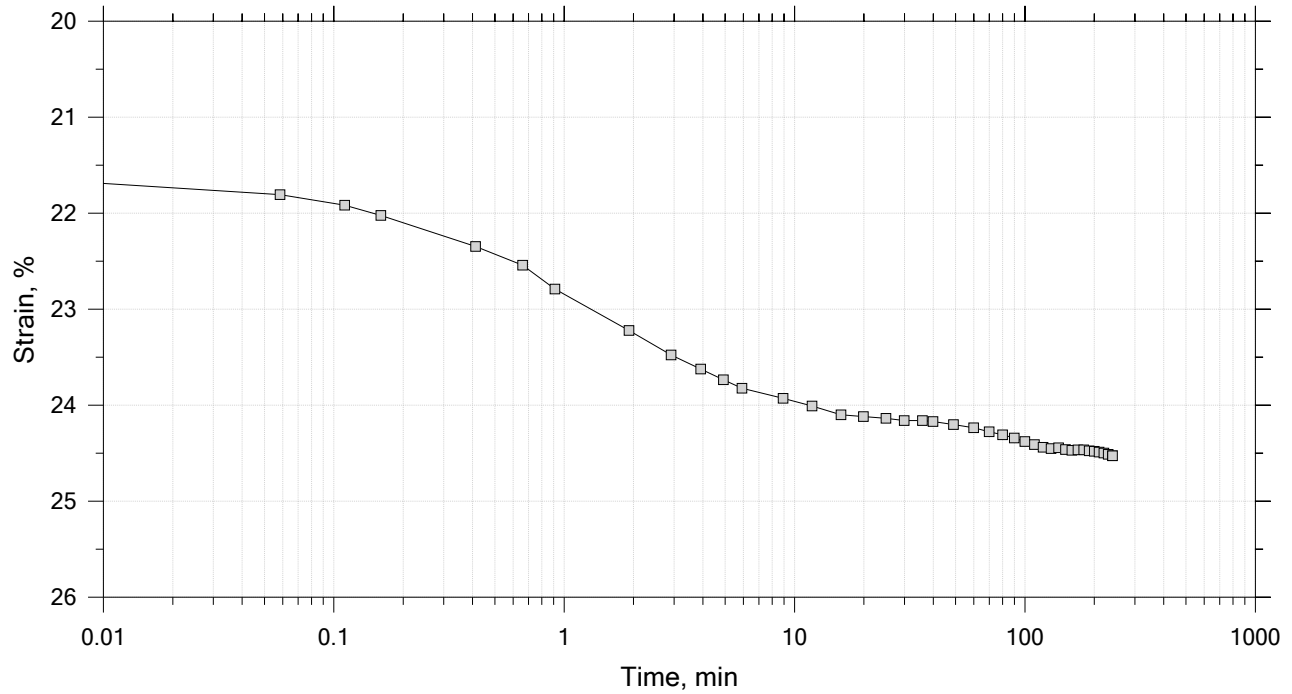
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



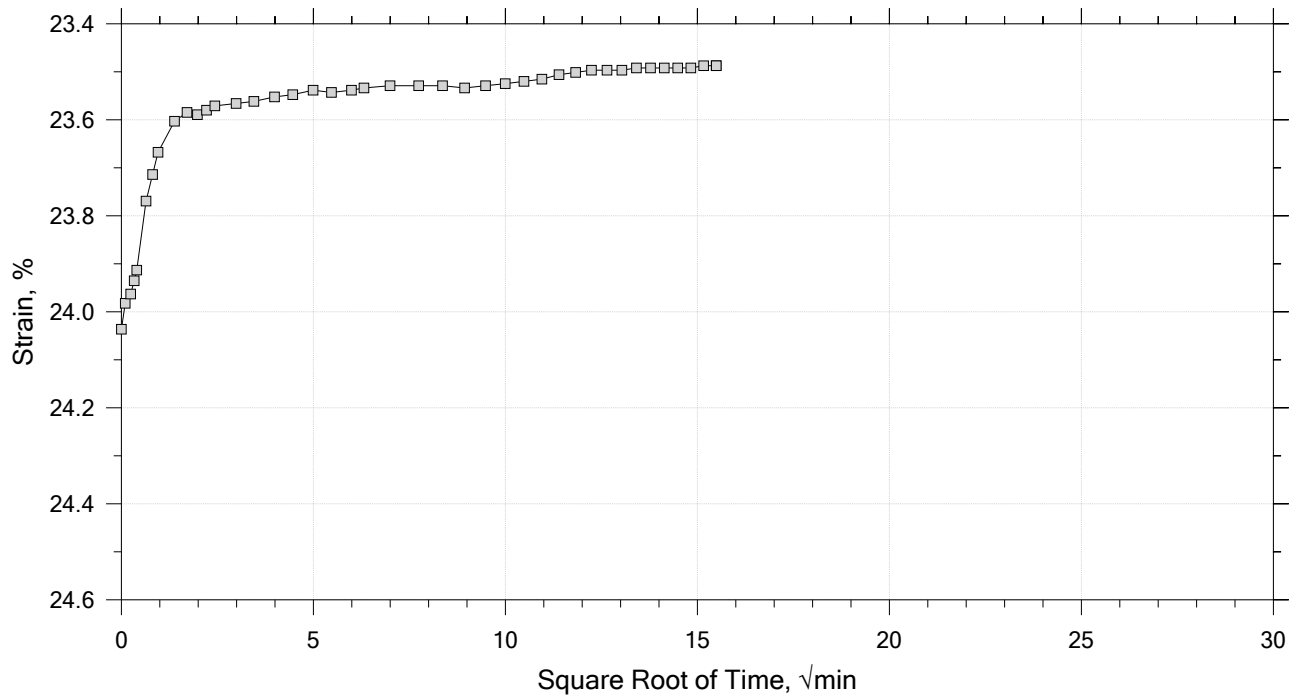
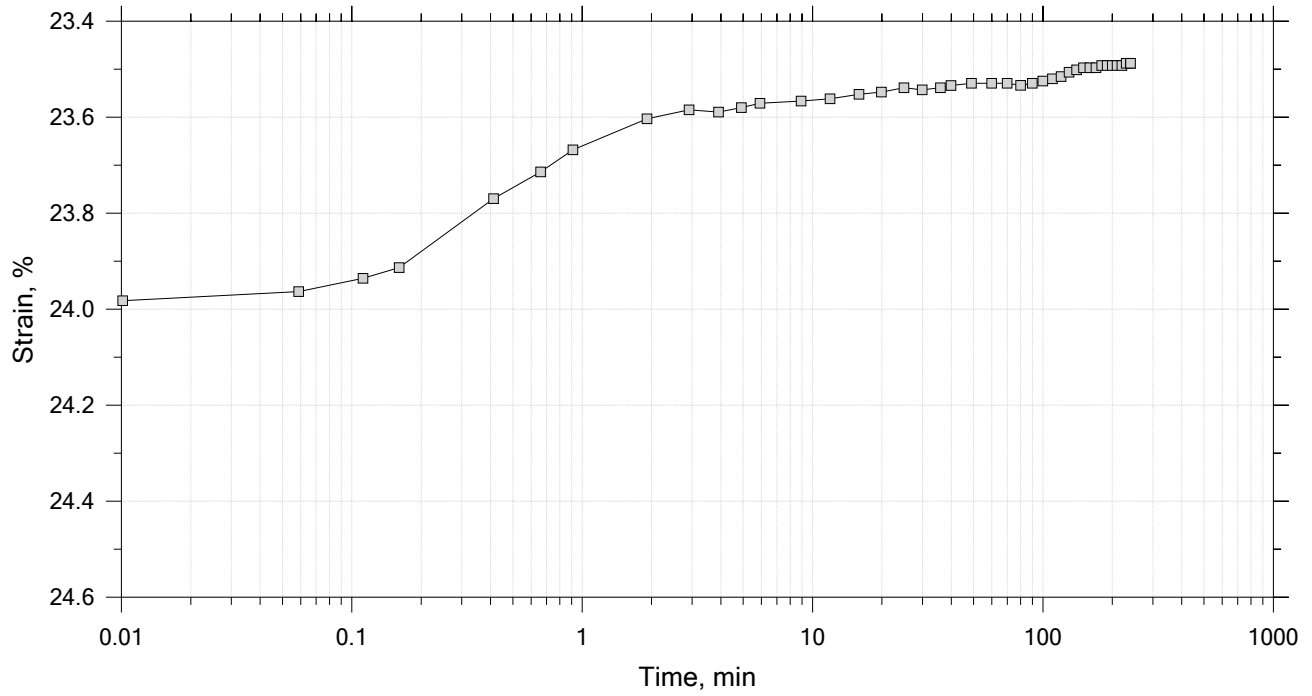
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



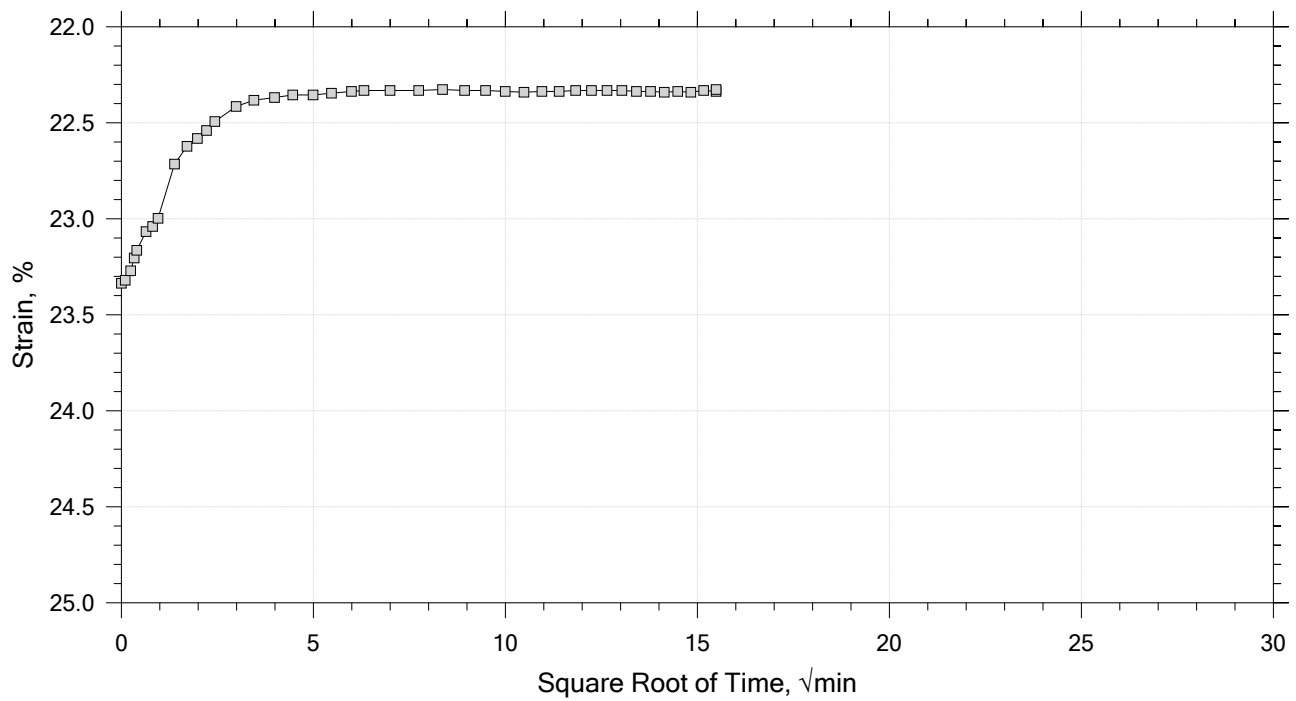
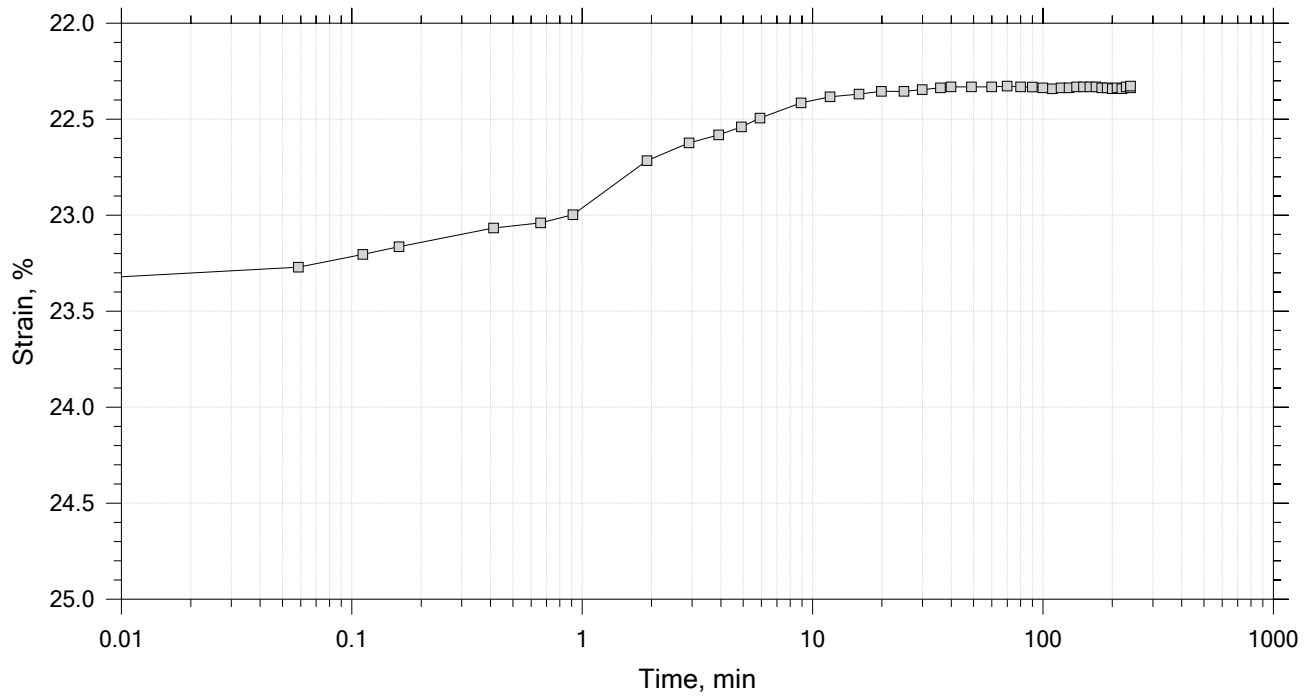
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	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



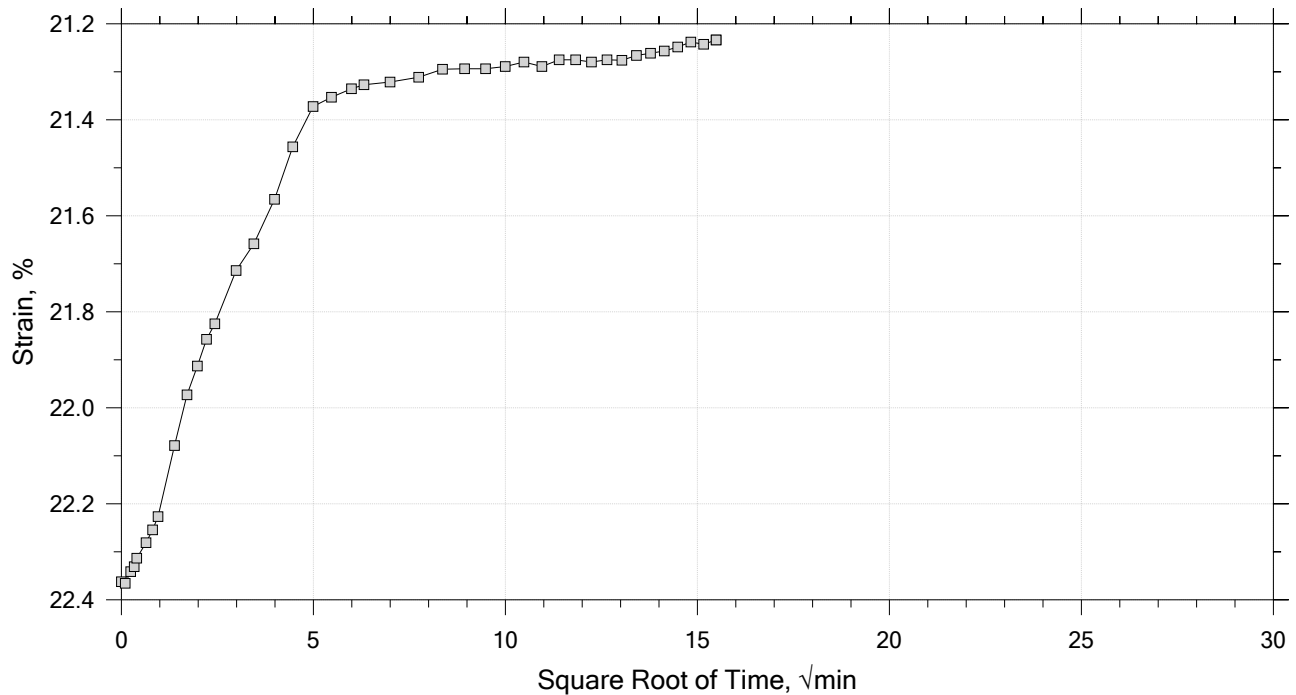
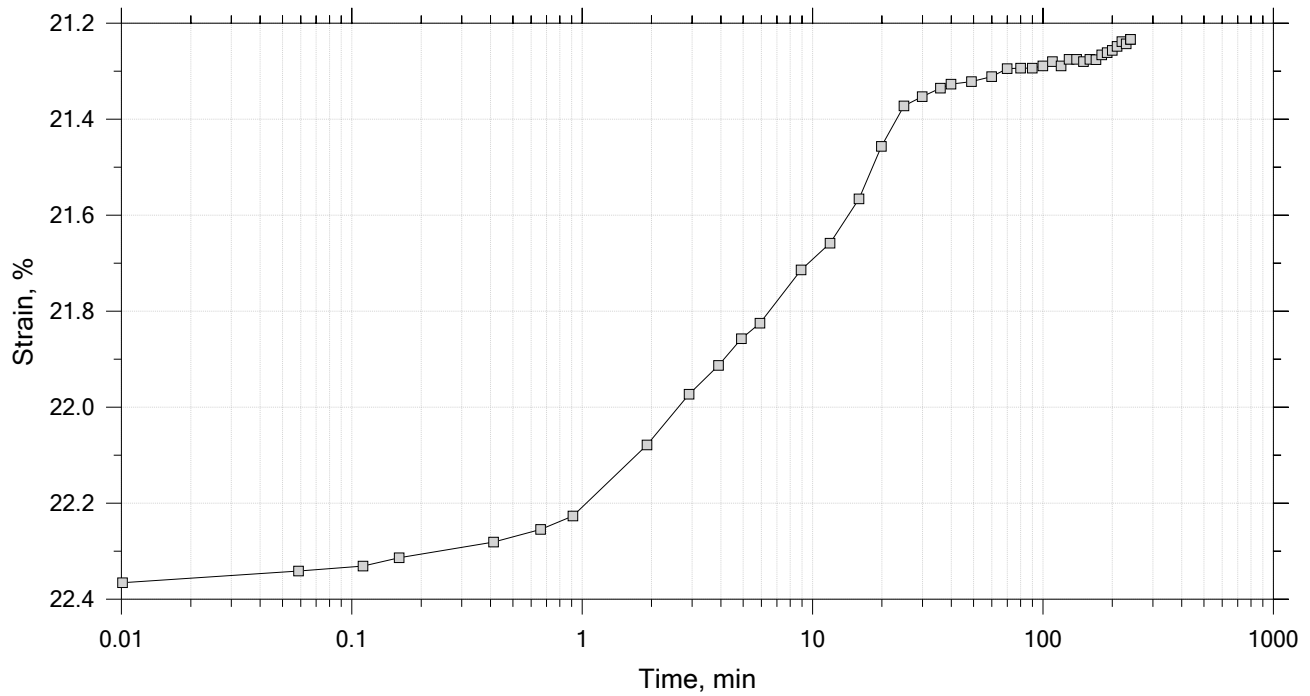
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



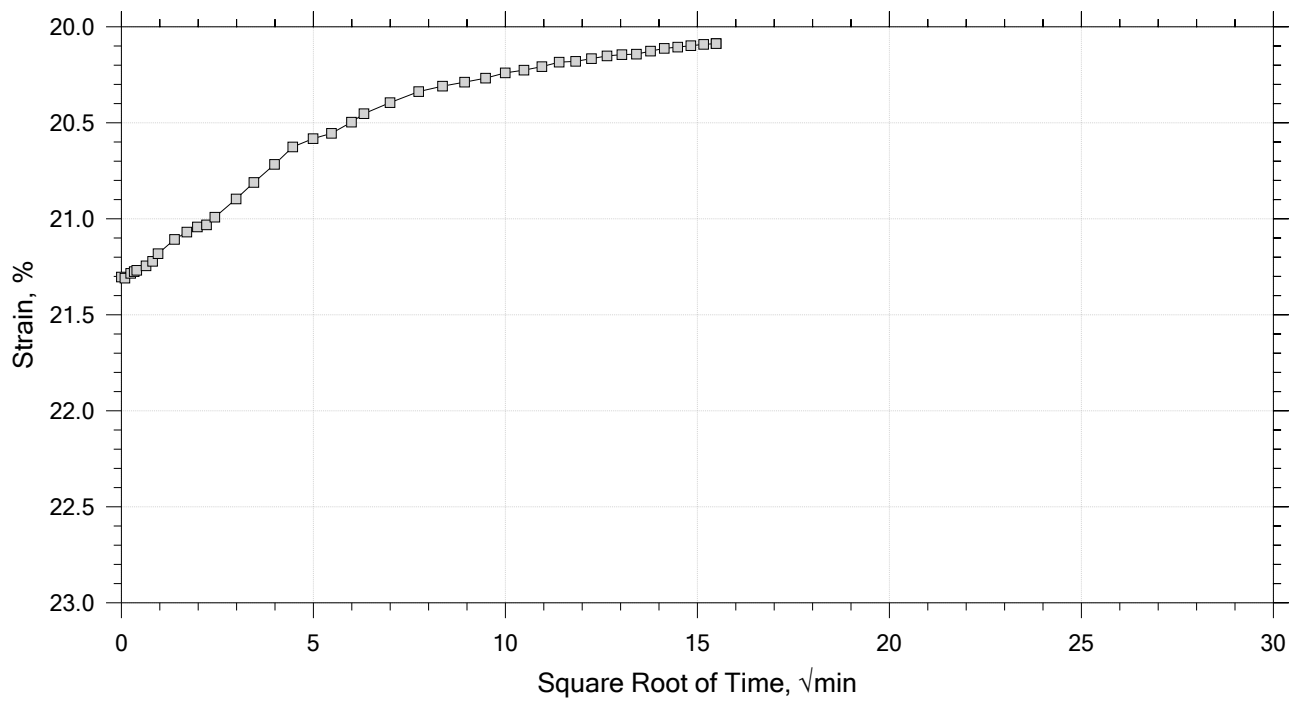
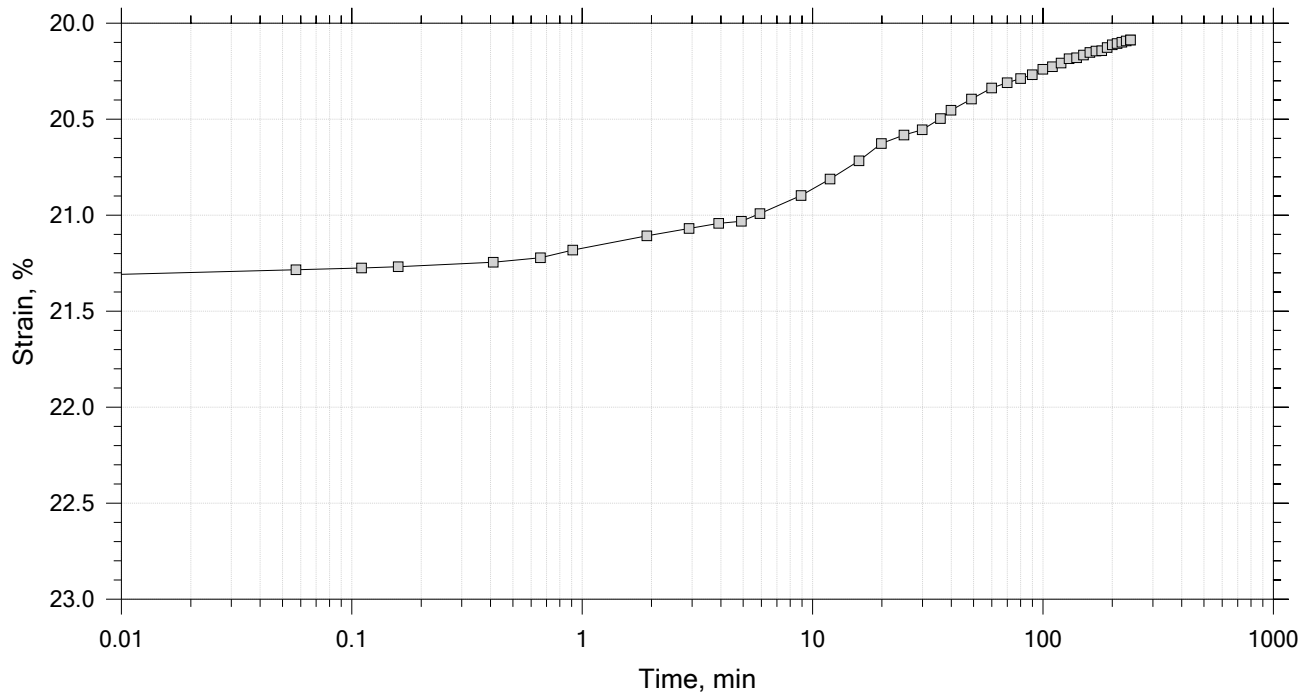
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



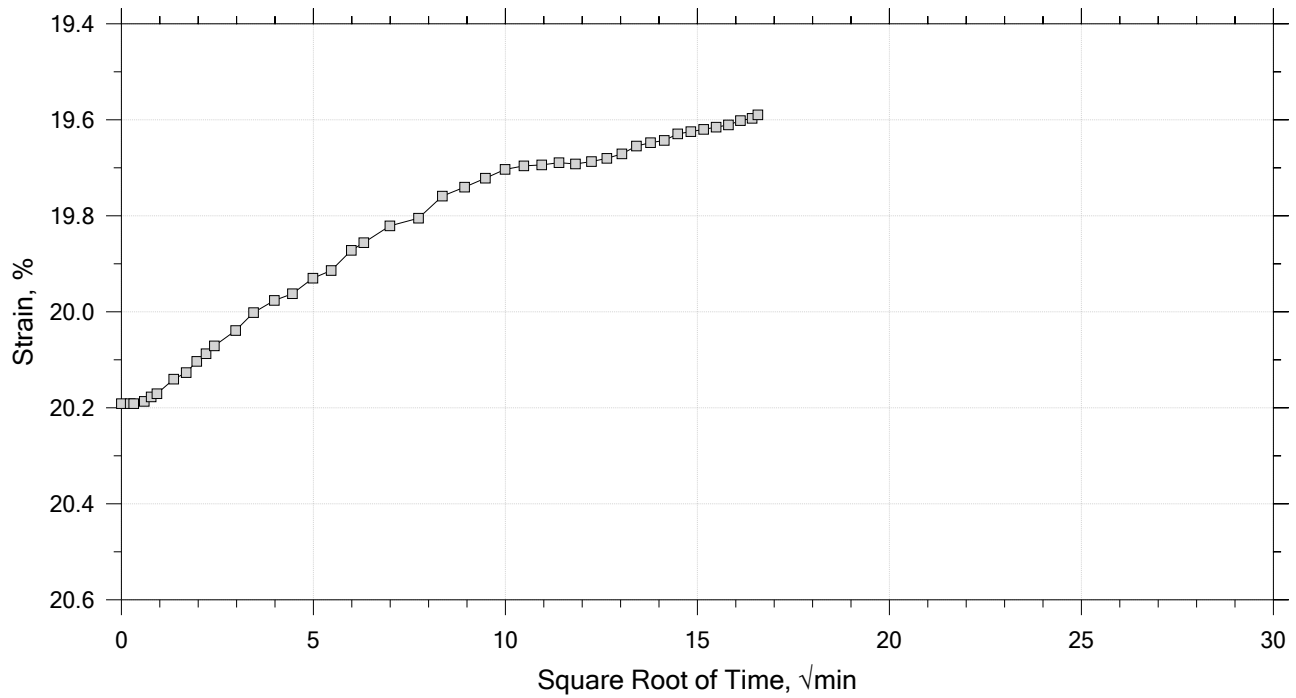
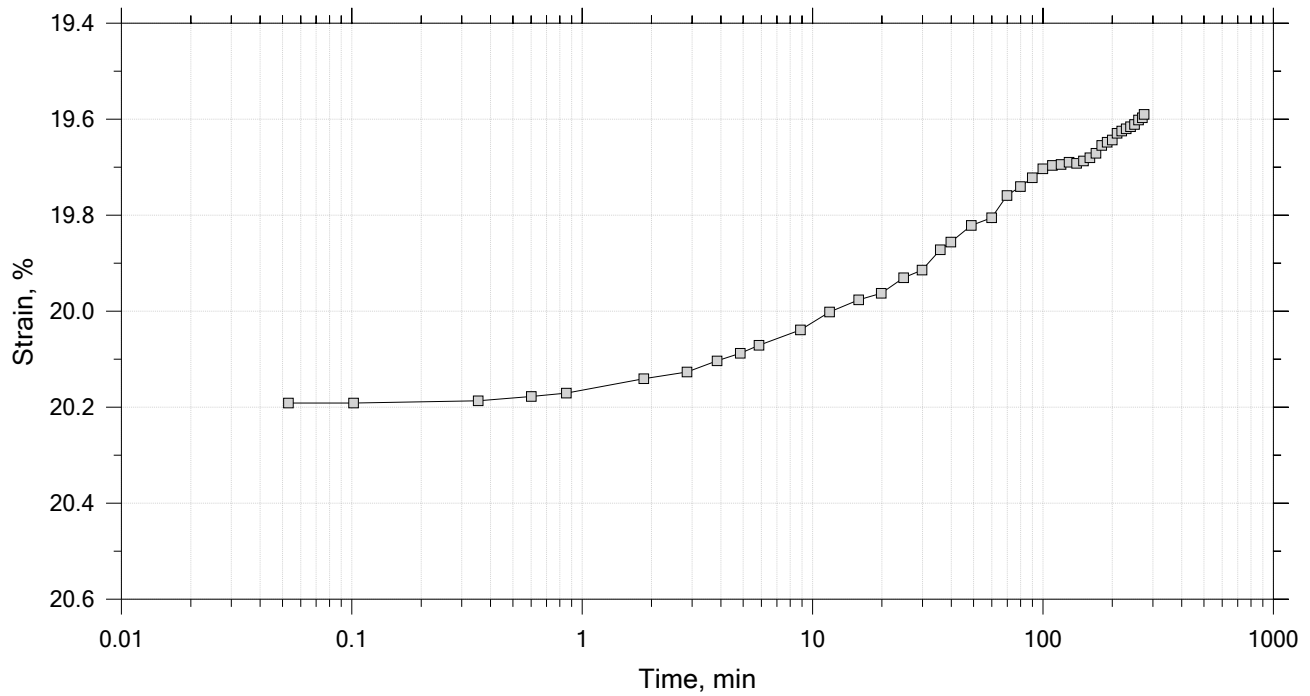
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.76	Liquid Limit: 38
Initial Height: 1.00 in	Initial Void Ratio: 0.952	Plastic Limit: 19
Final Height: 0.80 in	Final Void Ratio: 0.57	Plasticity Index: 19

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A-1250	RING		A-3059
Mass Container, gm	8.43	107.87	107.87	8.31
Mass Container + Wet Soil, gm	141.72	259.4	245.06	145.65
Mass Container + Dry Soil, gm	106.38	221.59	221.59	122.15
Mass Dry Soil, gm	97.95	113.72	113.72	113.84
Water Content, %	36.08	33.25	20.64	20.64
Void Ratio	---	0.95	0.57	---
Degree of Saturation, %	---	96.39	100.00	---
Dry Unit Weight, pcf	---	88.253	109.75	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

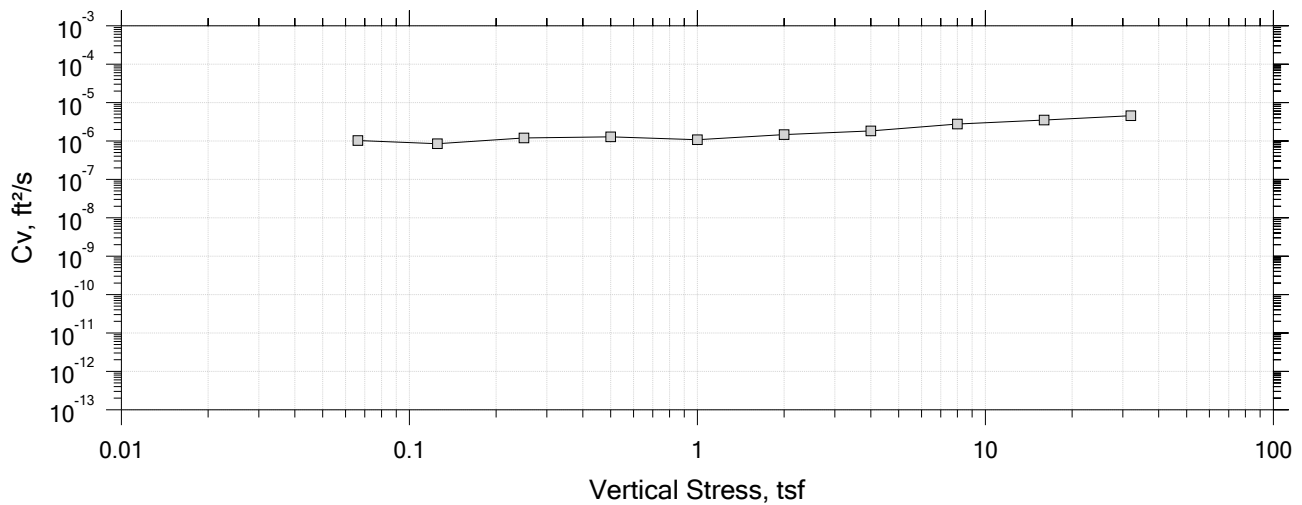
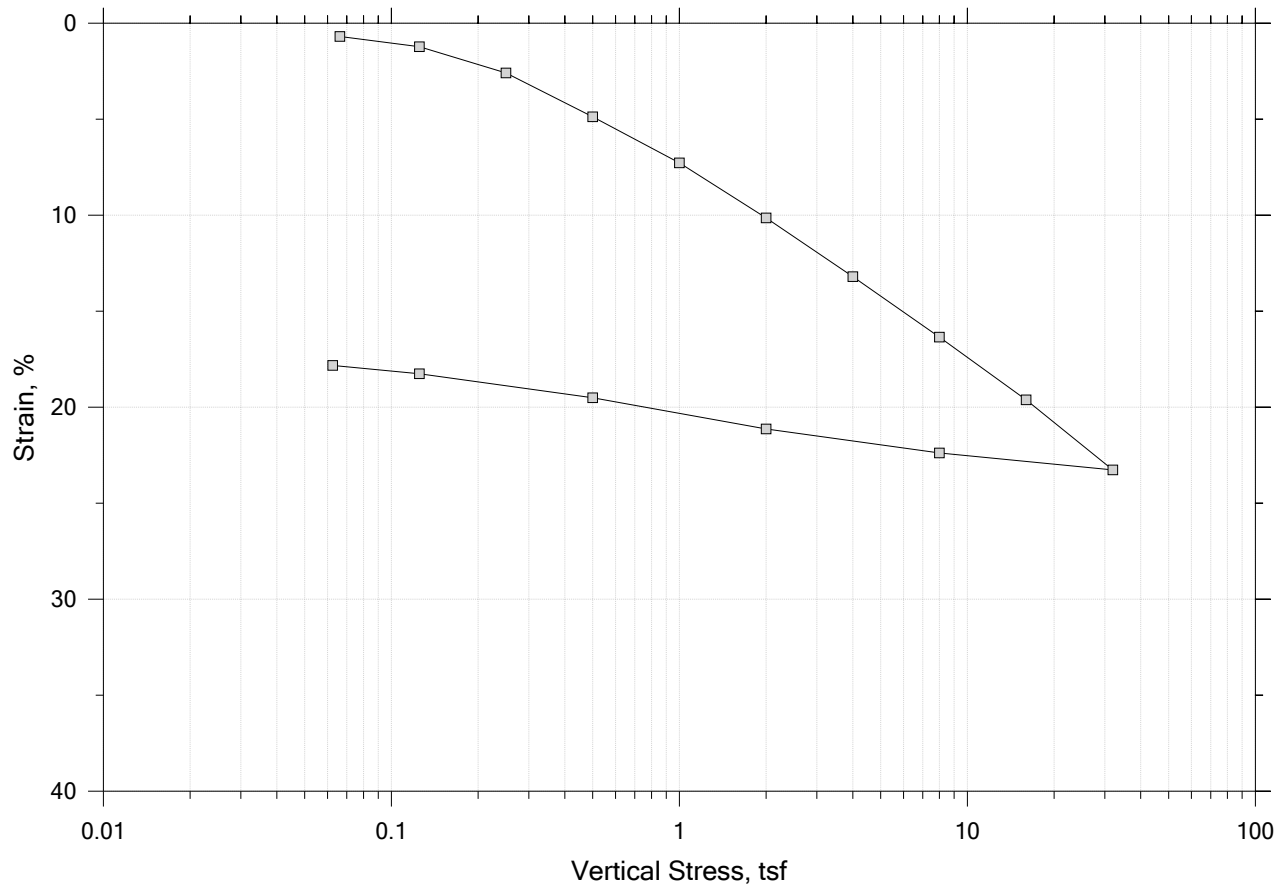
Square Root of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-103	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 10-12 ft
	Test No.: IP-14	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System V, Swell Pressure = 0.0703 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

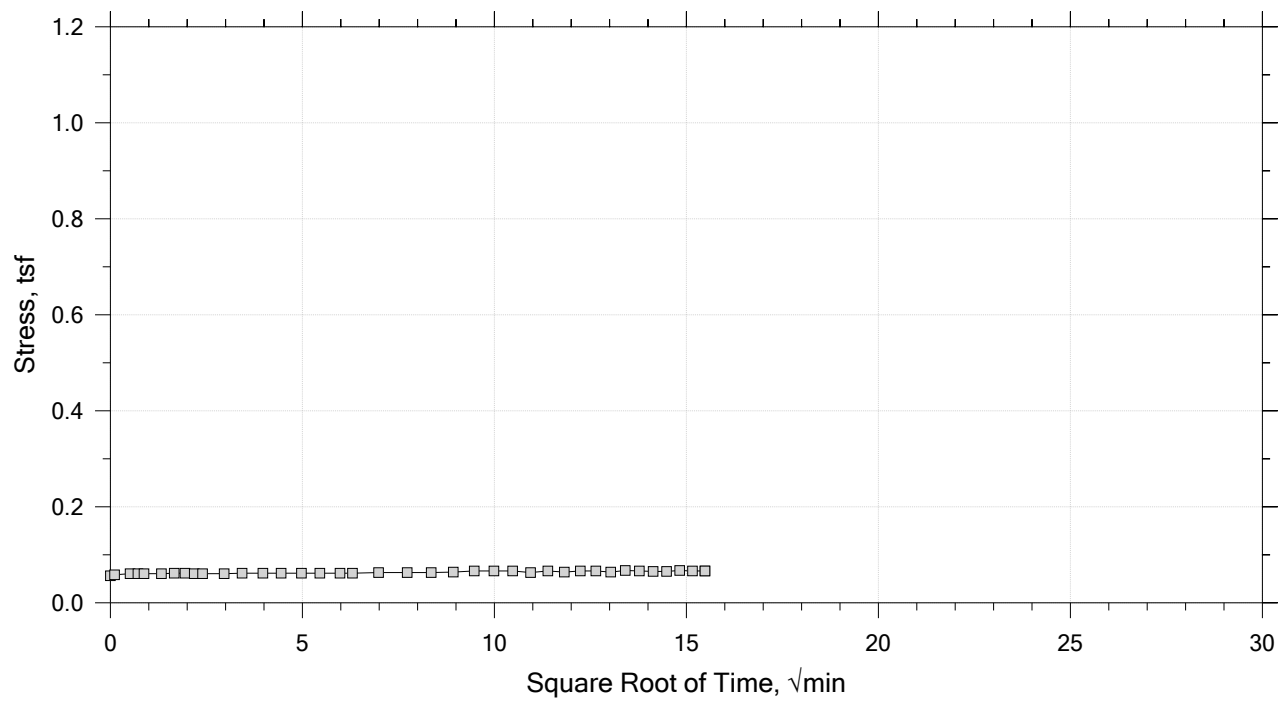
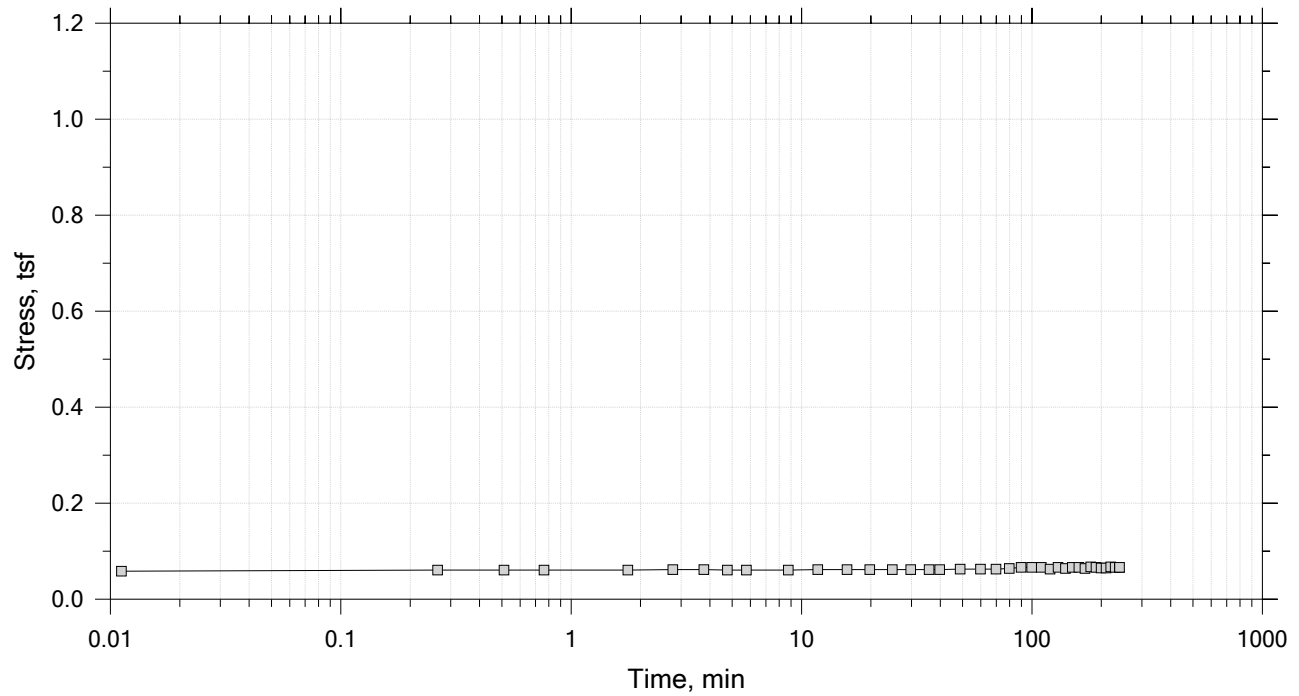
Summary Report




	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0662 tsf



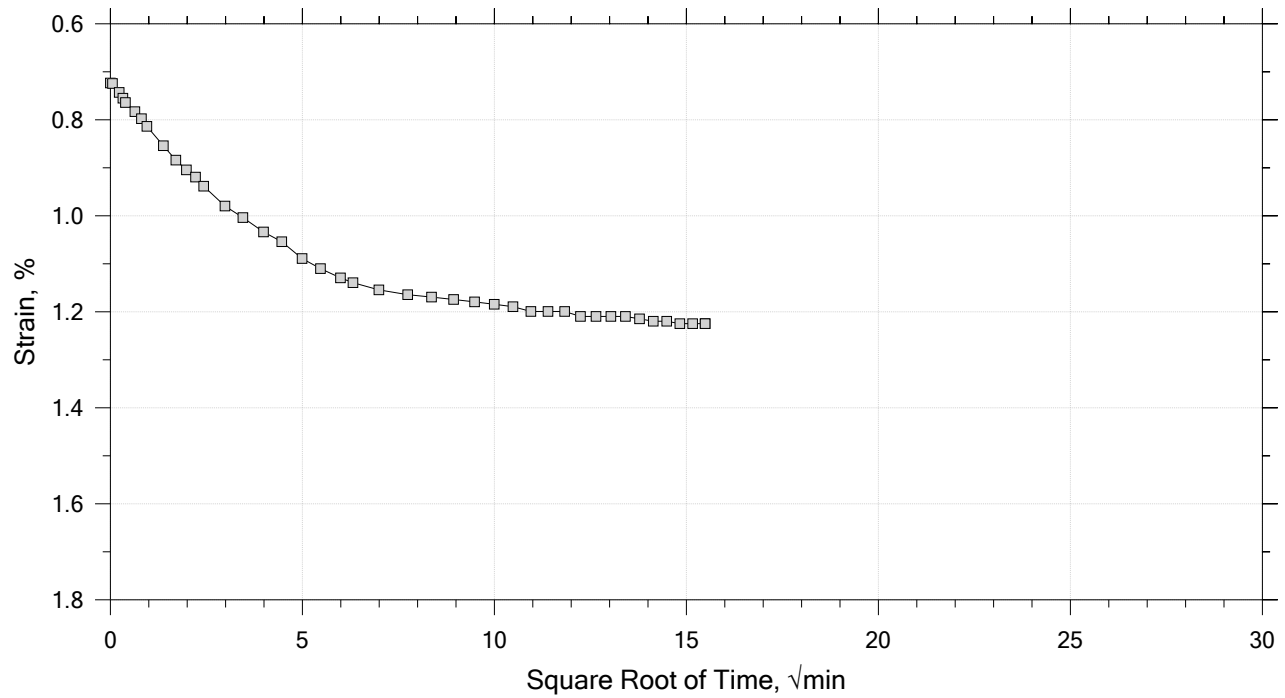
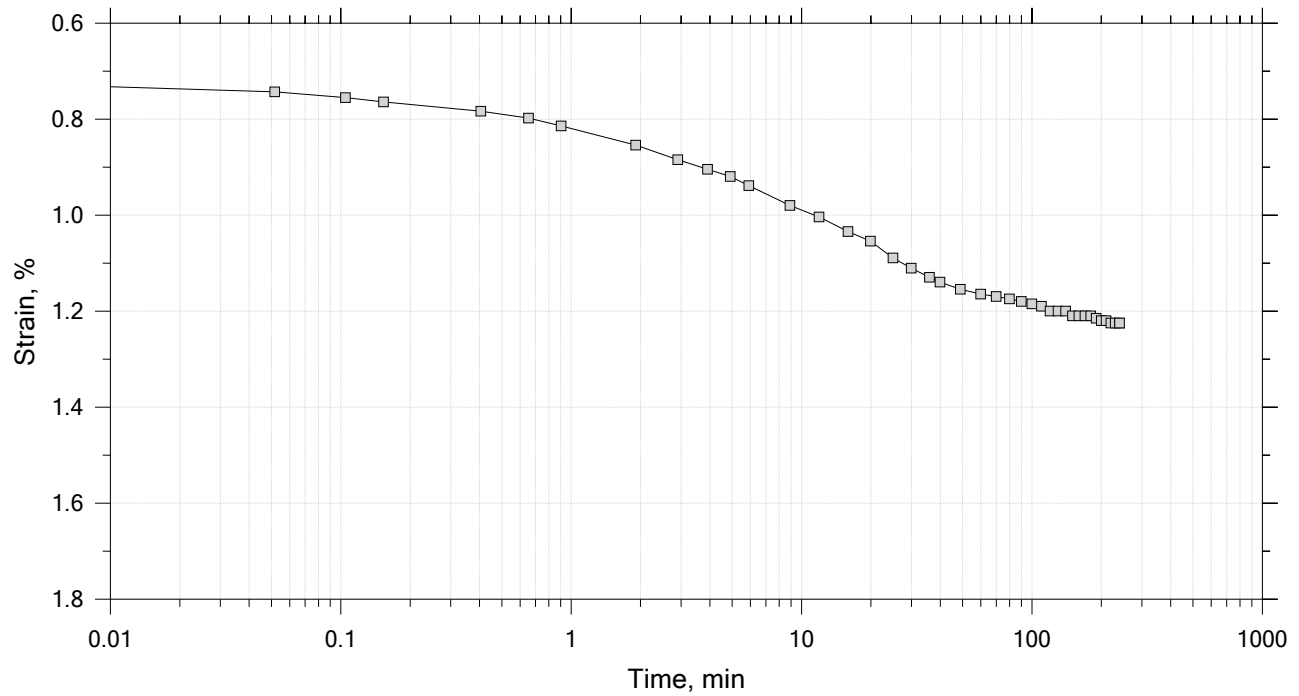
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



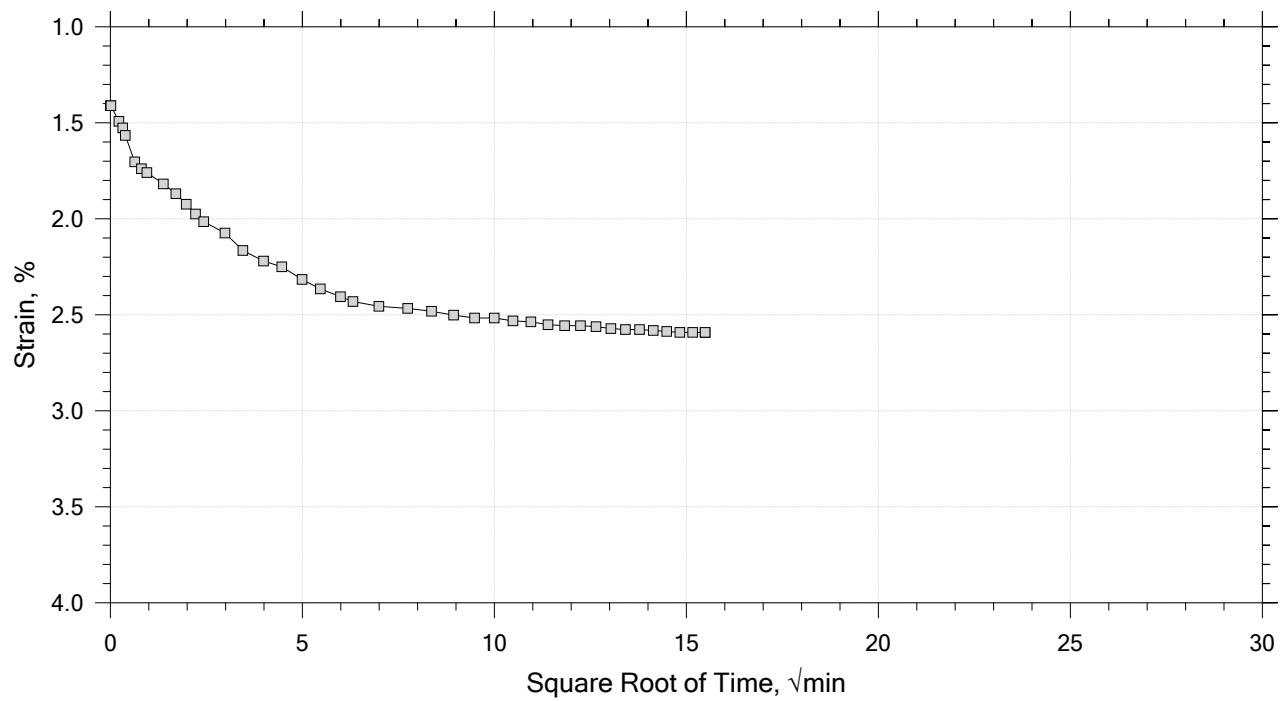
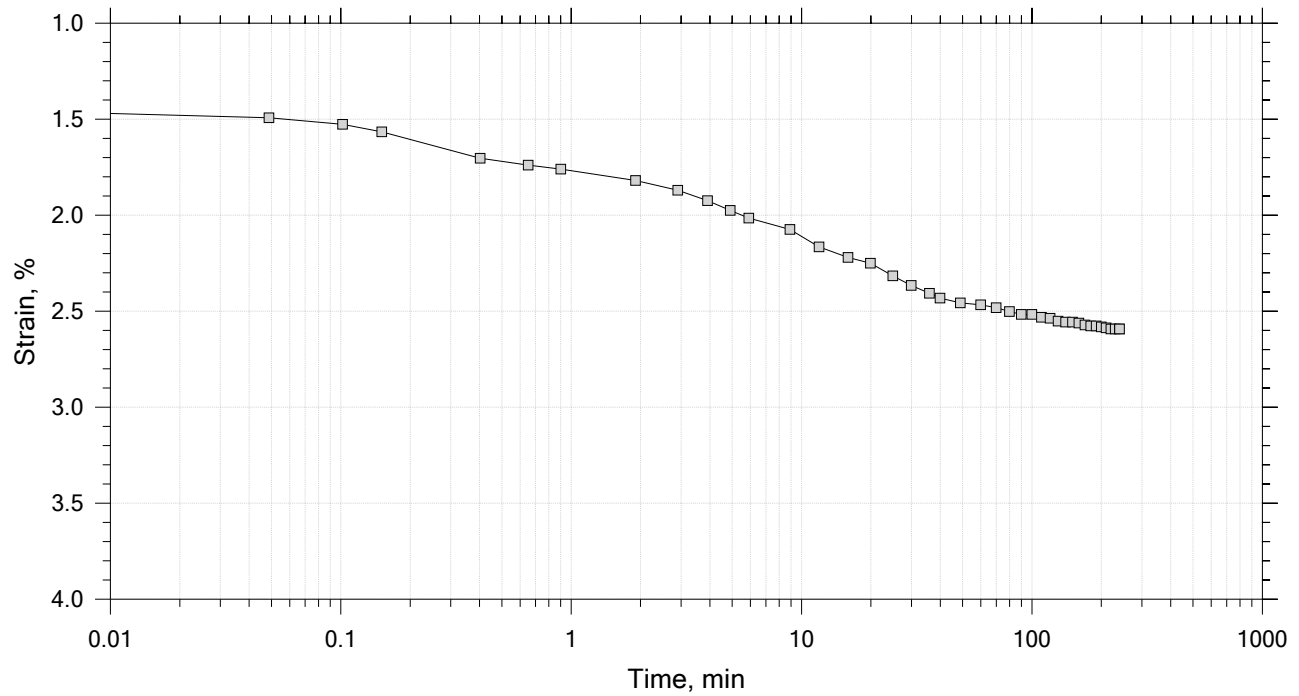
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



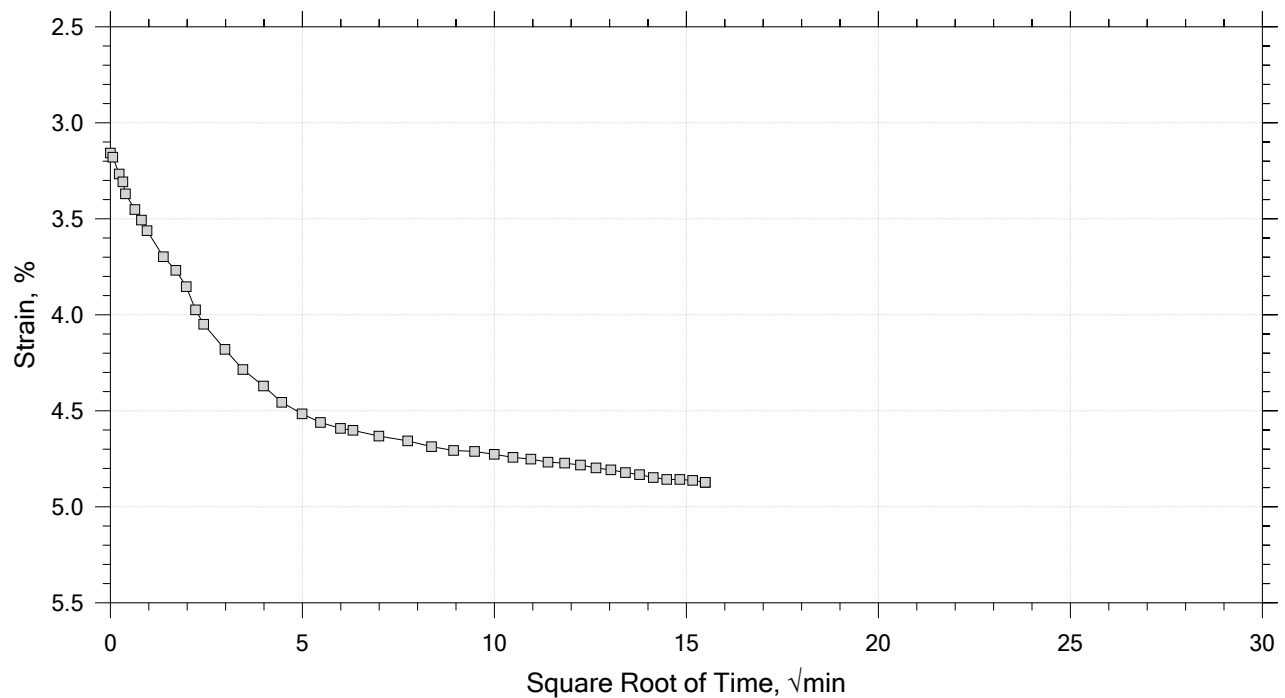
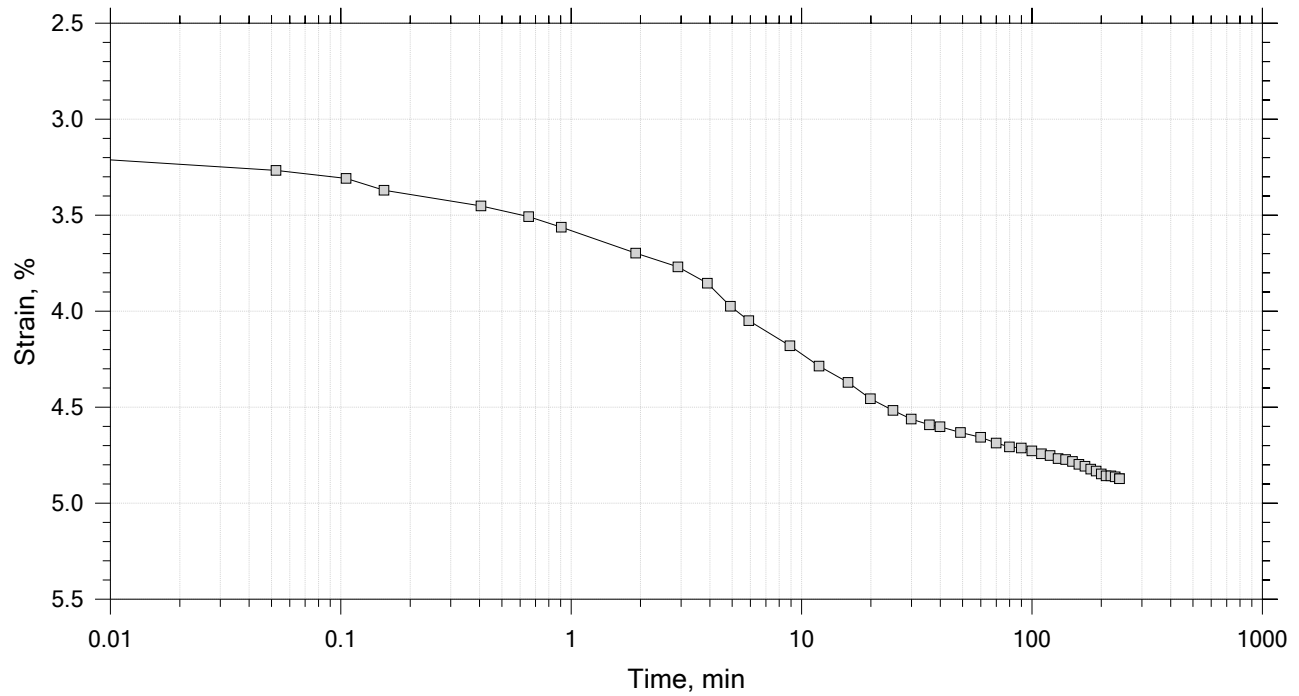
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



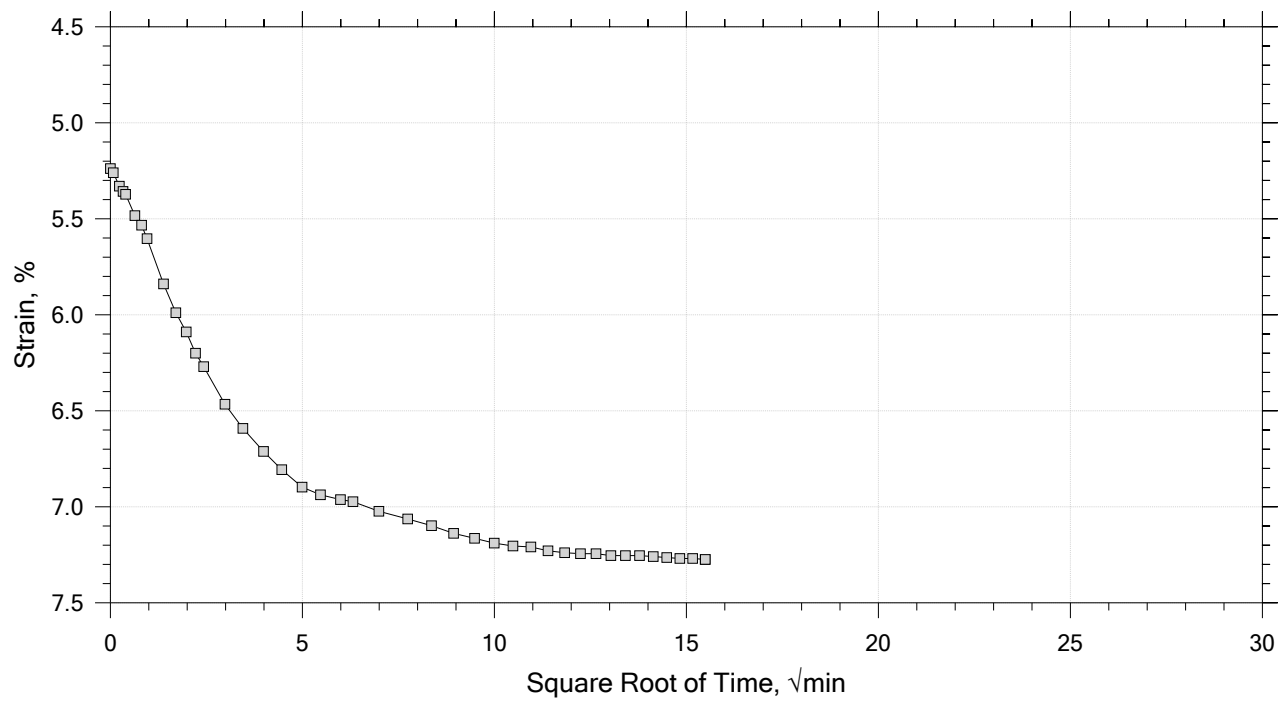
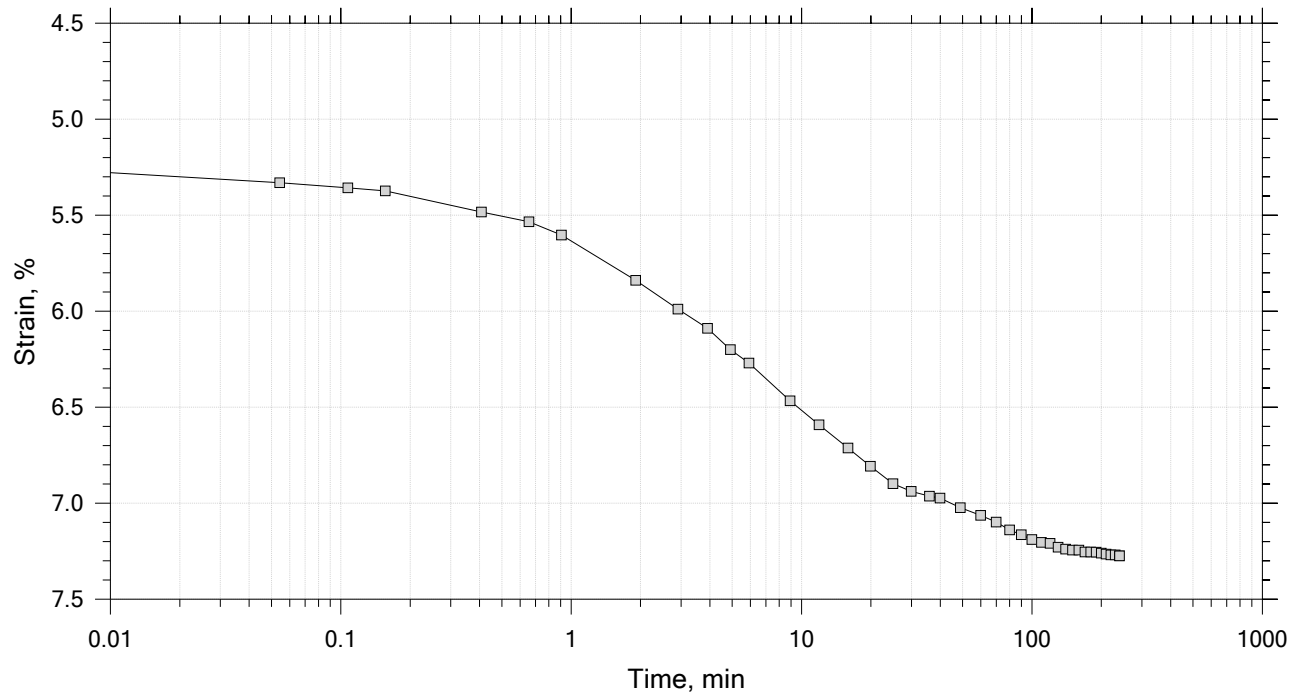
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



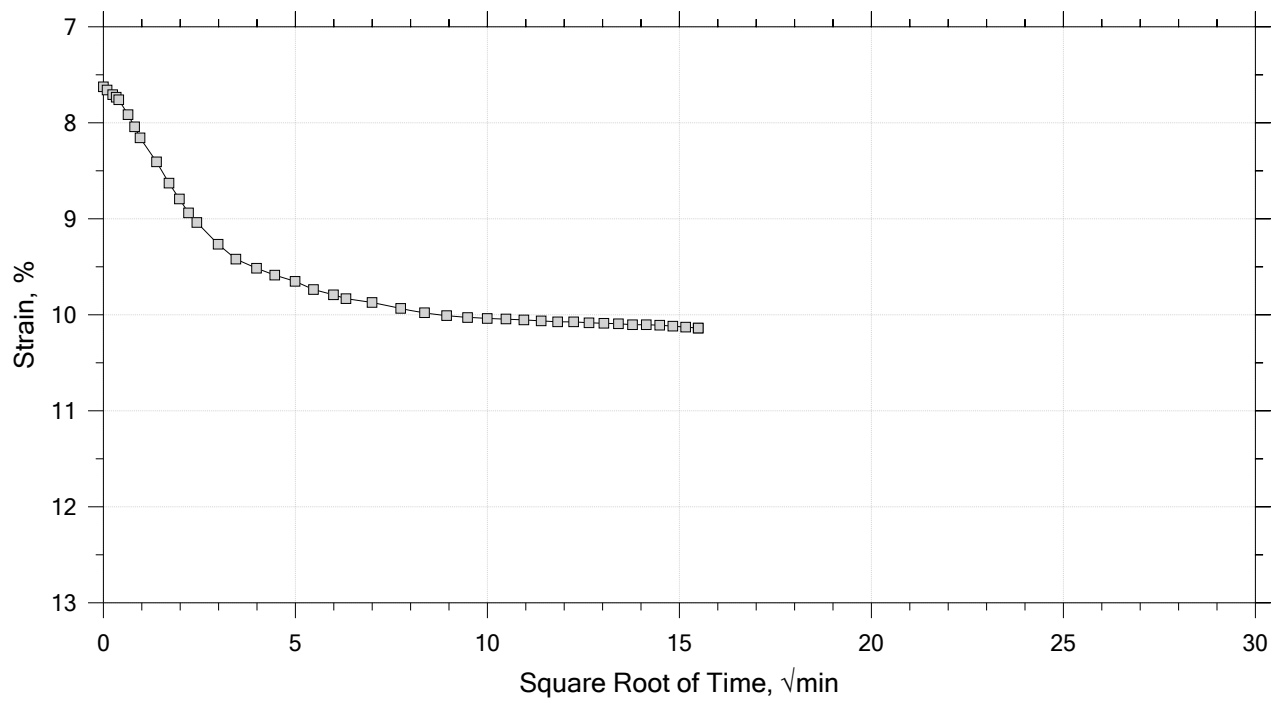
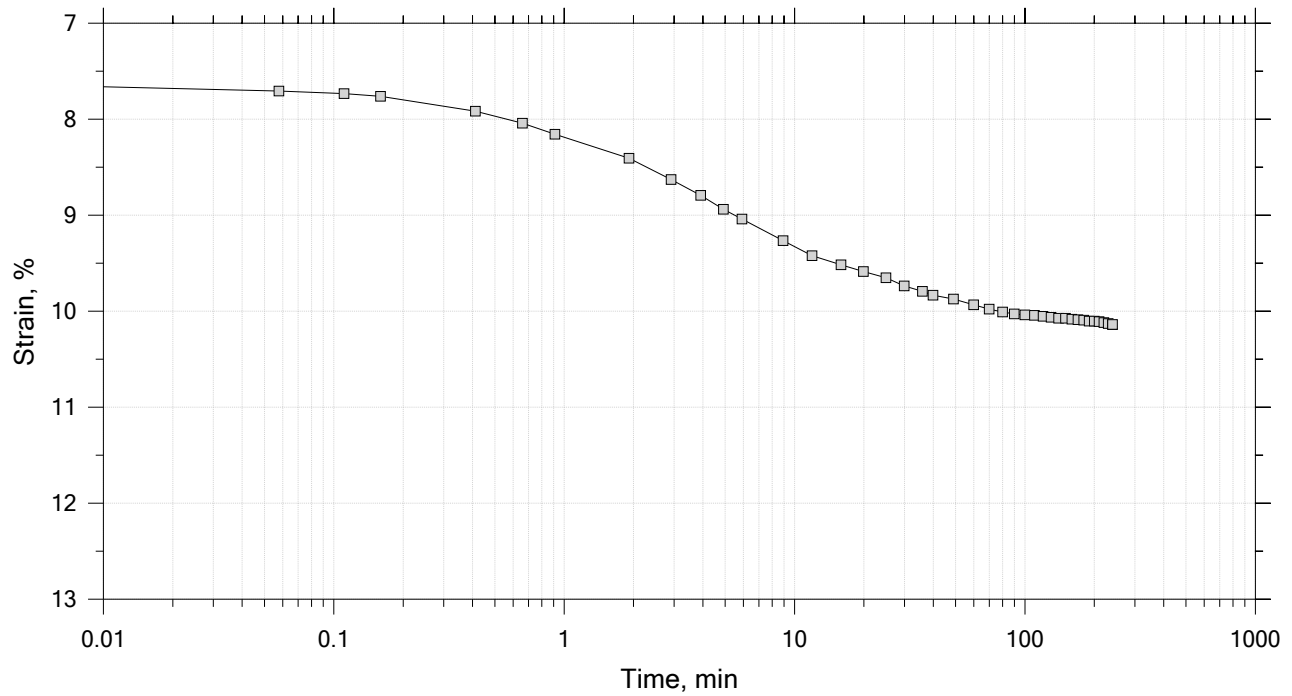
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



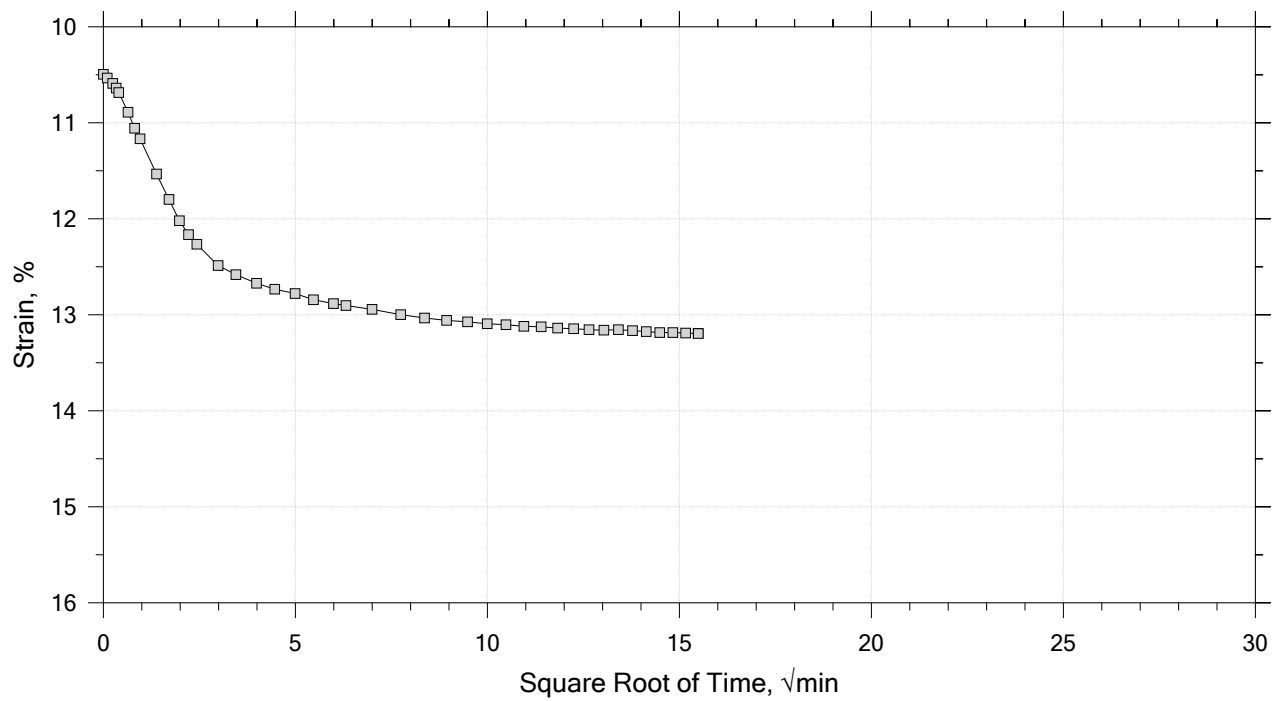
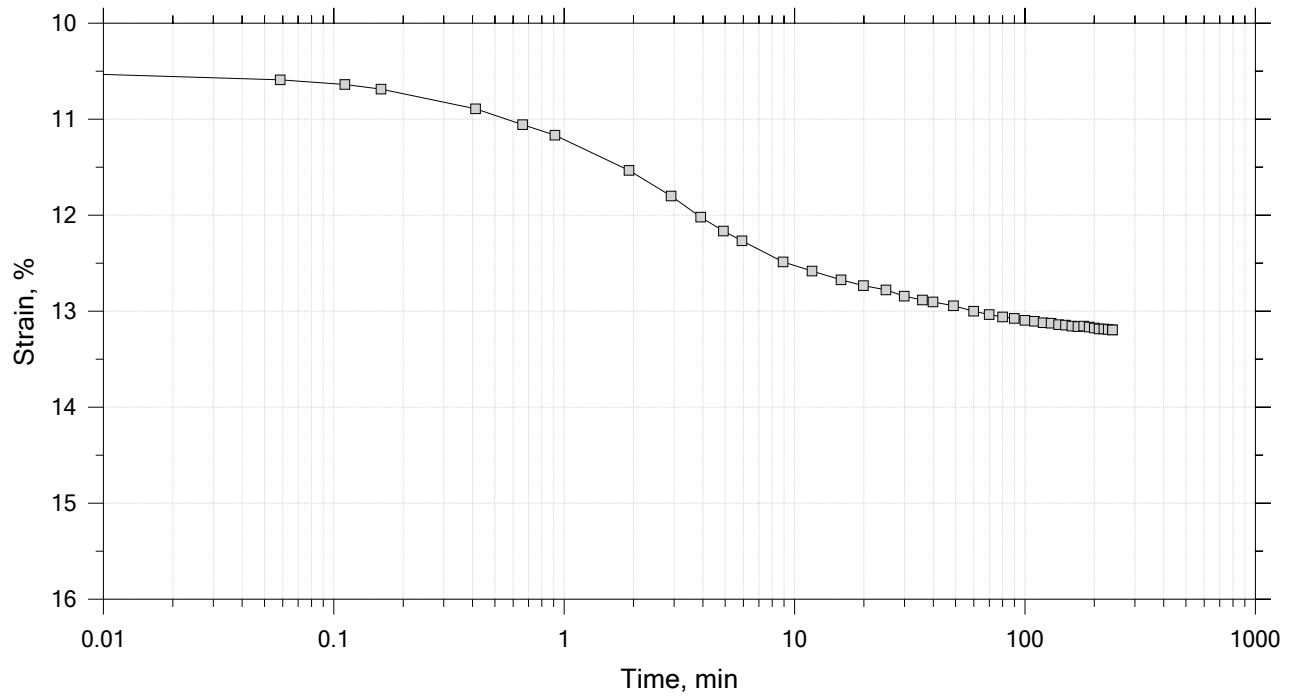
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



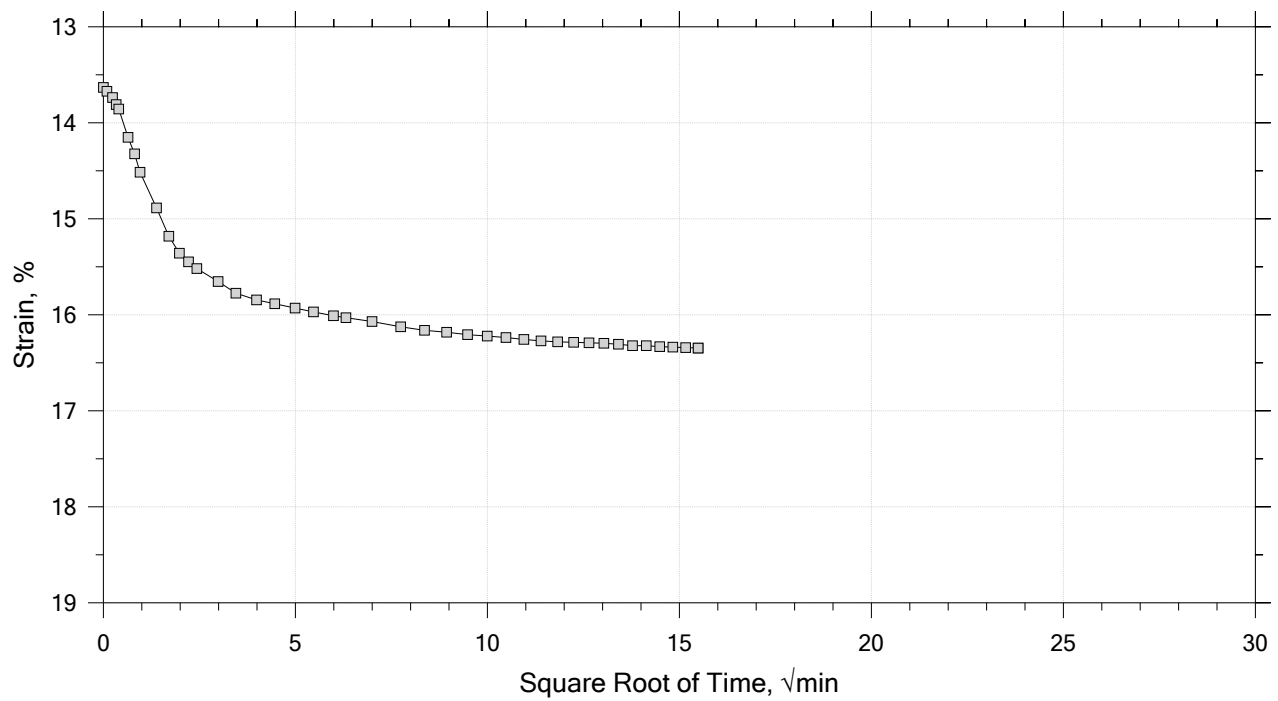
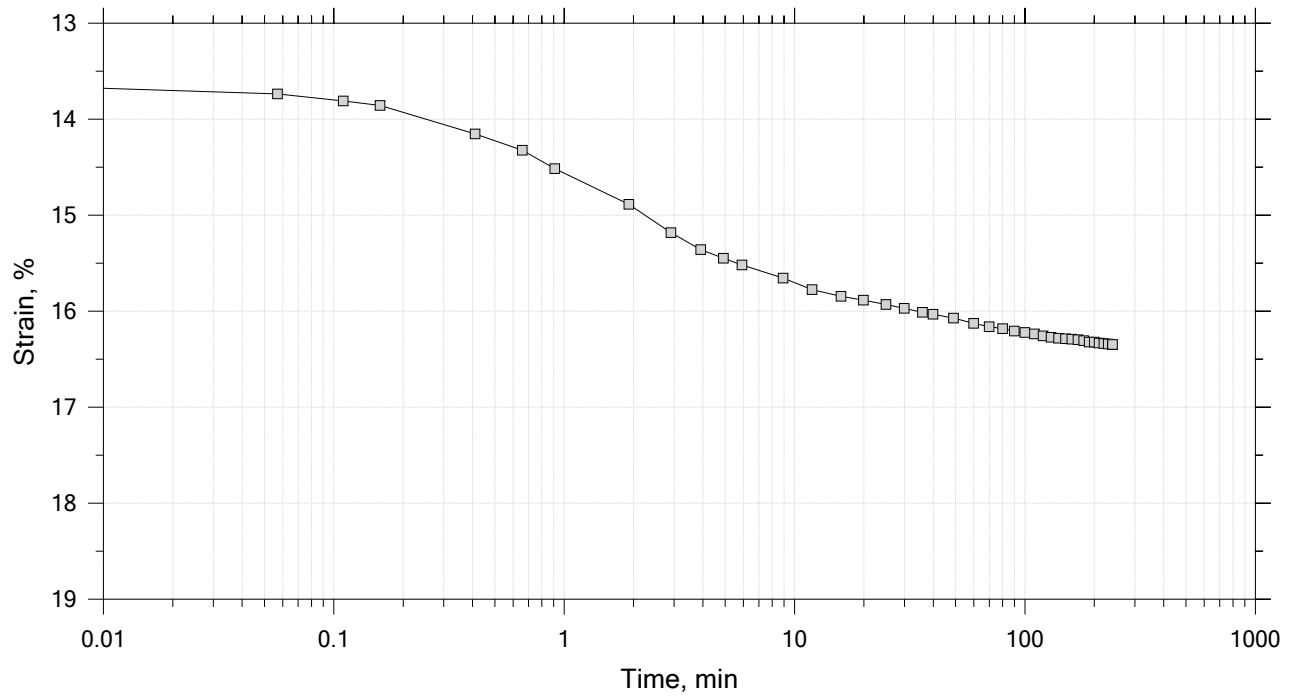
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



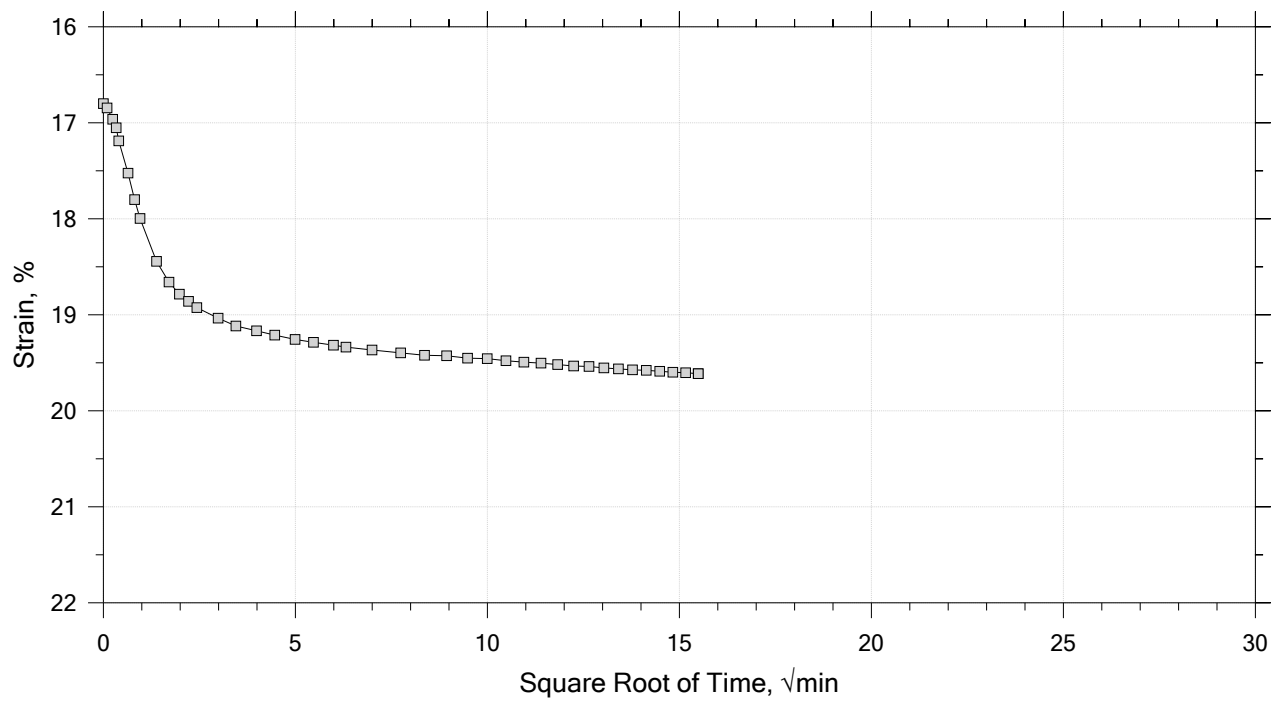
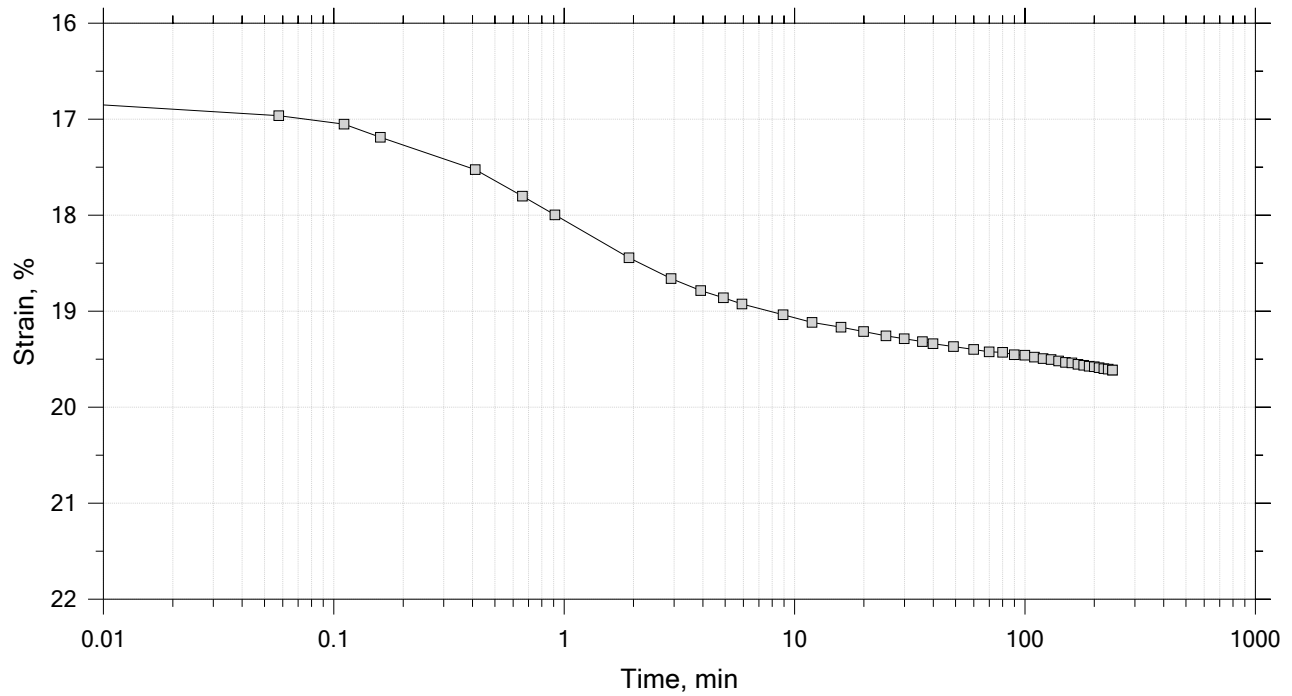
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



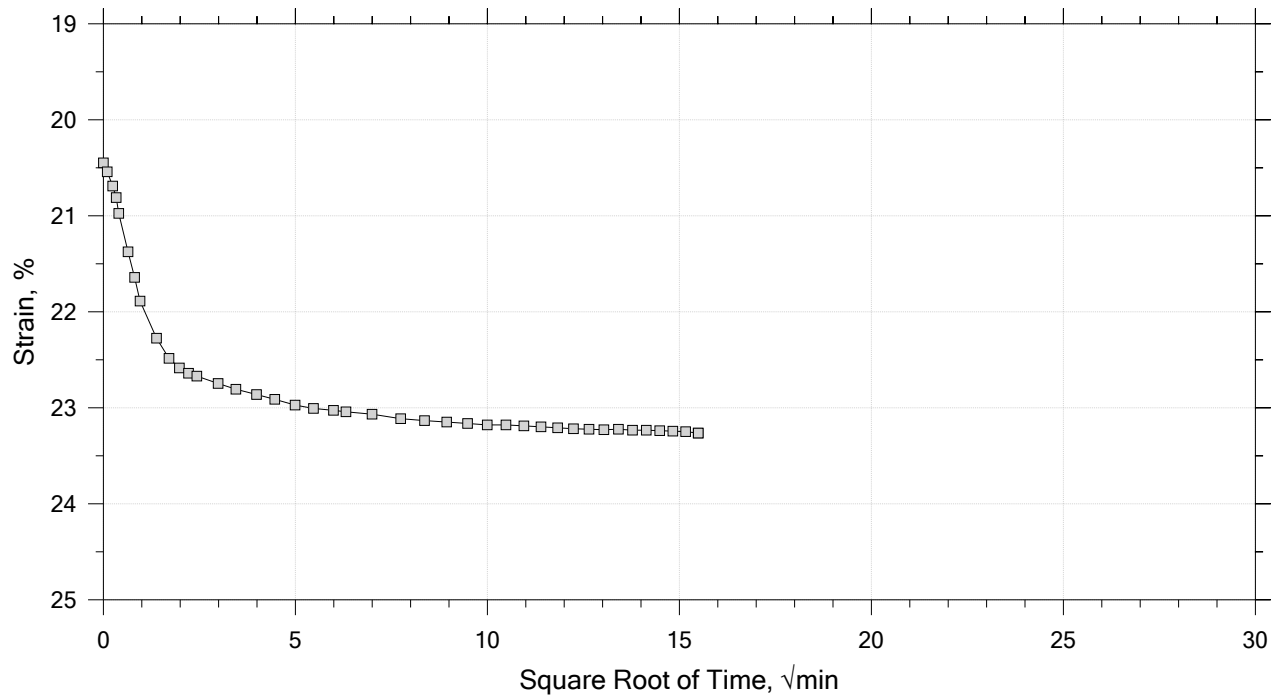
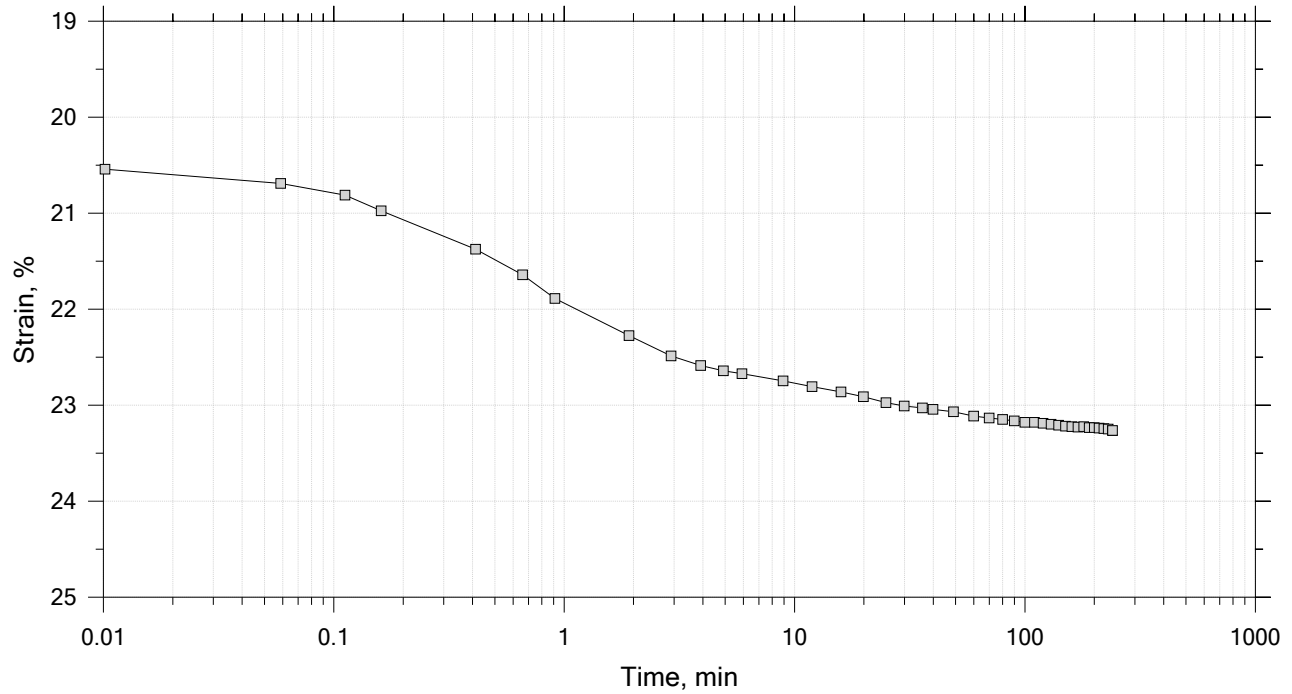
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



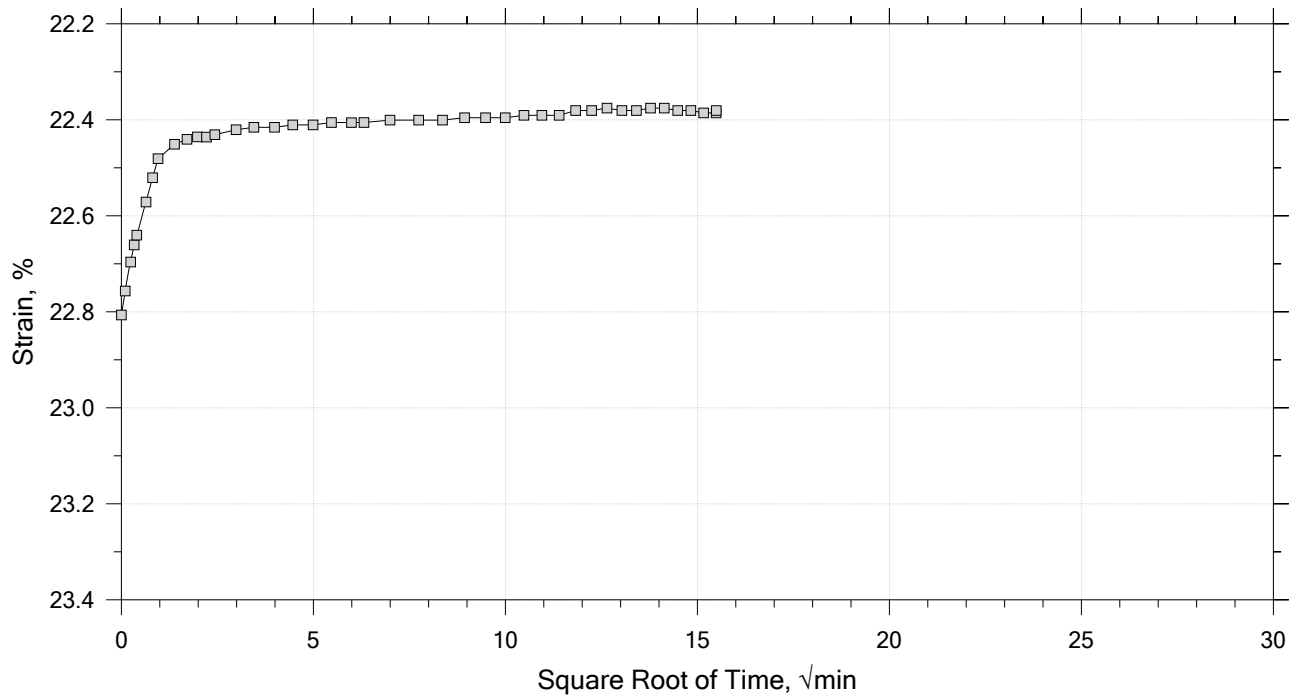
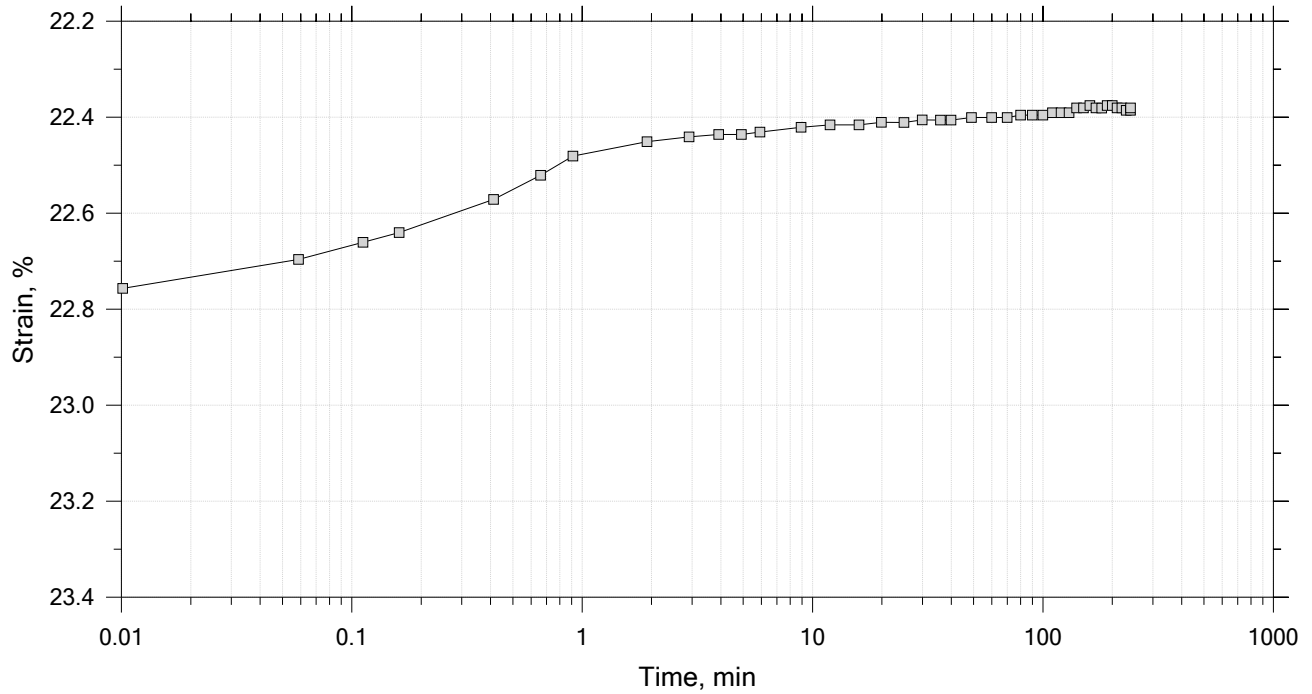
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



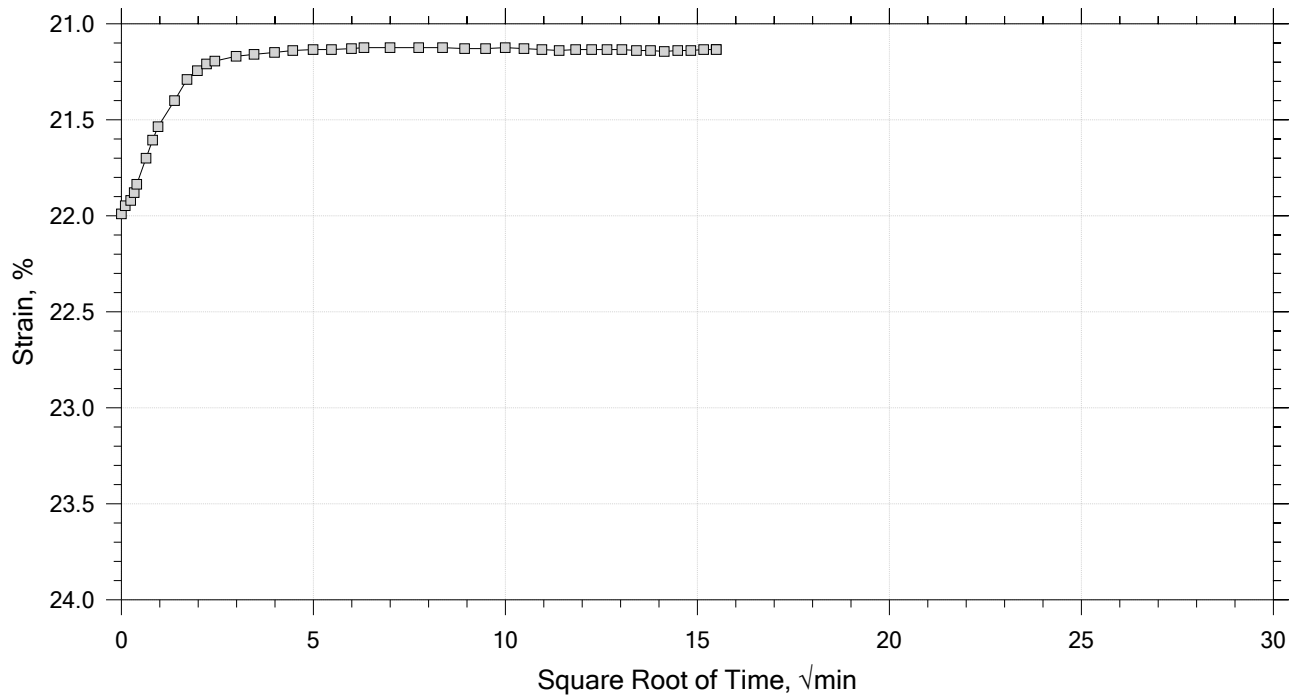
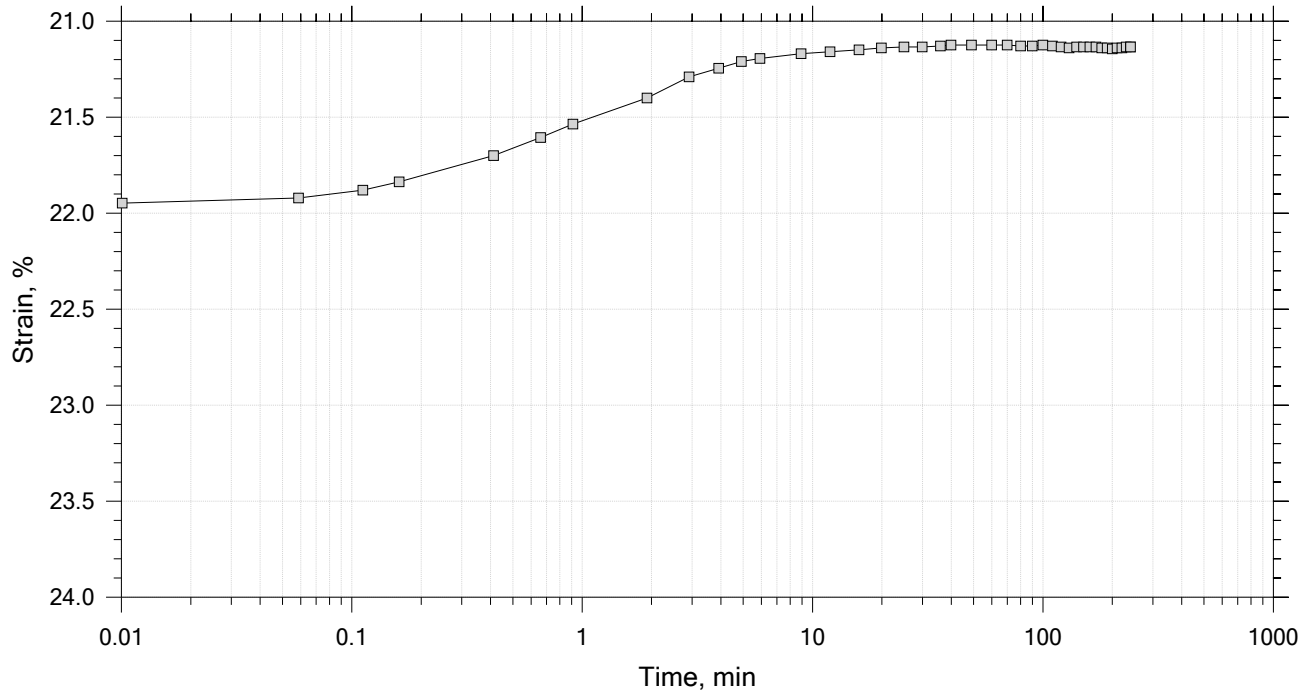
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



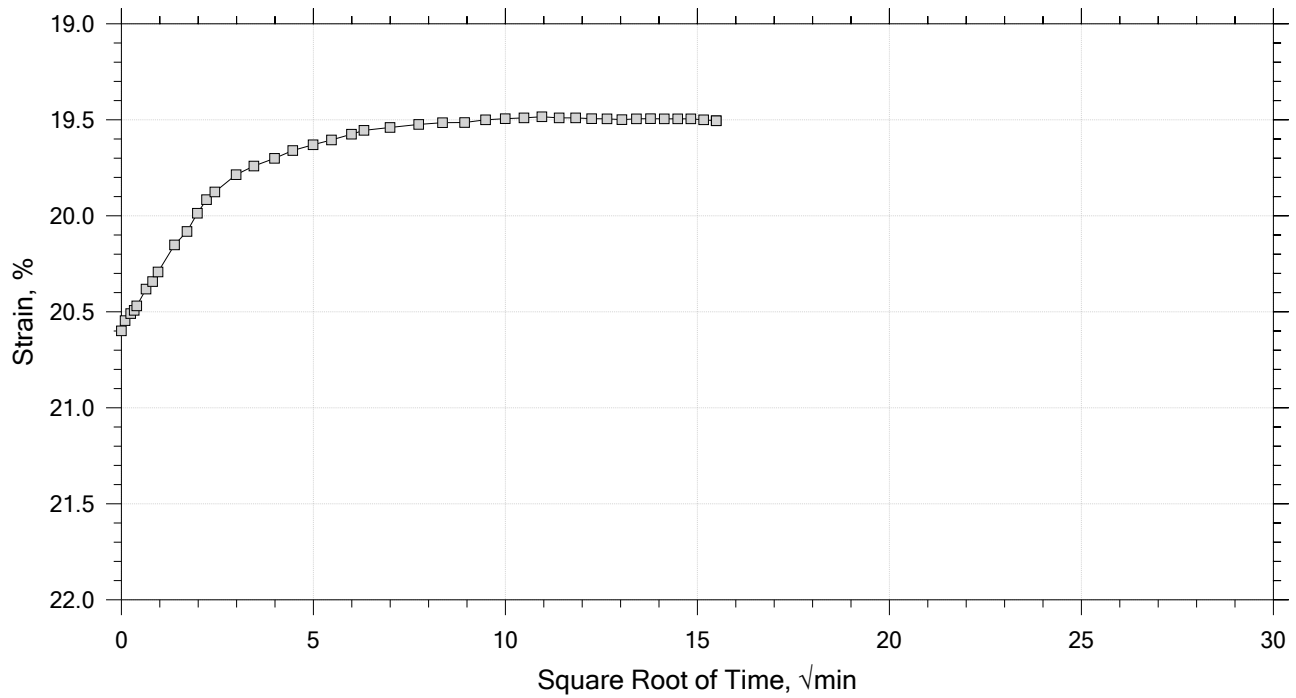
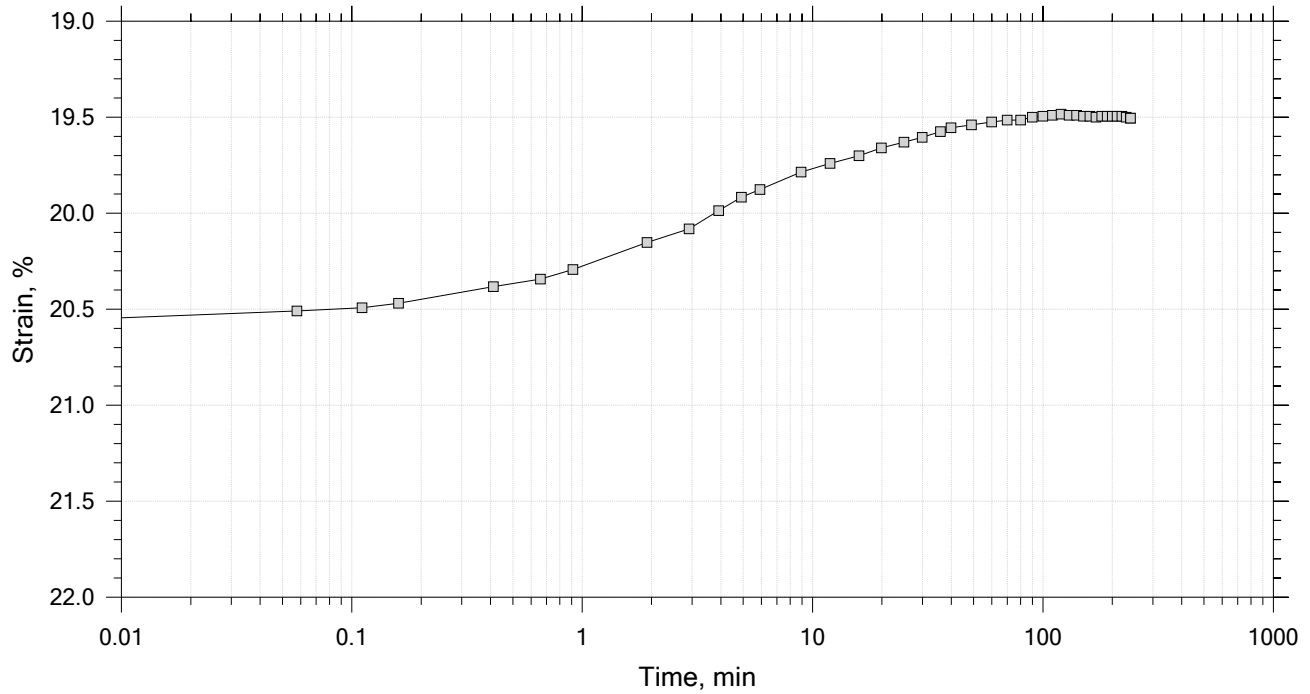
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



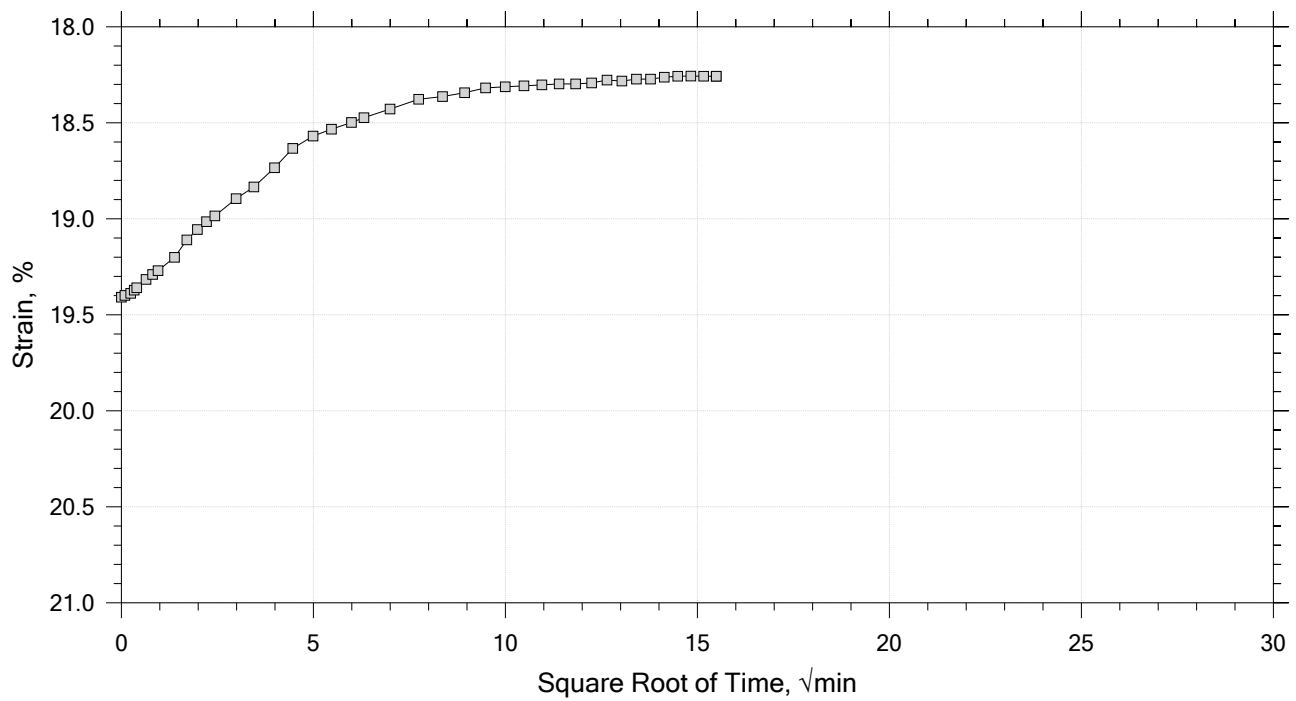
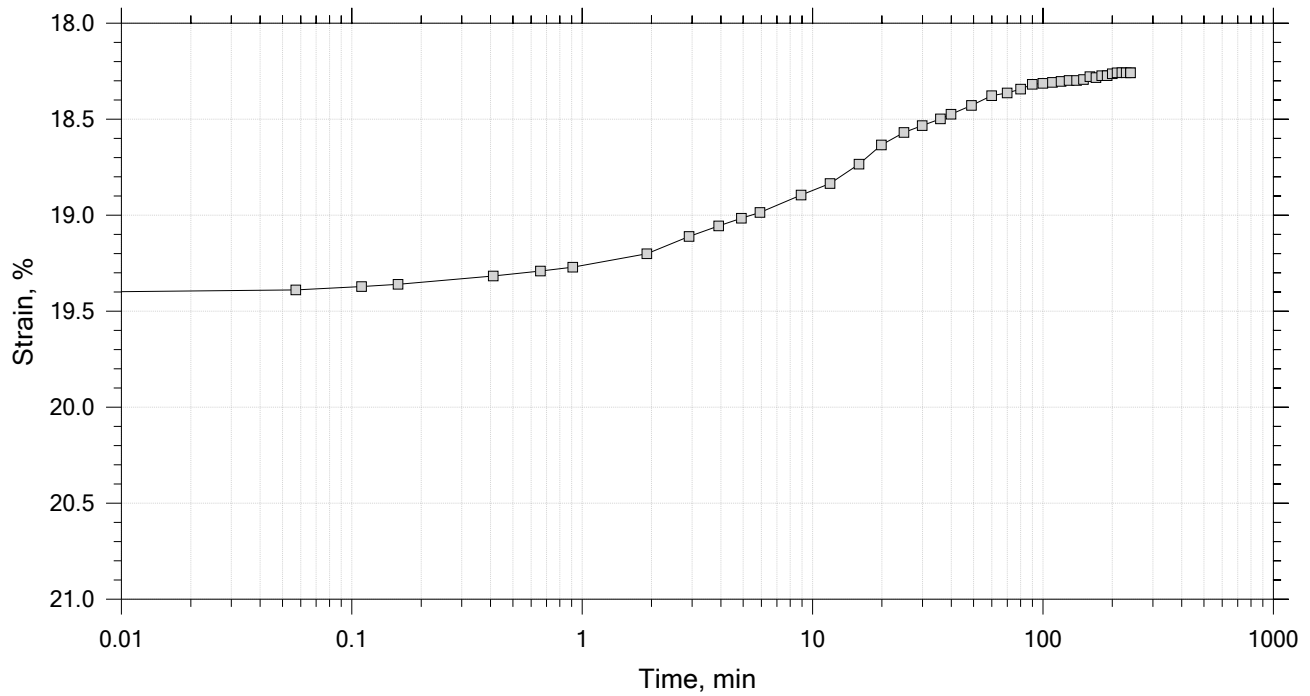
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	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



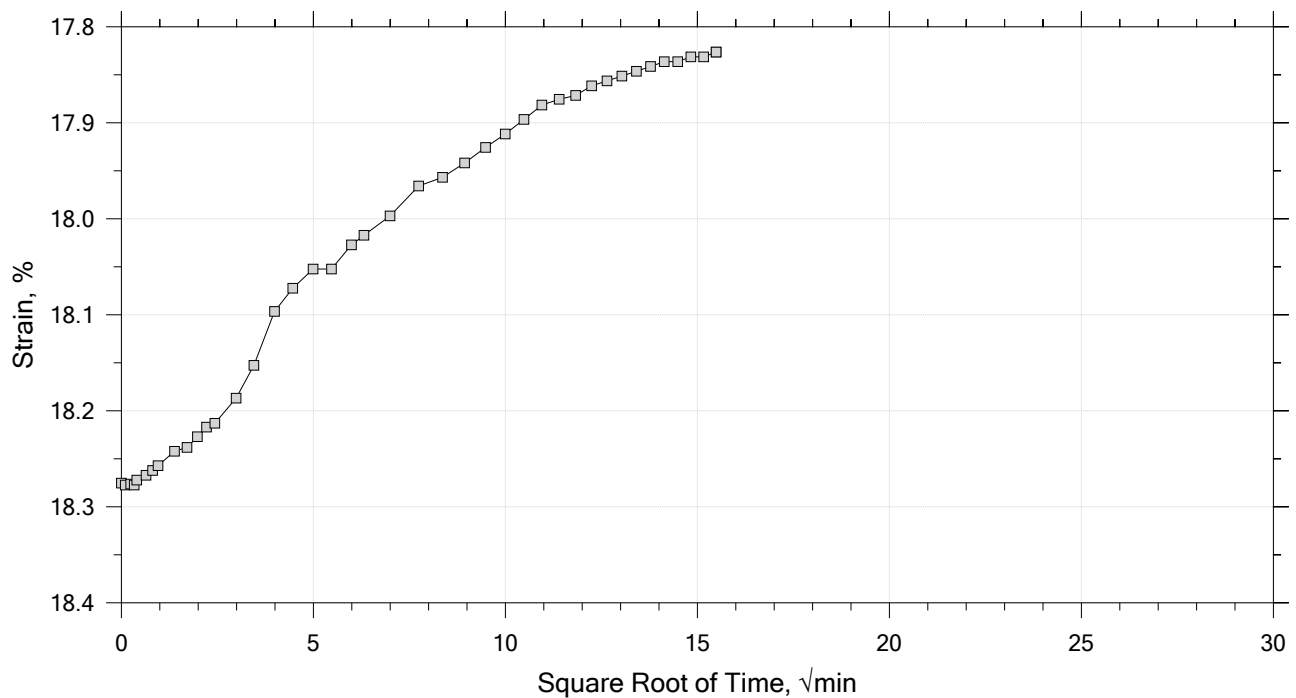
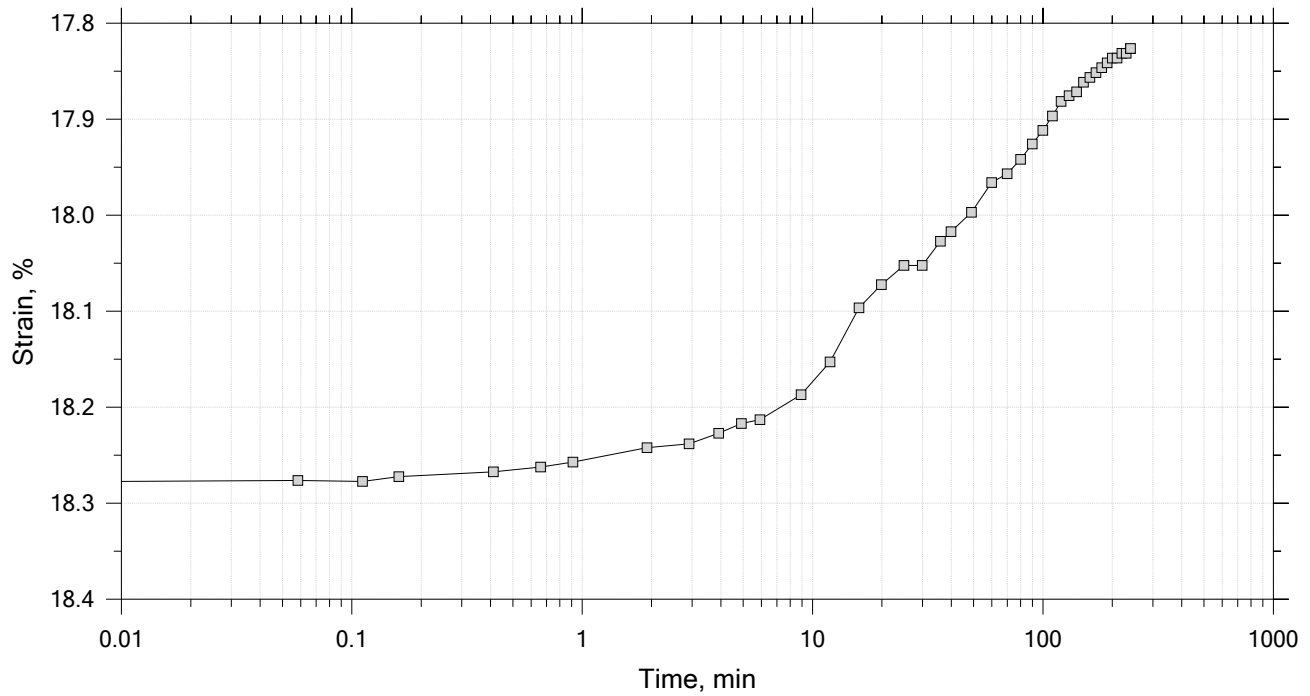
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 0.909	Plastic Limit: 20
Final Height: 0.82 in	Final Void Ratio: 0.569	Plasticity Index: 15

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A-2387	RING		D-693
Mass Container, gm	8.2	109.53	109.53	8.23
Mass Container + Wet Soil, gm	139.33	263.67	250.81	149.22
Mass Container + Dry Soil, gm	106.05	226.85	226.85	125.31
Mass Dry Soil, gm	97.85	117.32	117.32	117.08
Water Content, %	34.01	31.38	20.42	20.42
Void Ratio	---	0.91	0.57	---
Degree of Saturation, %	---	96.14	100.00	---
Dry Unit Weight, pcf	---	91.051	110.8	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

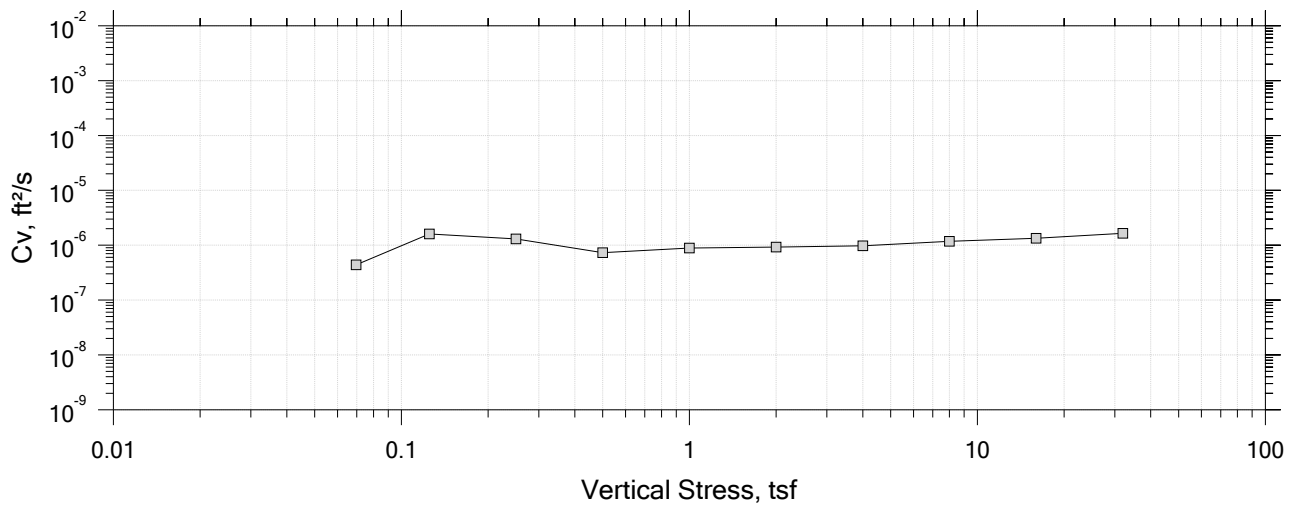
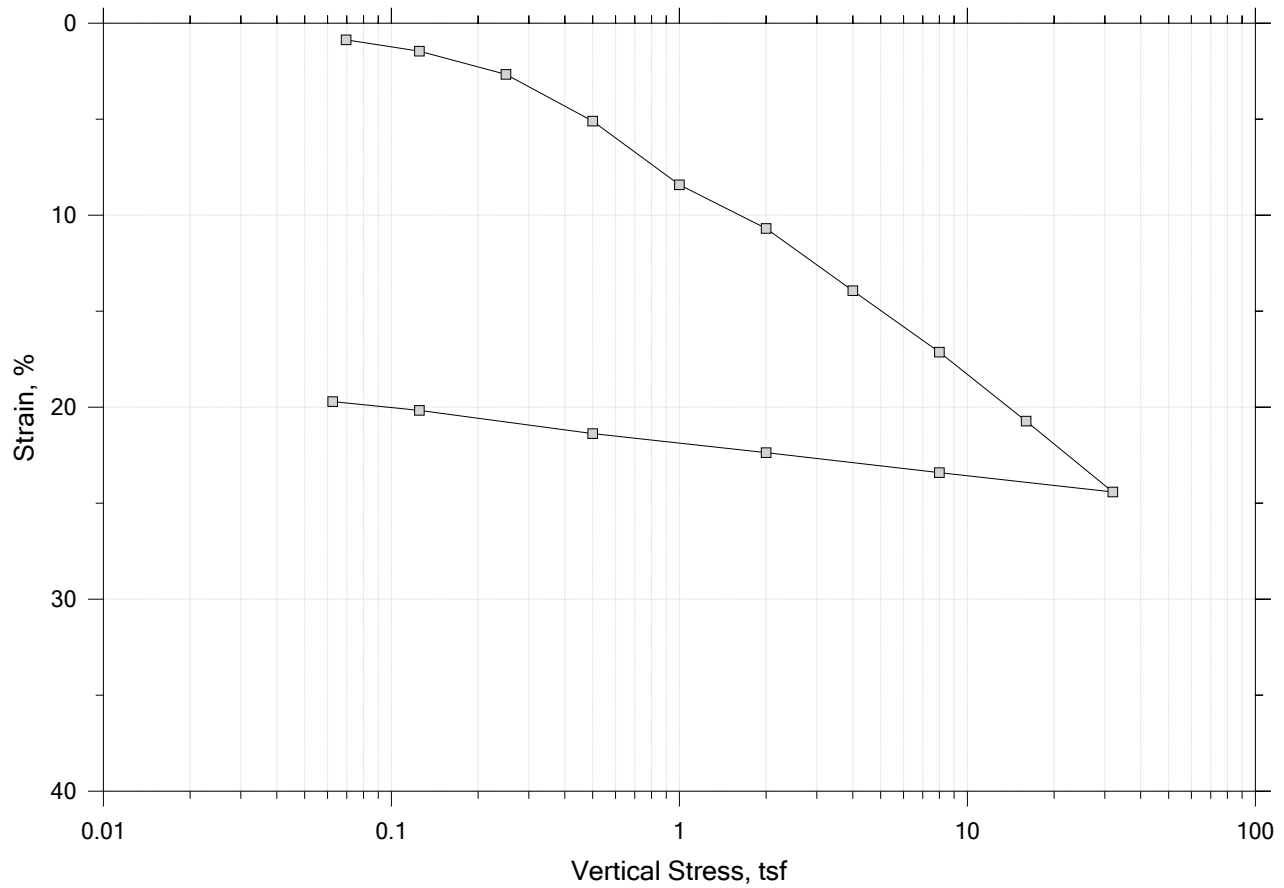
Square Root of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BEB-104	Tested By: md	Checked By: mcm
	Sample No.: 3U	Test Date: 07/17/19	Depth: 15-17 ft
	Test No.: IP-15	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0662 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

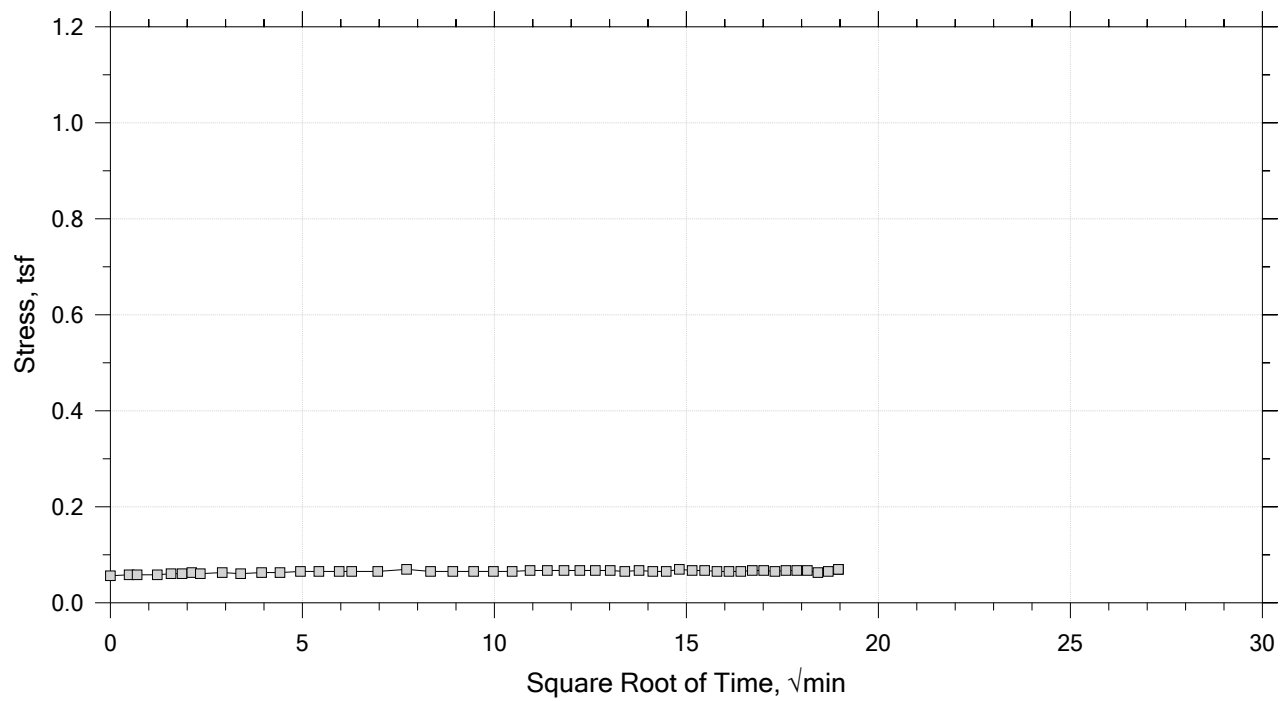
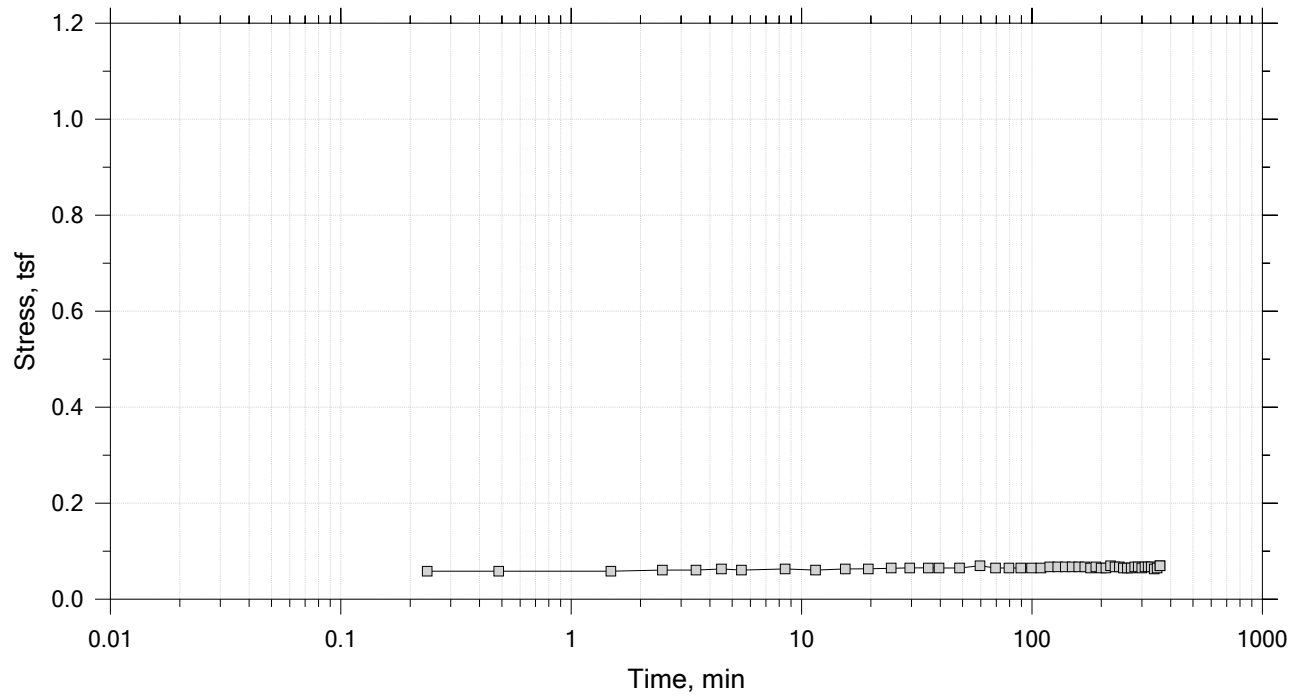
Summary Report




	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0696 tsf



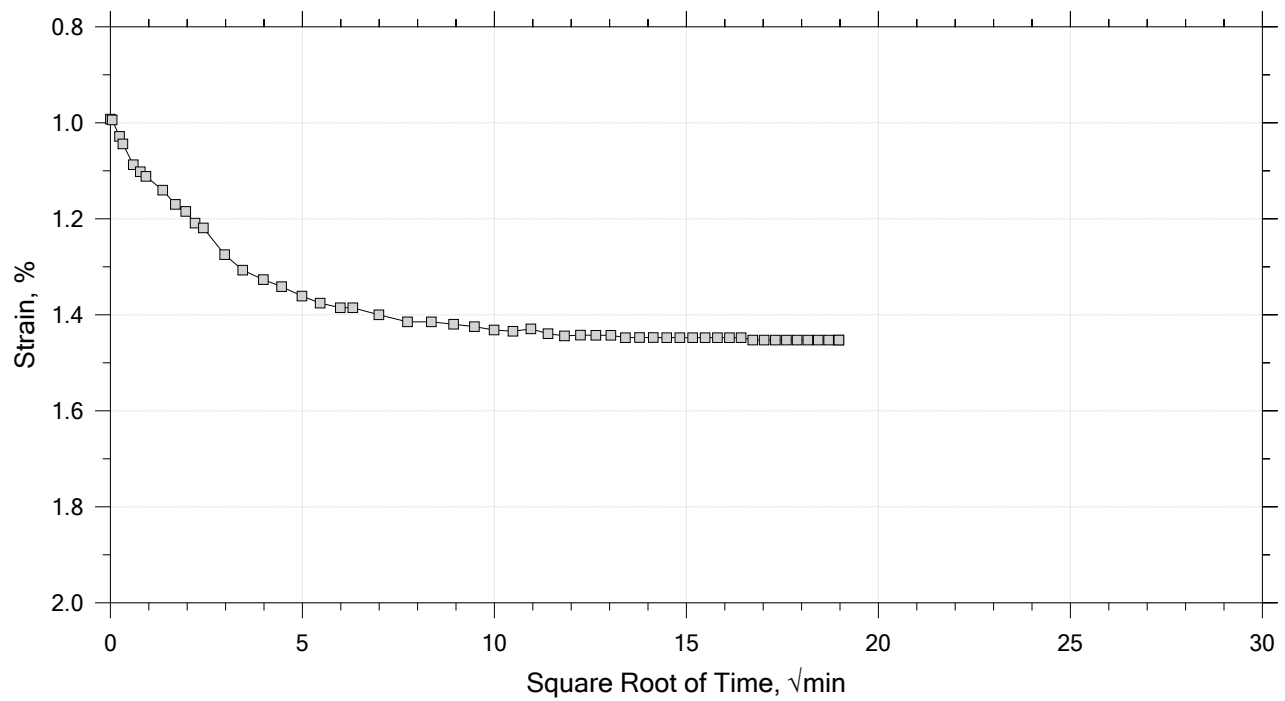
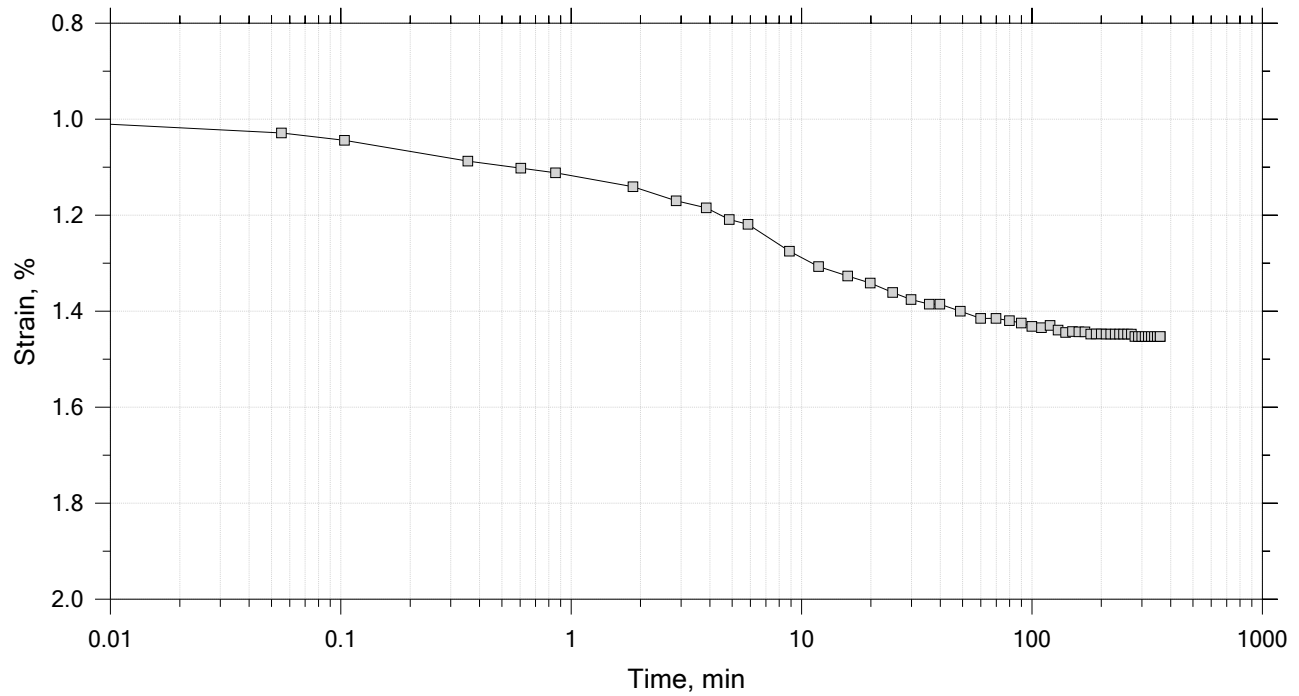
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	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



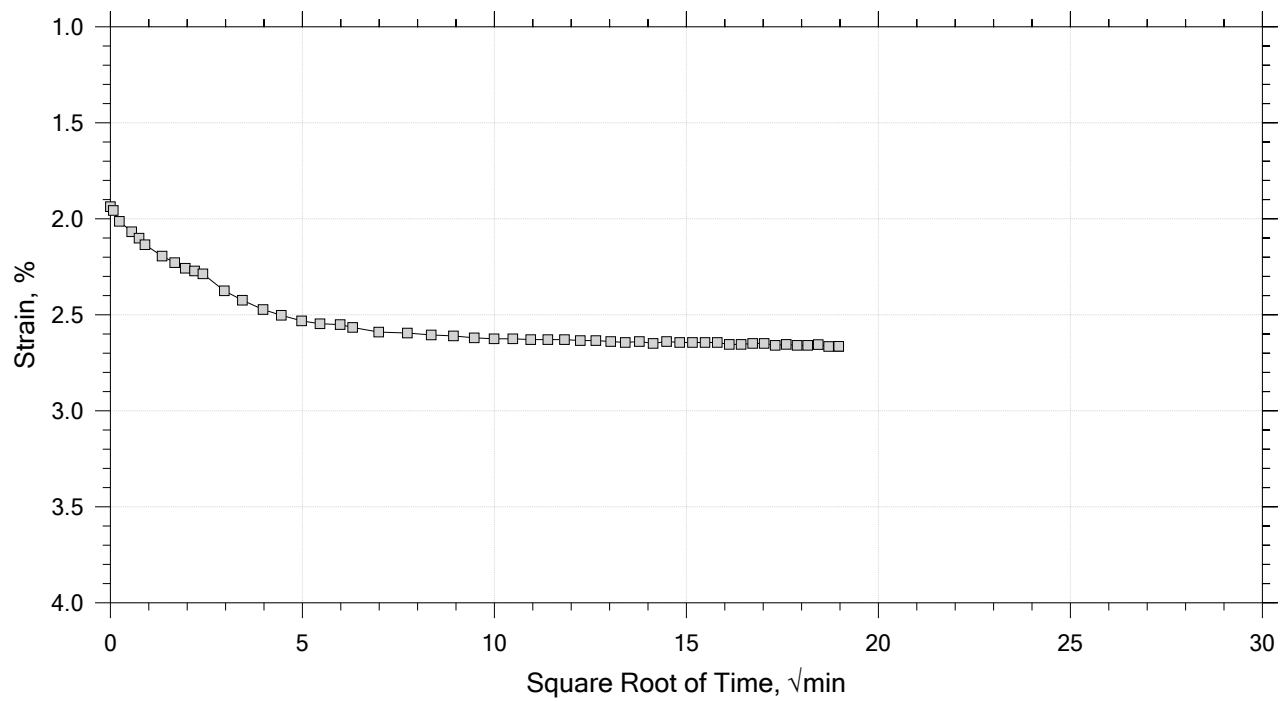
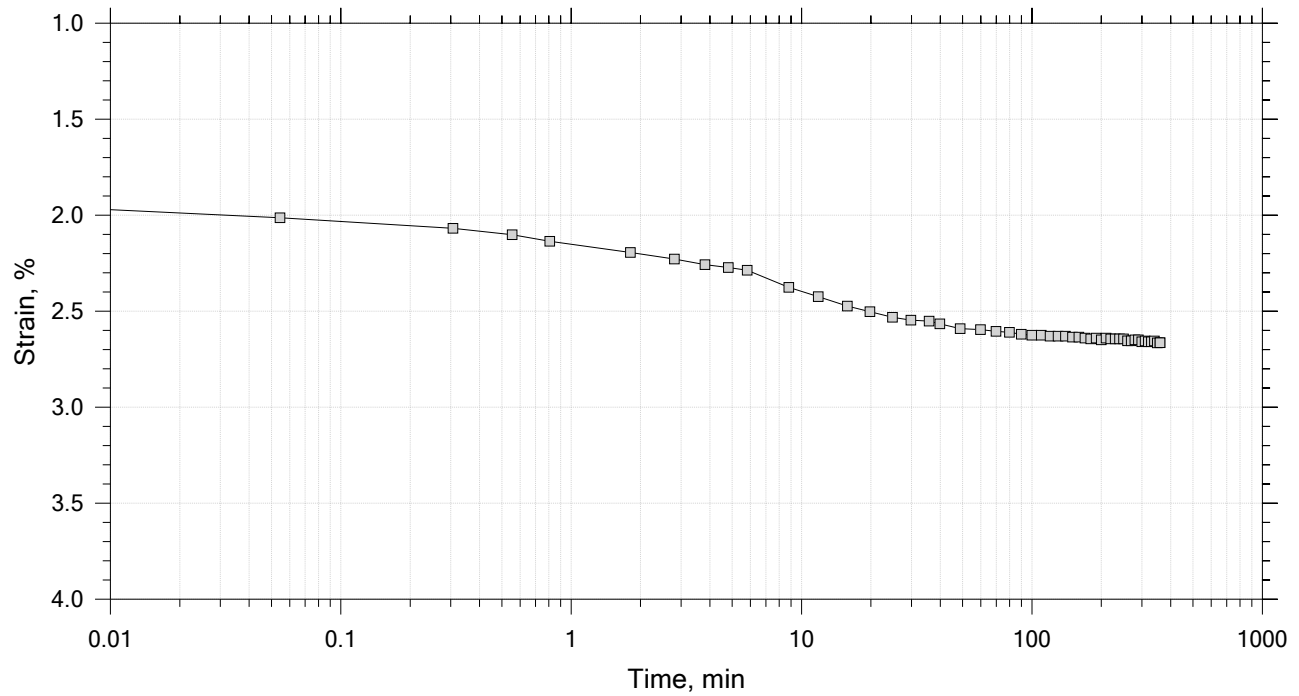
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	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



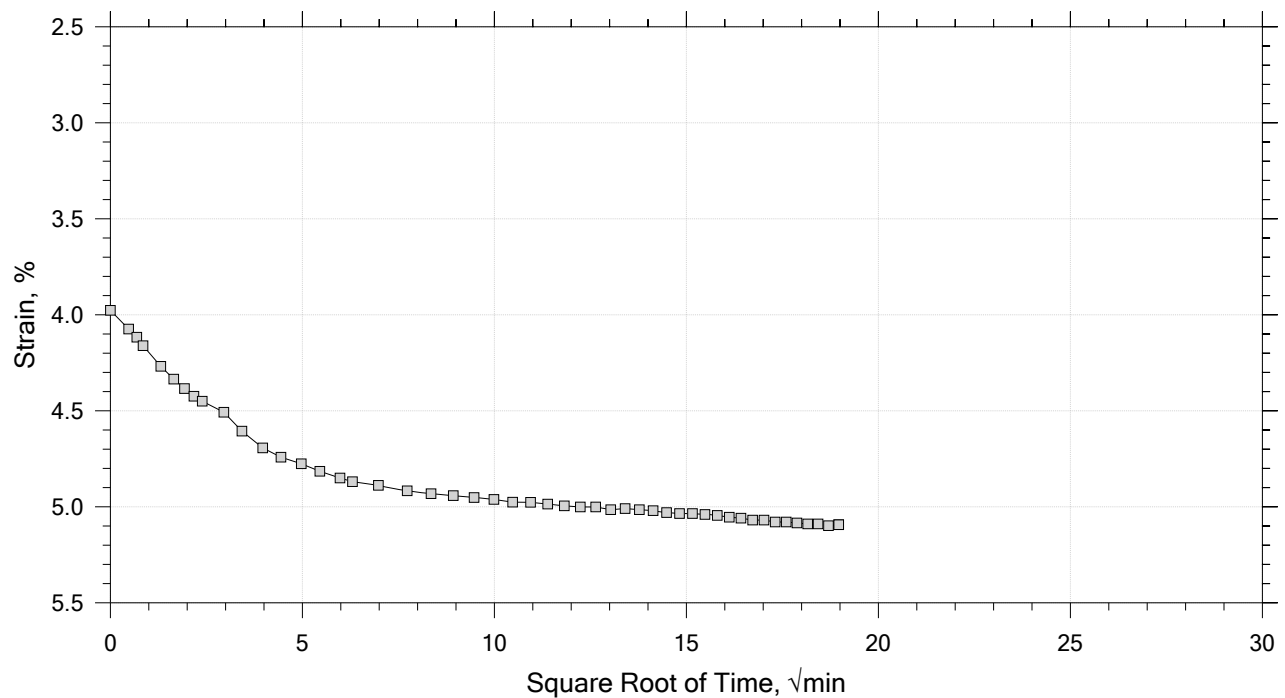
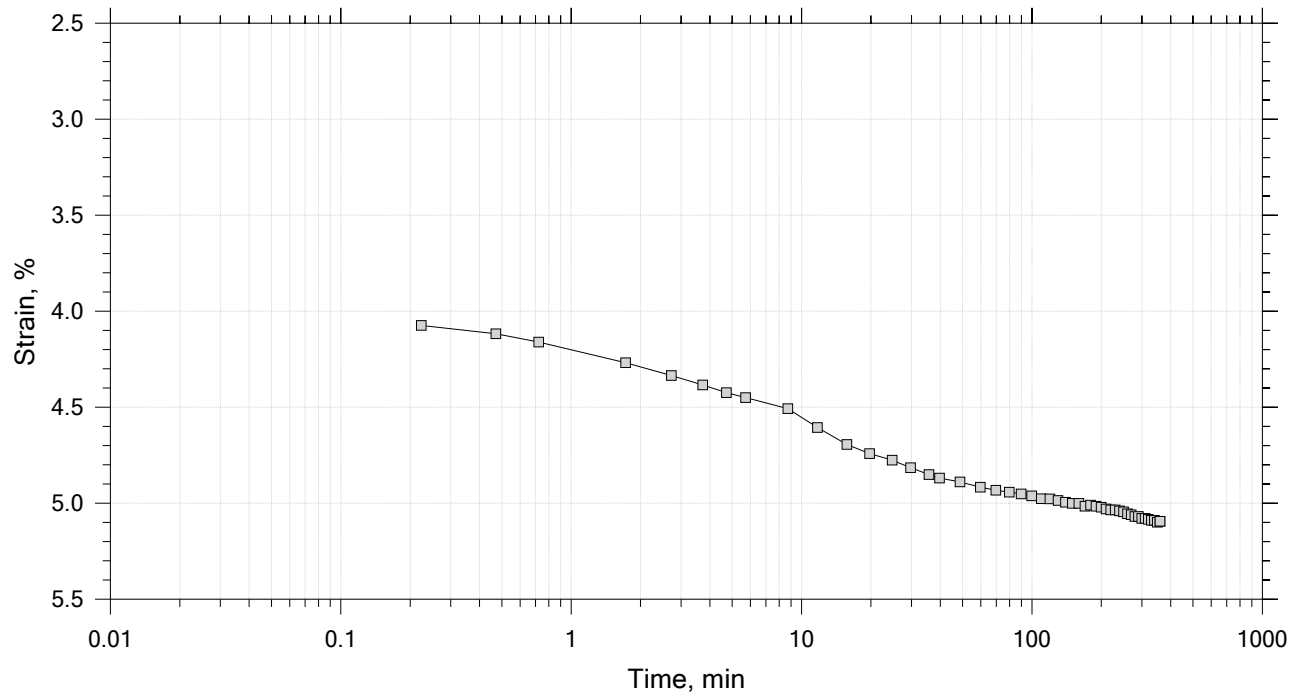
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



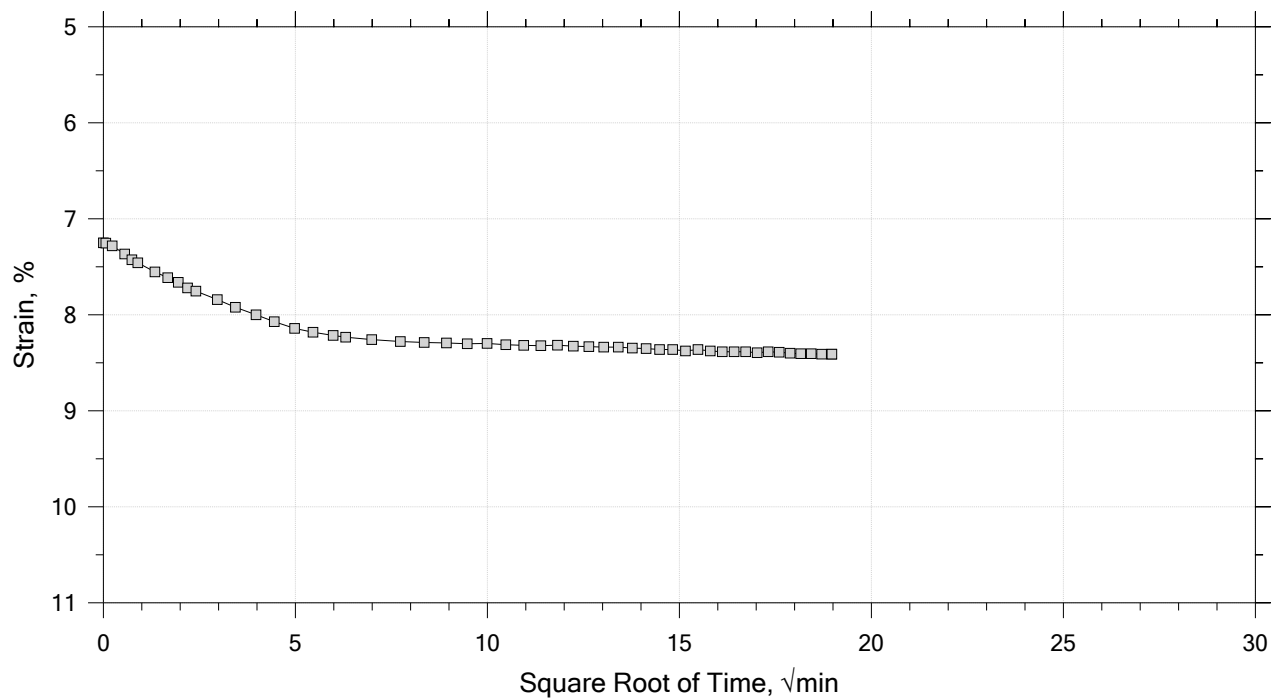
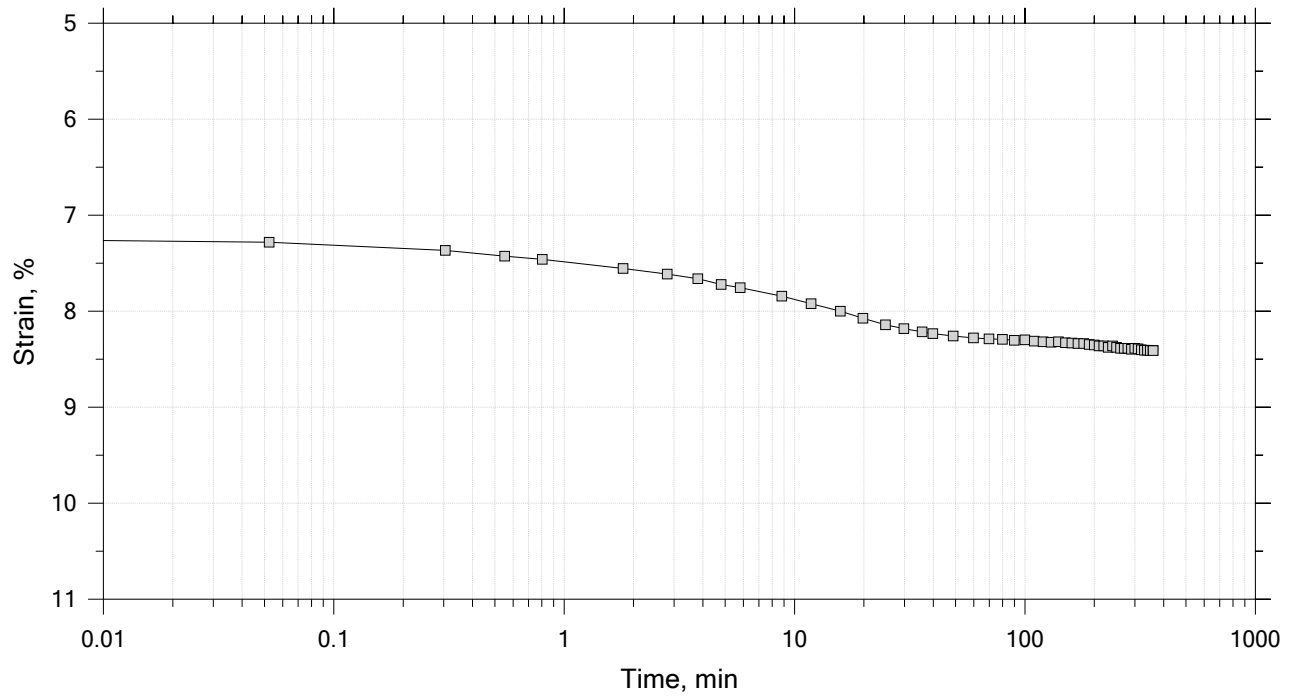
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



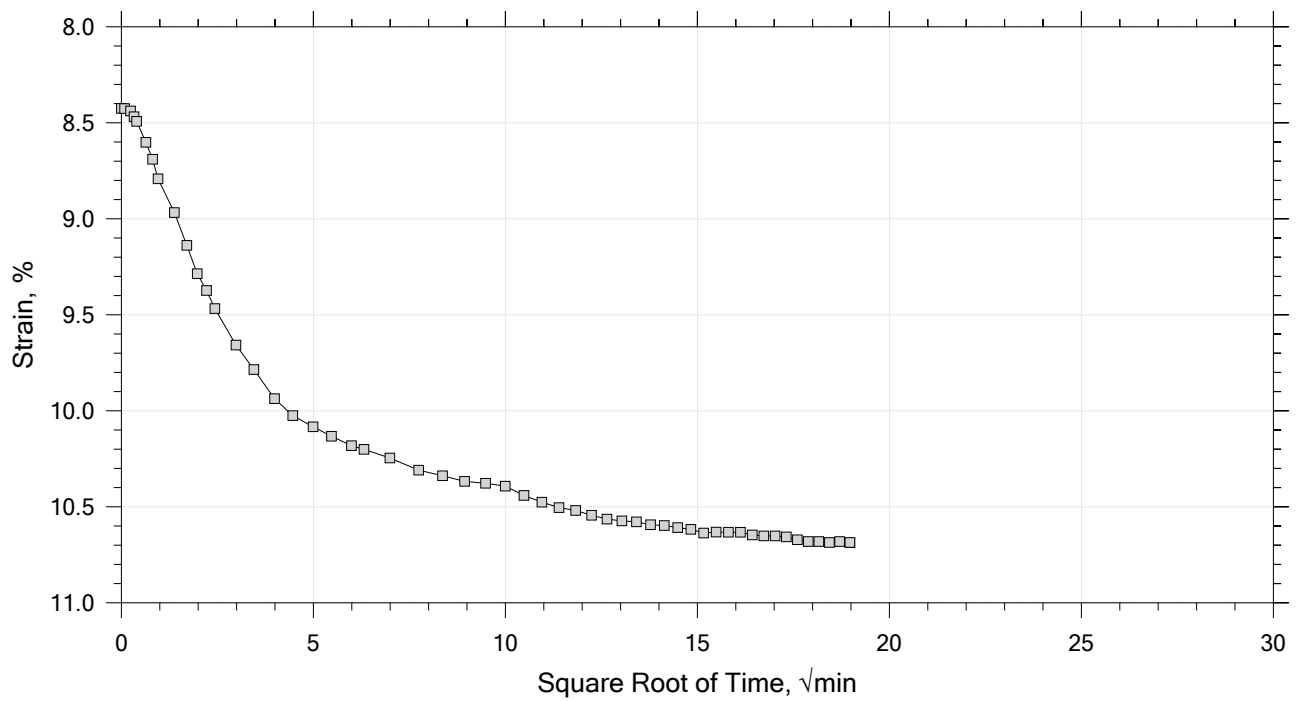
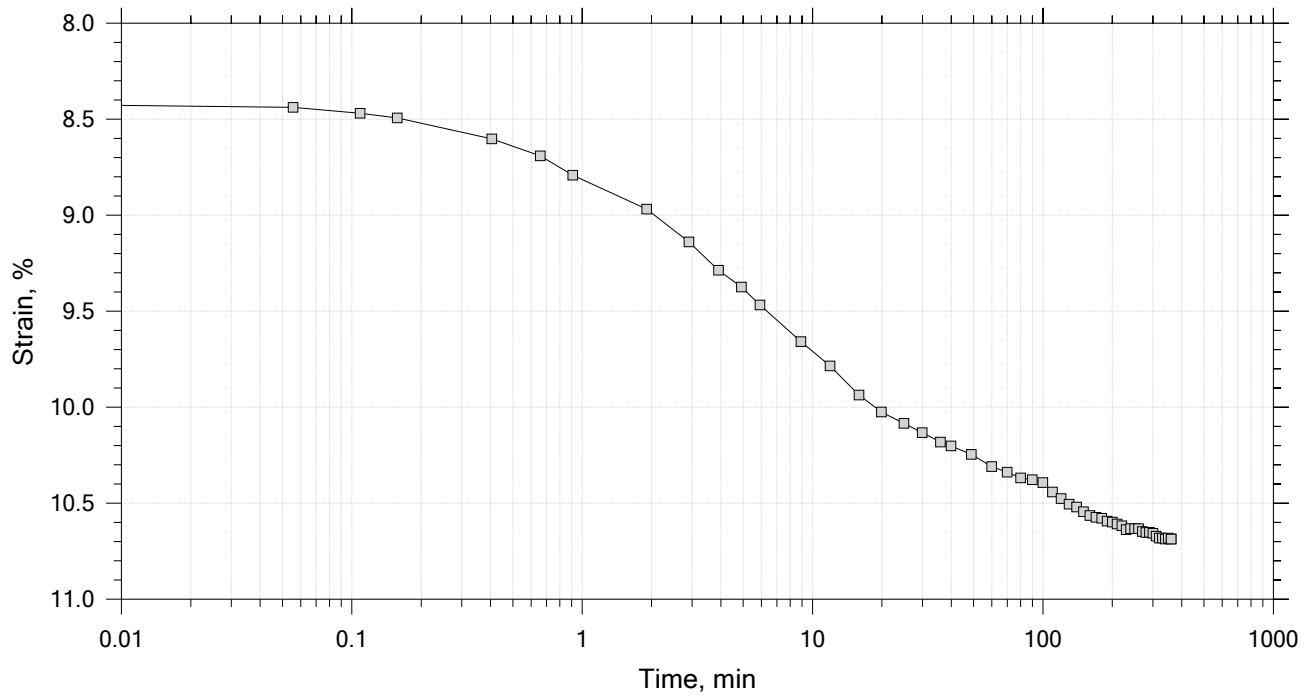
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



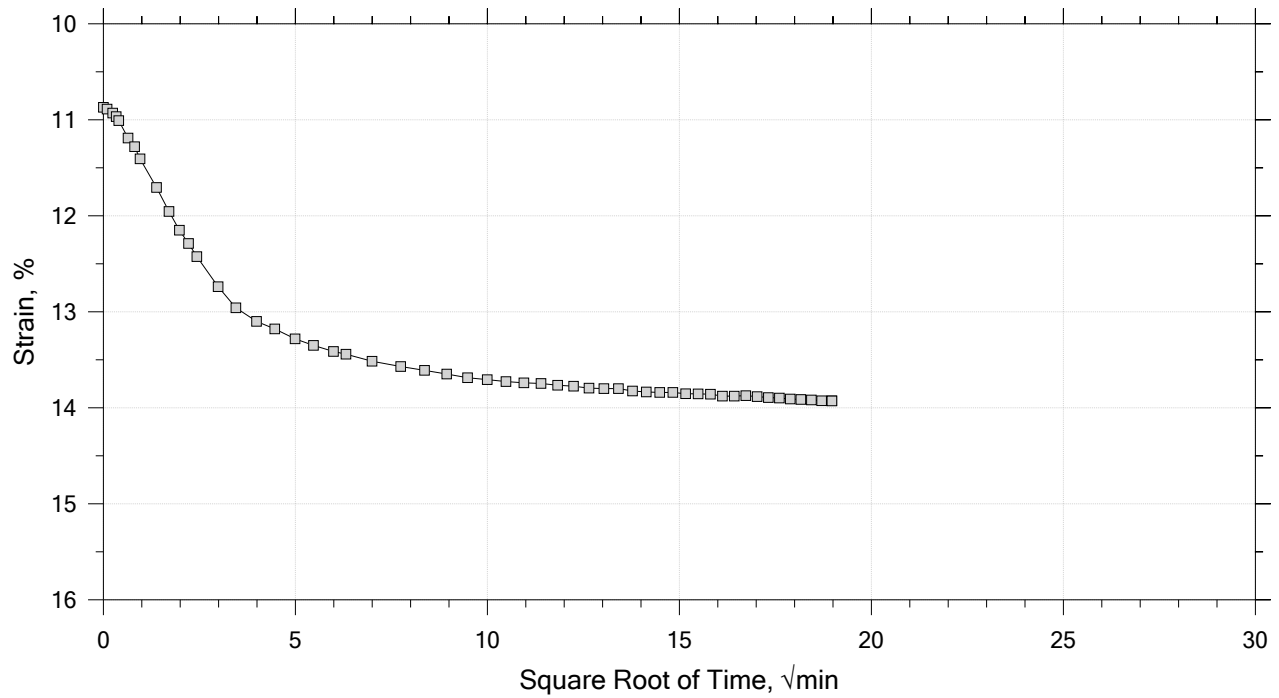
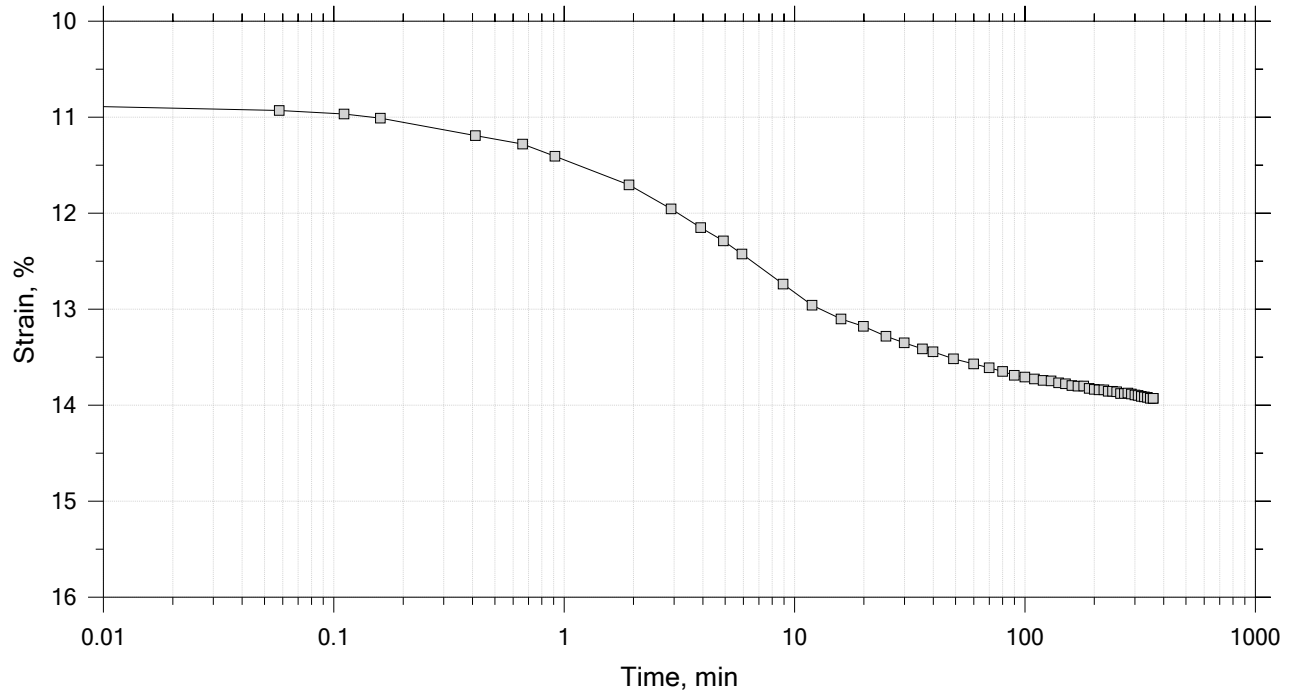
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



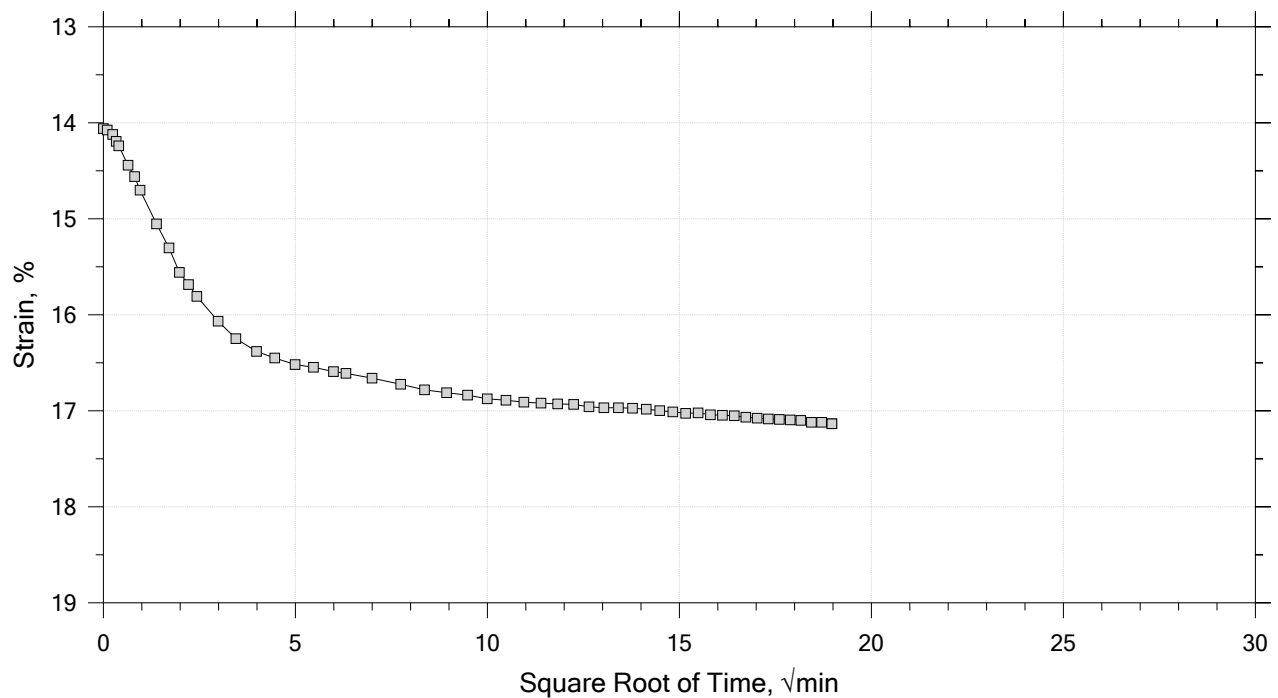
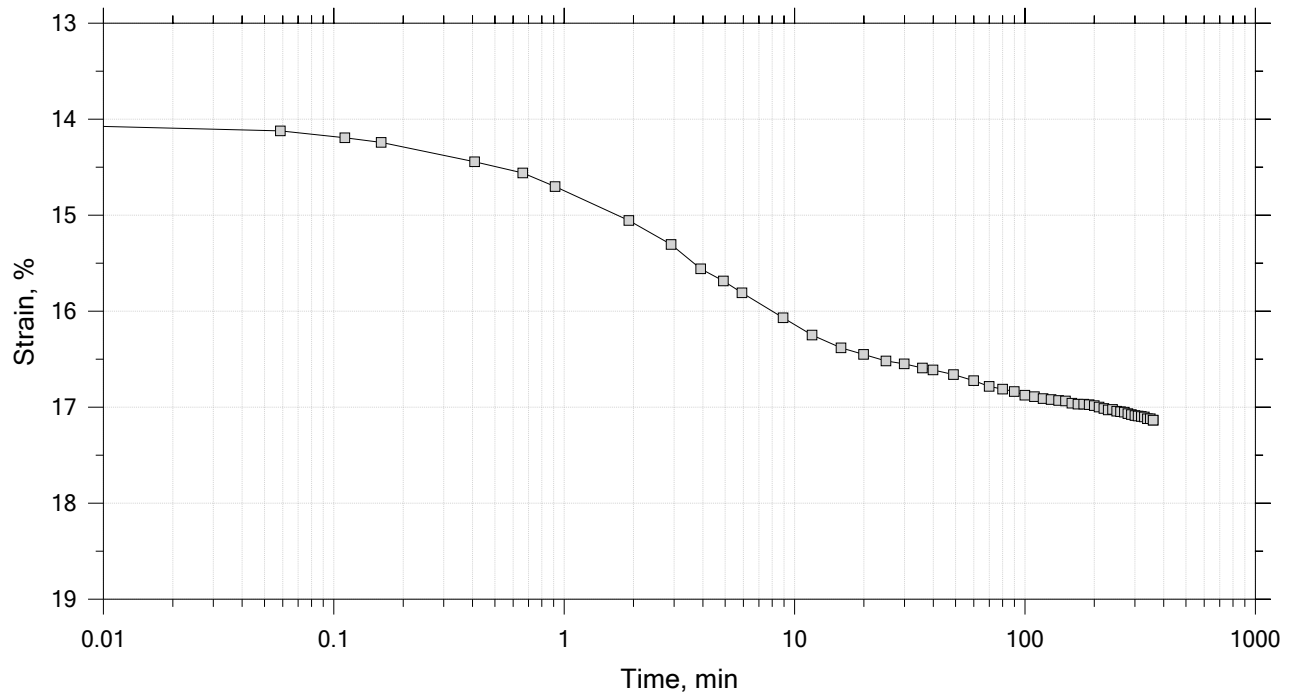
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	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



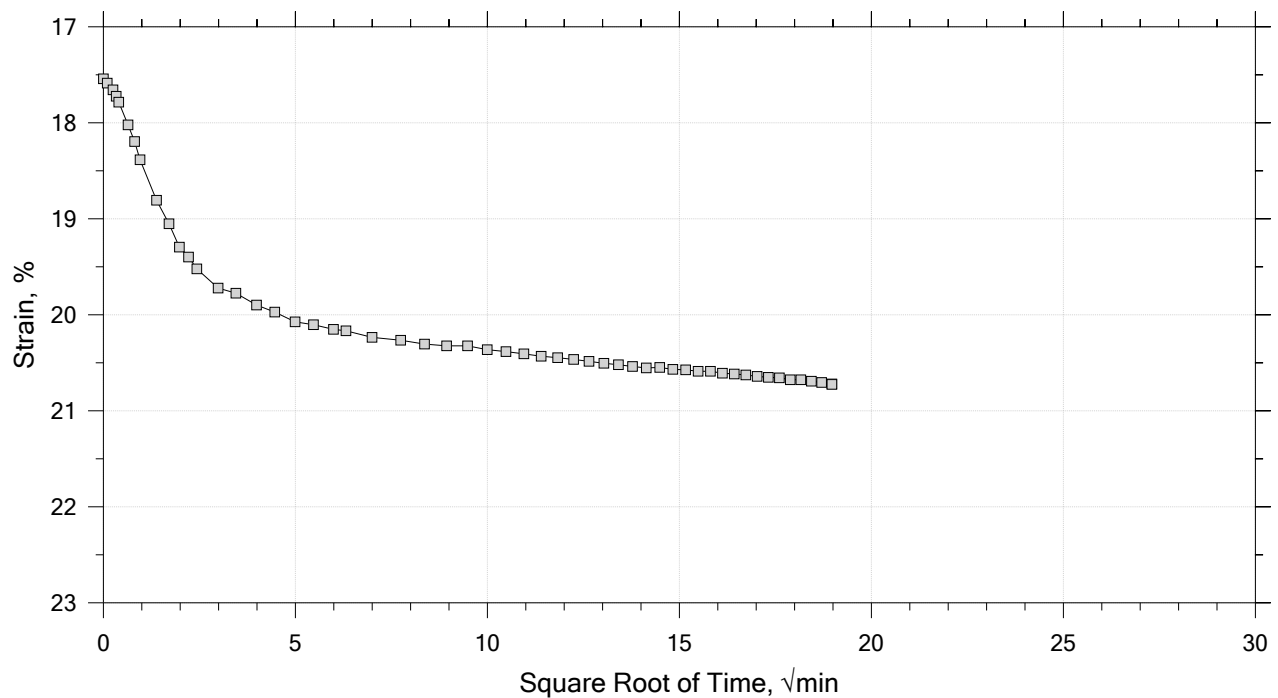
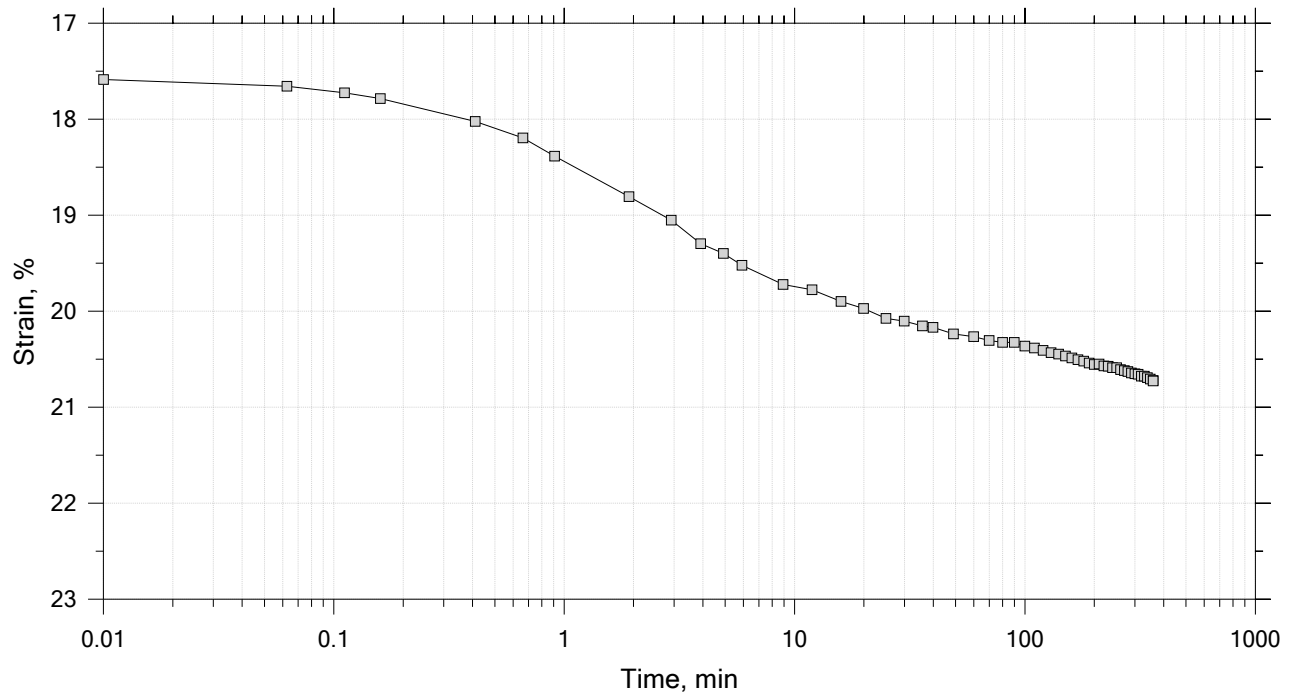
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



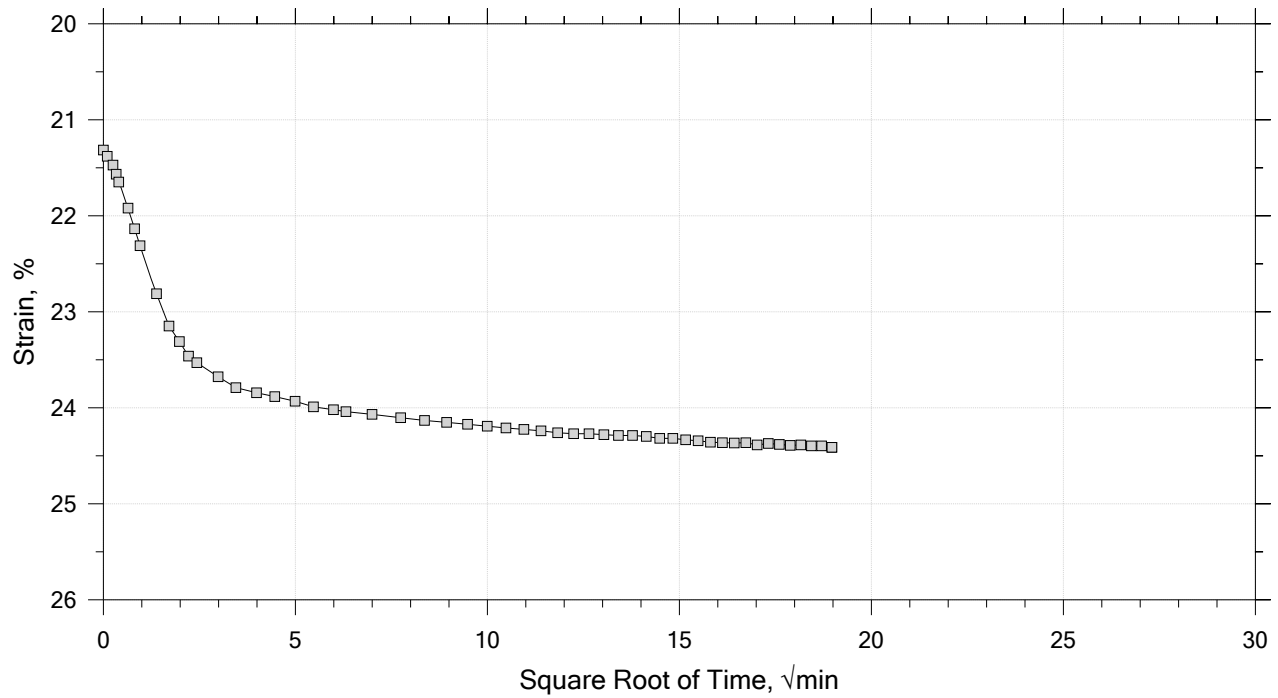
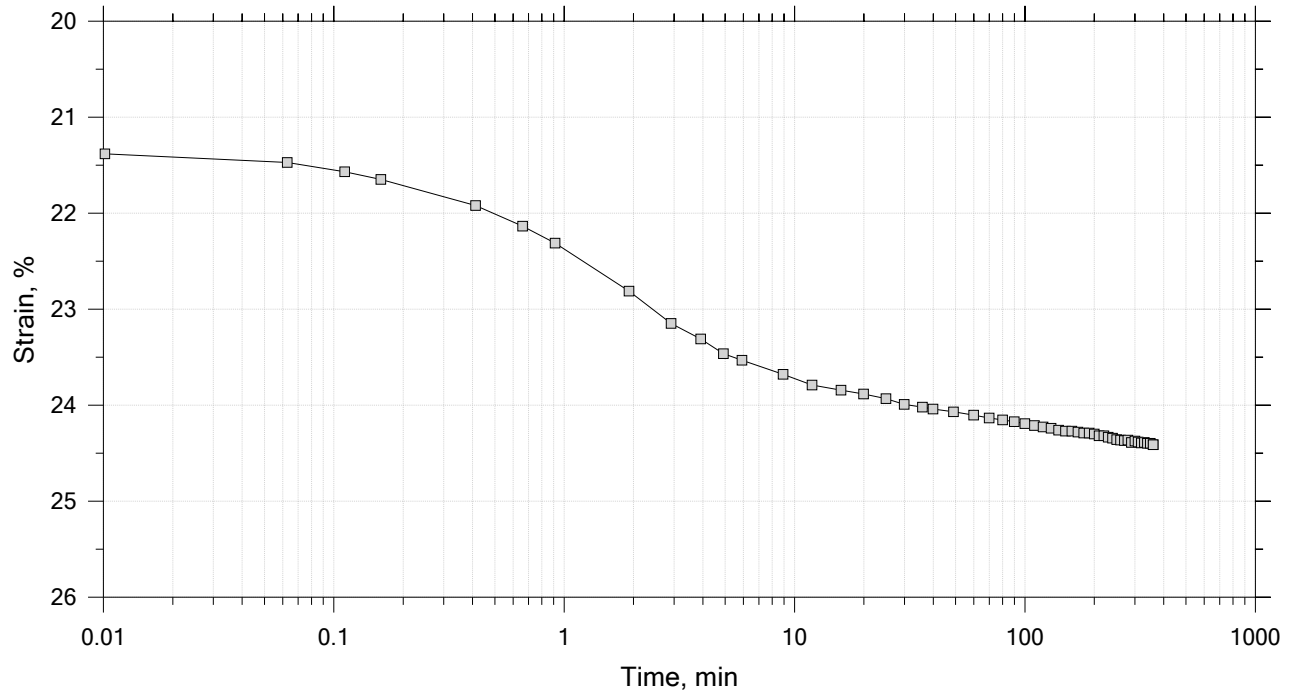
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



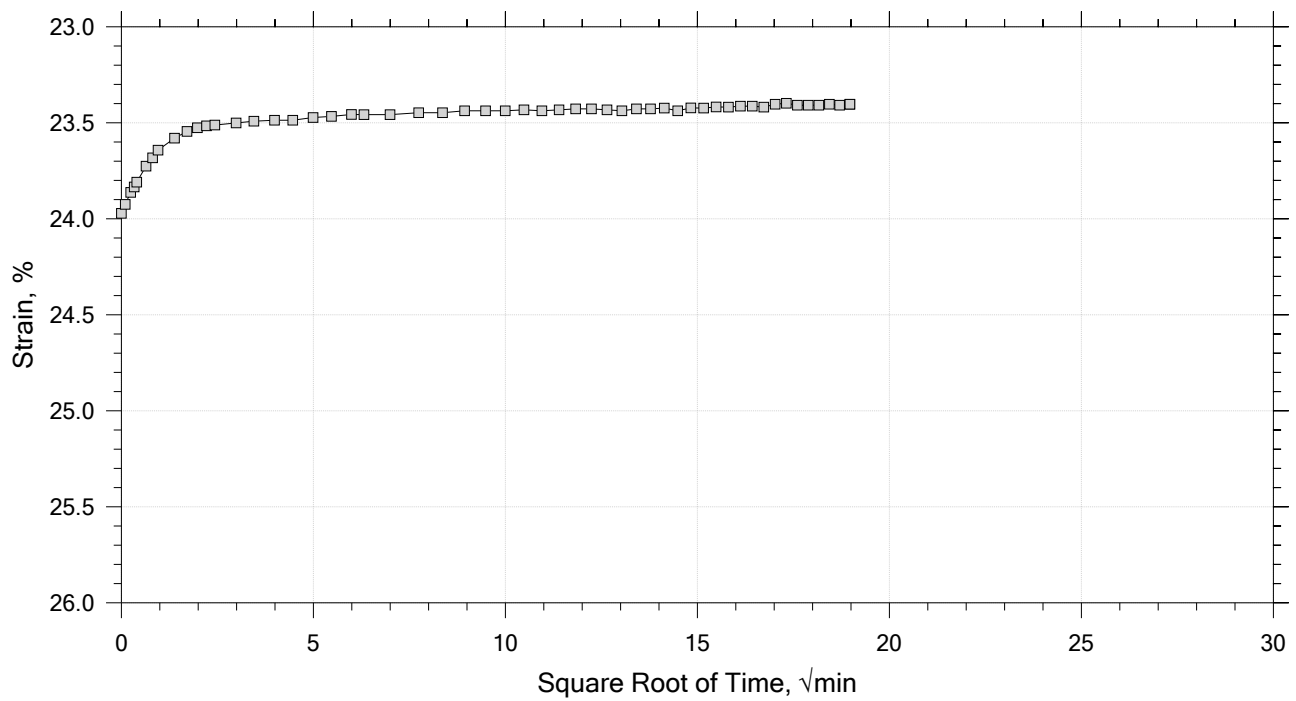
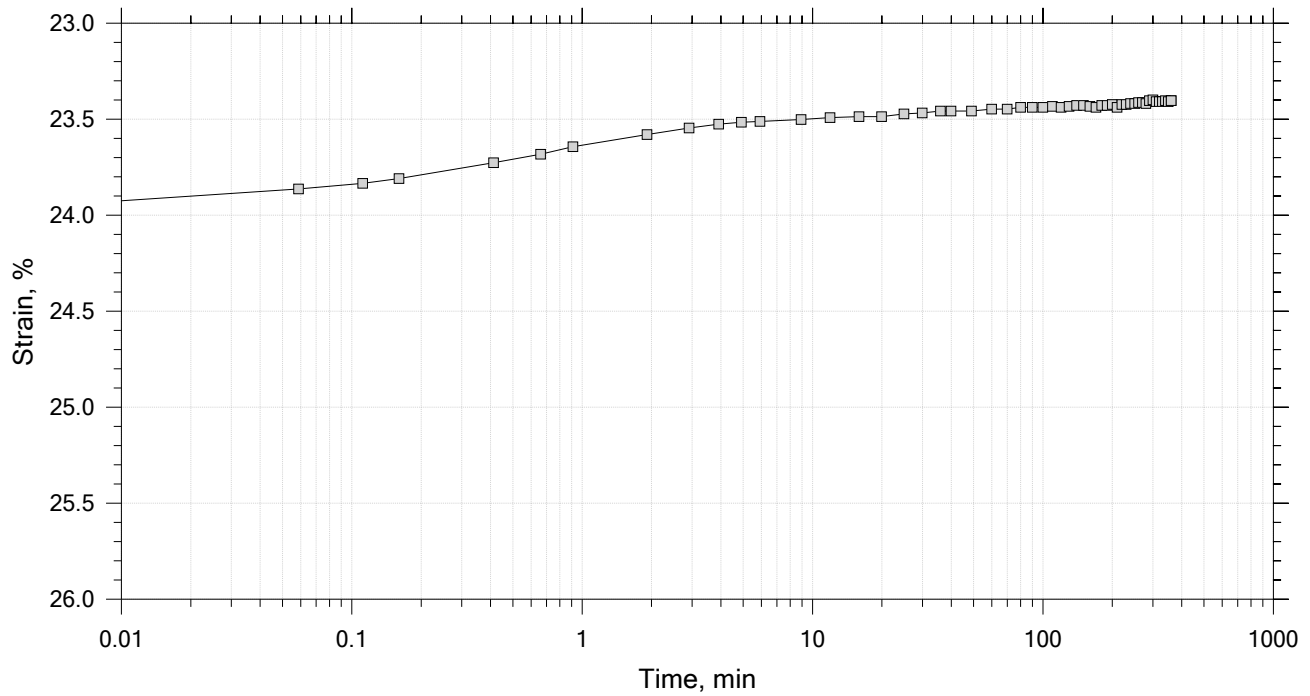
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	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



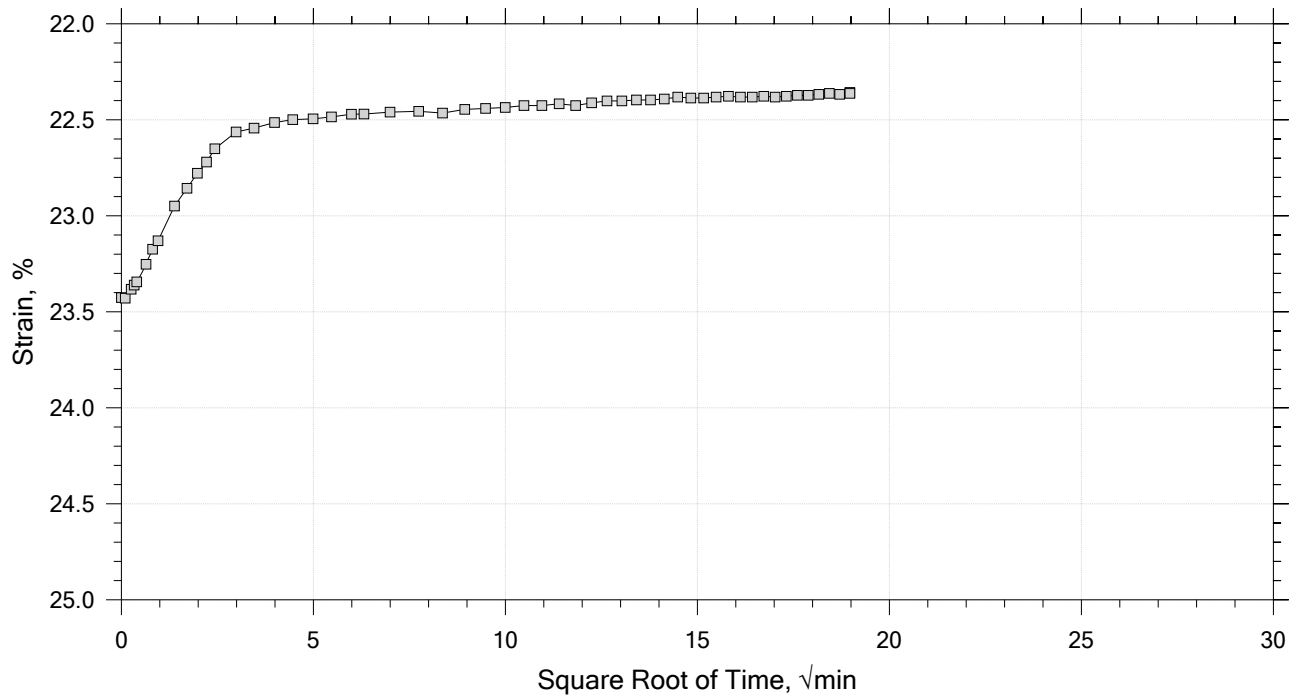
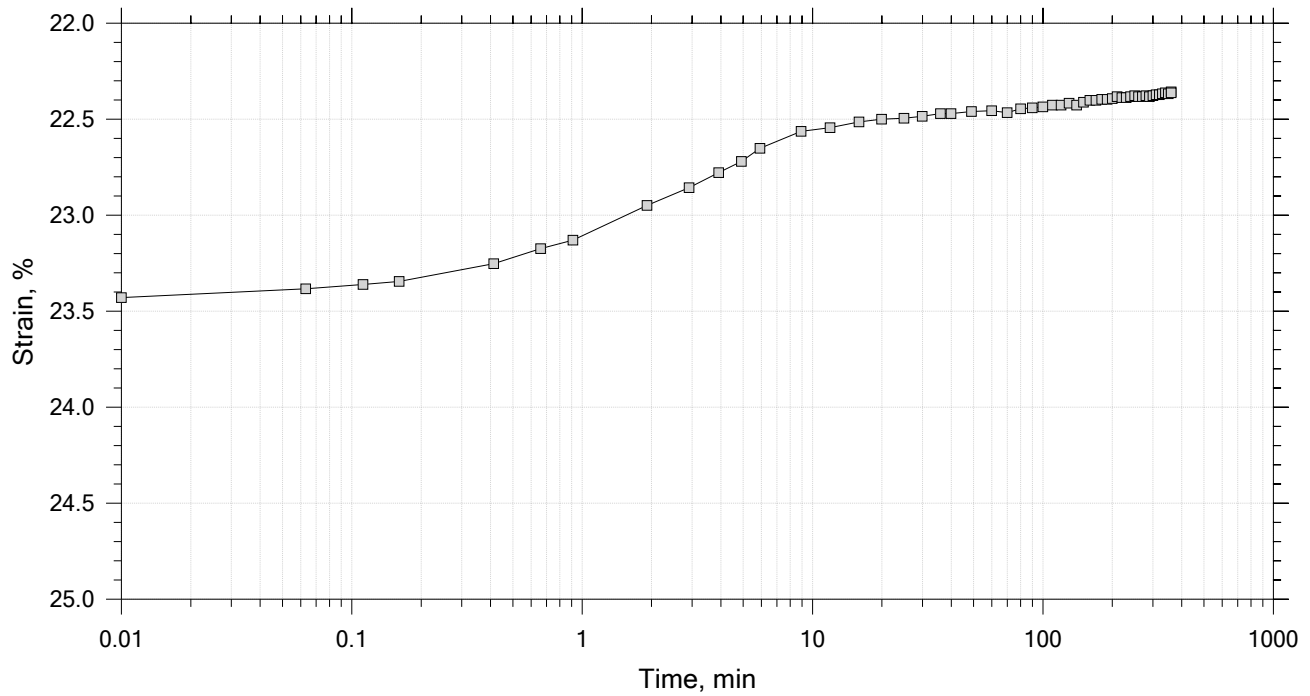
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



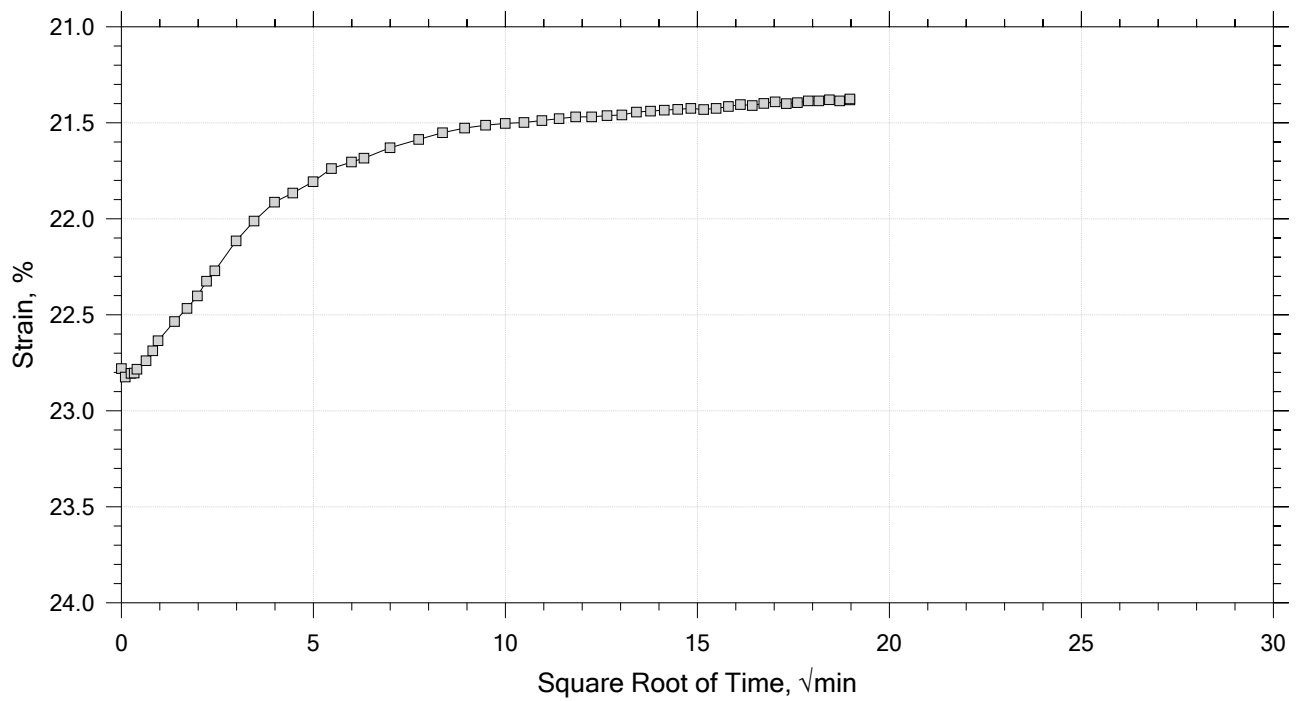
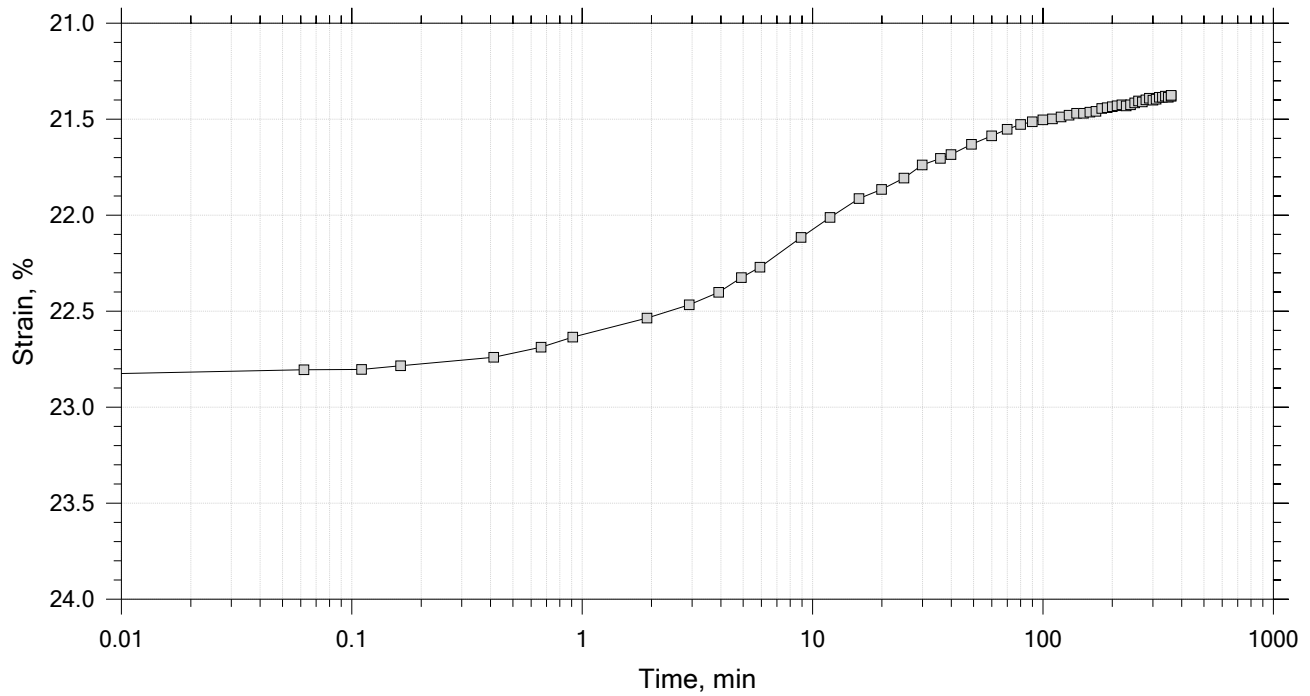
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	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



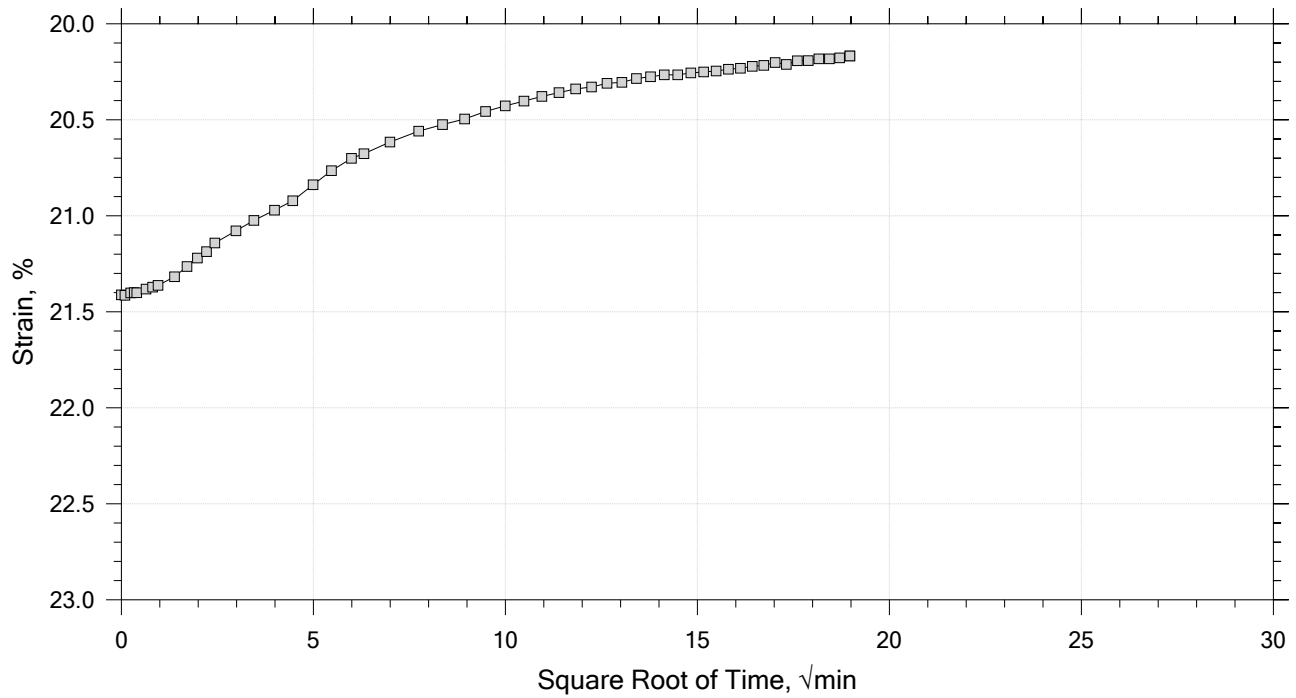
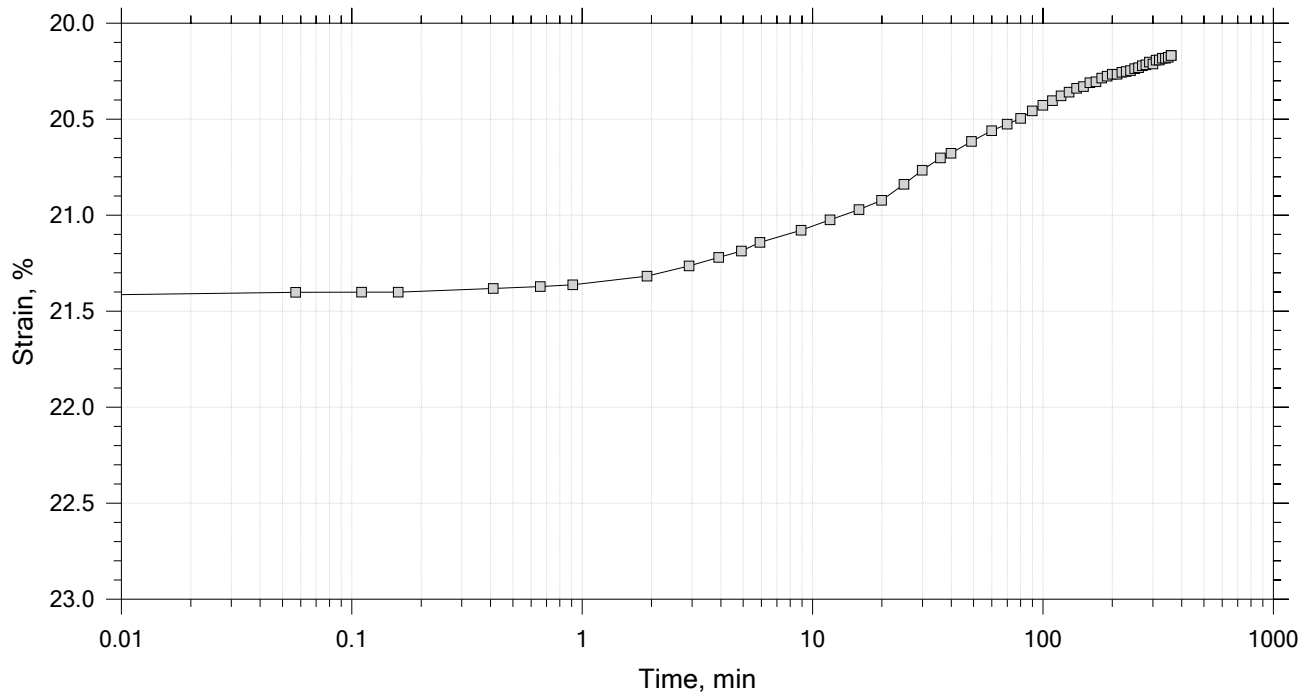
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



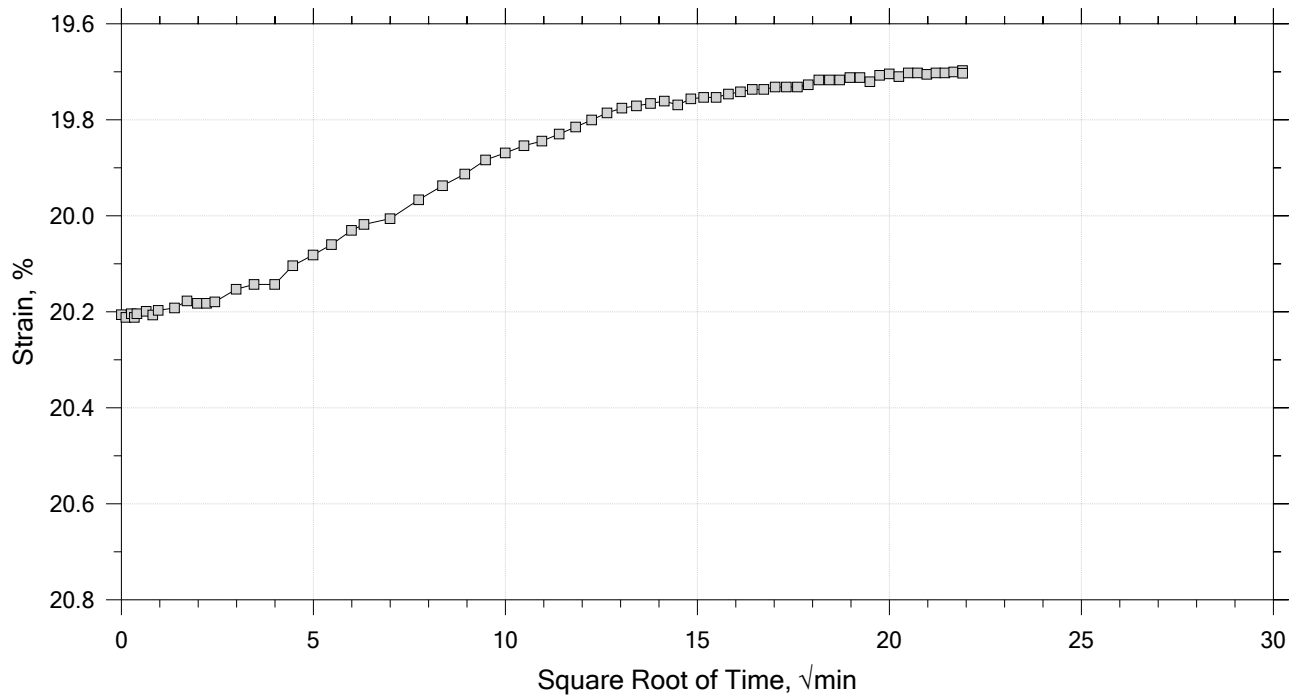
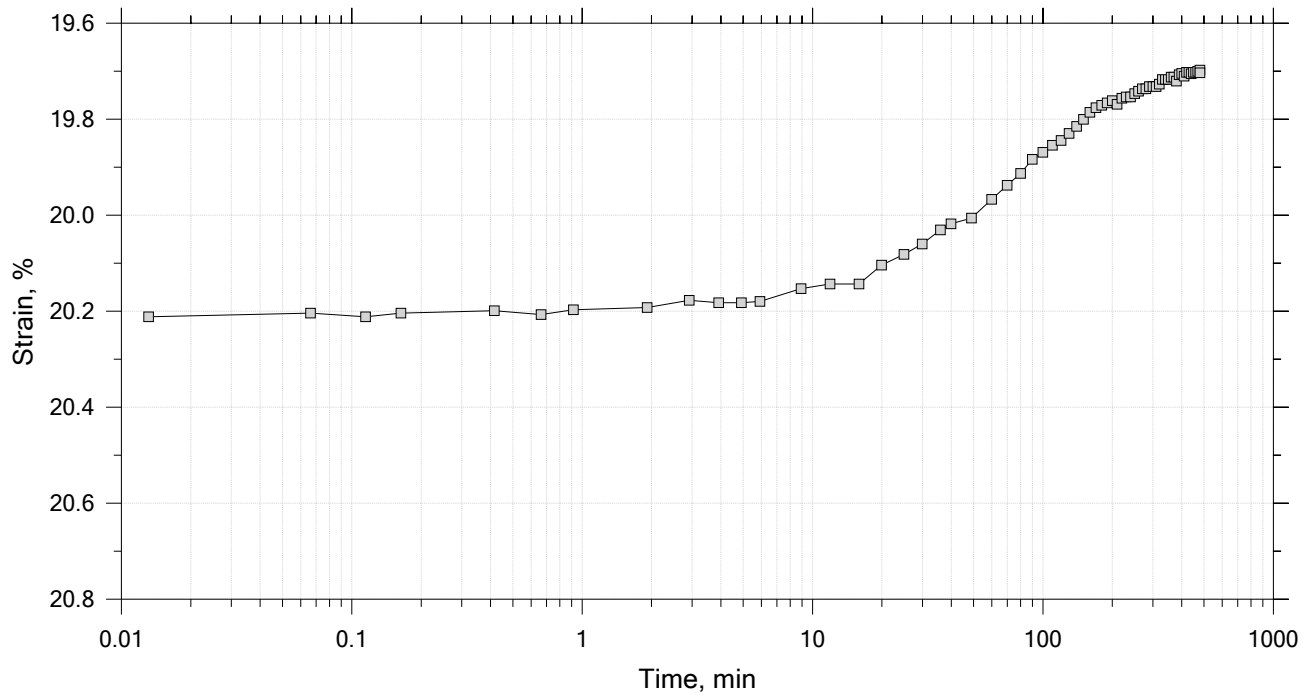
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 36
Initial Height: 1.00 in	Initial Void Ratio: 0.828	Plastic Limit: 18
Final Height: 0.85 in	Final Void Ratio: 0.554	Plasticity Index: 18

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E-2599	RING		A1902
Mass Container, gm	8.22	109.73	109.73	8.29
Mass Container + Wet Soil, gm	167.94	265.46	255	153.18
Mass Container + Dry Soil, gm	128.62	230.63	230.63	128.87
Mass Dry Soil, gm	120.4	120.9	120.9	120.58
Water Content, %	32.66	28.81	20.16	20.16
Void Ratio	---	0.83	0.55	---
Degree of Saturation, %	---	95.59	100.00	---
Dry Unit Weight, pcf	---	93.825	110.38	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

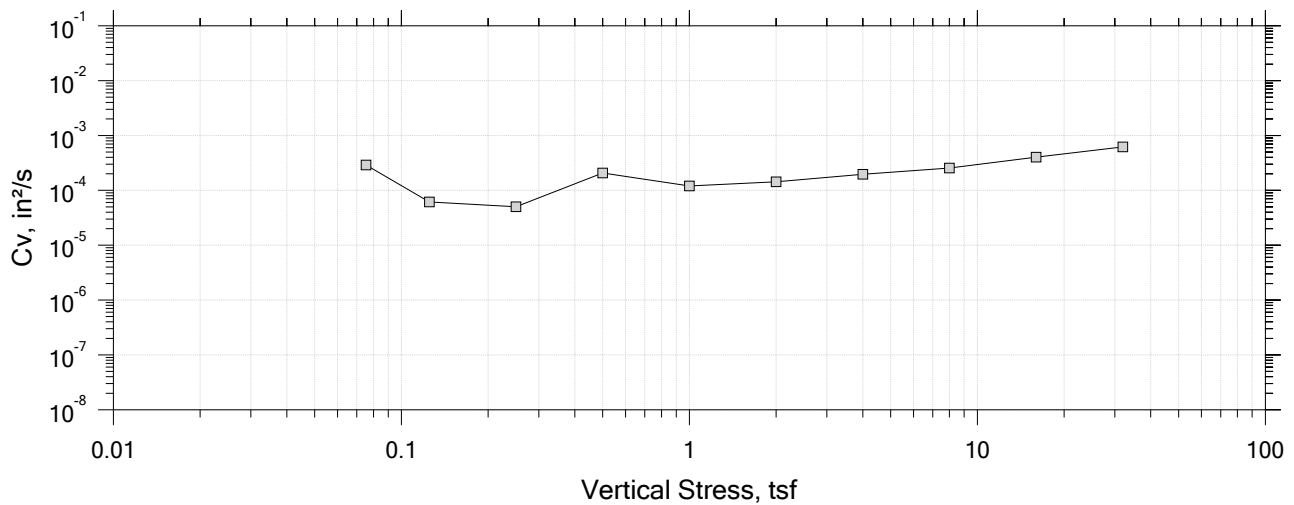
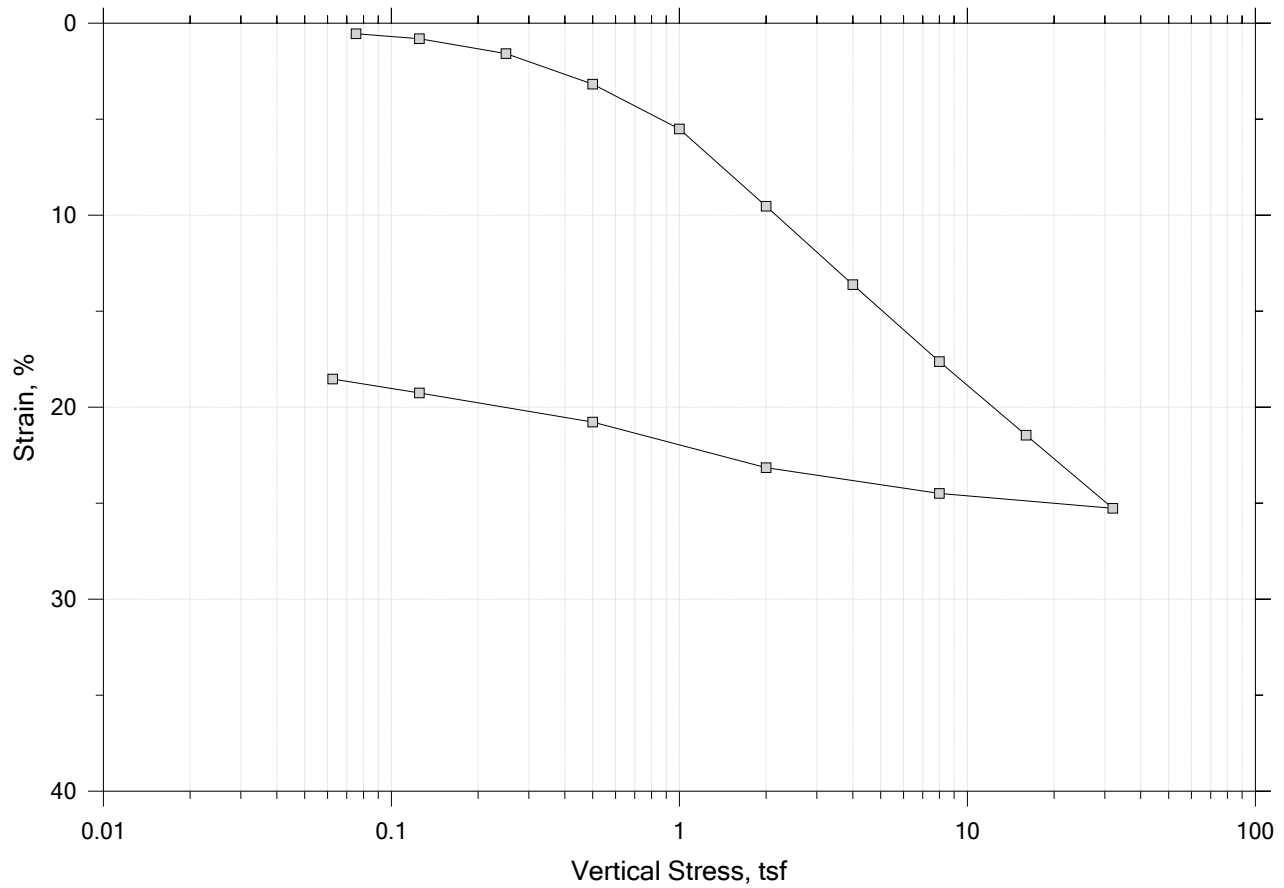
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 5-7 ft
	Test No.: IP-2	Sample Type: intact	Elevation:
	Description: Moist, gray clay		
	Remarks: System V, Swell Pressure = 0.0696 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

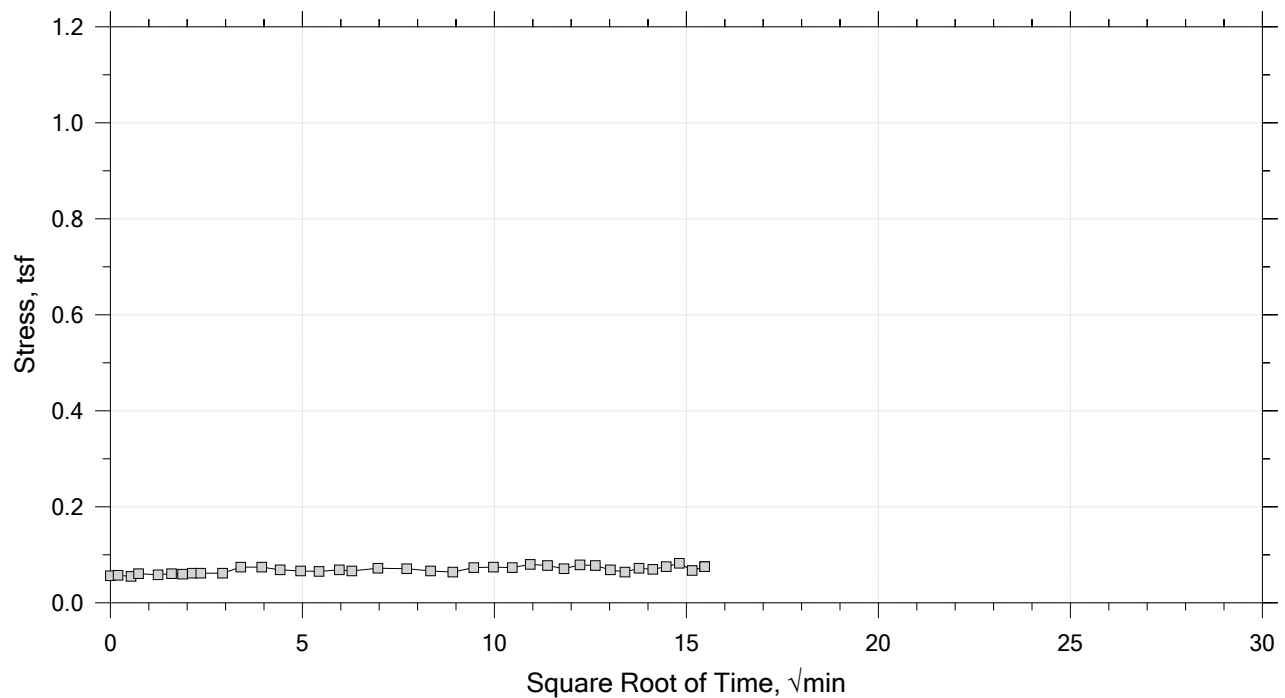
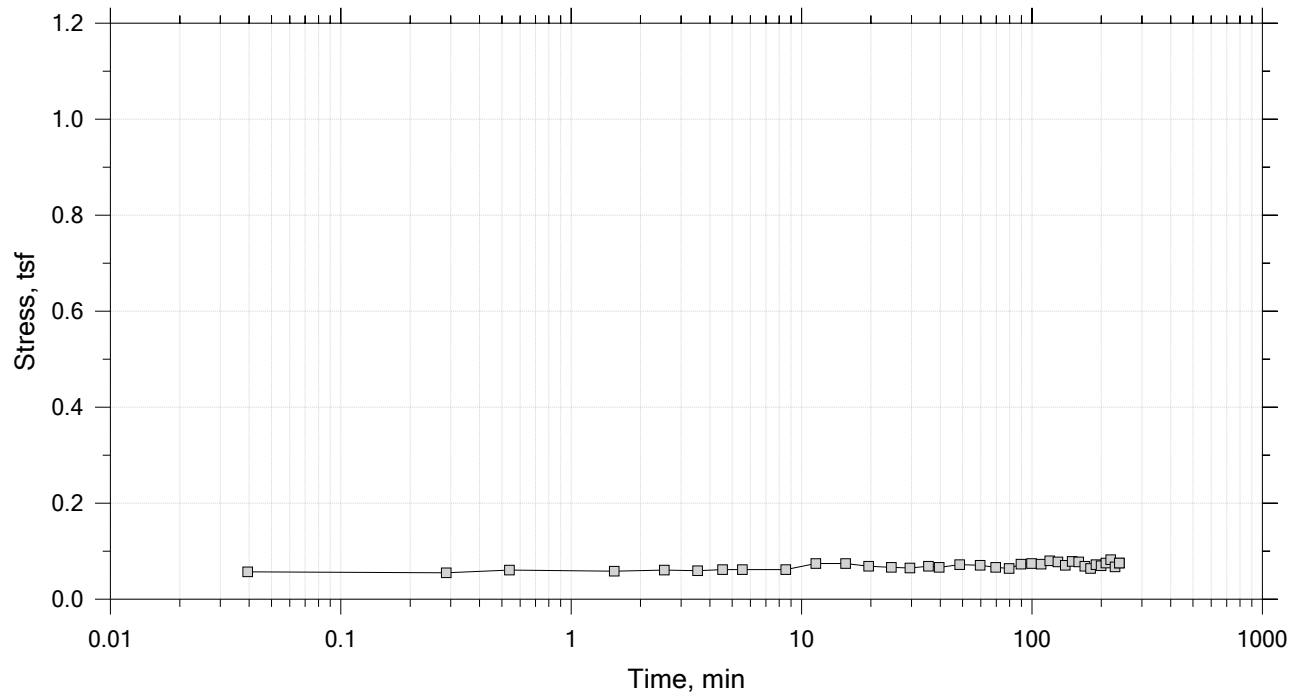
Summary Report




	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0754 tsf



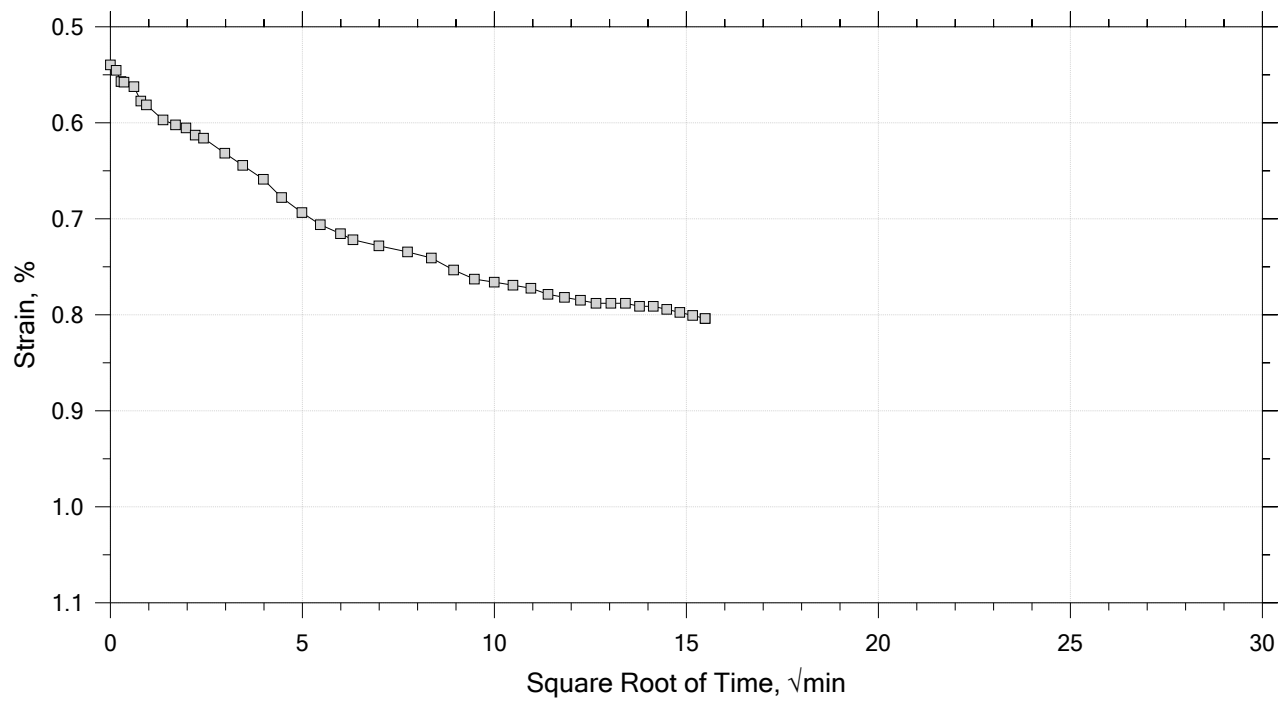
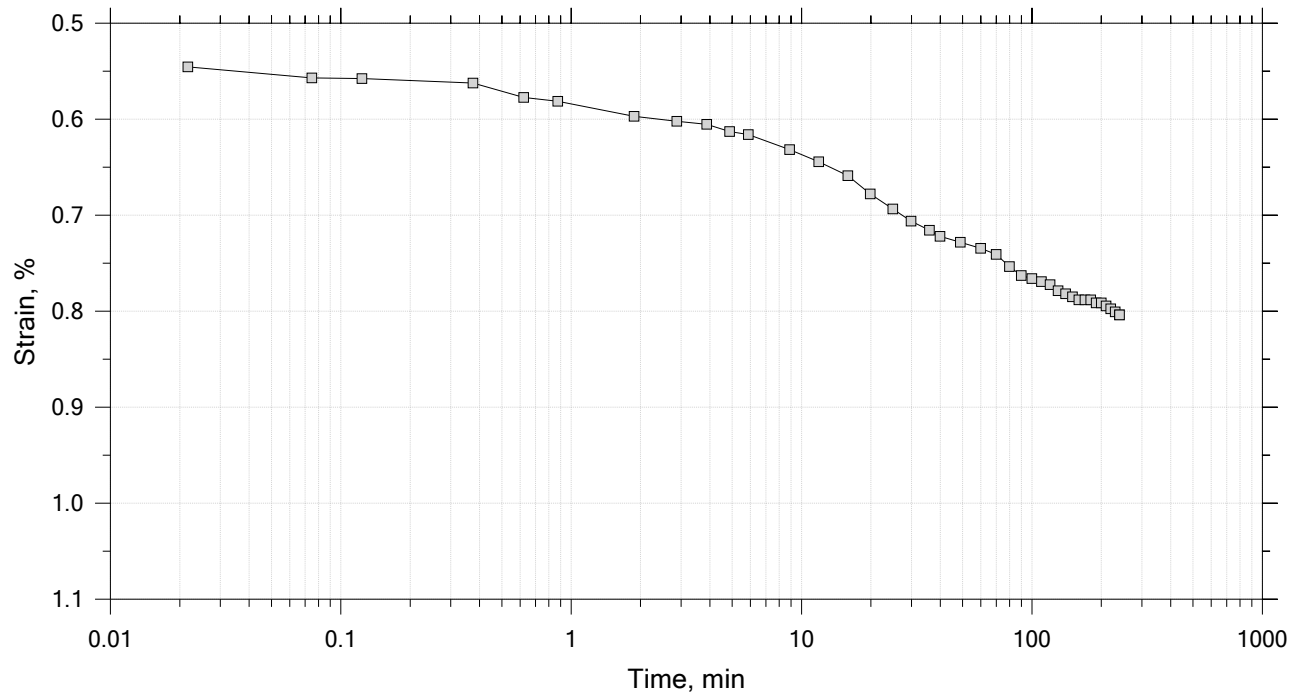
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



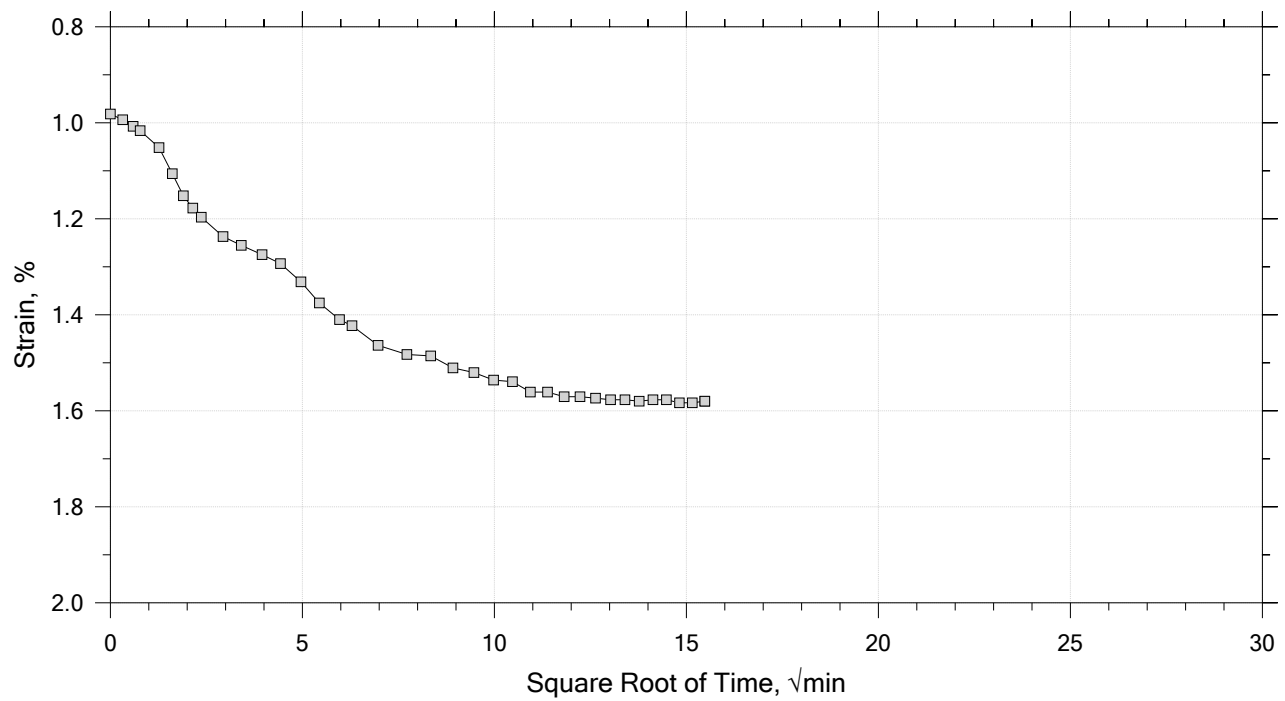
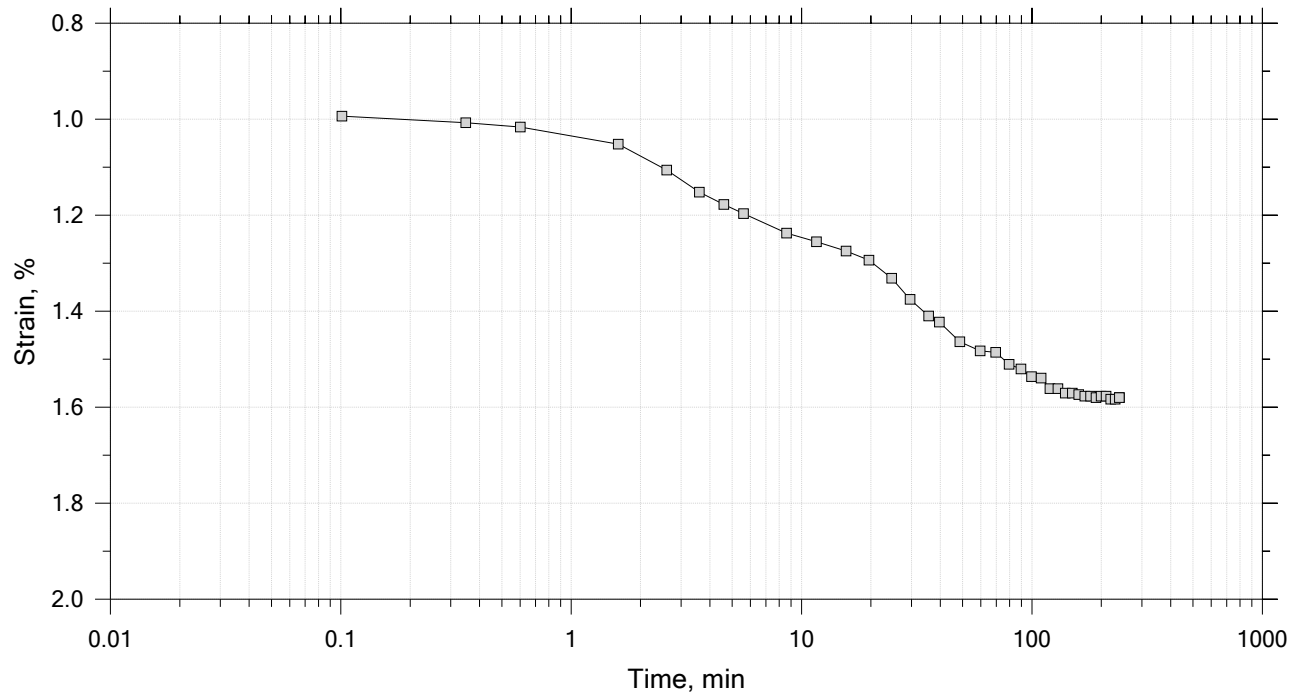
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



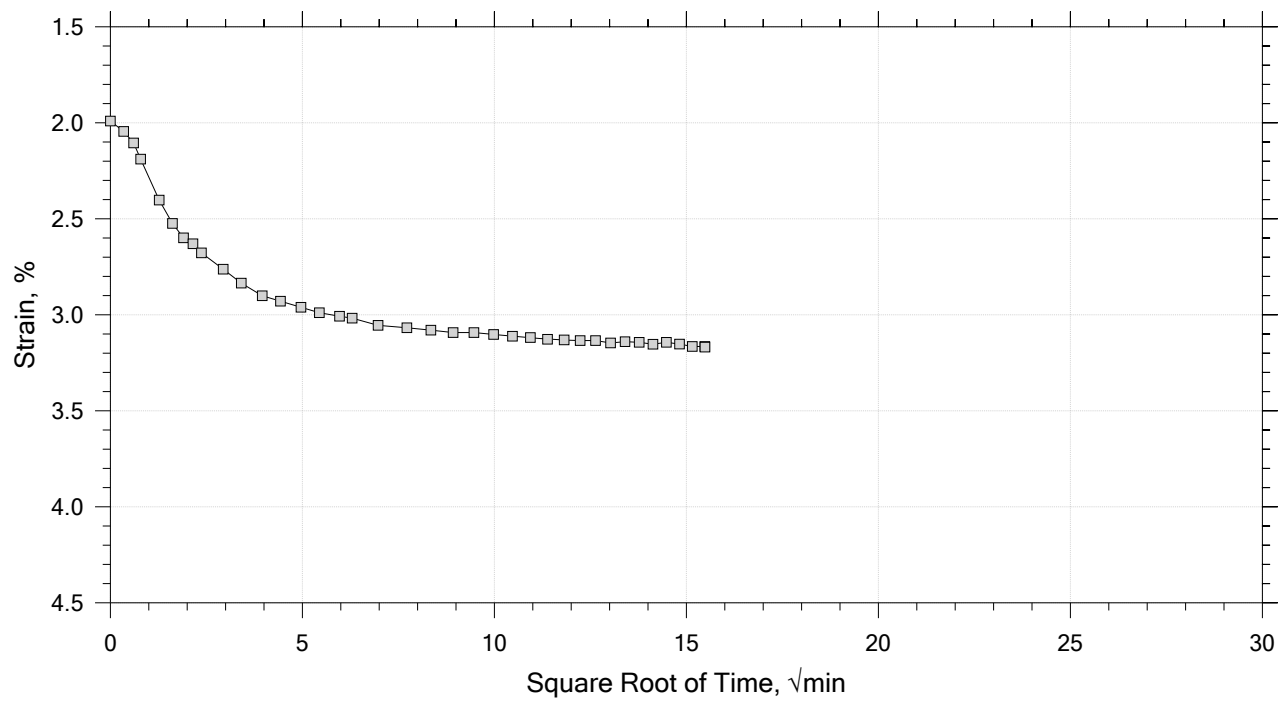
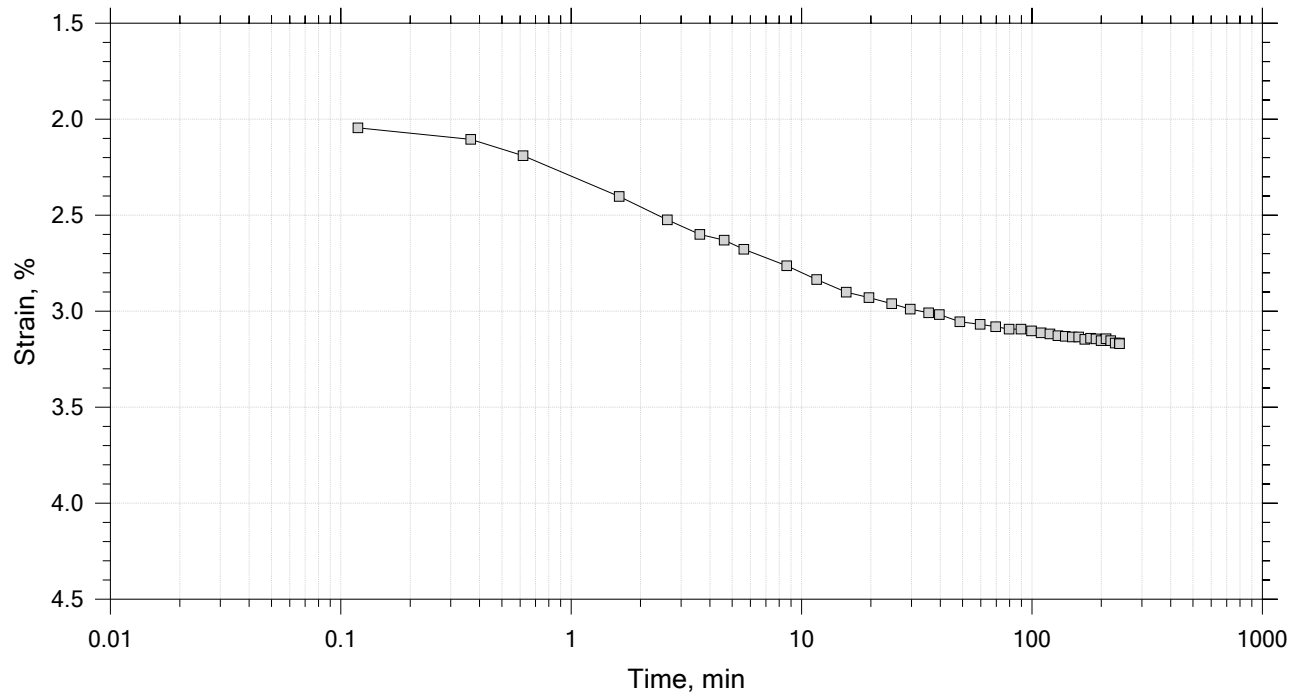
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



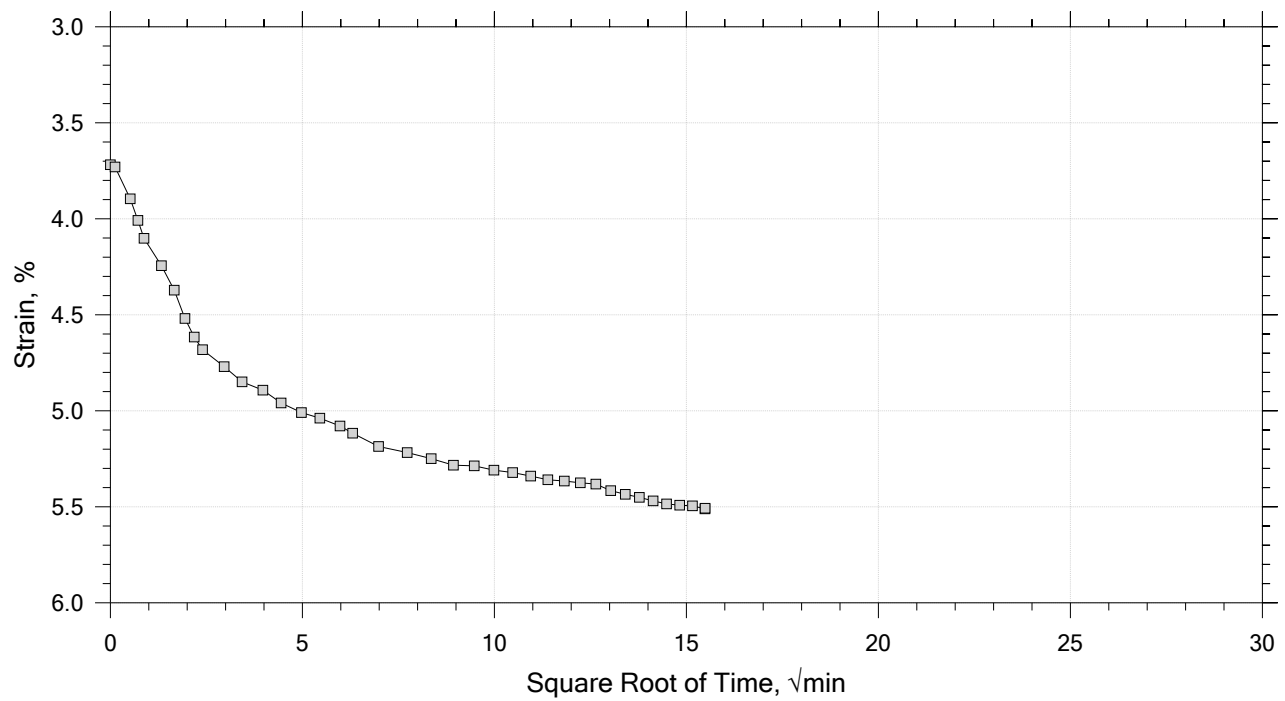
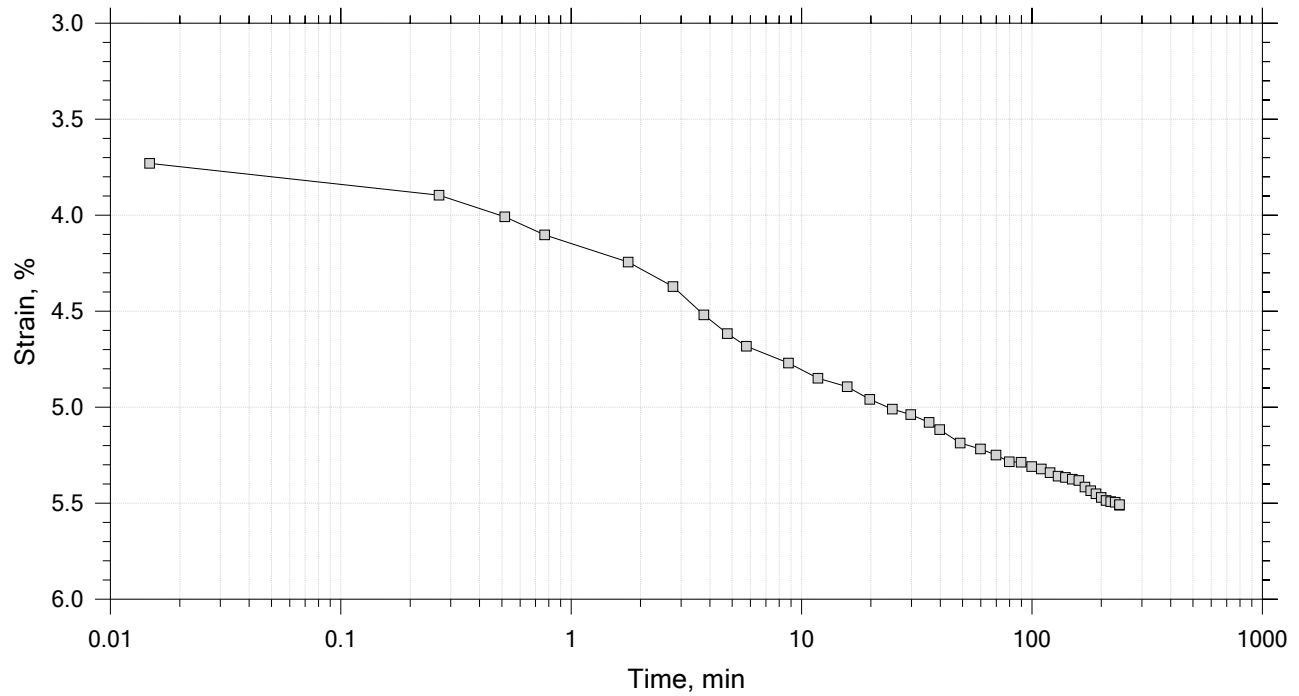
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



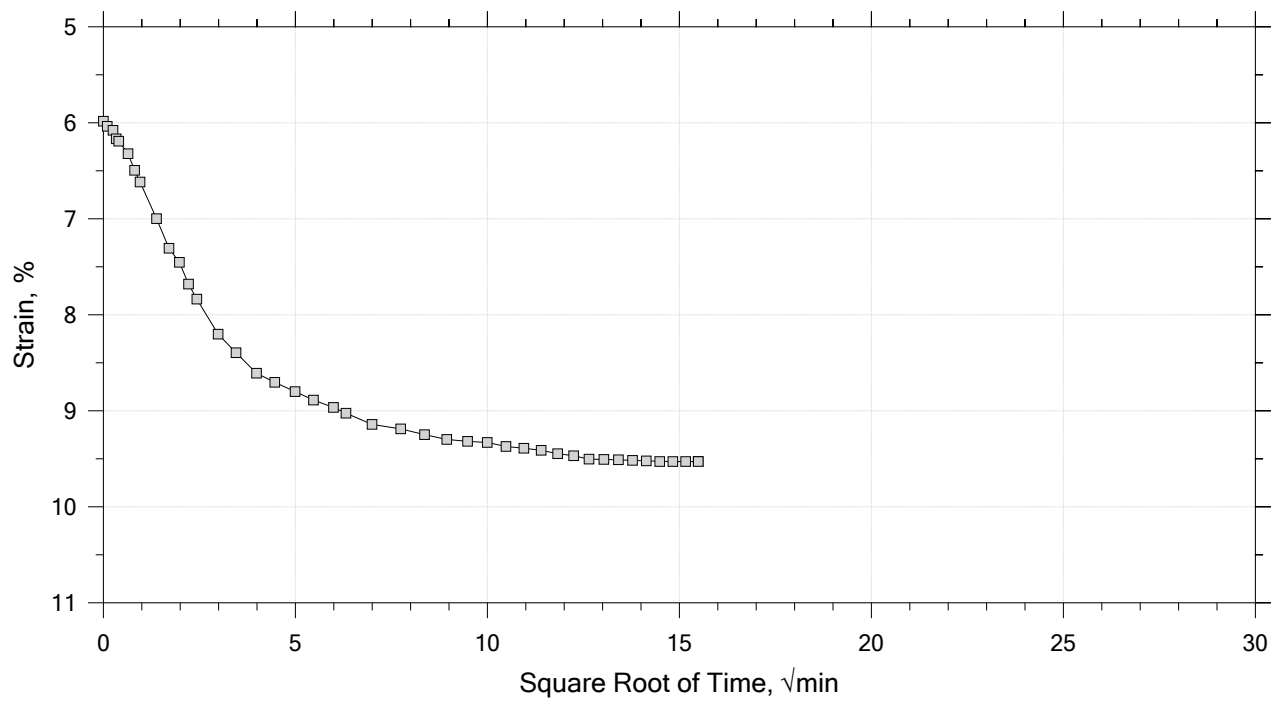
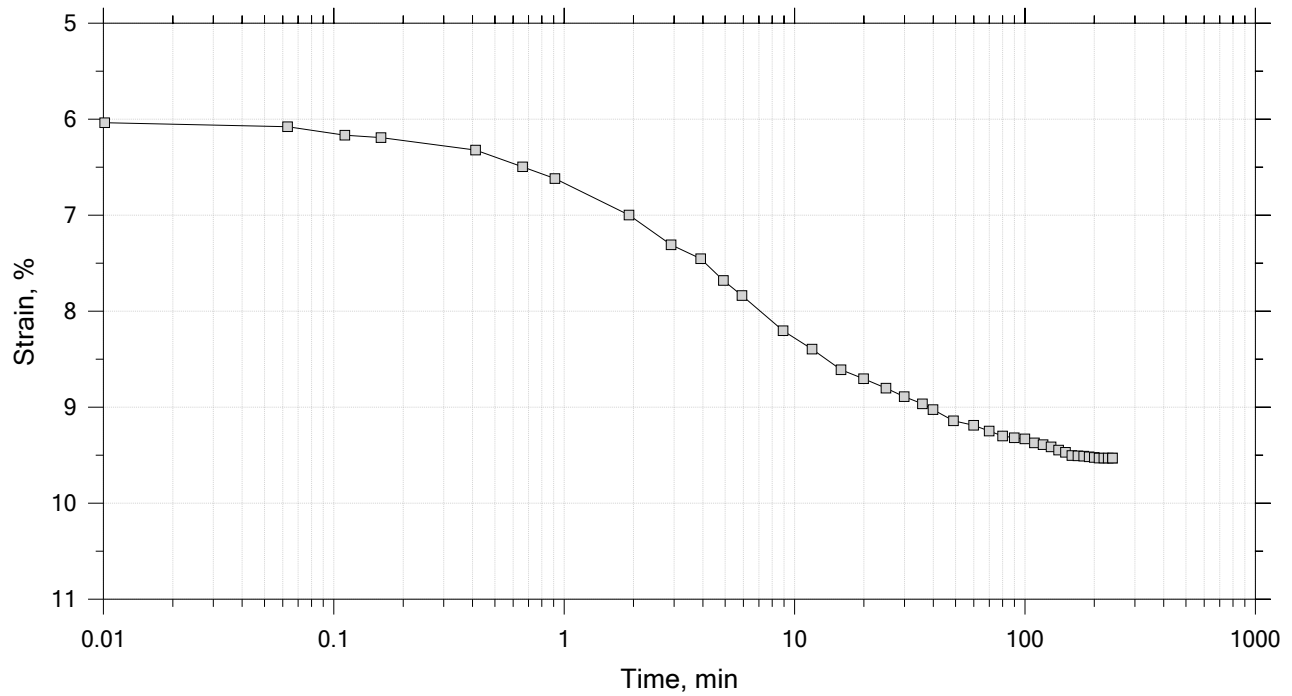
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



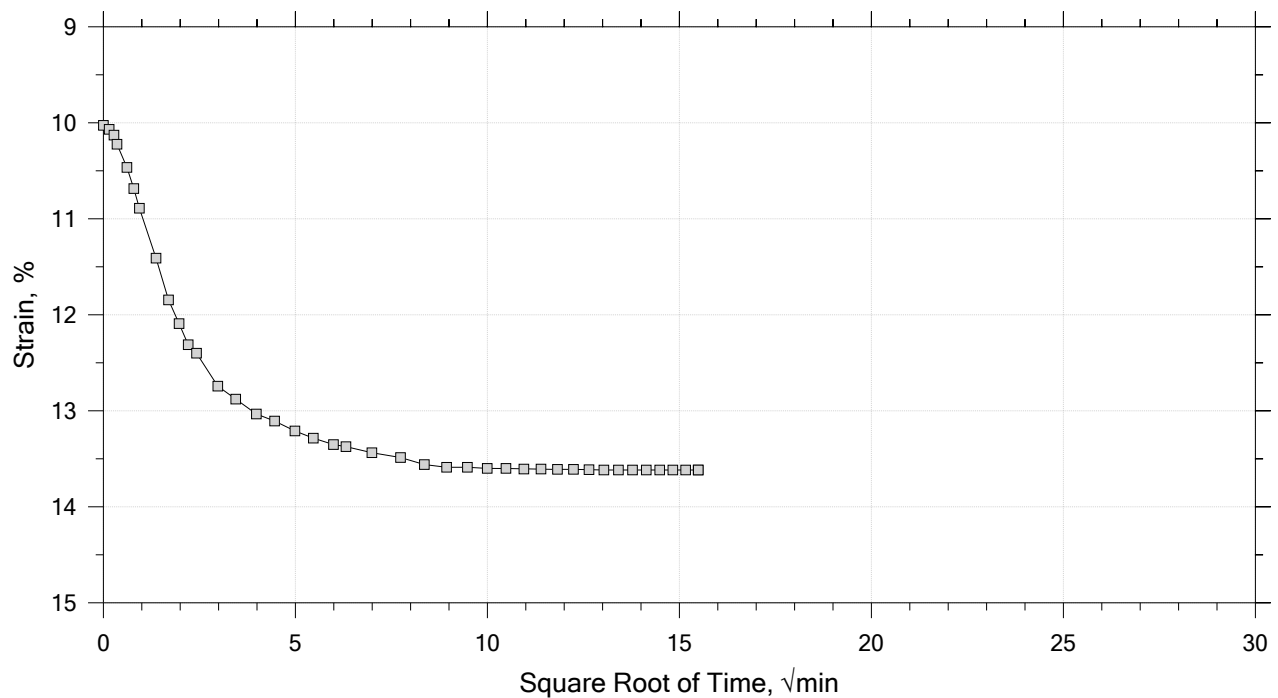
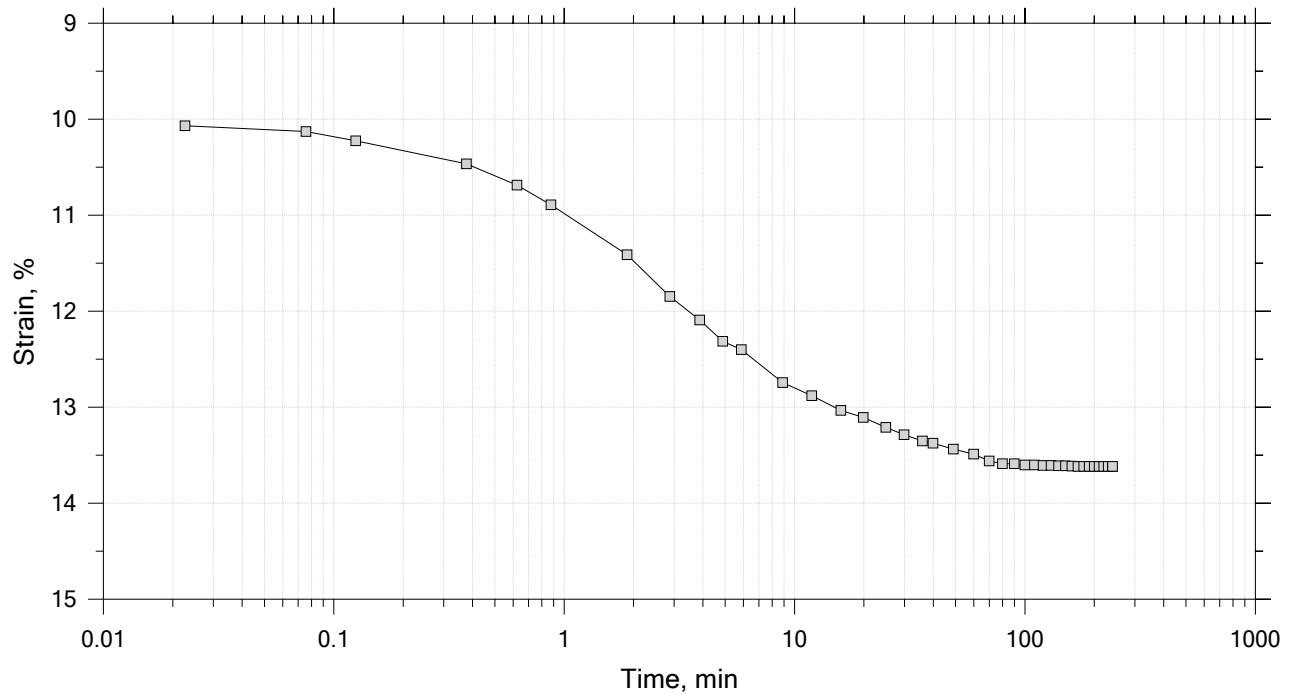
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



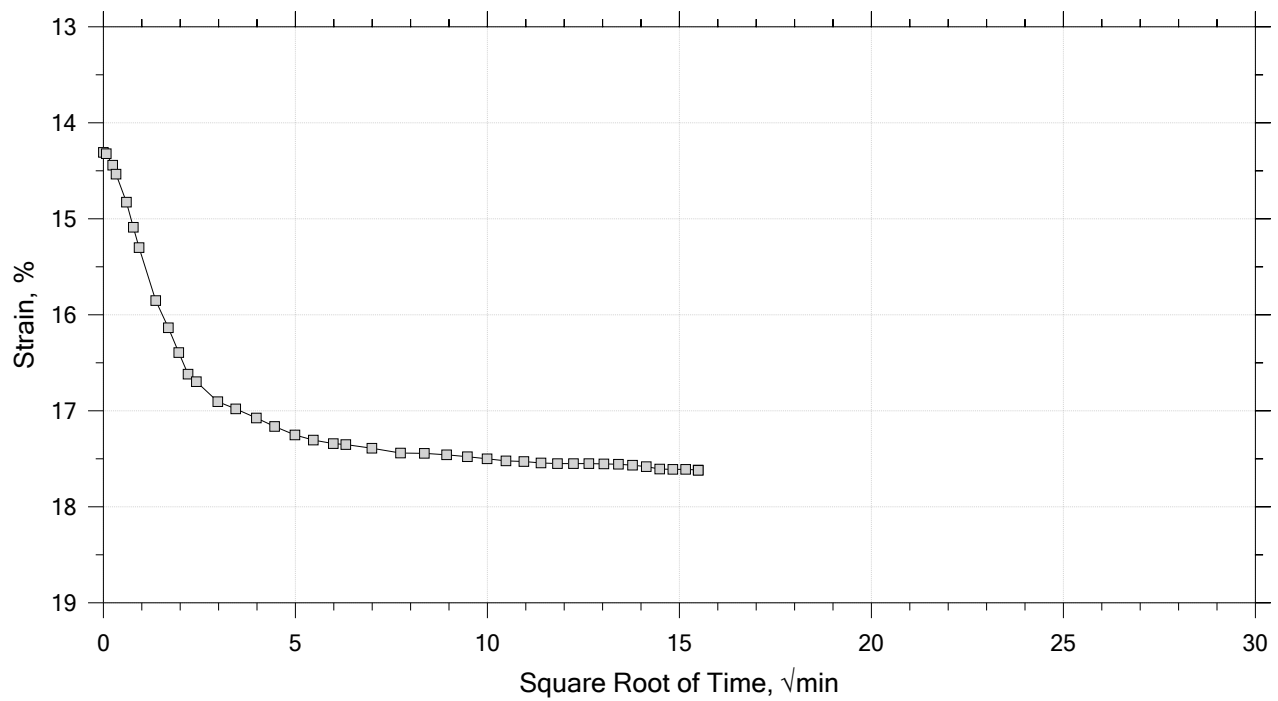
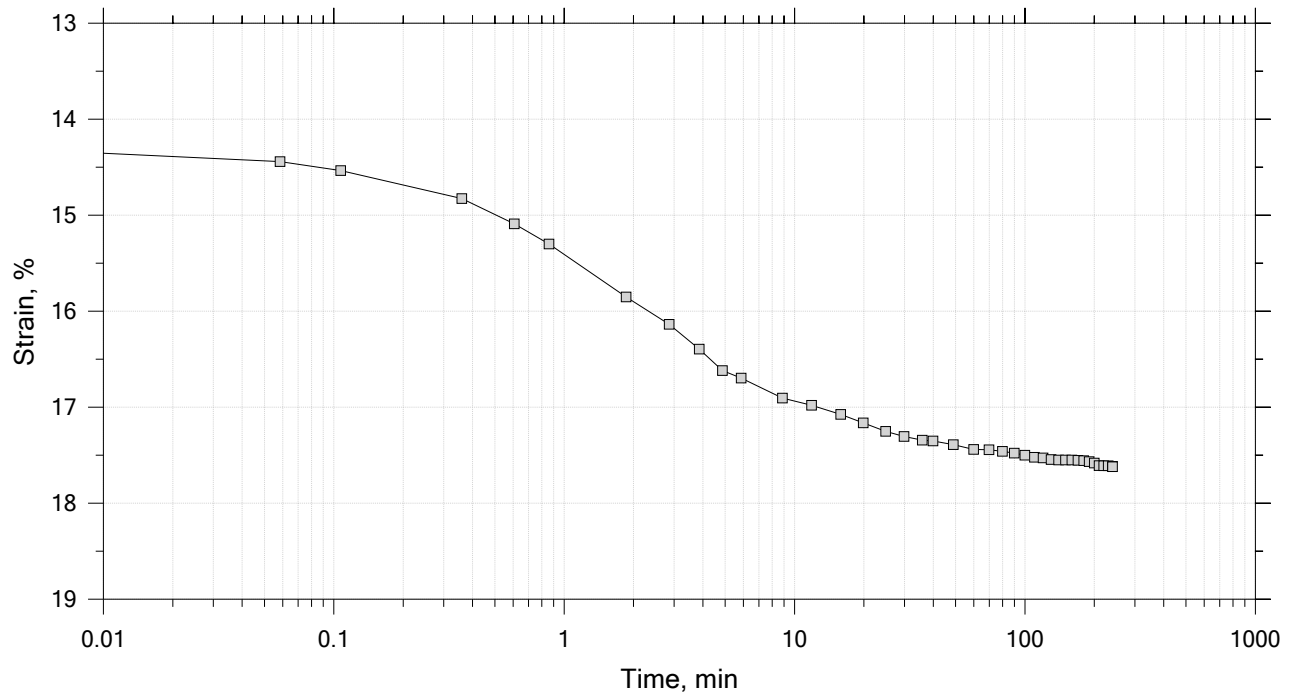
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



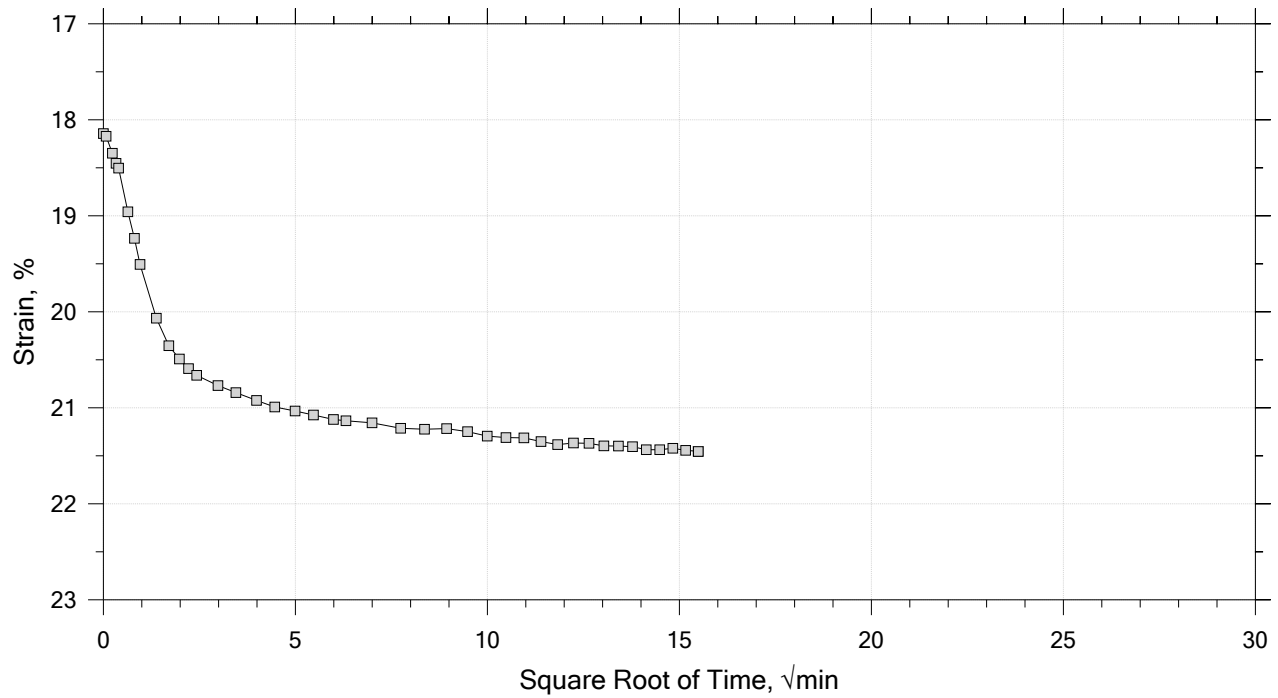
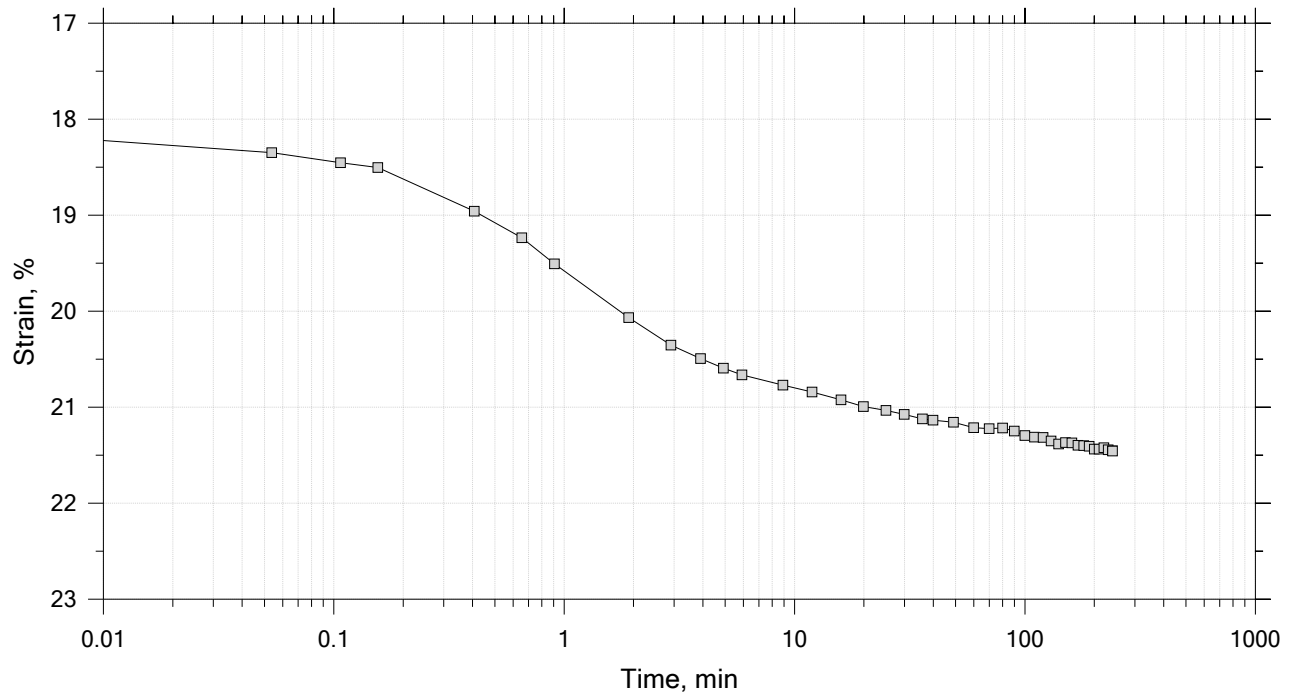
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



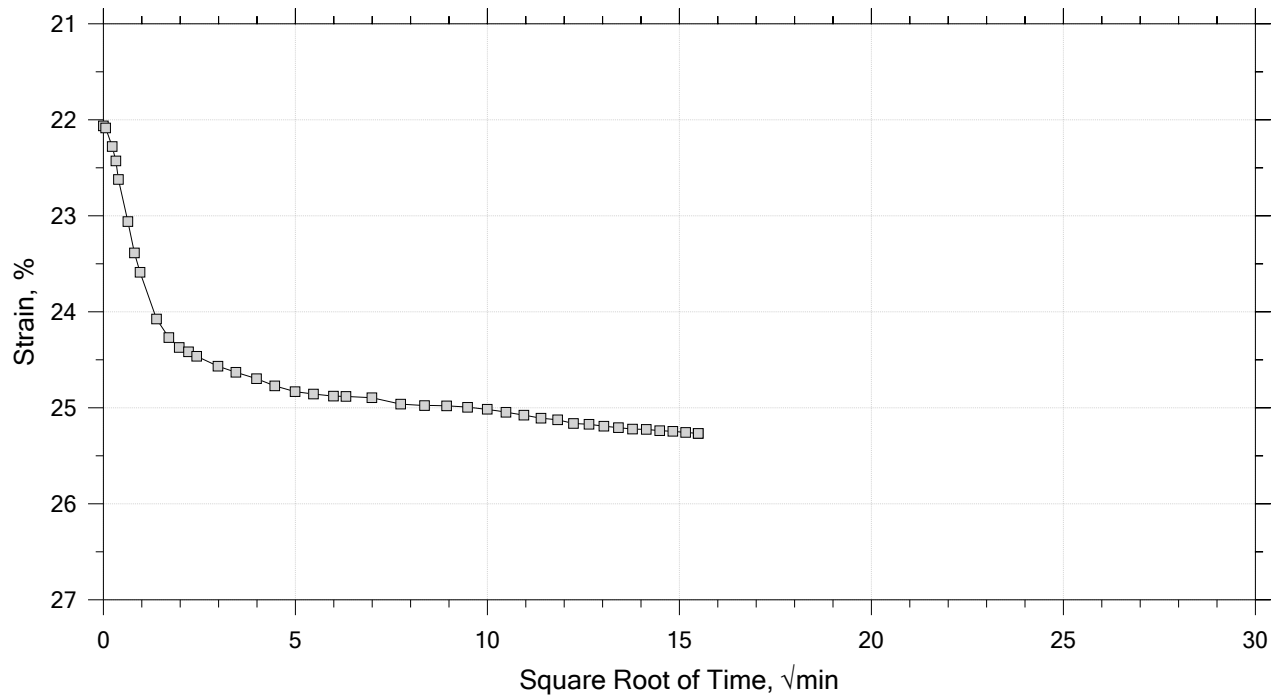
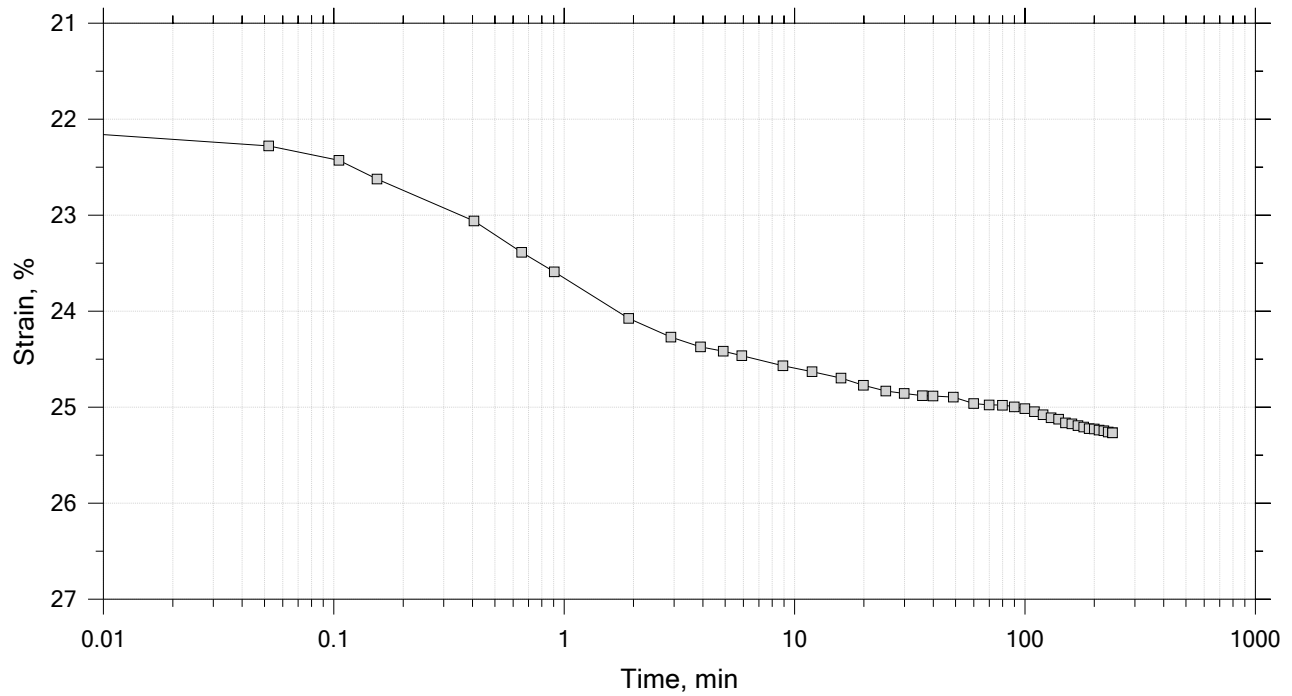
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



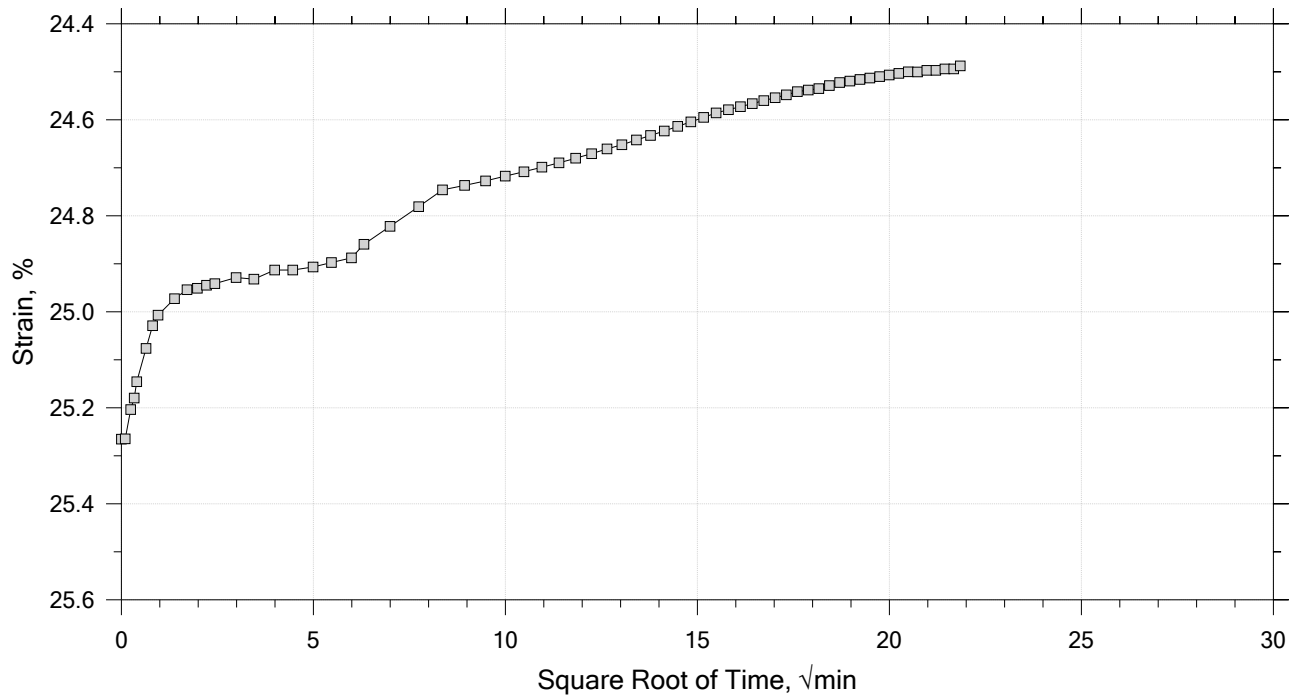
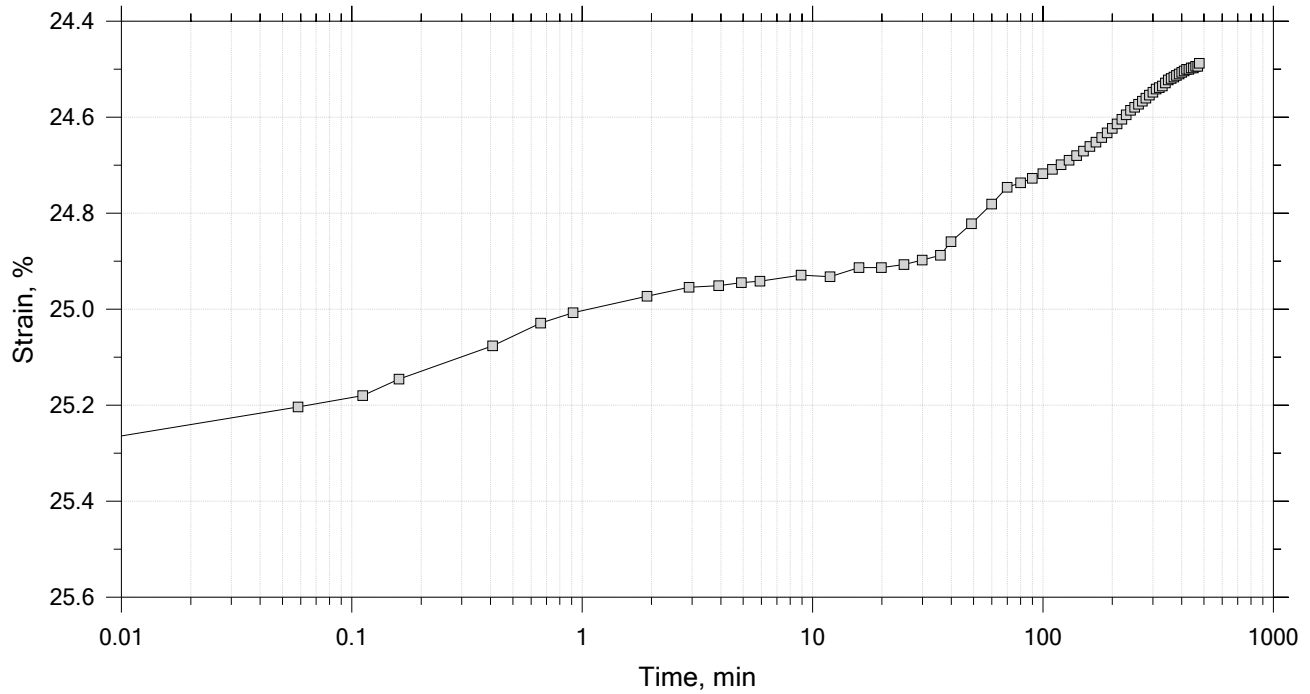
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



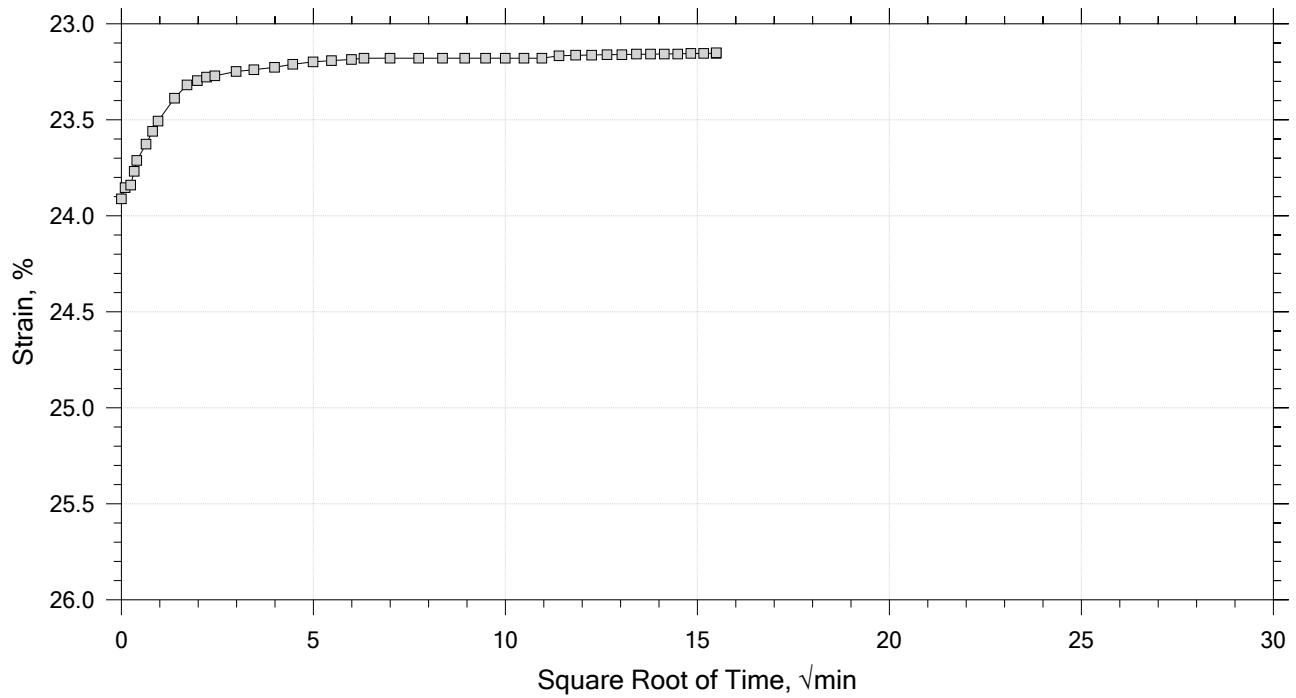
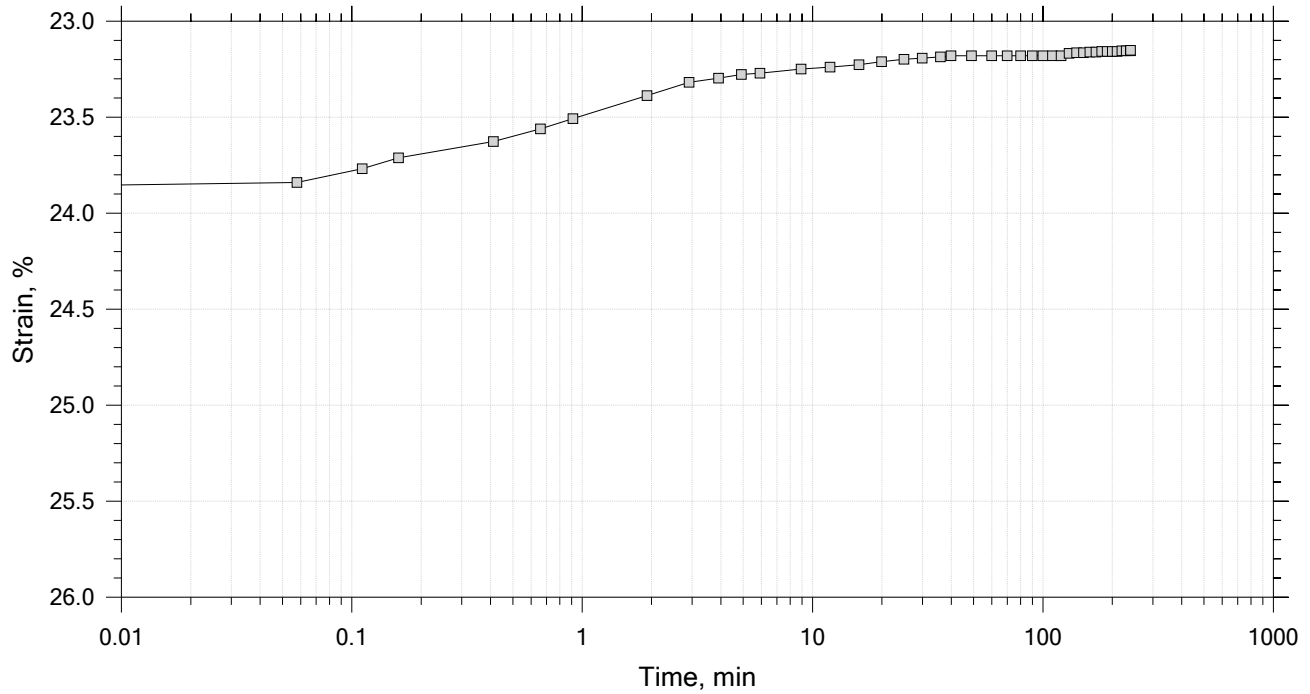
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



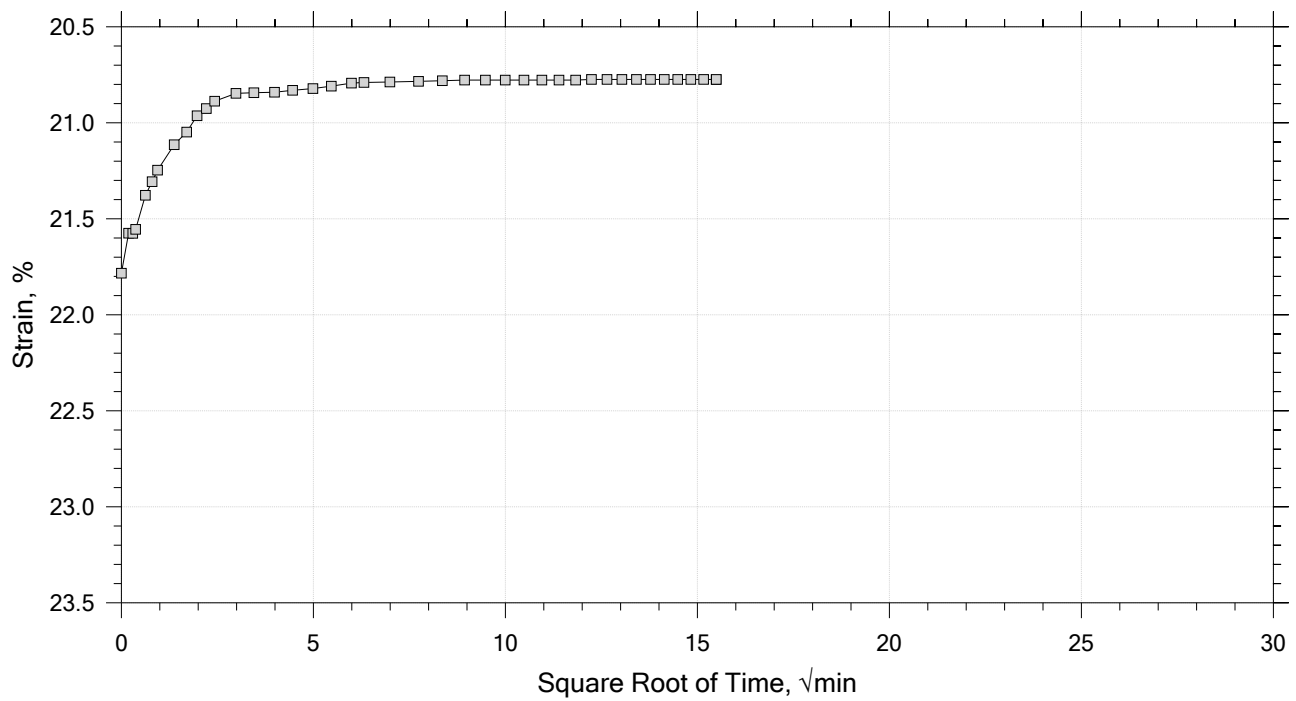
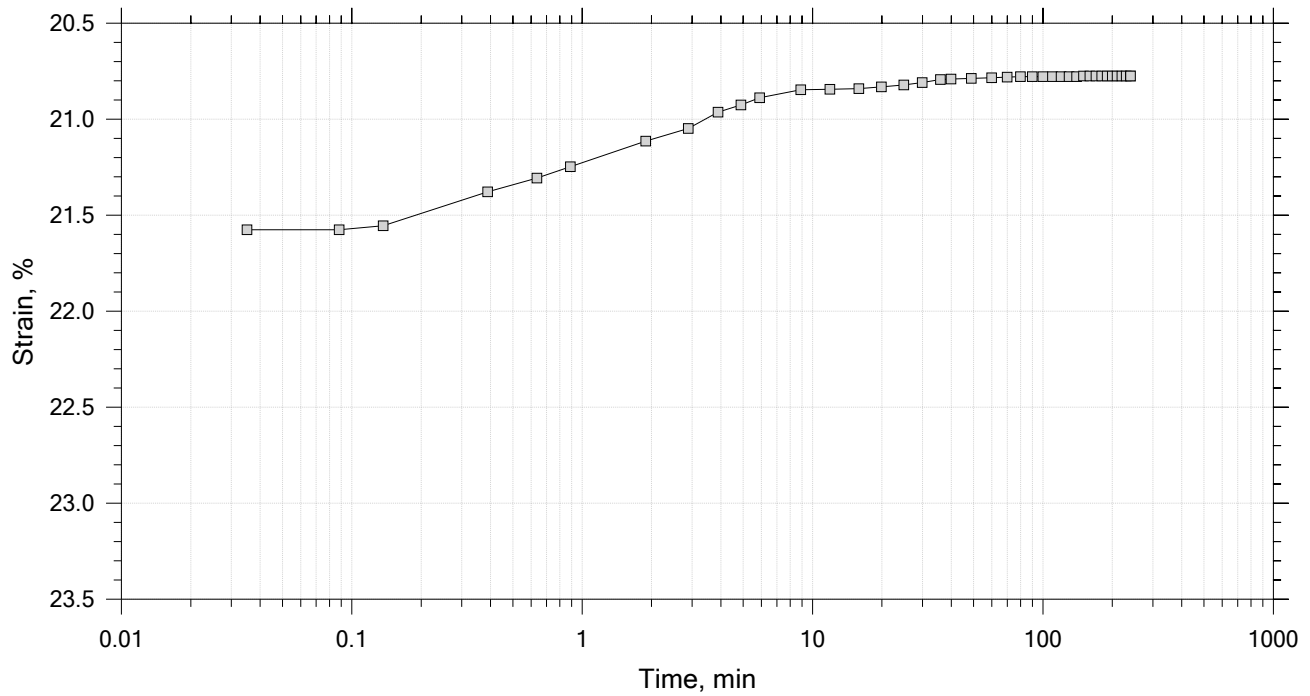
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



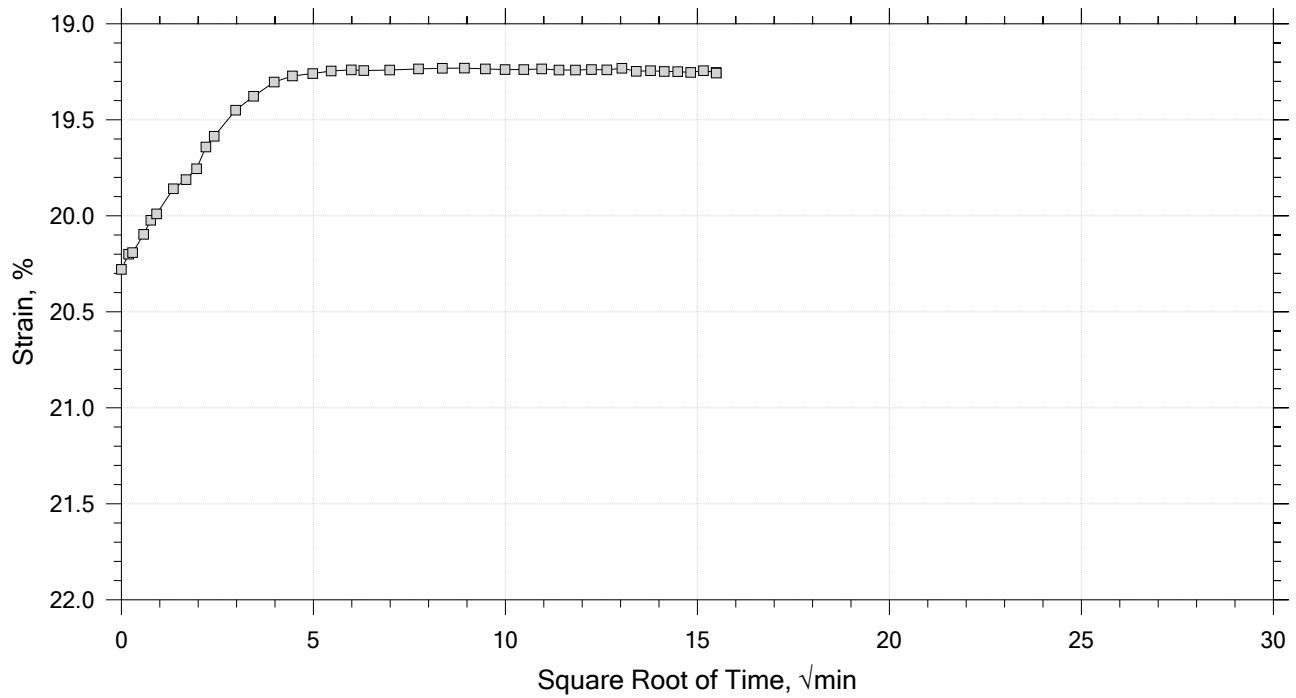
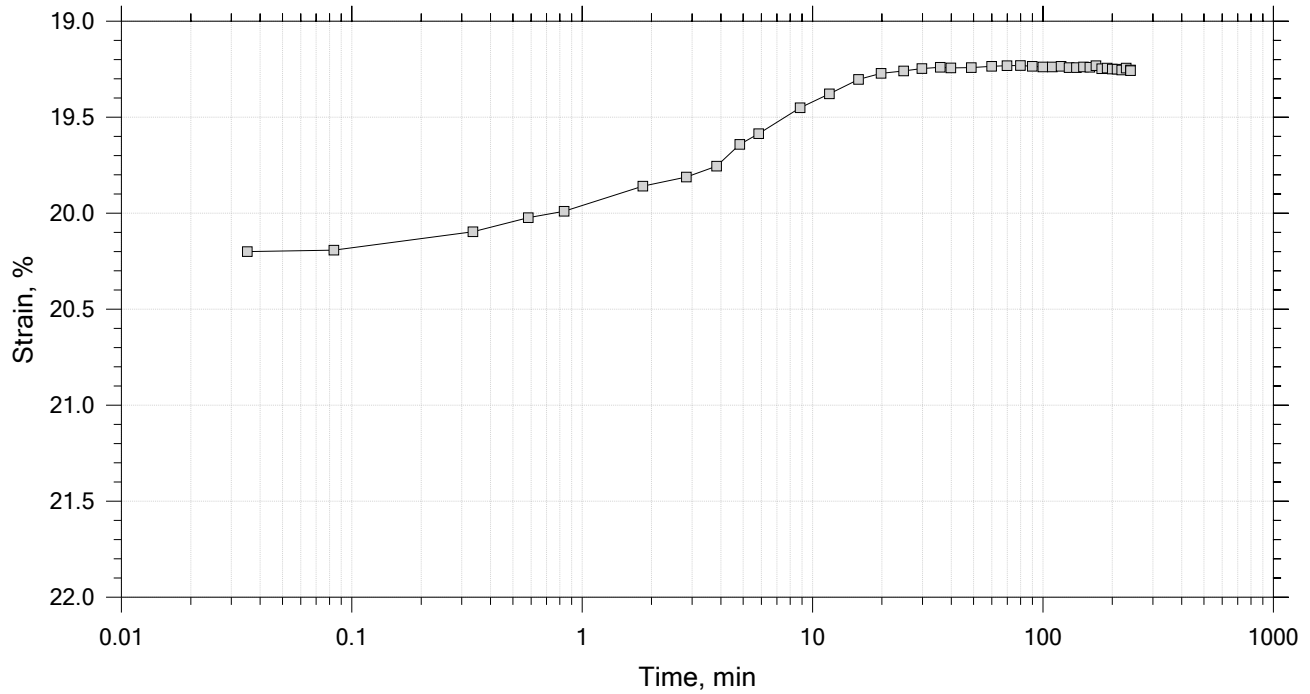
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



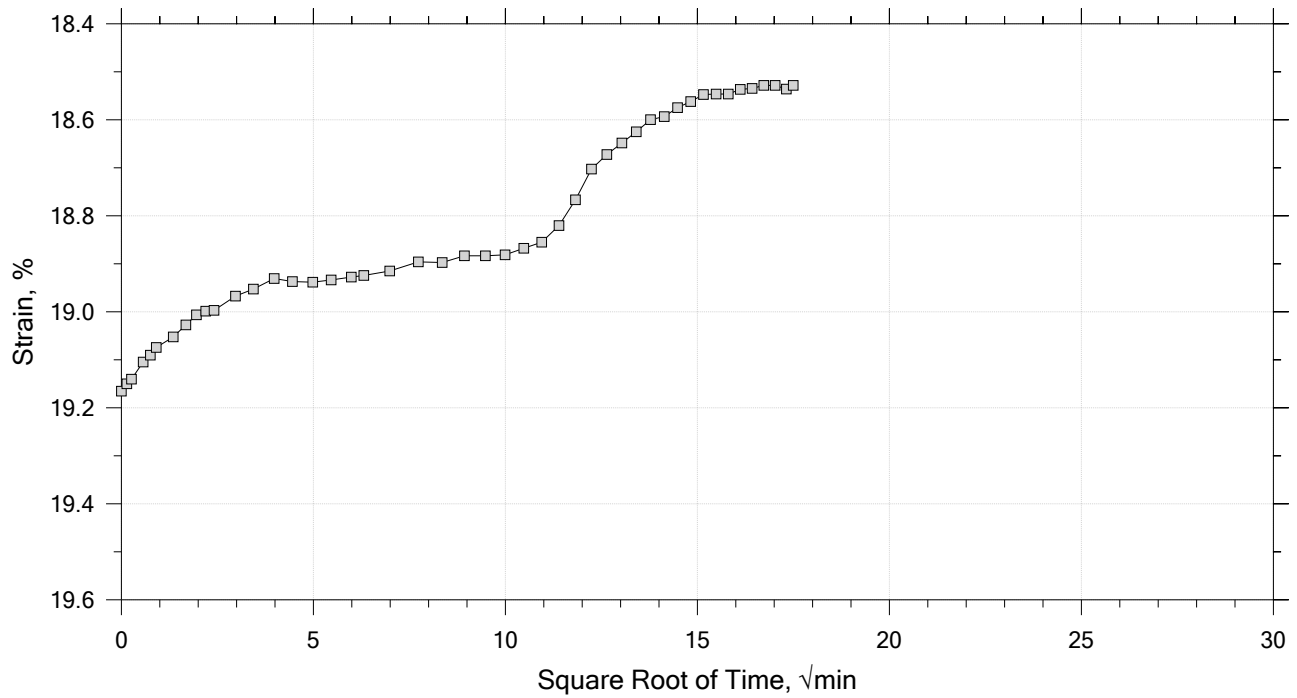
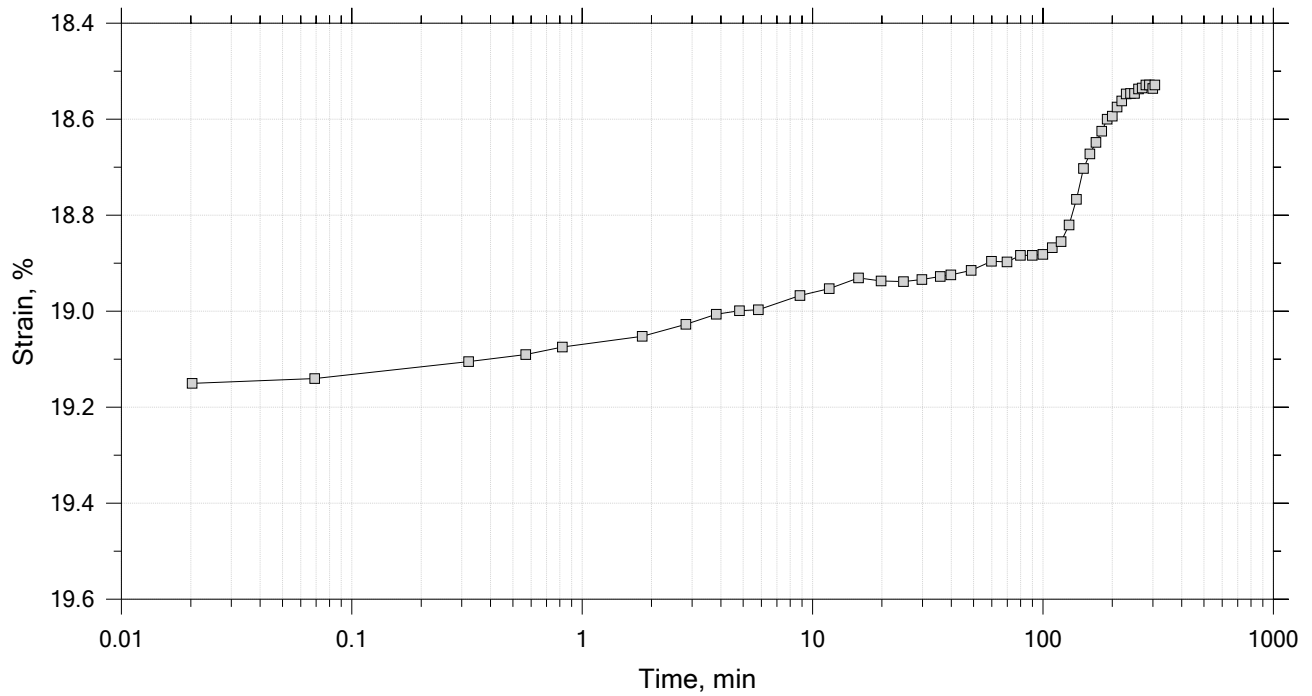
	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 32
Initial Height: 1.00 in	Initial Void Ratio: 0.994	Plastic Limit: 17
Final Height: 0.81 in	Final Void Ratio: 0.615	Plasticity Index: 15

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E 2058	RING		E-0613
Mass Container, gm	8.24	110.31	110.31	8.54
Mass Container + Wet Soil, gm	198.02	261.38	246.18	145.1
Mass Container + Dry Soil, gm	150.26	221.37	221.37	120.16
Mass Dry Soil, gm	142.02	111.06	111.06	111.62
Water Content, %	33.63	36.03	22.34	22.34
Void Ratio	---	0.99	0.62	---
Degree of Saturation, %	---	99.79	100.00	---
Dry Unit Weight, pcf	---	86.189	106.41	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

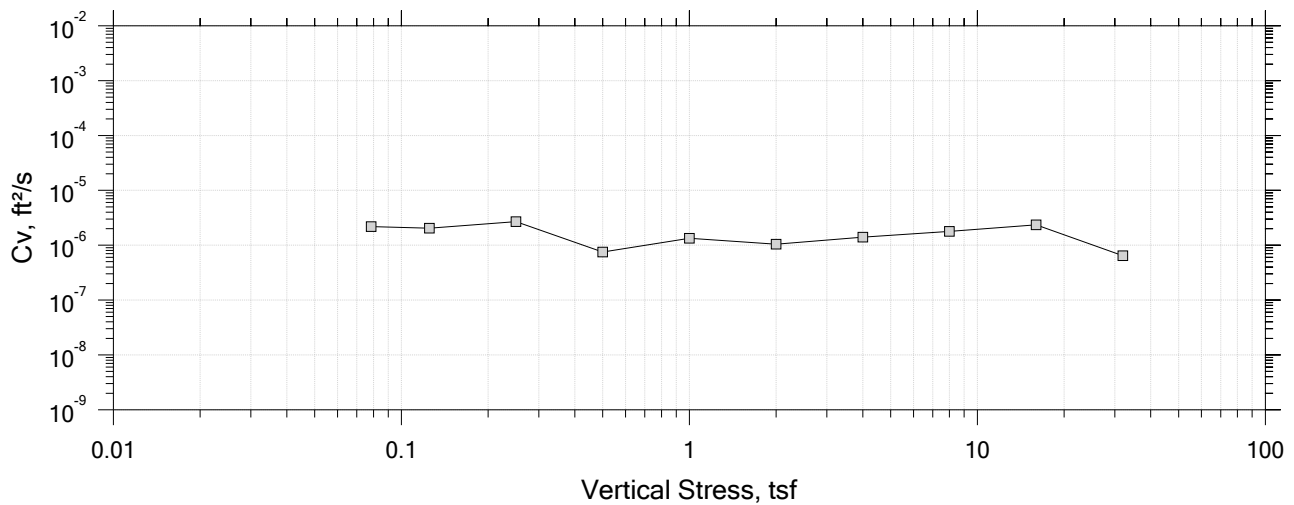
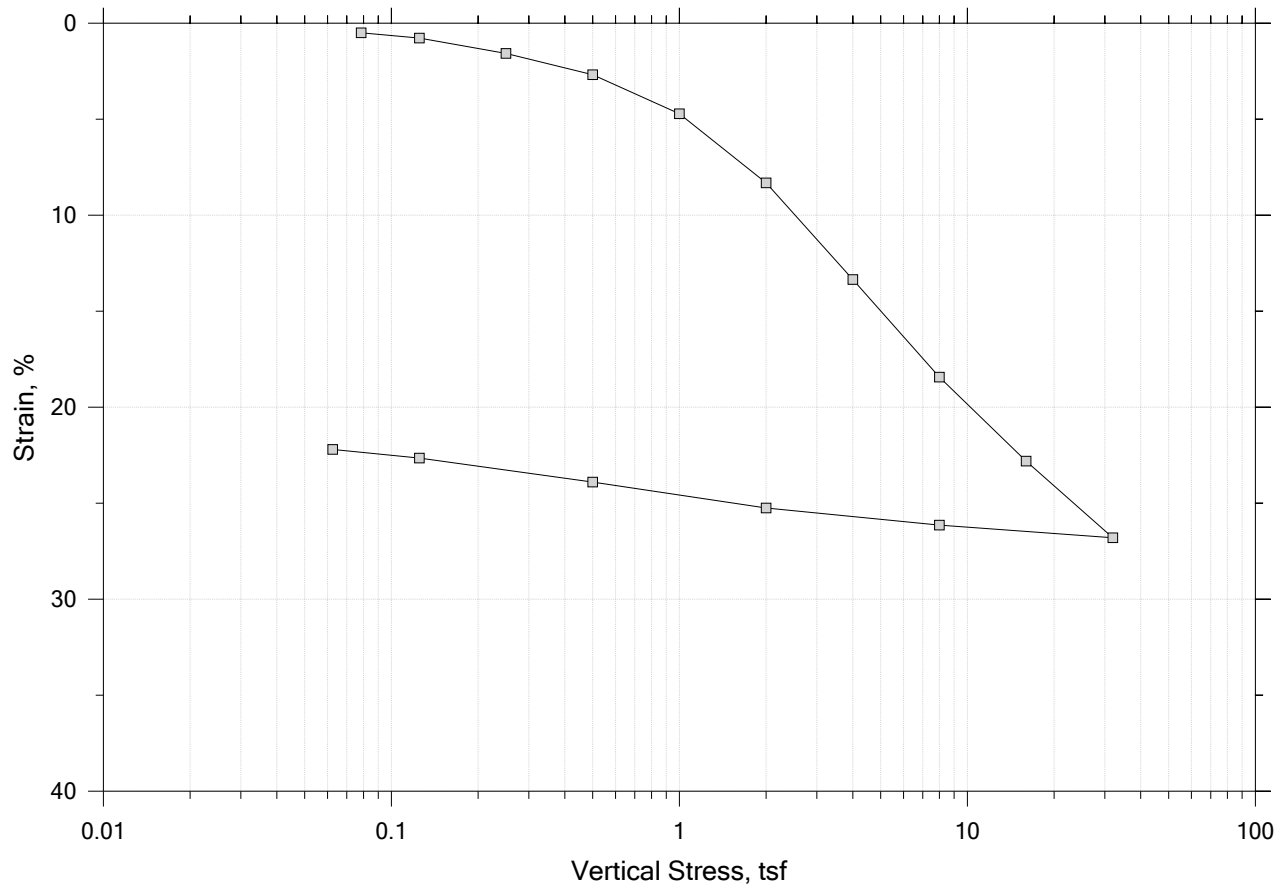
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 2)	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-202	Tested By: mp	Checked By: njh
	Sample No.: U2	Test Date: 4/26/21	Depth: 15-17
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0754 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

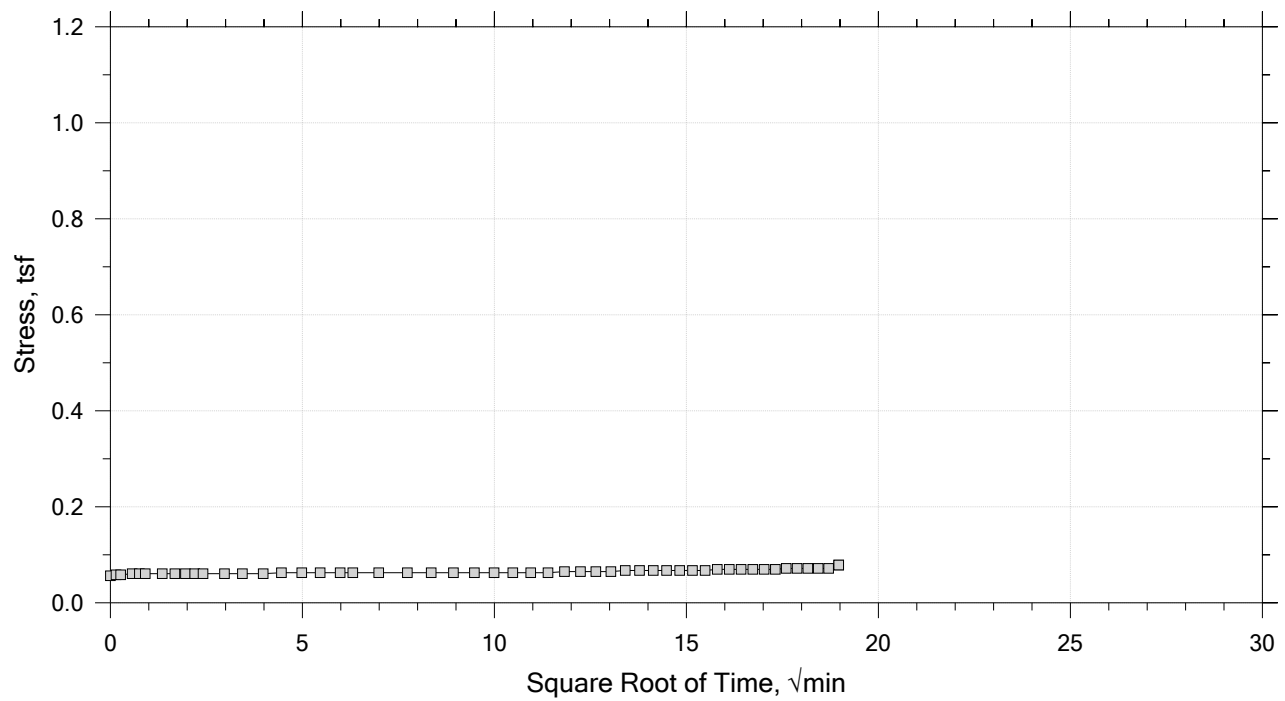
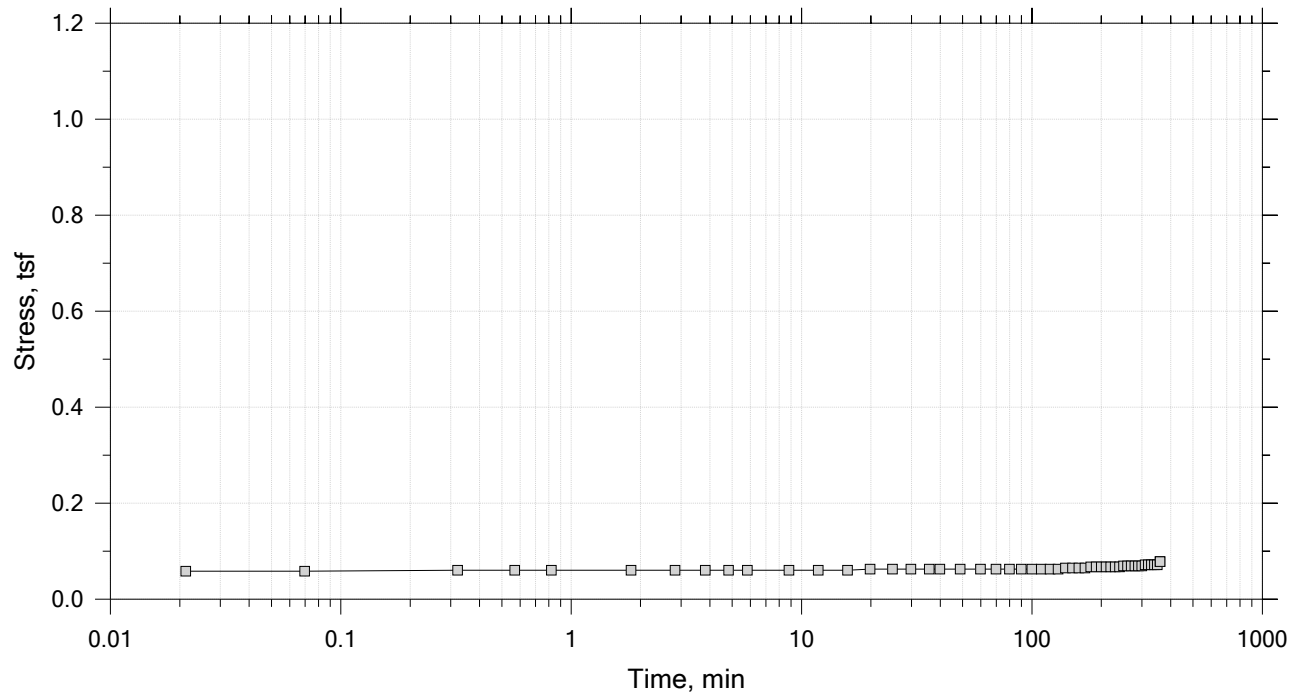
Summary Report




	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0785 tsf



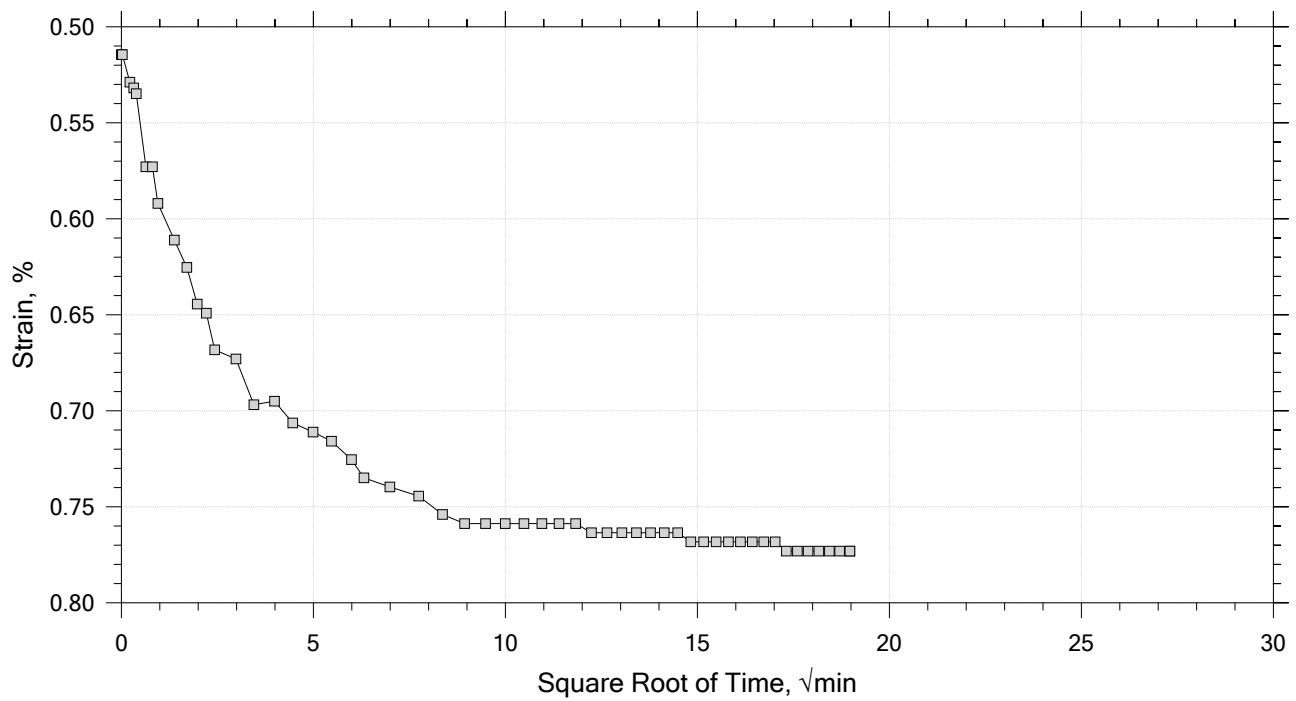
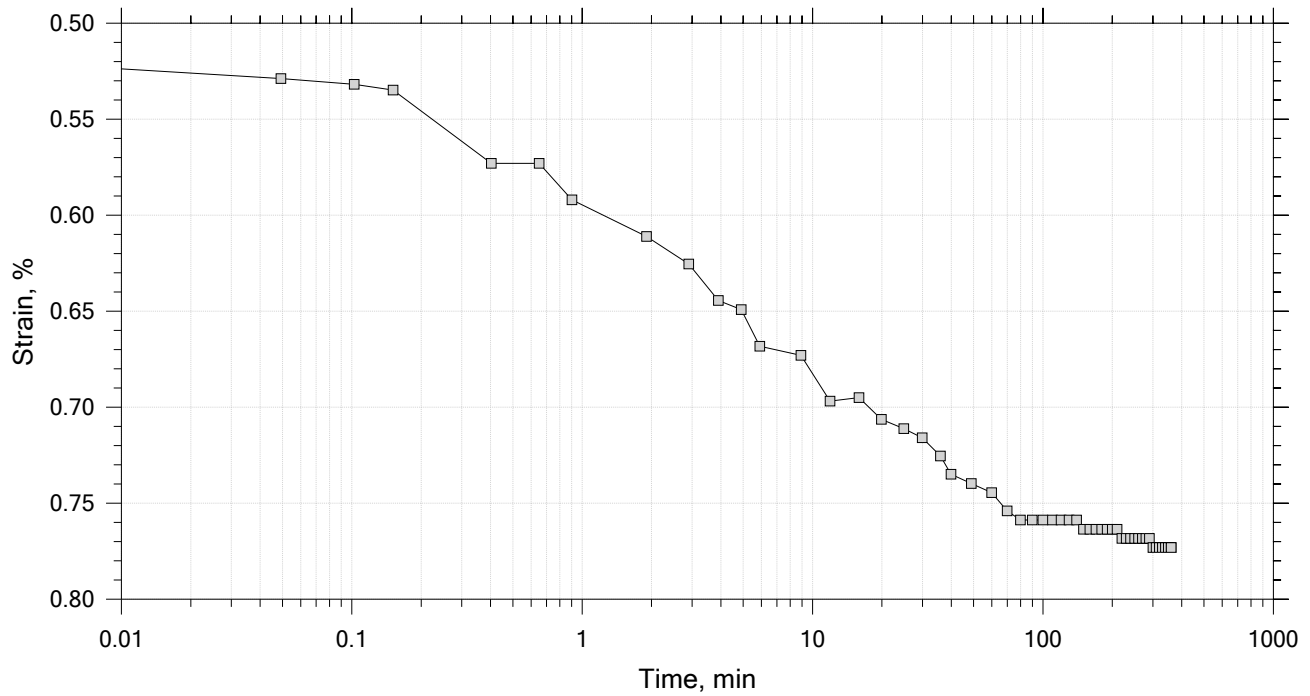
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



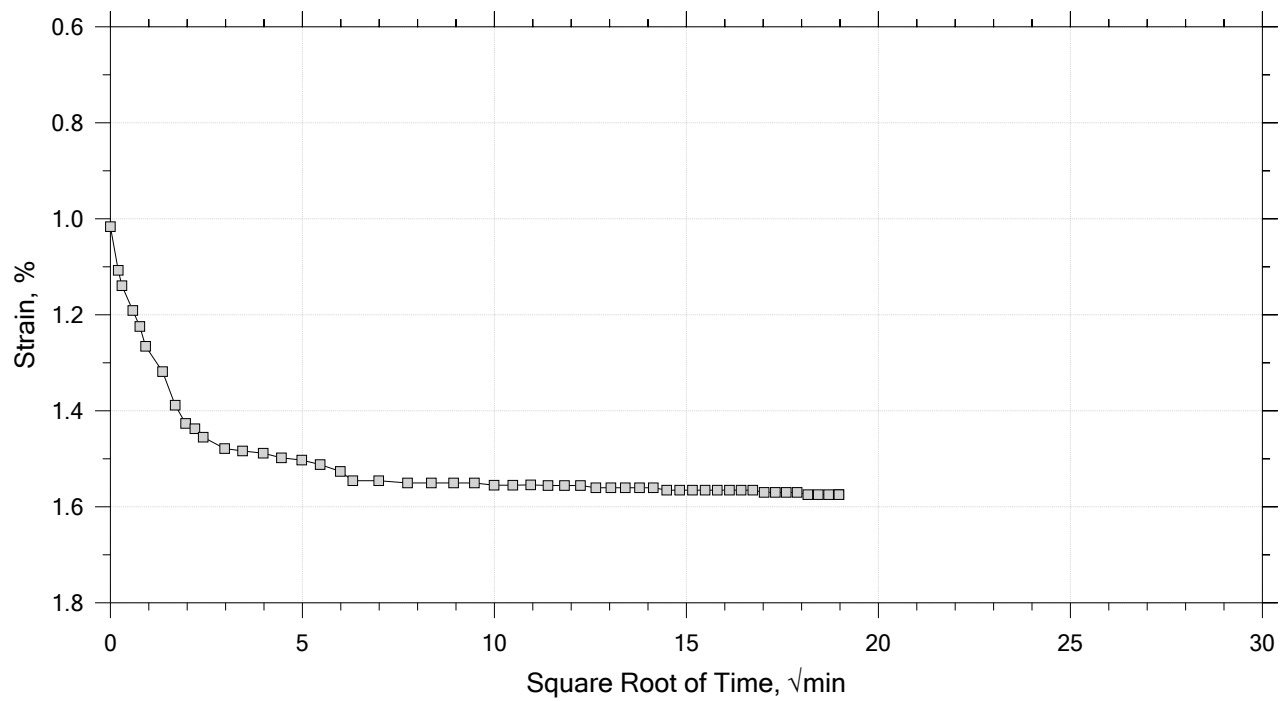
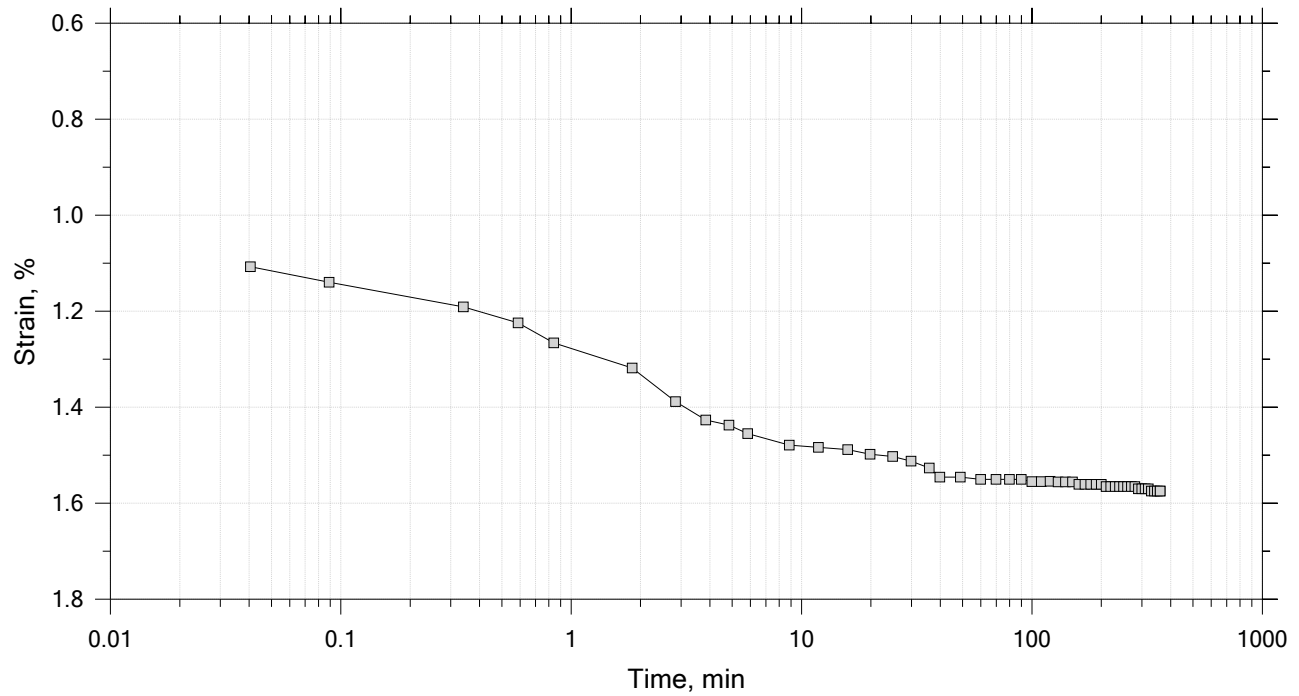
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



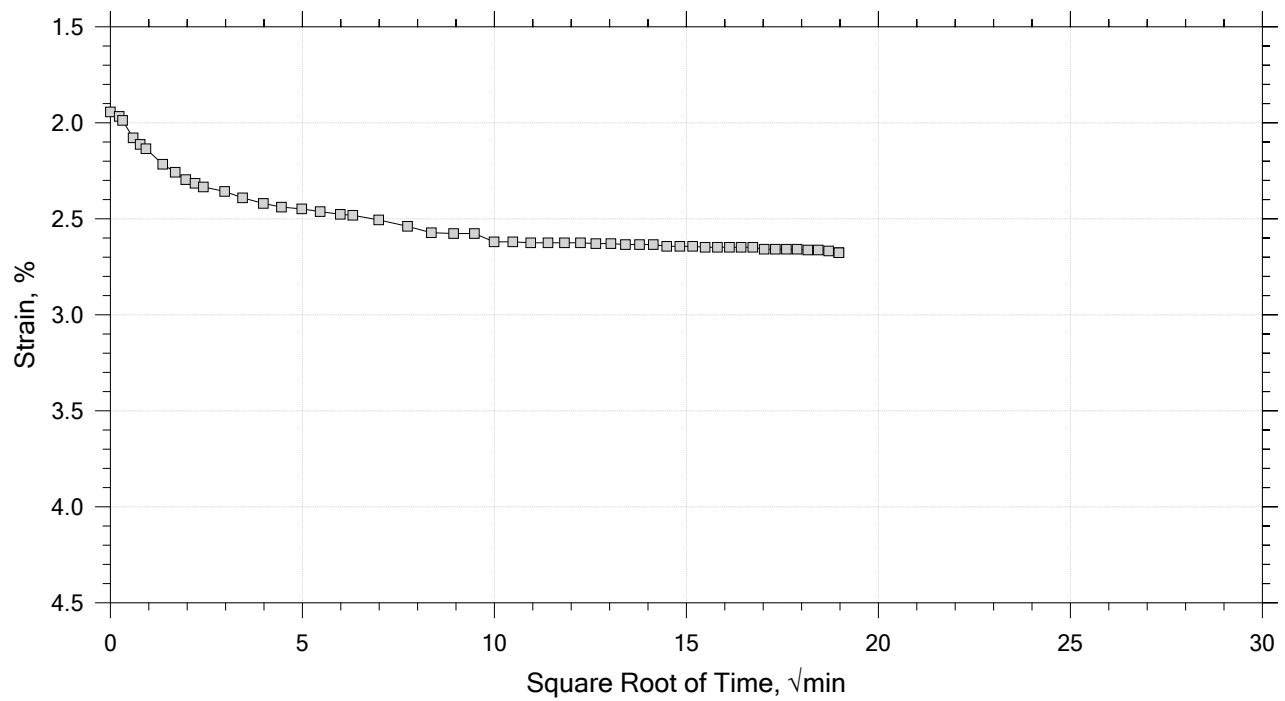
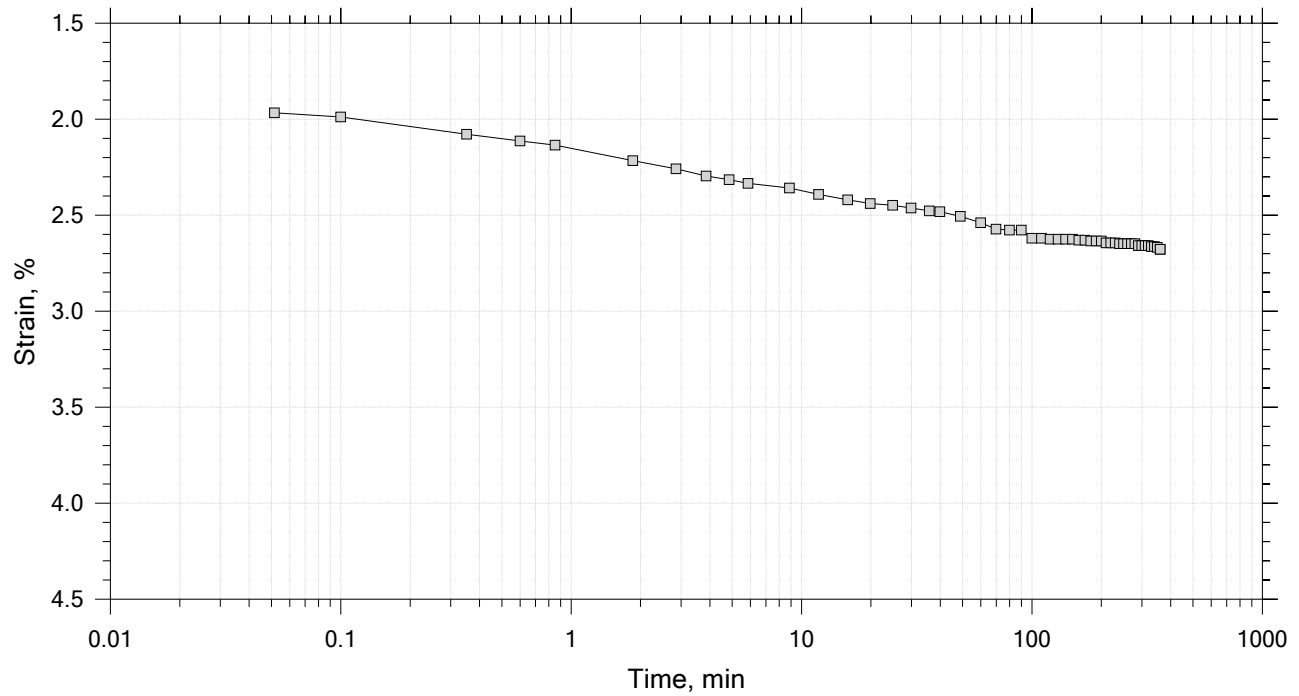
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



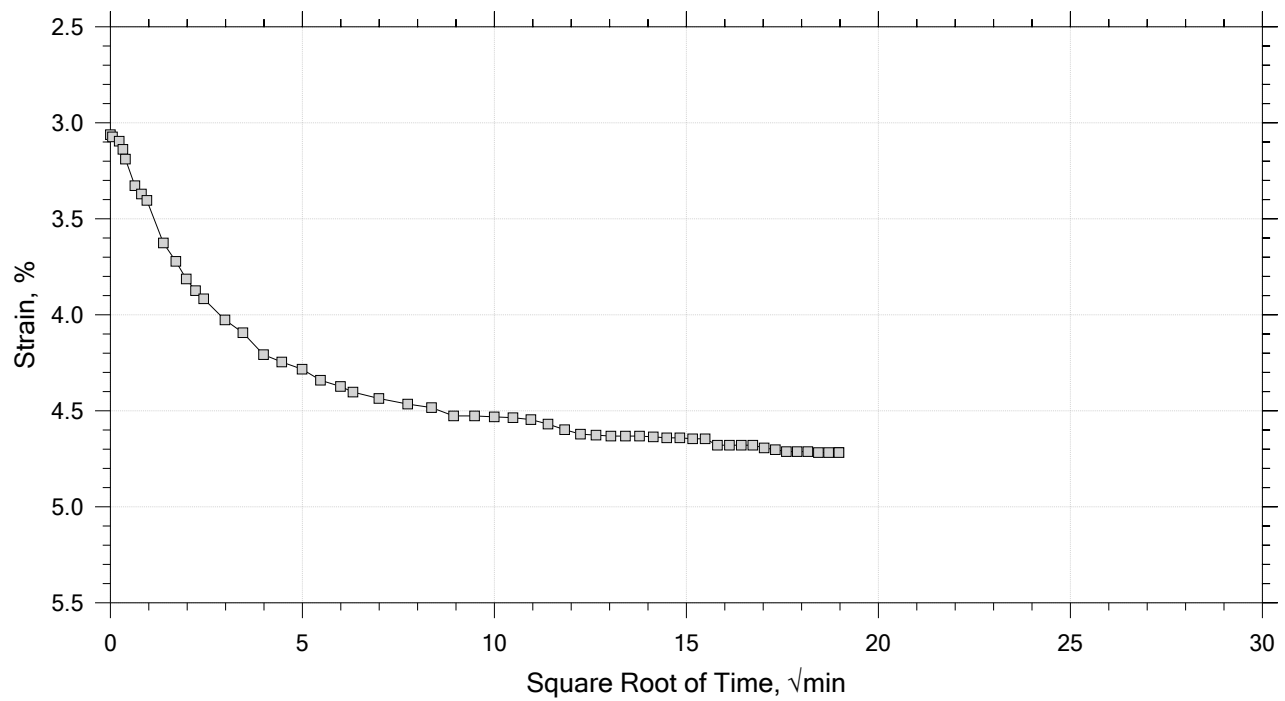
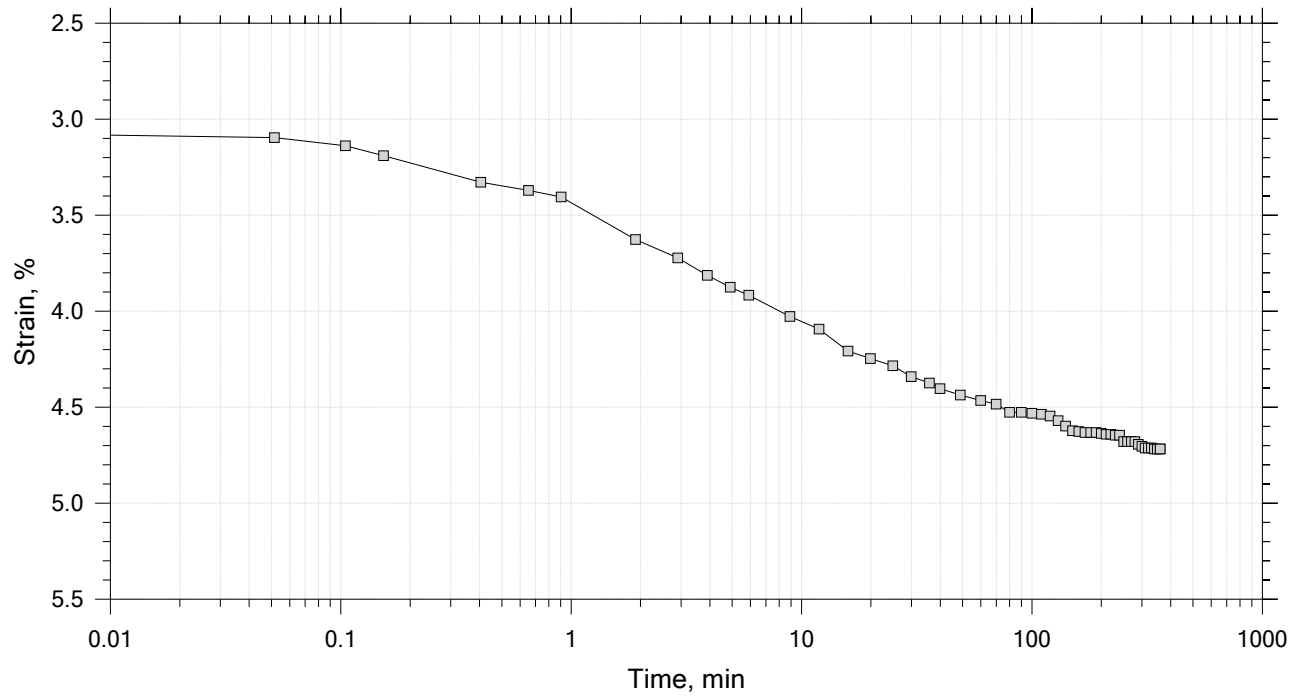
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



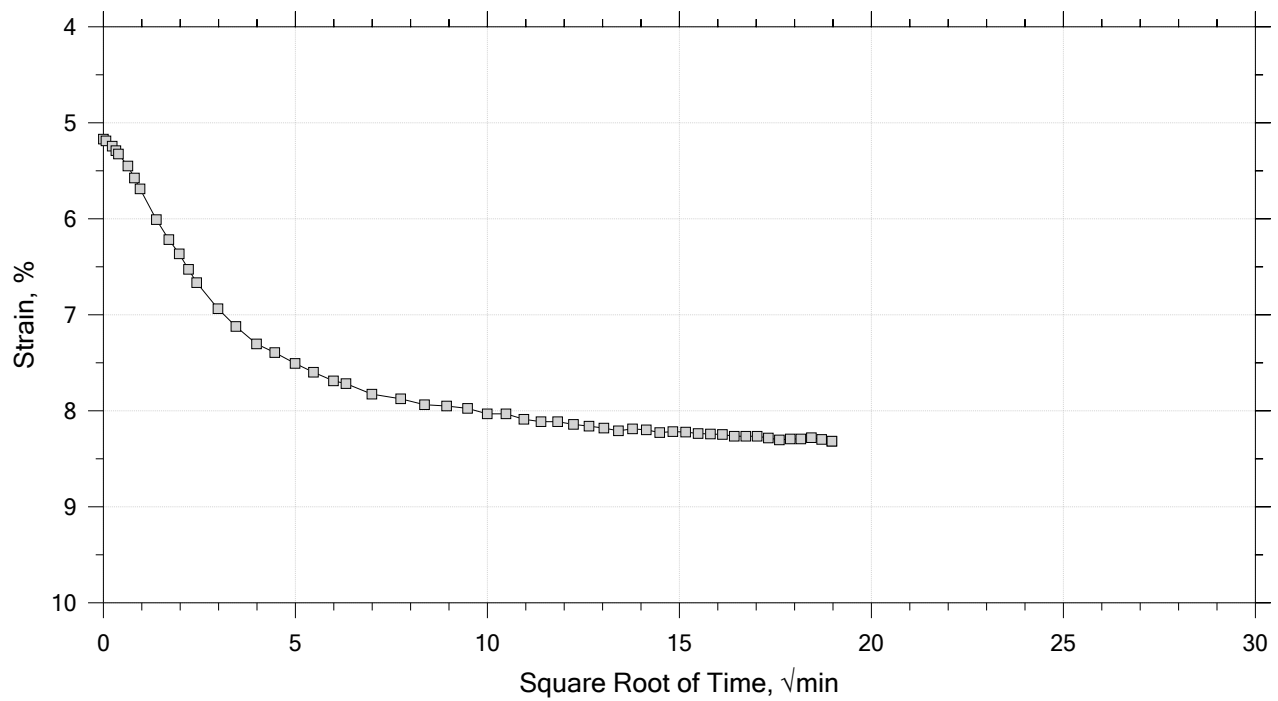
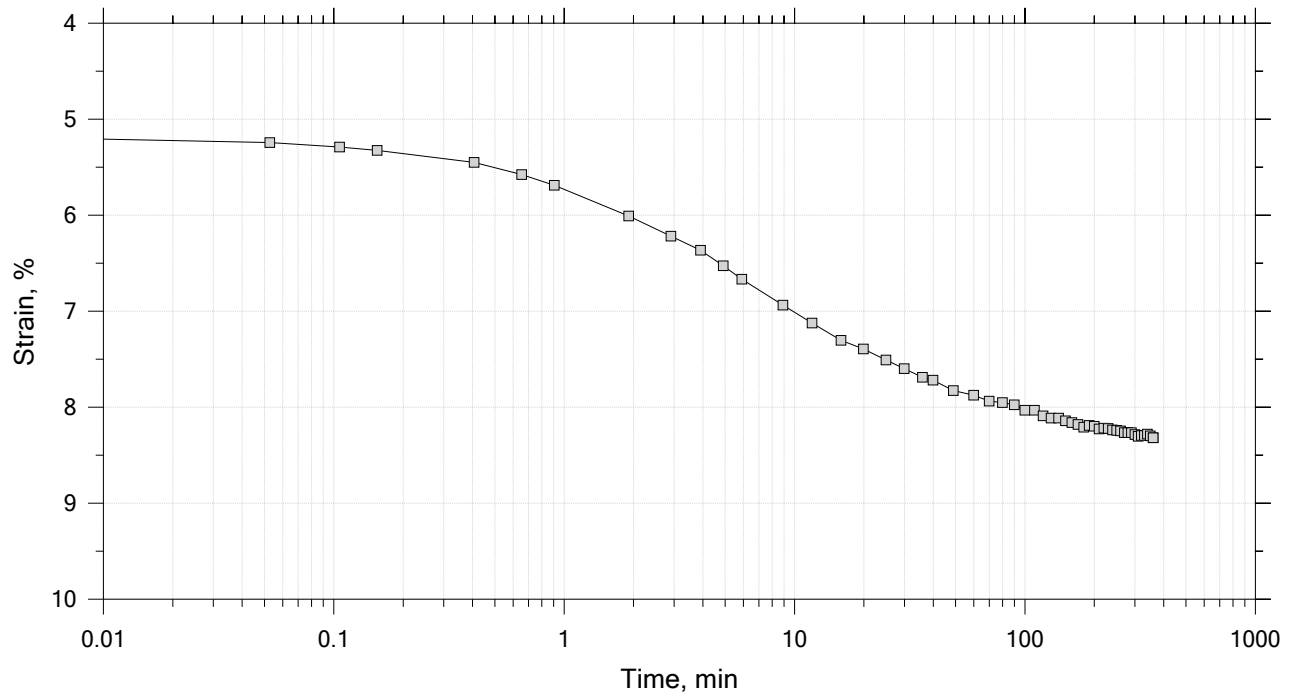
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



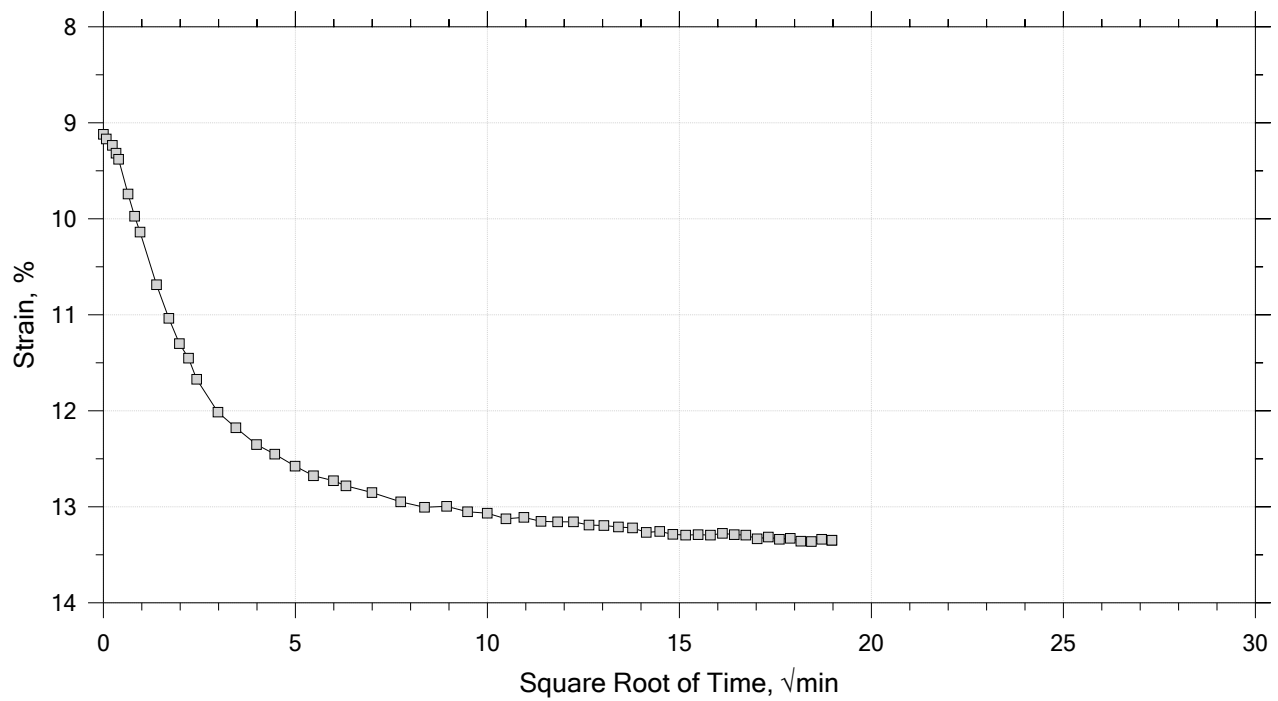
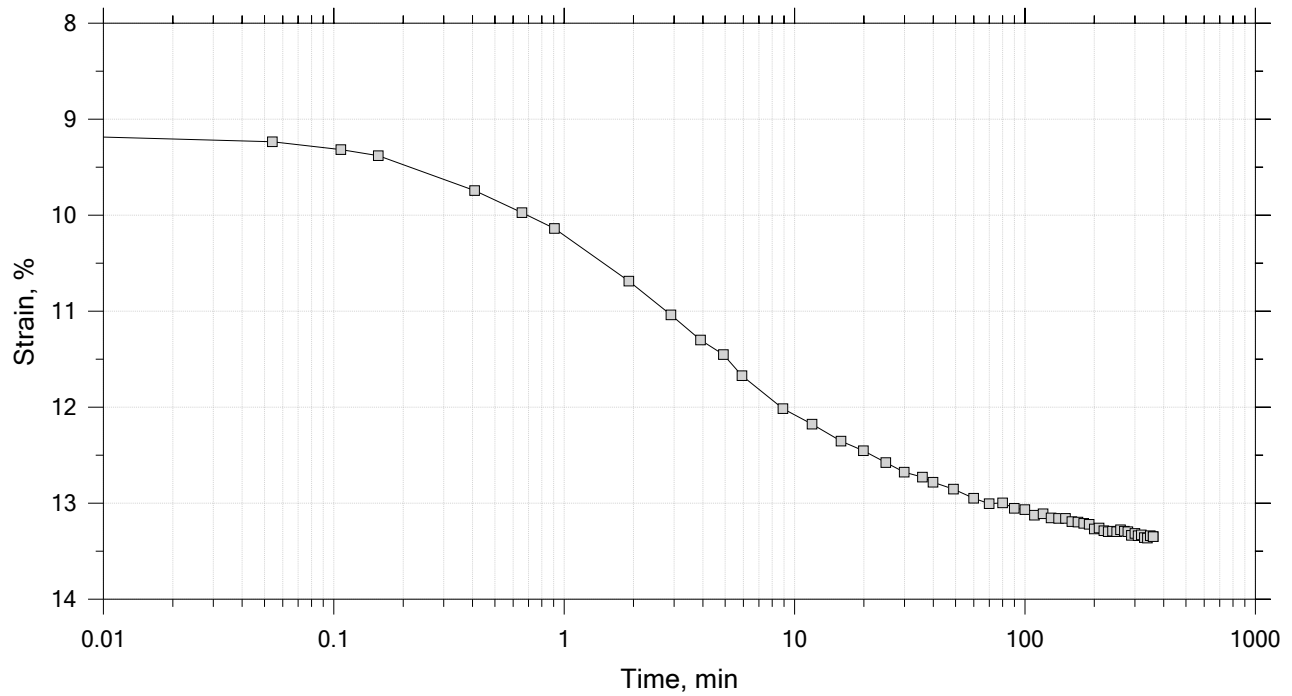
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



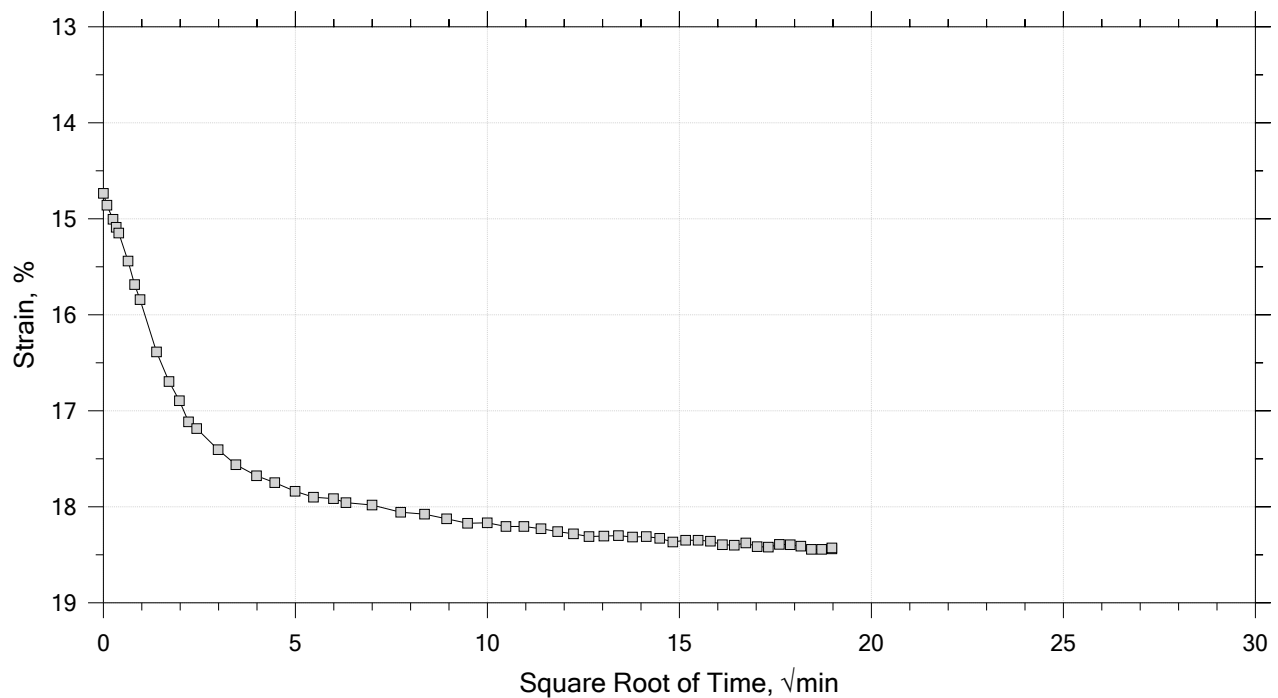
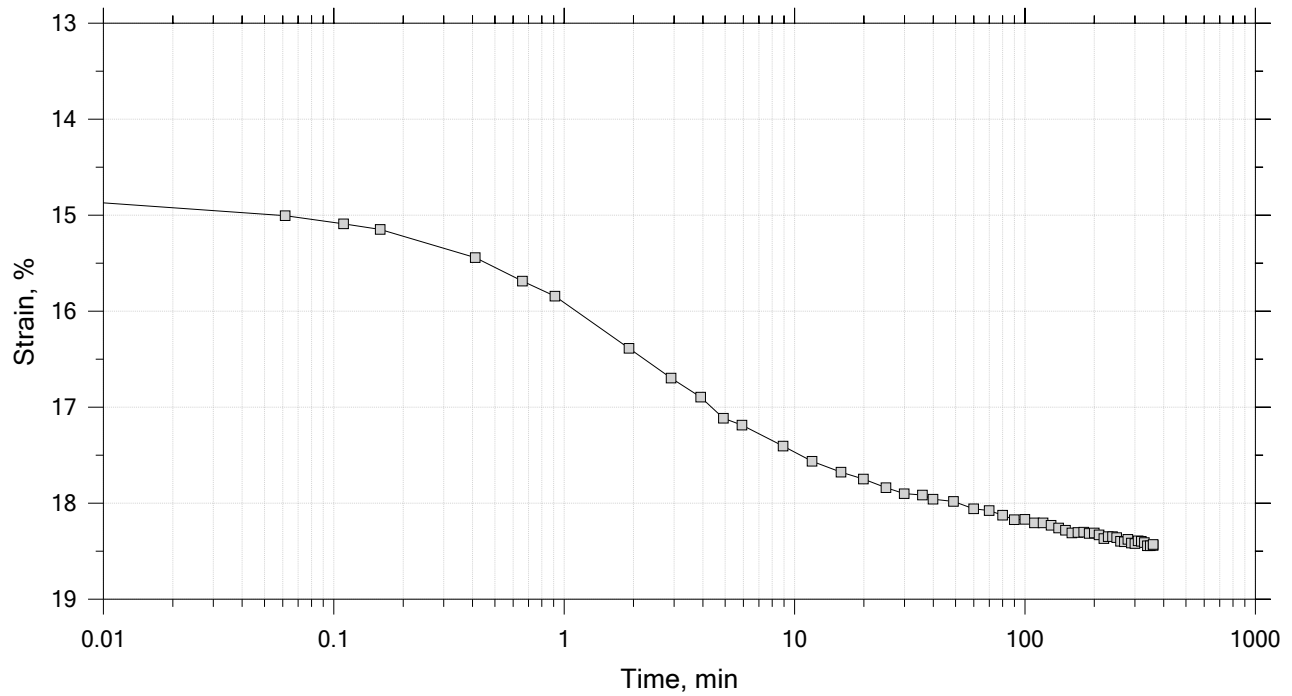
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	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



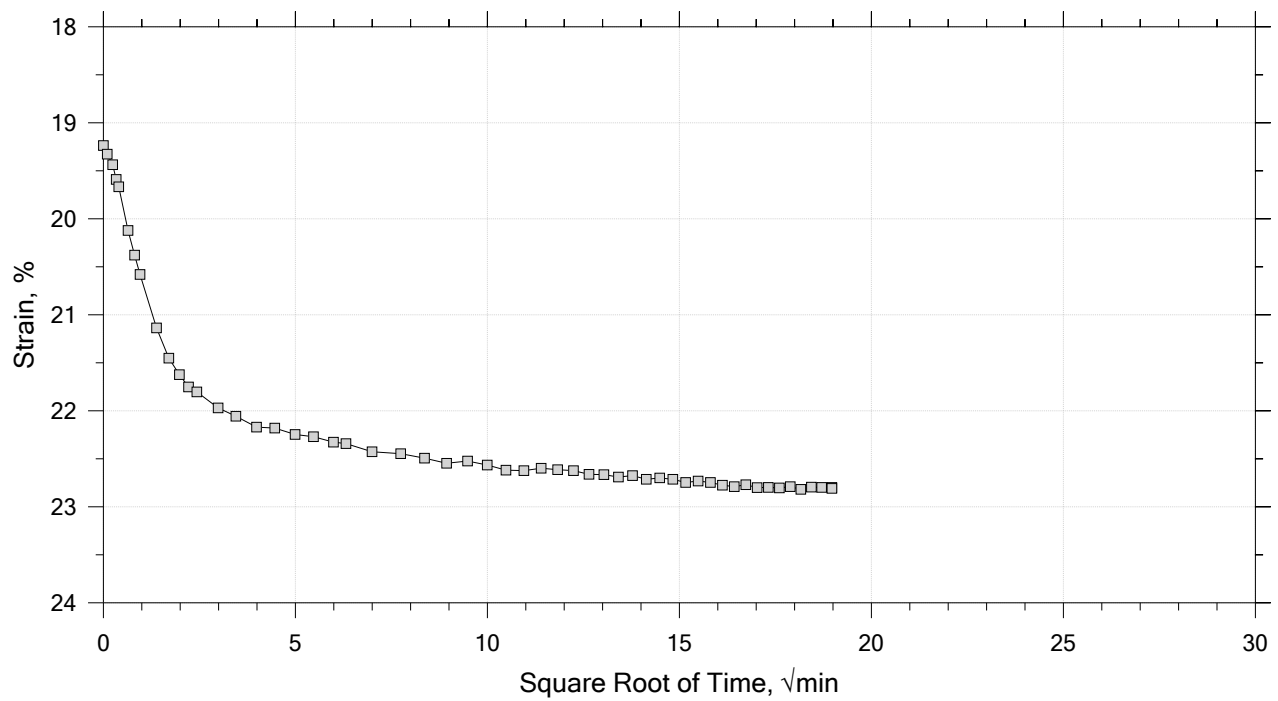
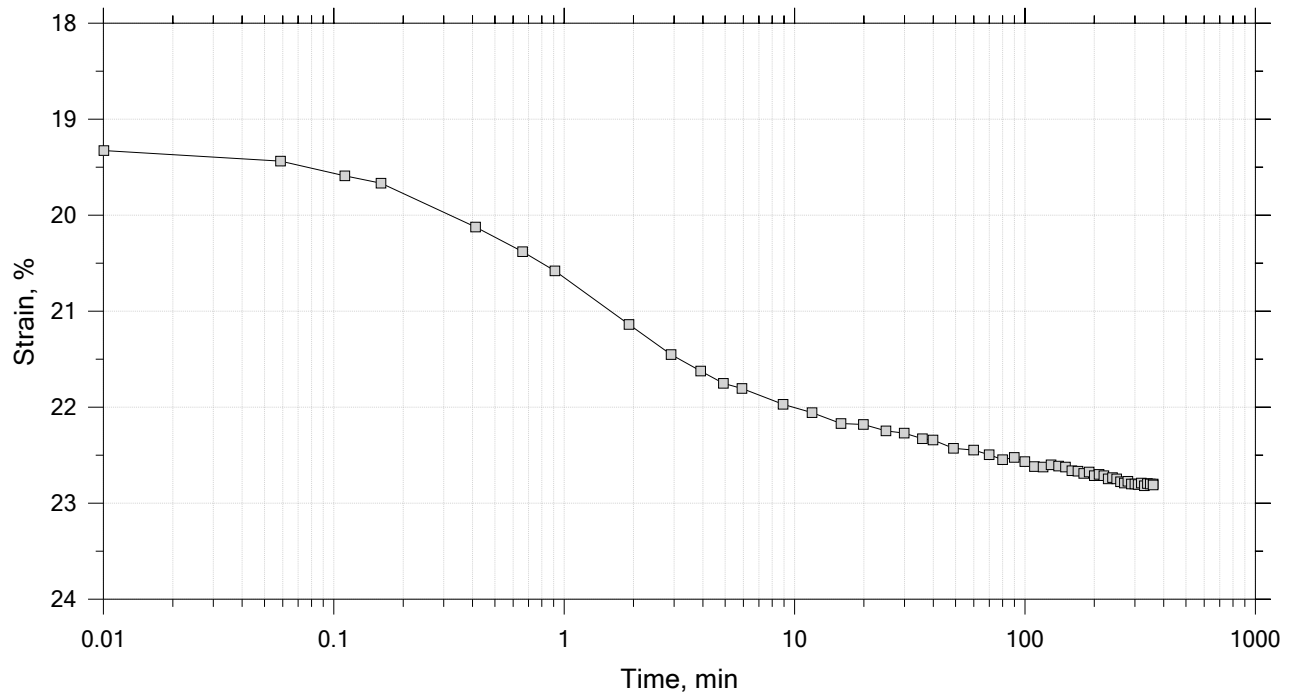
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



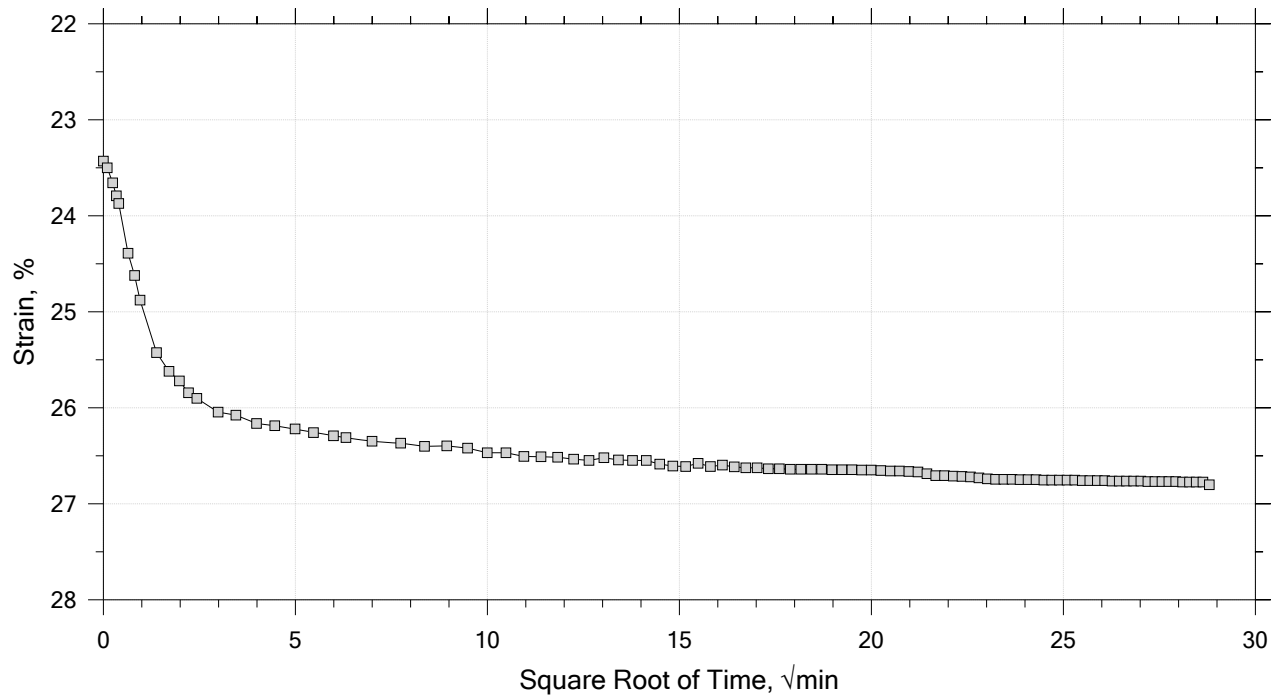
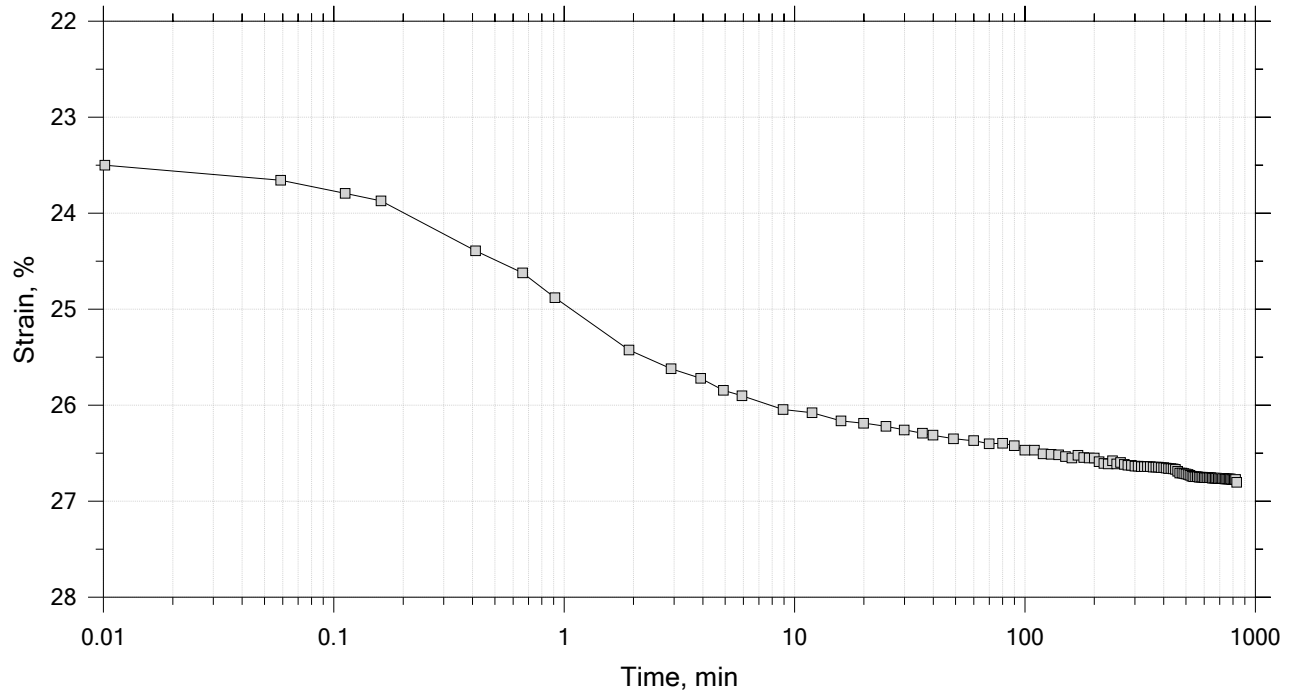
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



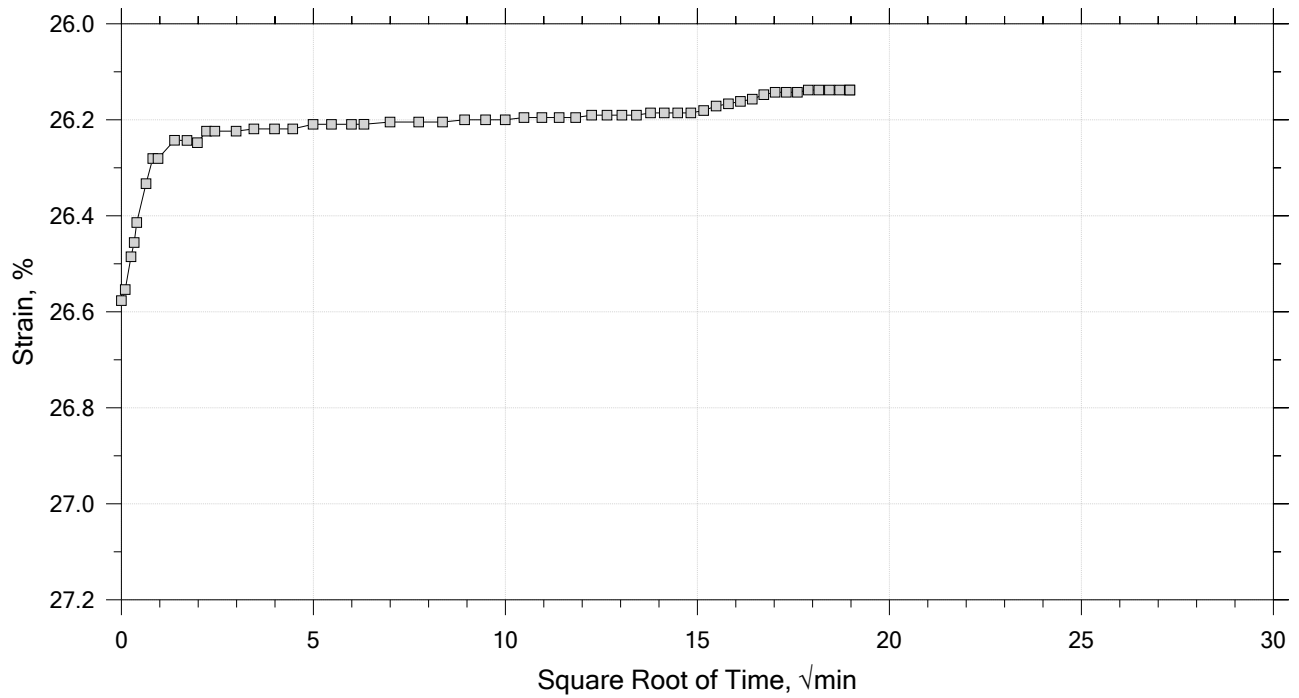
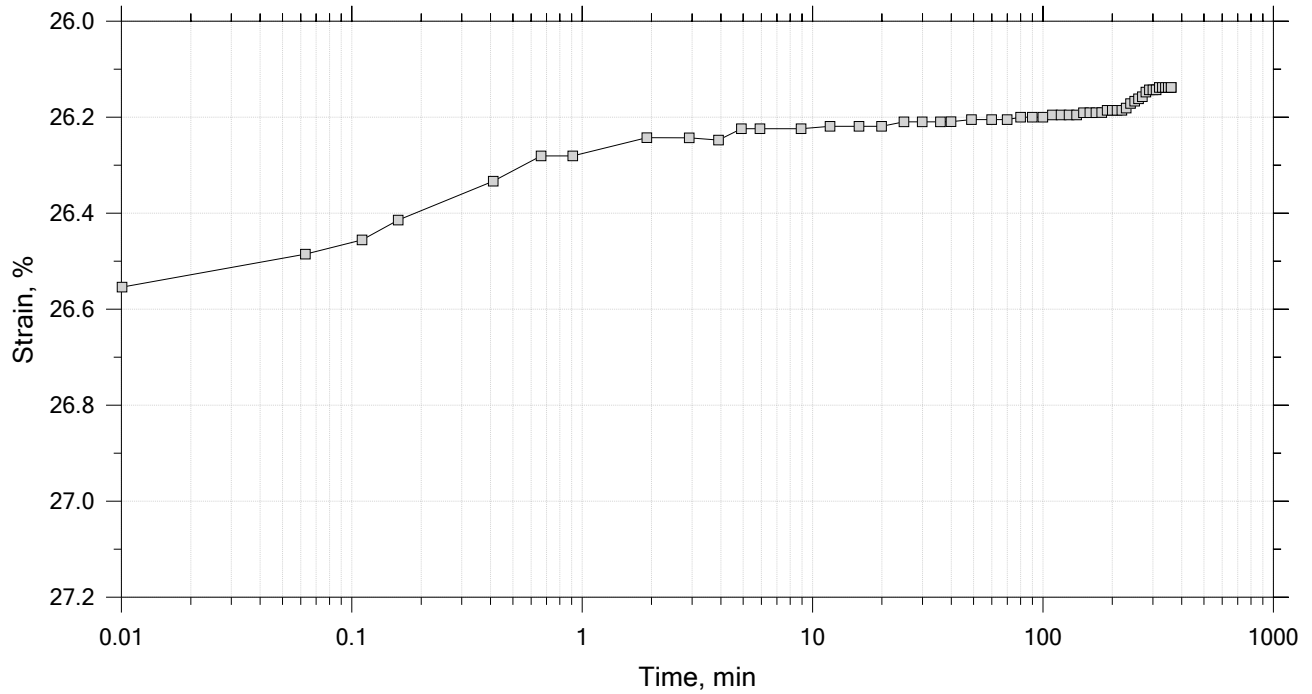
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



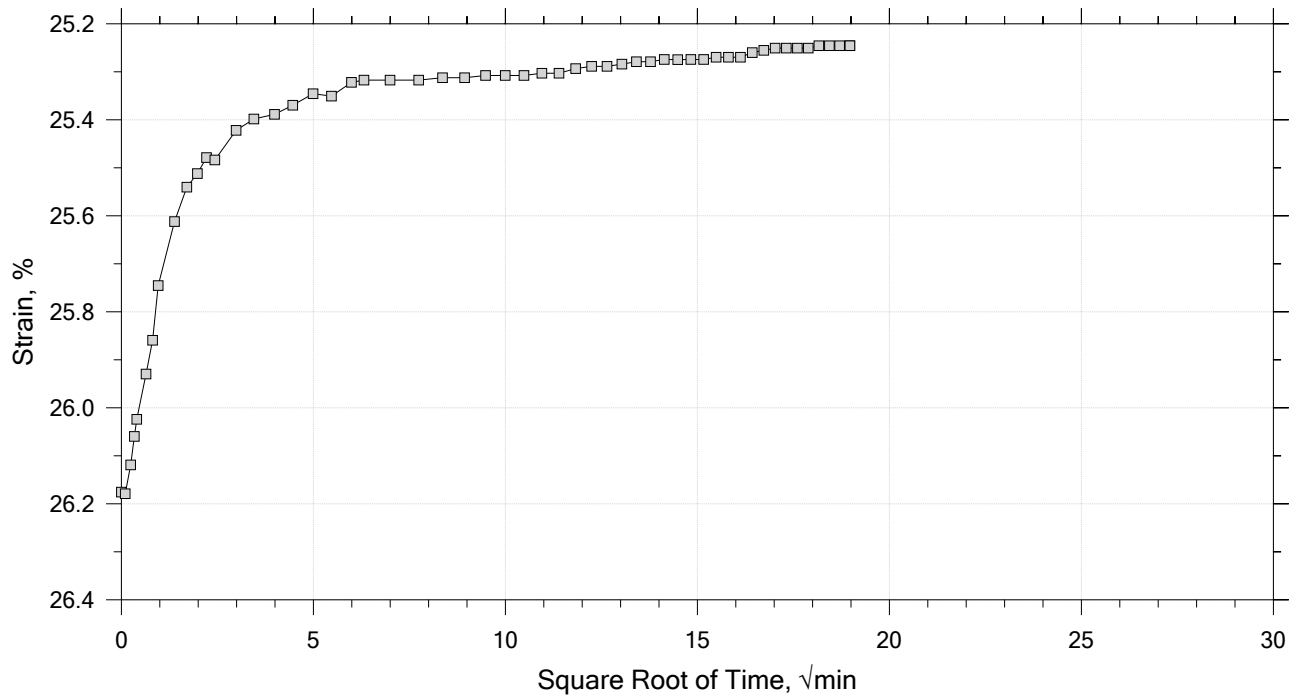
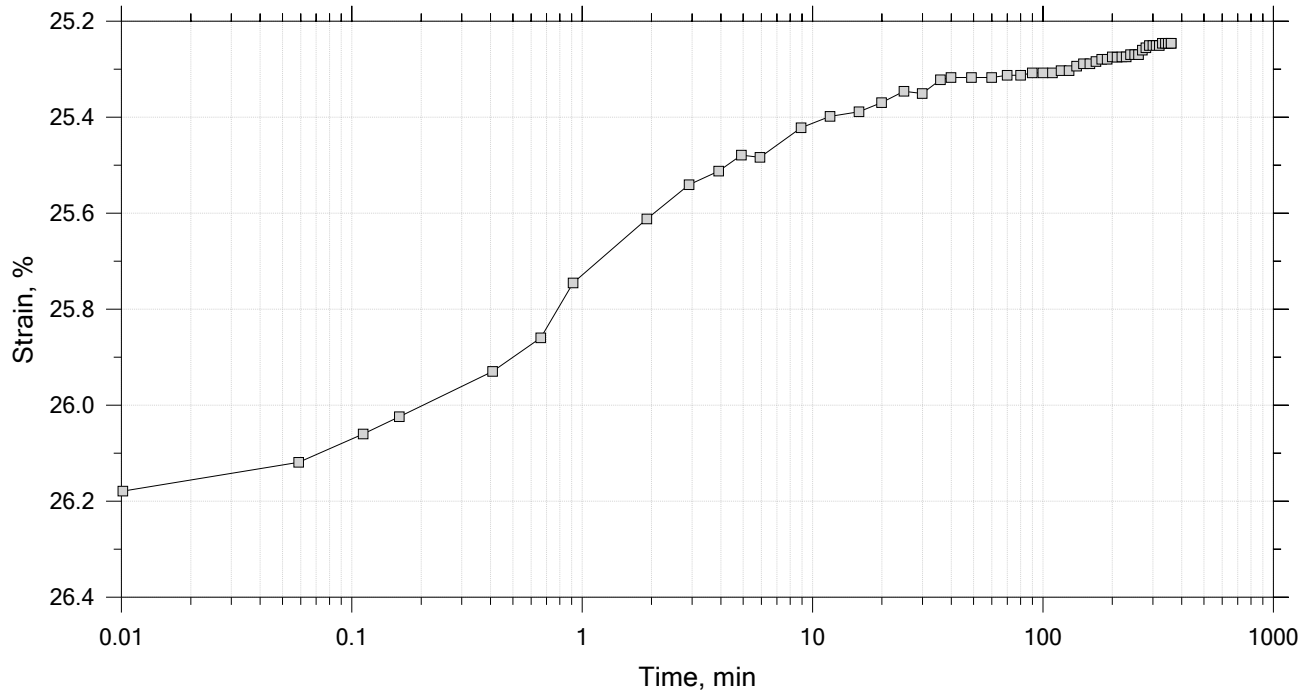
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



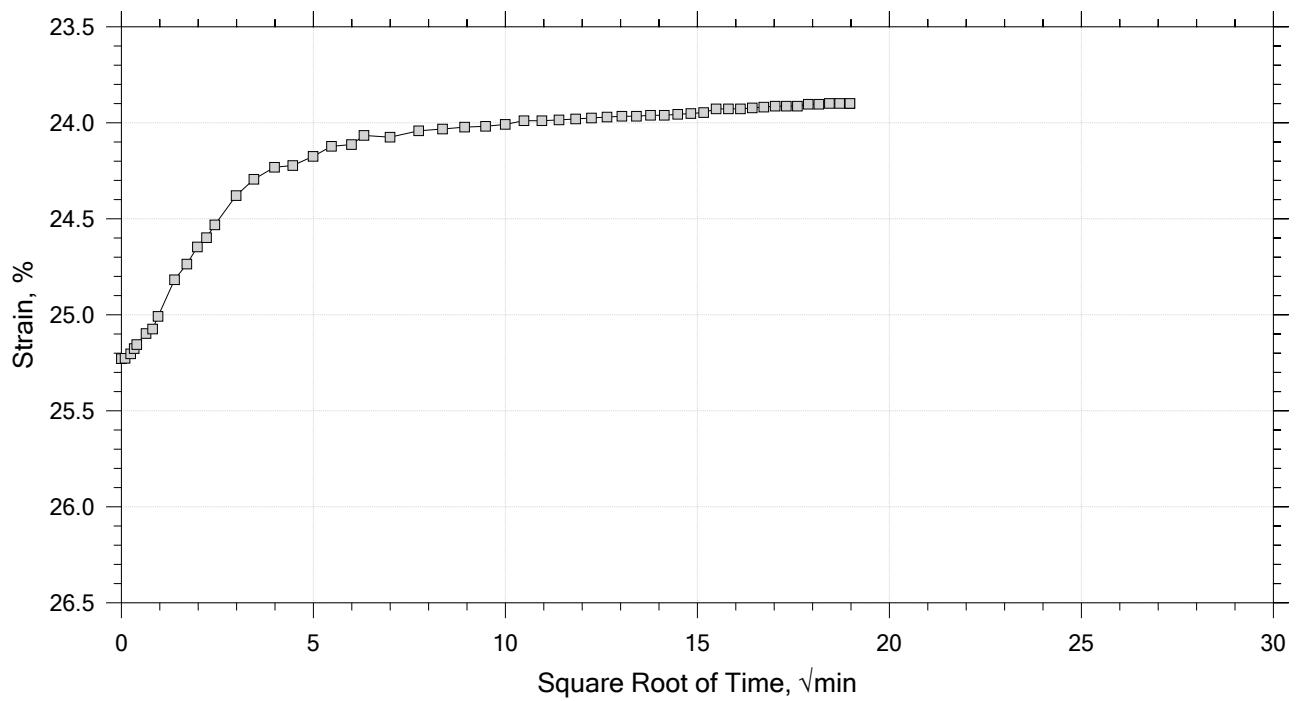
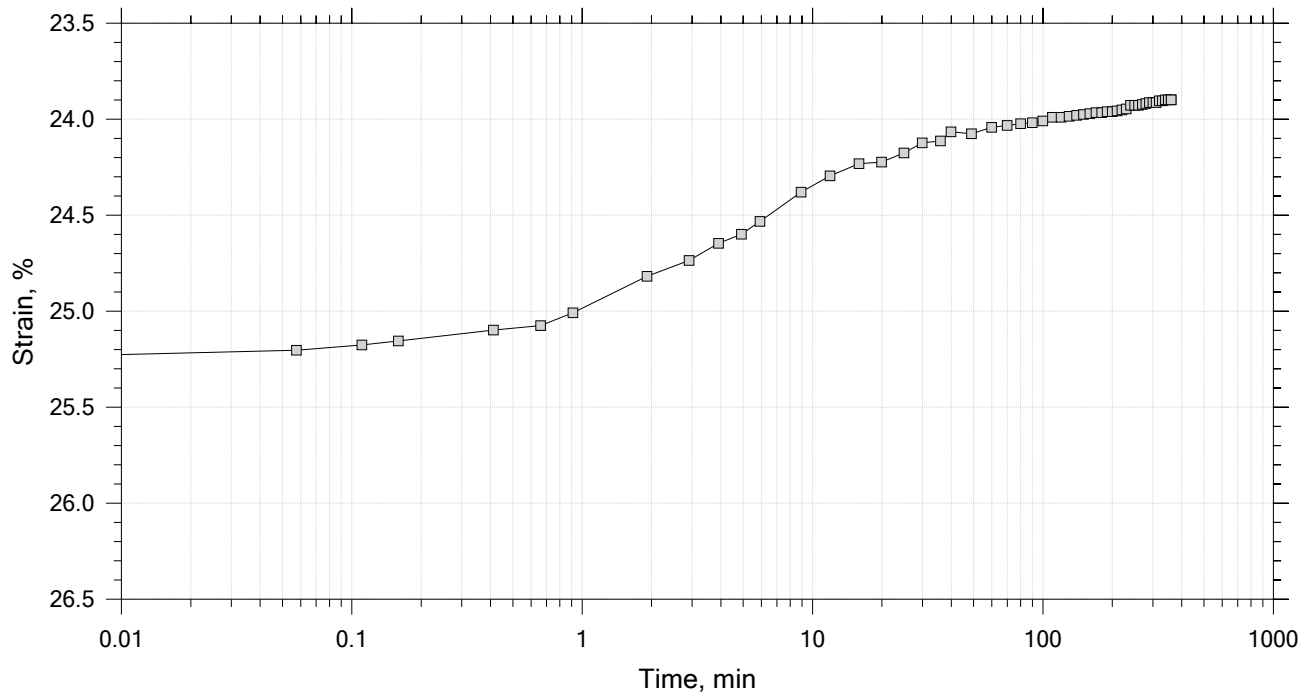
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	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



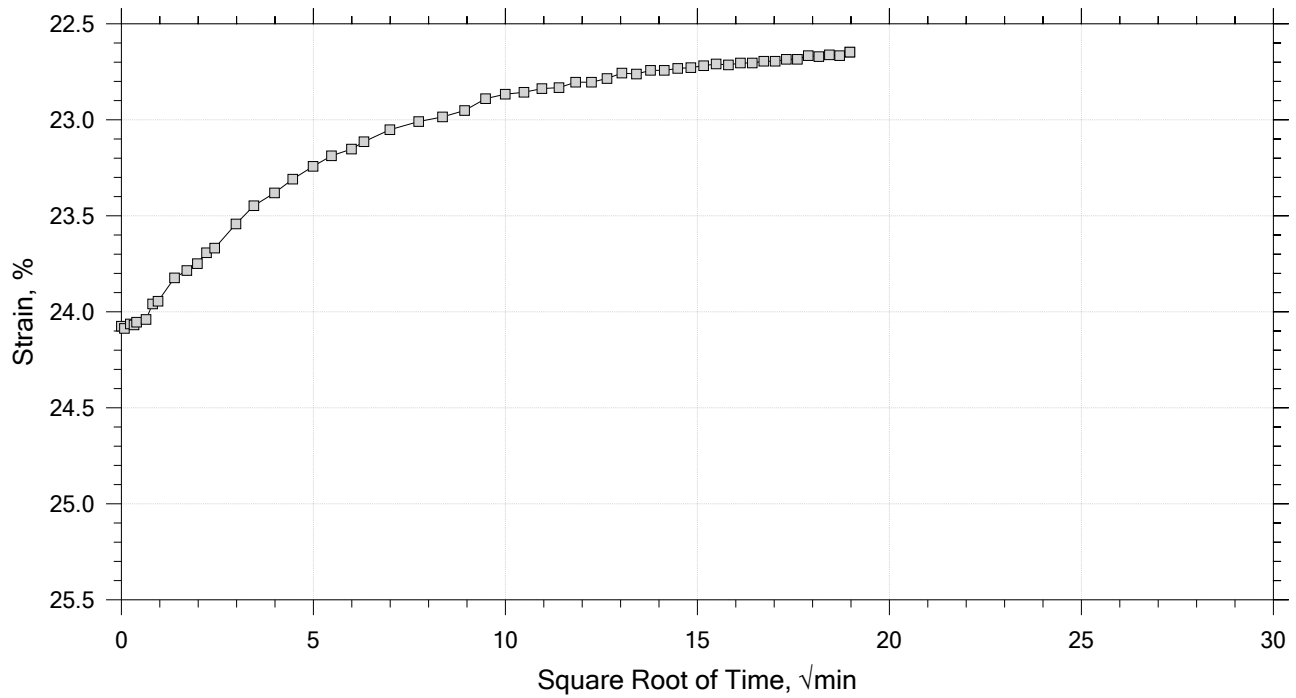
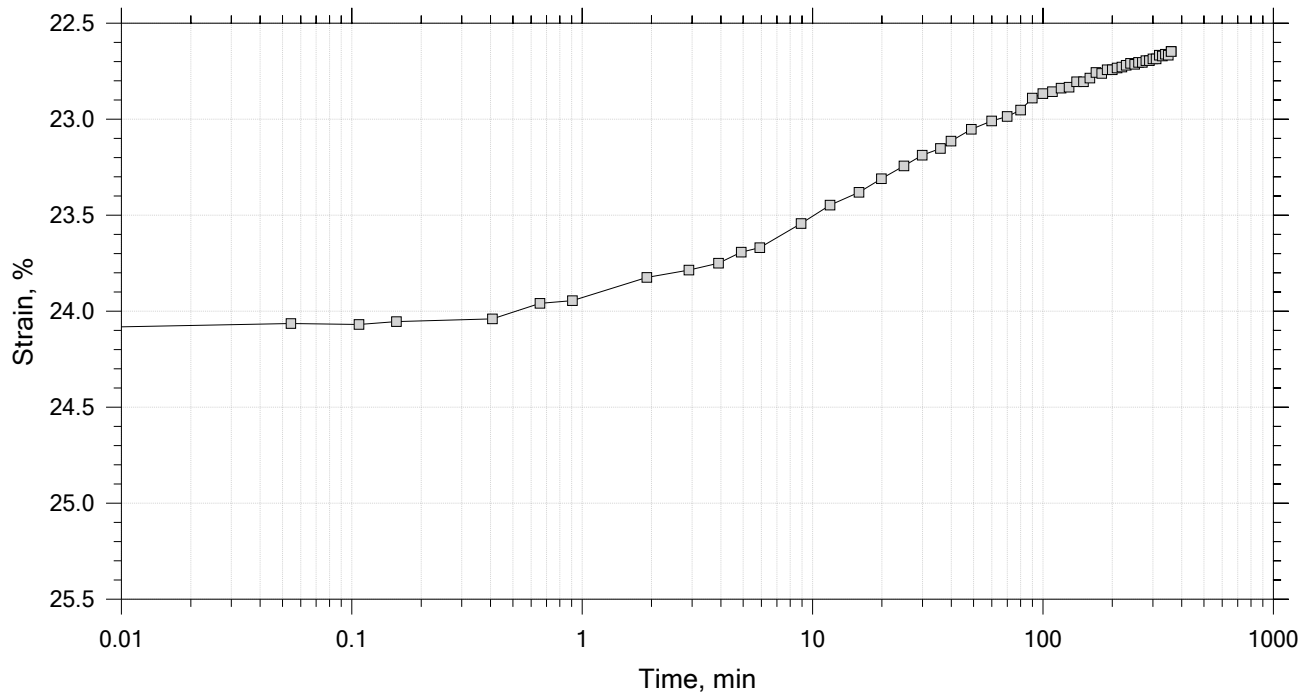
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



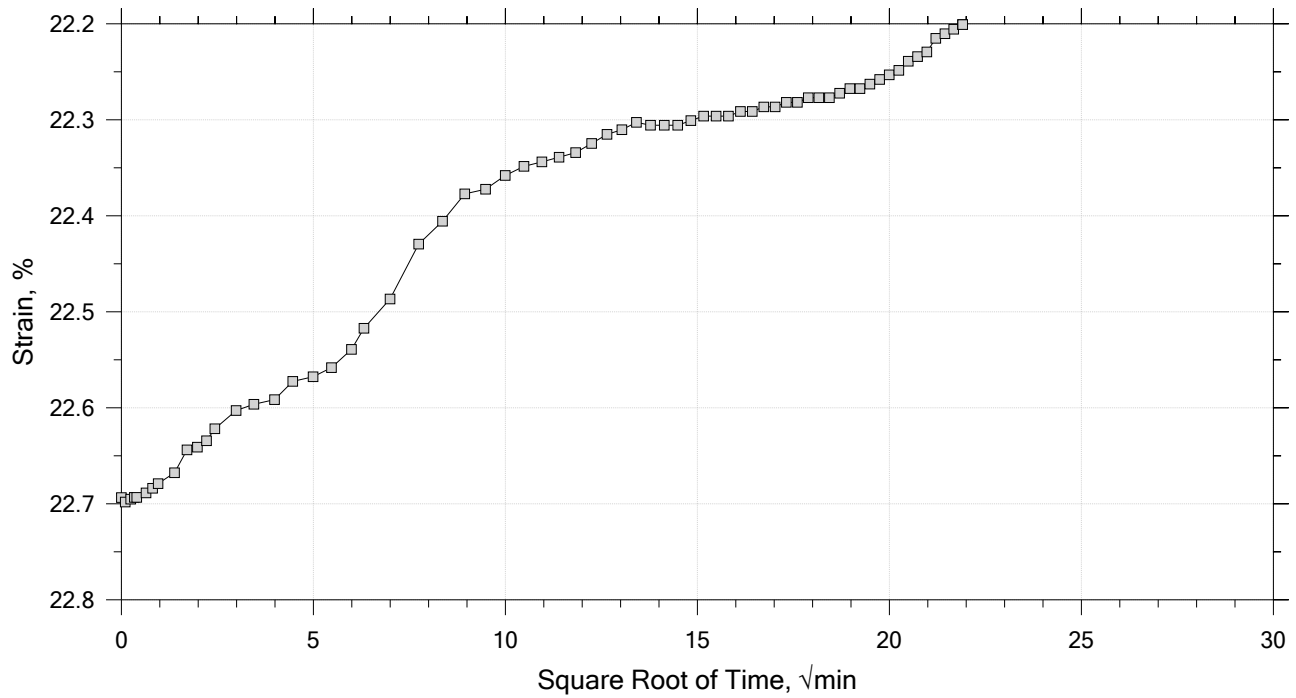
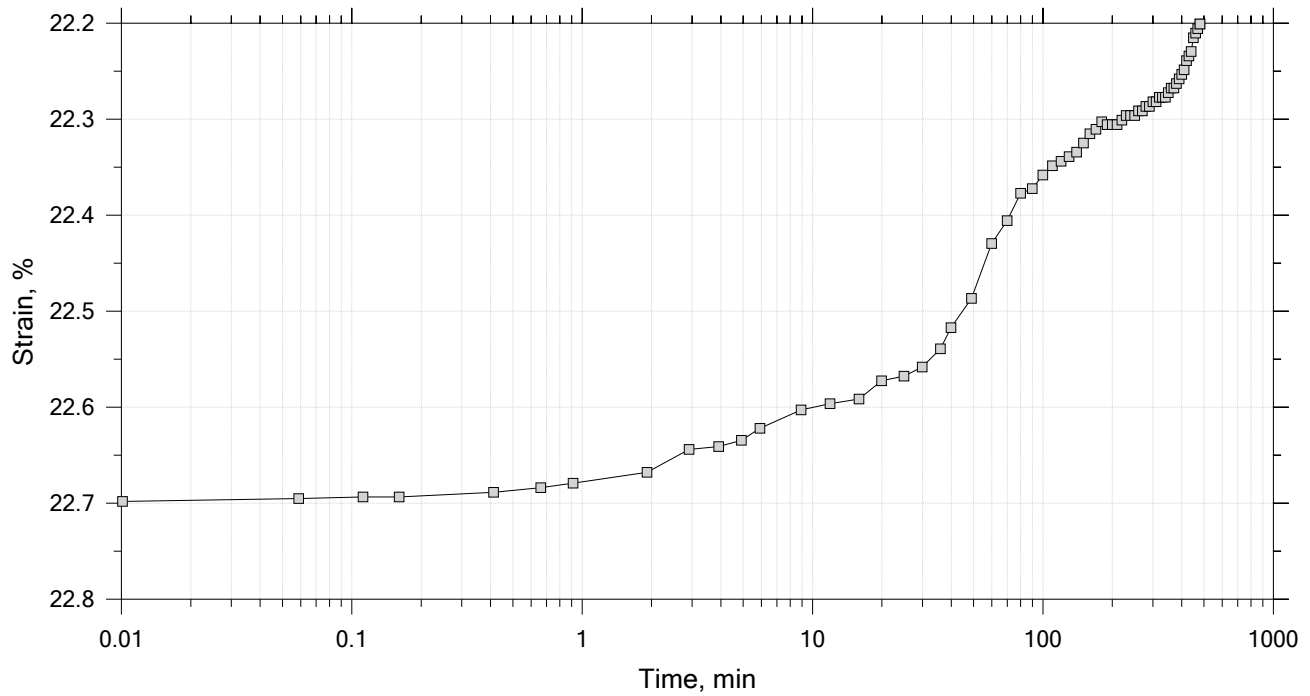
	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 34
Initial Height: 1.00 in	Initial Void Ratio: 0.981	Plastic Limit: 18
Final Height: 0.83 in	Final Void Ratio: 0.644	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E-2530	RING		D2840
Mass Container, gm	8.15	109.06	109.06	8.2
Mass Container + Wet Soil, gm	281.18	260.61	247	145.5
Mass Container + Dry Soil, gm	207.73	220.85	220.85	119.47
Mass Dry Soil, gm	199.58	111.79	111.79	111.27
Water Content, %	36.80	35.57	23.39	23.39
Void Ratio	---	0.98	0.64	---
Degree of Saturation, %	---	99.84	100.00	---
Dry Unit Weight, pcf	---	86.757	104.53	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

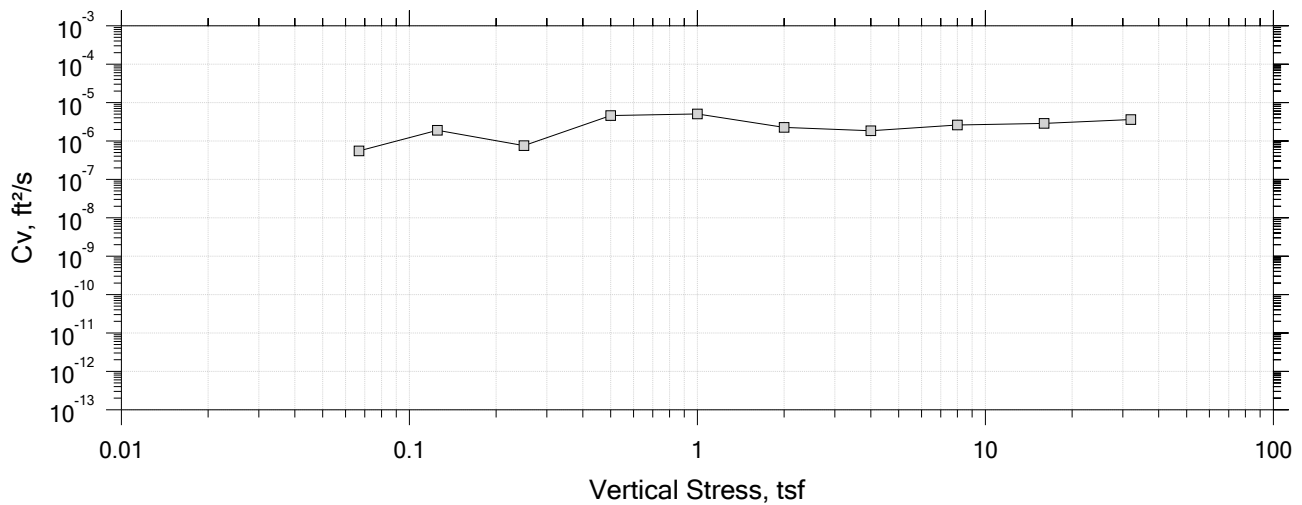
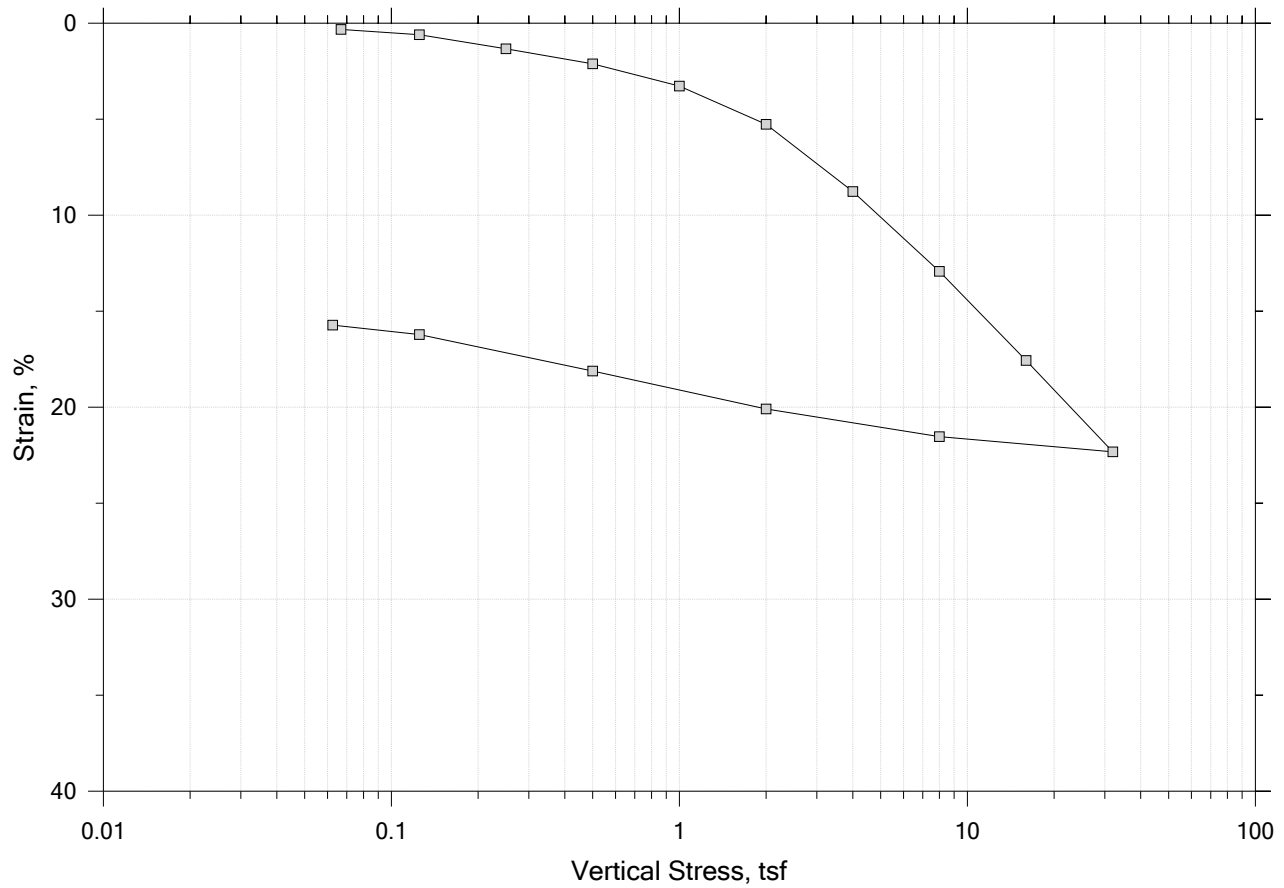
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector	Location: Brewer-Eddington, ME	Project No.: GTX-313196
	Boring No.: BB-BEB-205	Tested By: md	Checked By: anm
	Sample No.: U1	Test Date: 03/16/21	Depth: 10-12 ft
	Test No.: IP-1	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0785 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

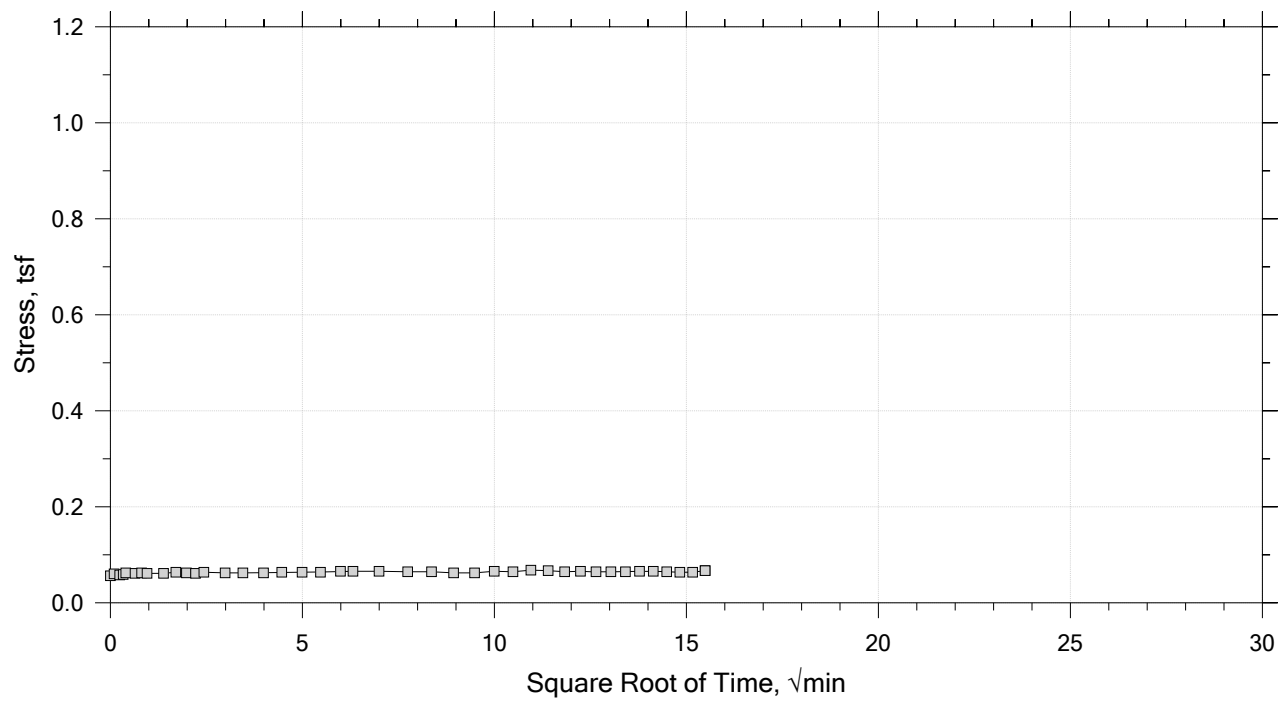
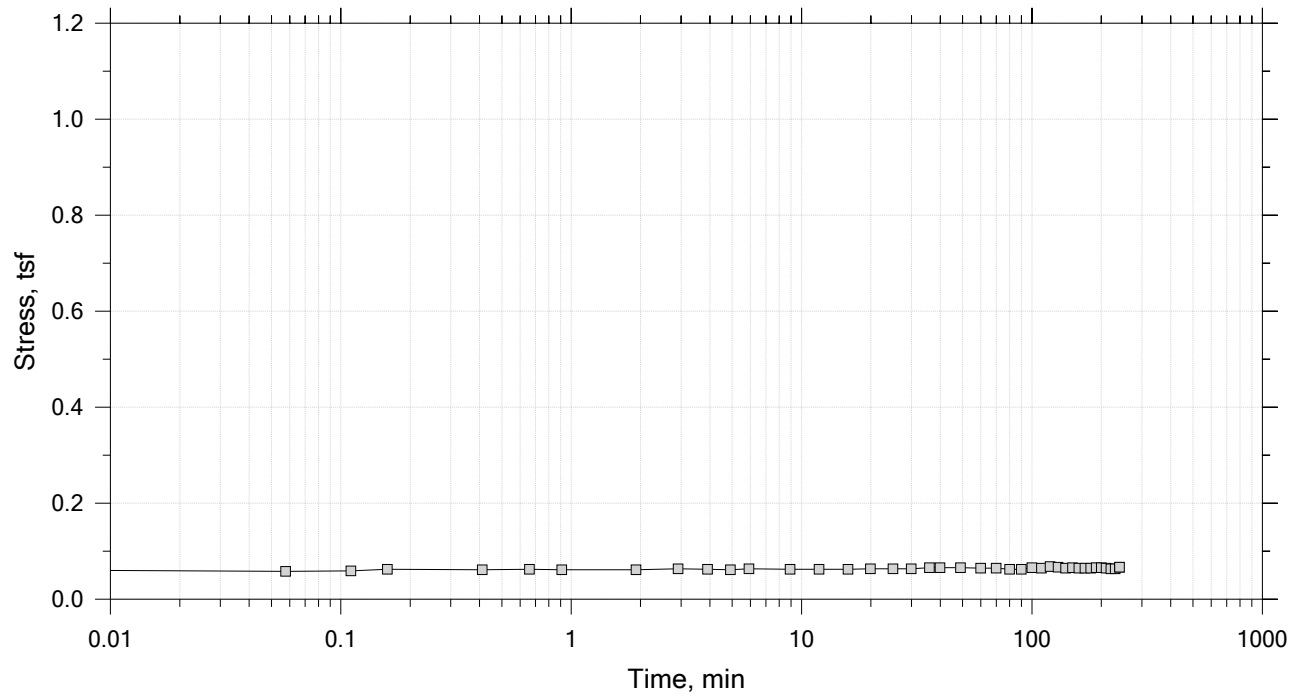
Summary Report




	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0668 tsf



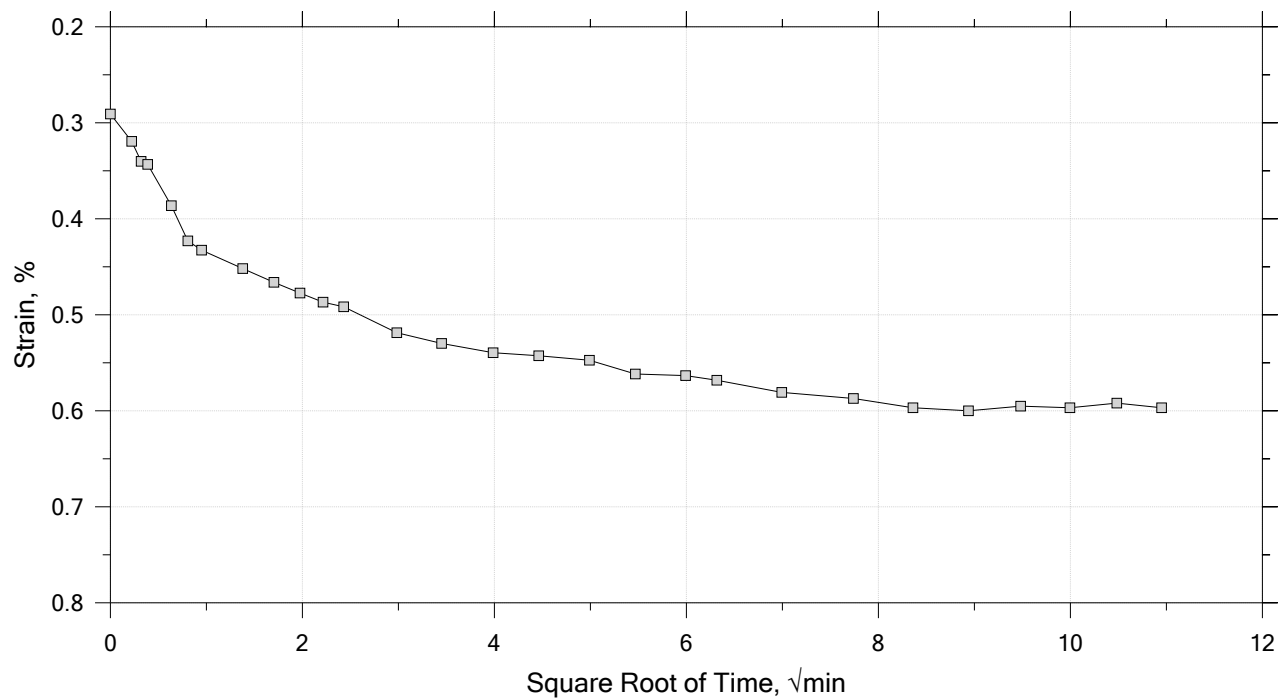
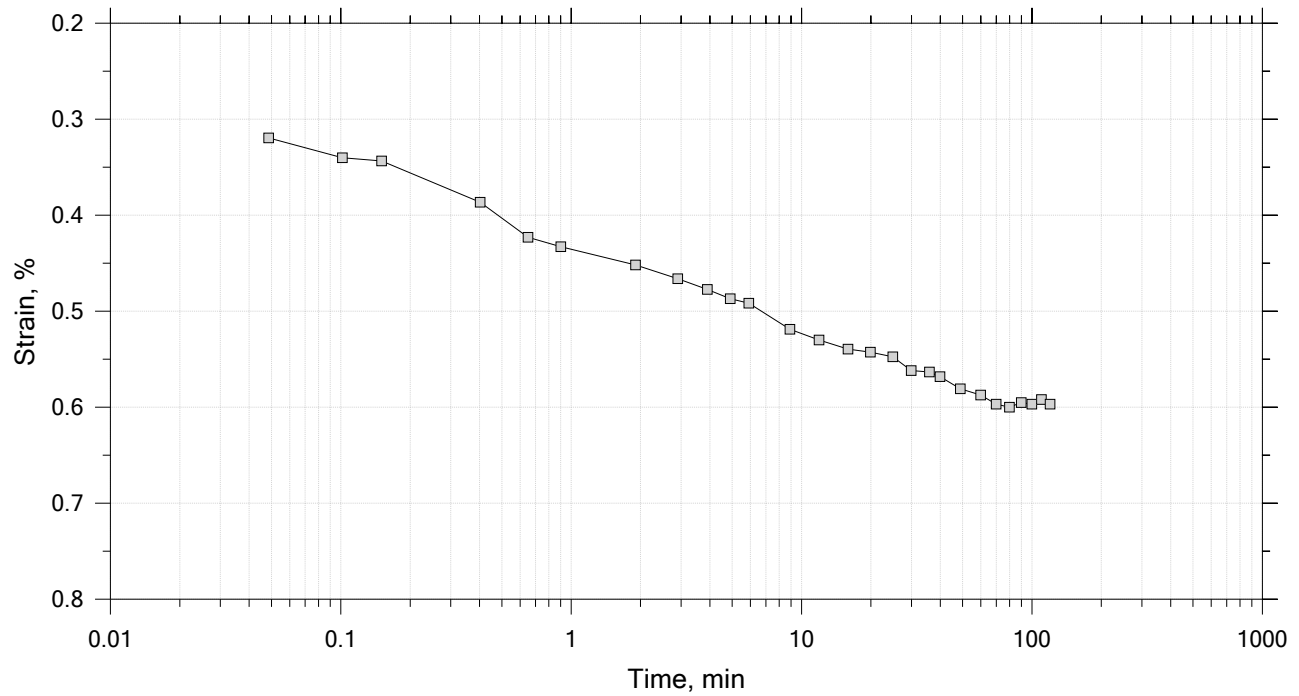
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	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



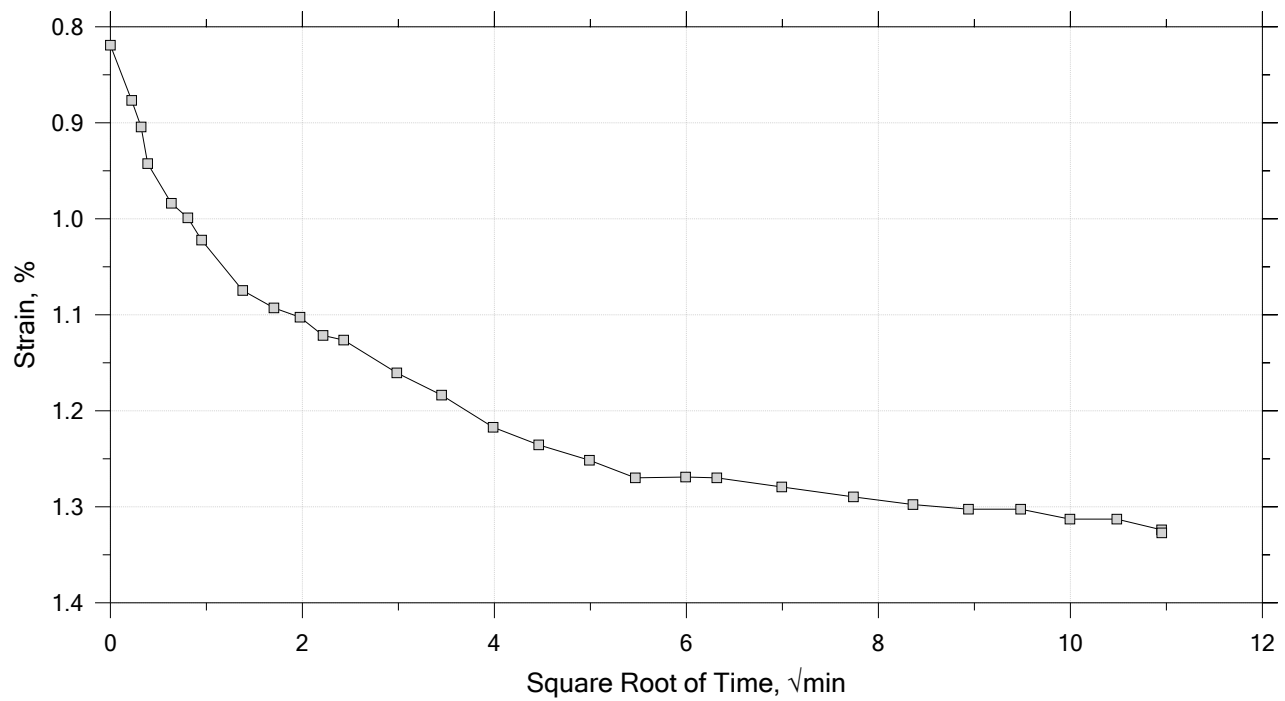
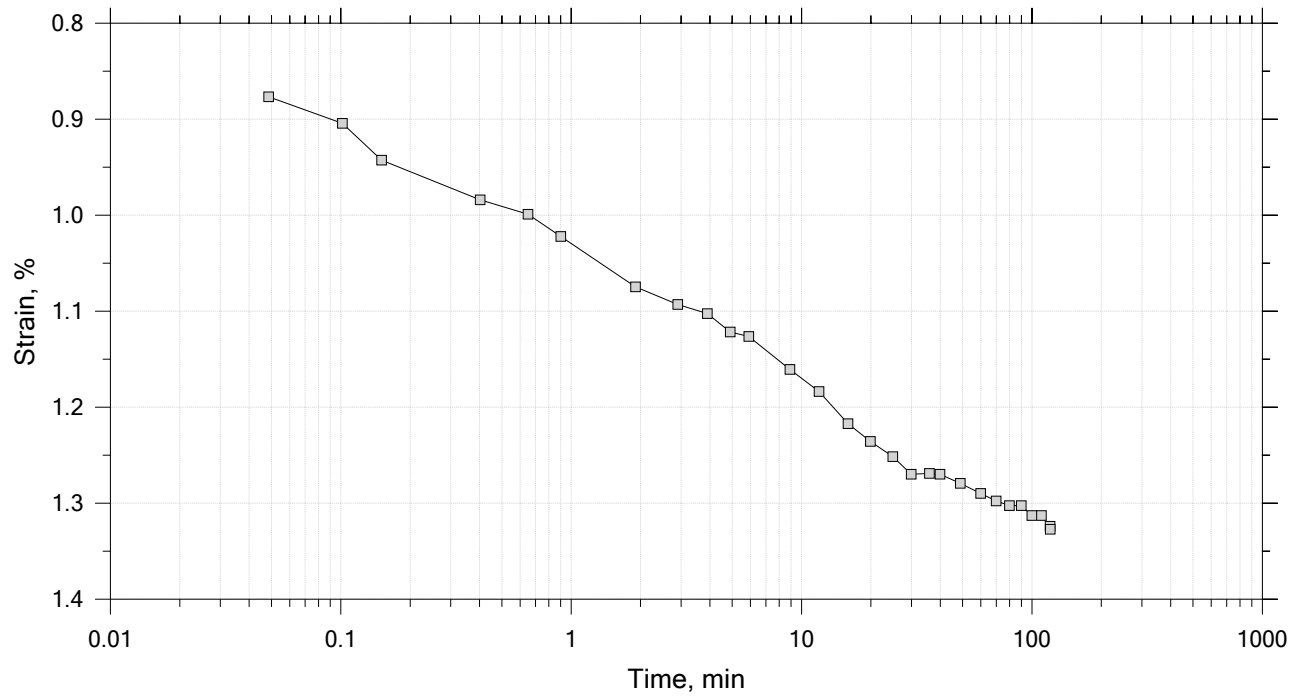
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	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



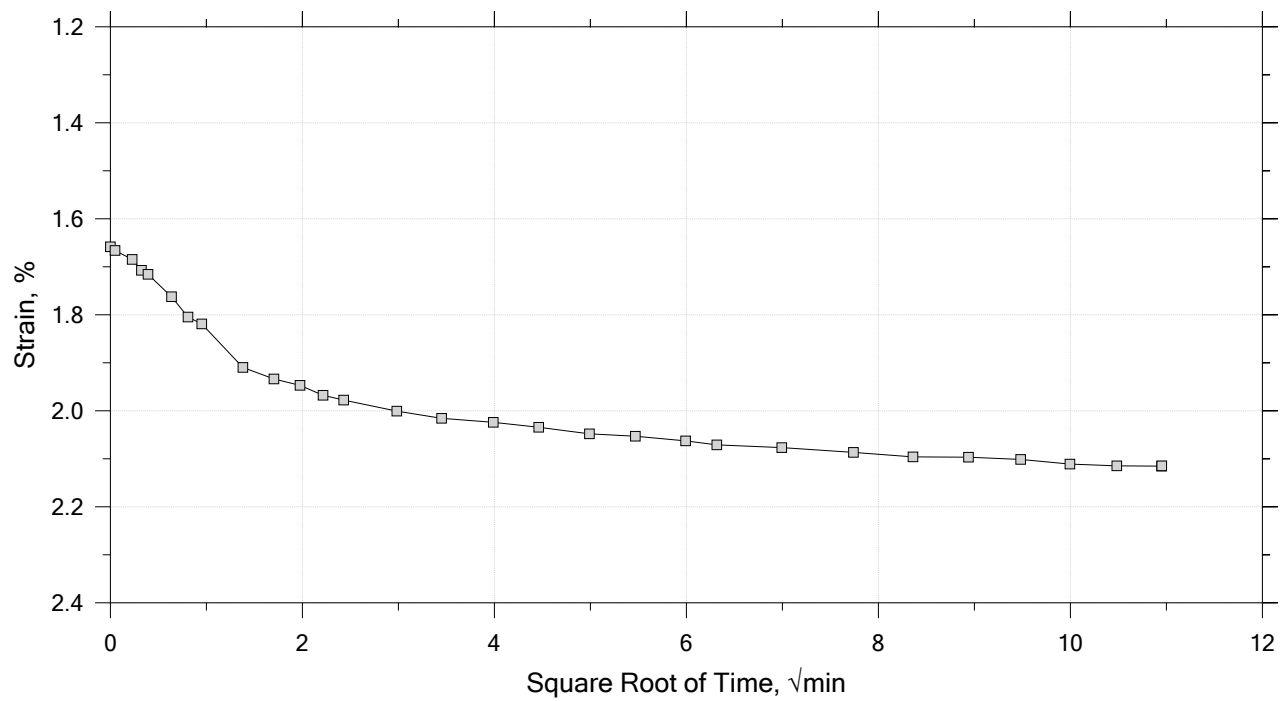
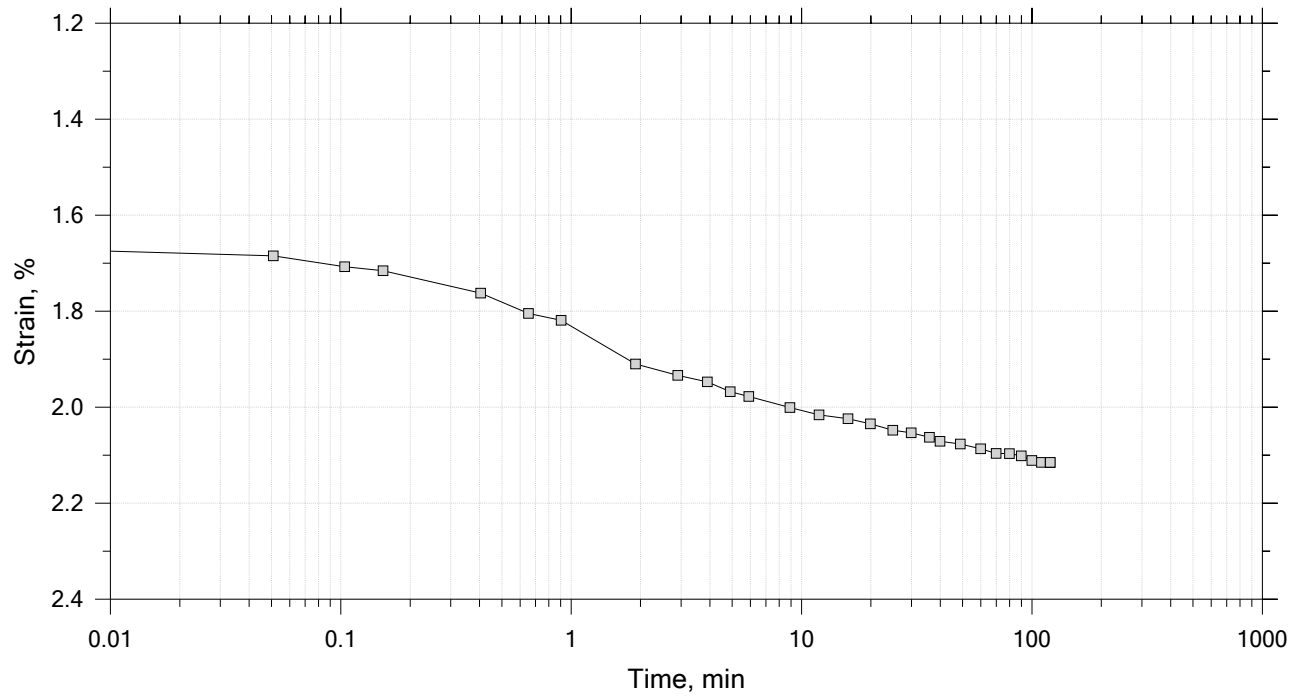
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	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



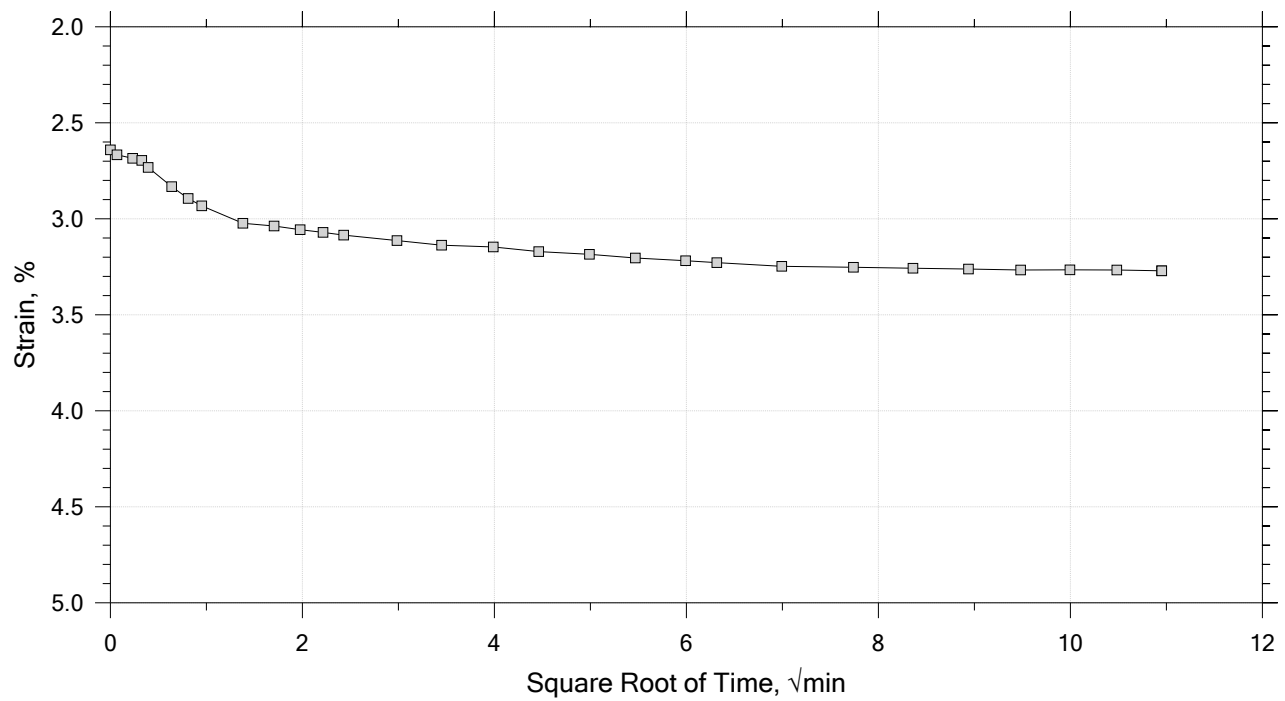
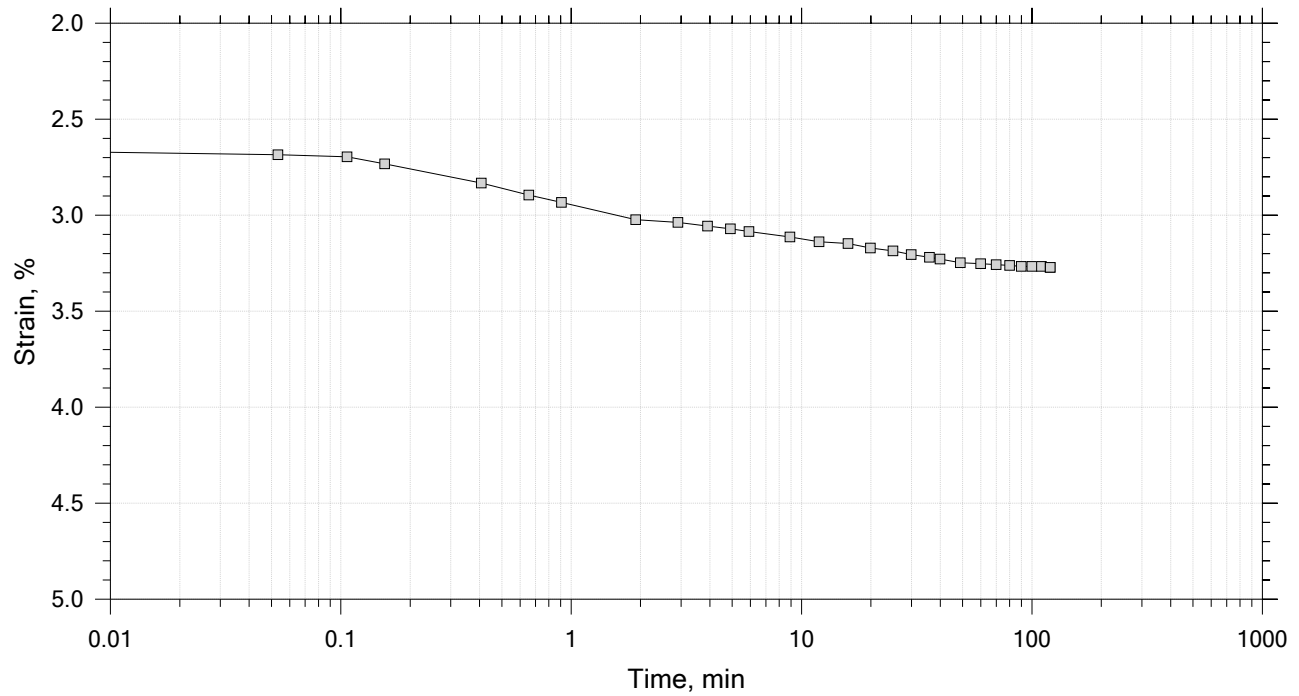
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



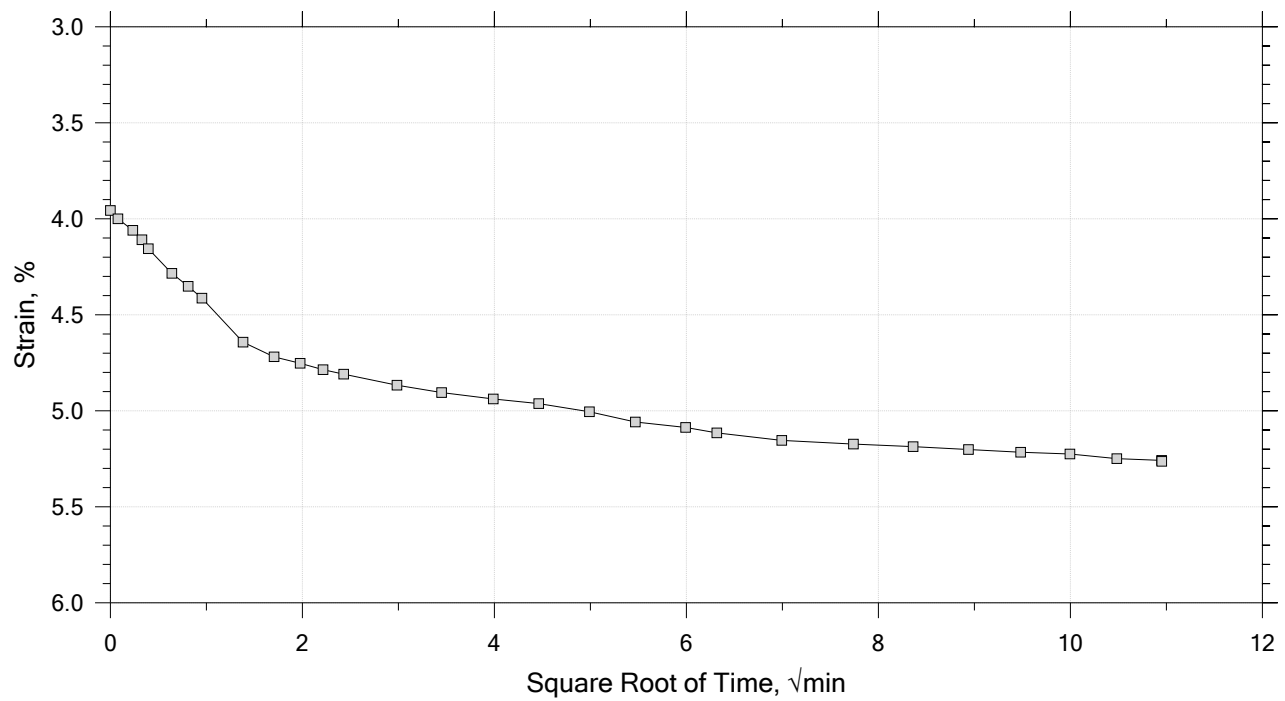
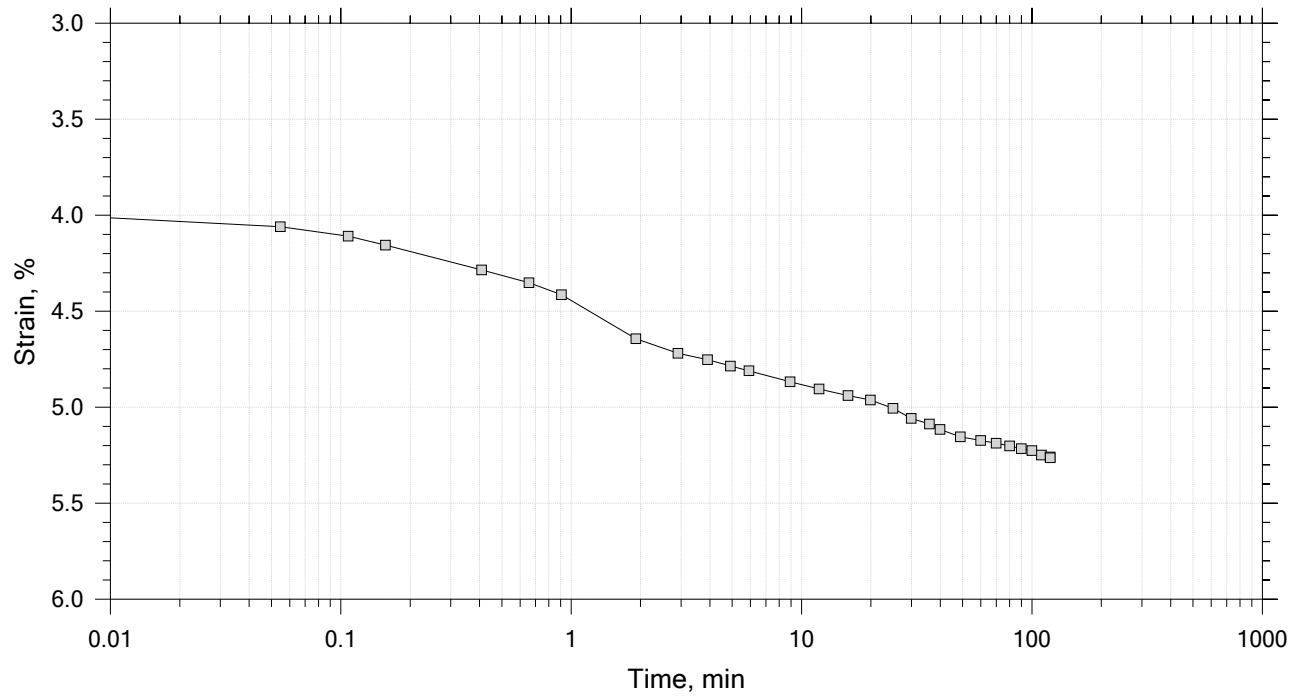
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



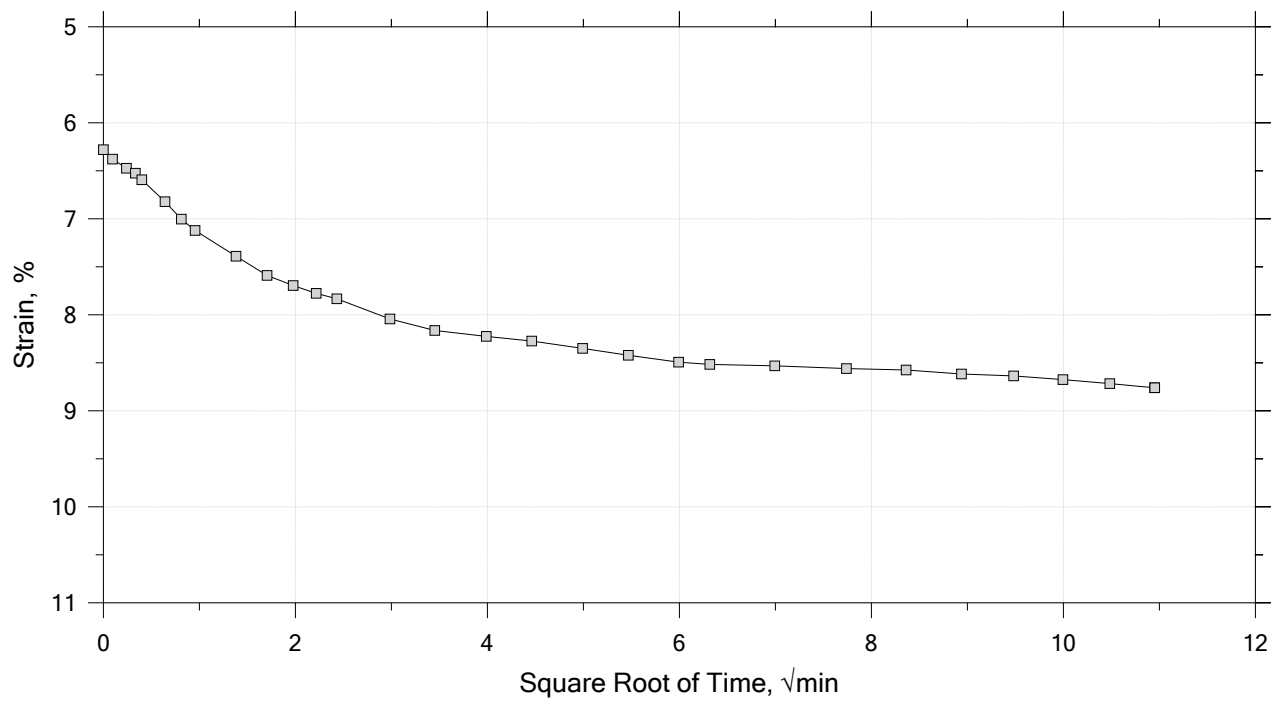
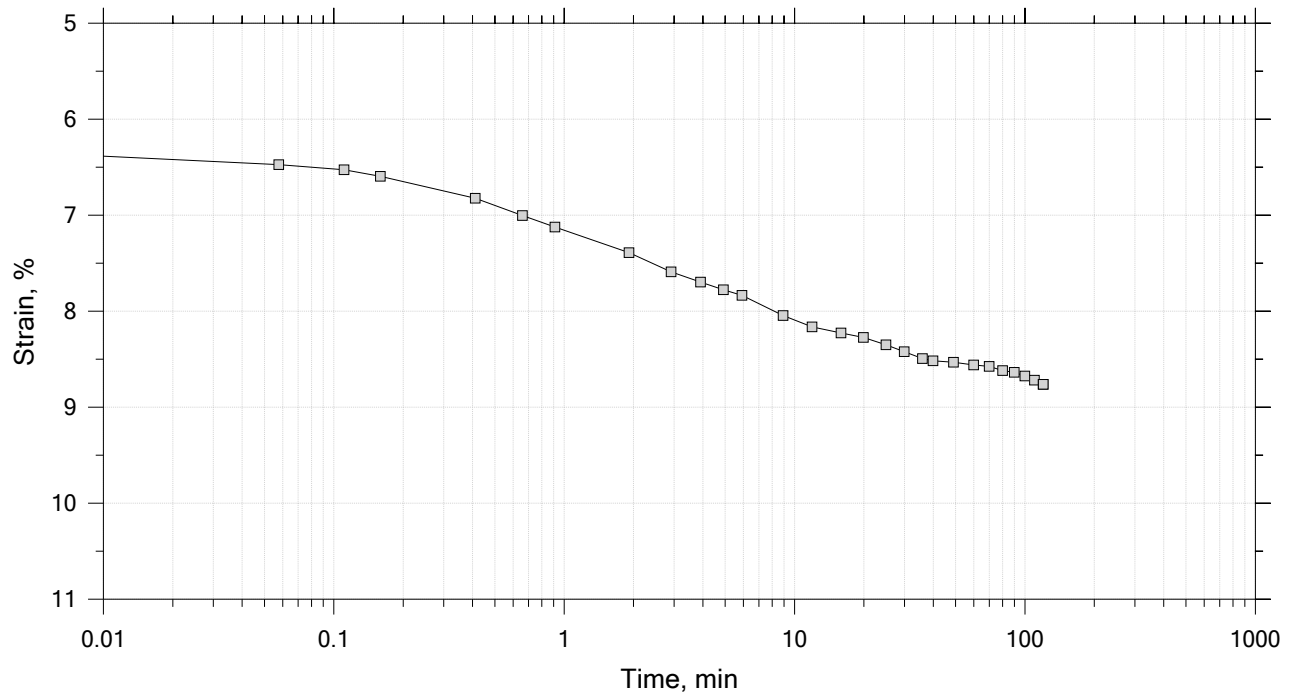
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



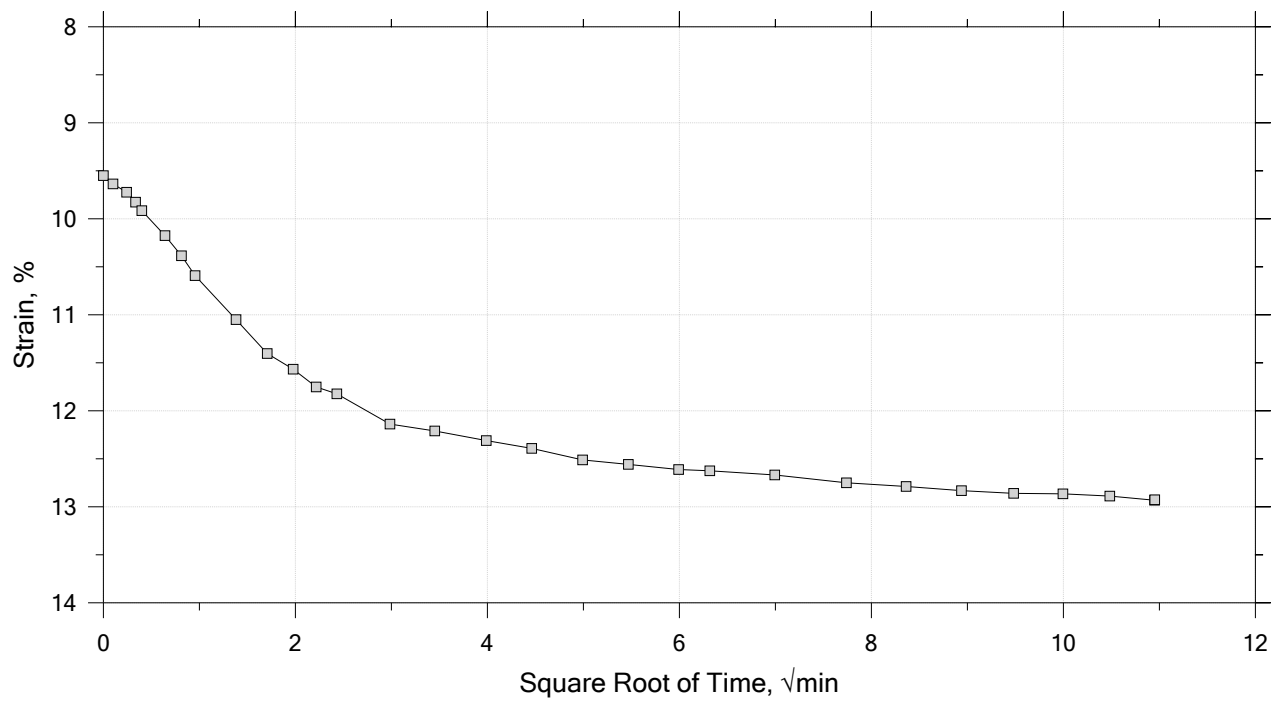
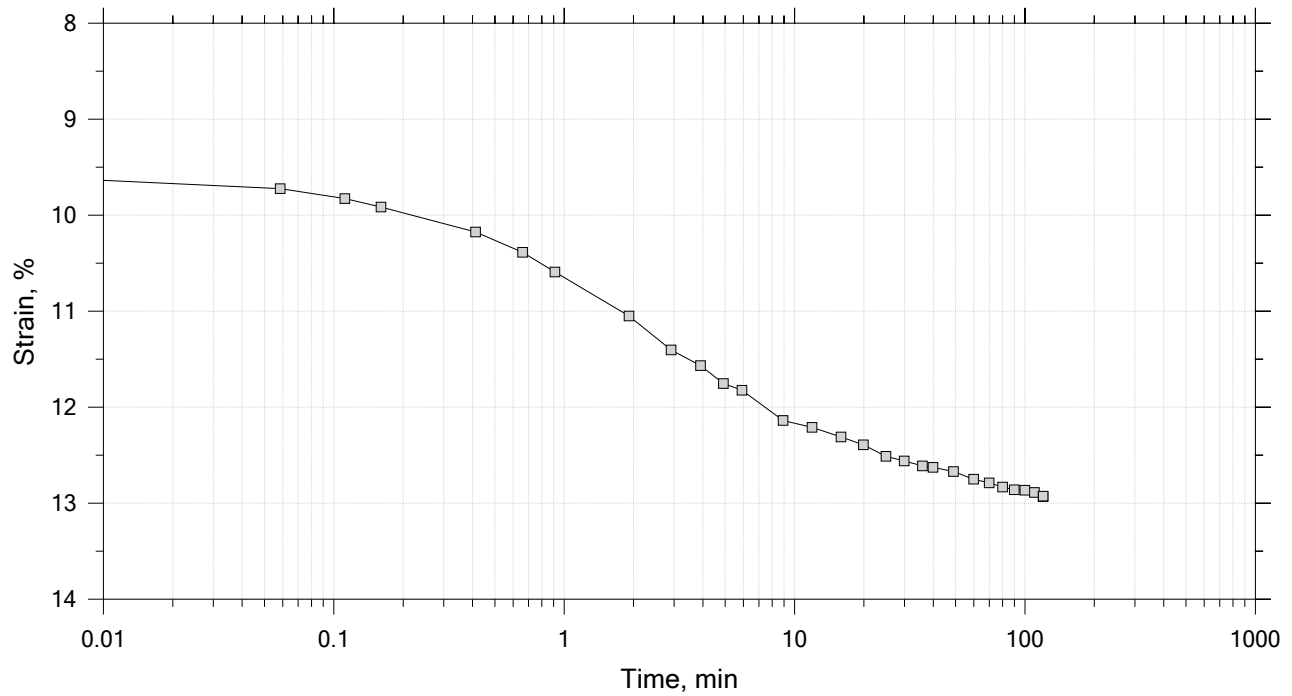
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	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



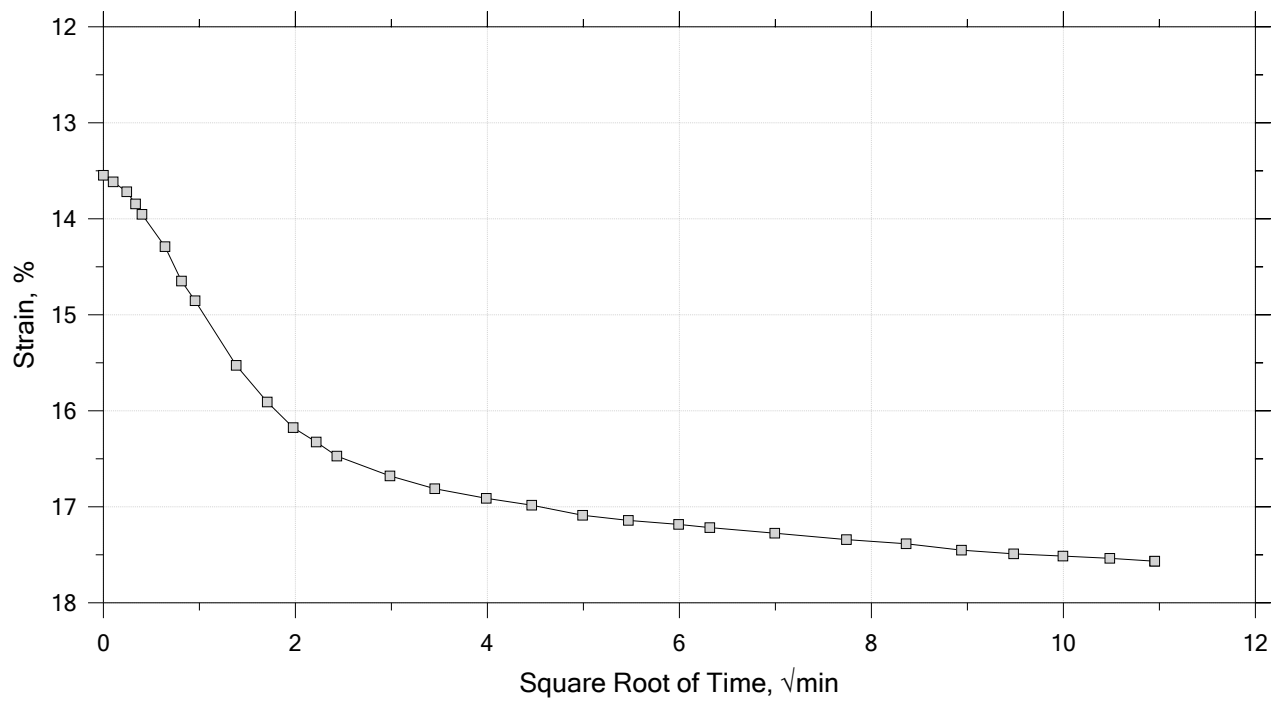
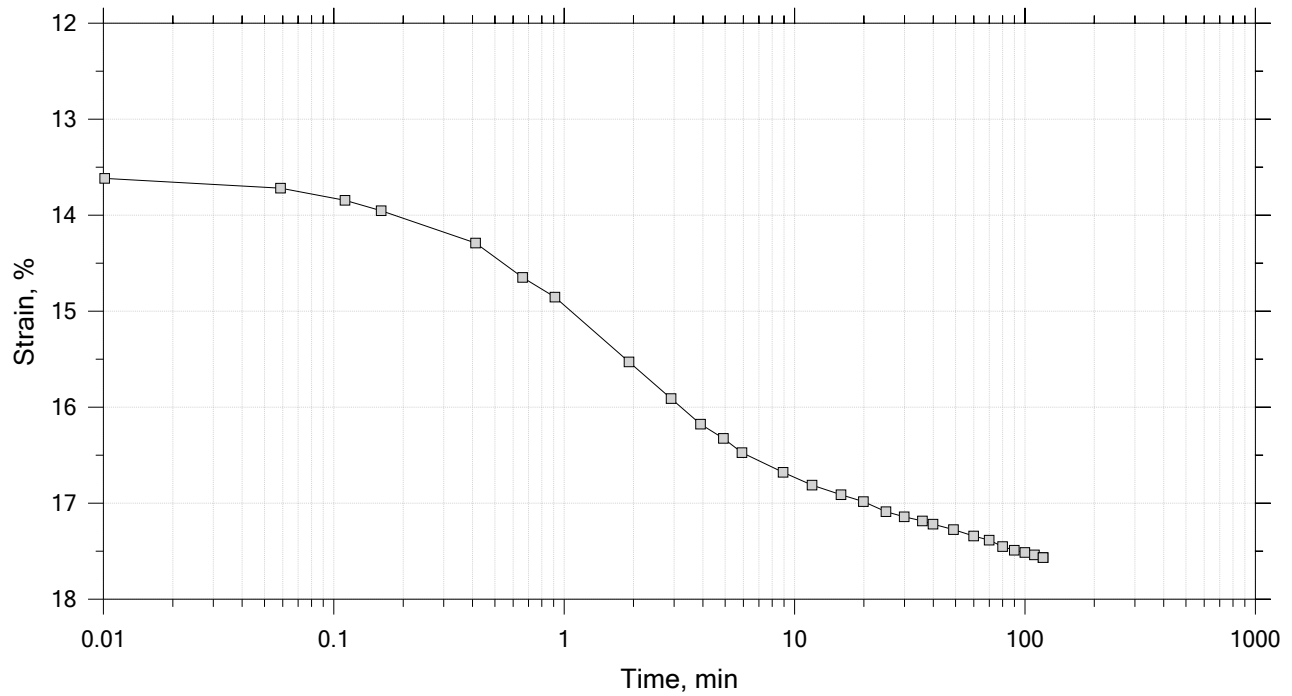
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



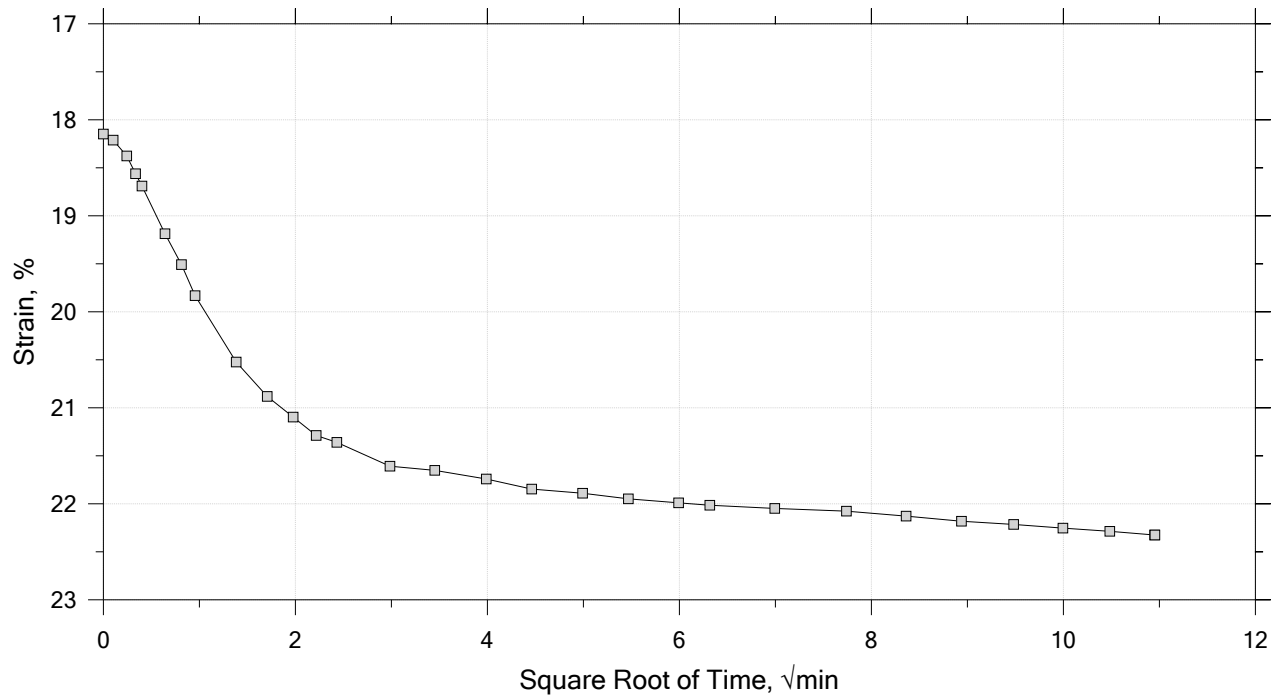
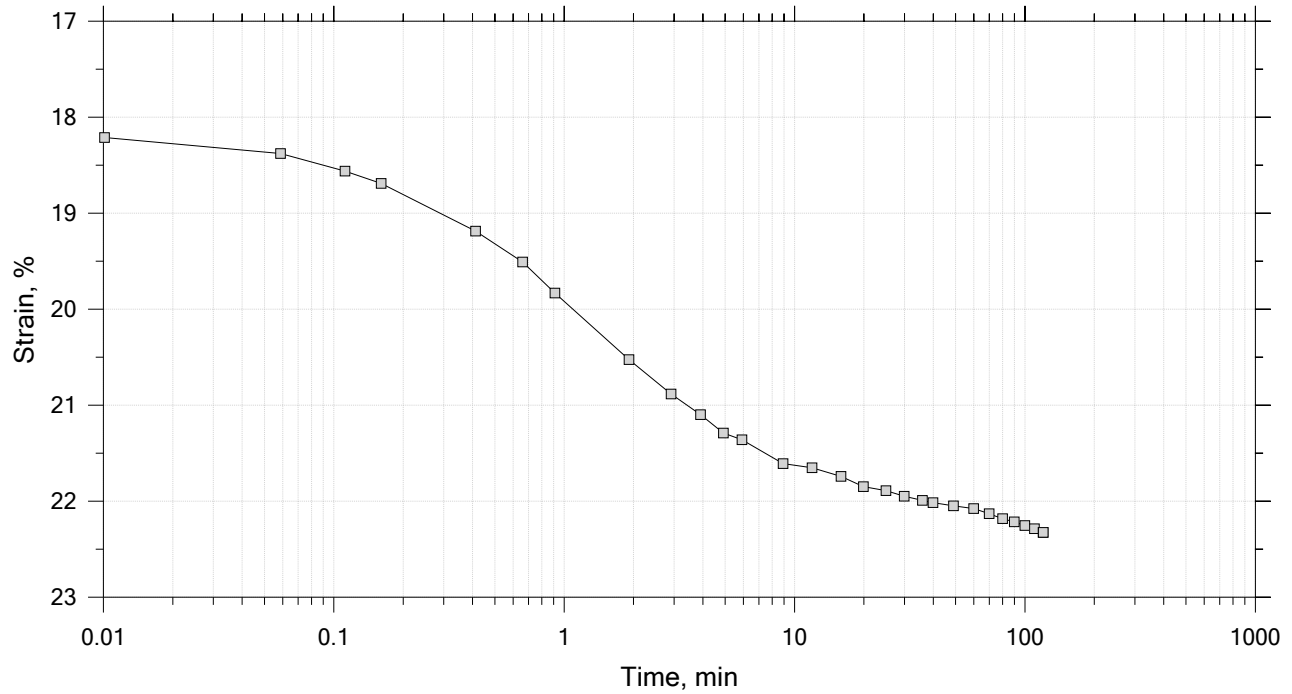
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	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



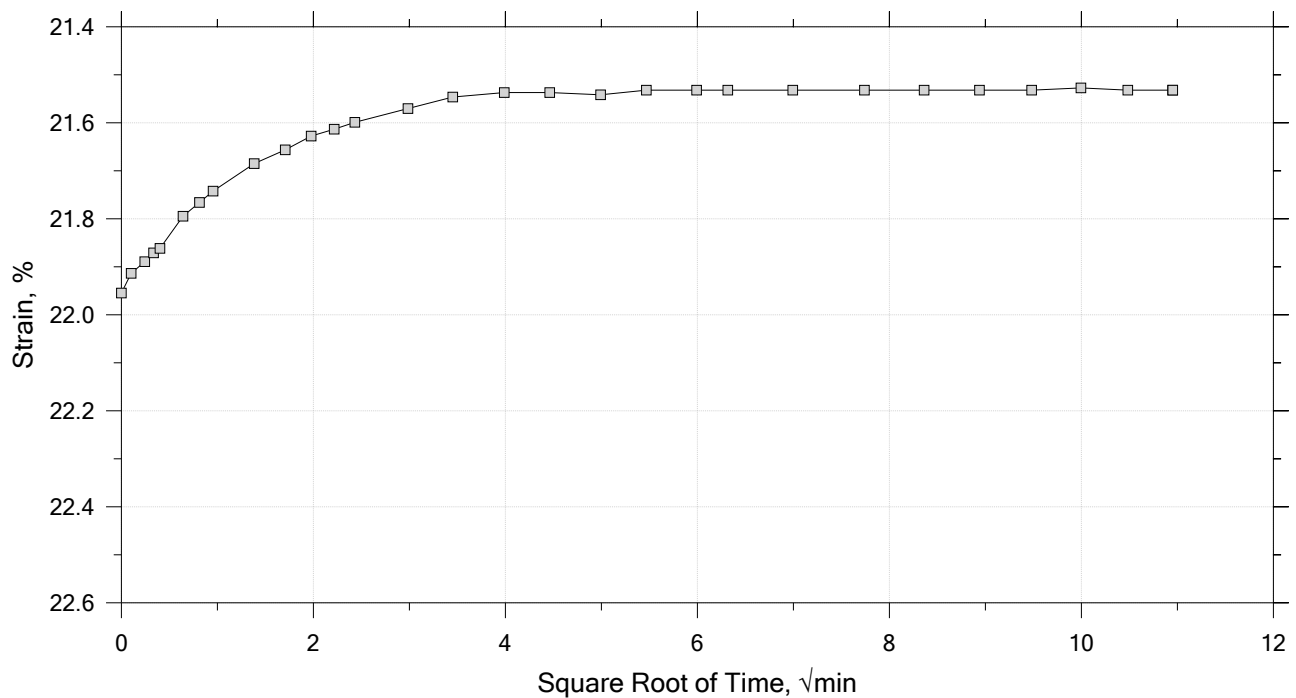
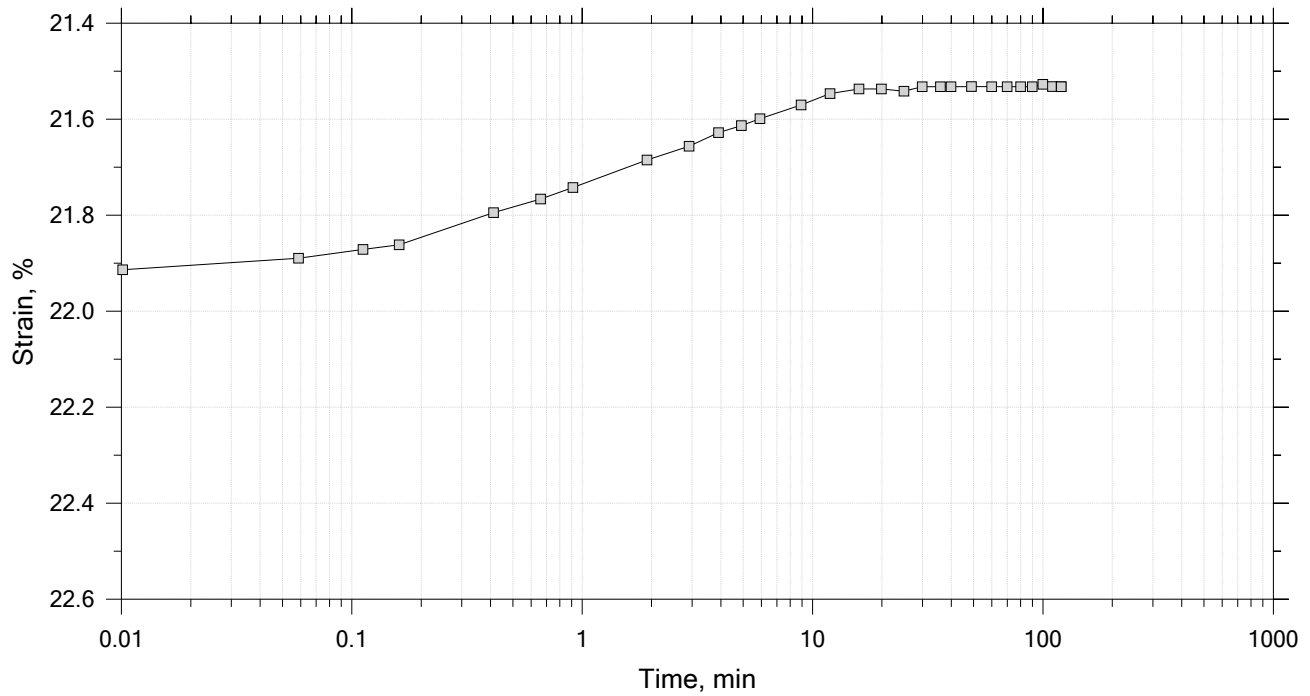
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



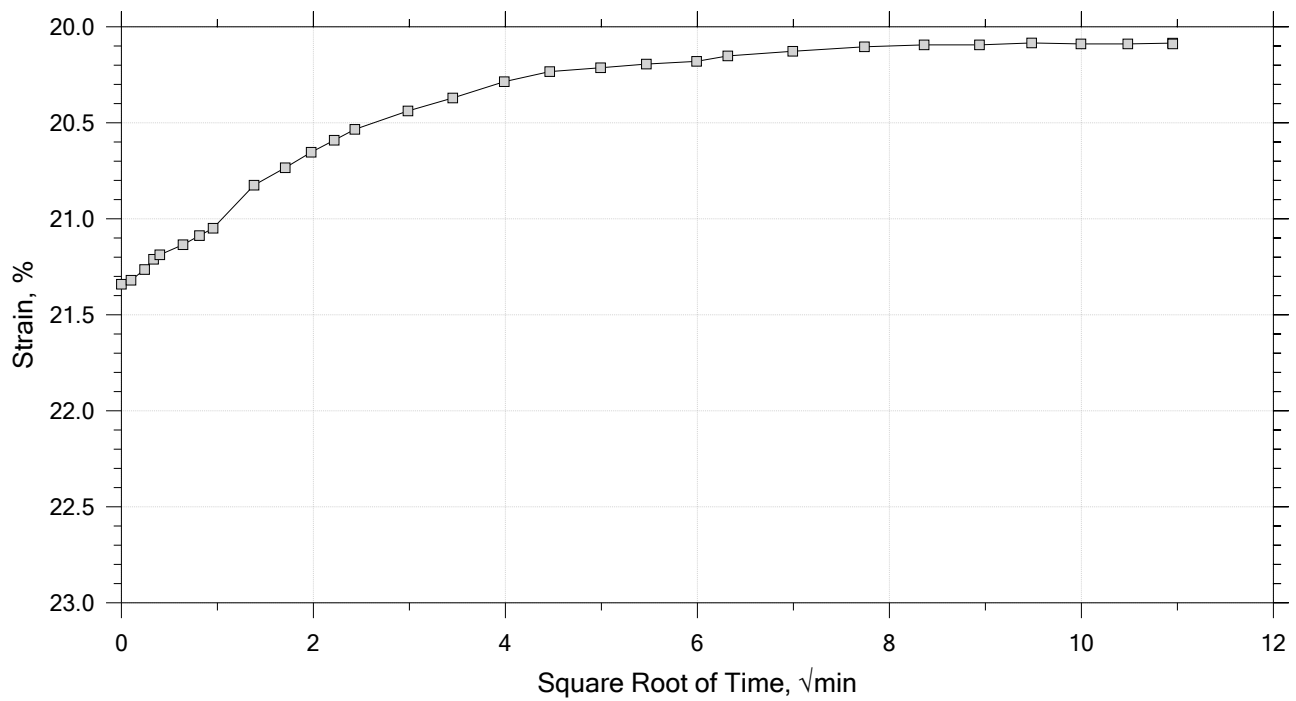
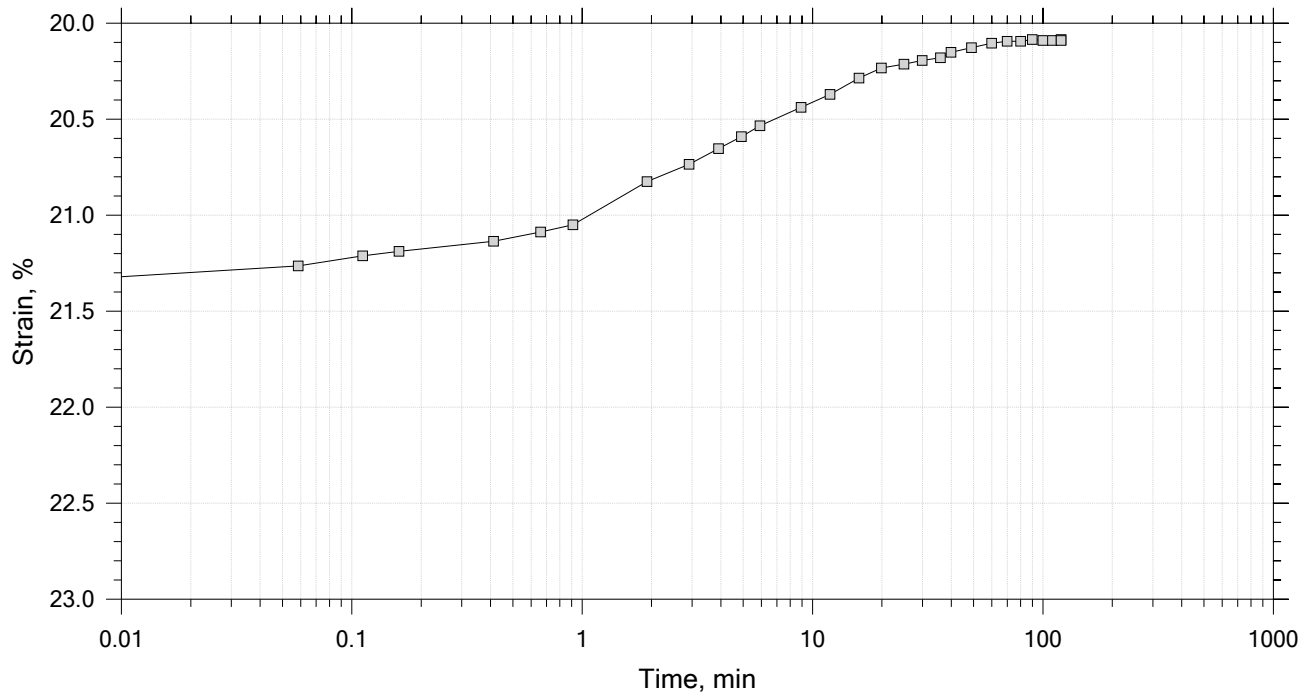
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



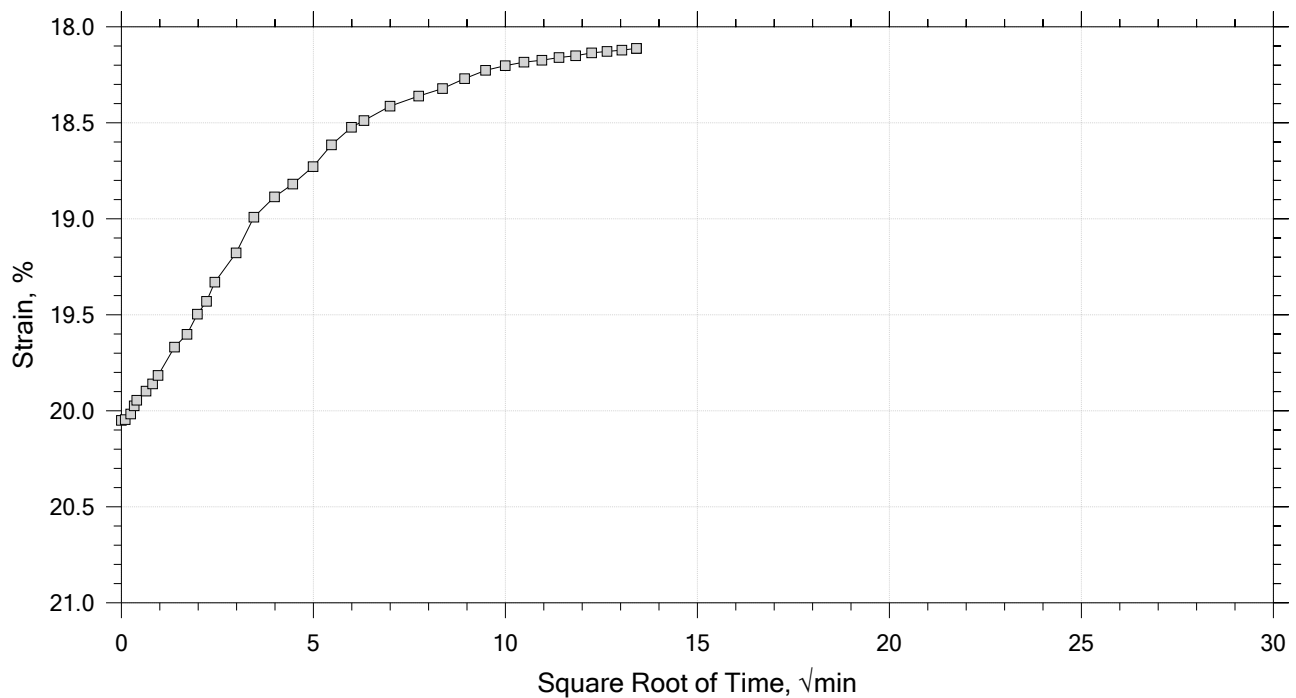
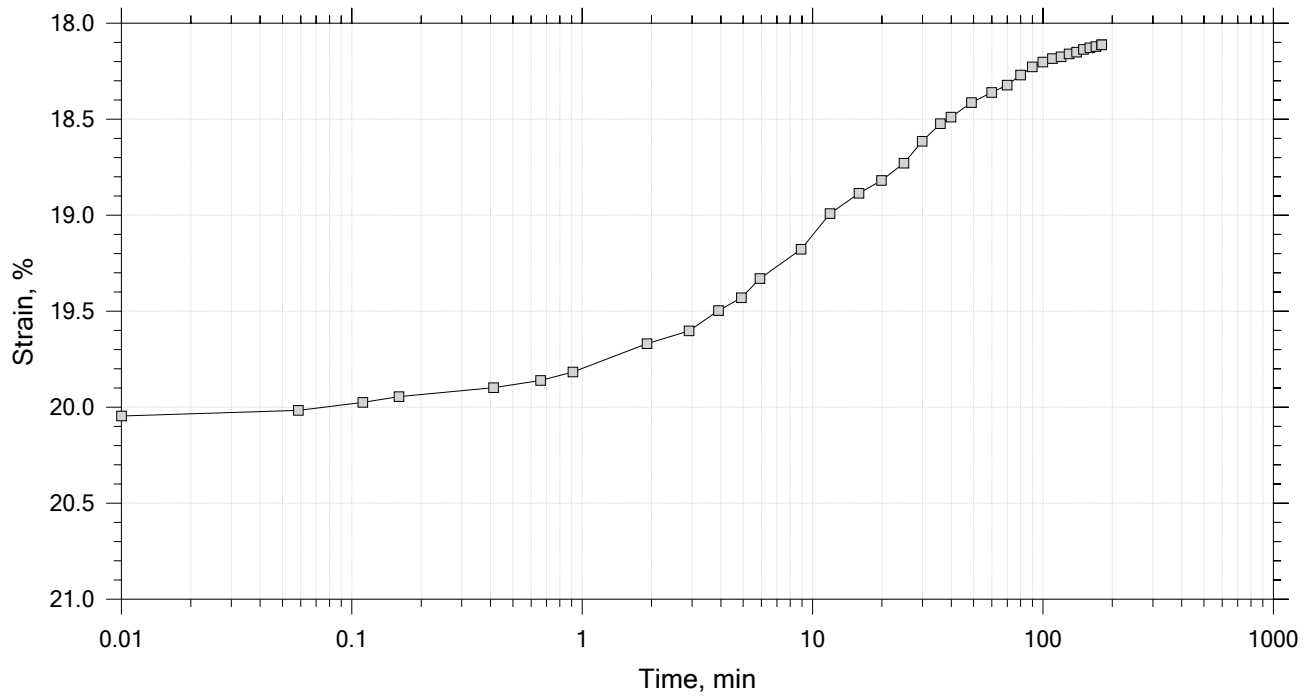
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HD-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



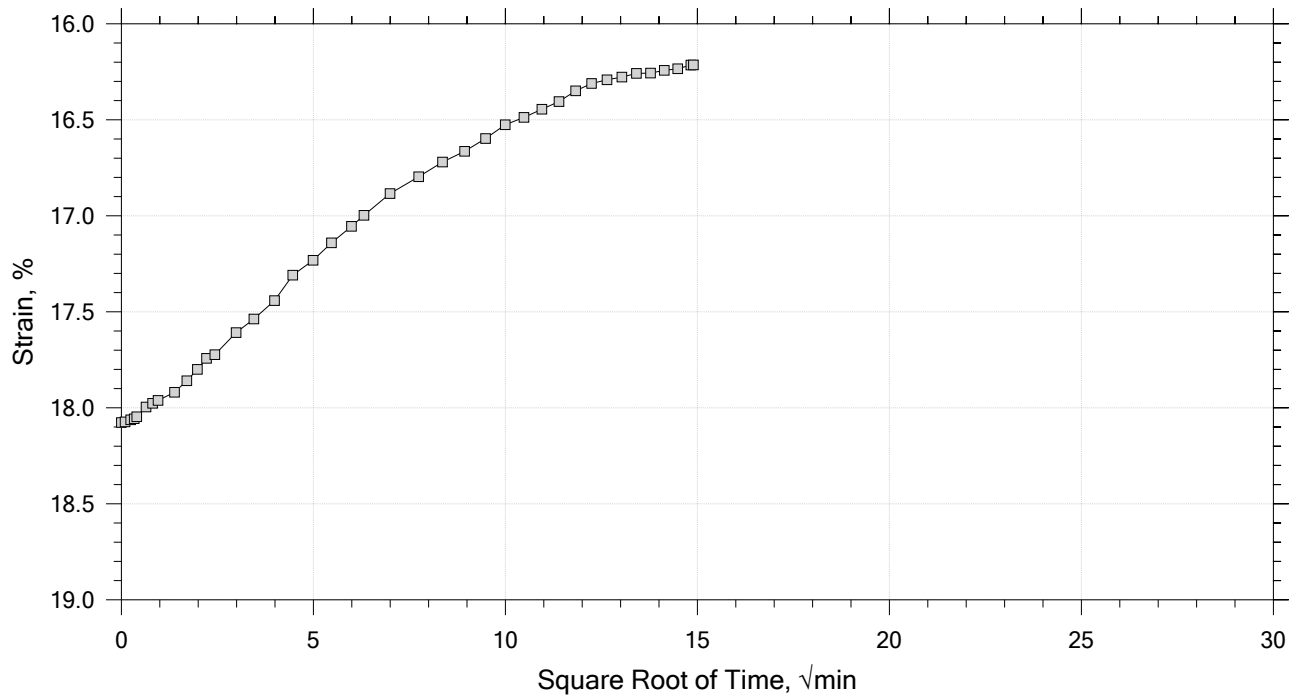
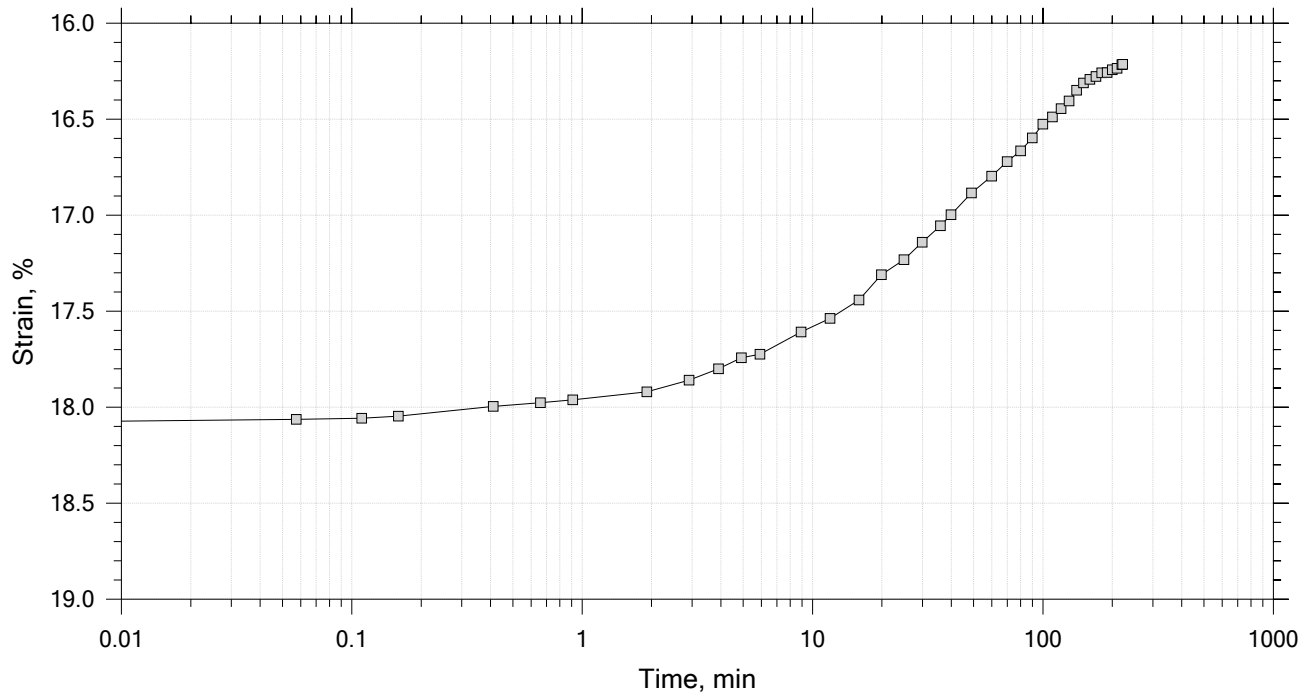
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	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



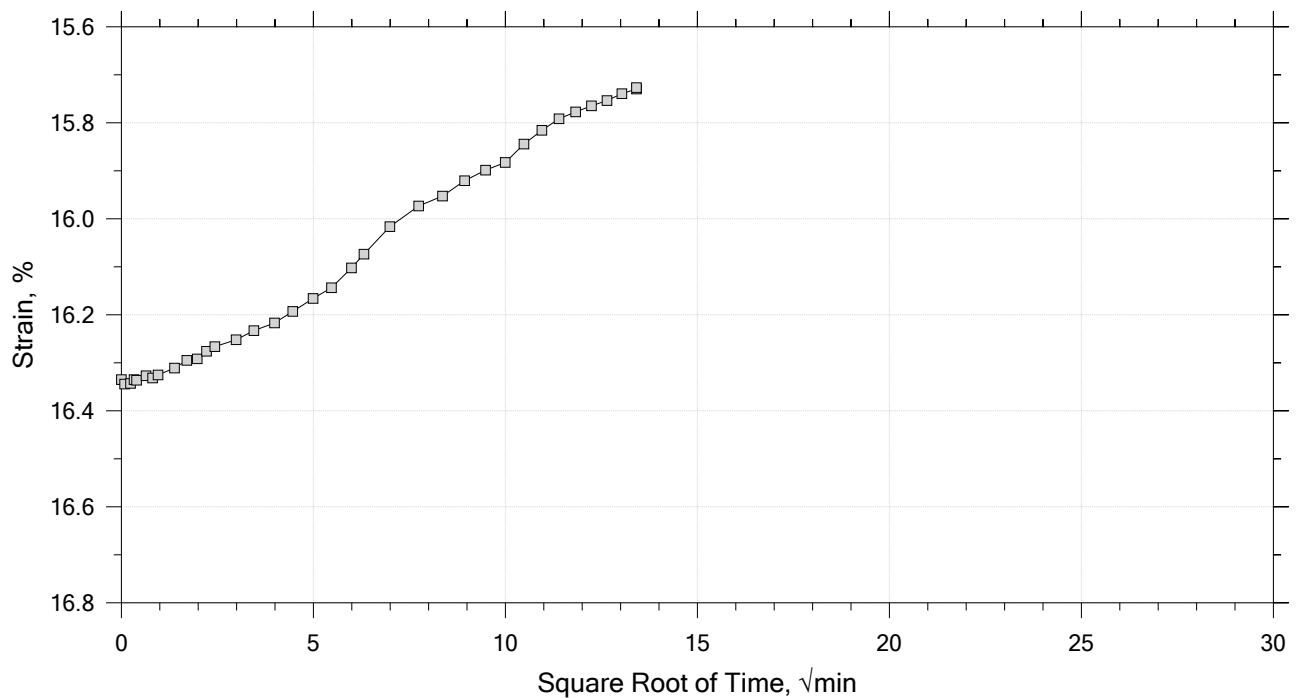
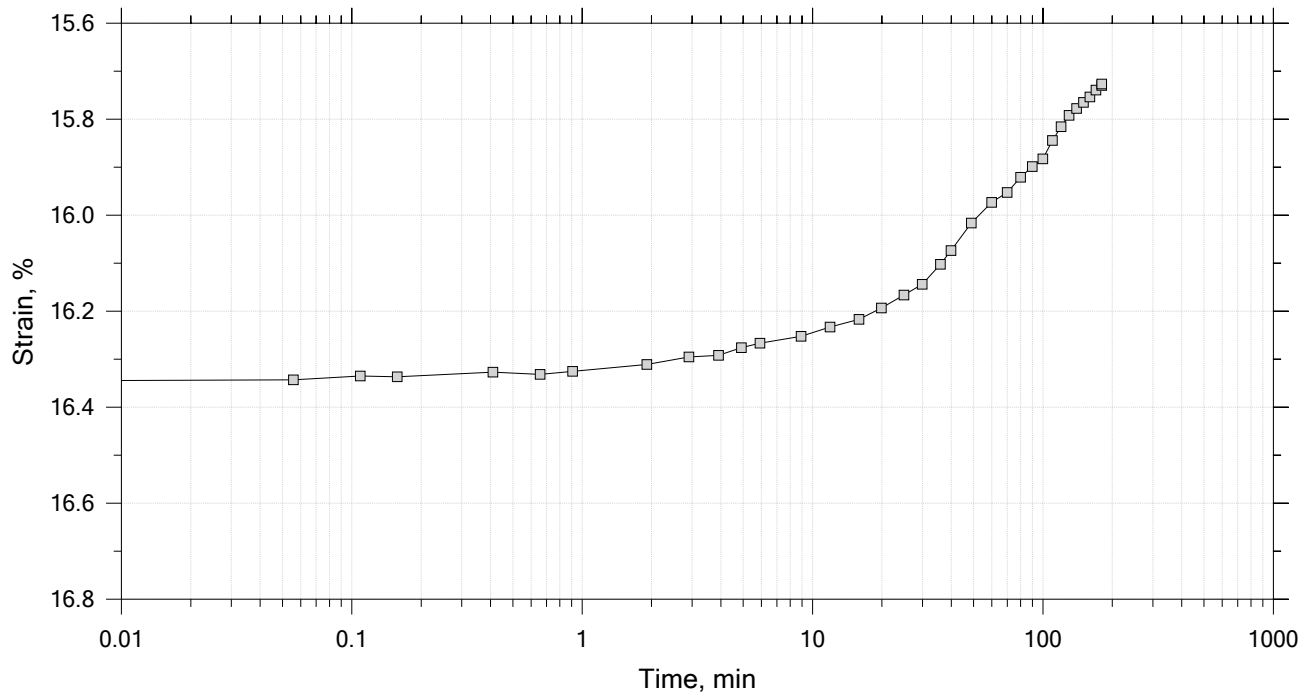
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.77	Liquid Limit: 41
Initial Height: 1.00 in	Initial Void Ratio: 1.14	Plastic Limit: 22
Final Height: 0.84 in	Final Void Ratio: 0.806	Plasticity Index: 19

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	C110	RING		15910
Mass Container, gm	8.31	109.47	109.47	8.53
Mass Container + Wet Soil, gm	117.88	253.28	243.75	141.37
Mass Container + Dry Soil, gm	88.51	213.51	213.51	111.45
Mass Dry Soil, gm	80.2	104.04	104.04	102.92
Water Content, %	36.62	38.23	29.07	29.07
Void Ratio	---	1.14	0.81	---
Degree of Saturation, %	---	92.73	100.00	---
Dry Unit Weight, pcf	---	80.74	95.808	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

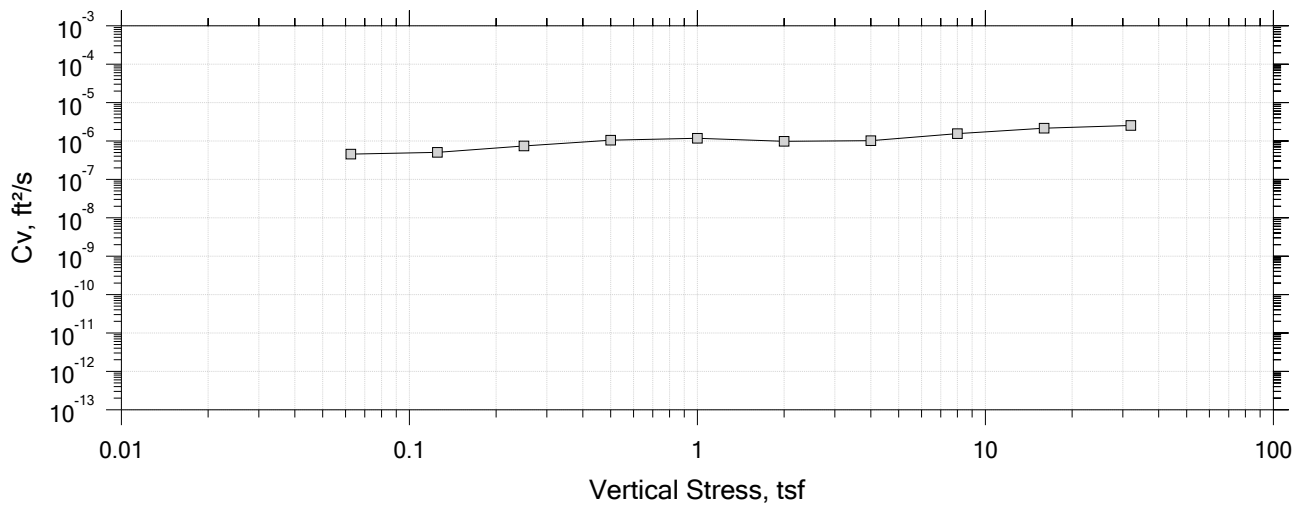
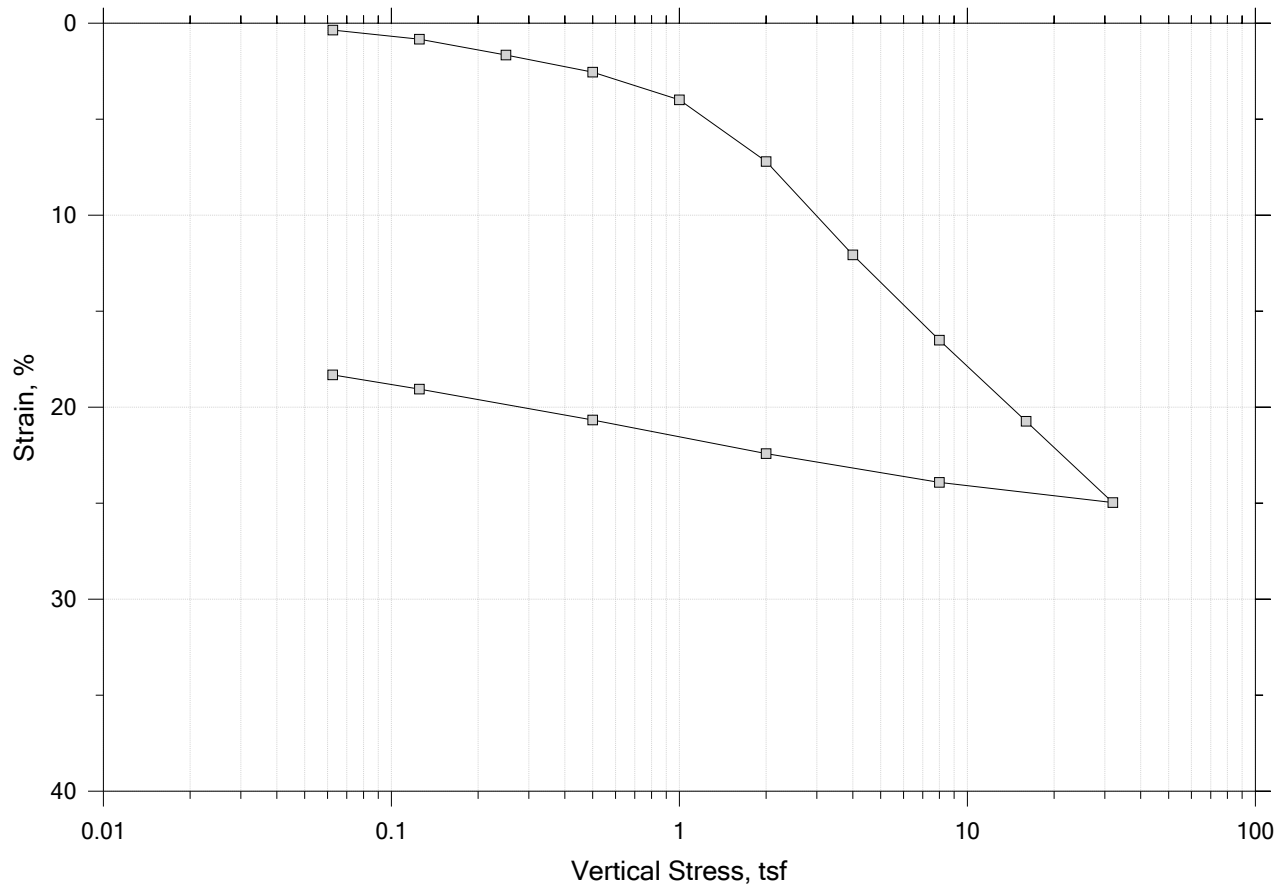
Square Root of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md/trm	Checked By: mcm
	Sample No.: 1-U	Test Date: 9/27/18	Depth: 5-7
	Test No.: IP-1 A	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

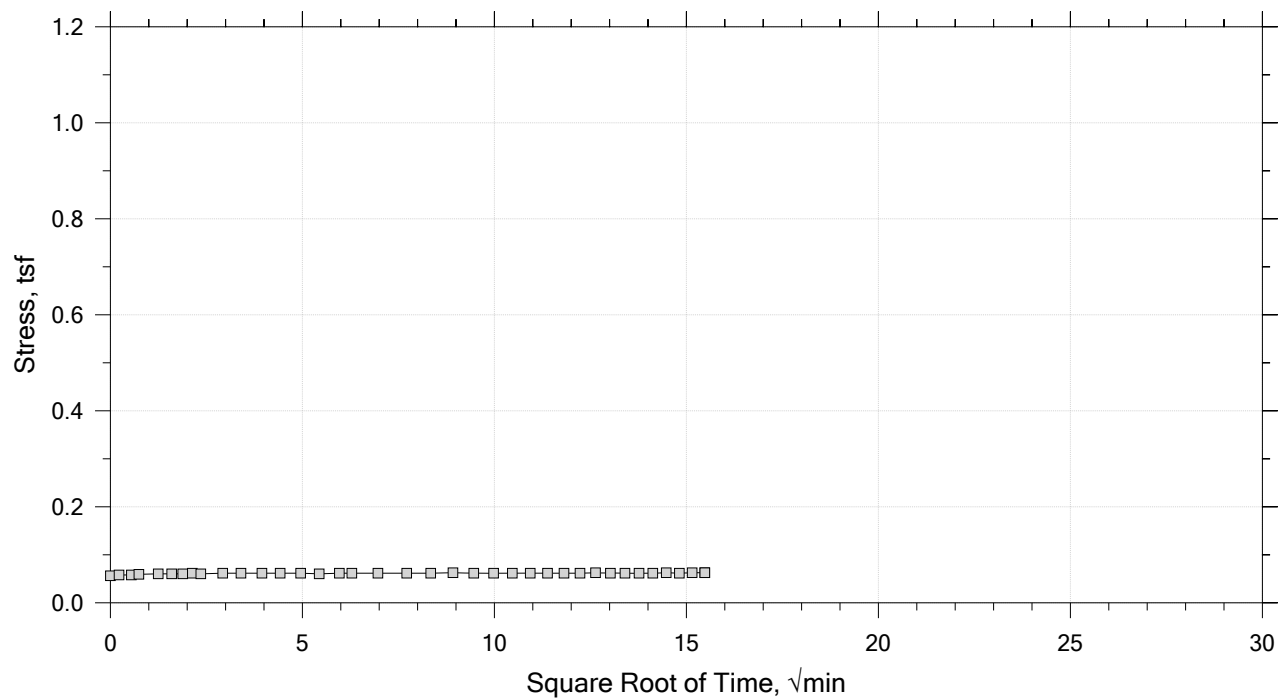
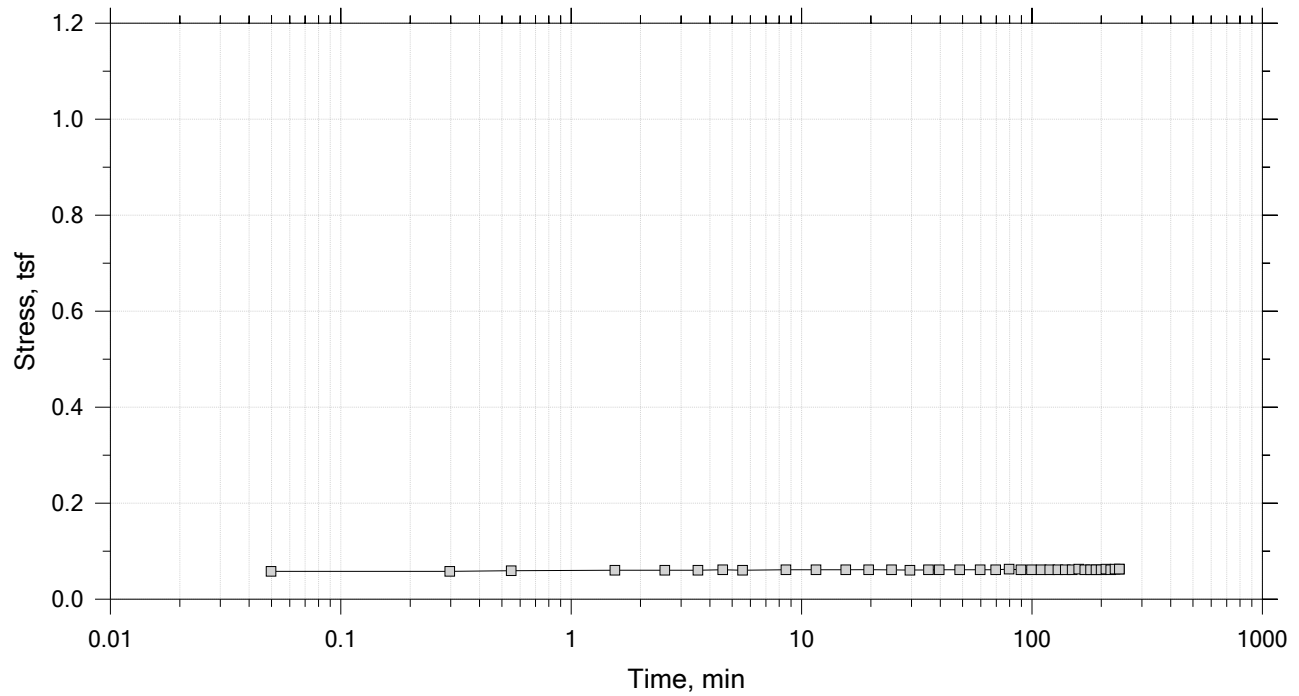
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HD-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0626 tsf



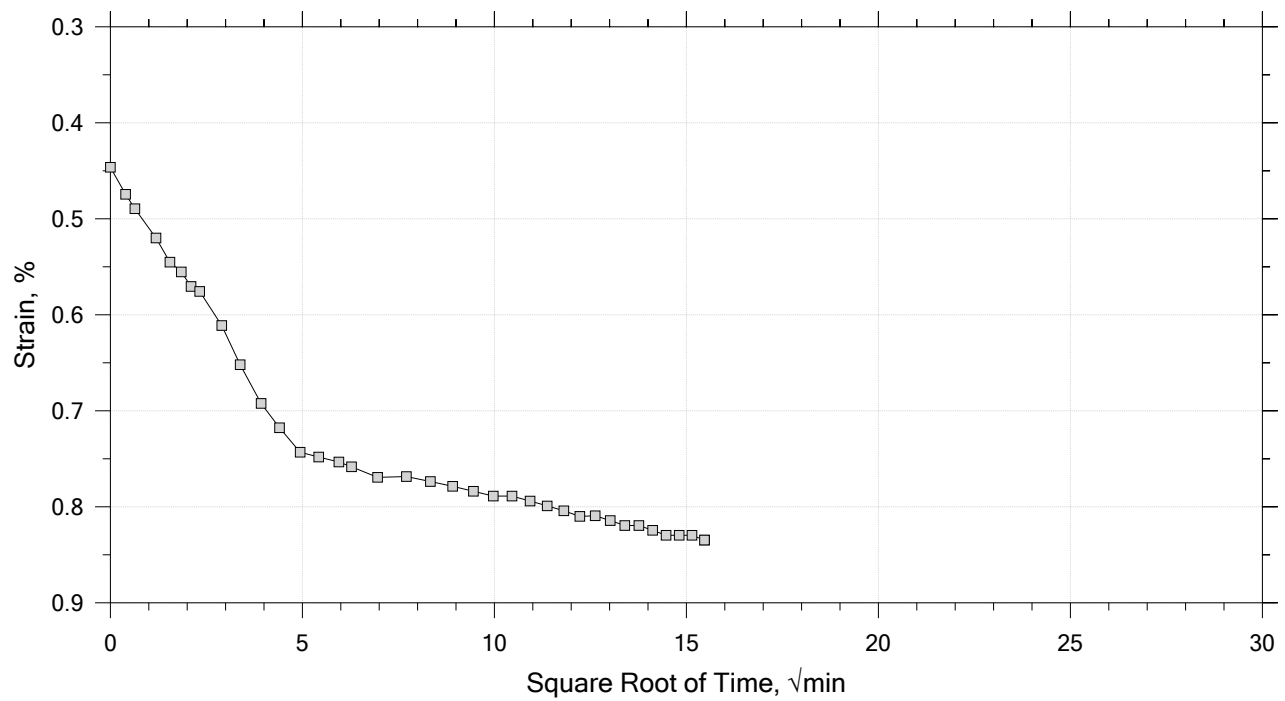
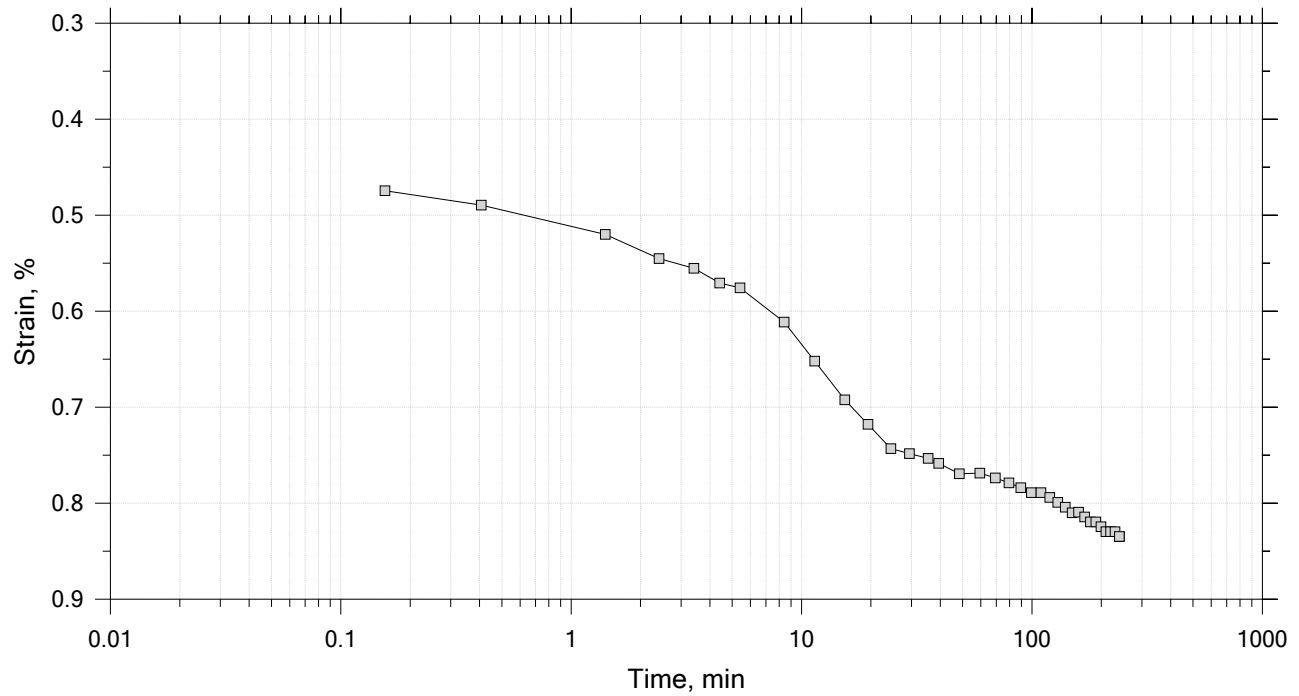
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



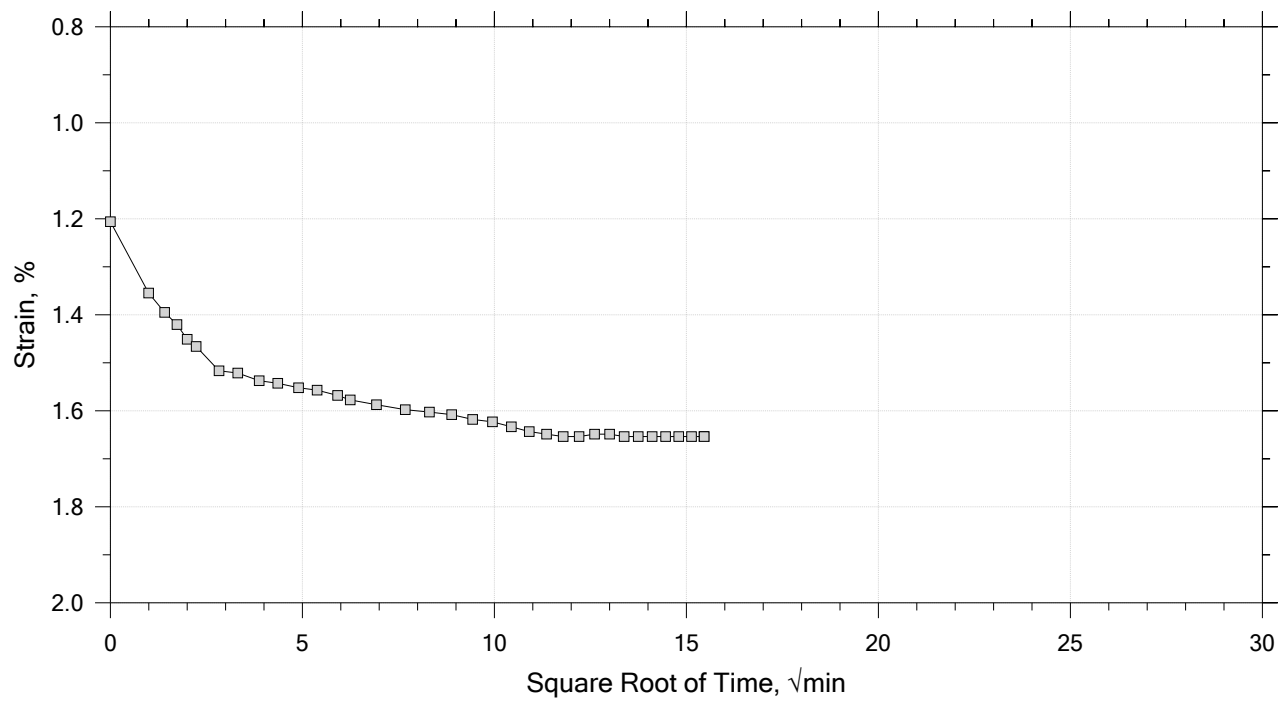
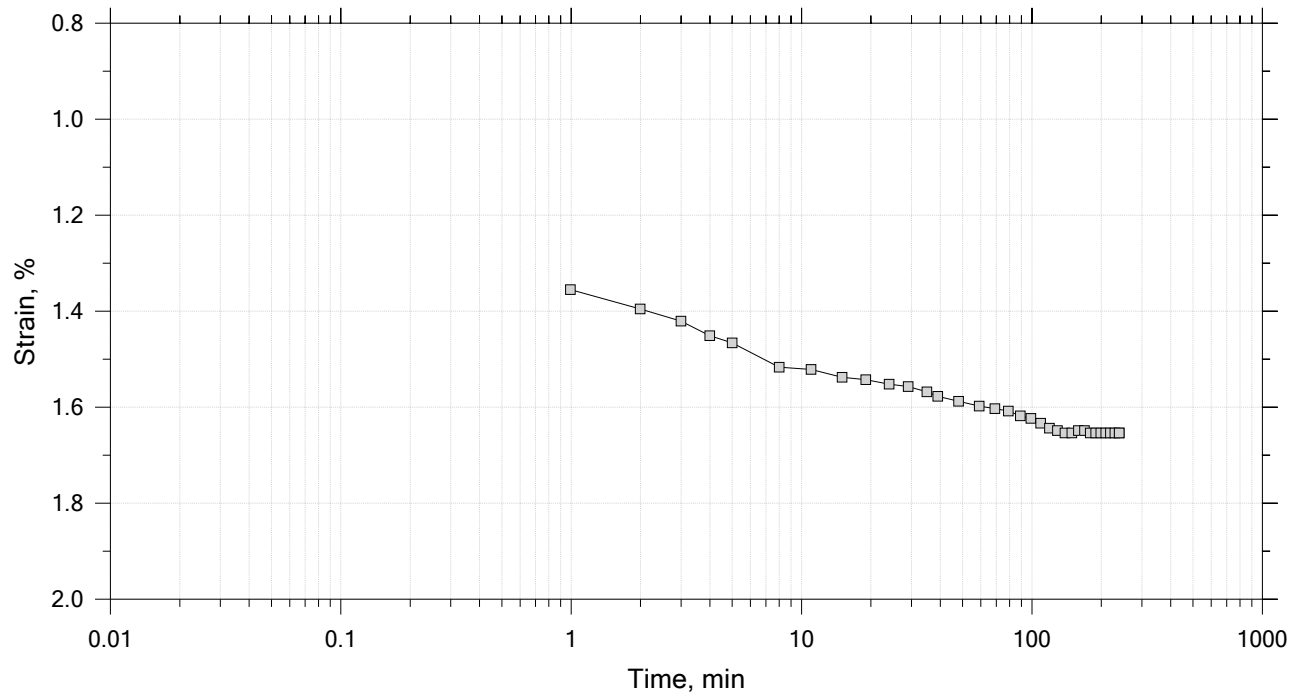
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



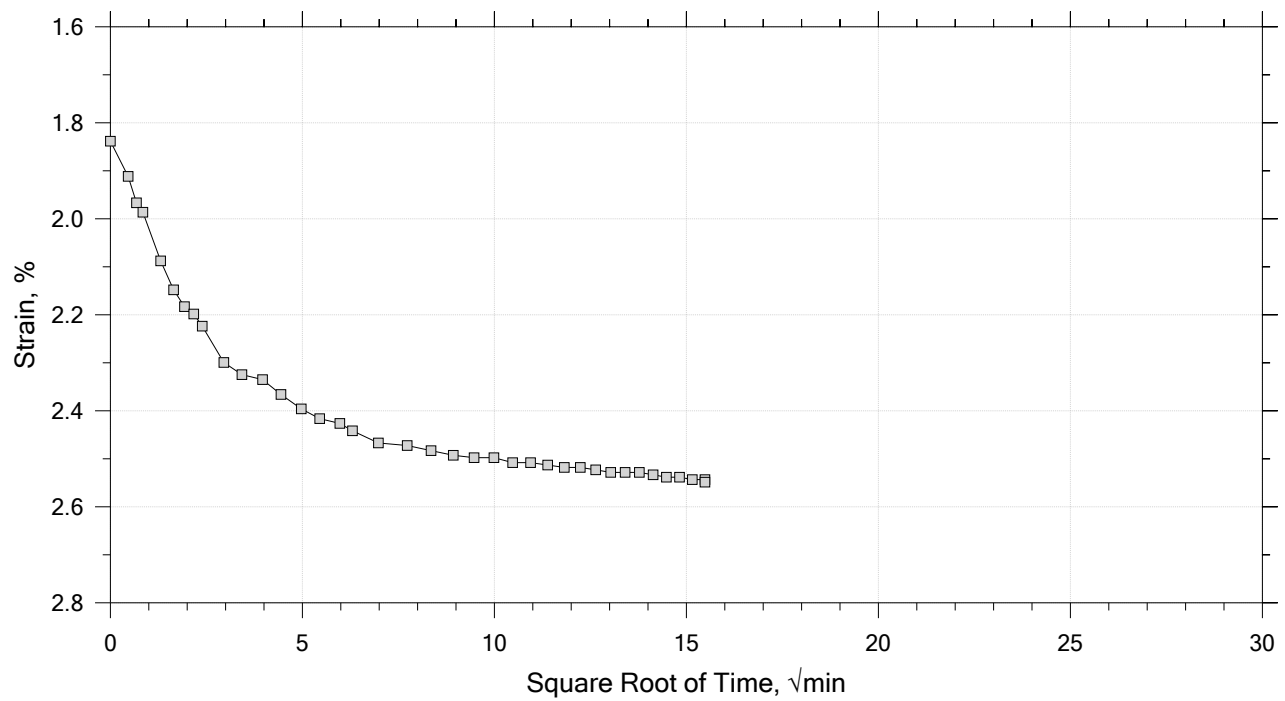
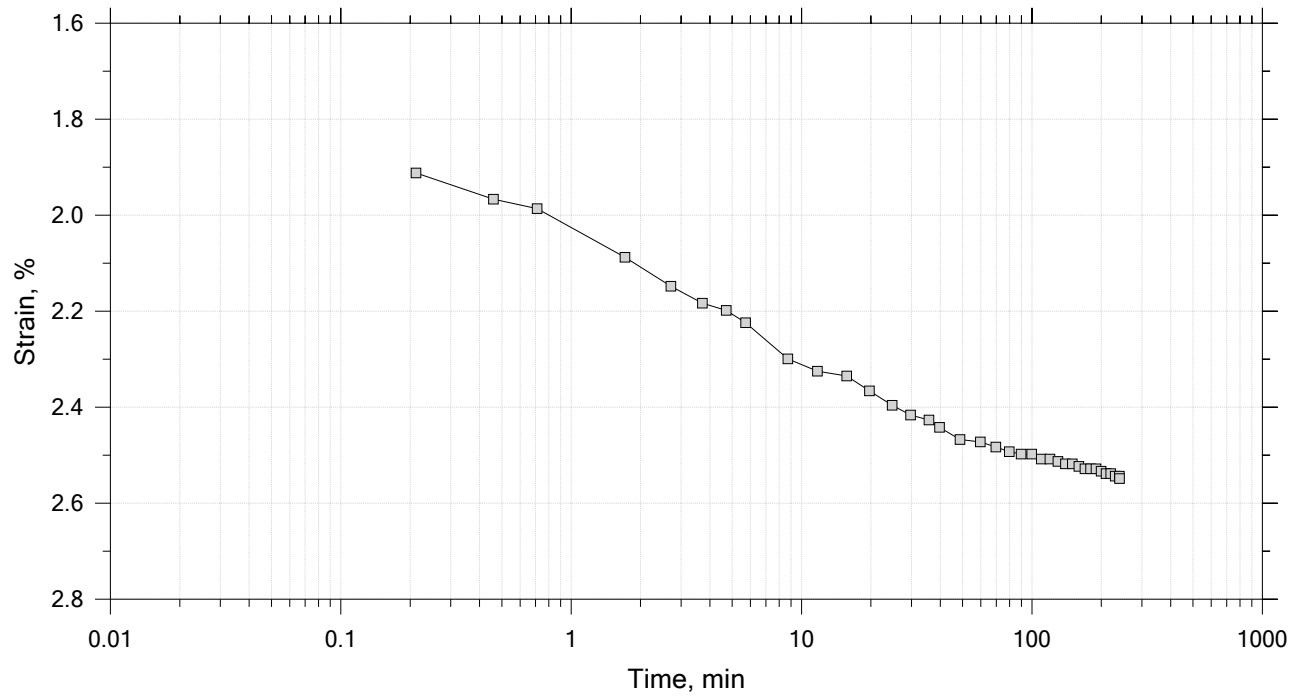
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



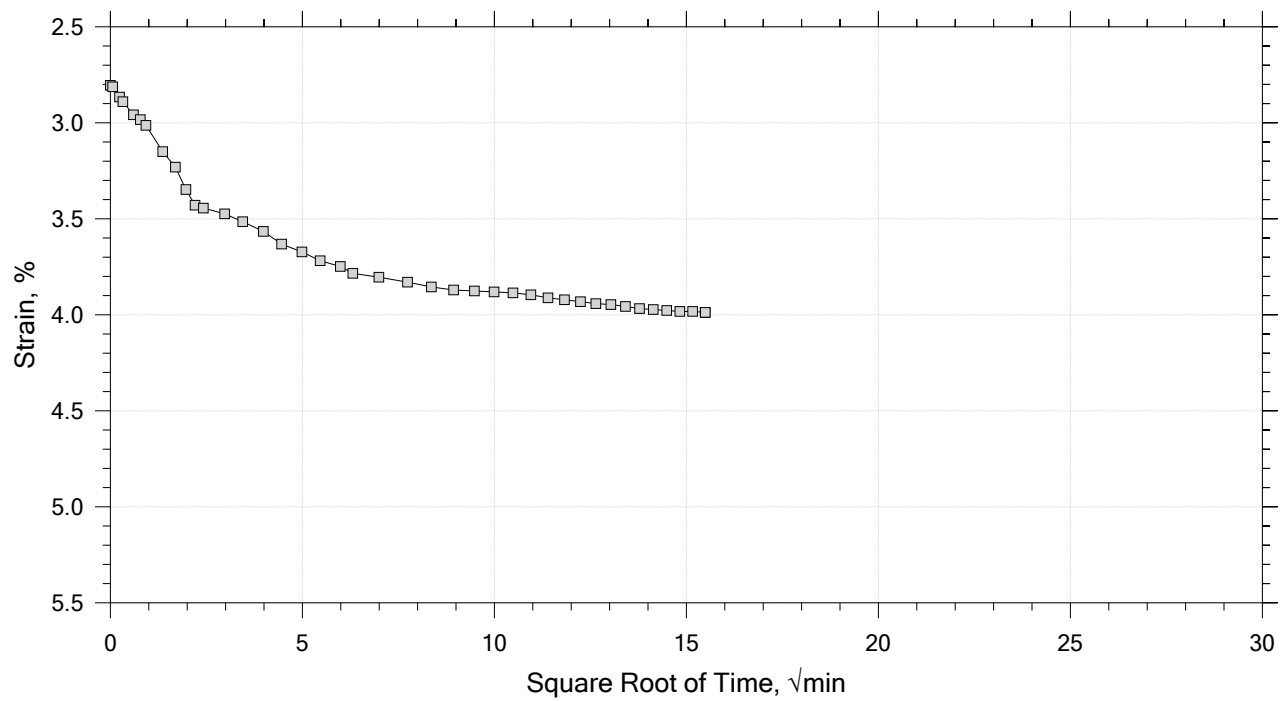
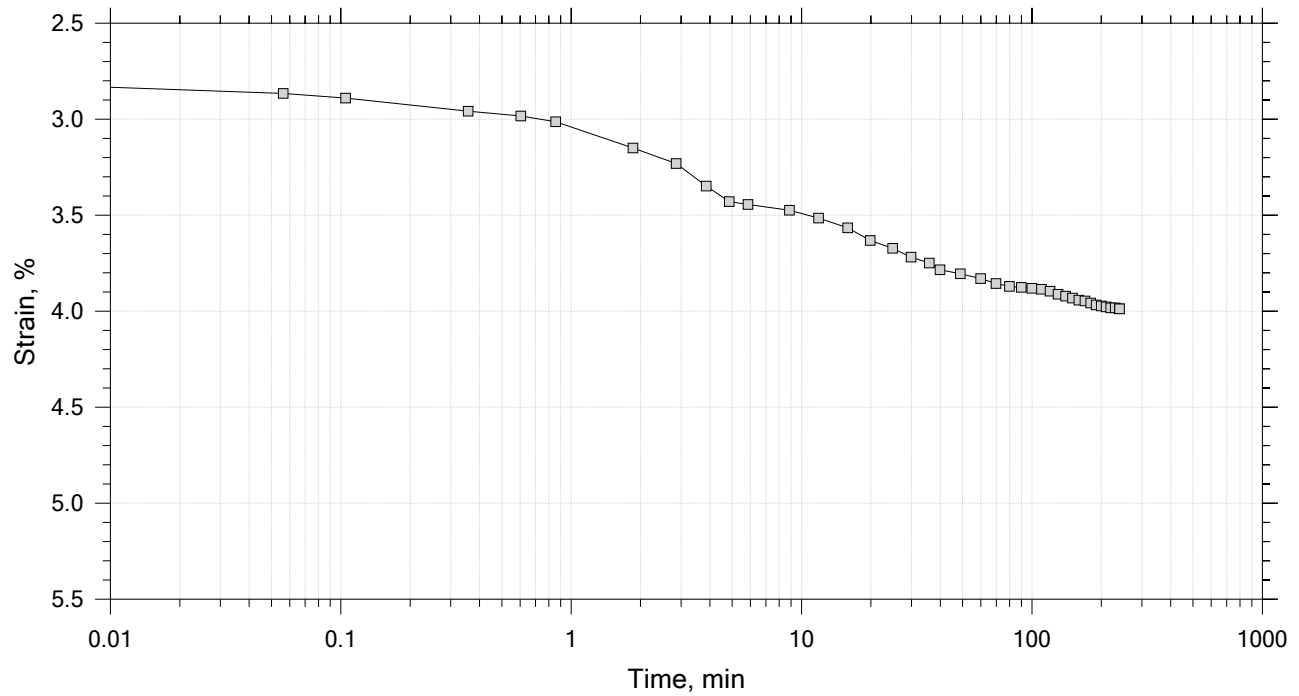
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HD-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



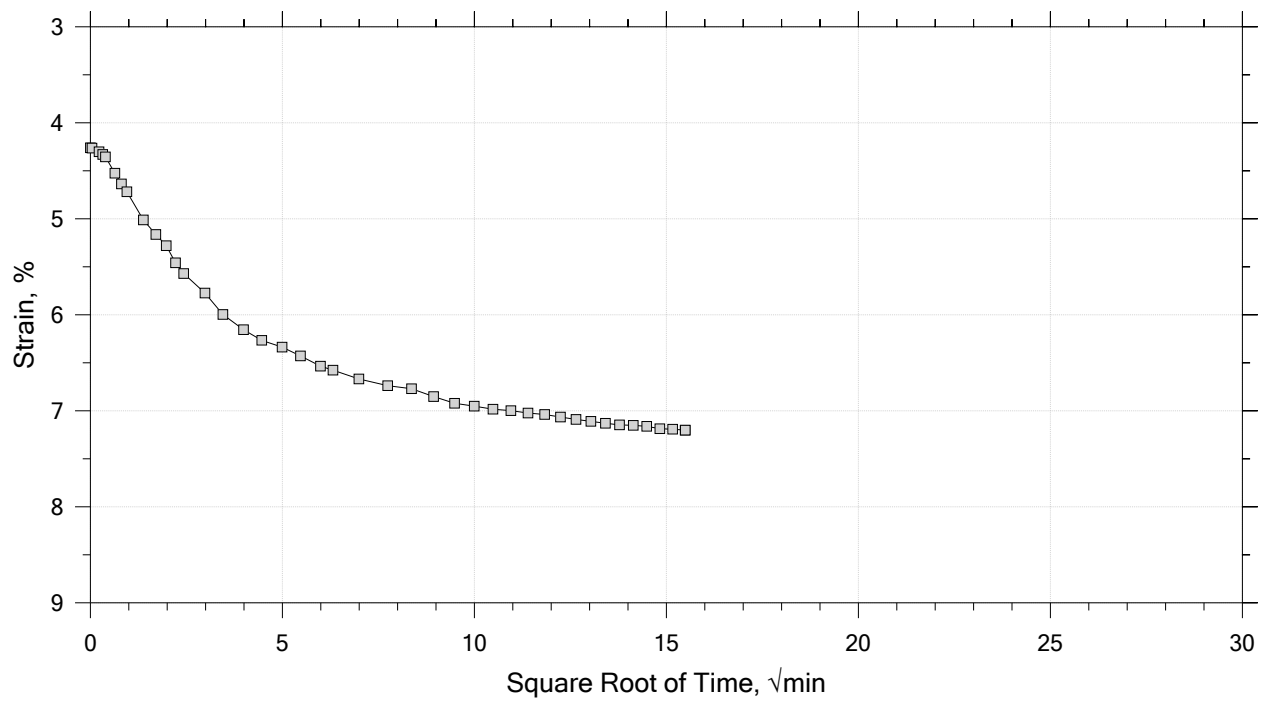
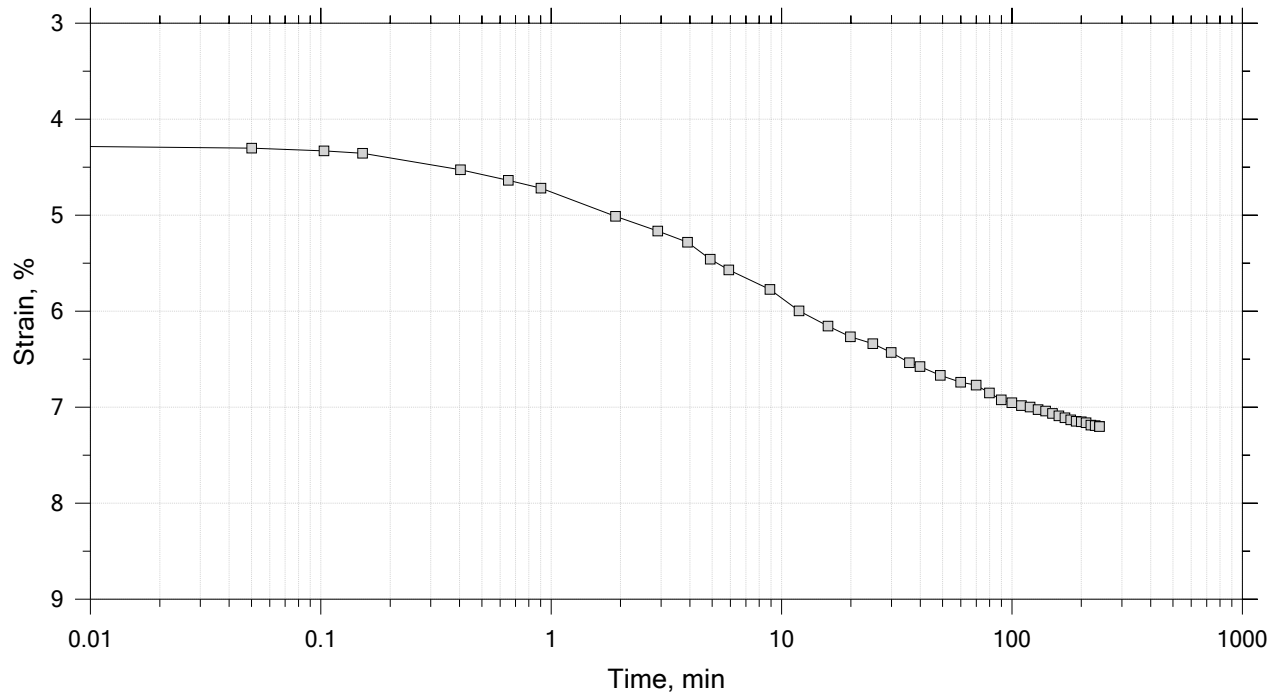
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



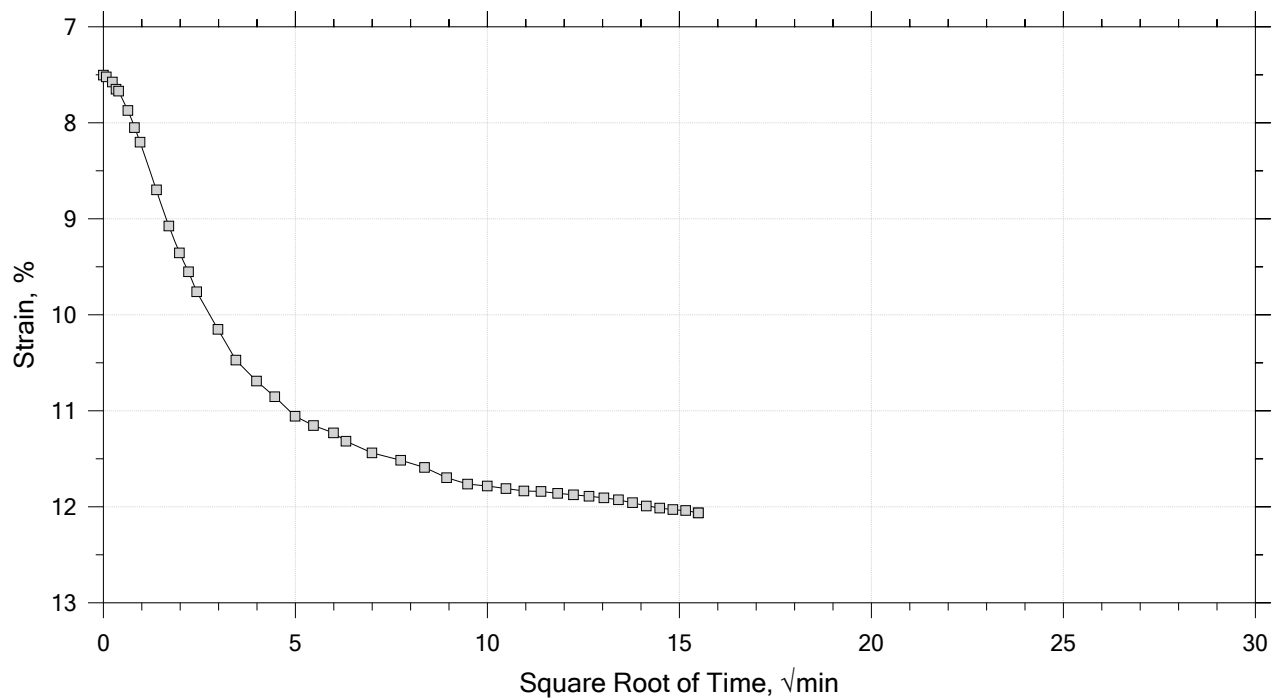
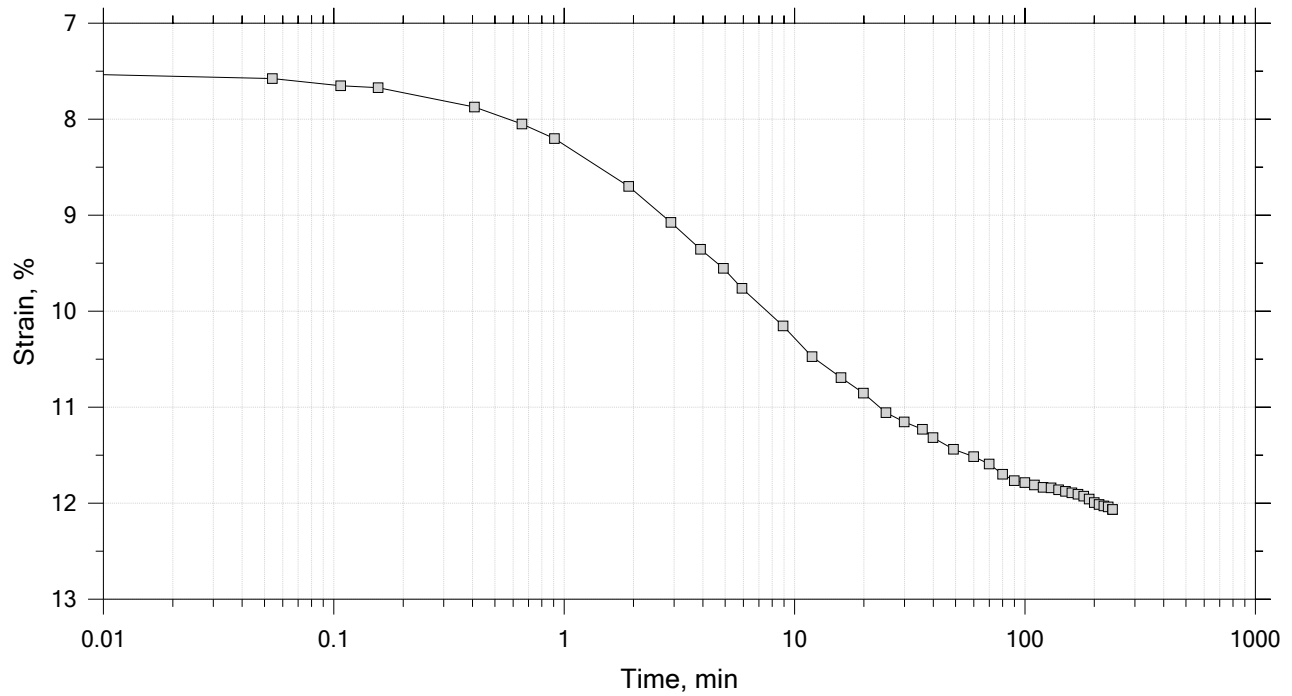
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



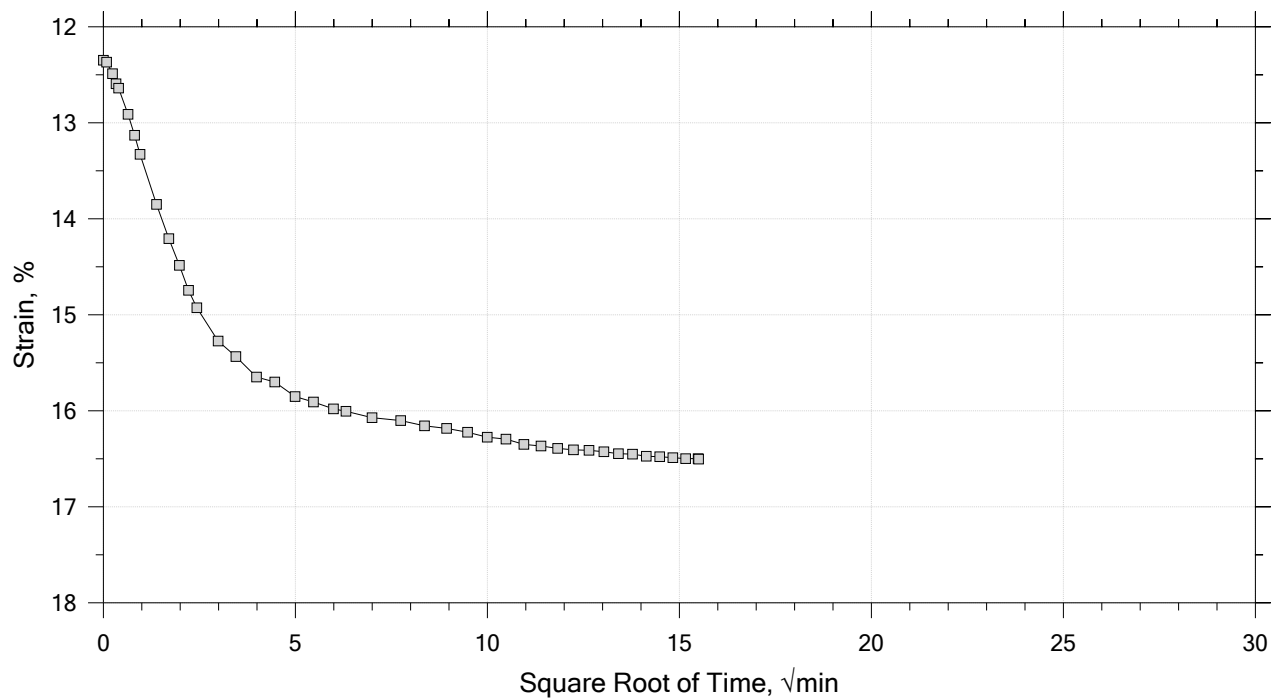
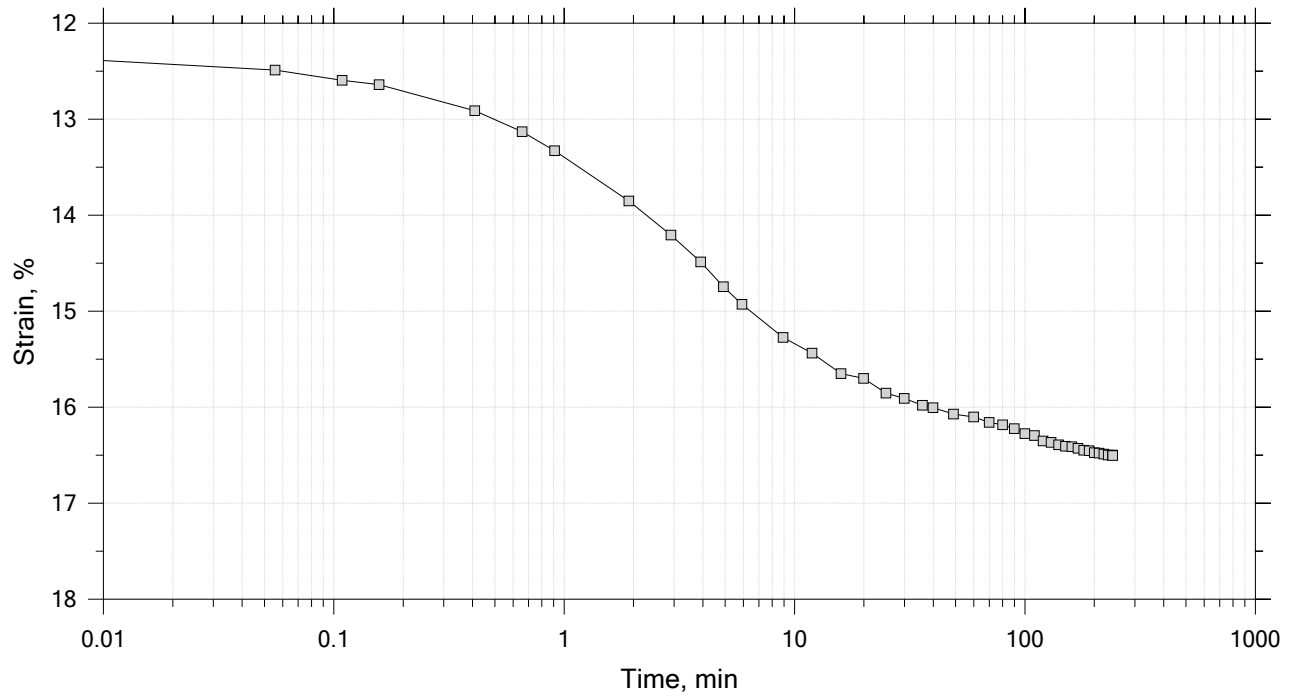
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



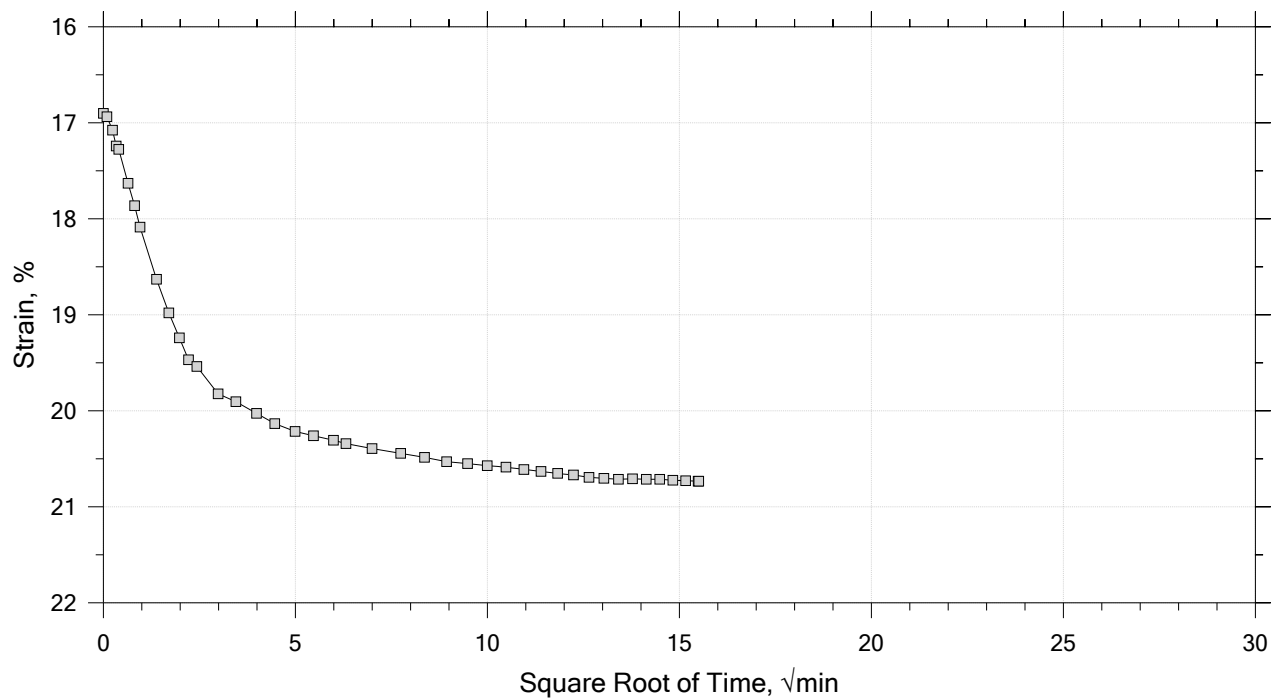
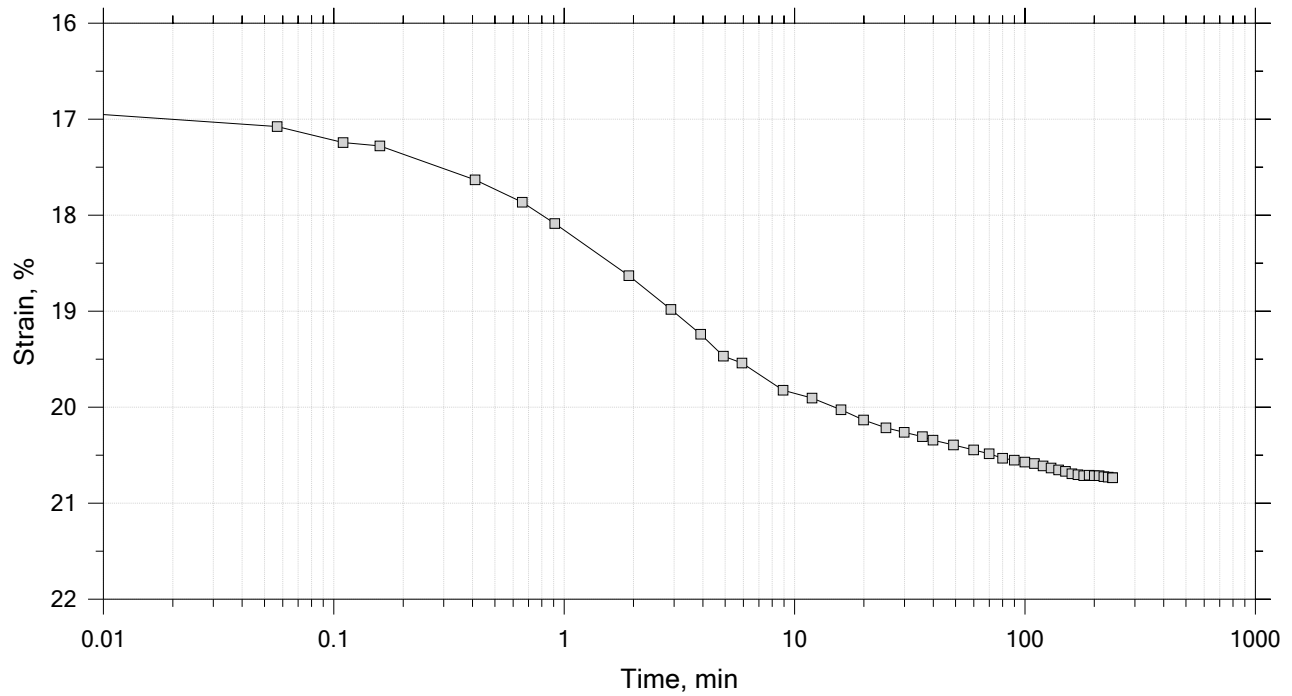
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



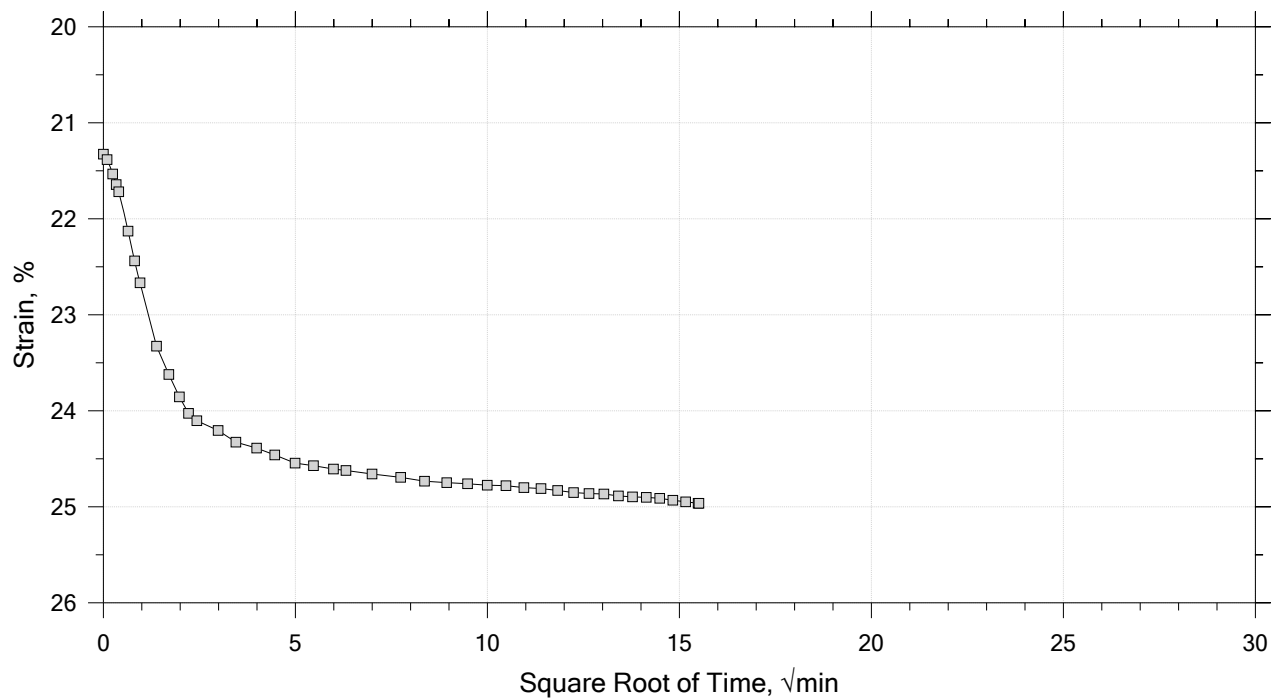
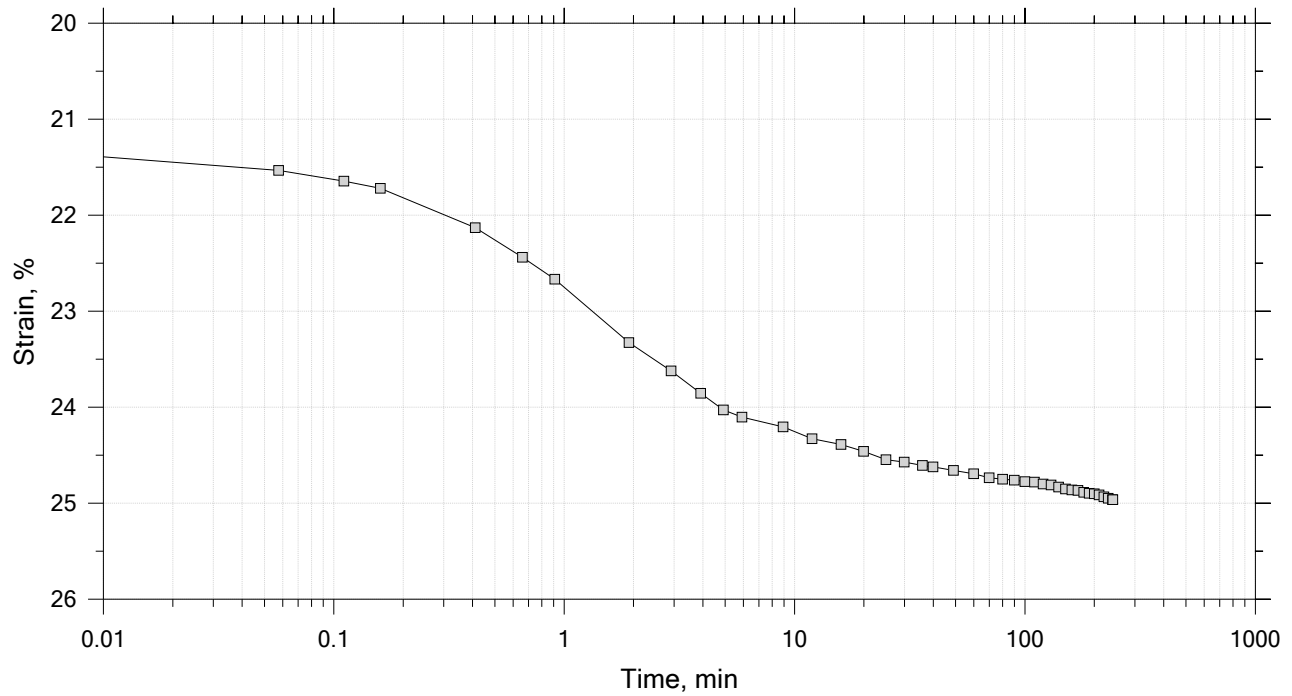
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



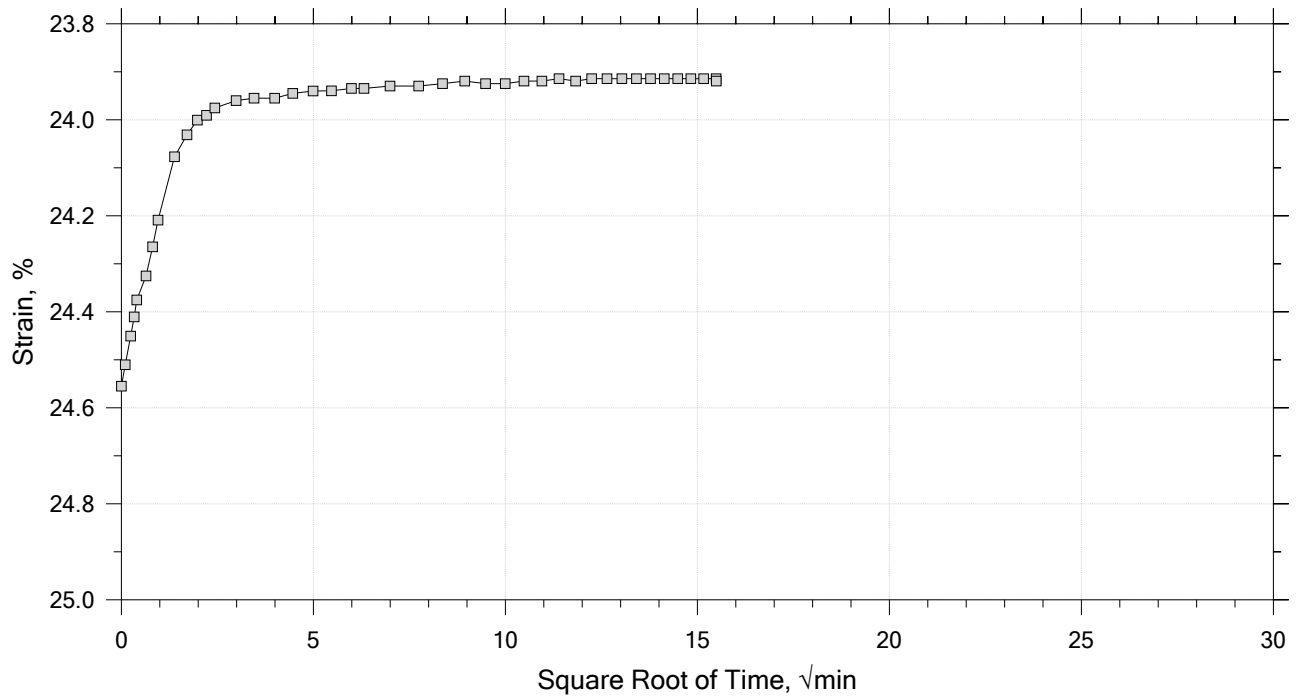
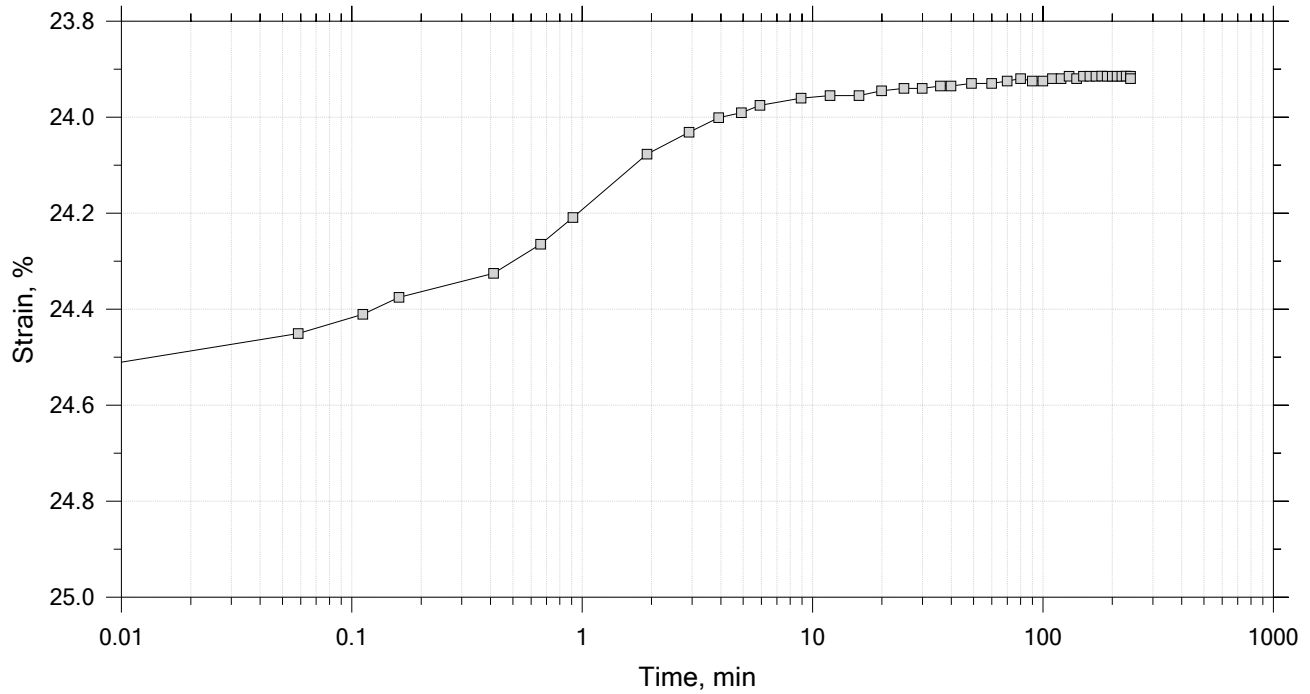
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



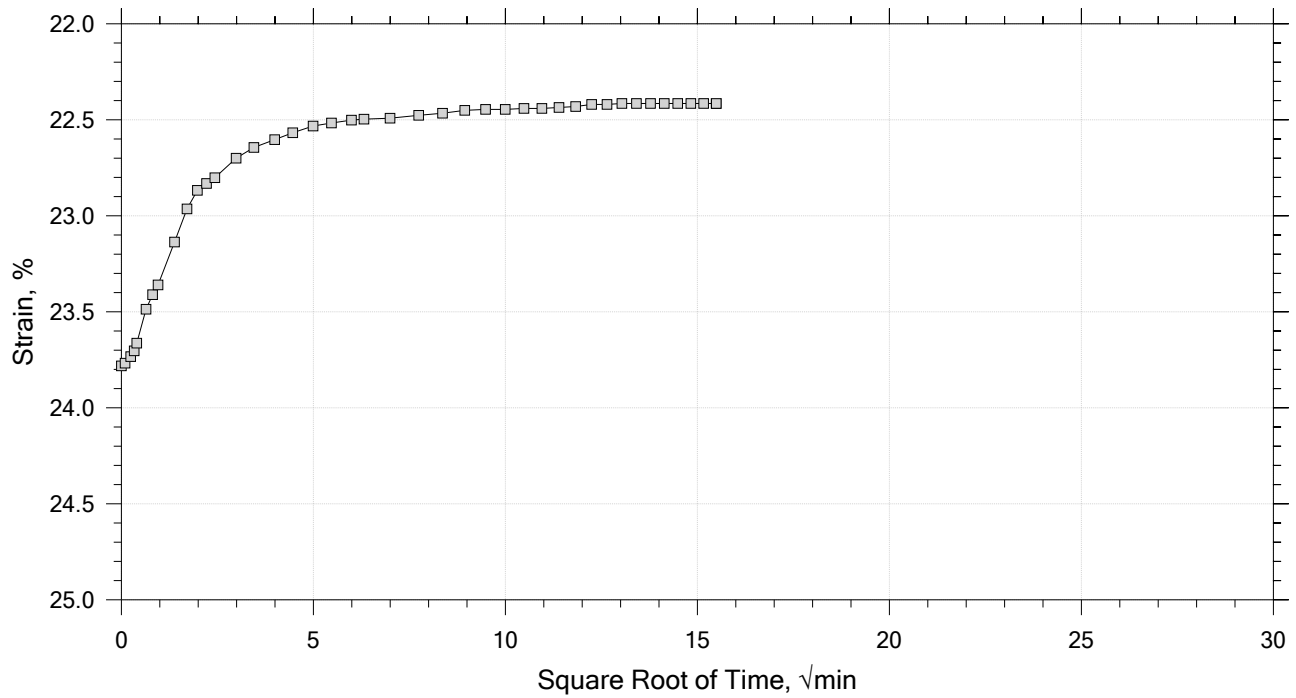
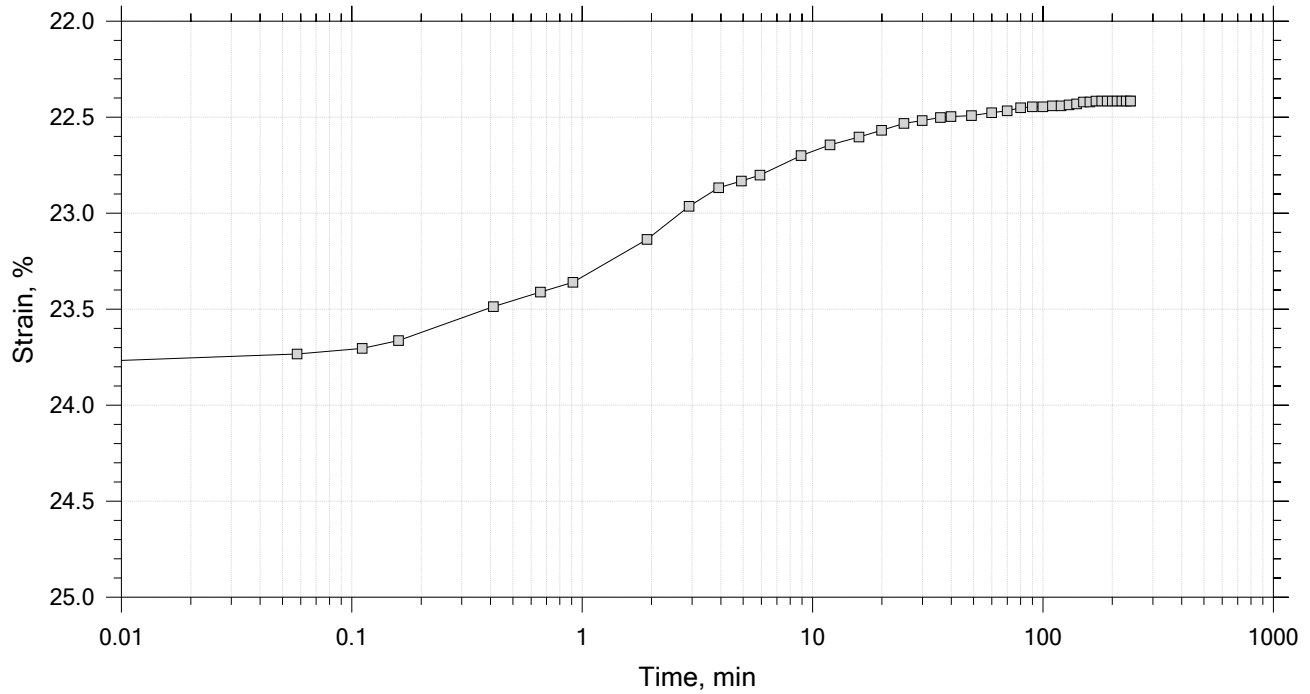
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



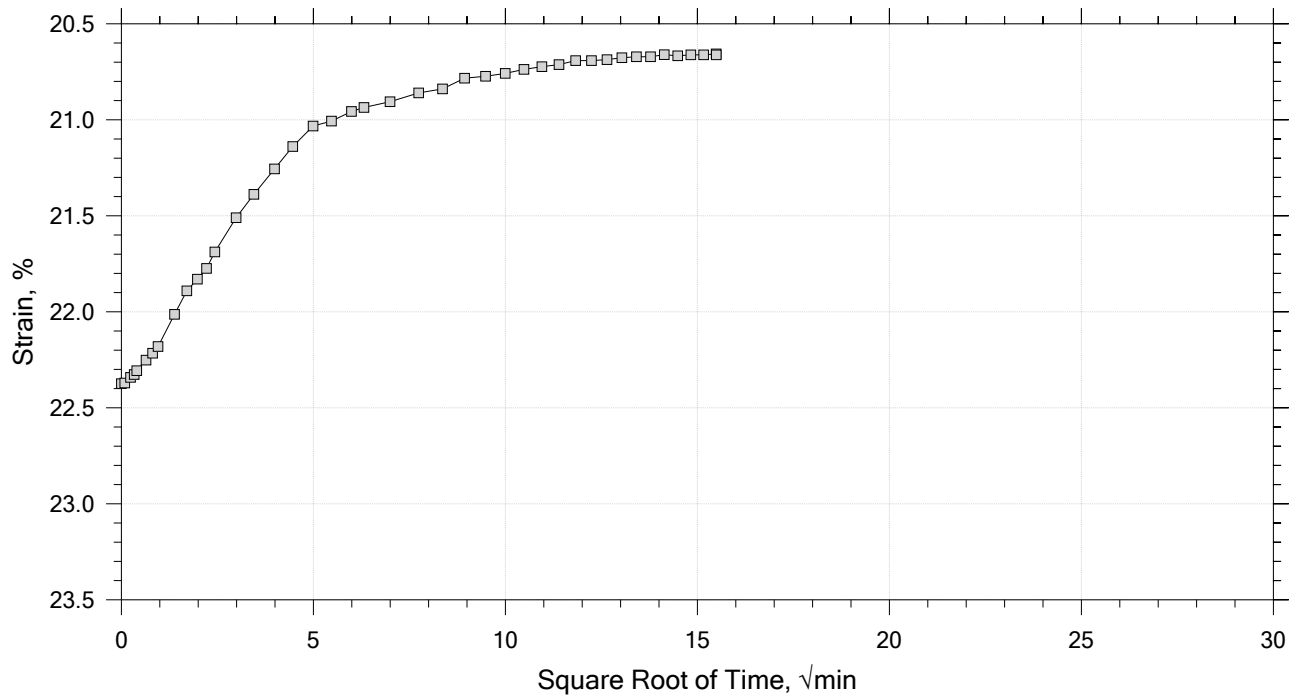
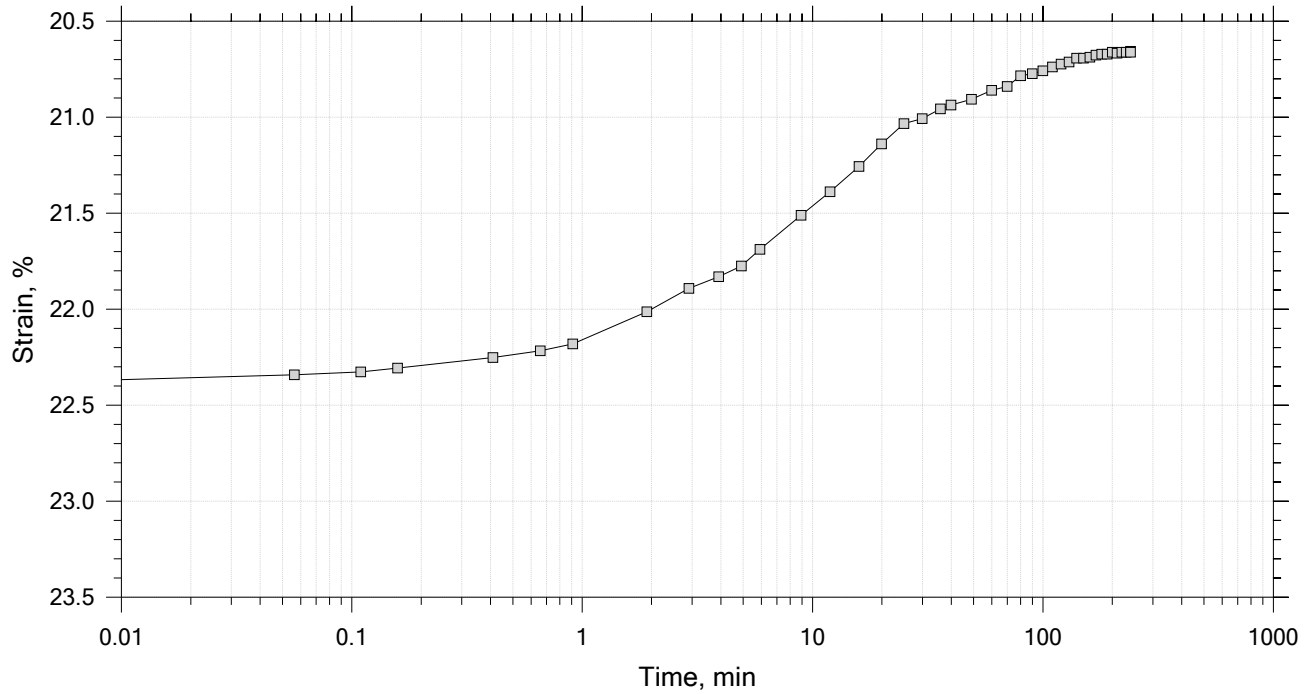
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



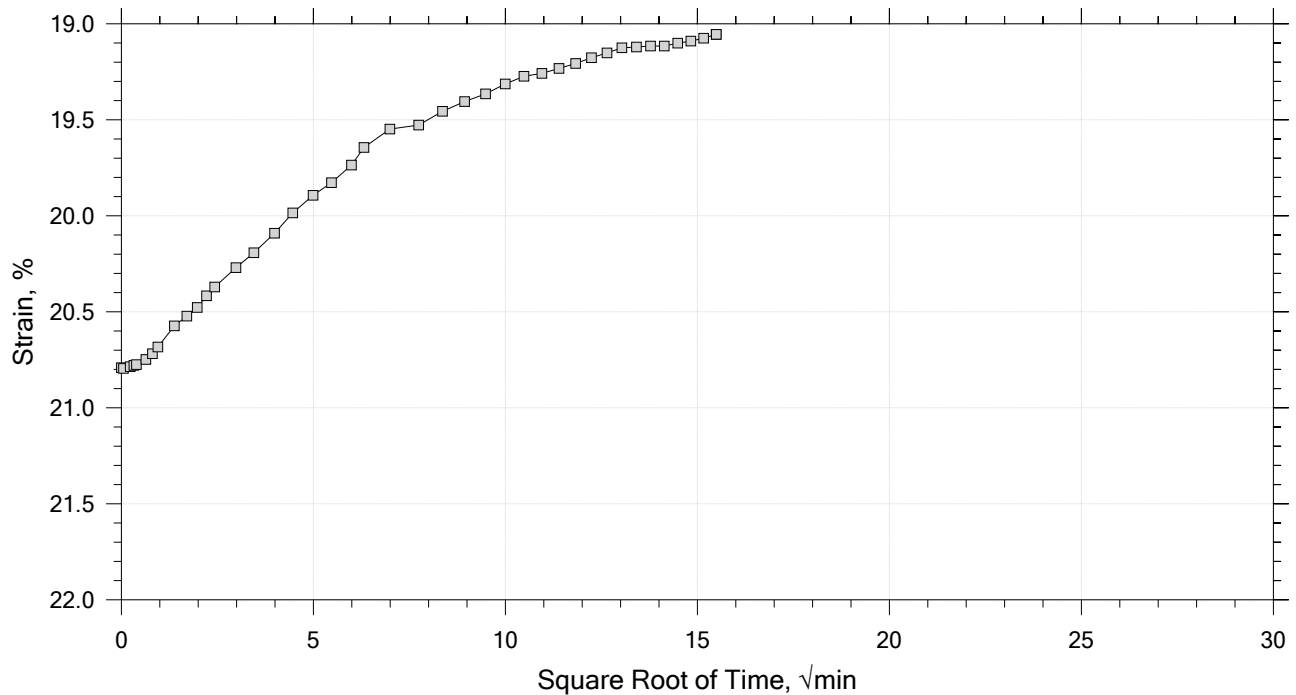
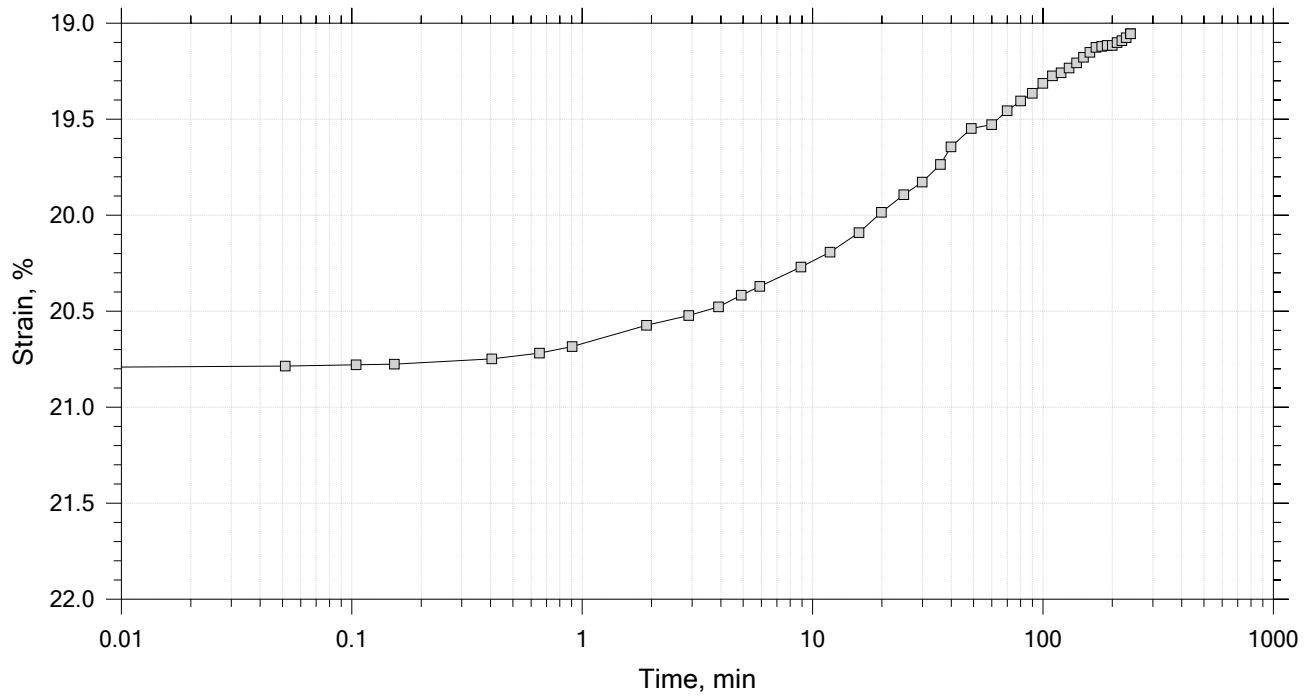
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



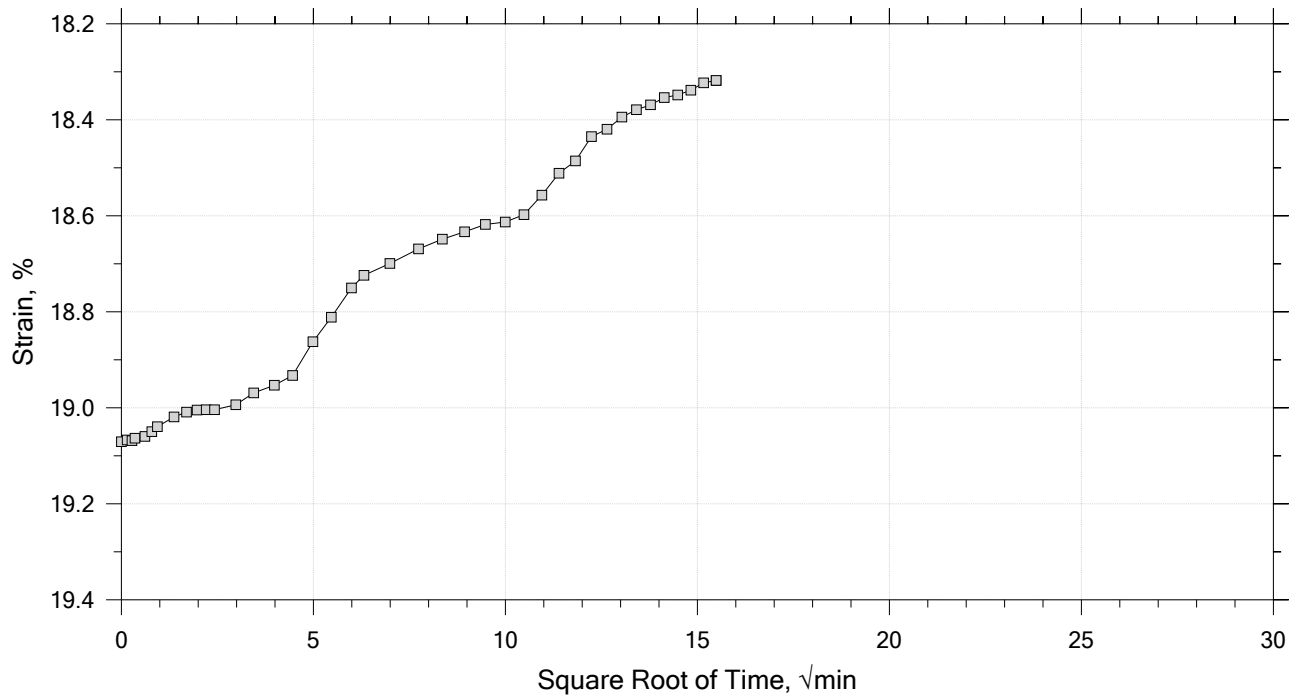
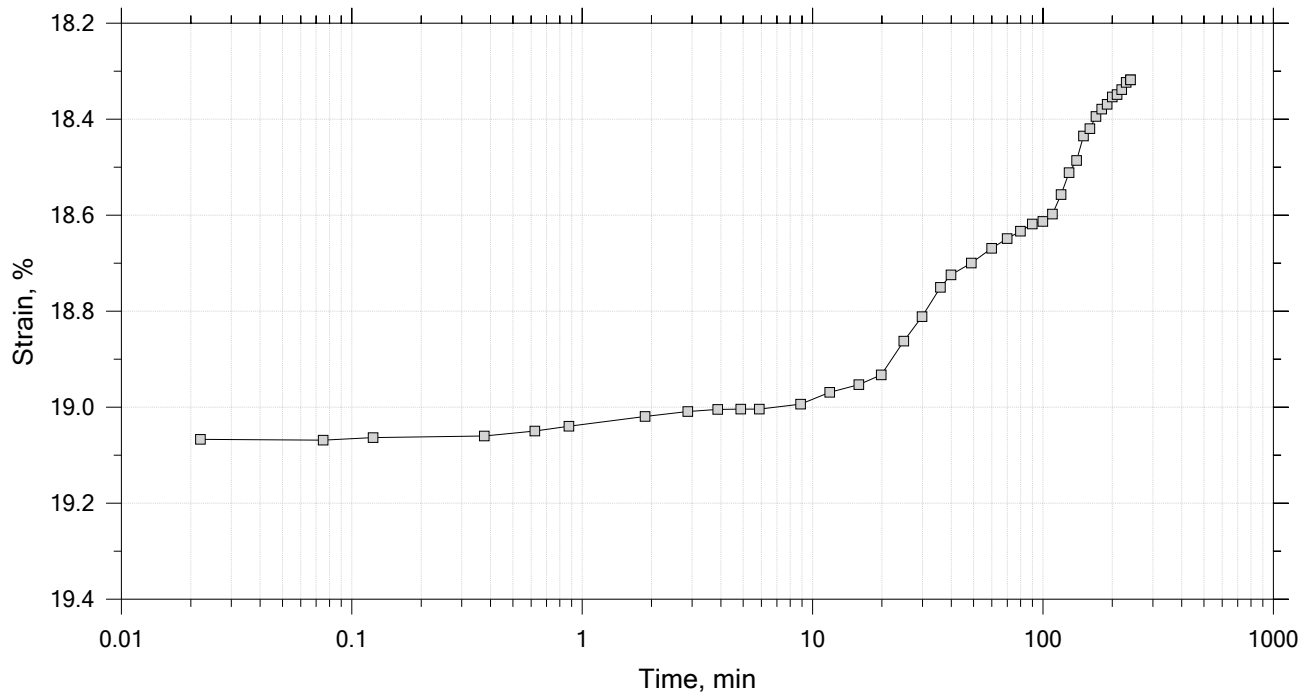
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.77	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 1.09	Plastic Limit: 20
Final Height: 0.83 in	Final Void Ratio: 0.737	Plasticity Index: 15

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	B-2002	RING		D1550
Mass Container, gm	8.26	111.18	111.18	8.32
Mass Container + Wet Soil, gm	149.69	259.44	245.93	142.05
Mass Container + Dry Soil, gm	108.98	217.6	217.6	113.93
Mass Dry Soil, gm	100.72	106.42	106.42	105.61
Water Content, %	40.42	39.32	26.63	26.63
Void Ratio	---	1.09	0.74	---
Degree of Saturation, %	---	99.61	100.00	---
Dry Unit Weight, pcf	---	82.587	99.503	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1 B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
Remarks: System T, Swell Pressure = 0.0626 tsf			
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

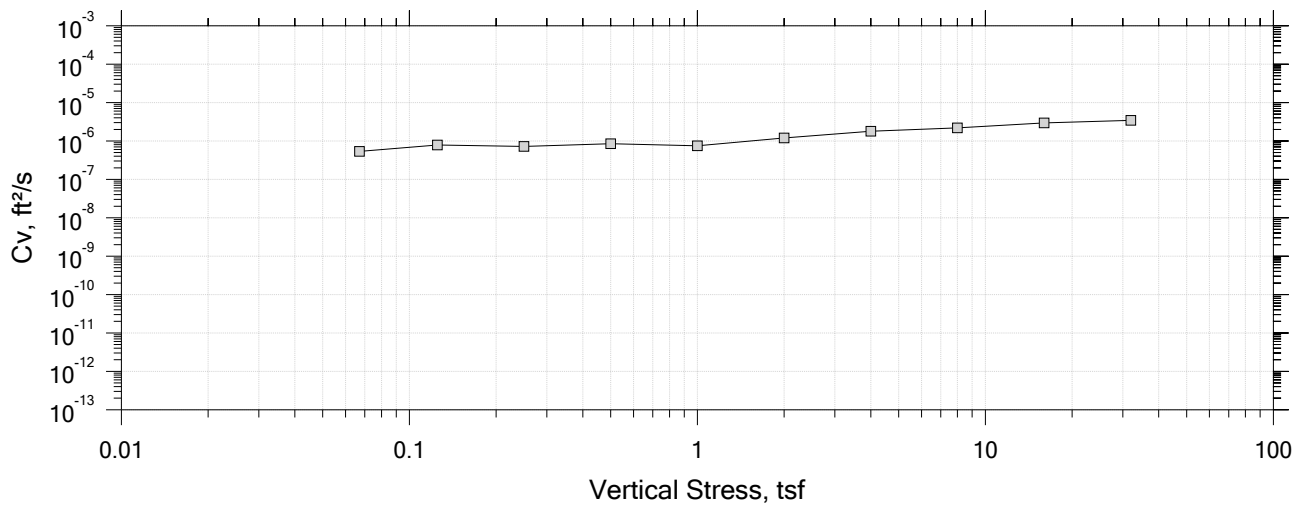
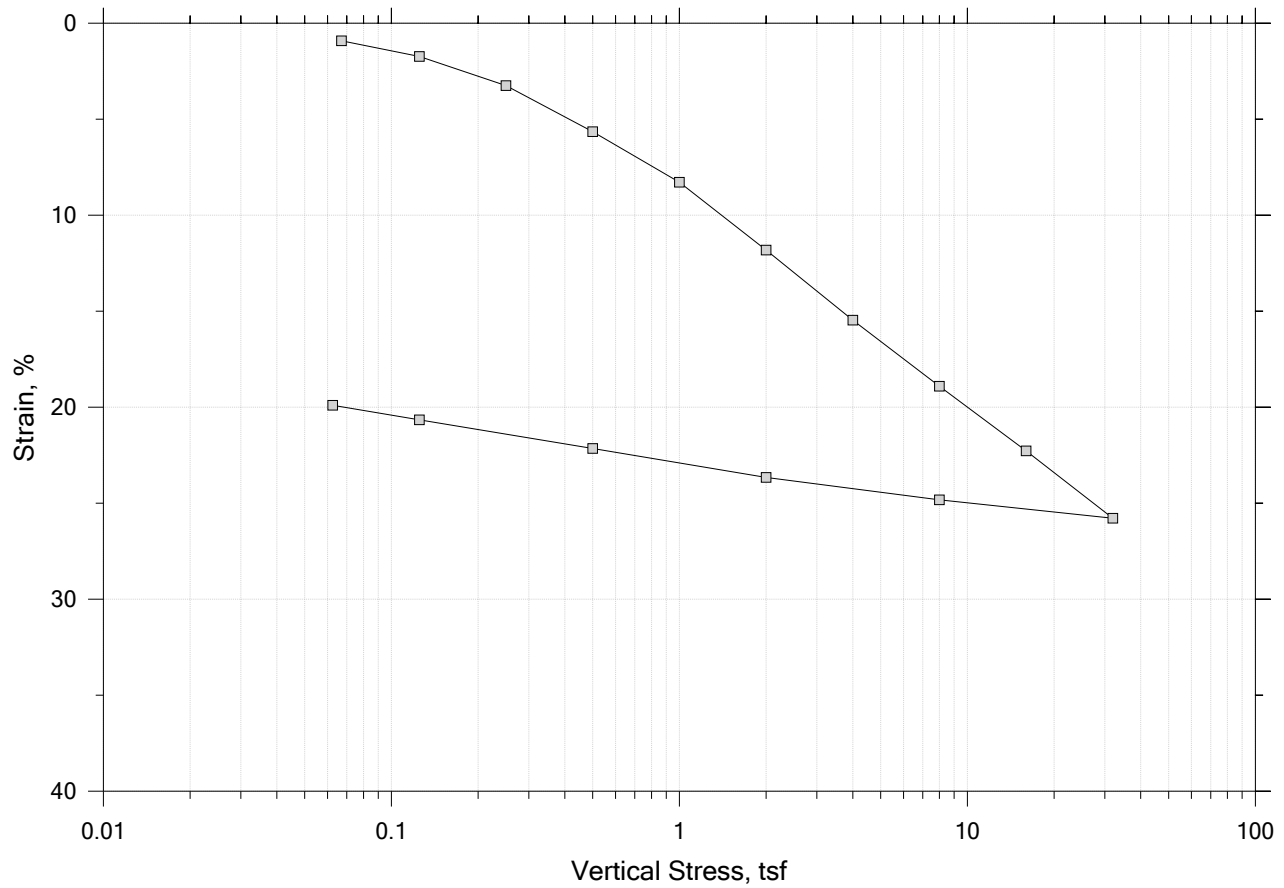
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BFB-101 BB-BFB-101	Tested By: md	Checked By: mcm
	Sample No.: 2U	Test Date: 09/26/18	Depth: 12-14 ft
	Test No.: IP-1B	Sample Type: intact	Elevation: ---
	Description: Wet, very dark greenish gray clay		
	Remarks: System T, Swell Pressure = 0.0626 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

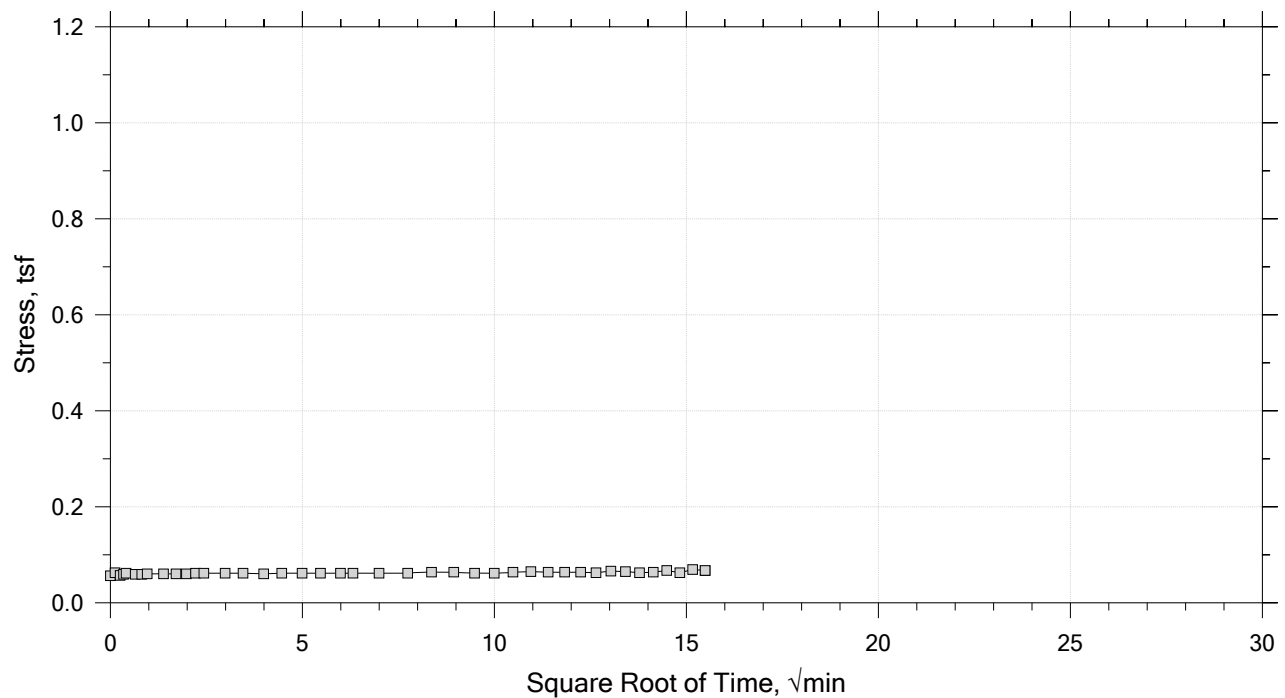
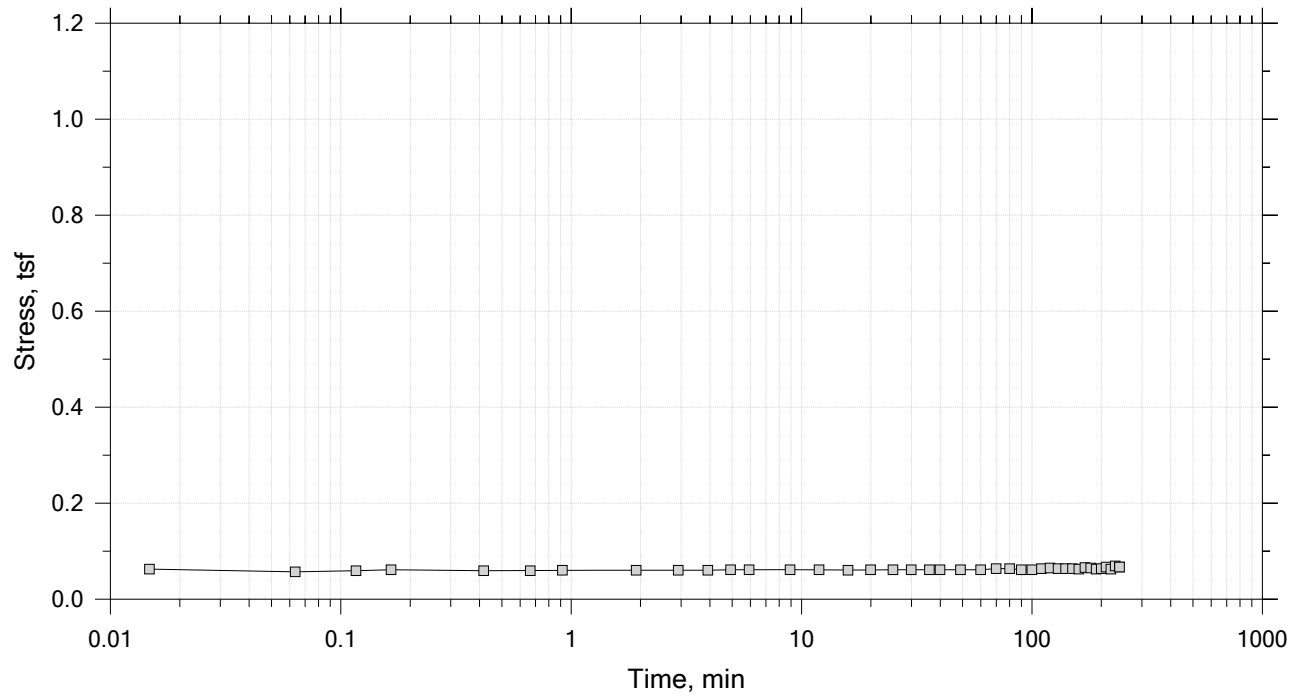
Summary Report




	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0671 tsf



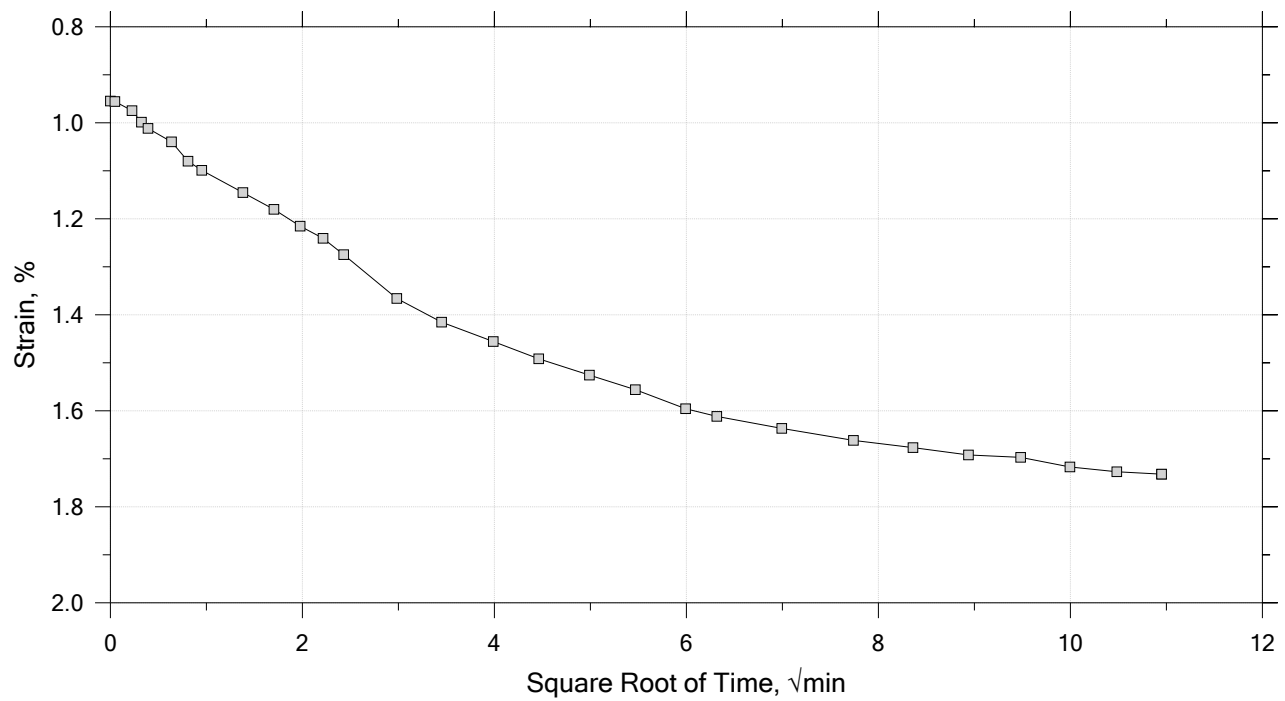
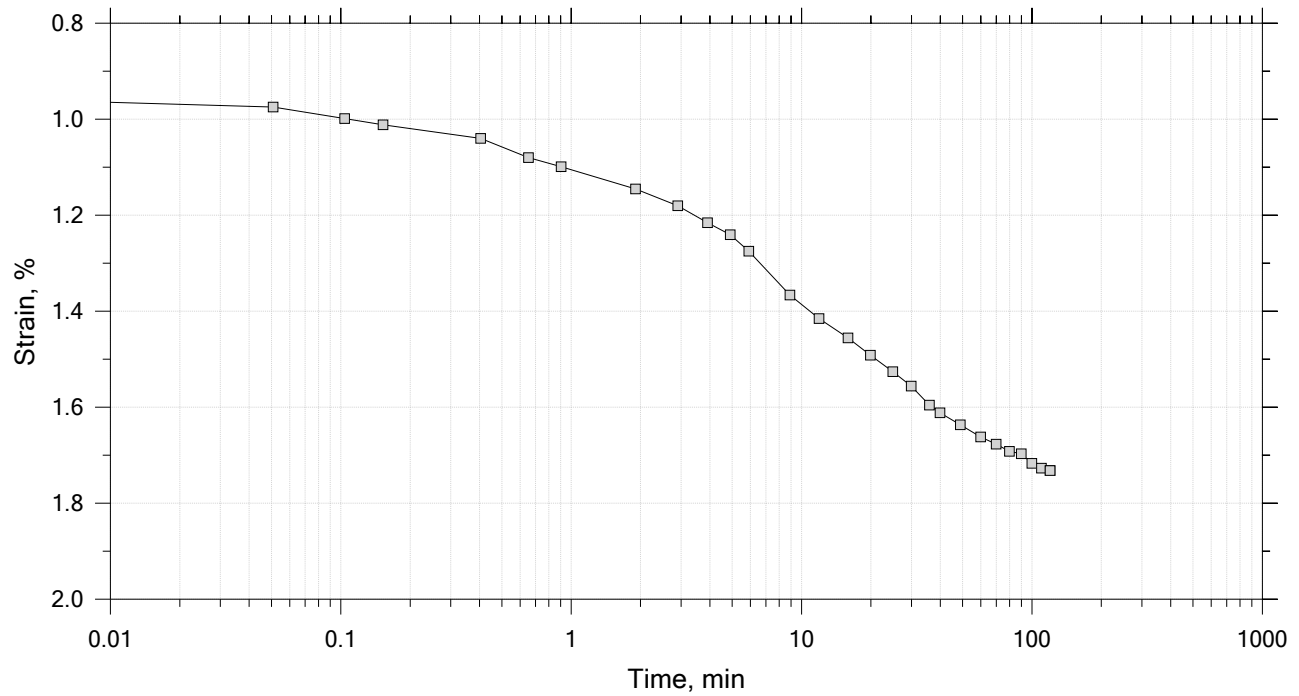
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



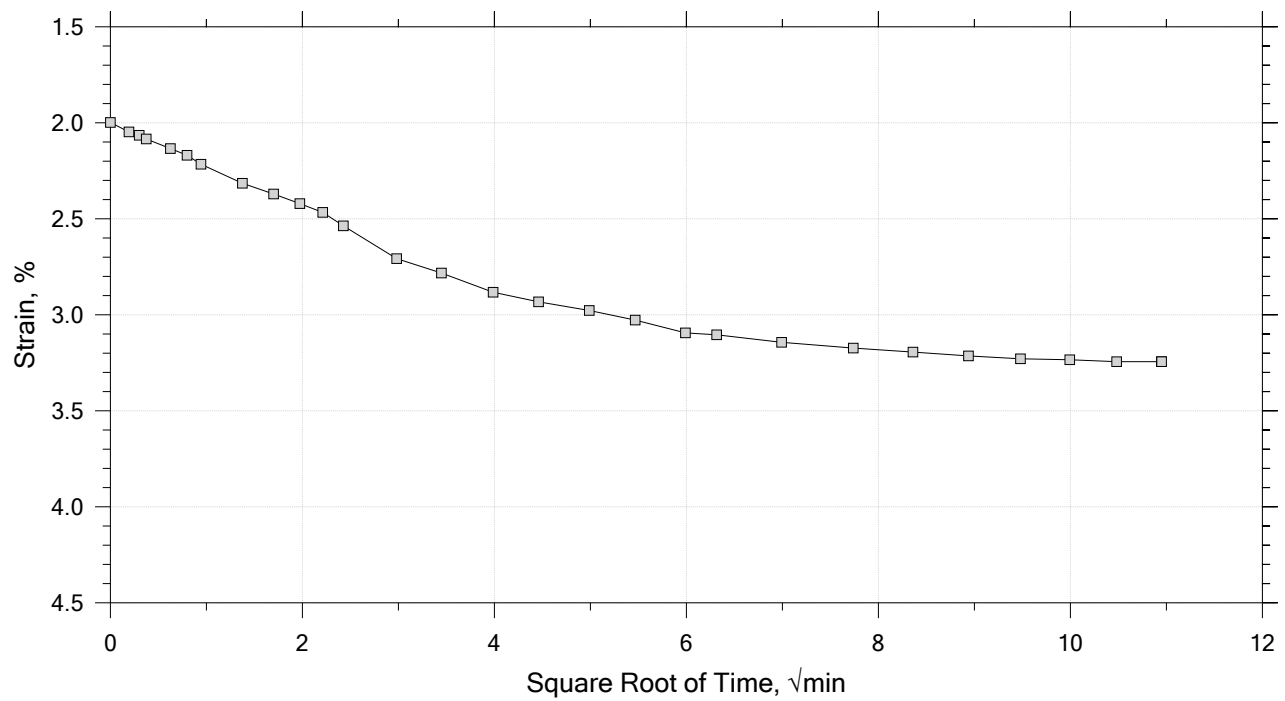
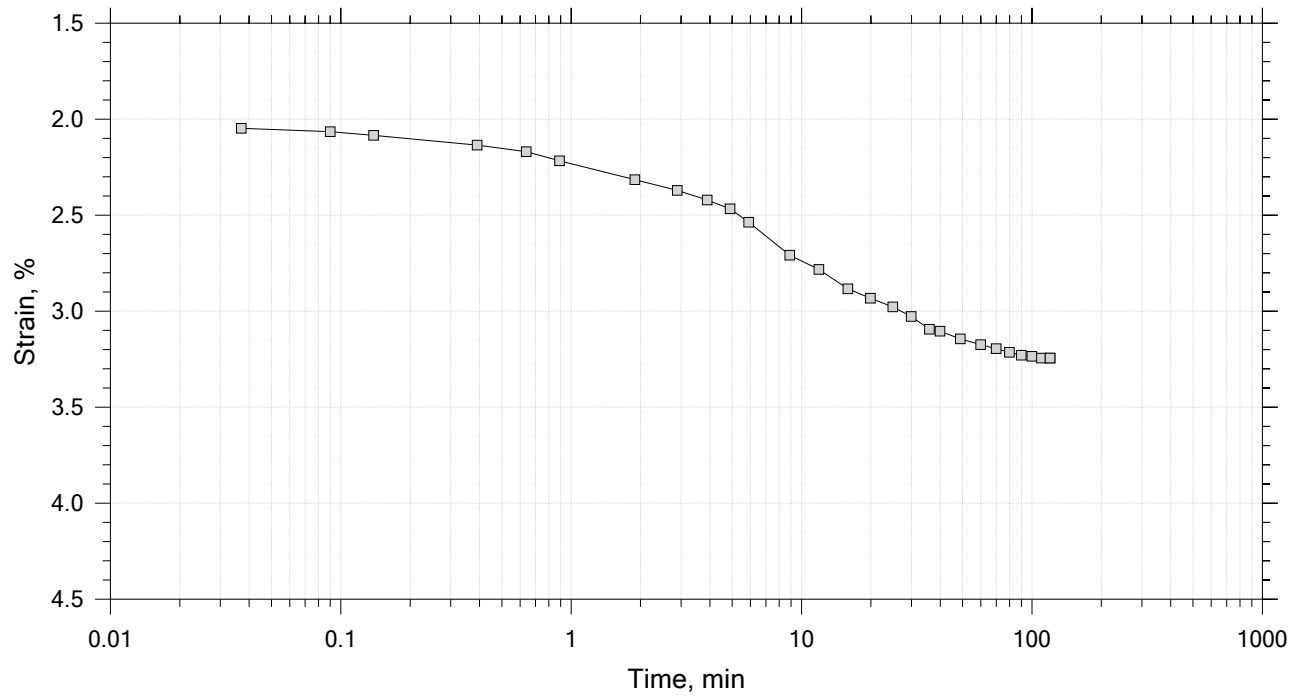
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



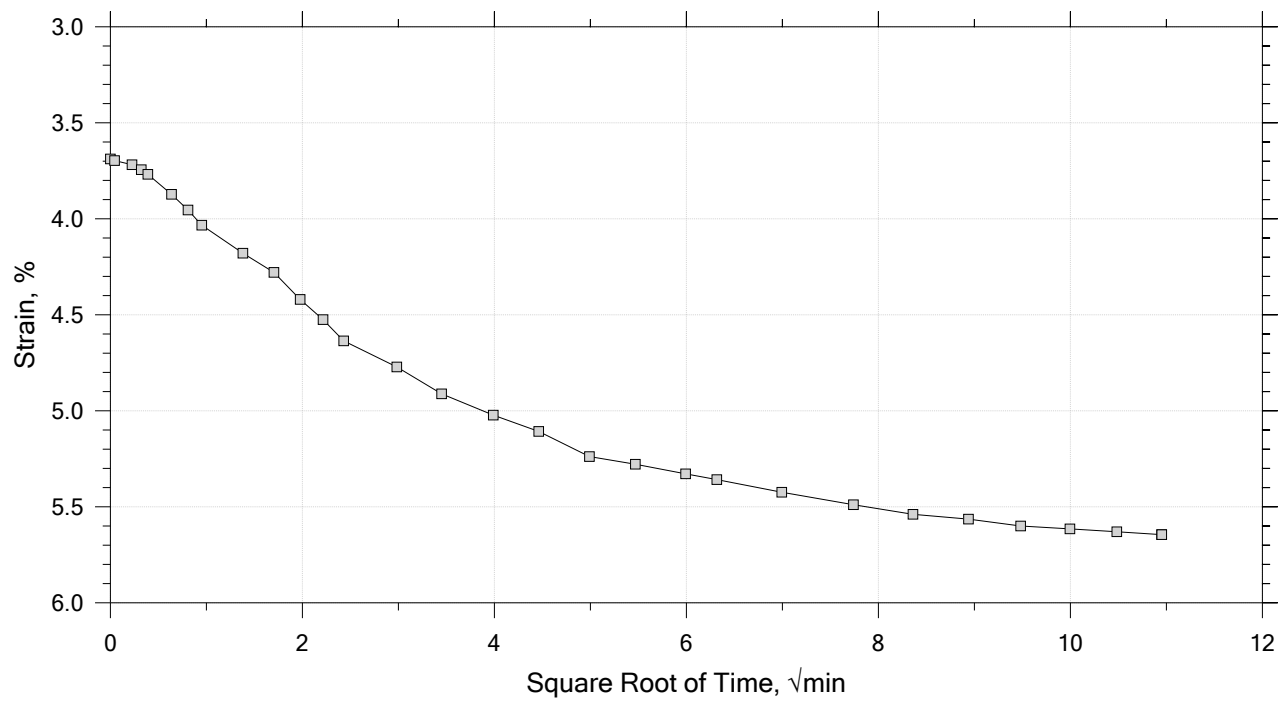
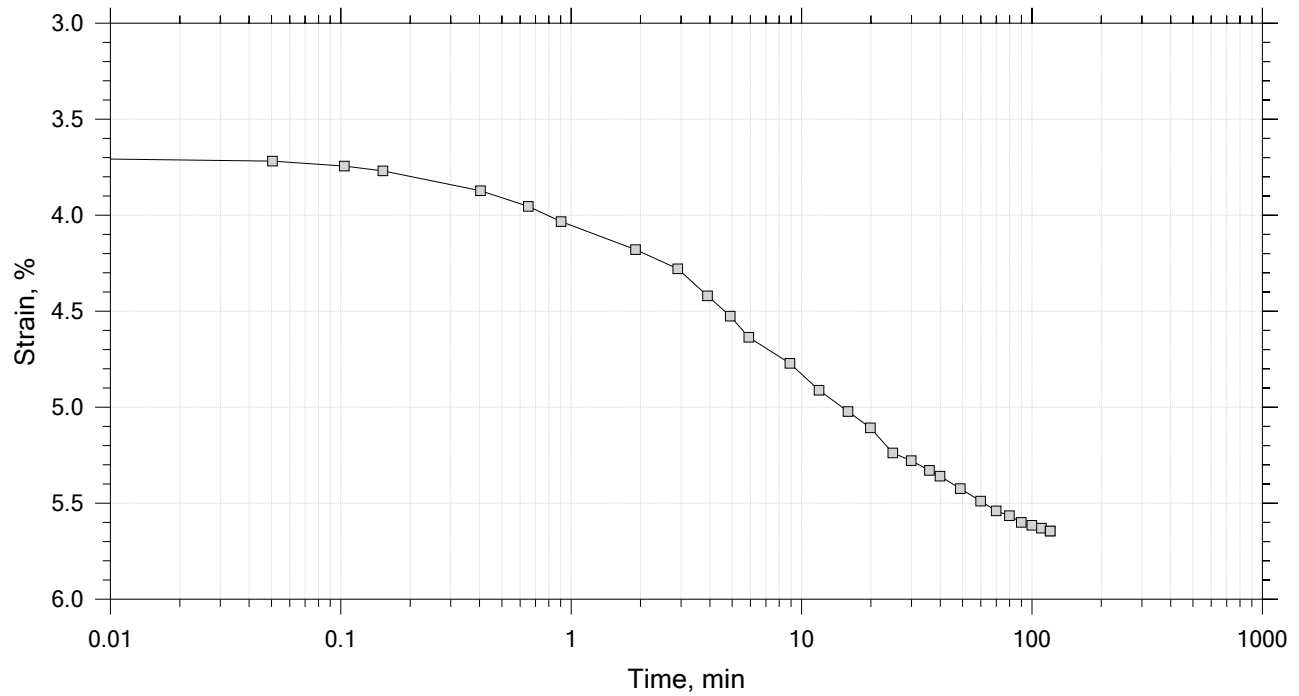
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HD-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



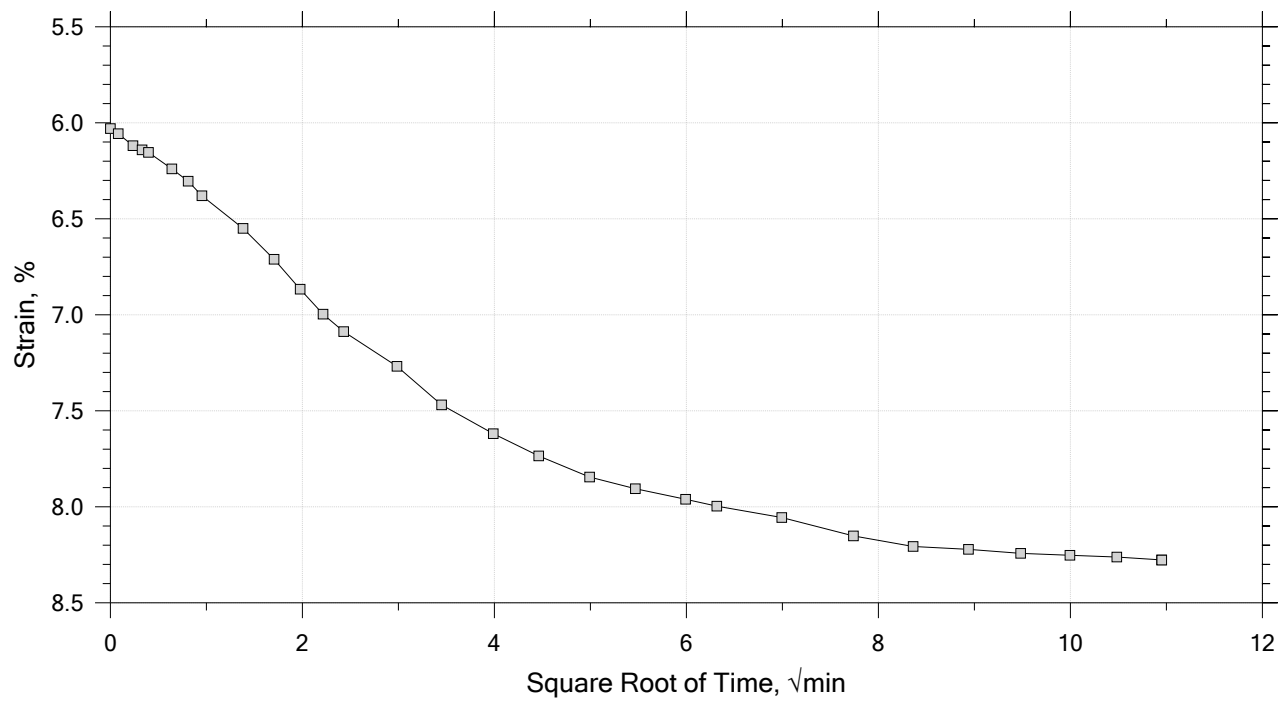
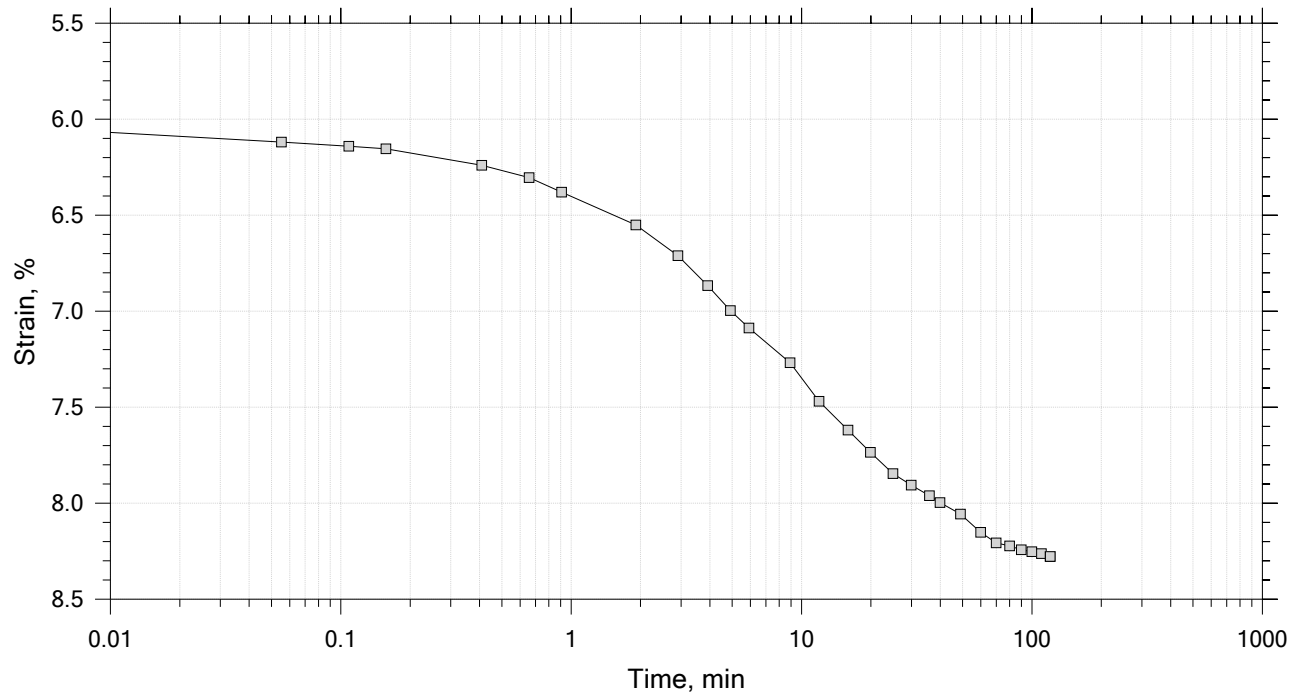
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



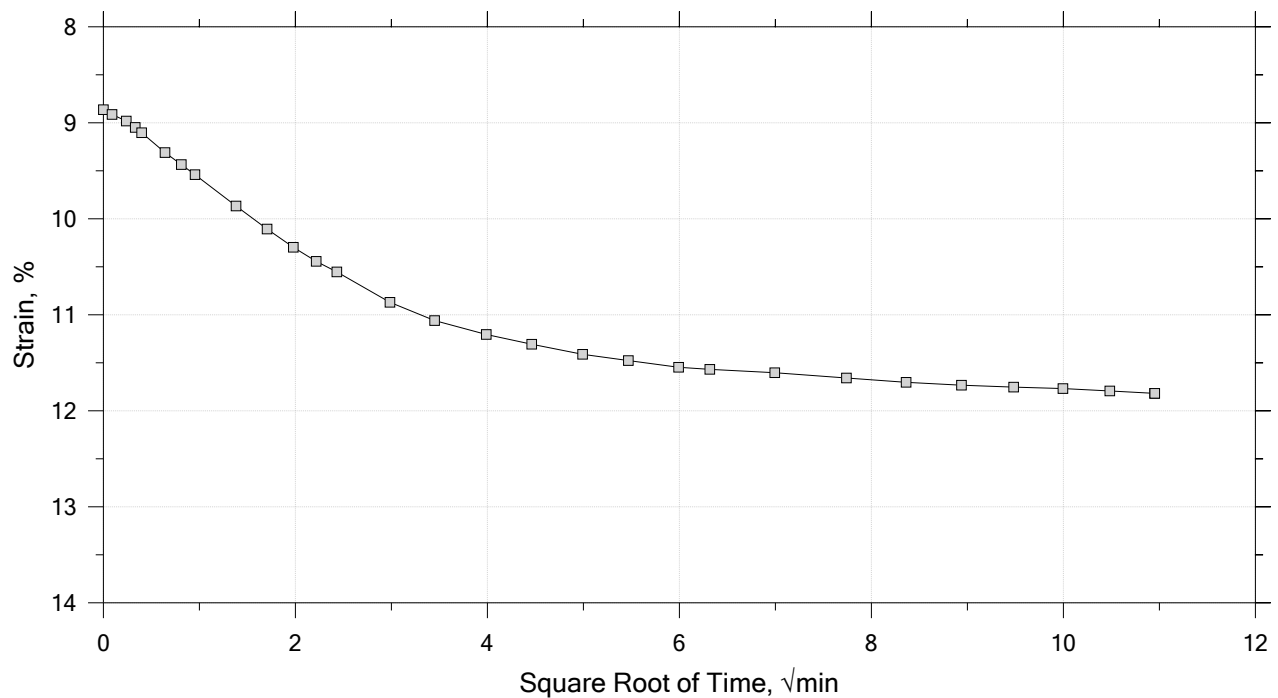
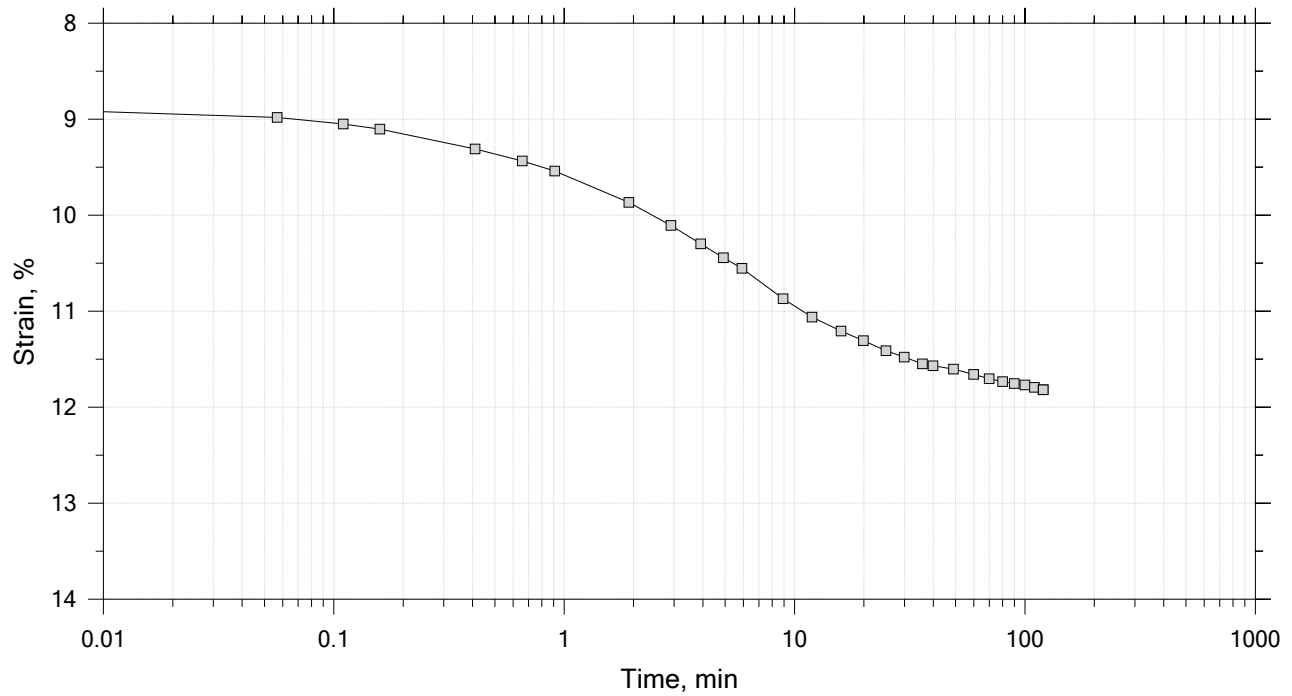
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



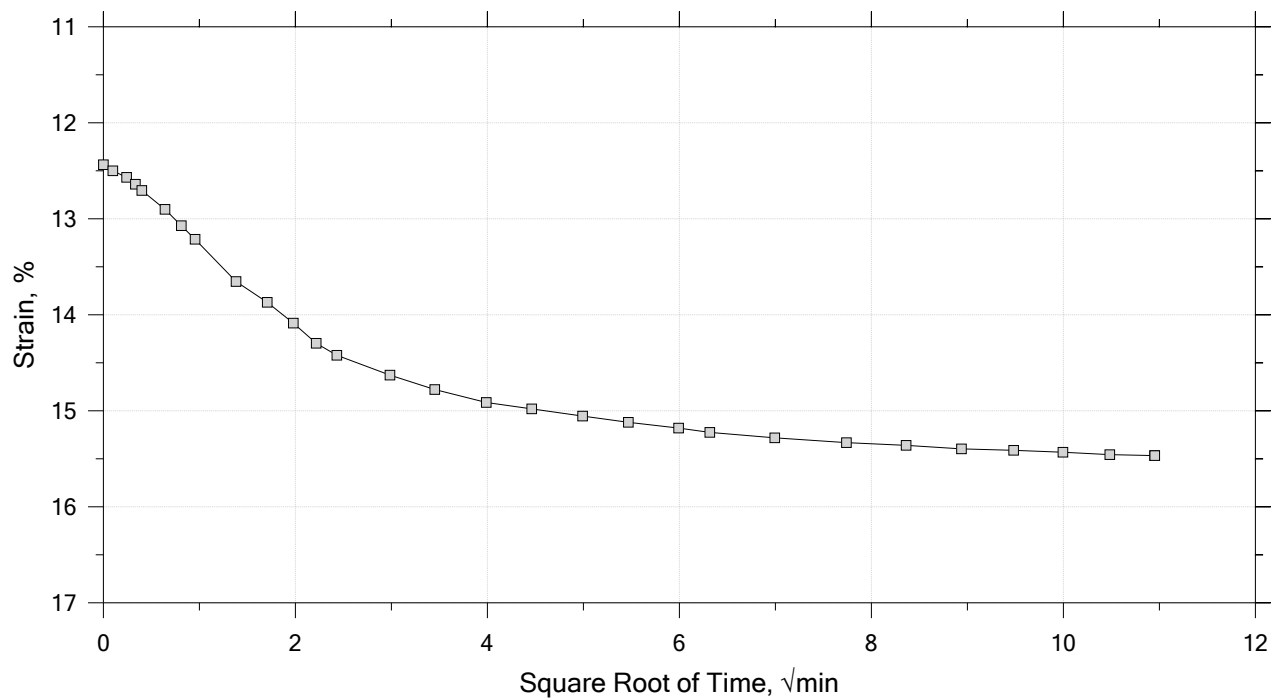
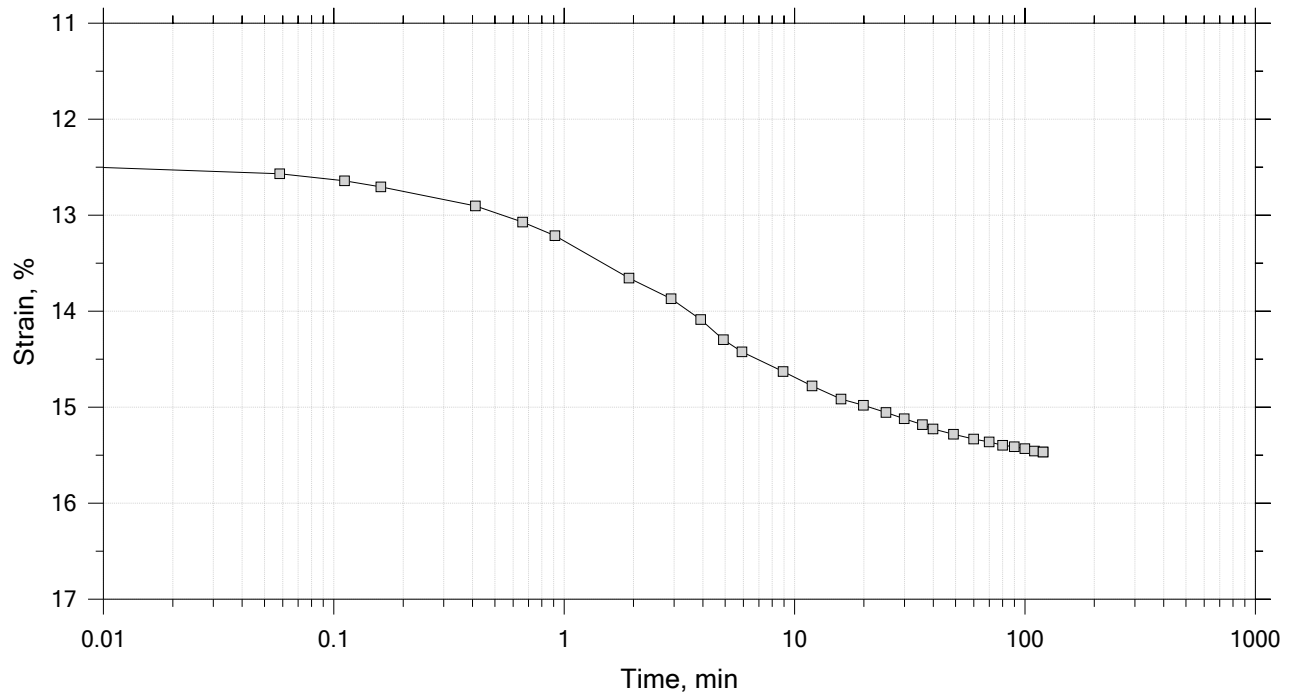
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



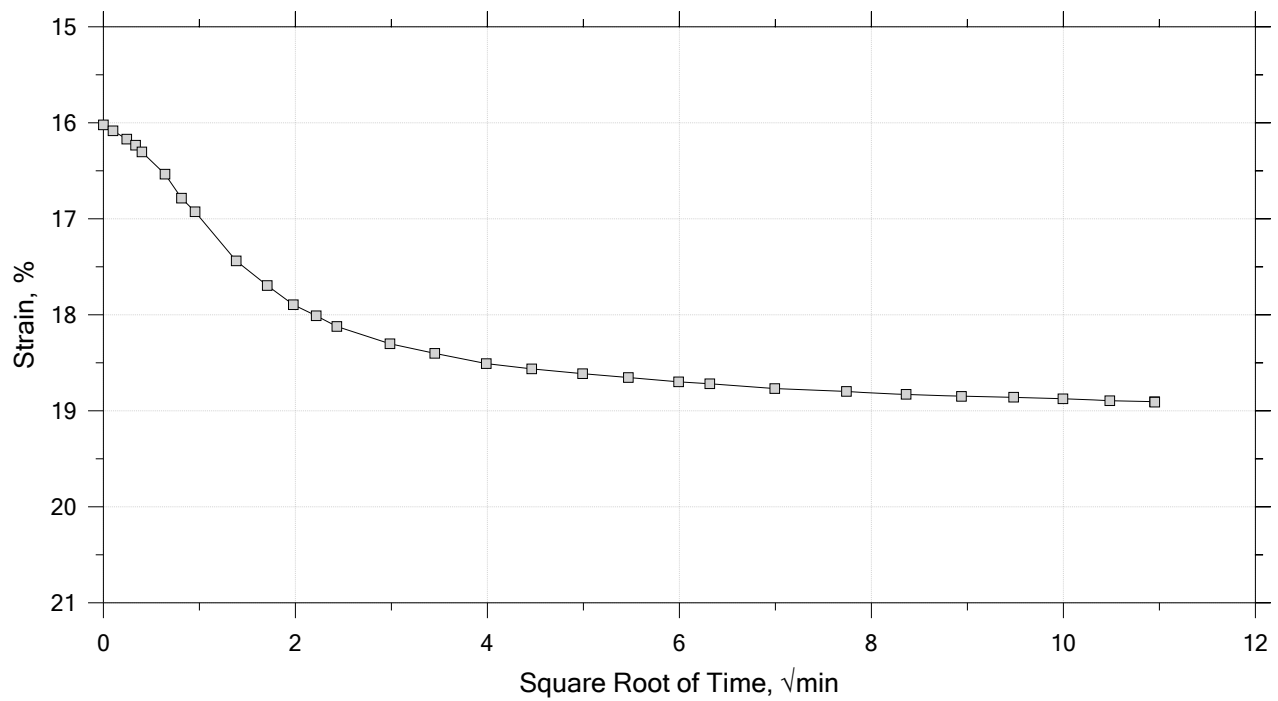
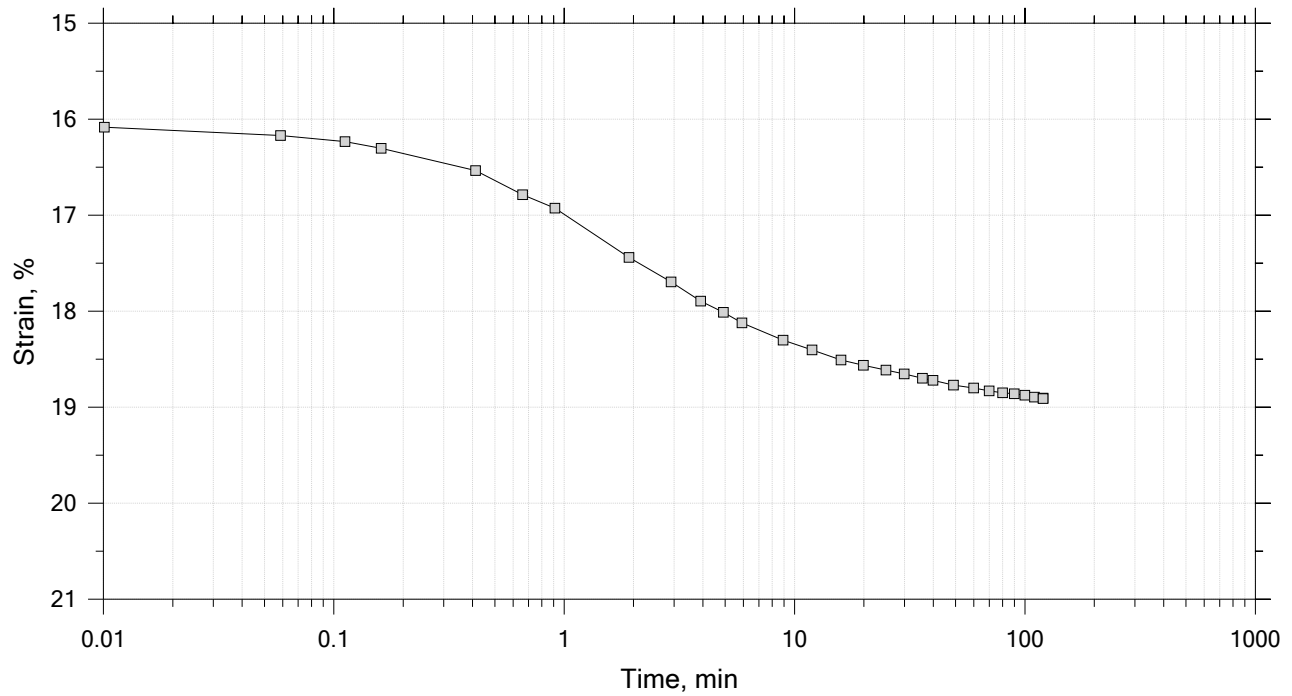
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



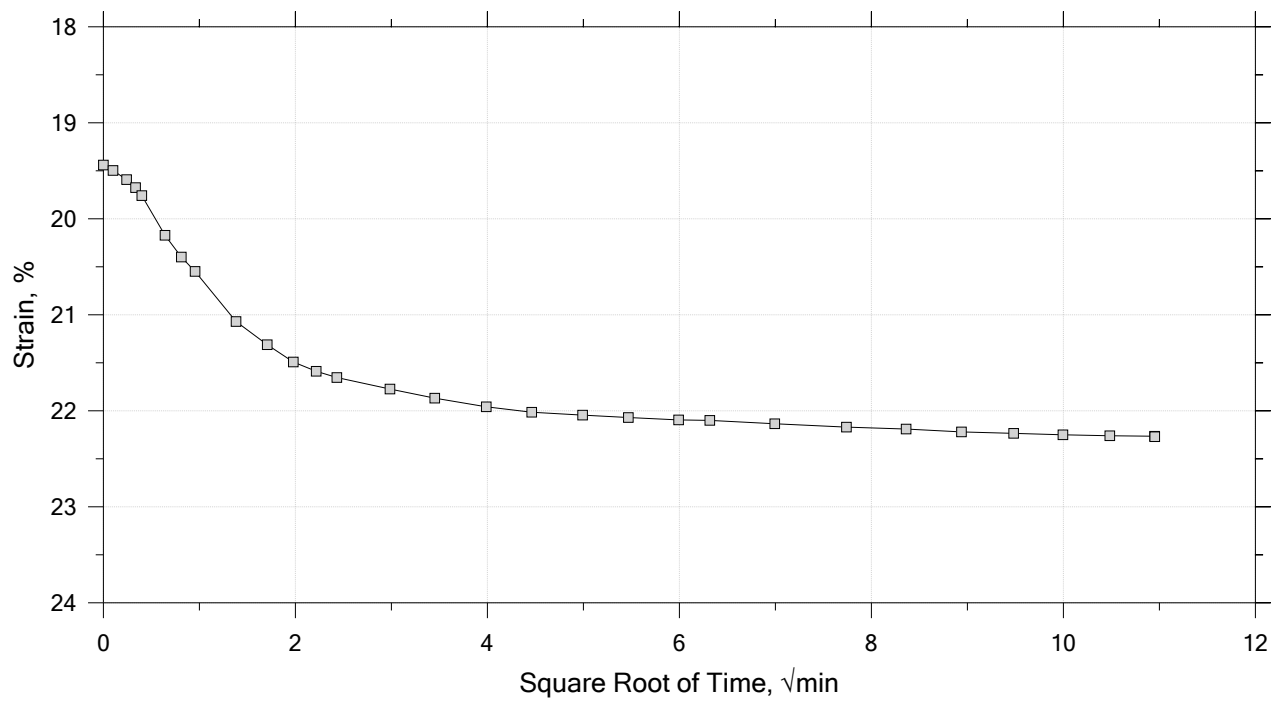
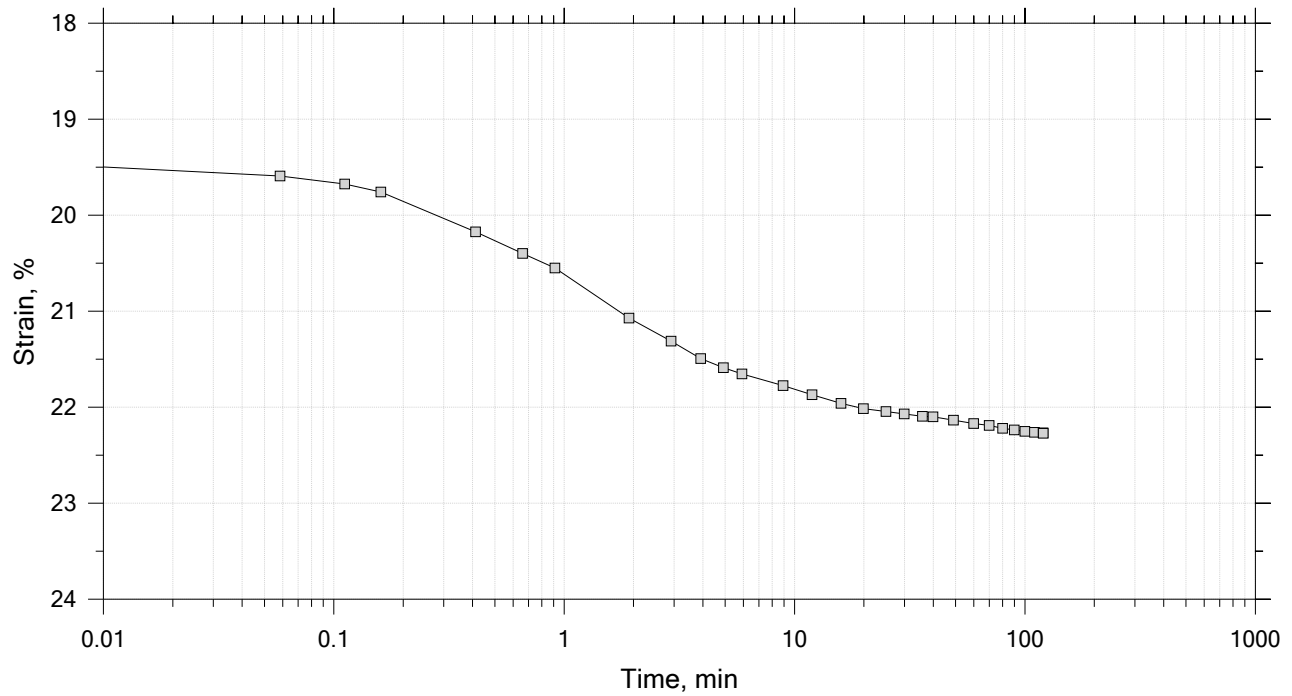
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



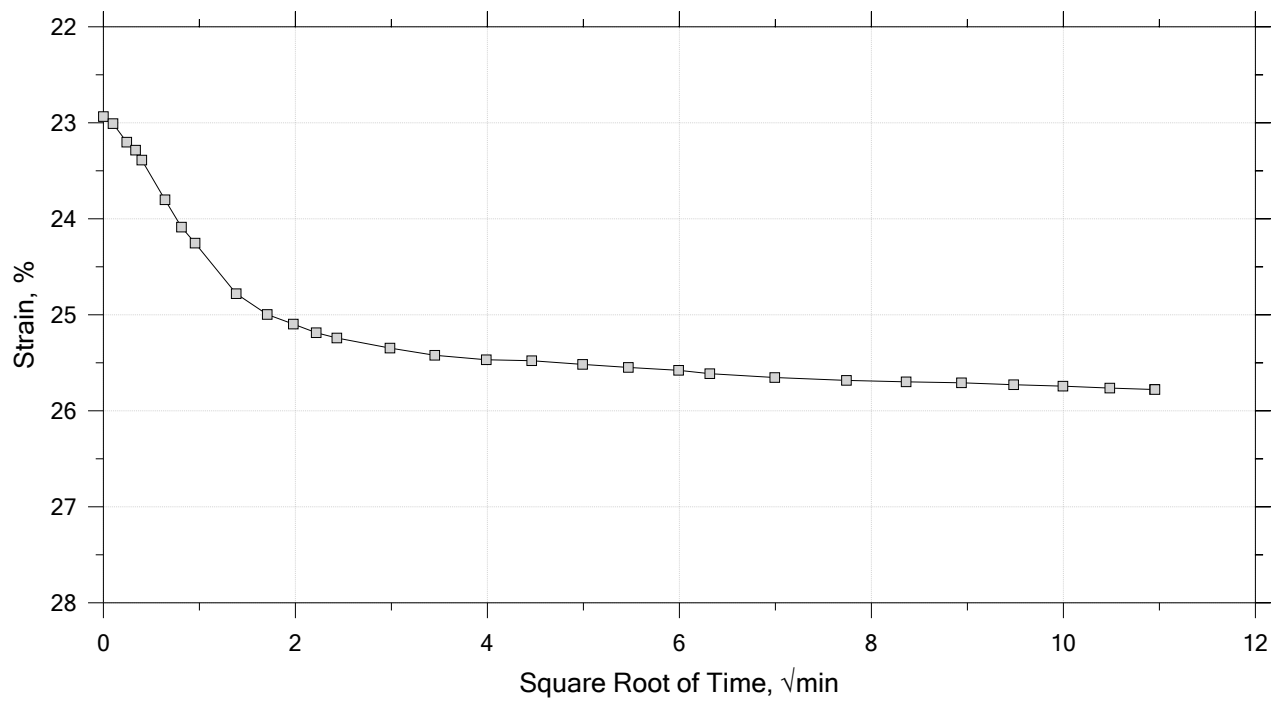
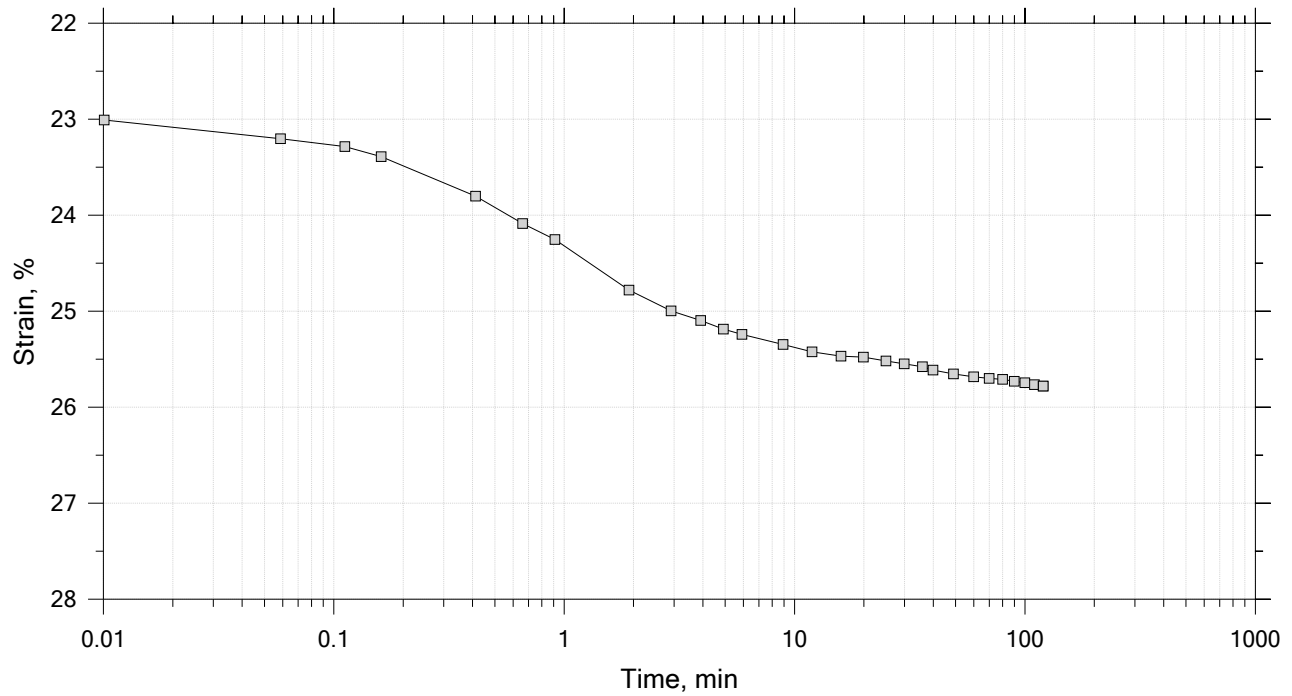
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



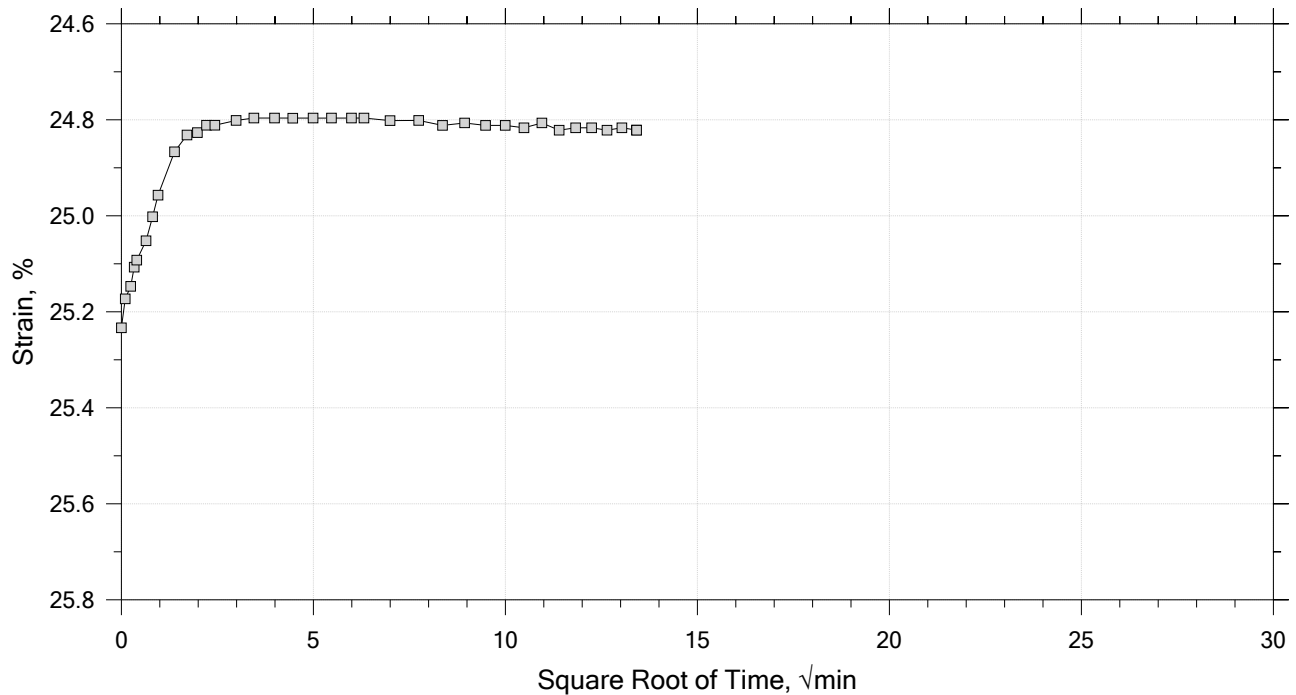
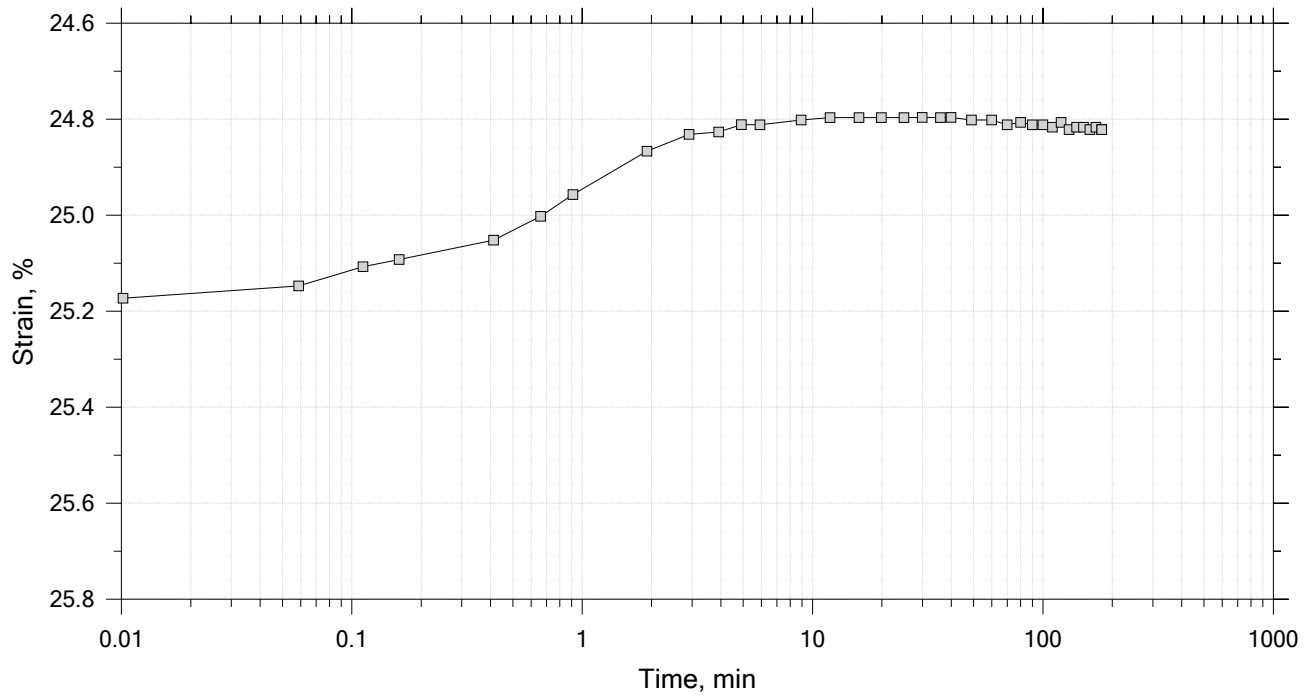
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



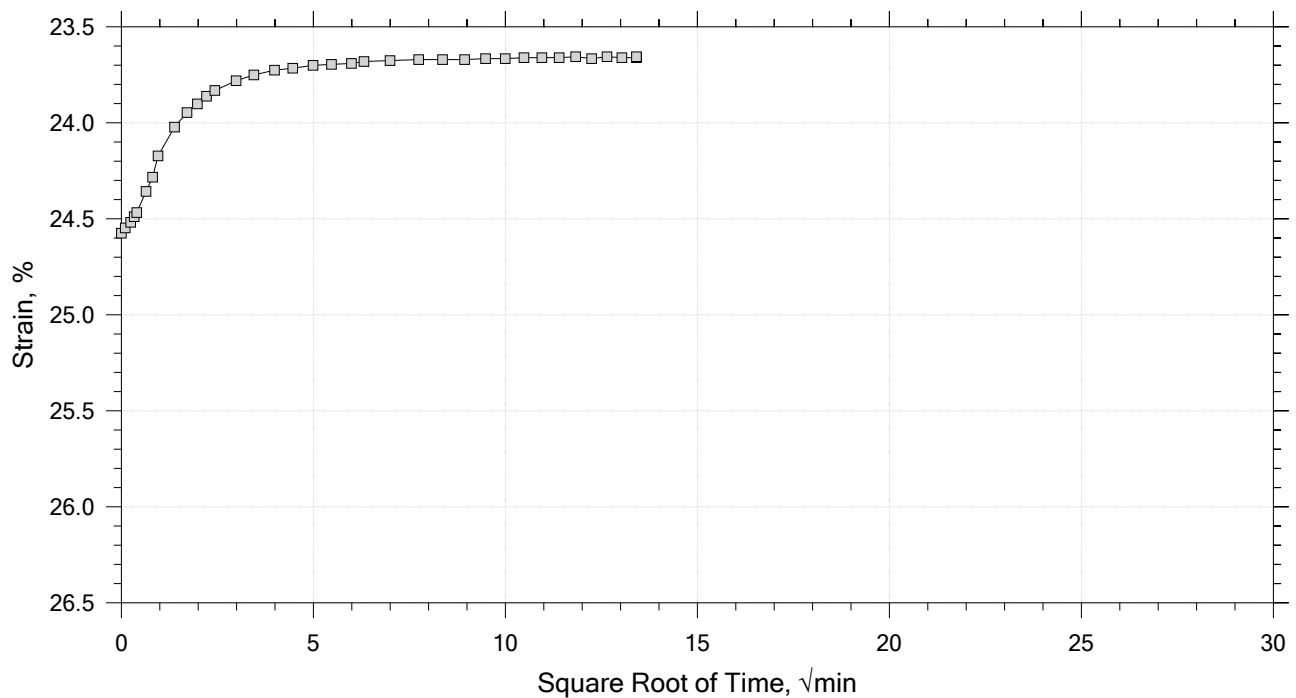
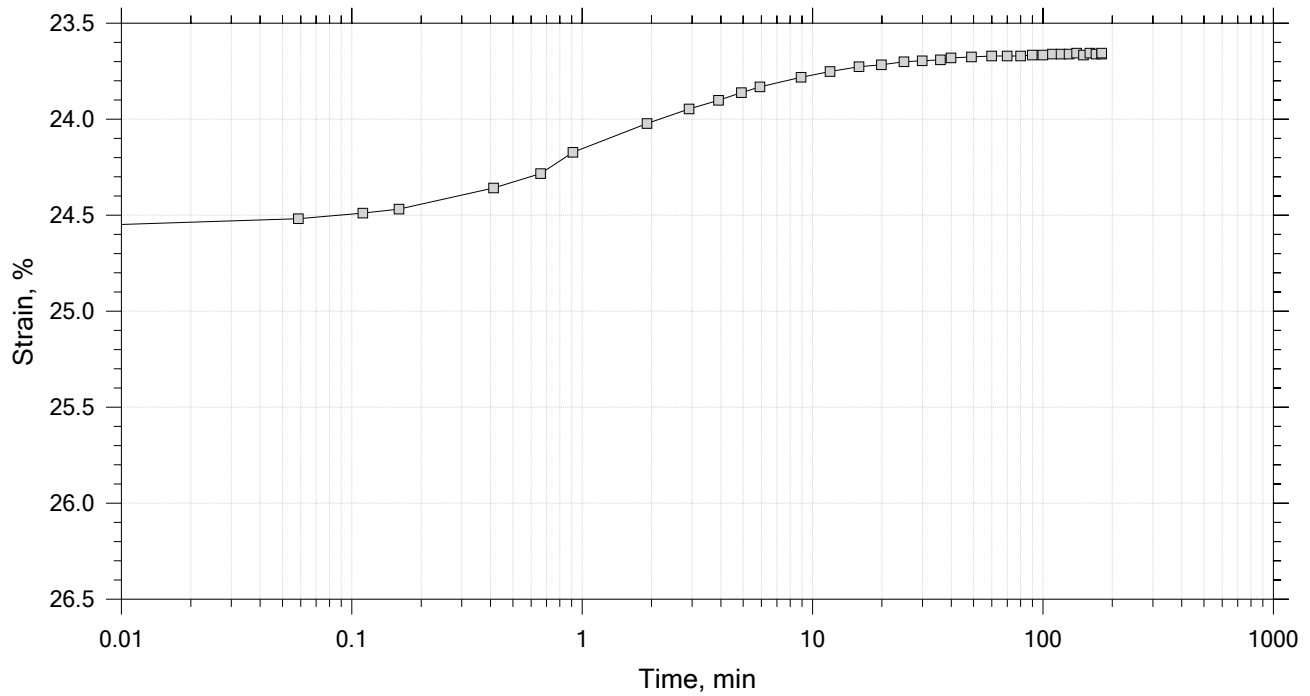
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



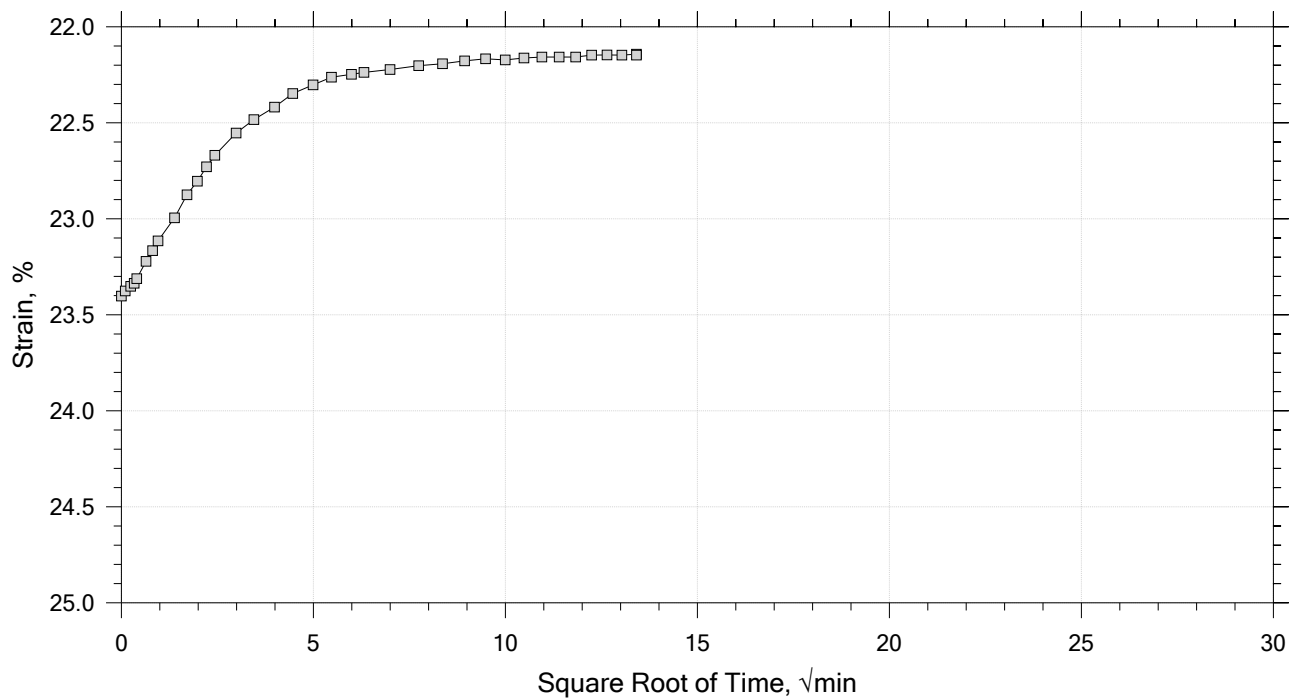
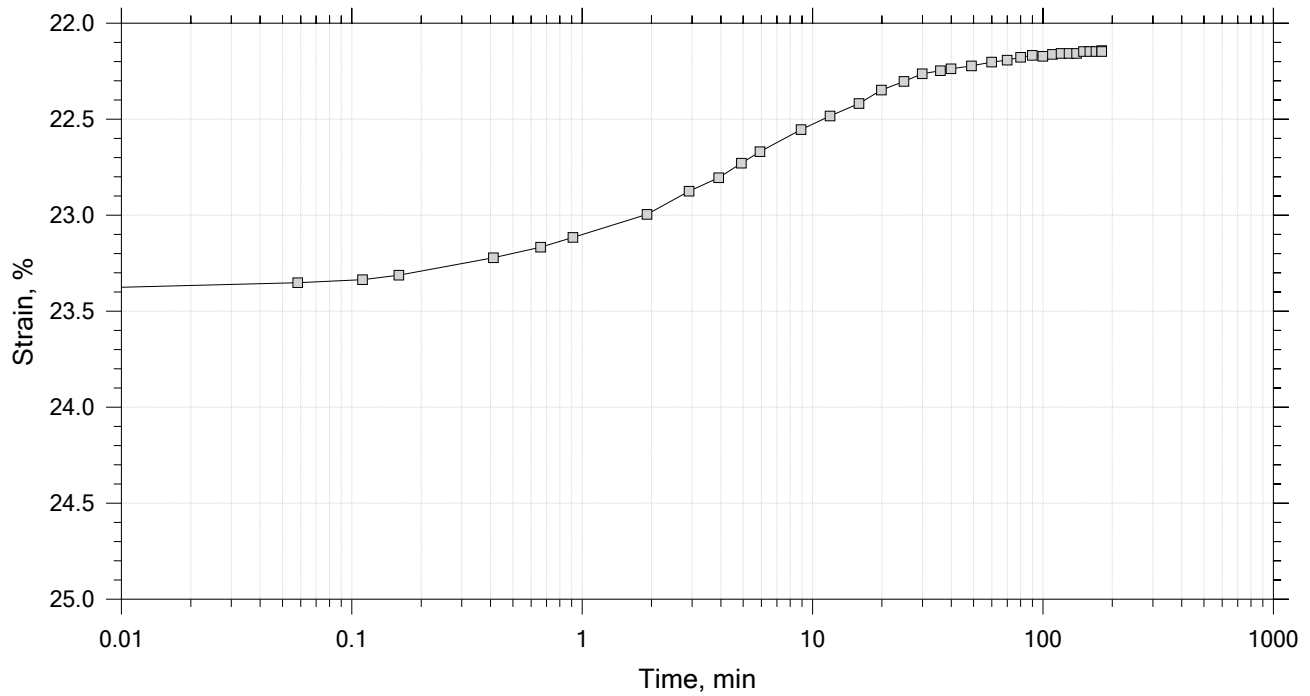
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



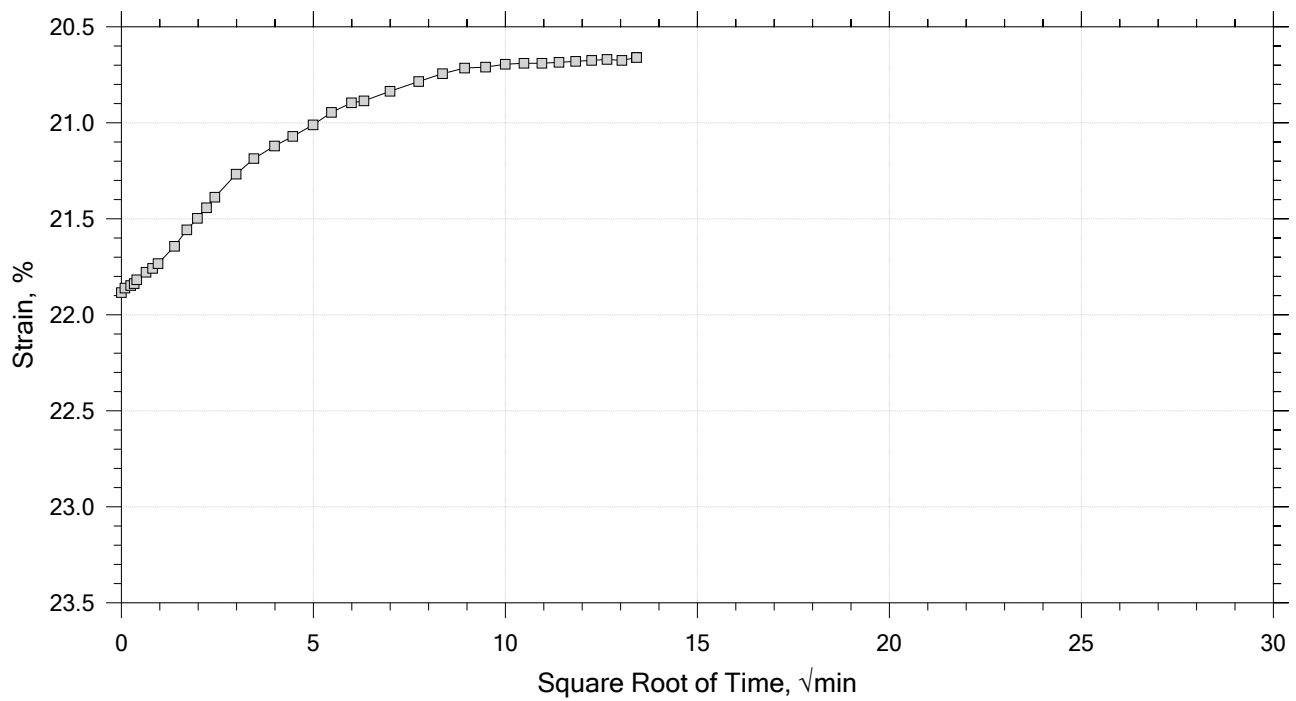
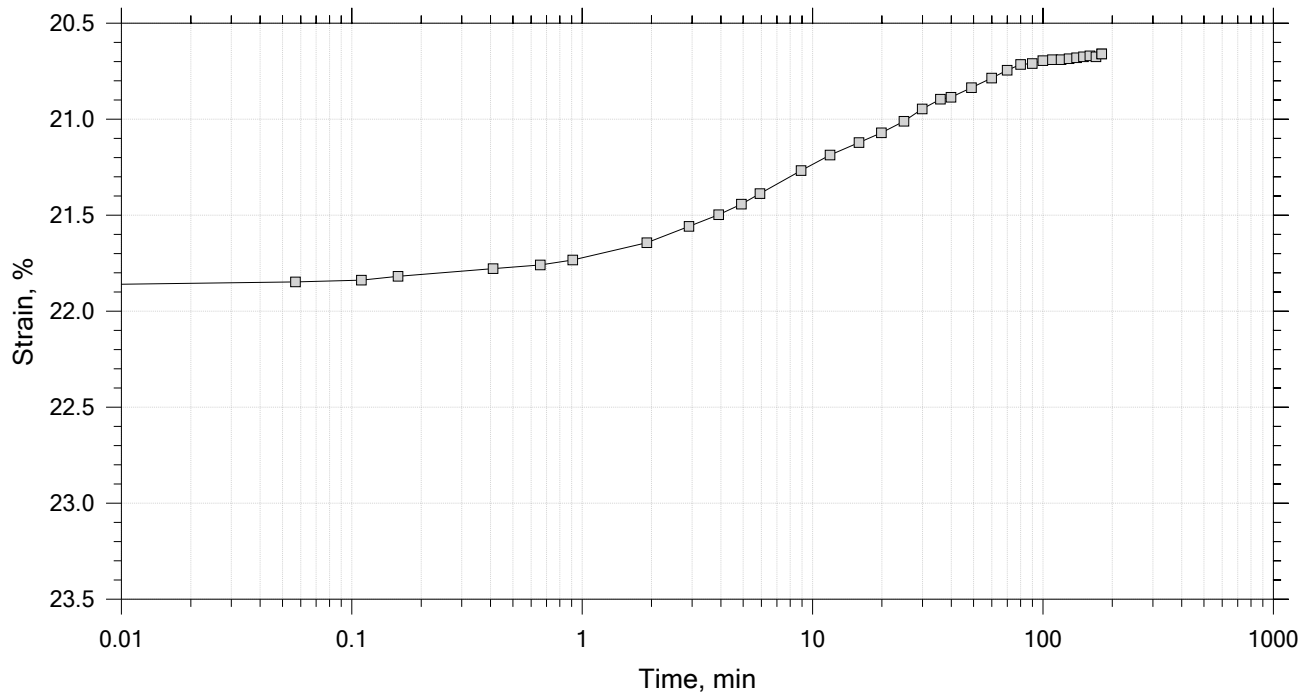
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



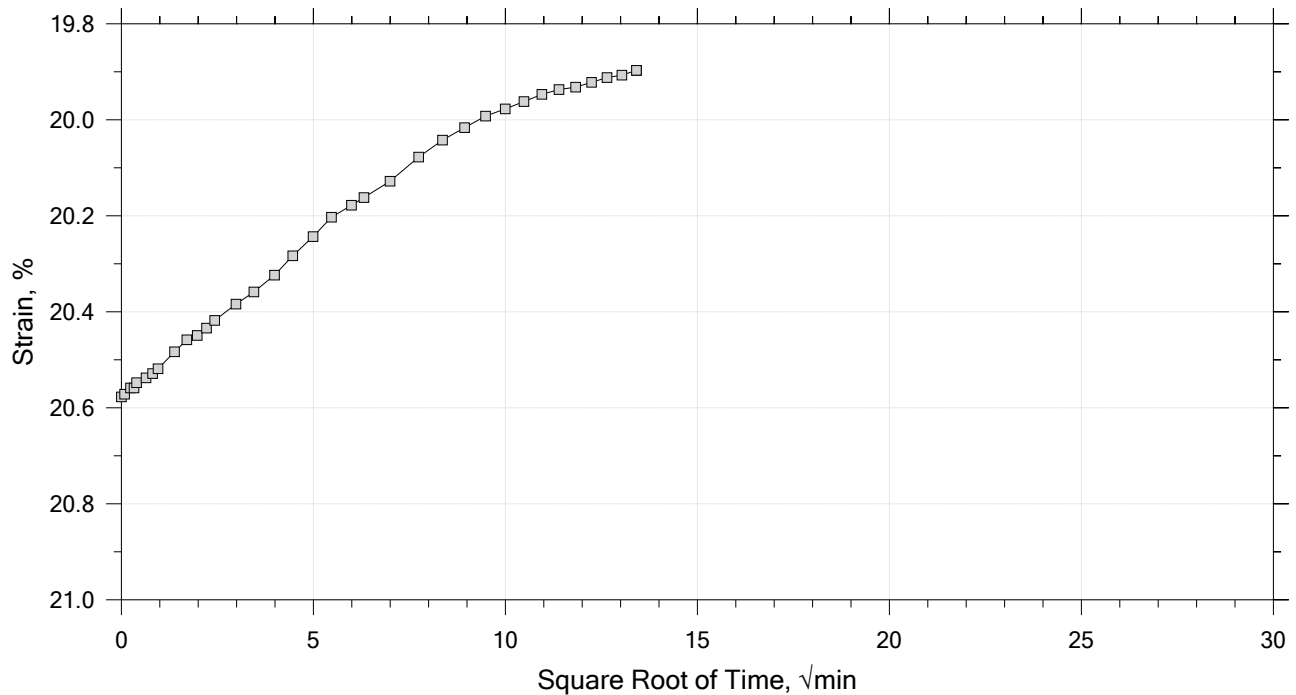
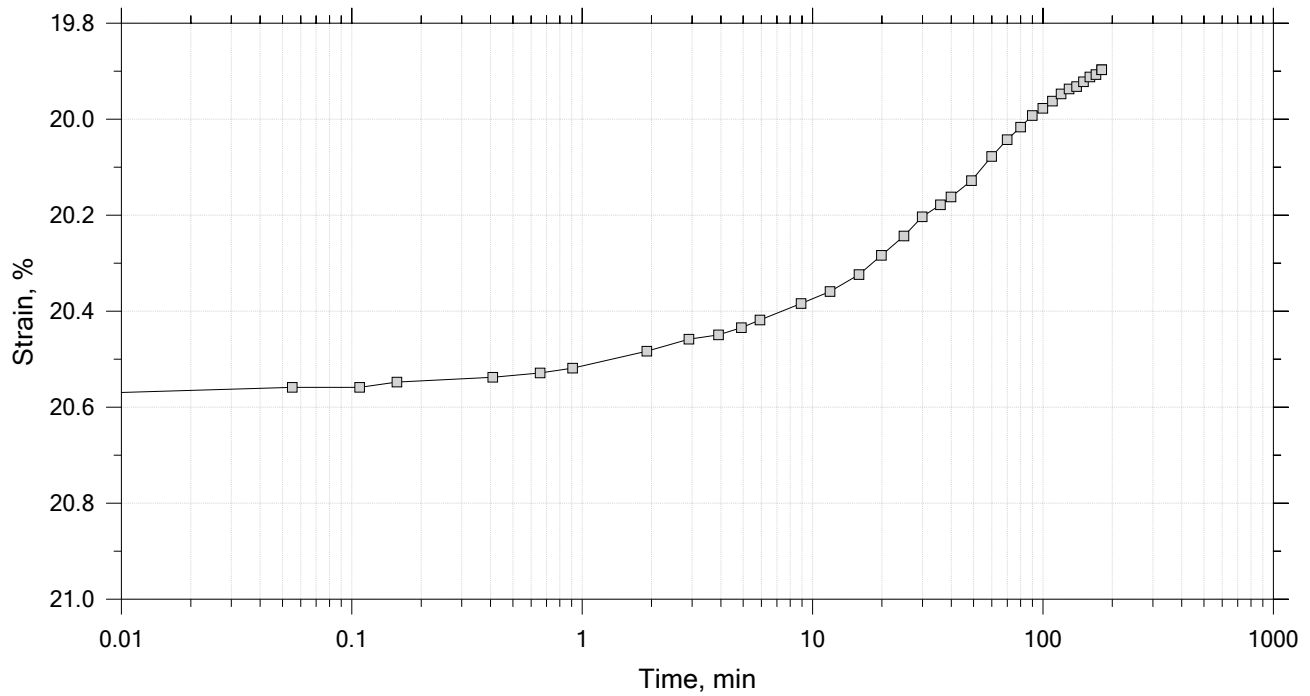
	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: BB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 34
Initial Height: 1.00 in	Initial Void Ratio: 0.933	Plastic Limit: 20
Final Height: 0.82 in	Final Void Ratio: 0.585	Plasticity Index: 14

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	C575	RING		A2478
Mass Container, gm	8.5	111.71	111.71	8
Mass Container + Wet Soil, gm	126.85	263.55	250.61	145.22
Mass Container + Dry Soil, gm	96.91	226.26	226.26	121.16
Mass Dry Soil, gm	88.41	114.55	114.55	113.16
Water Content, %	33.86	32.56	21.26	21.26
Void Ratio	---	0.93	0.59	---
Degree of Saturation, %	---	96.04	100.00	---
Dry Unit Weight, pcf	---	88.897	108.41	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

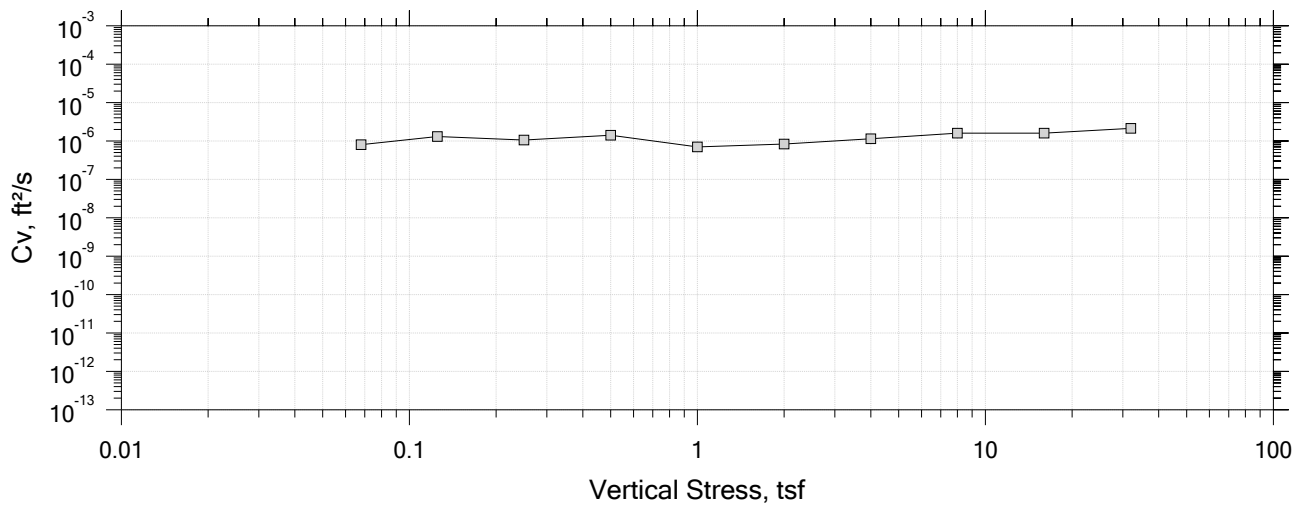
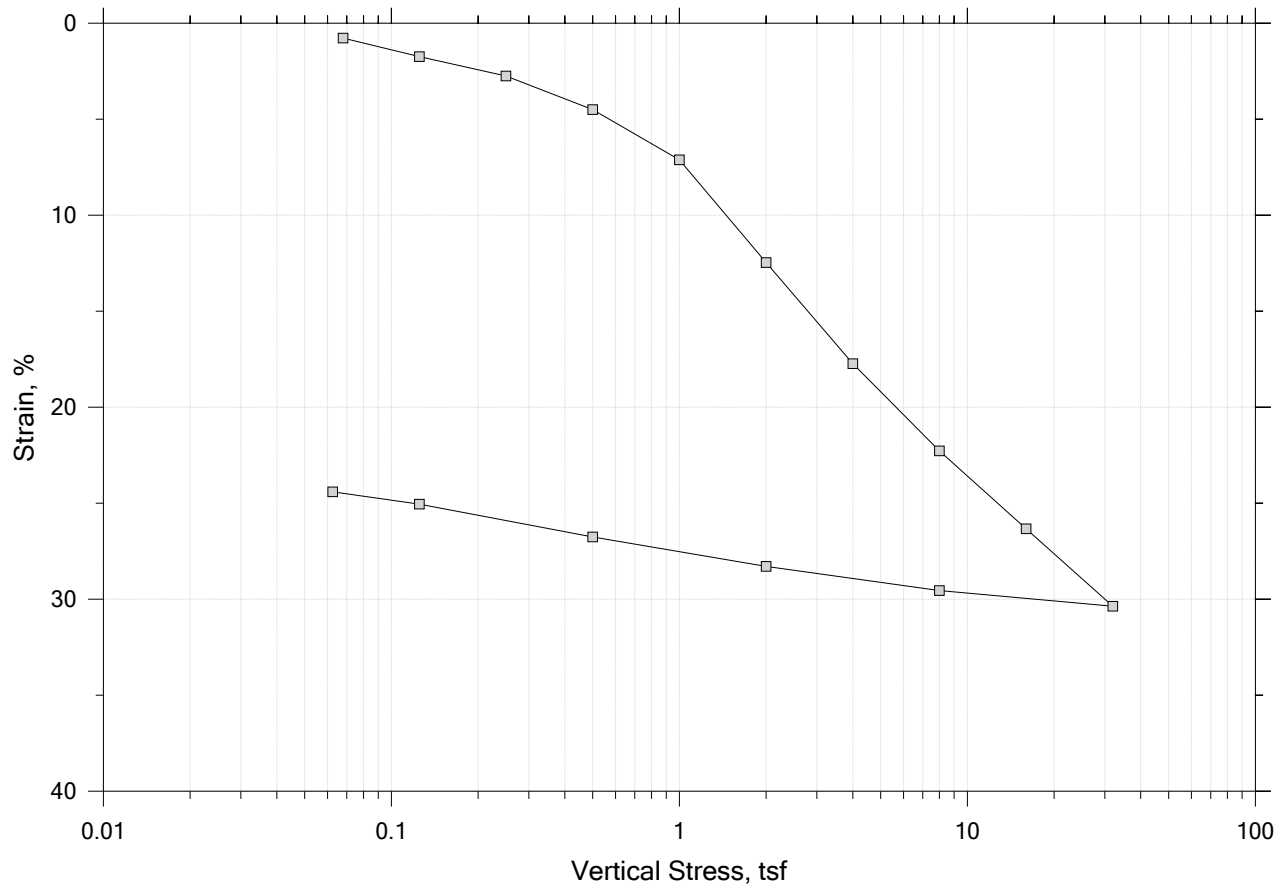
Square Root of Time Coefficients


[illegible]

	Project: Rt-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101 BB-BFB-101	Tested By: trm	Checked By: mcm
	Sample No.: 4U	Test Date: 9/28/2018	Depth: 30-32 ft
	Test No.: IP-5	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0671 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

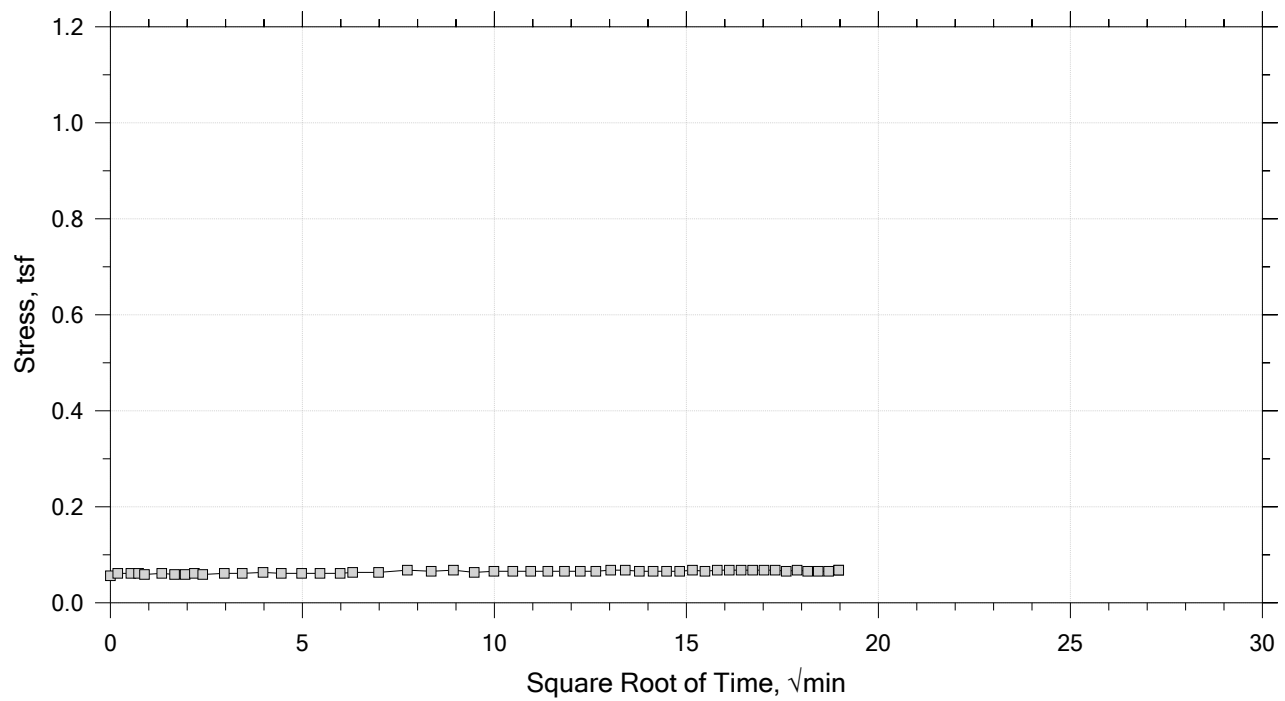
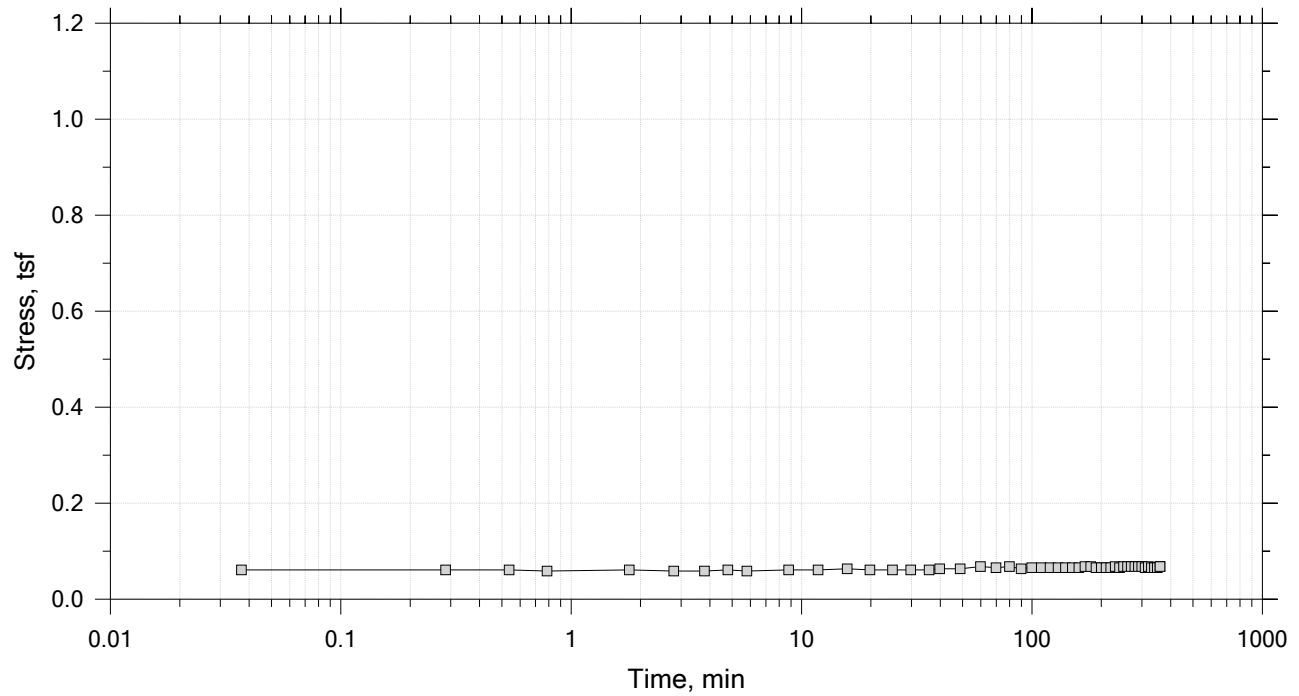
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0679 tsf



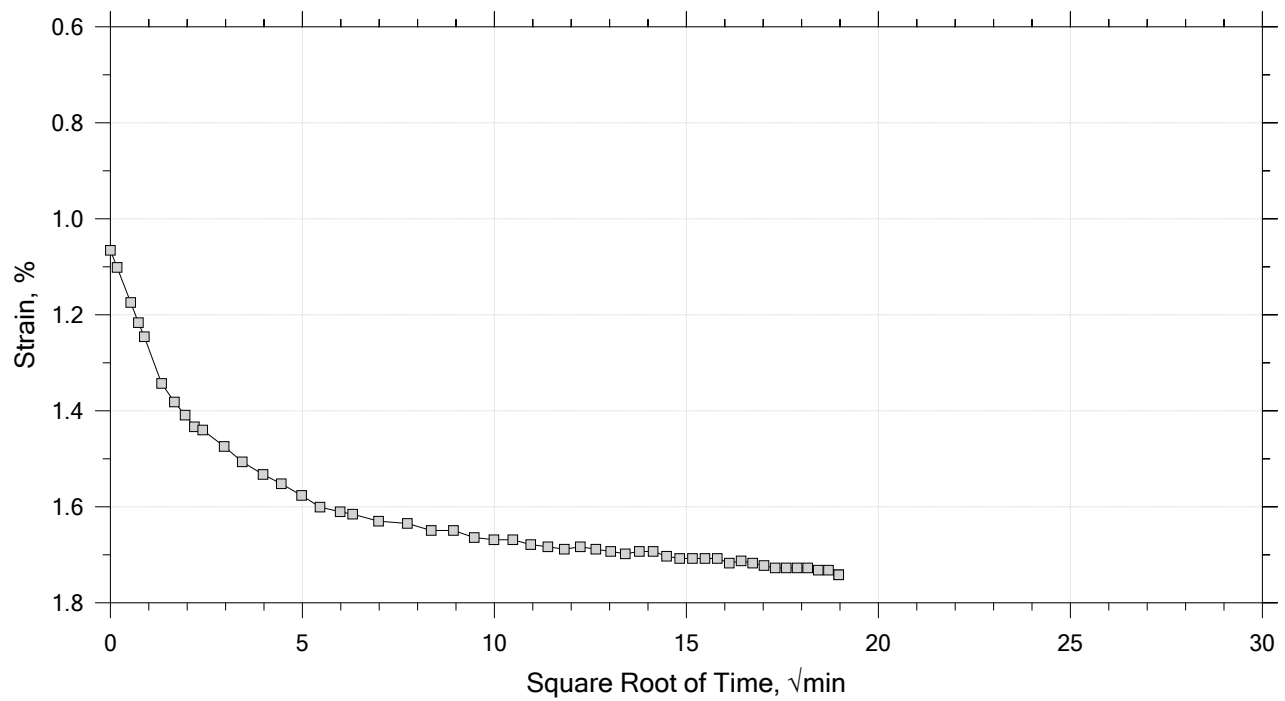
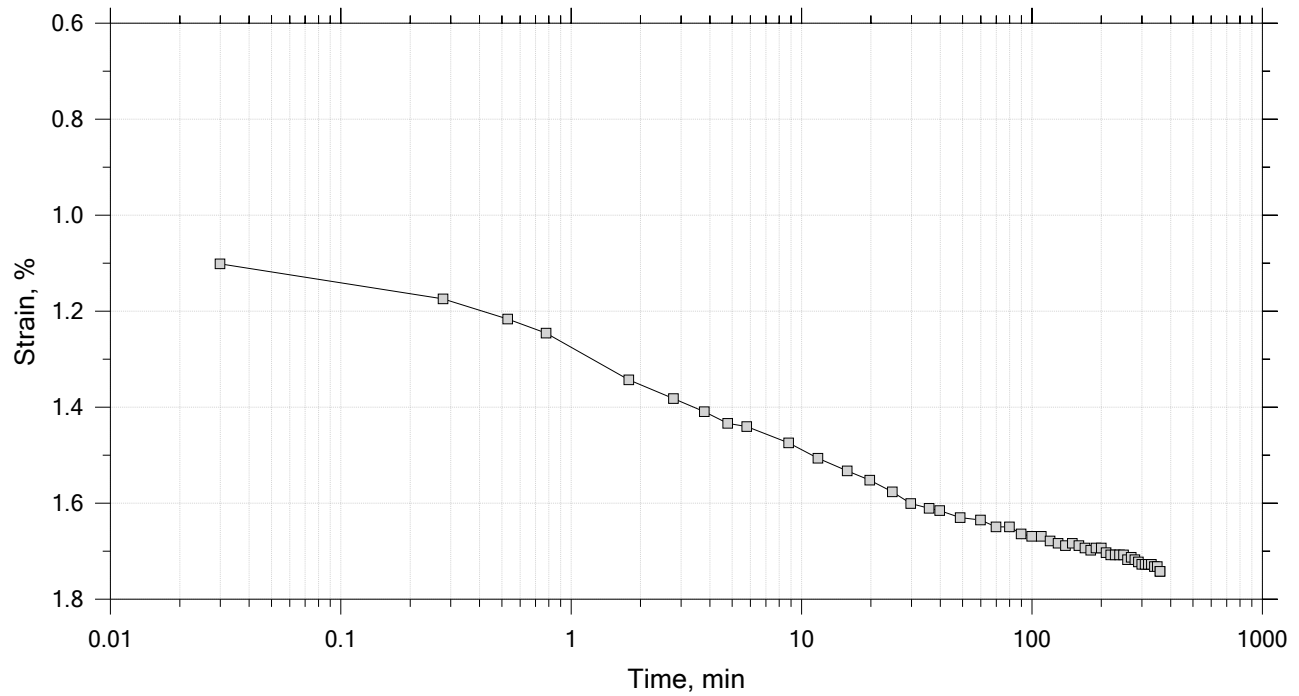
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



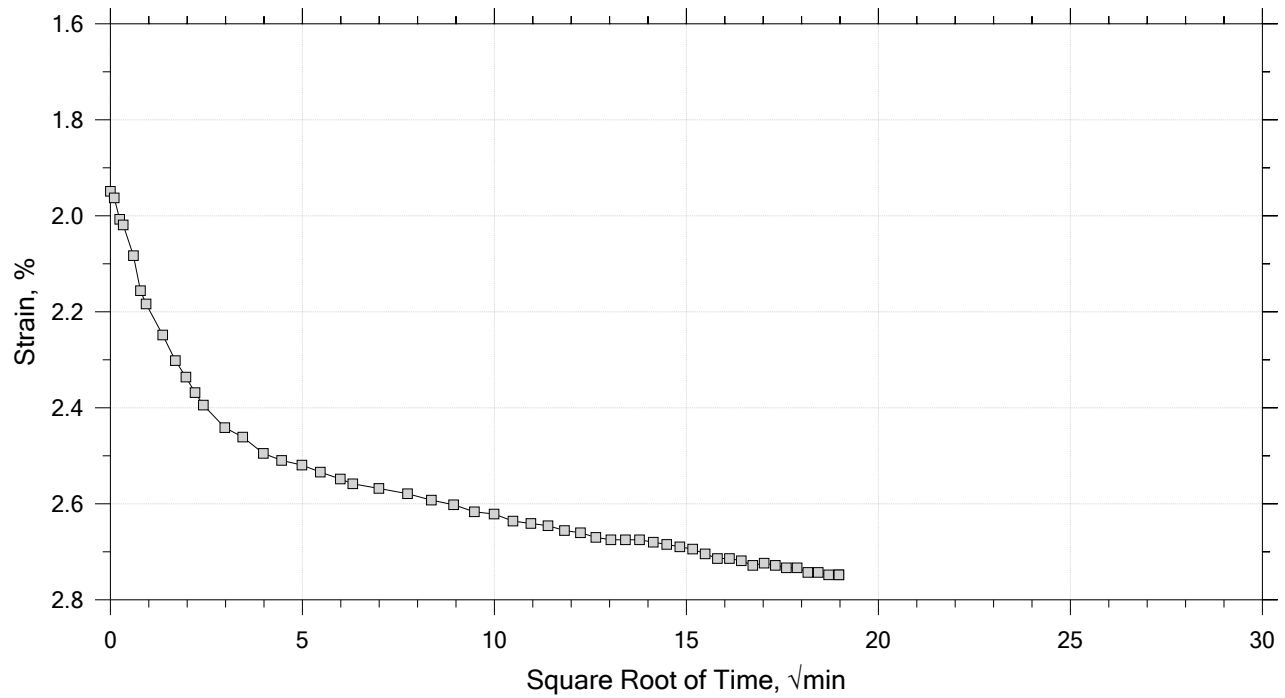
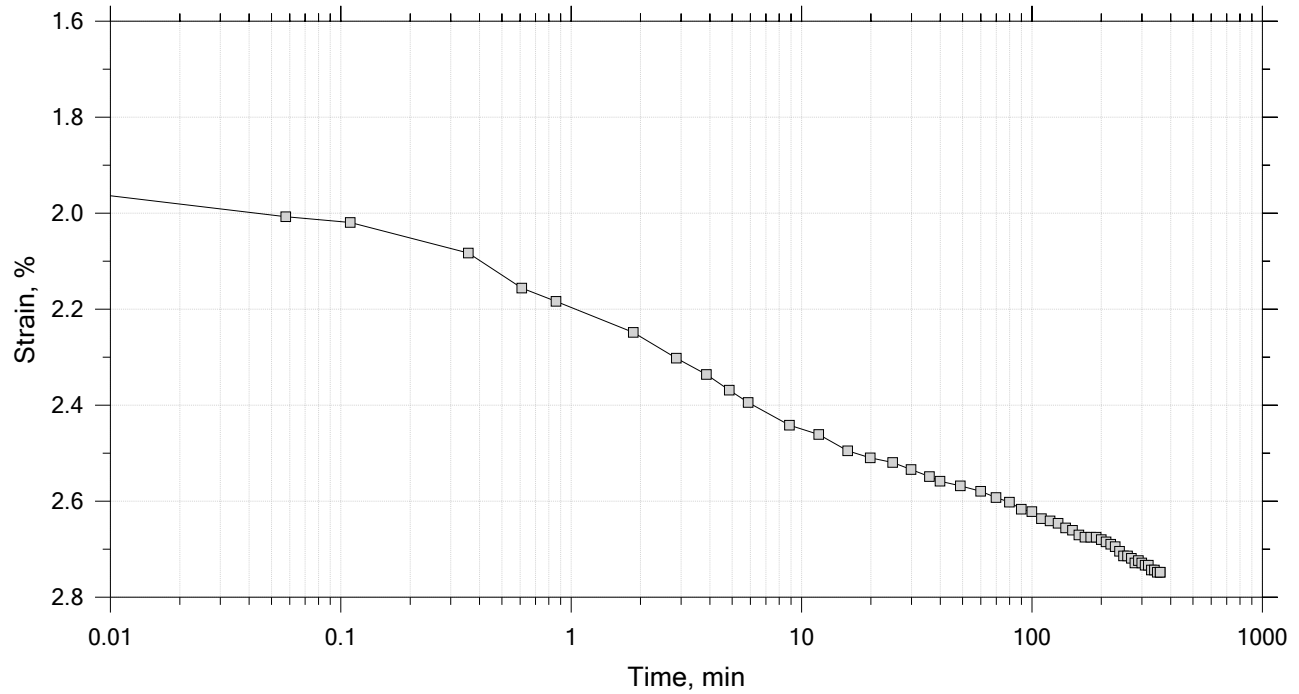
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



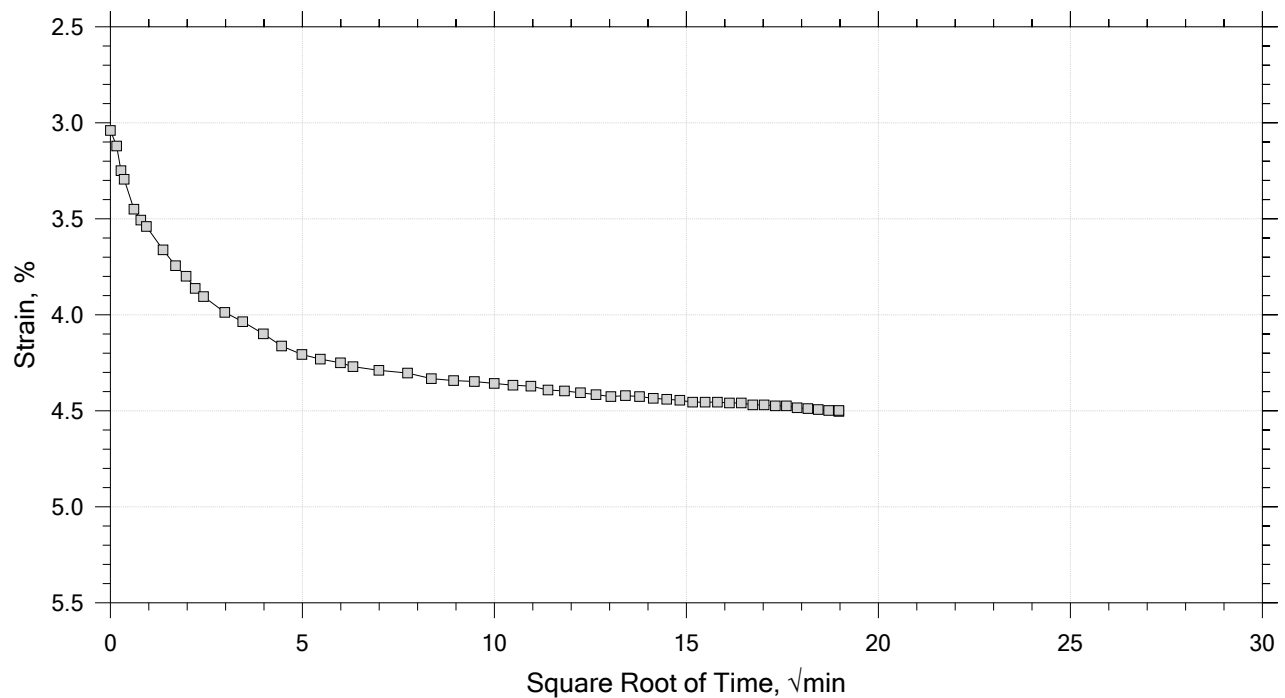
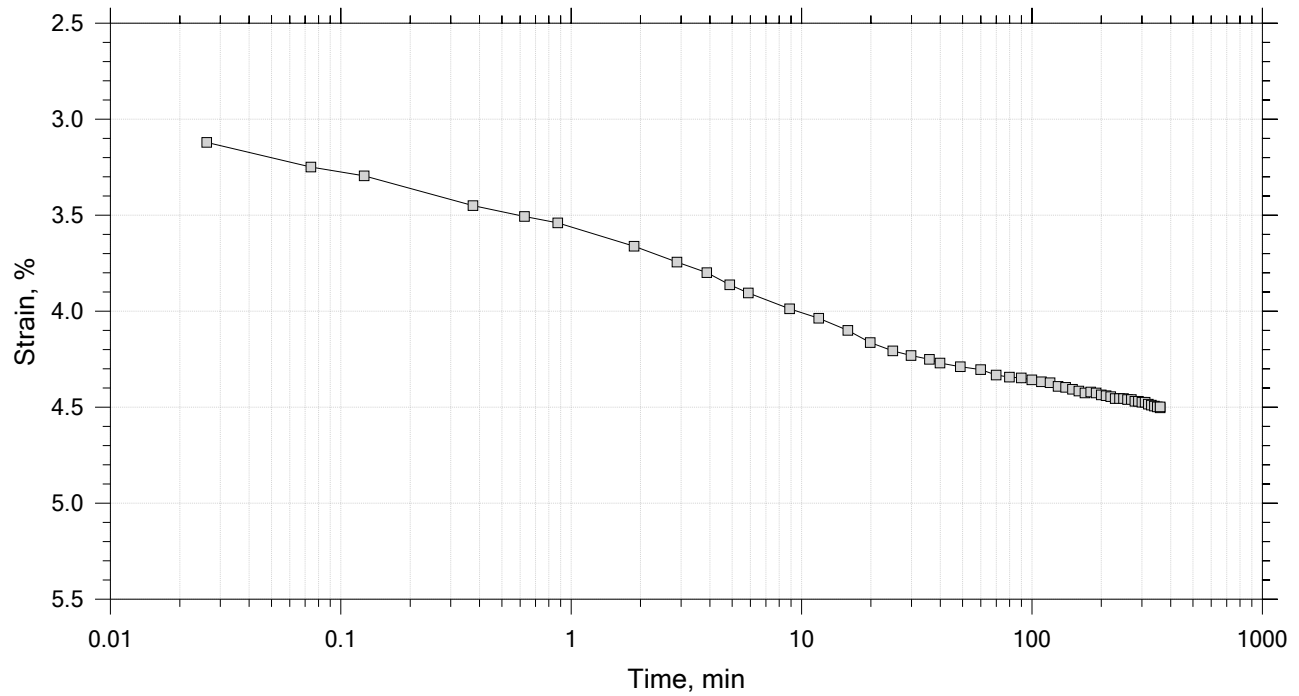
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf

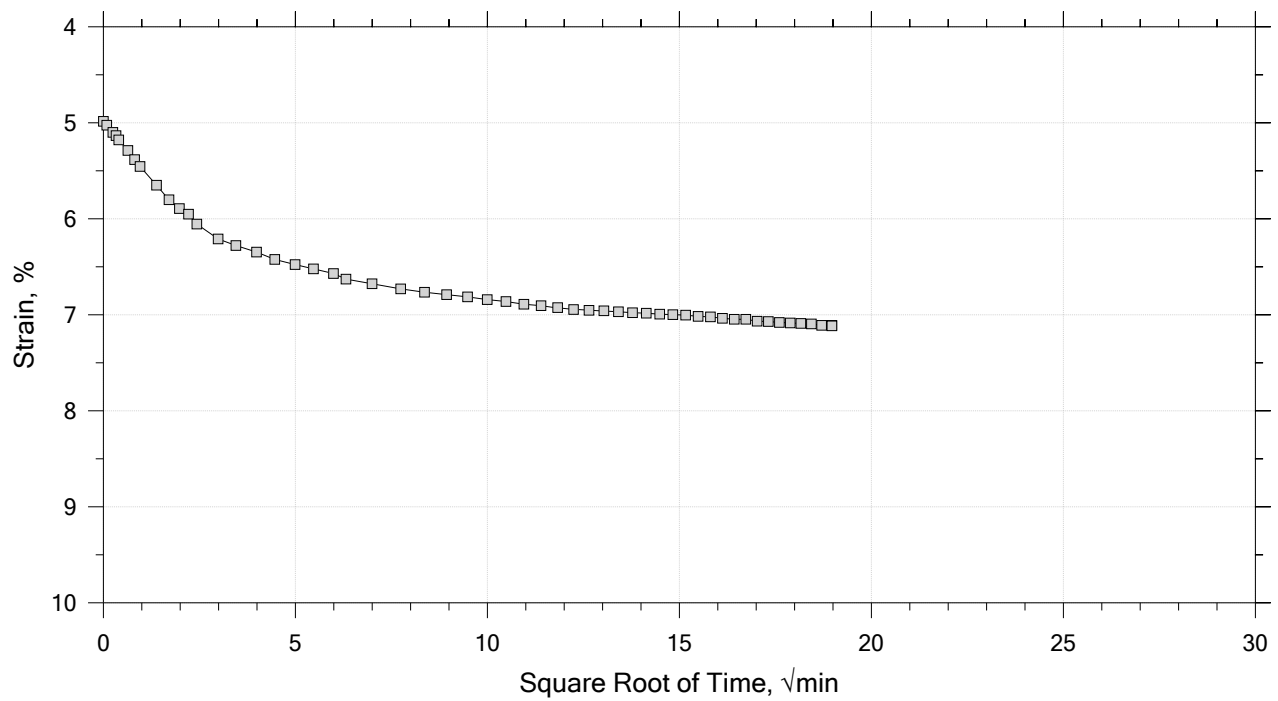
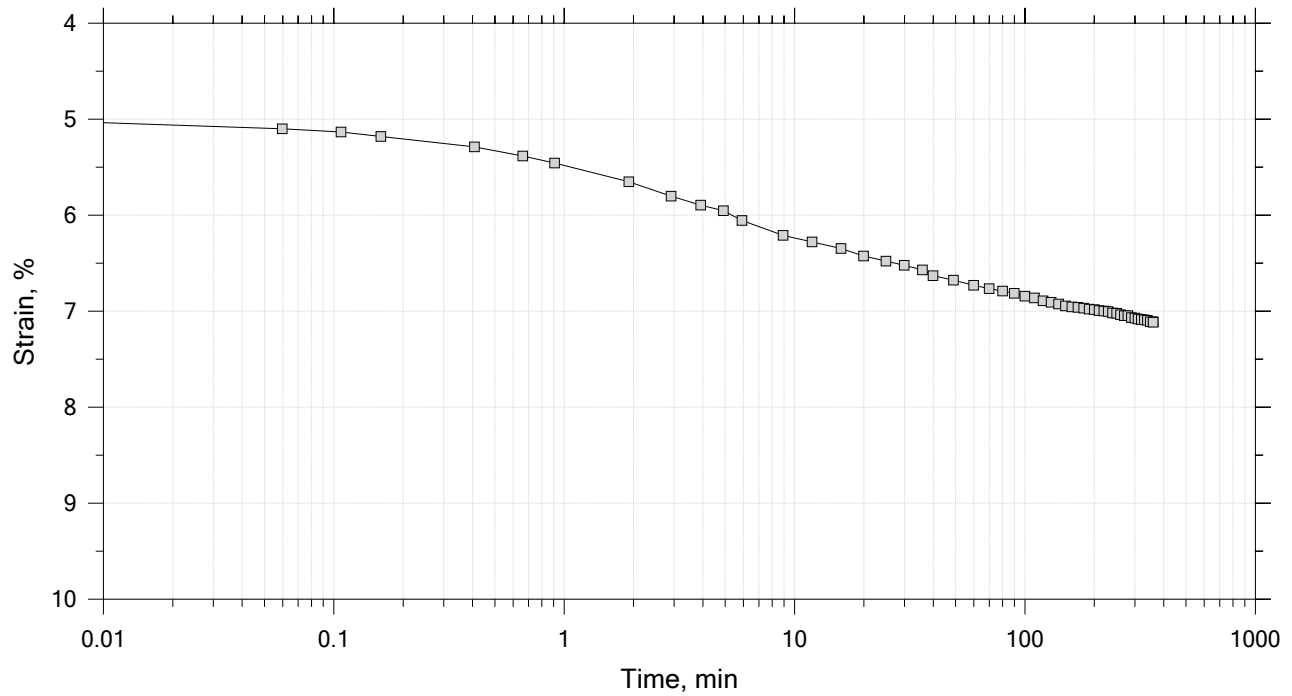



One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



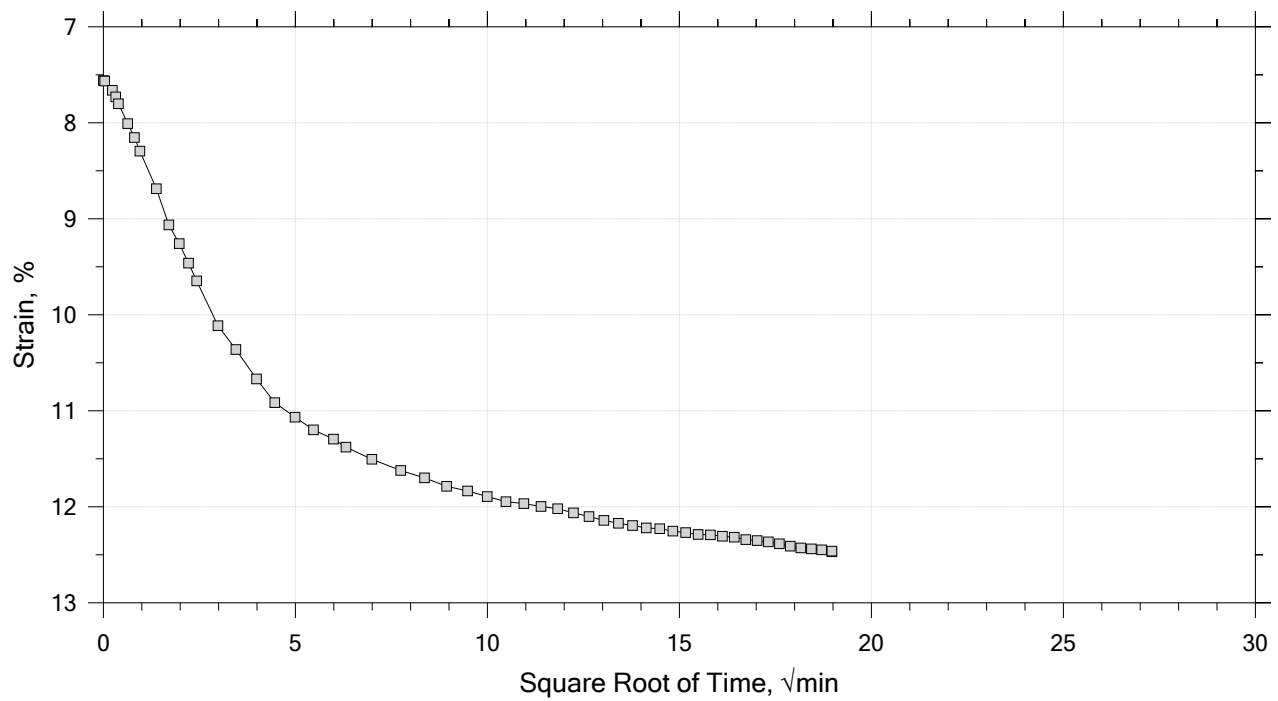
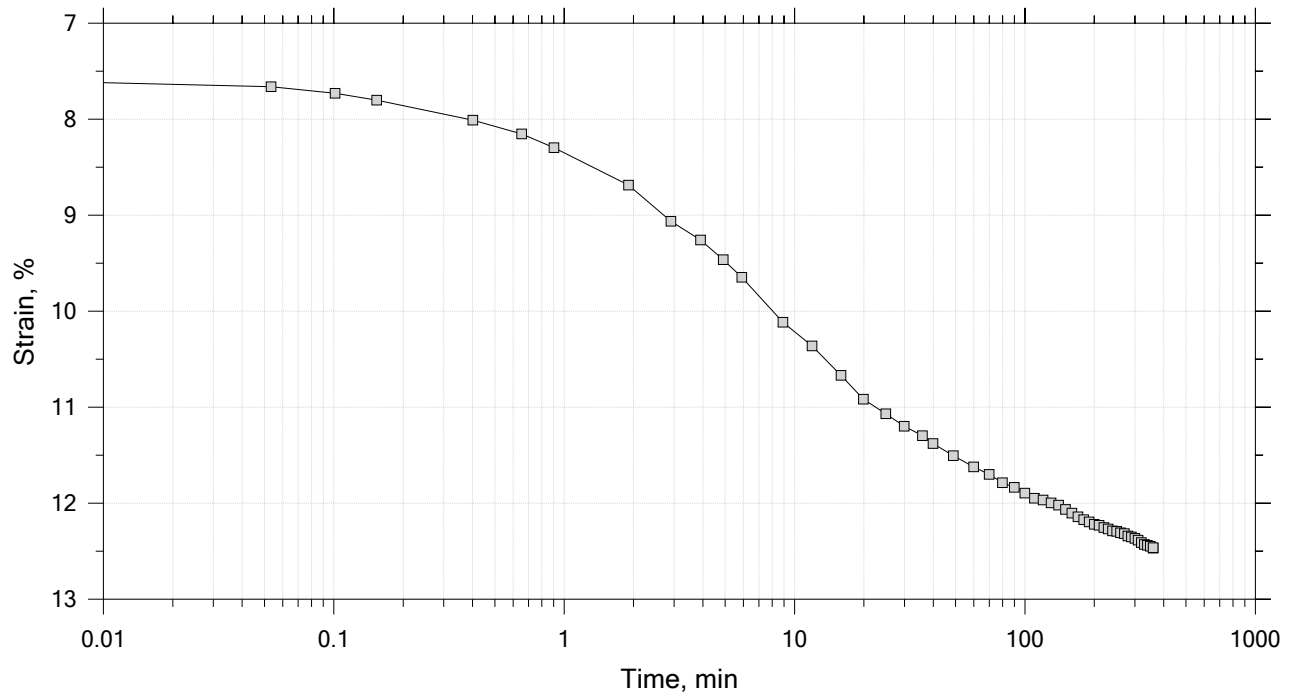
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



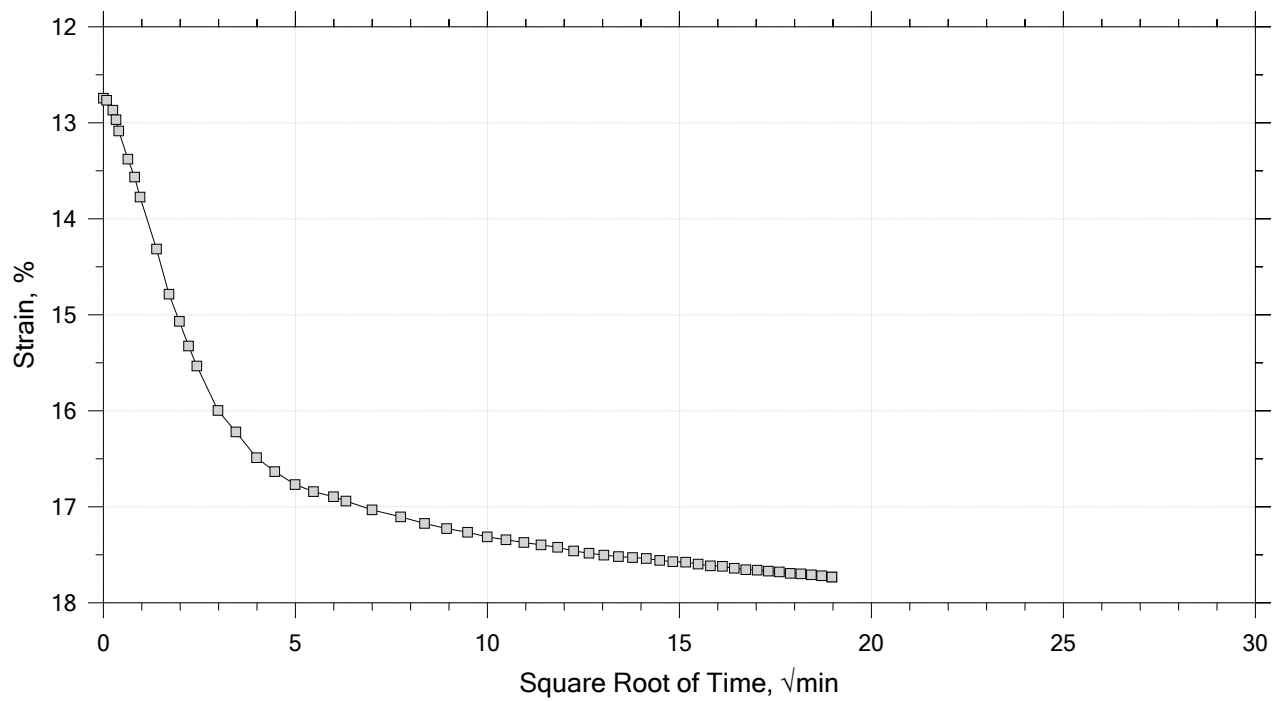
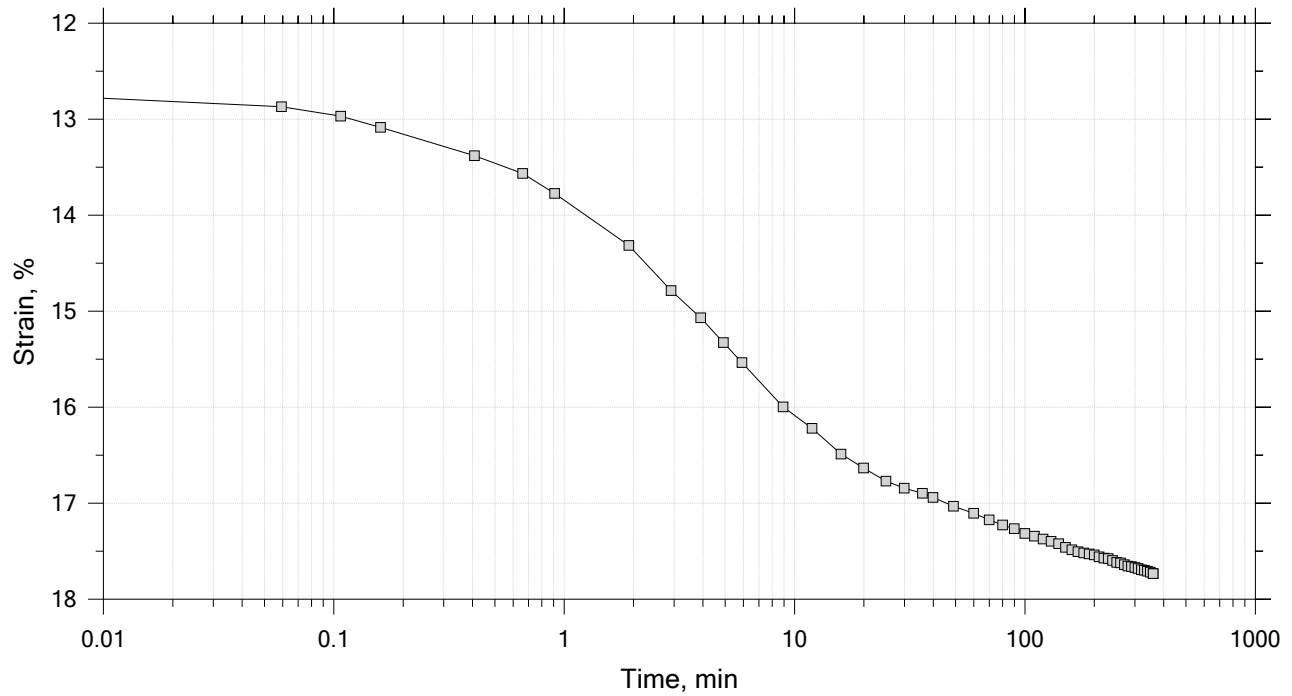
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



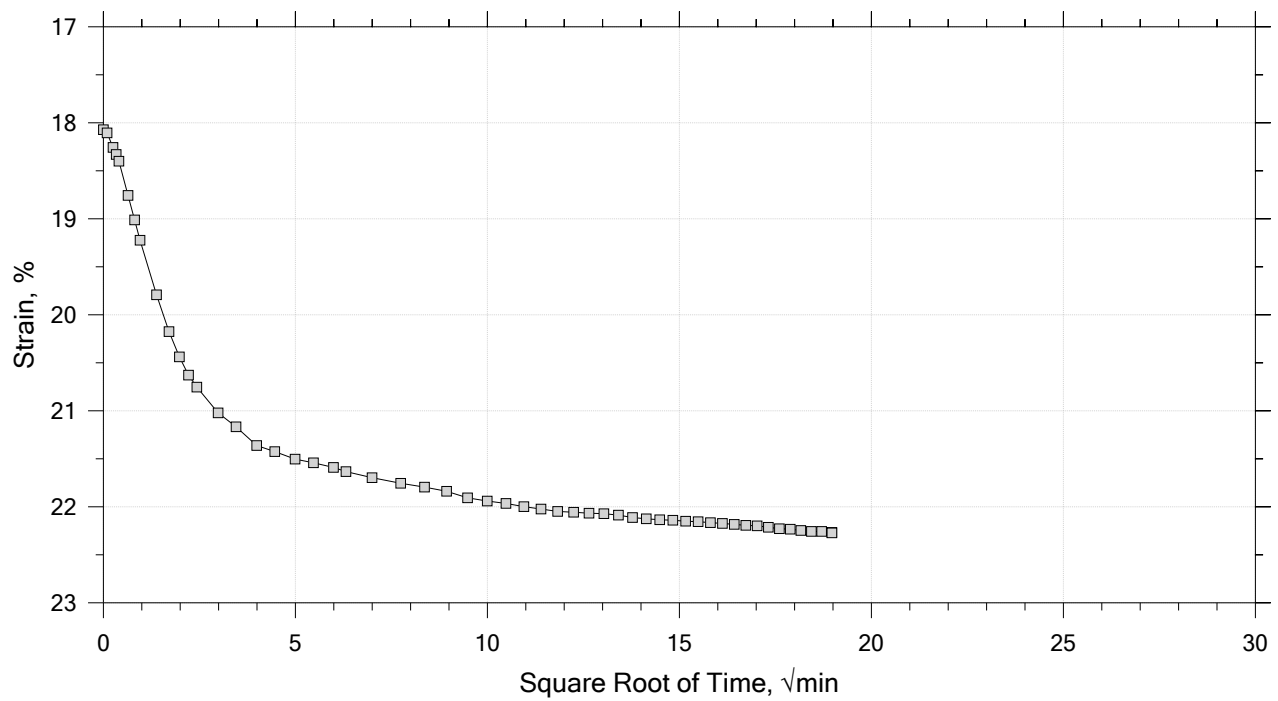
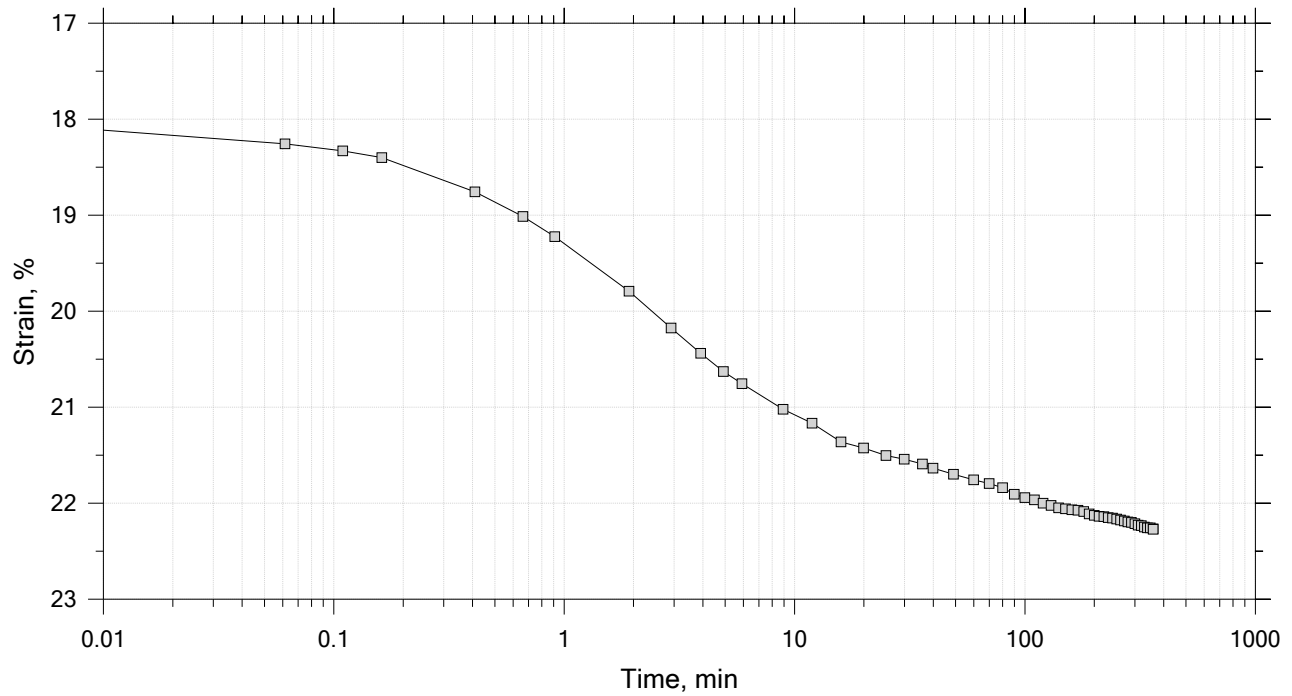
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



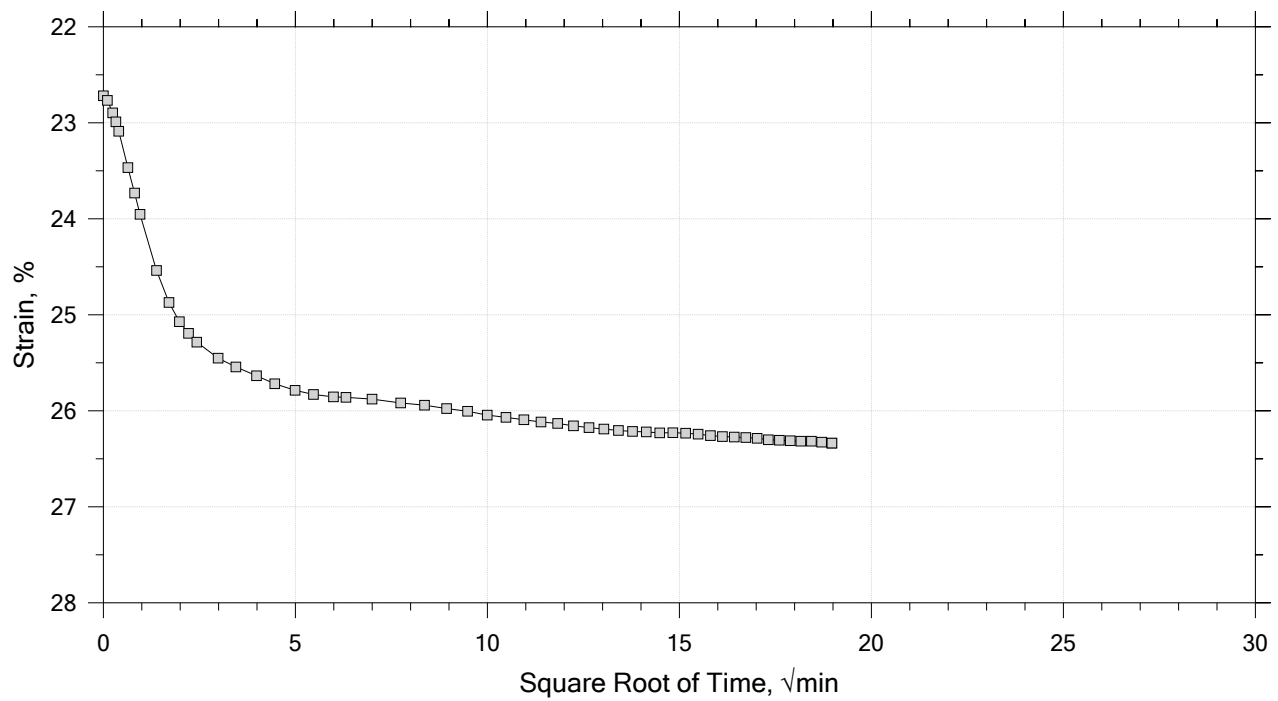
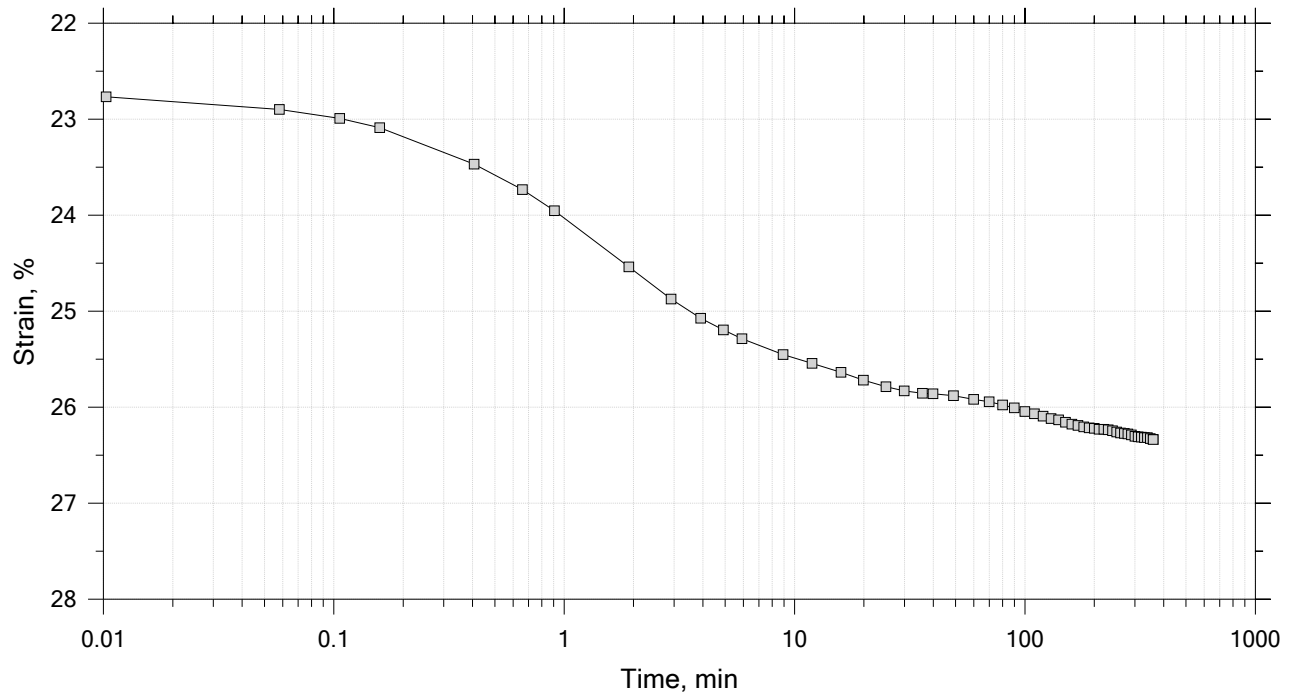
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



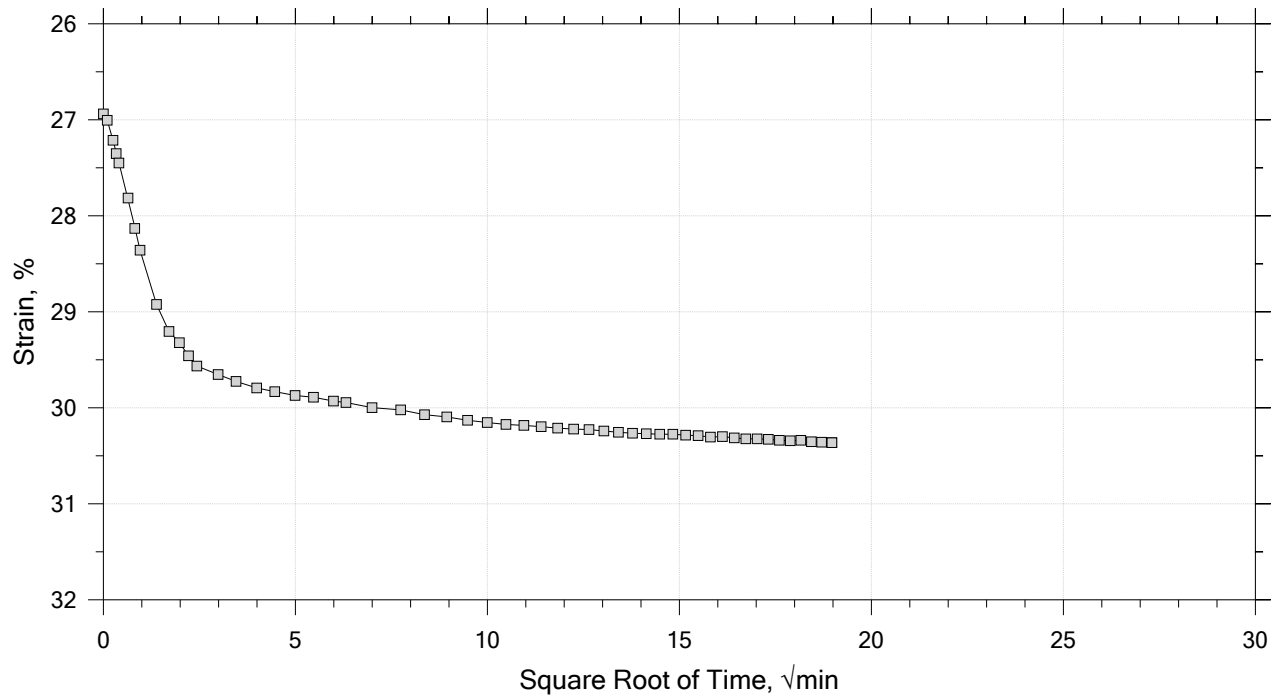
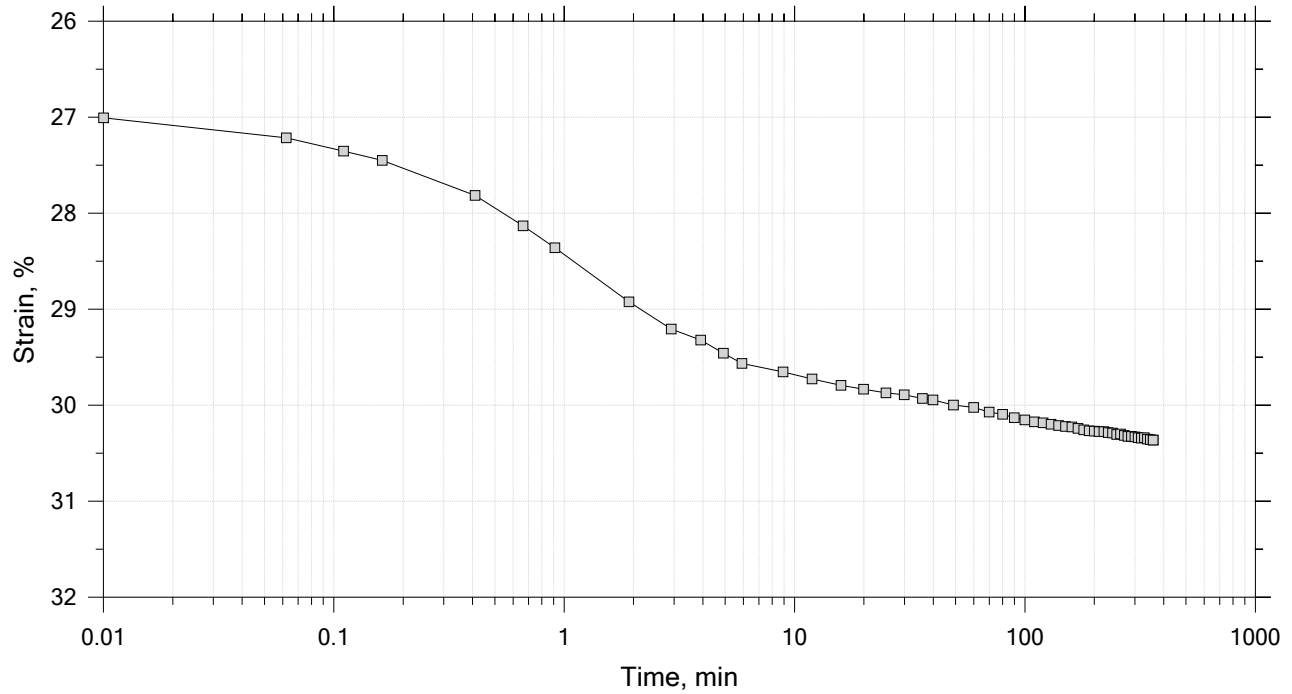
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



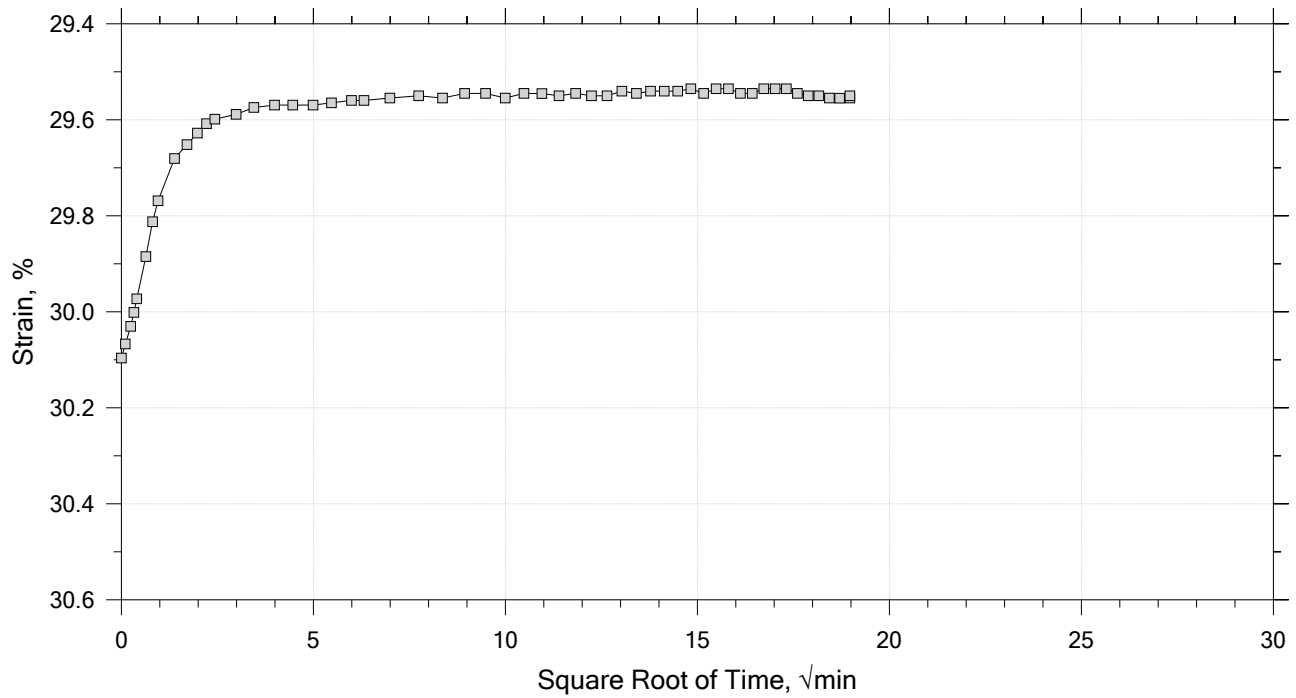
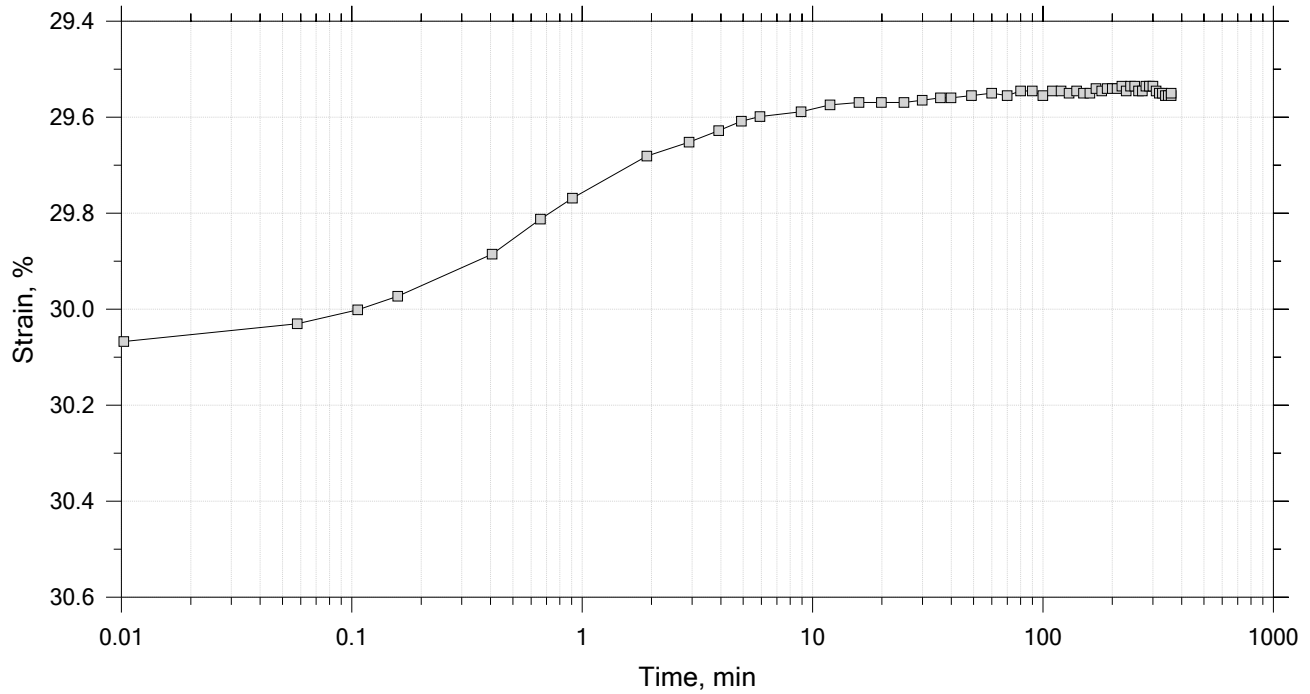
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	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



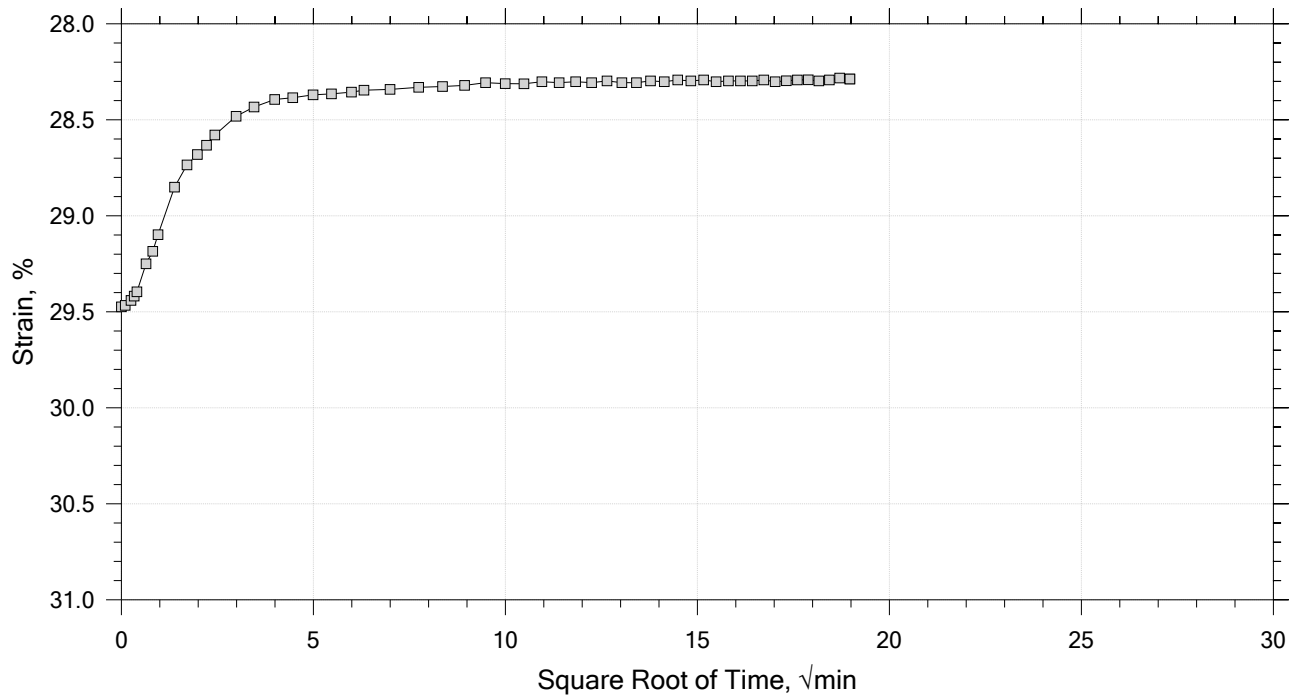
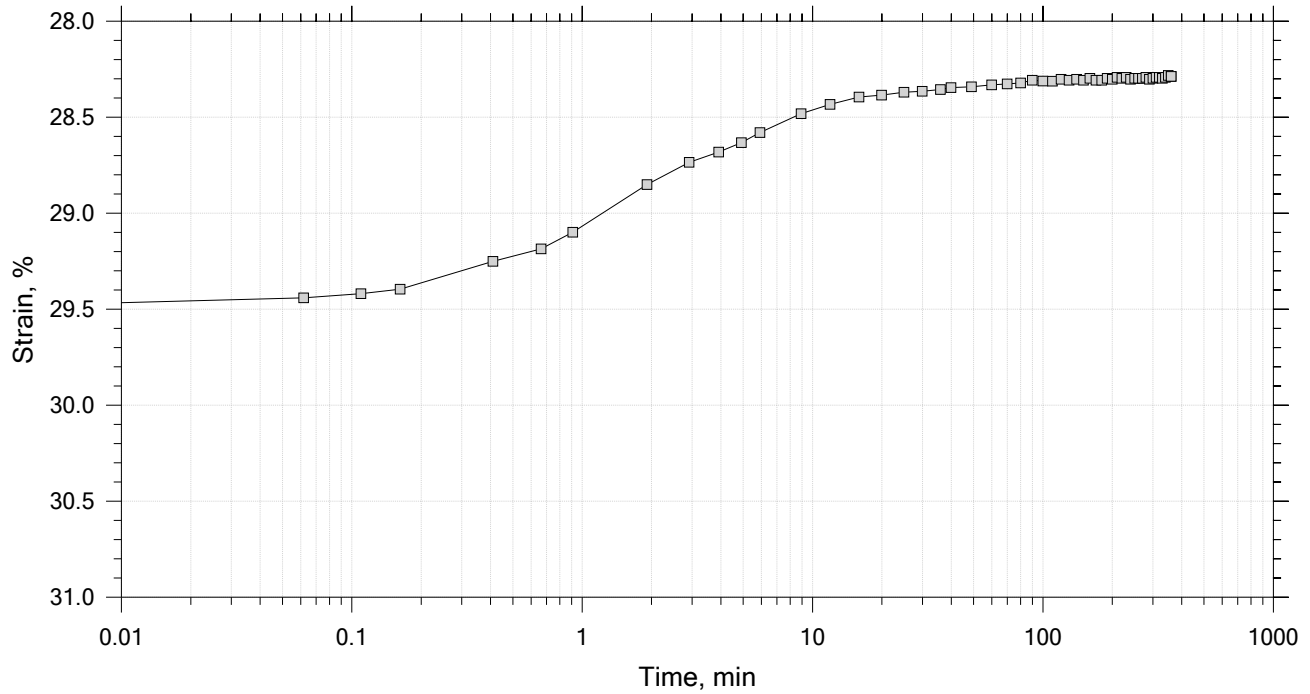
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



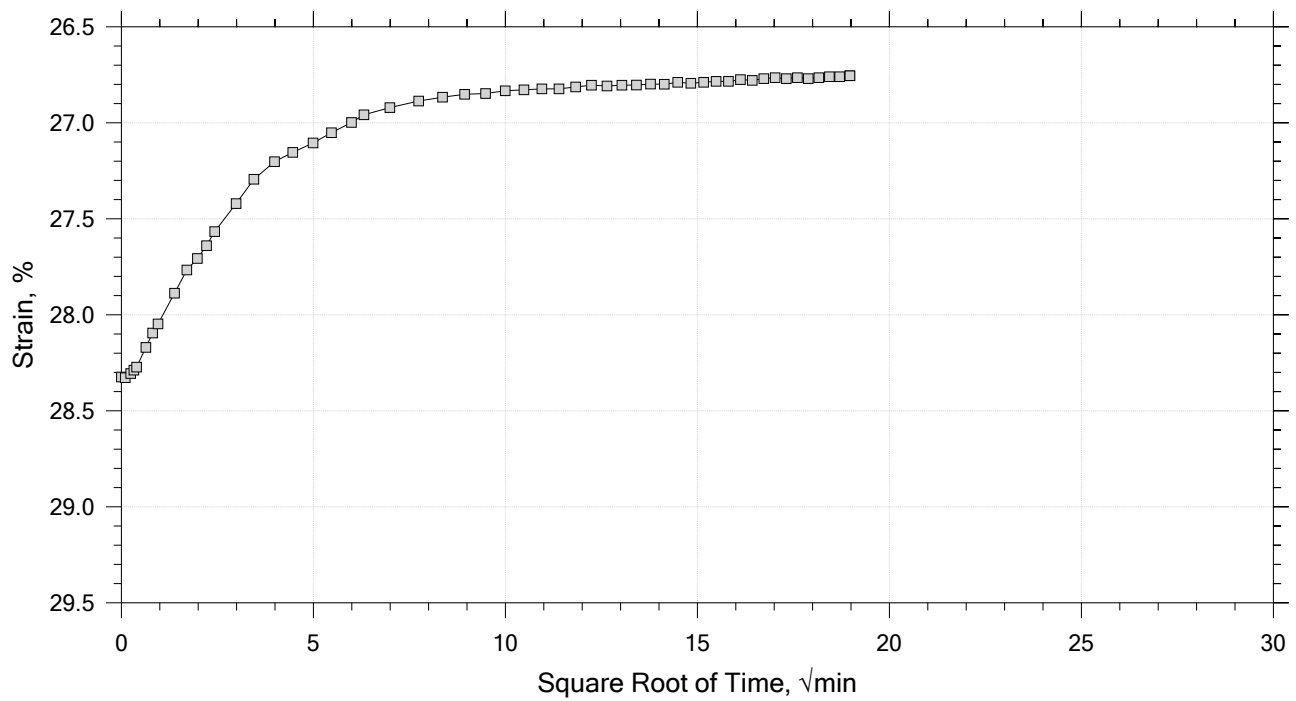
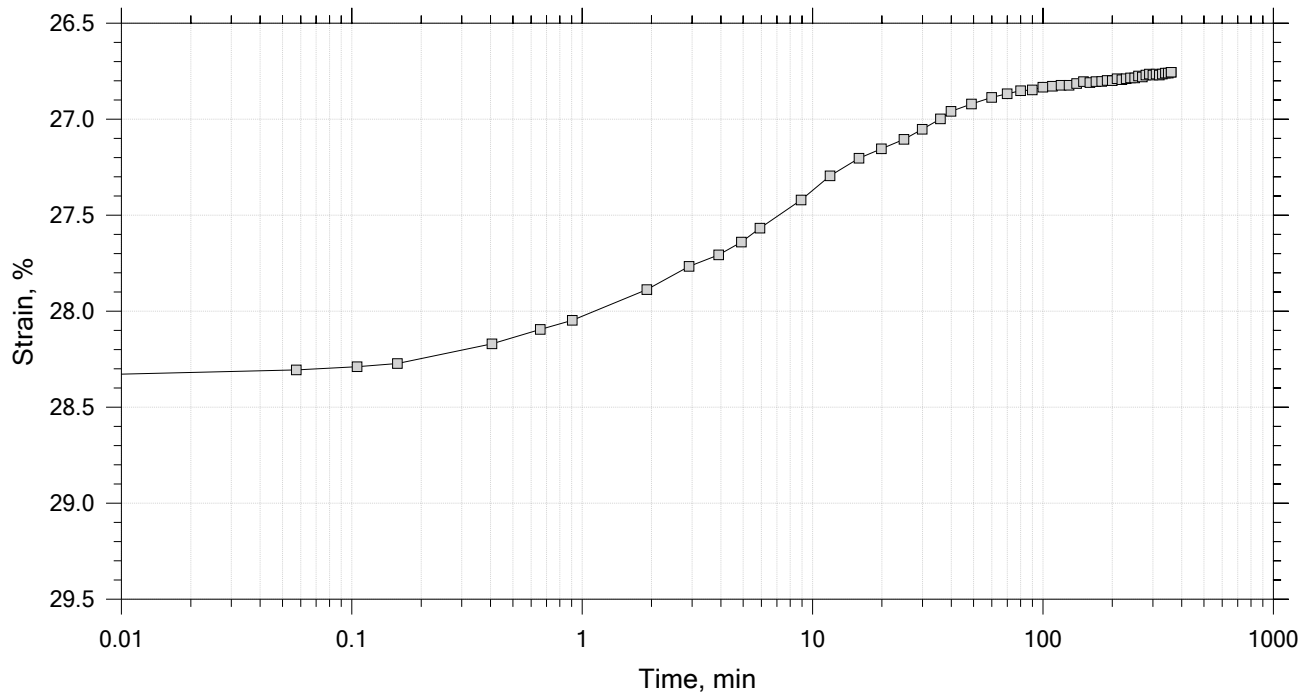
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



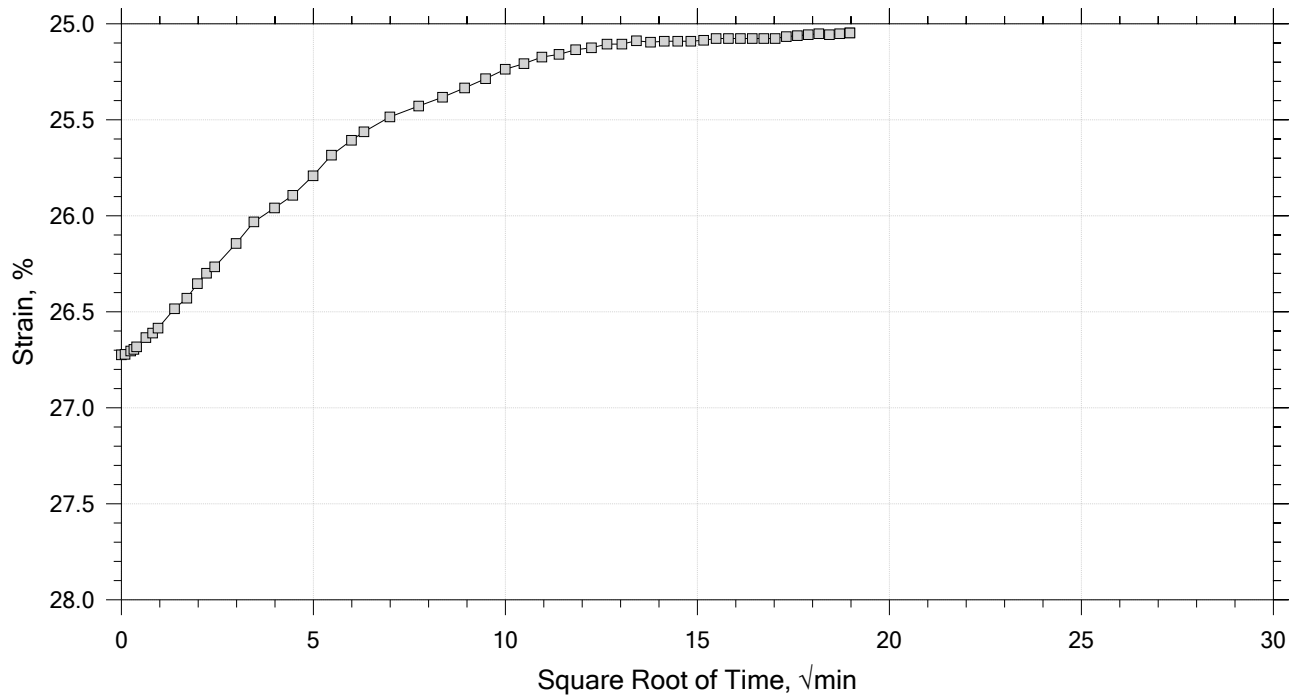
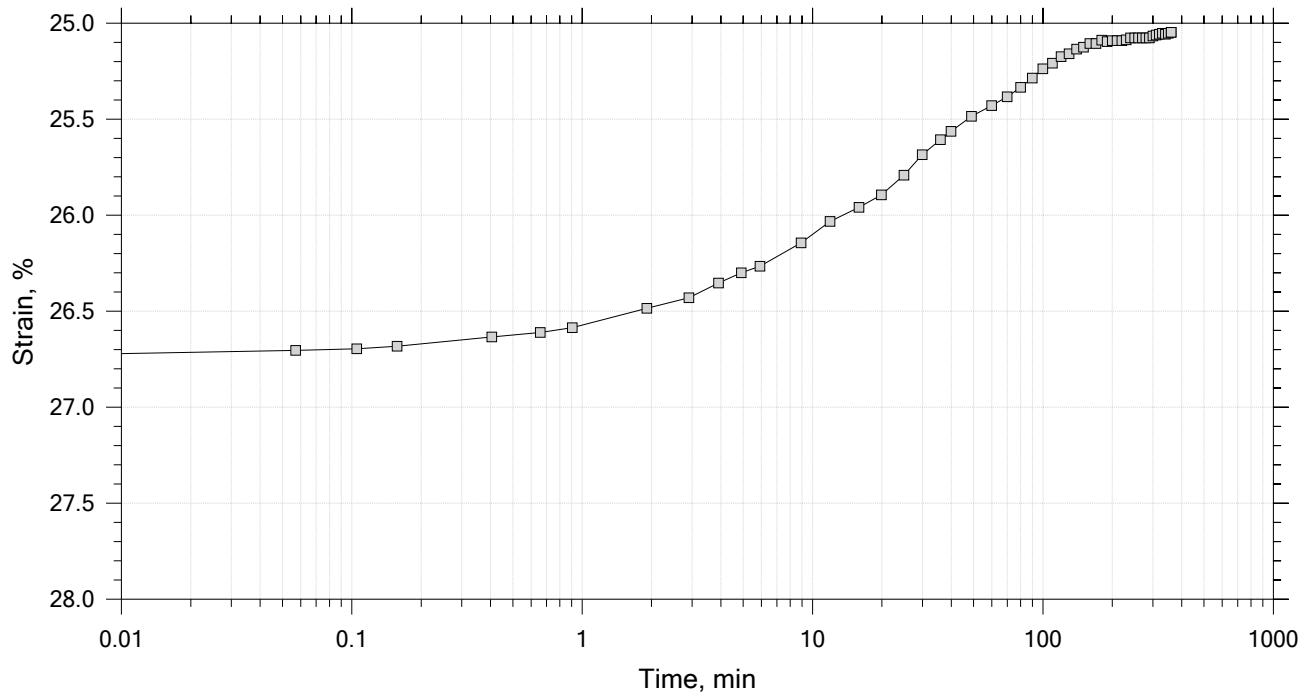
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



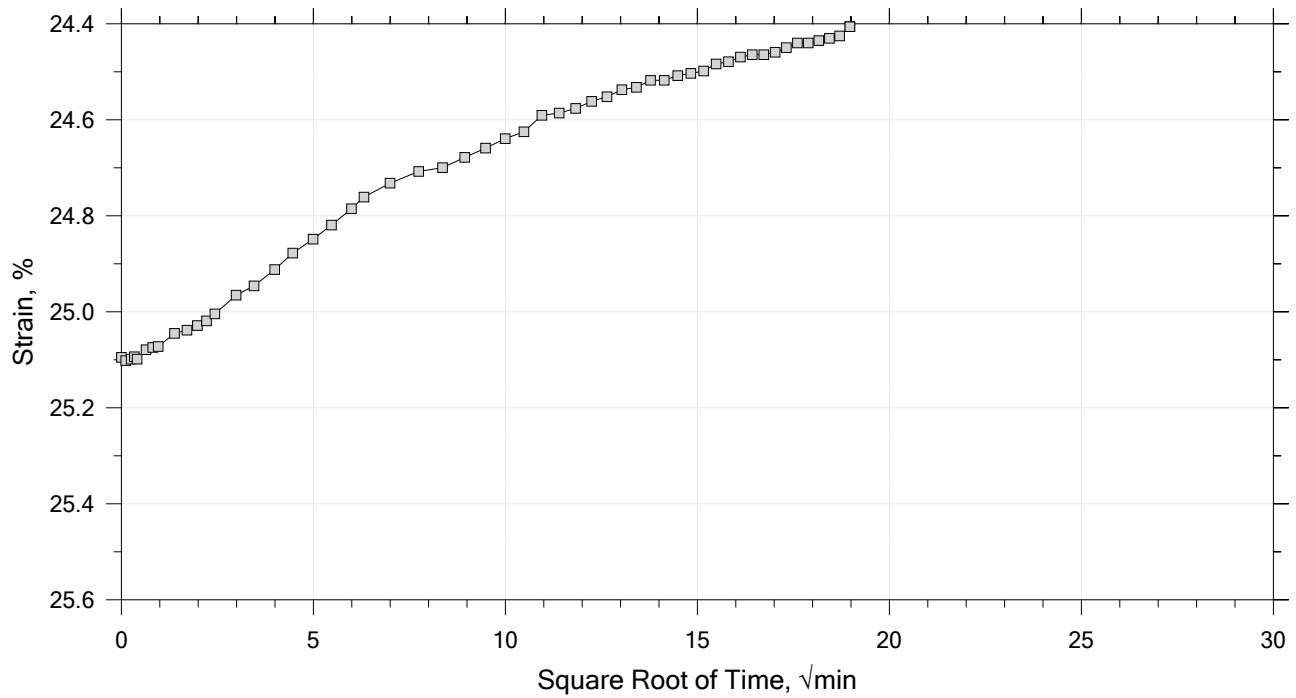
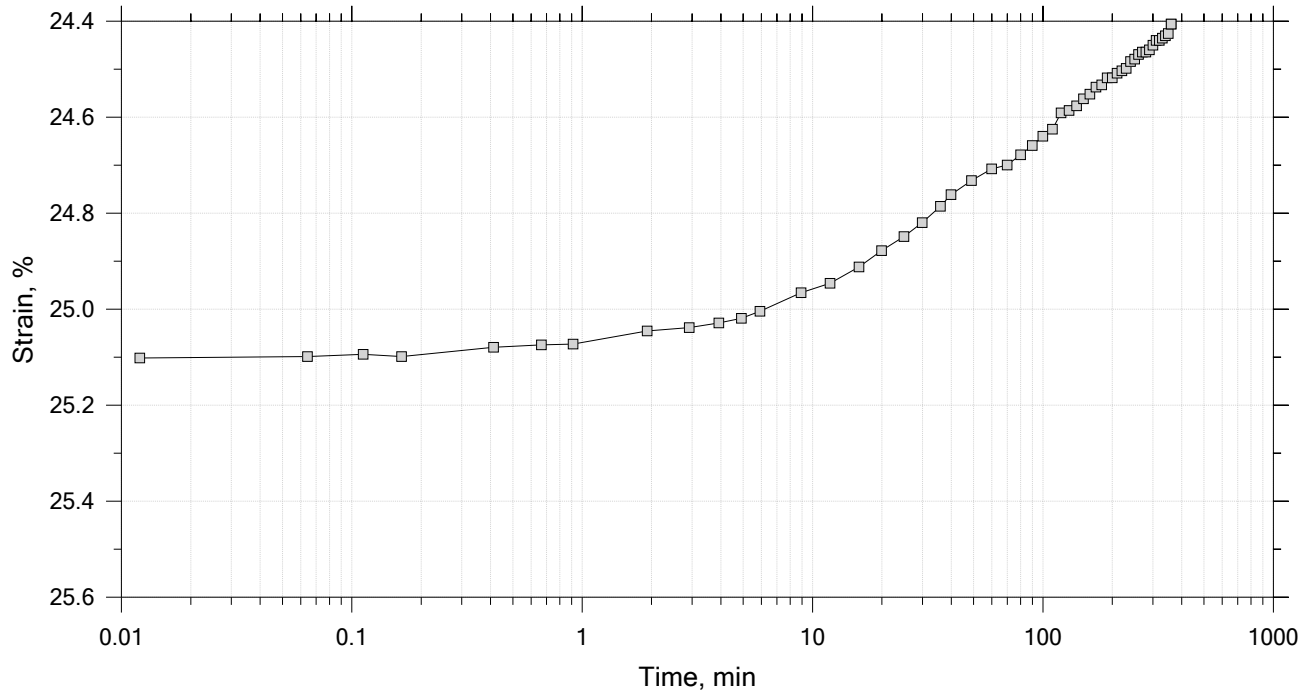
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 38
Initial Height: 1.00 in	Initial Void Ratio: 1.25	Plastic Limit: 19
Final Height: 0.76 in	Final Void Ratio: 0.697	Plasticity Index: 19

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	B-3093	RING		D2974
Mass Container, gm	8.23	108.75	108.75	8.16
Mass Container + Wet Soil, gm	201.16	250.02	232.09	131.6
Mass Container + Dry Soil, gm	141.71	207.11	207.11	106.6
Mass Dry Soil, gm	133.48	98.36	98.36	98.44
Water Content, %	44.54	43.63	25.40	25.40
Void Ratio	---	1.25	0.70	---
Degree of Saturation, %	---	96.18	100.00	---
Dry Unit Weight, pcf	---	76.336	100.98	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

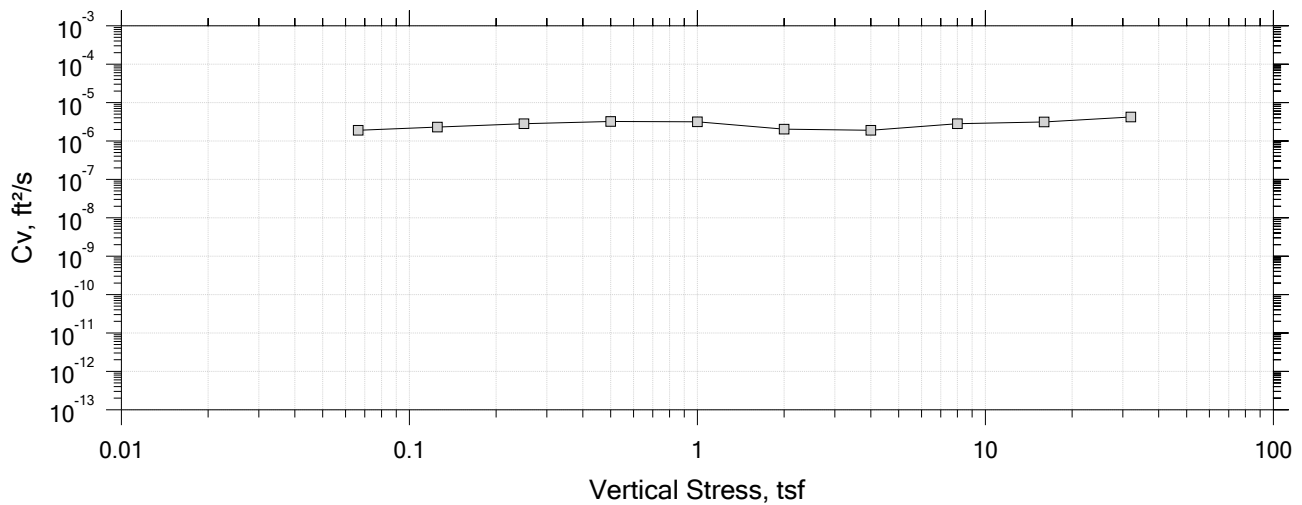
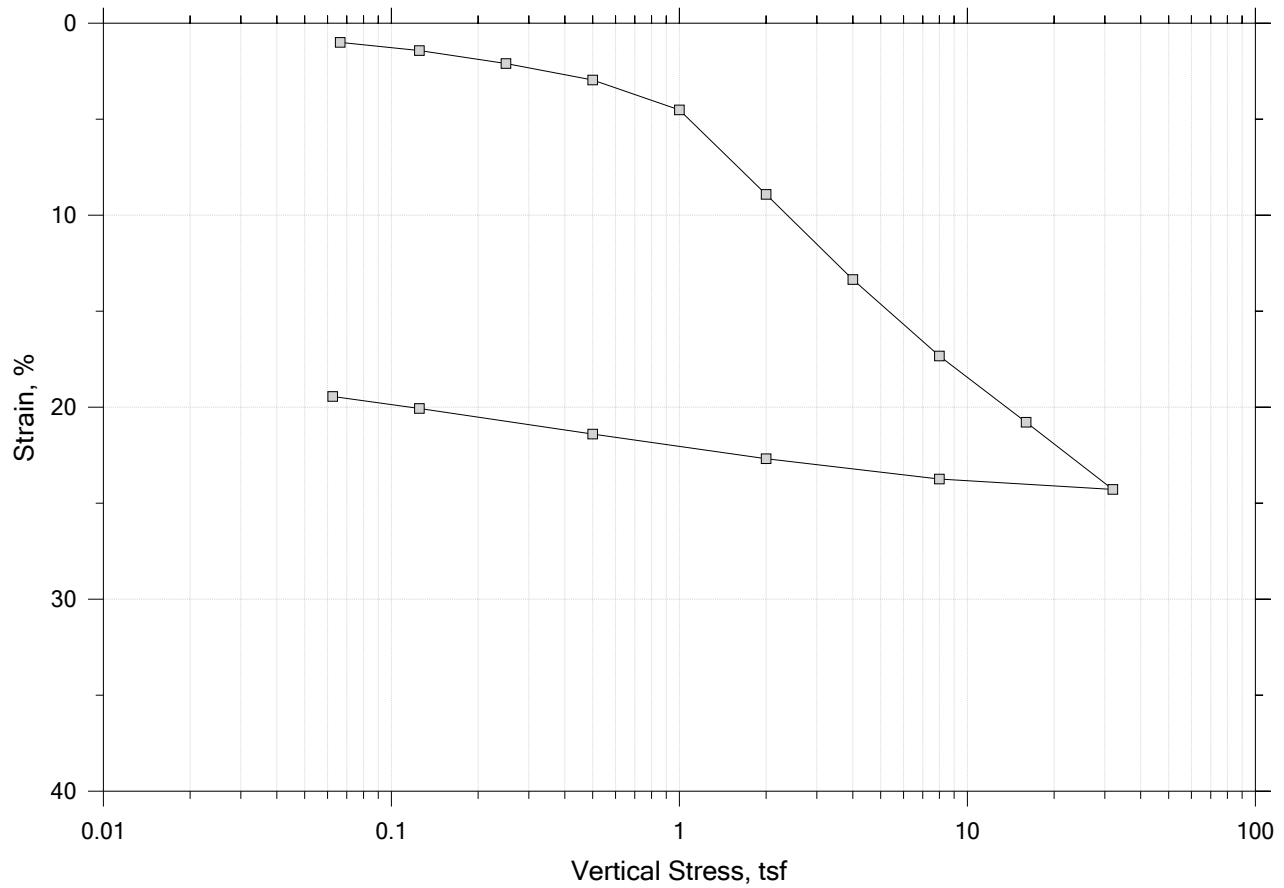
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB-202	Tested By: md	Checked By: mcm
	Sample No.: U-1	Test Date: 03/05/21	Depth: 18-20
	Test No.: IP-17	Sample Type: Intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System X, Swell Pressure = 0.0679 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

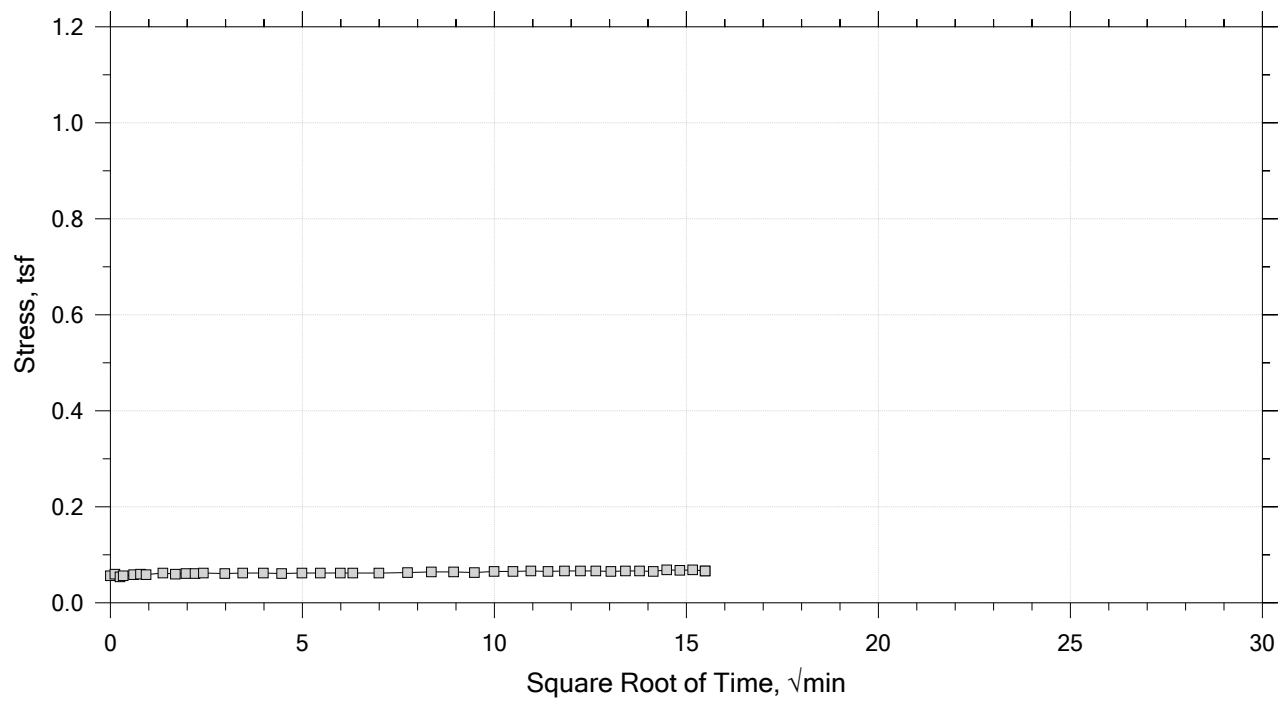
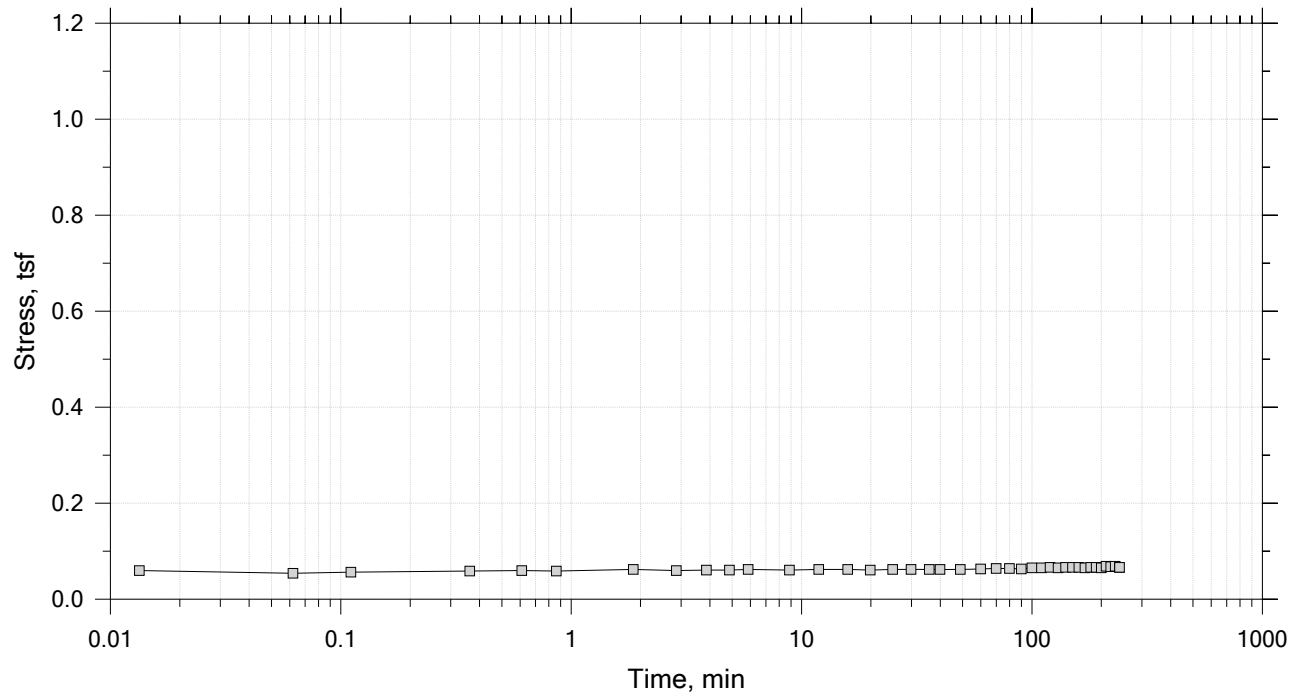
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

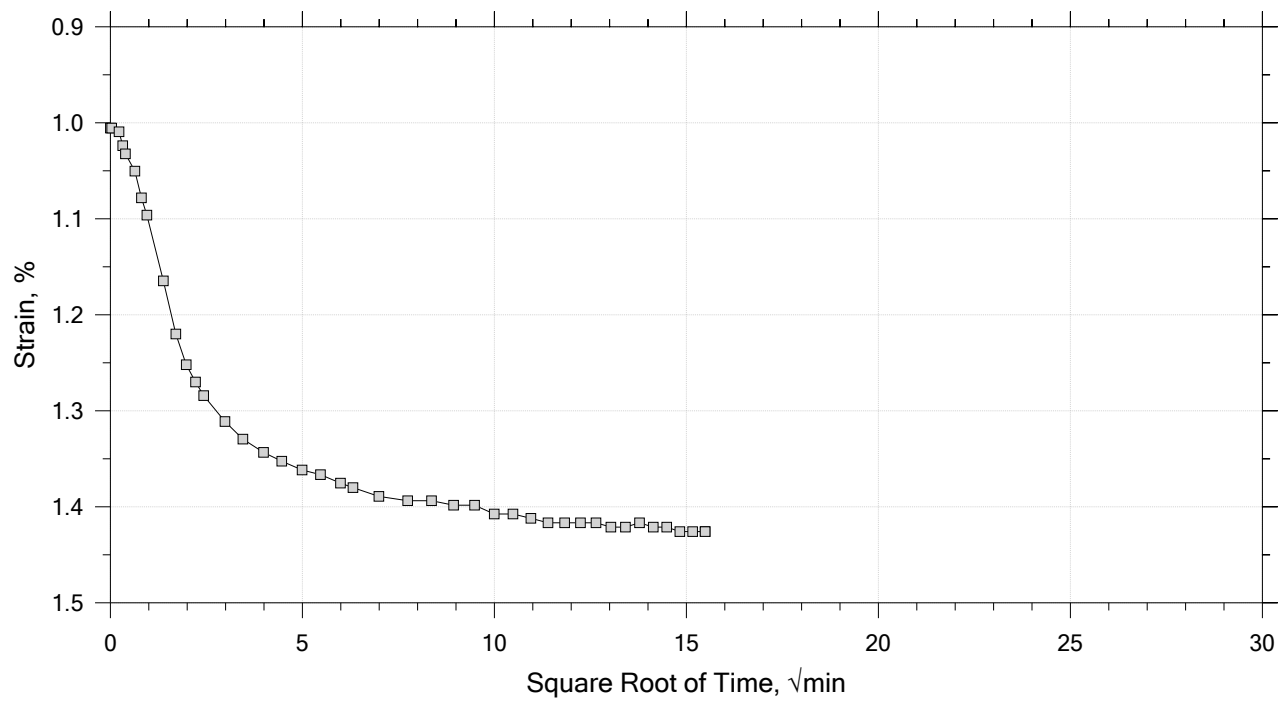
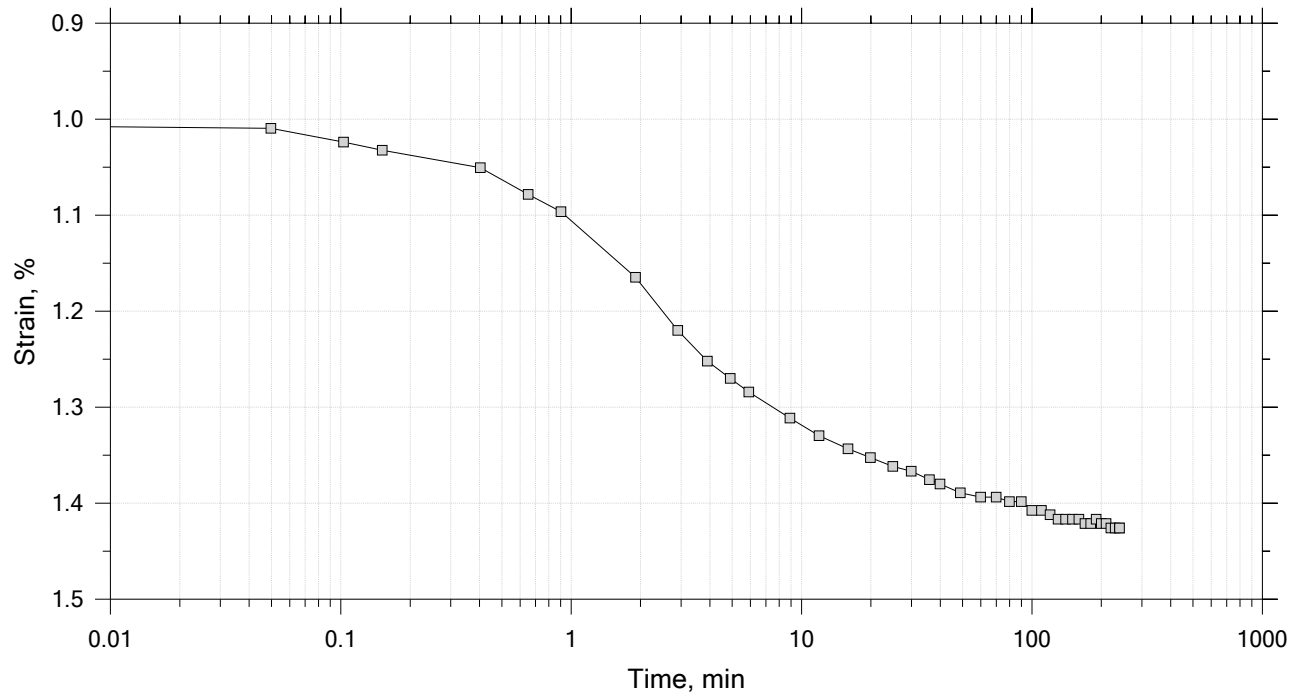
Time Curve 1 of 15
Constant Volume Step
Stress: 0.0664 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15
Constant Load Step
Stress: 0.125 tsf



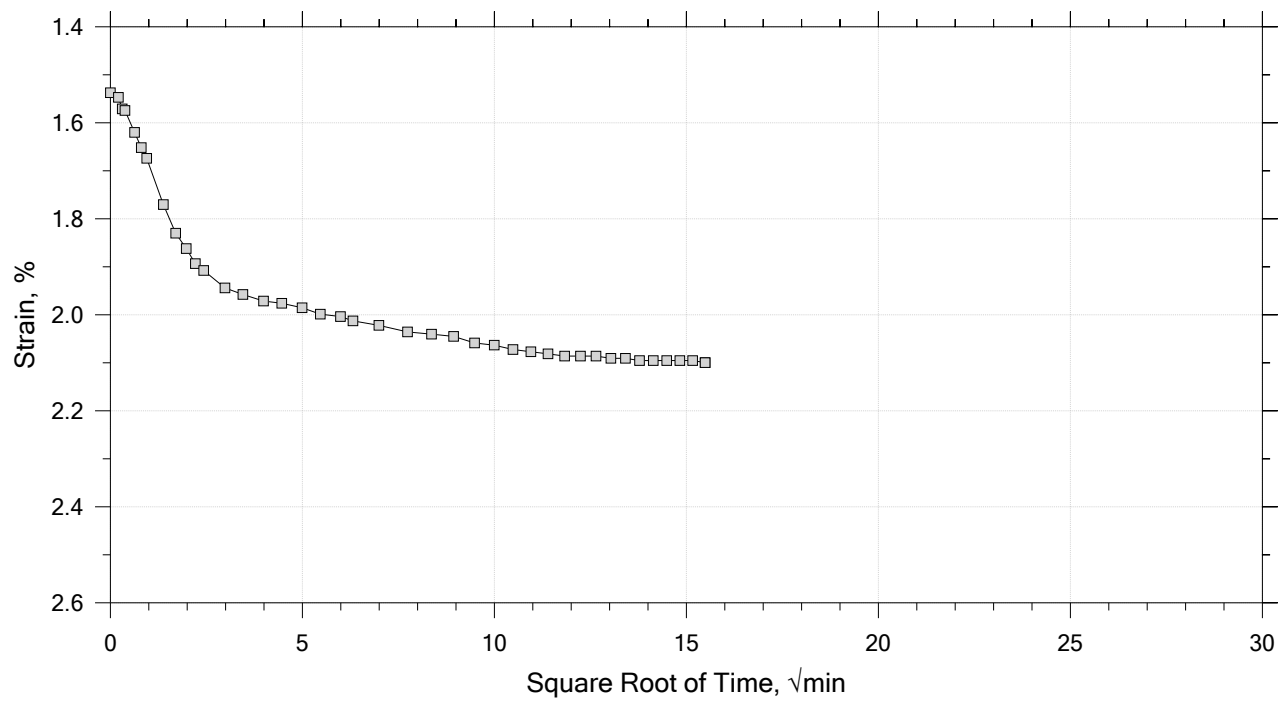
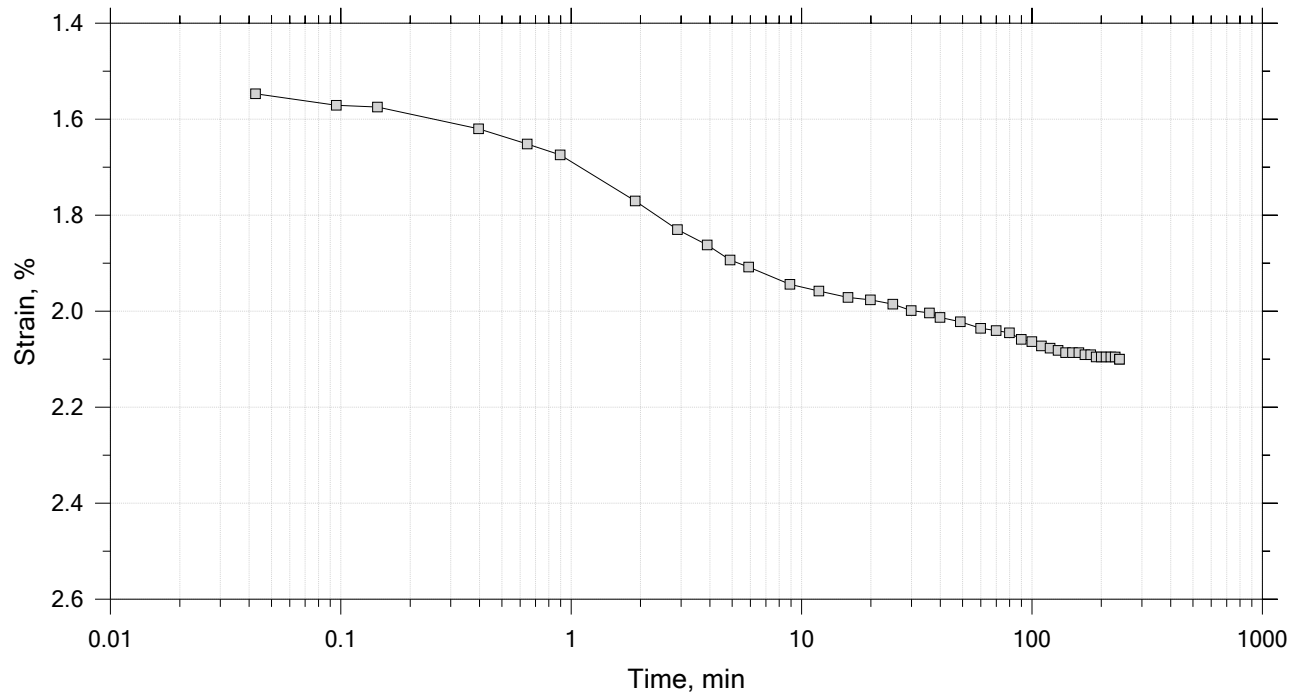
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



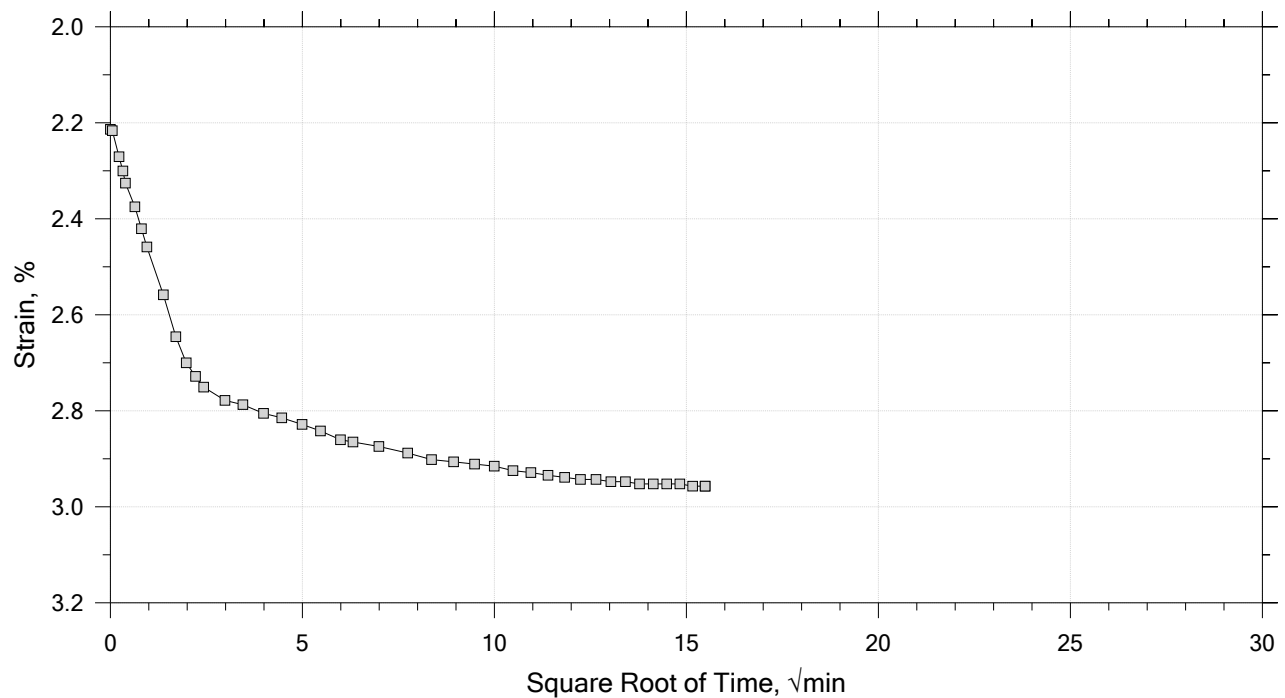
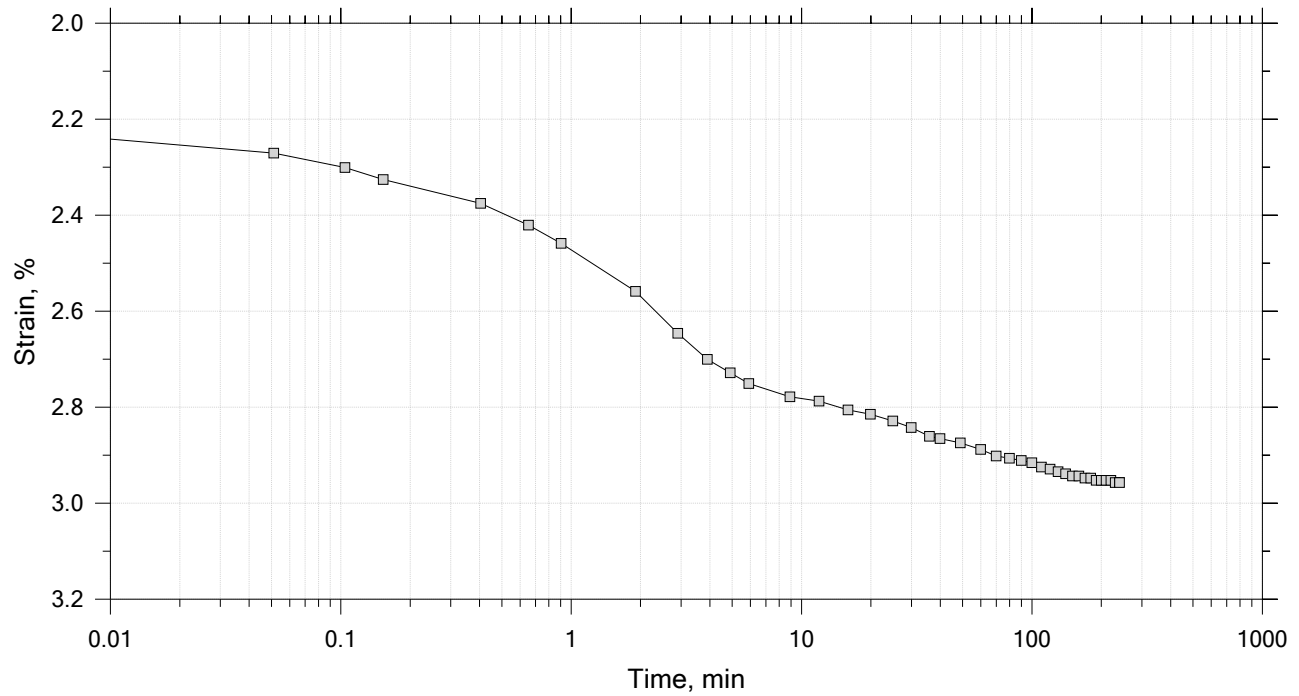
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



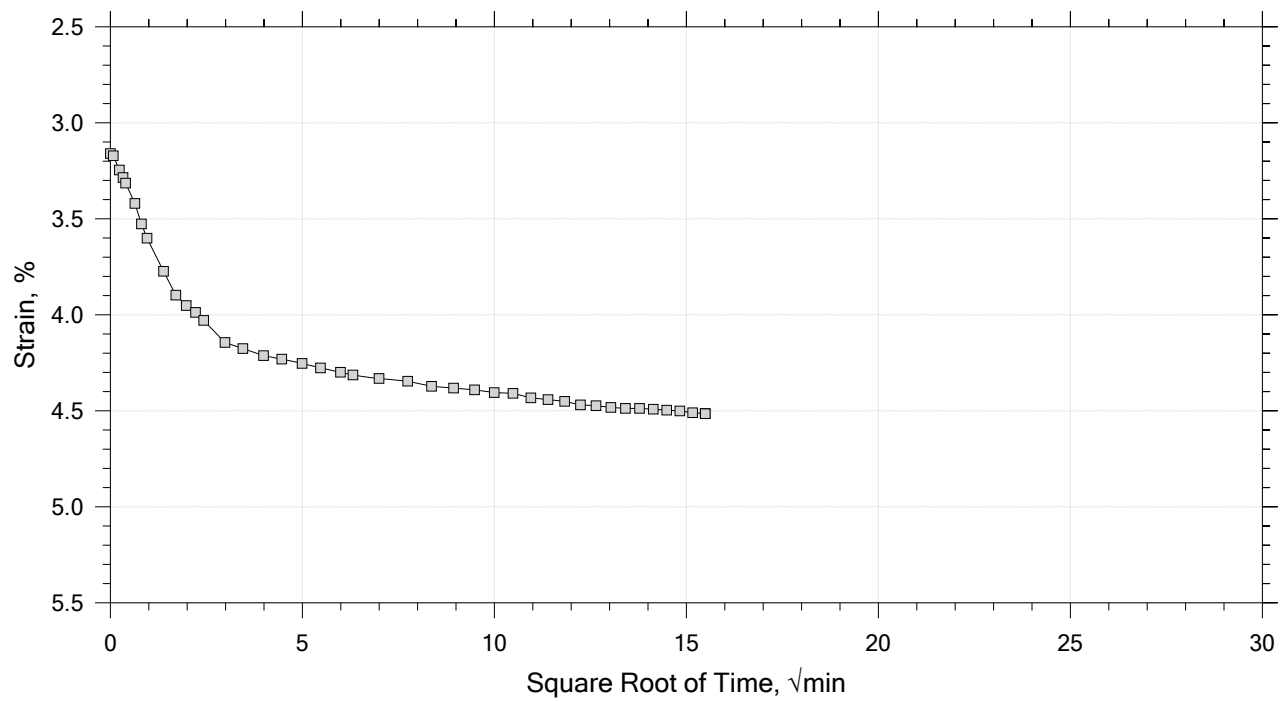
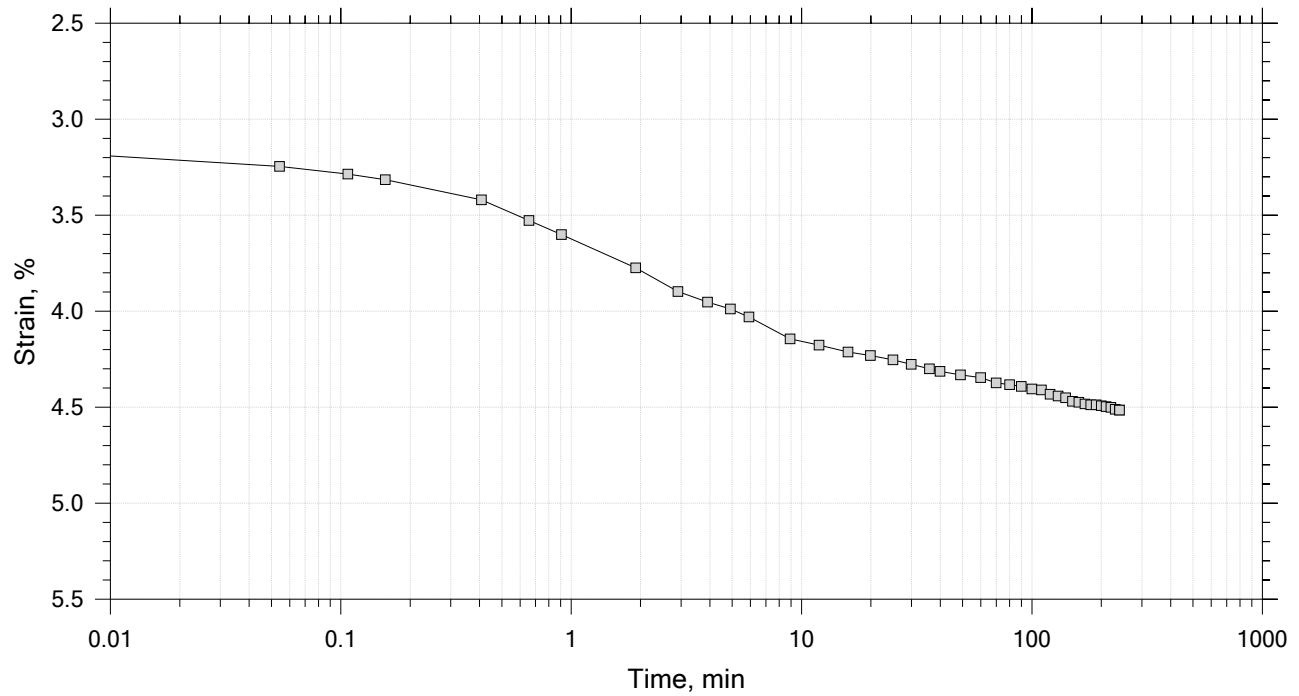
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



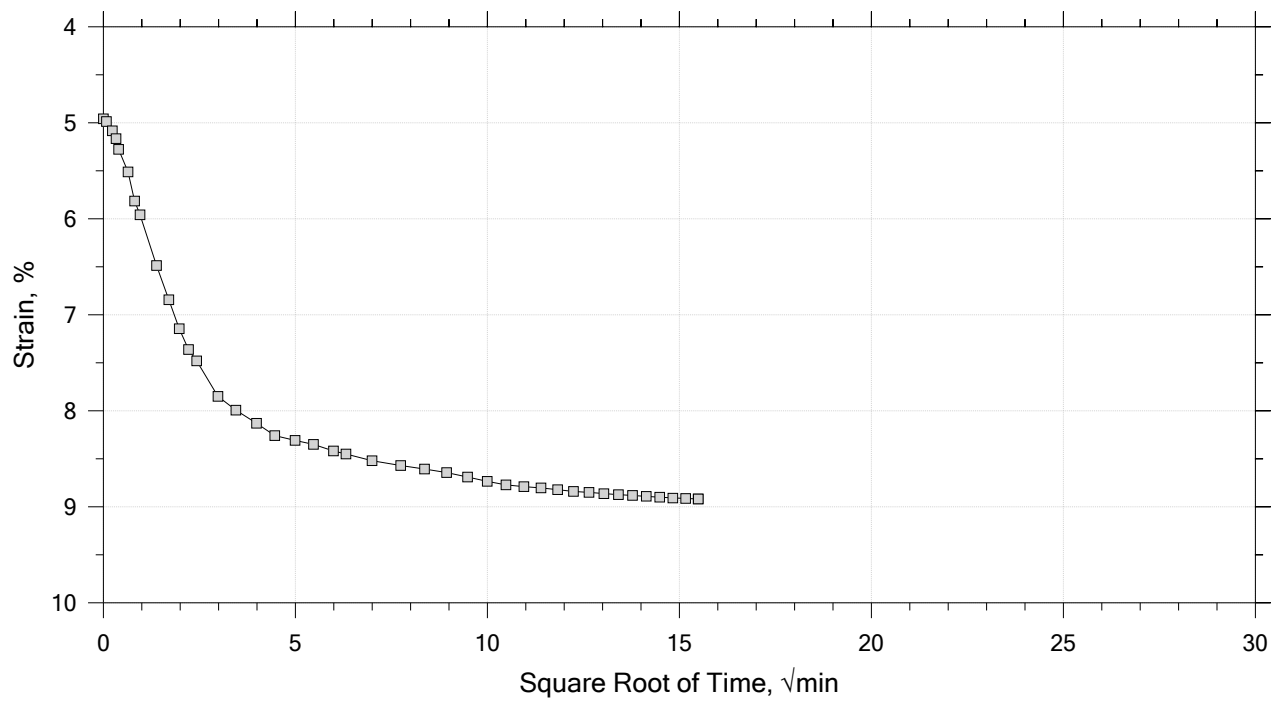
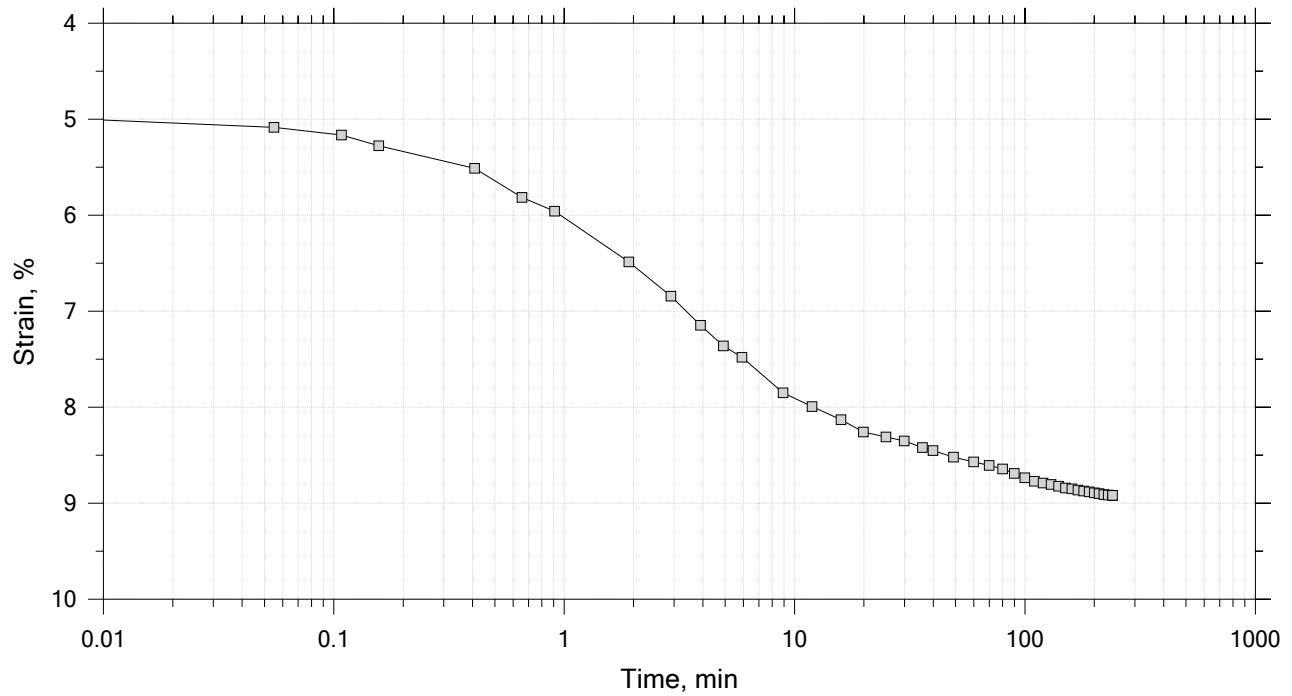
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



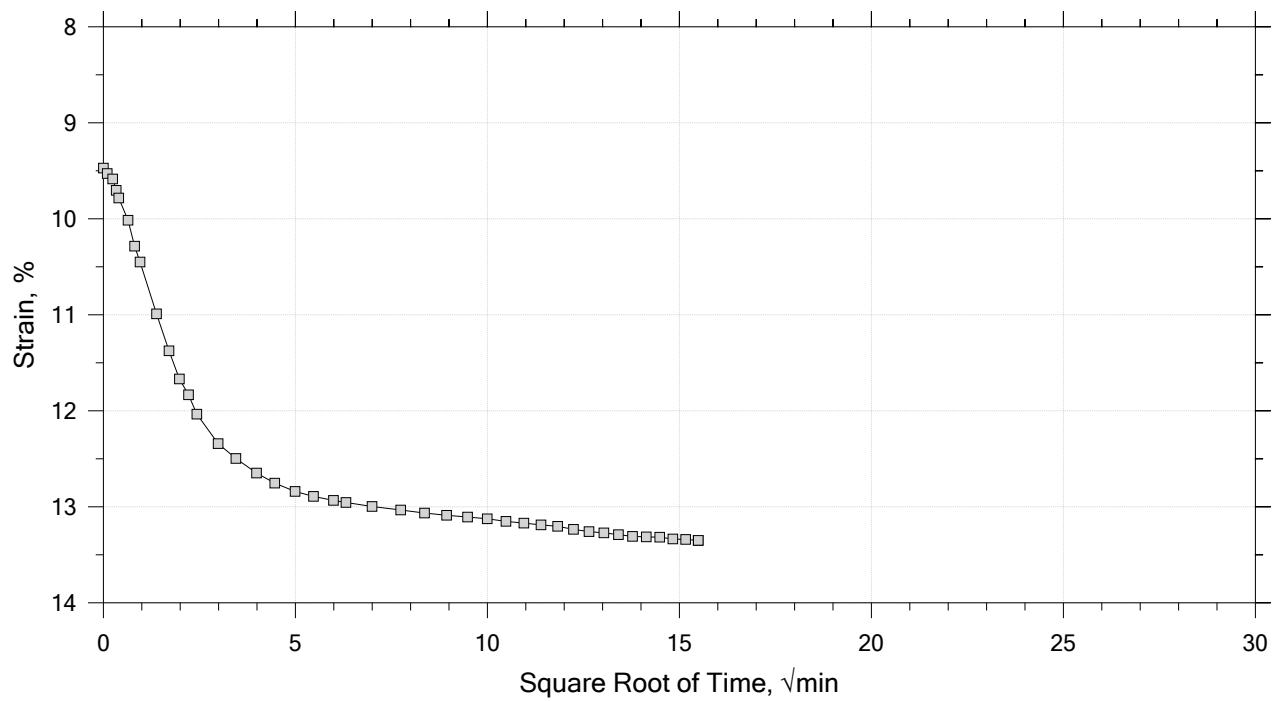
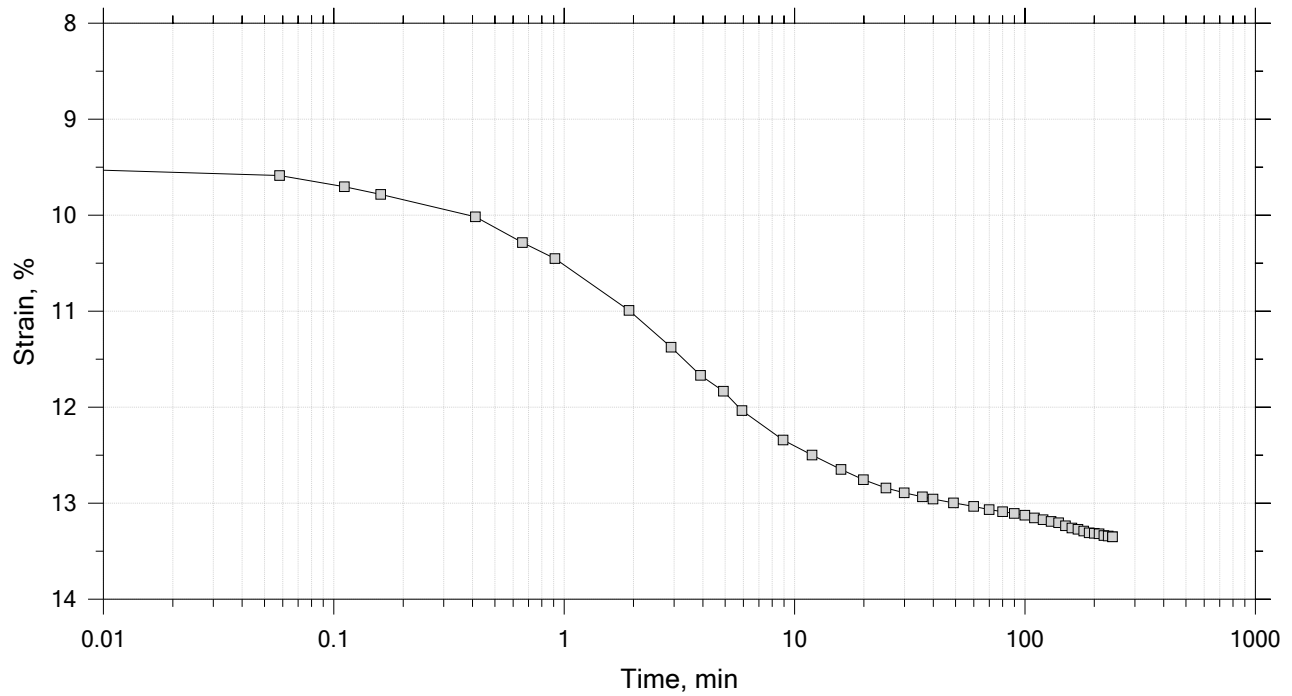
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



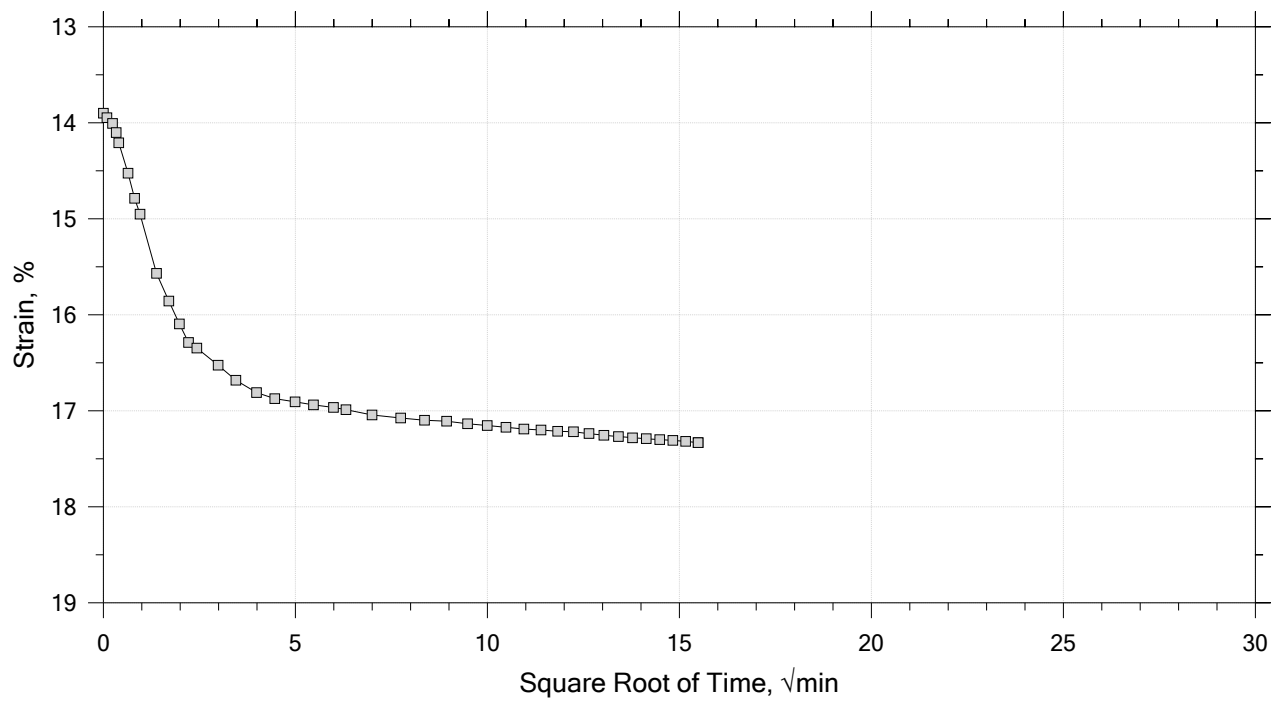
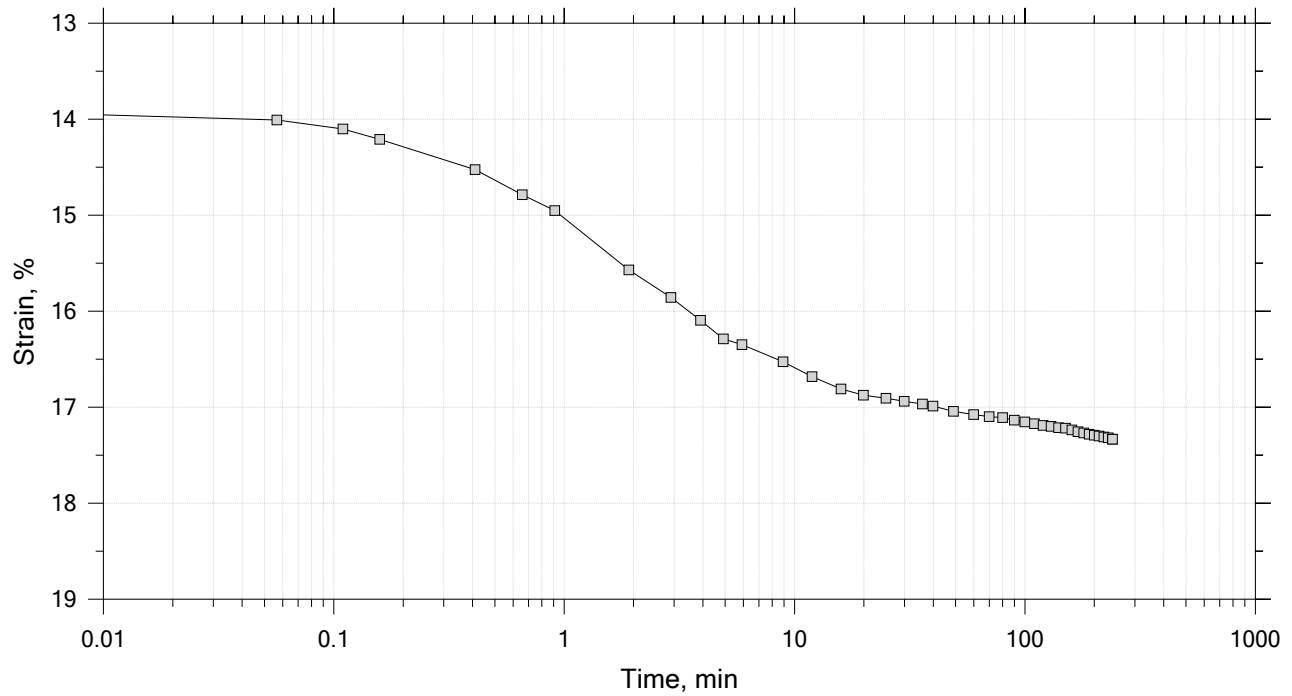
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



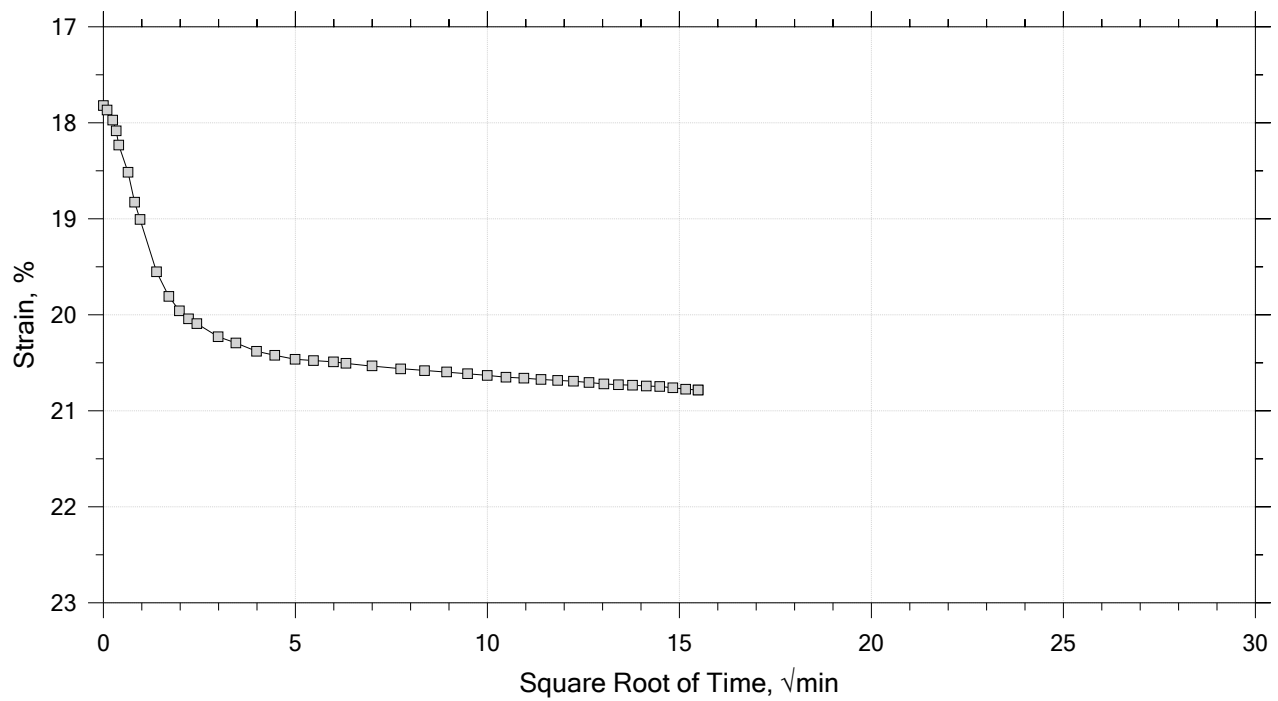
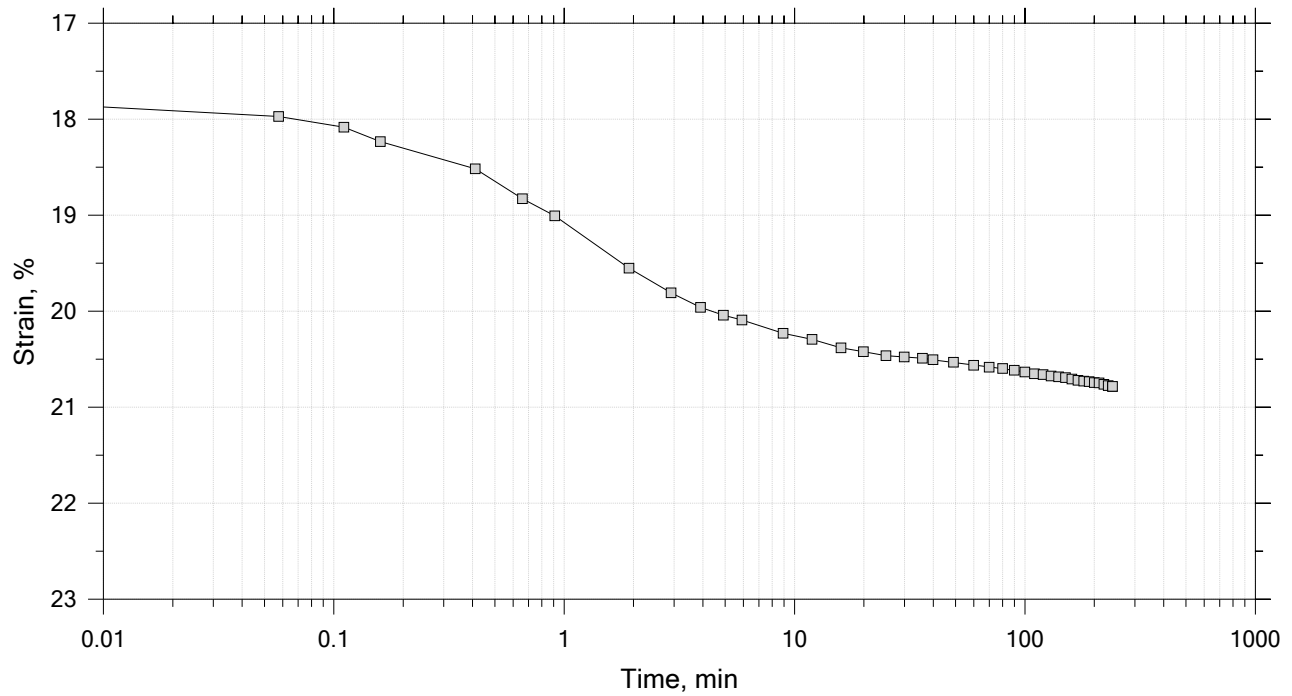
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



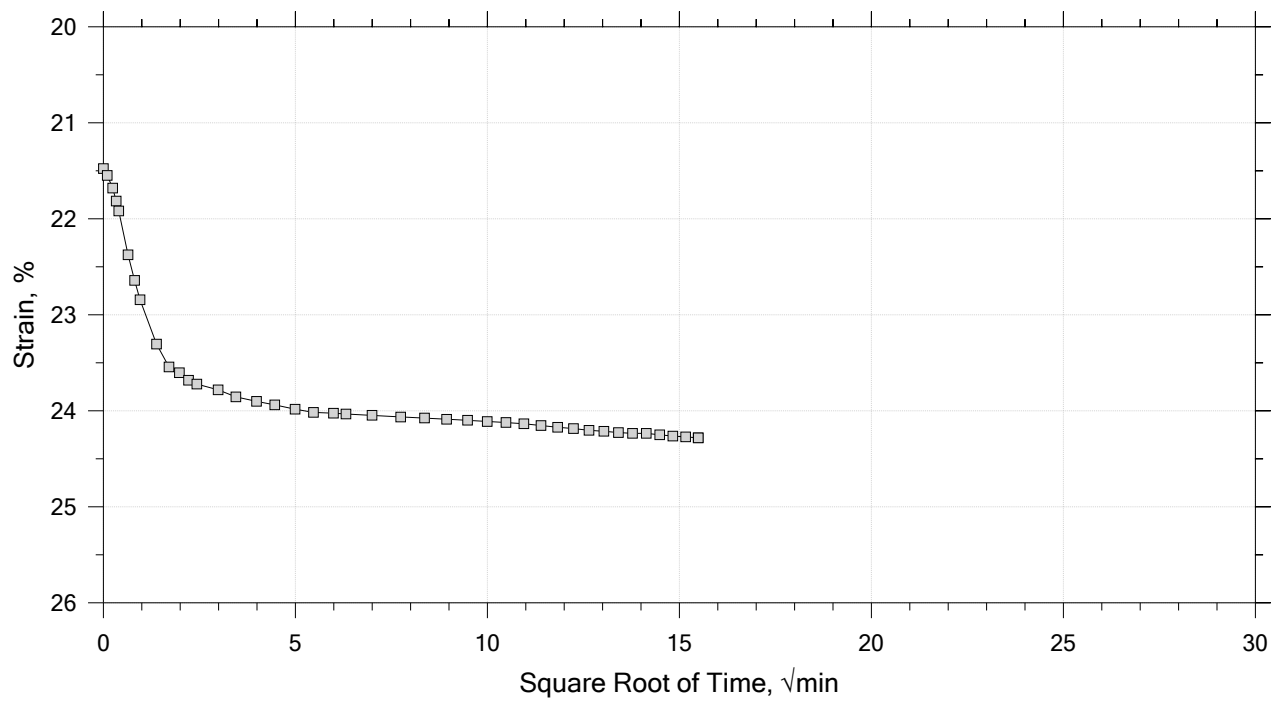
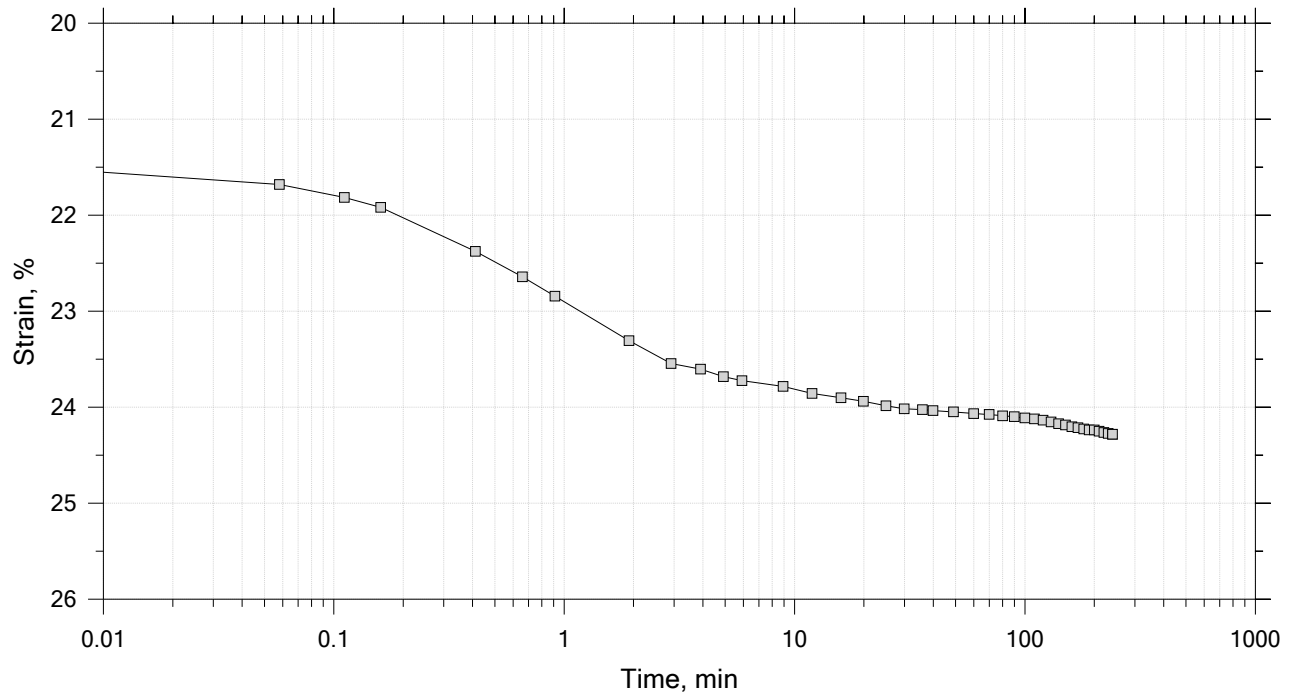
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



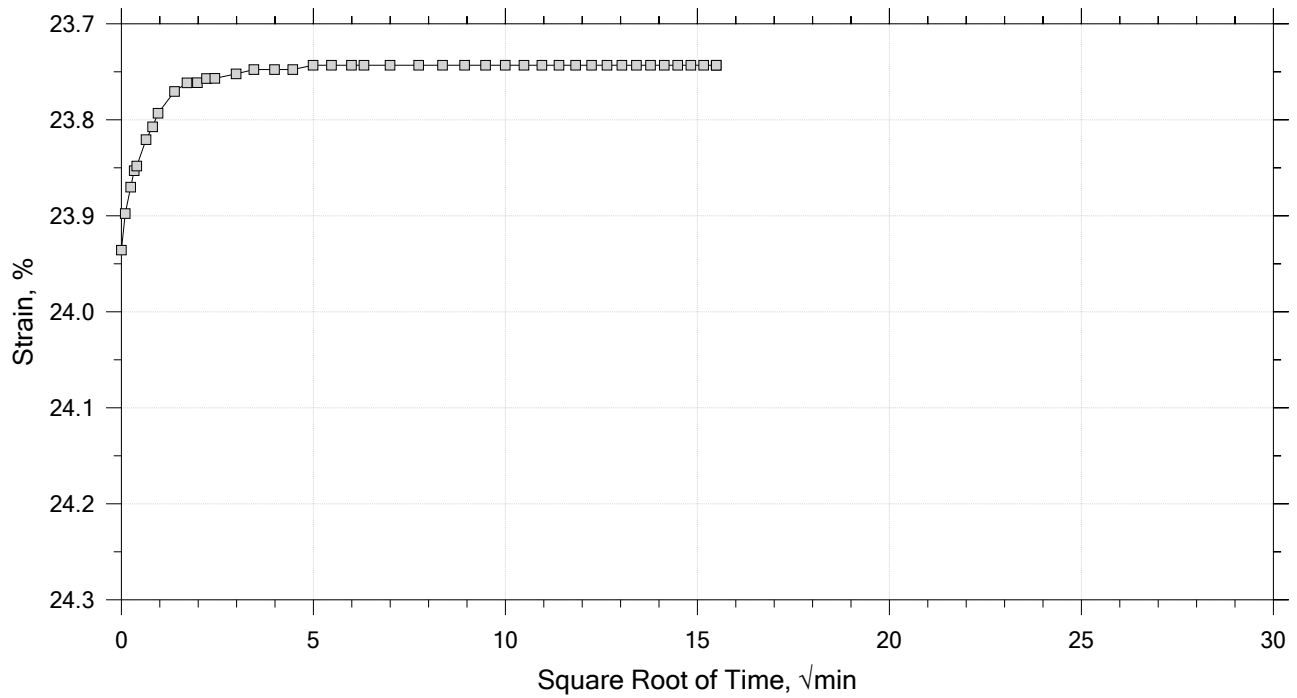
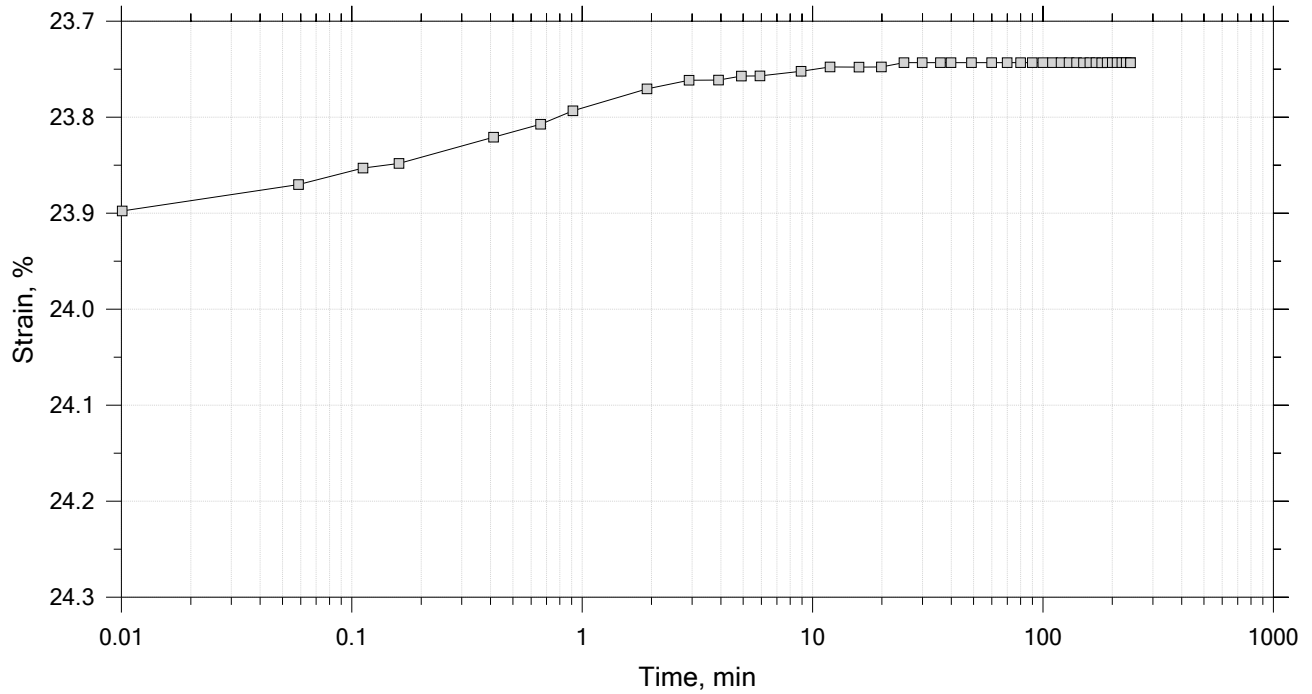
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



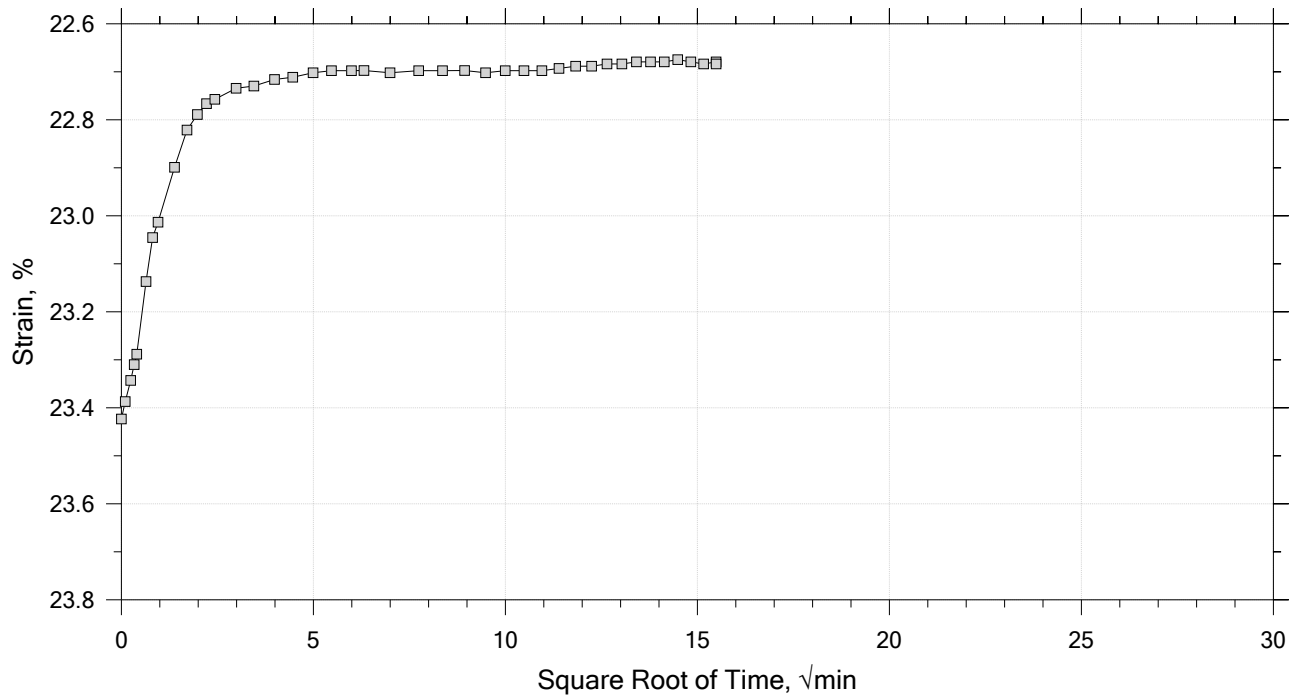
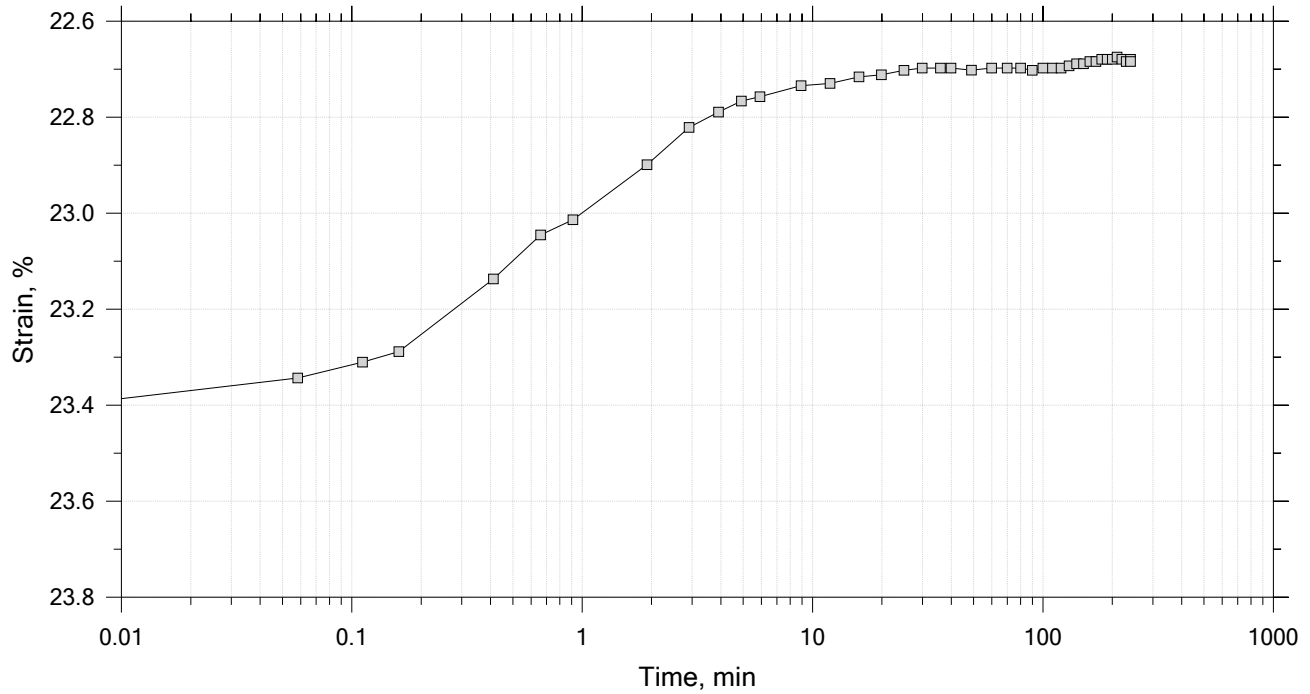
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



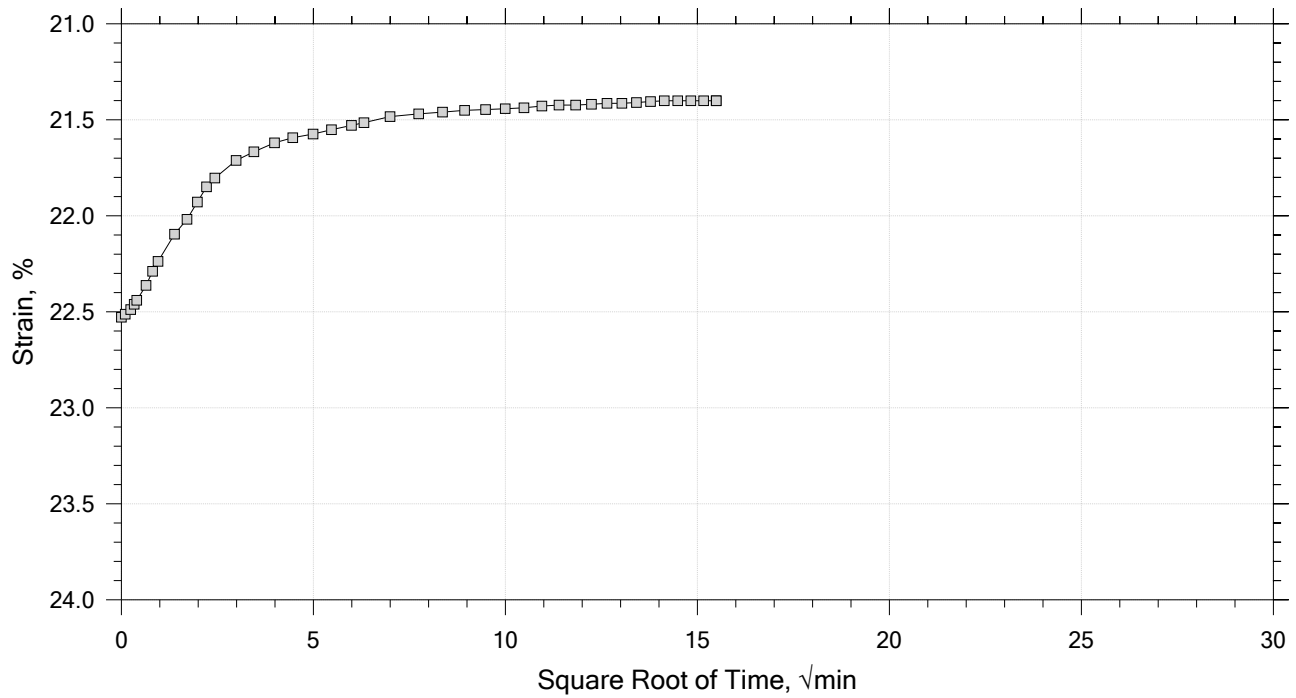
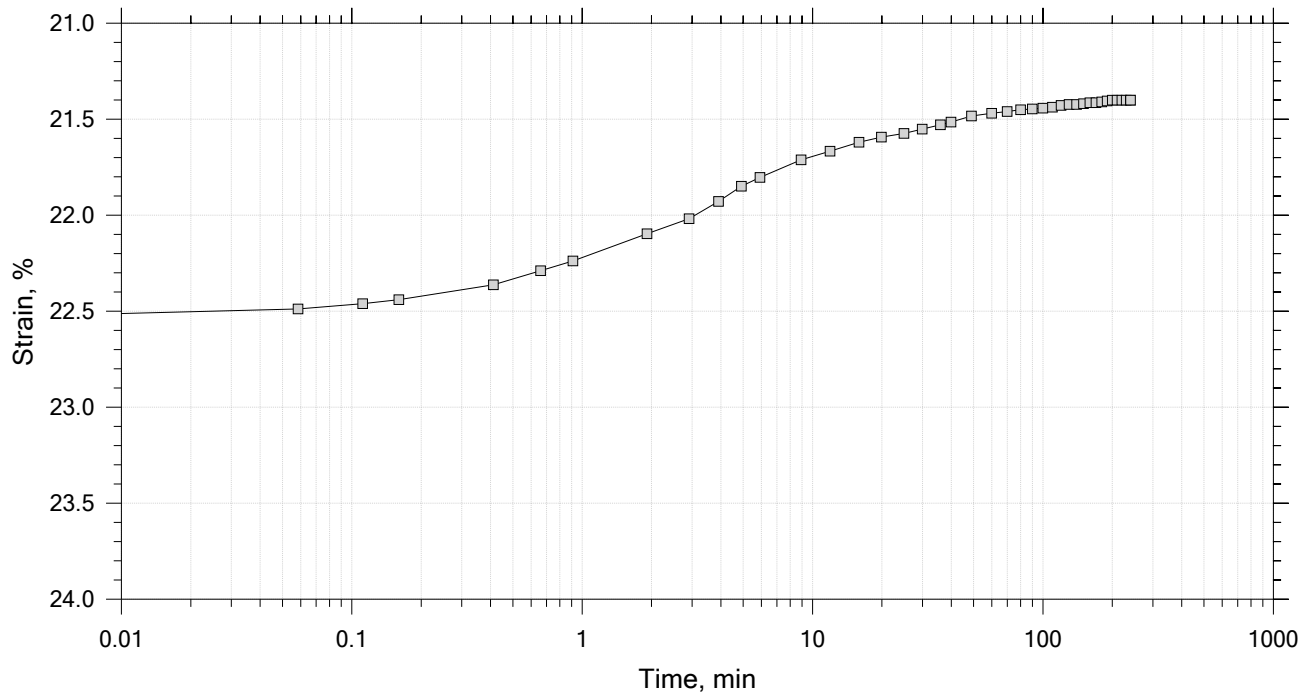
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



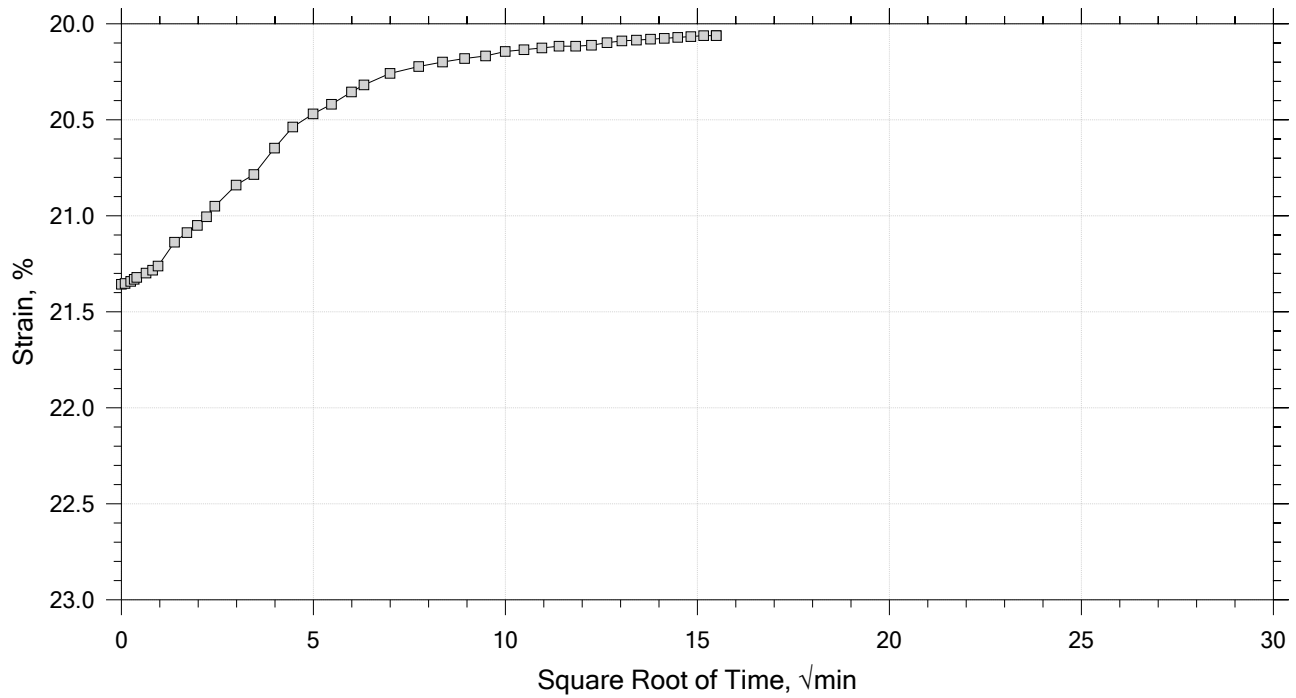
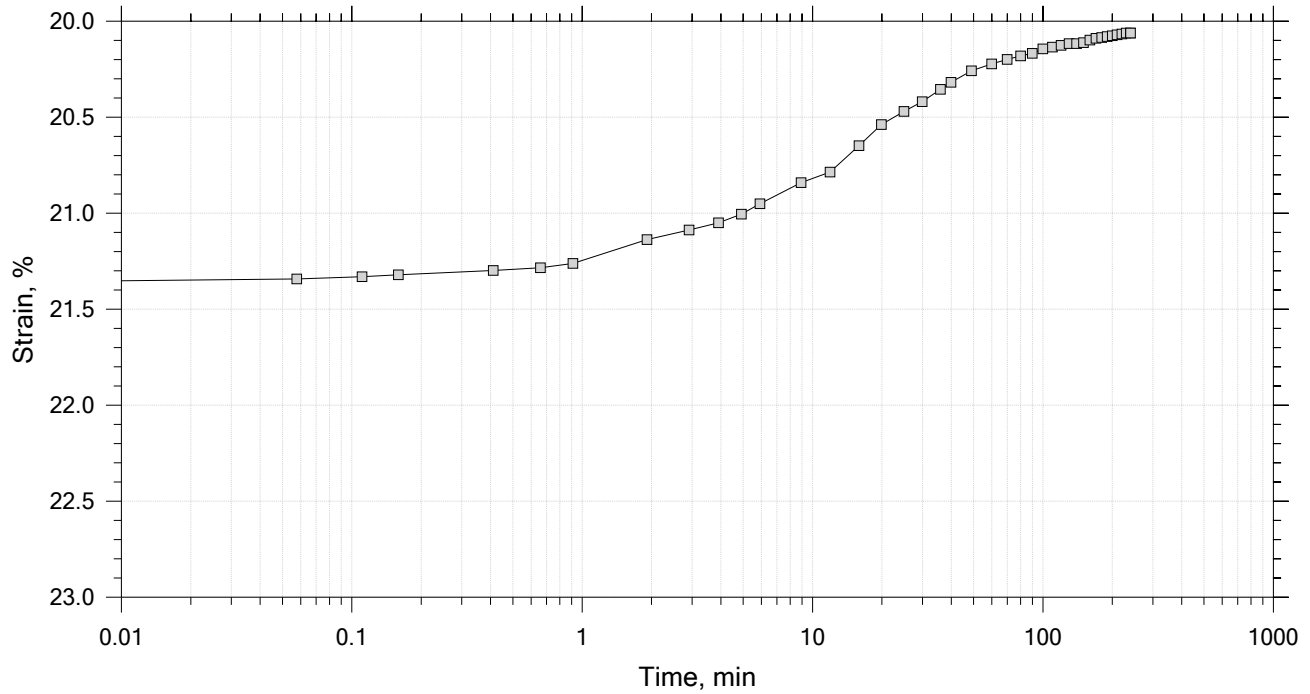
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



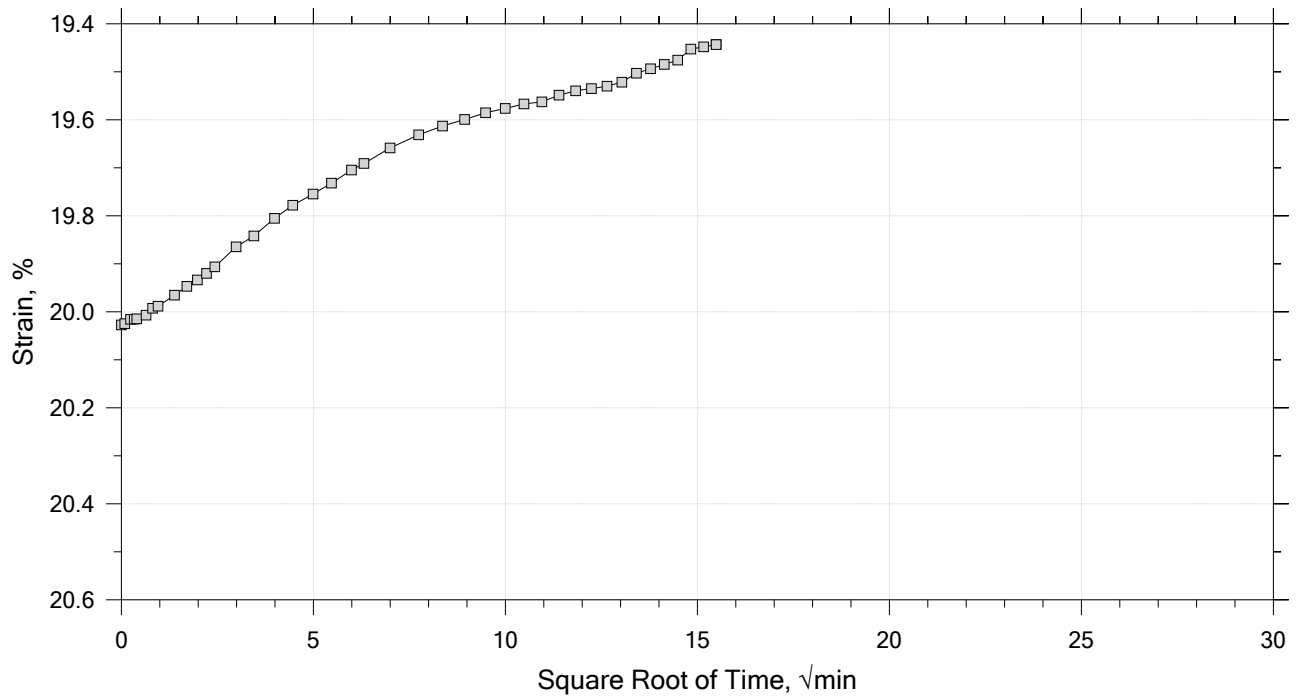
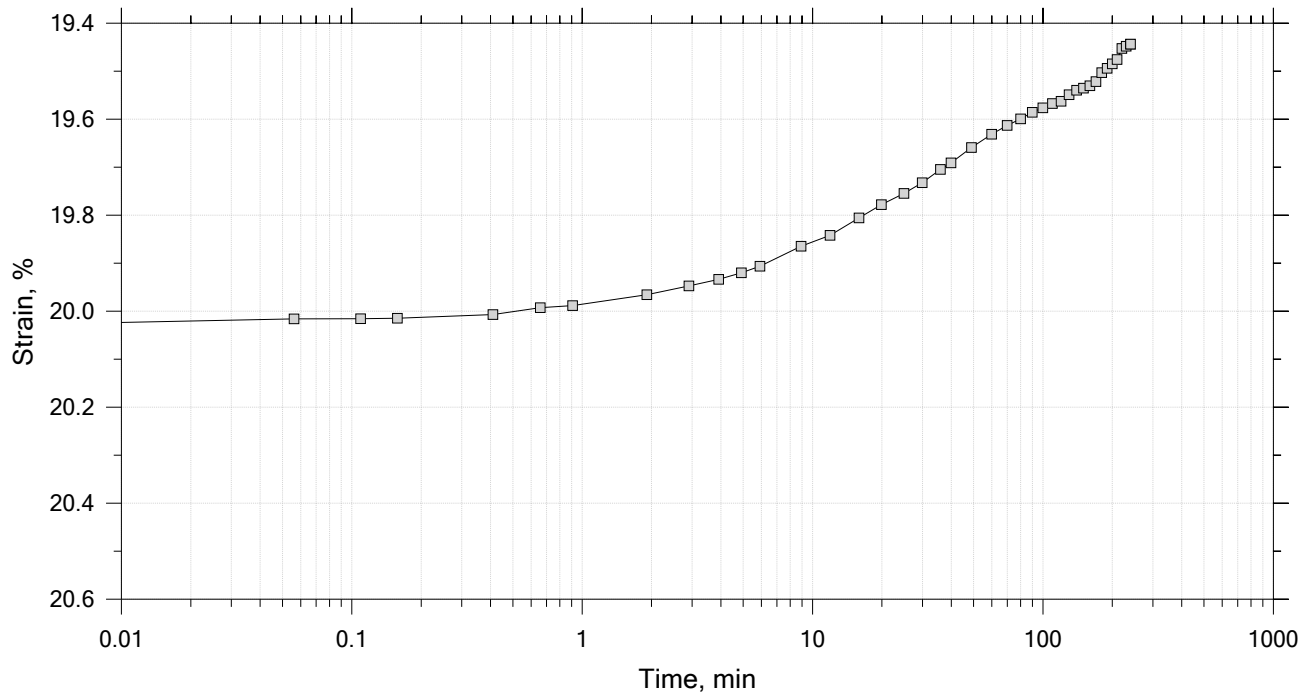
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 38
Initial Height: 1.00 in	Initial Void Ratio: 1.1	Plastic Limit: 20
Final Height: 0.81 in	Final Void Ratio: 0.69	Plasticity Index: 18

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A2291	RING		B-1837
Mass Container, gm	8.44	108.75	108.75	8.41
Mass Container + Wet Soil, gm	206.5	255.95	241.97	140.28
Mass Container + Dry Soil, gm	153.02	215.51	215.51	114.09
Mass Dry Soil, gm	144.58	106.76	106.76	105.68
Water Content, %	36.99	37.88	24.78	24.78
Void Ratio	---	1.10	0.69	---
Degree of Saturation, %	---	96.06	100.00	---
Dry Unit Weight, pcf	---	82.856	102.85	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

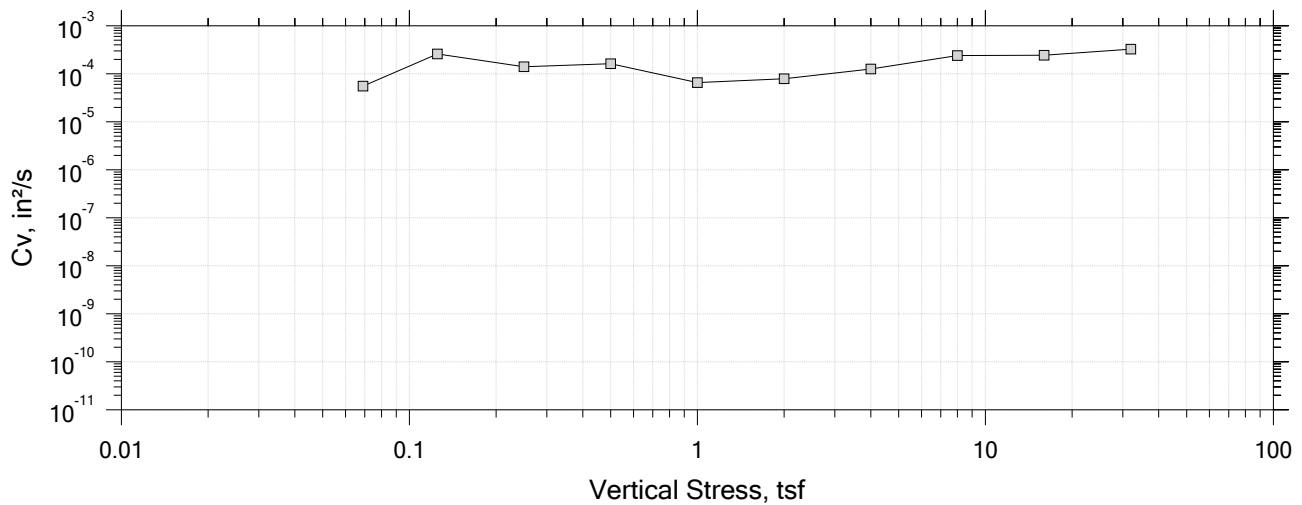
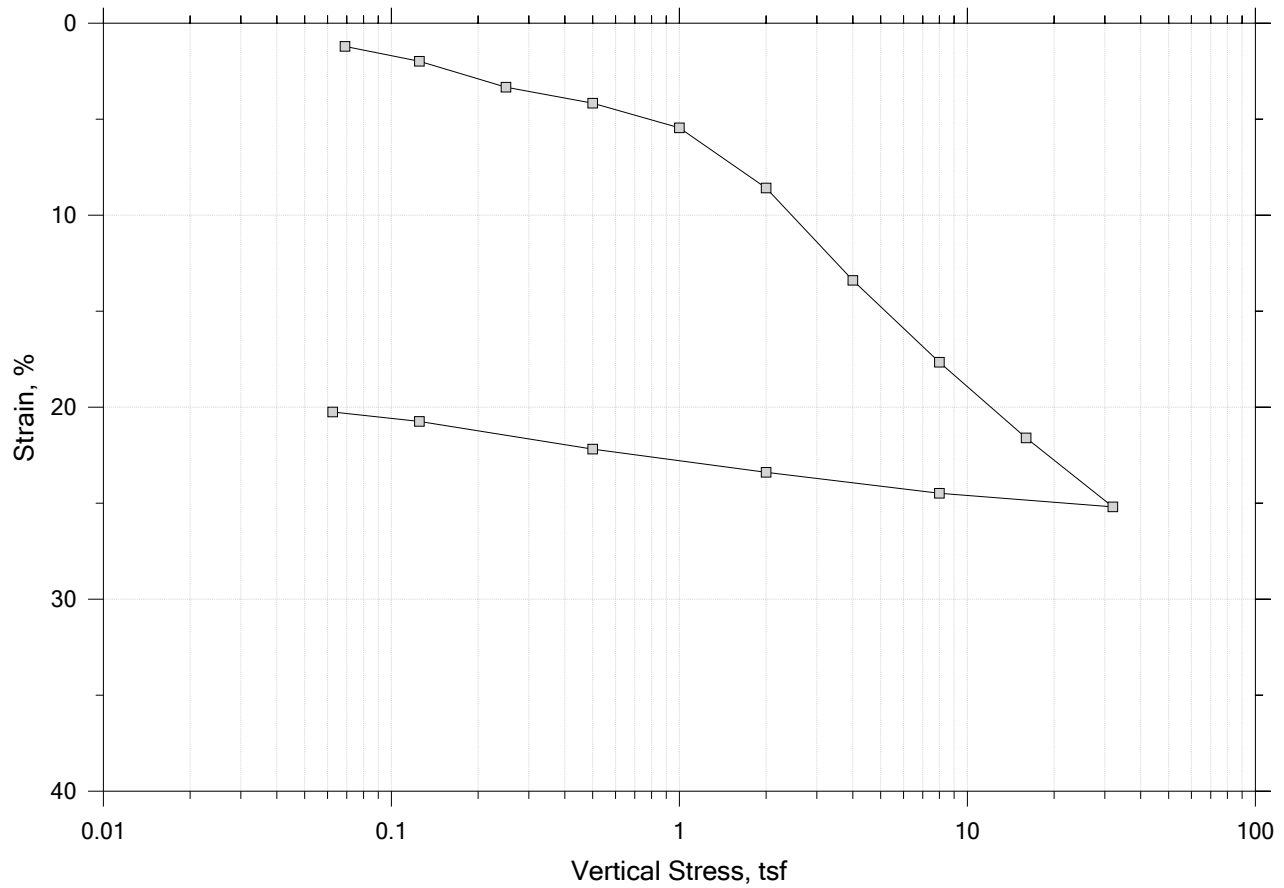
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BFB1-101	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/31/19	Depth: 15-16.4 ft
	Test No.: IP-11B	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0664 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

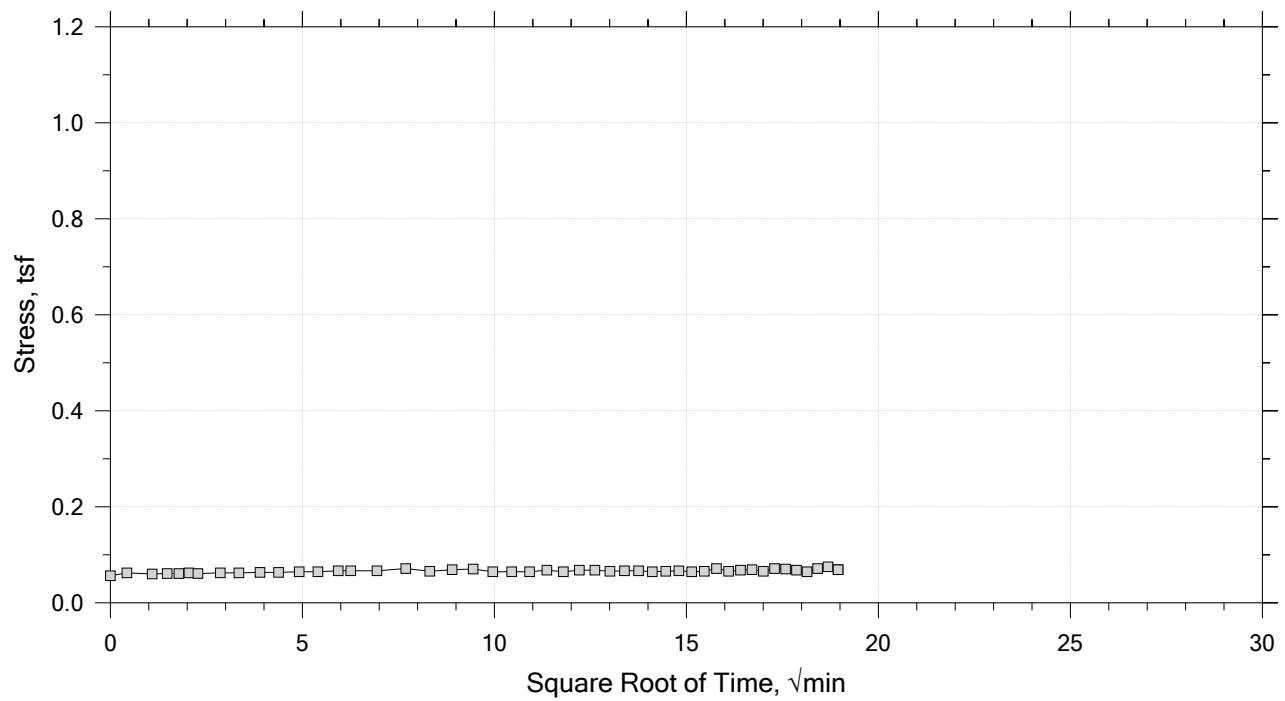
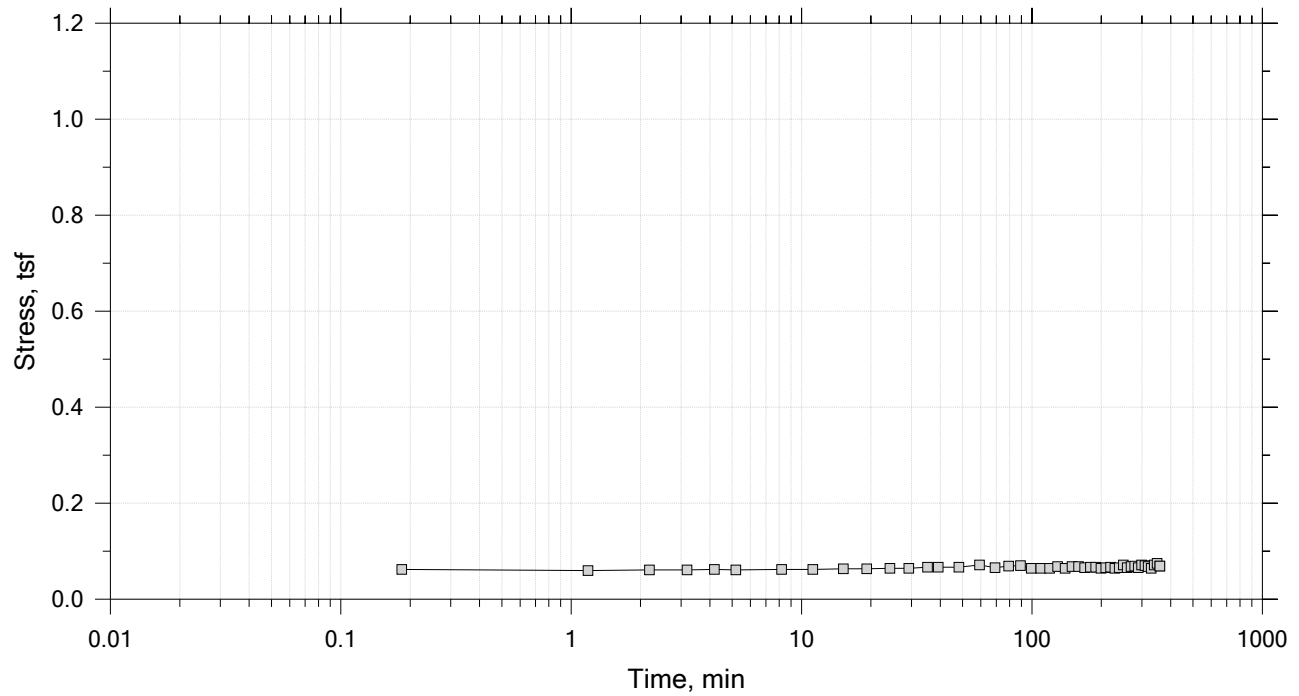
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.069 tsf



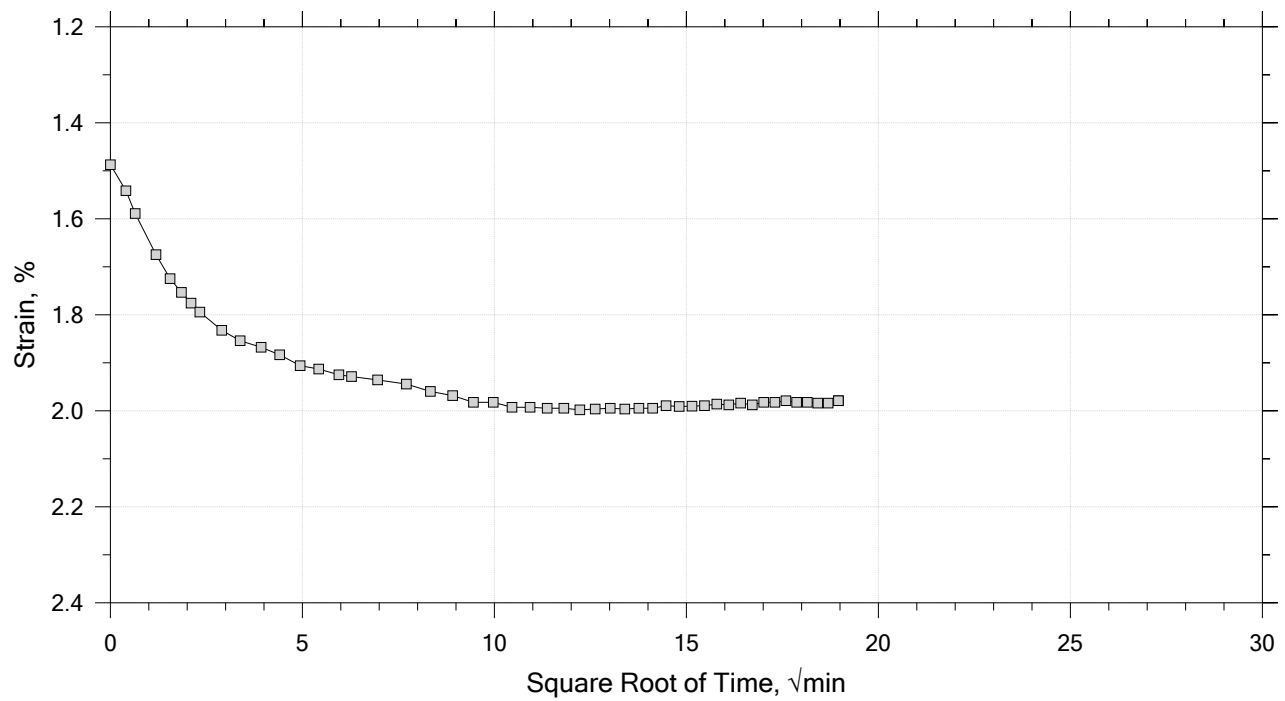
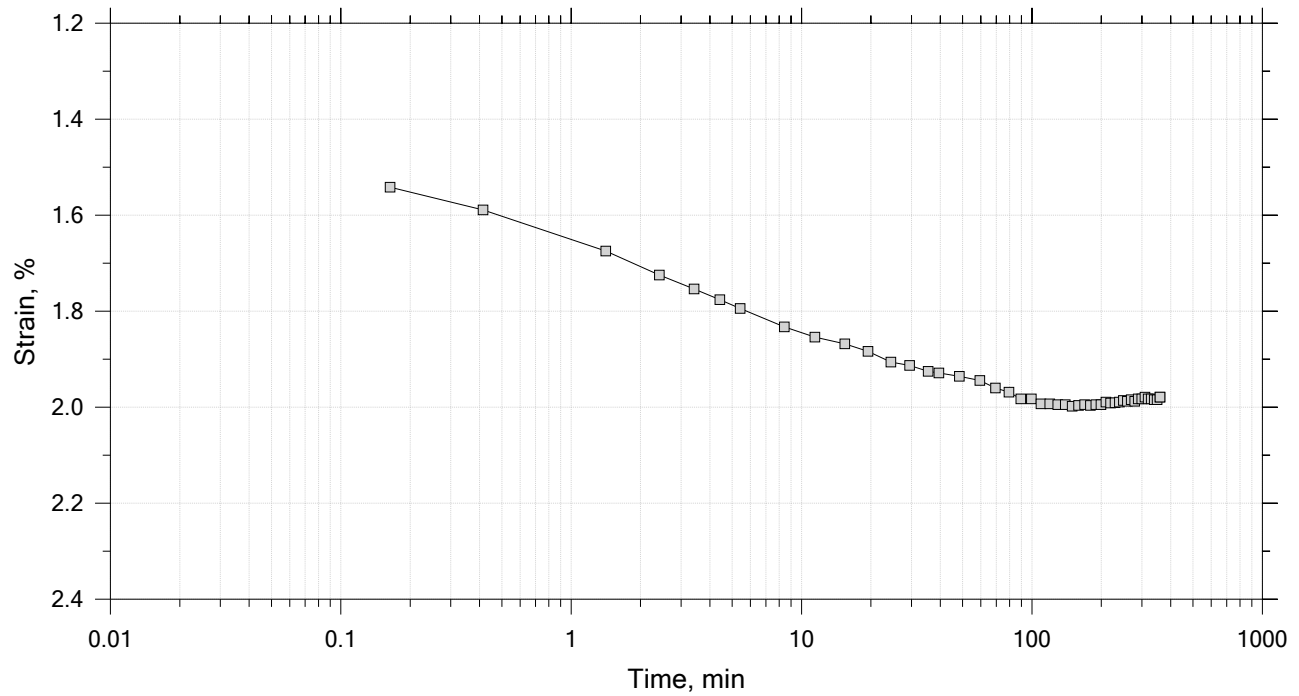
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



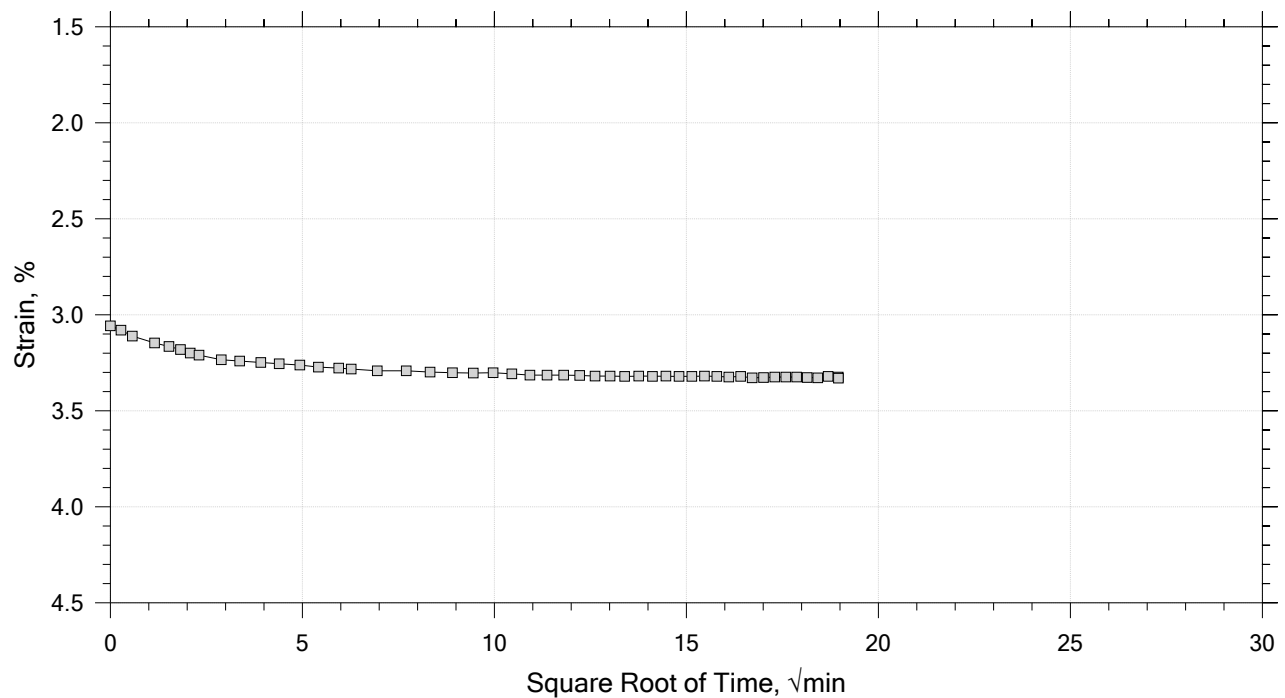
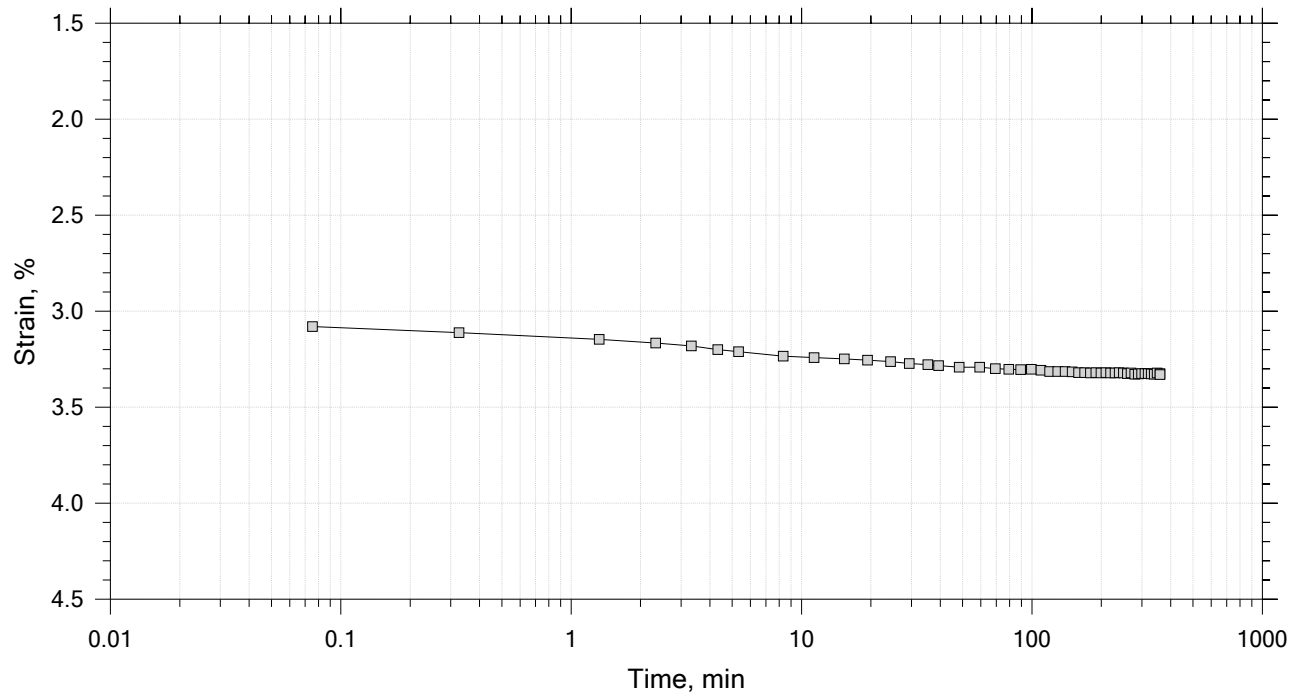
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



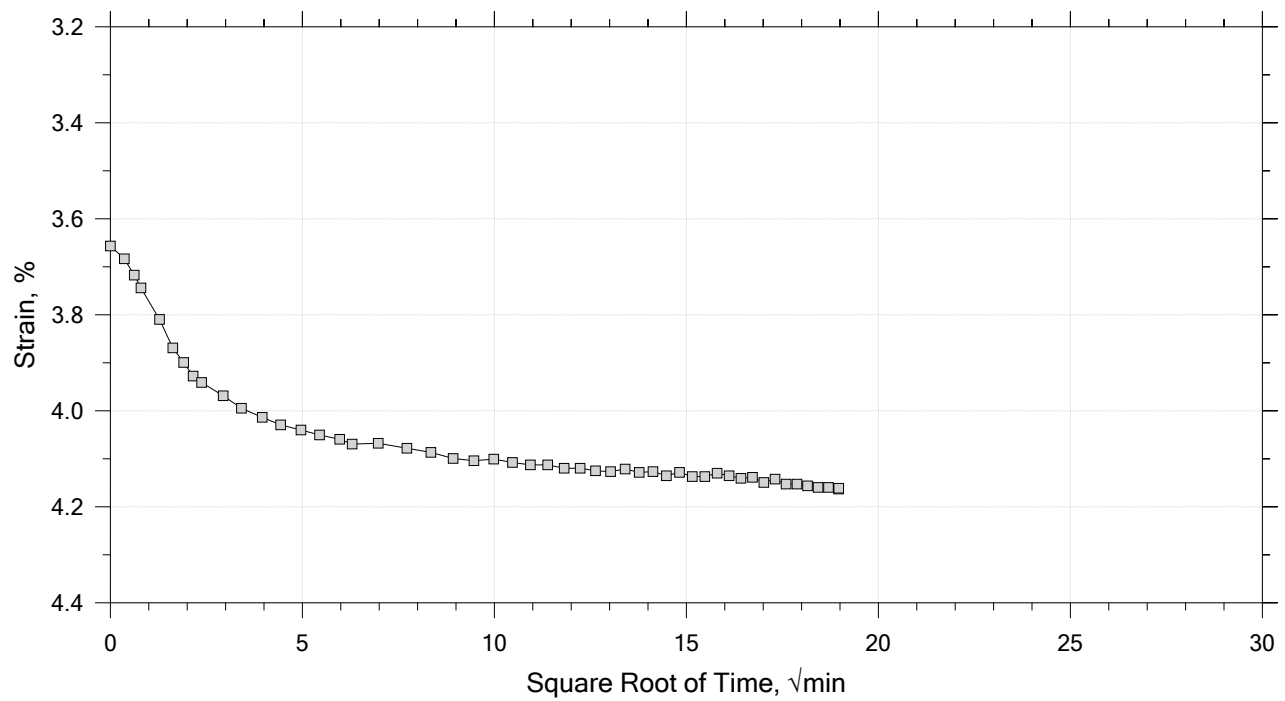
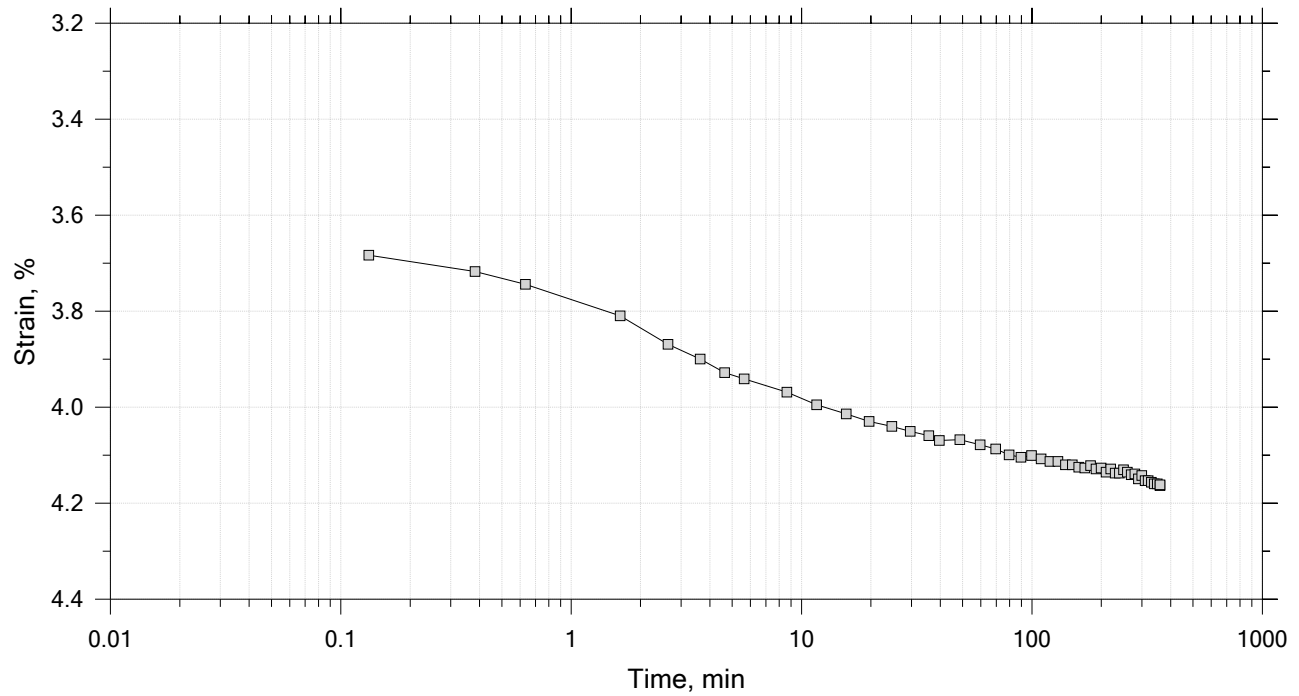
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



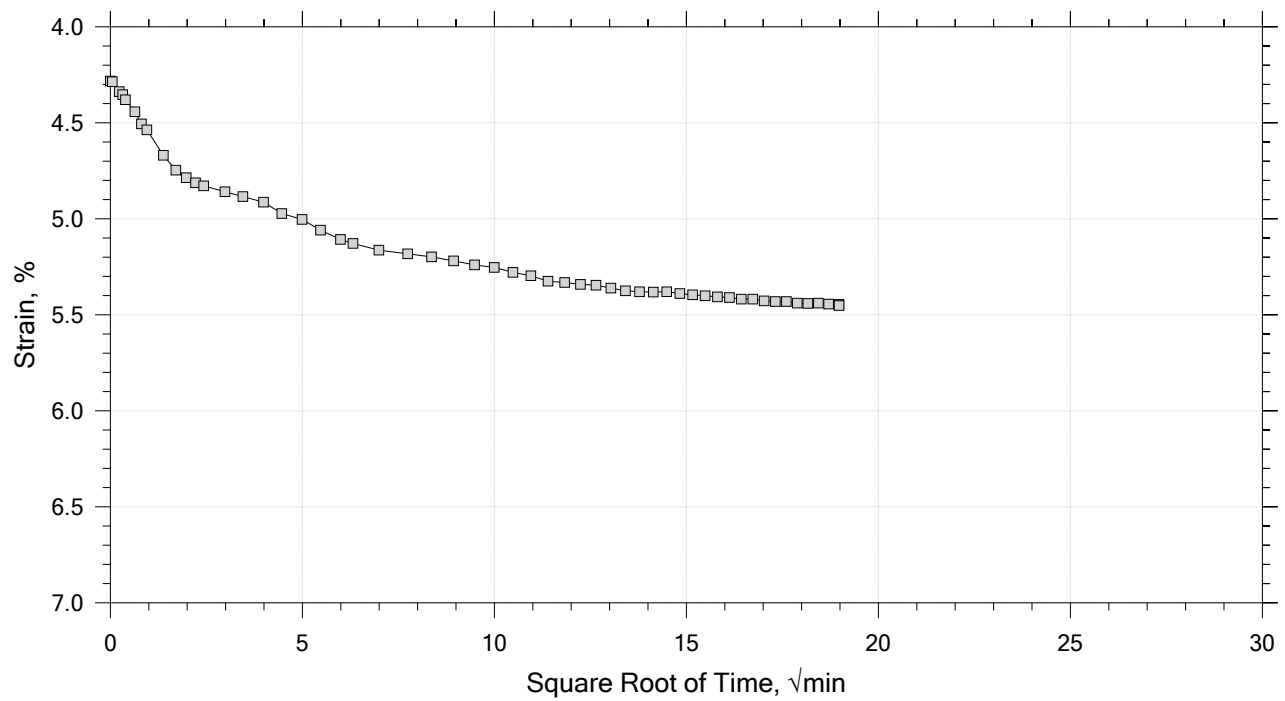
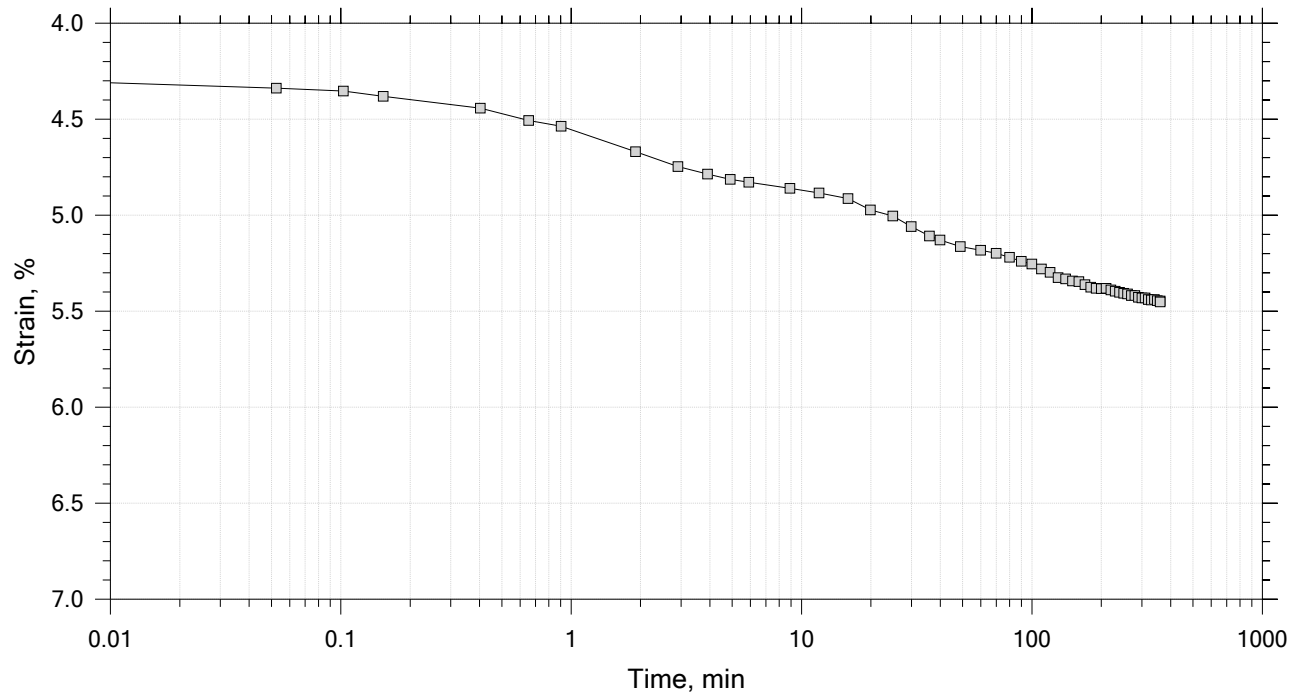
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



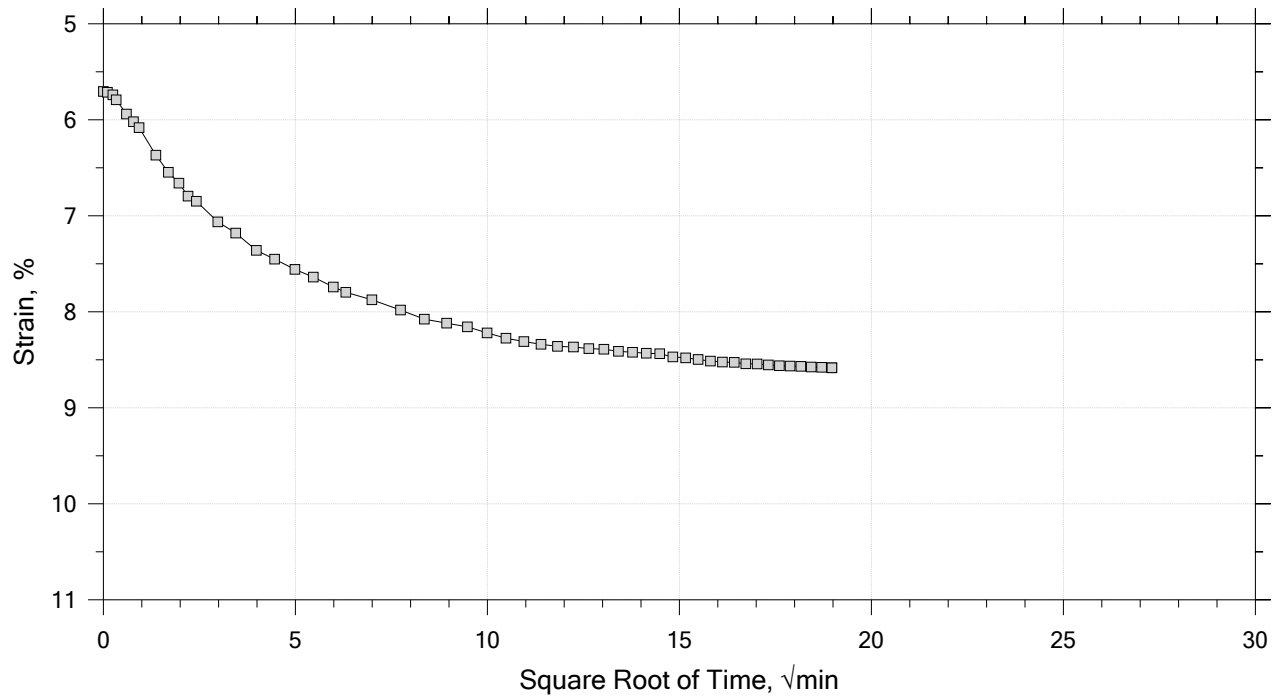
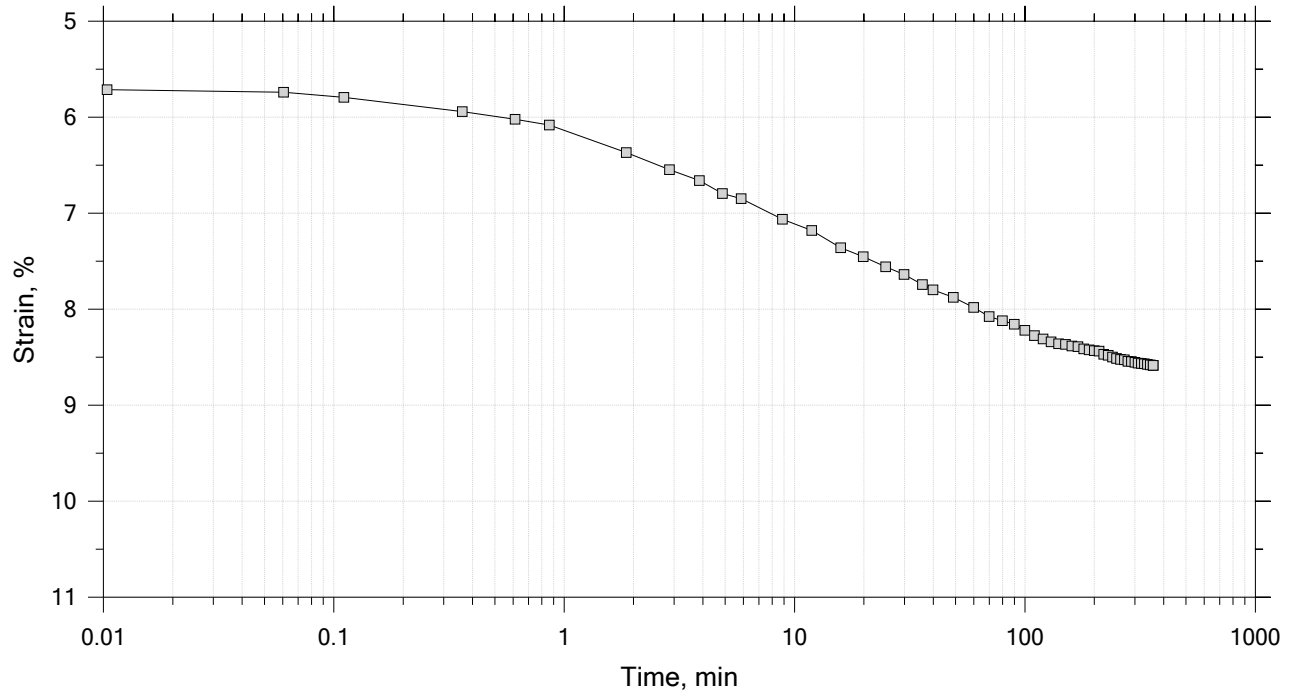
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



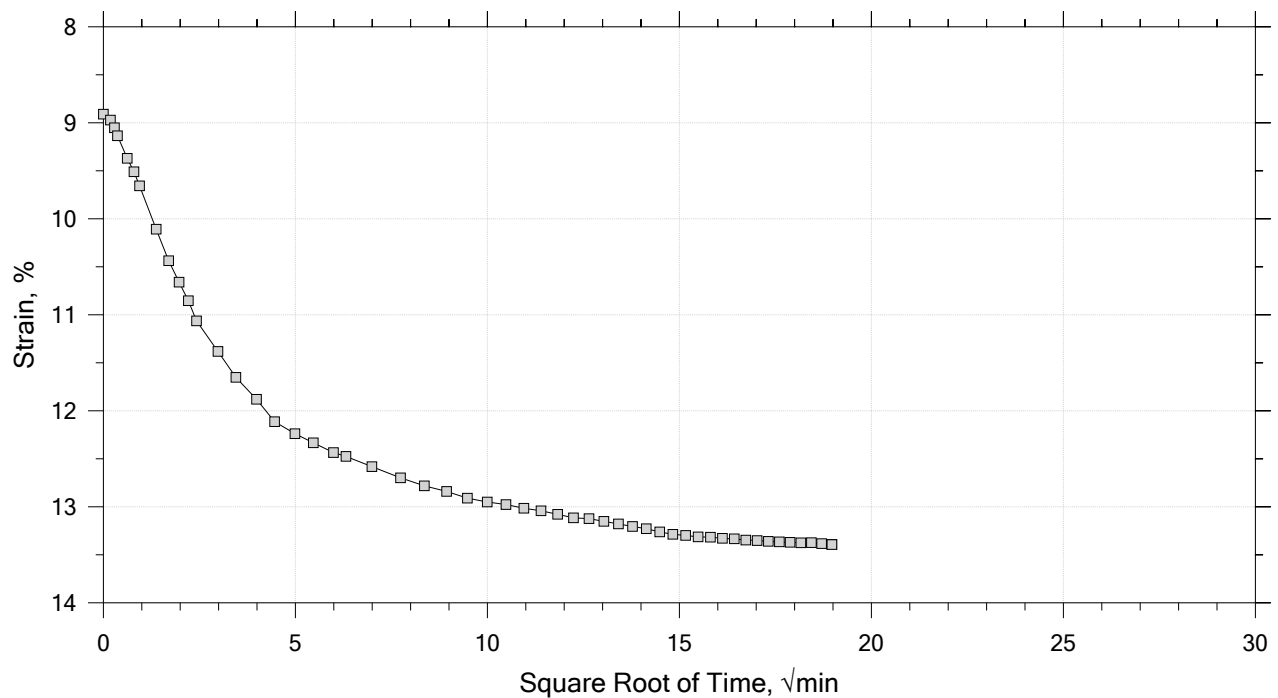
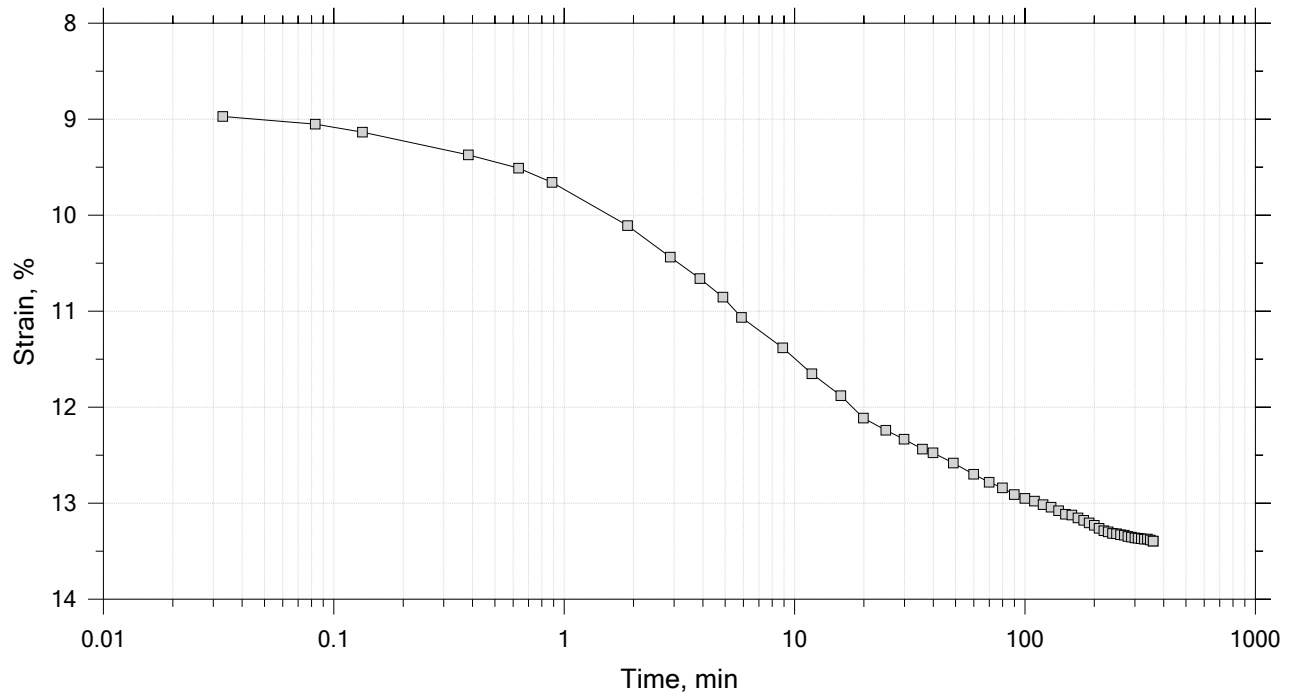
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



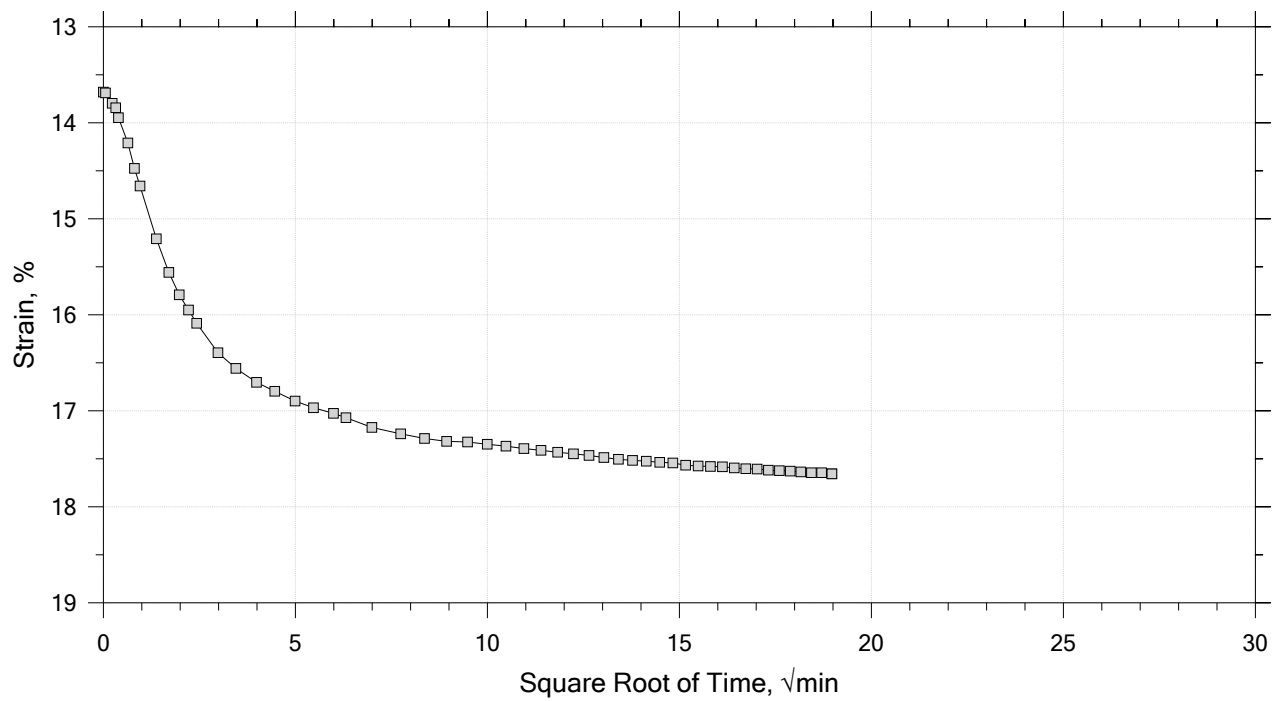
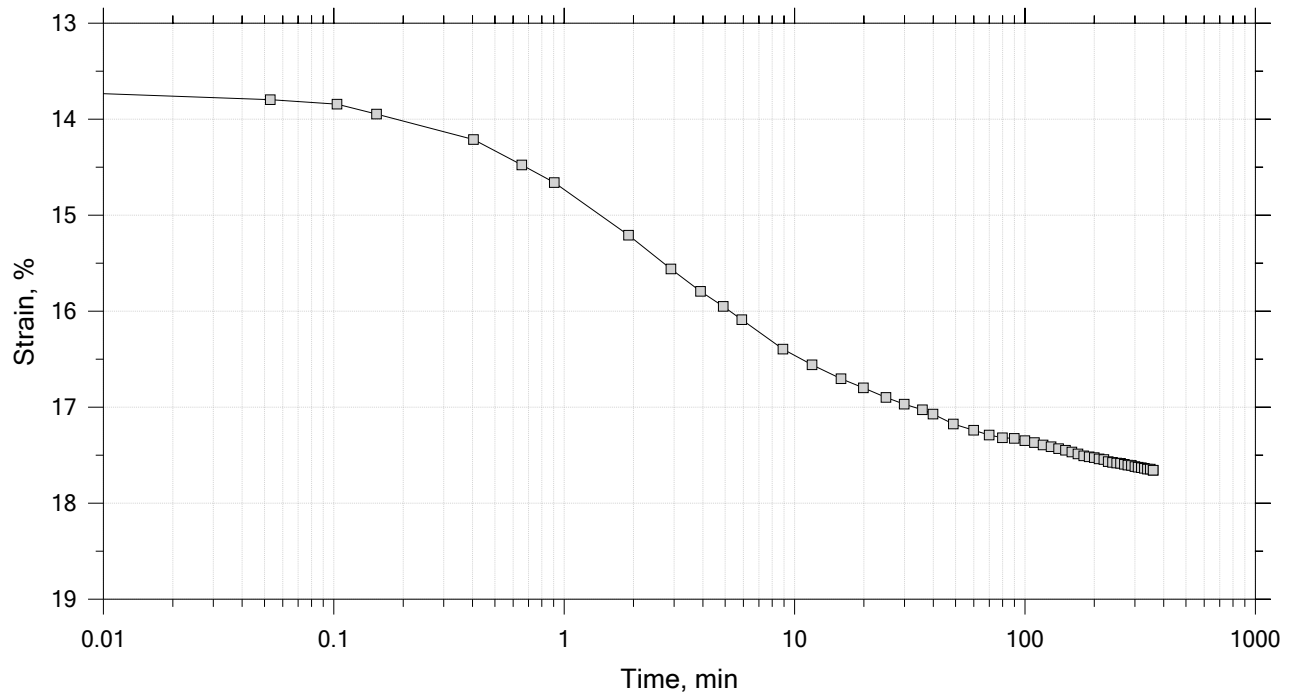
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



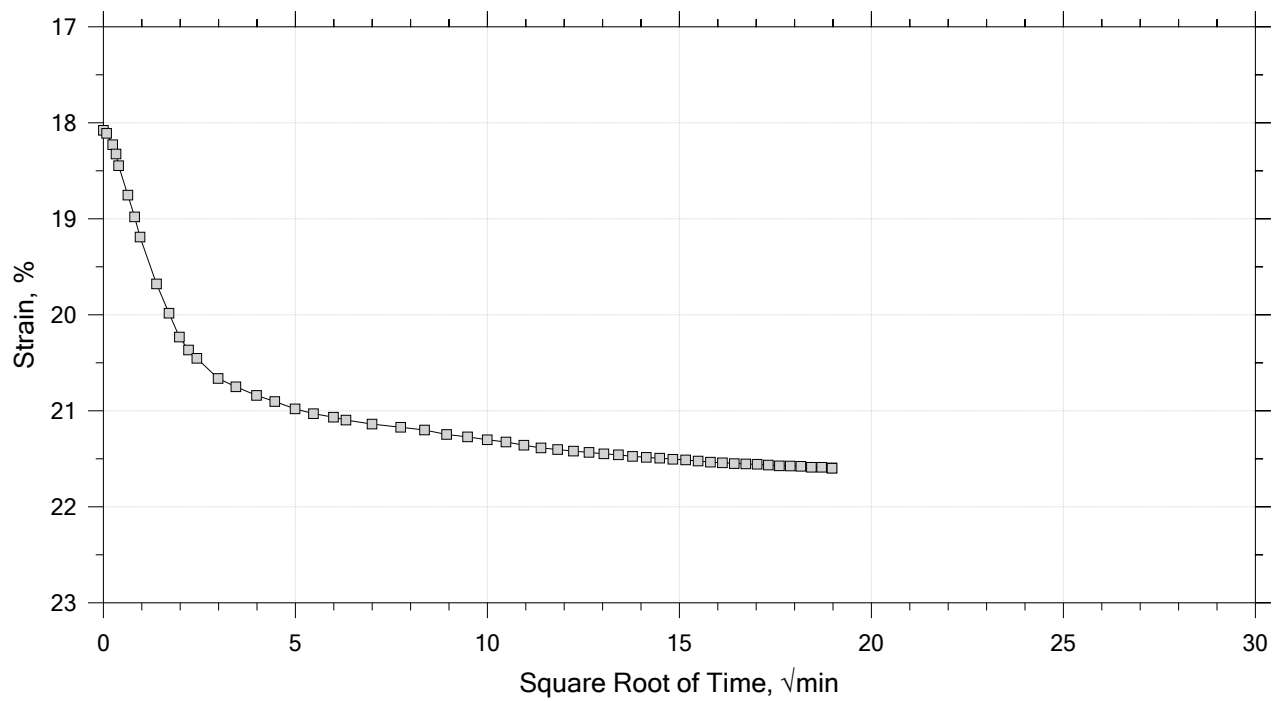
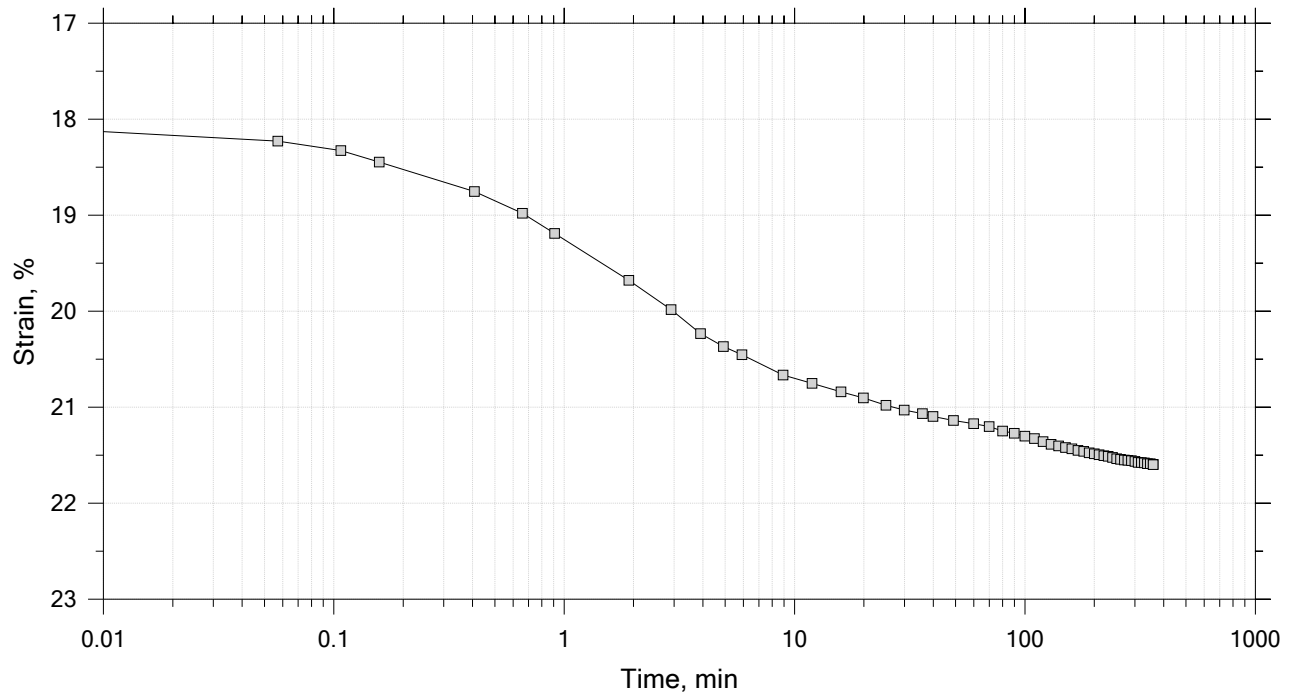
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



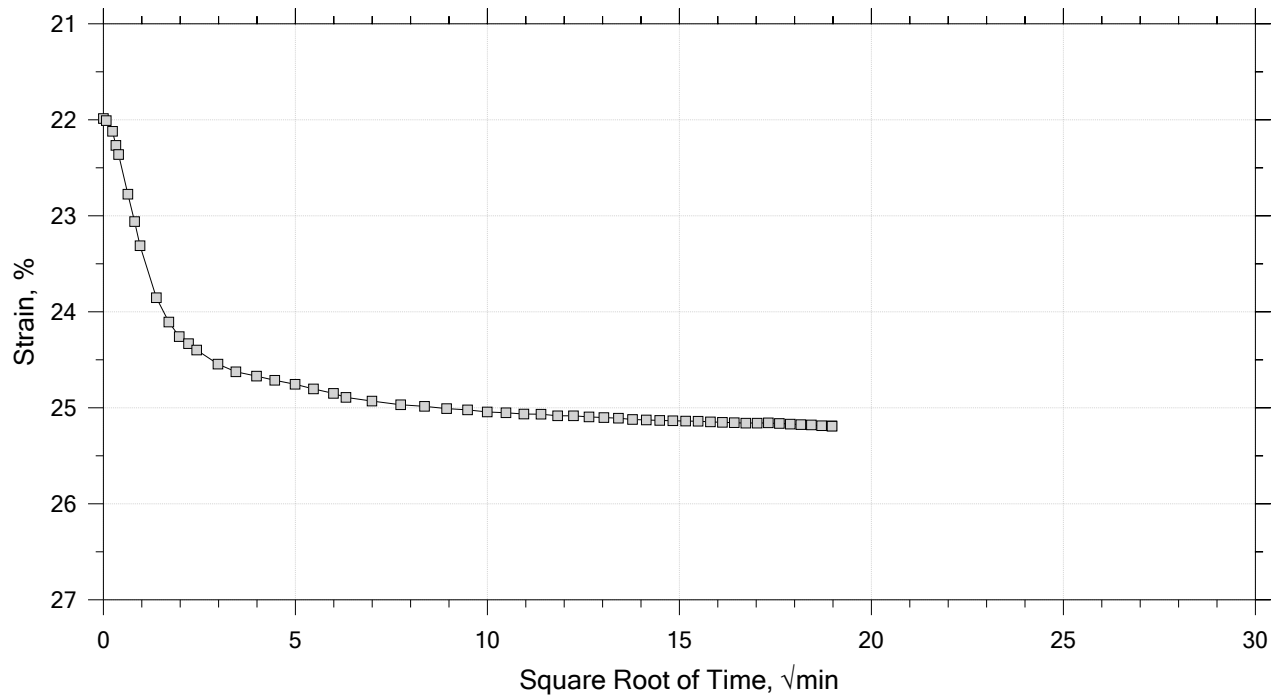
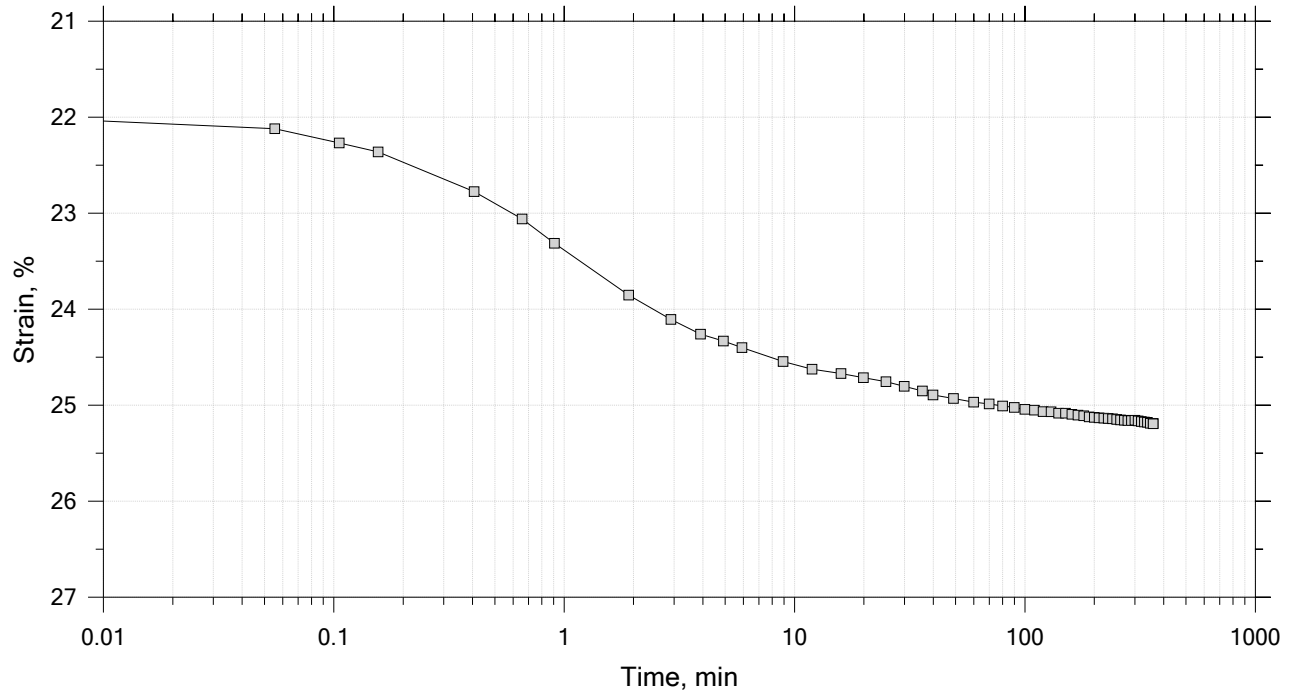
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



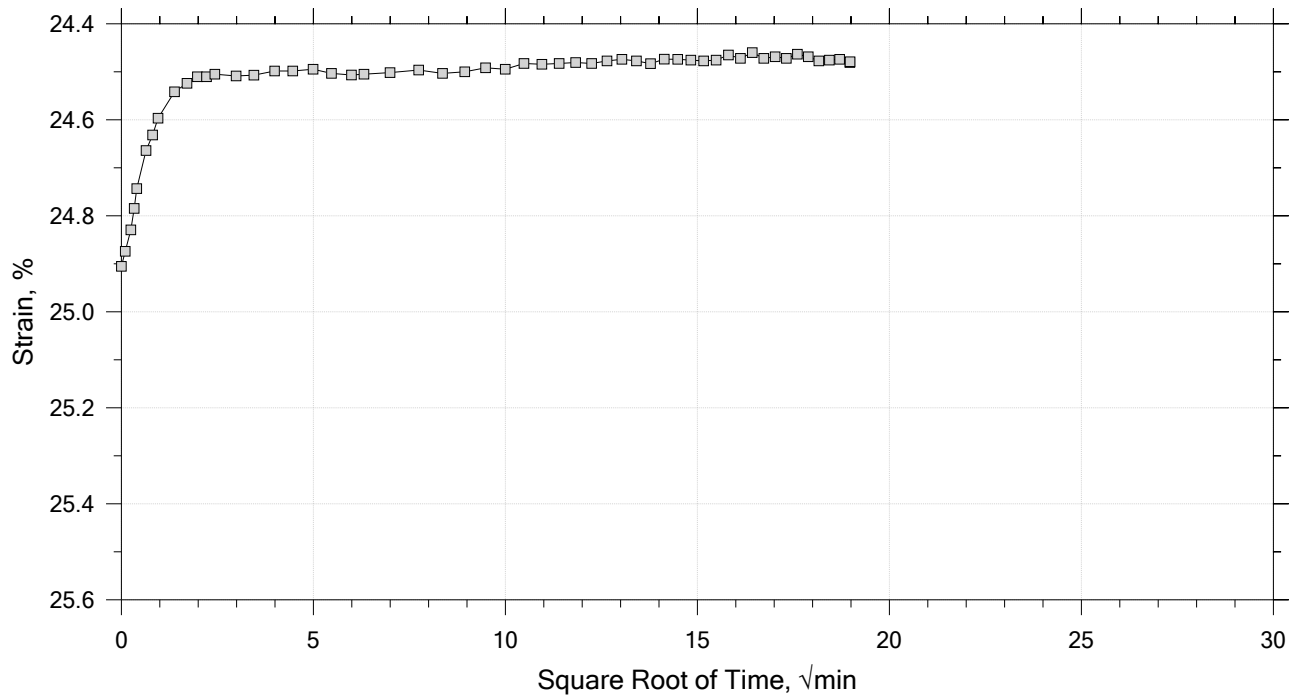
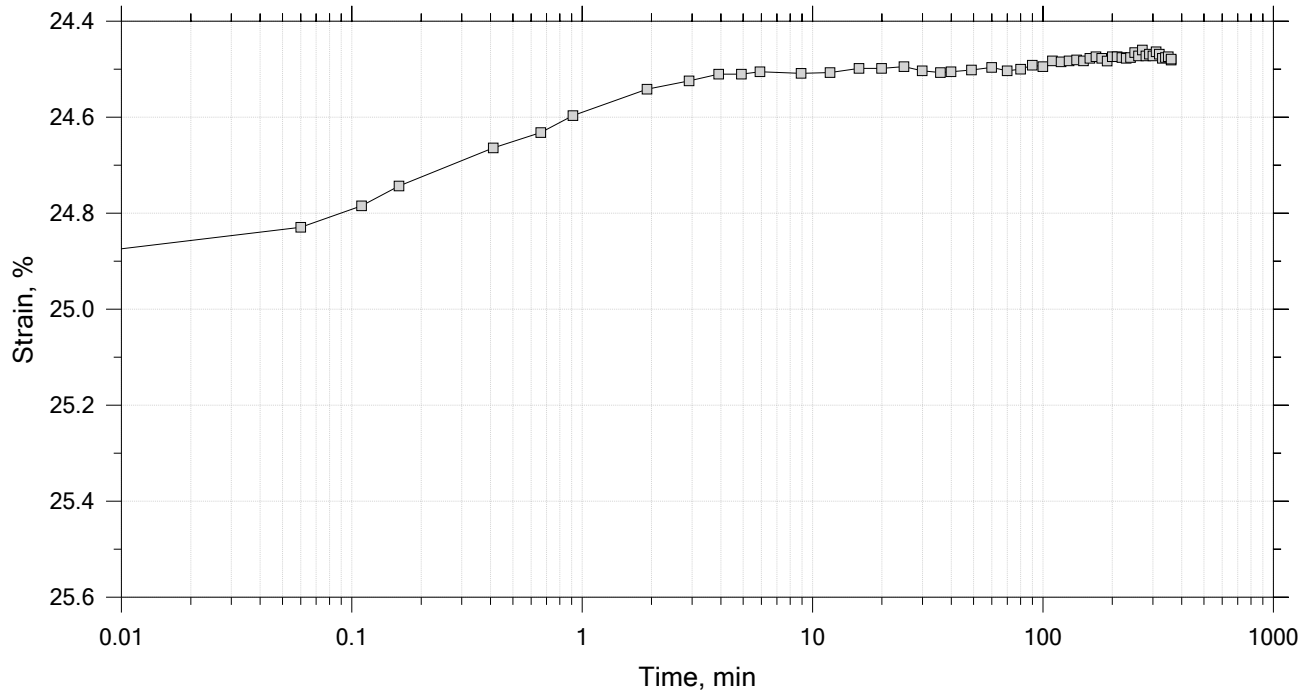
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



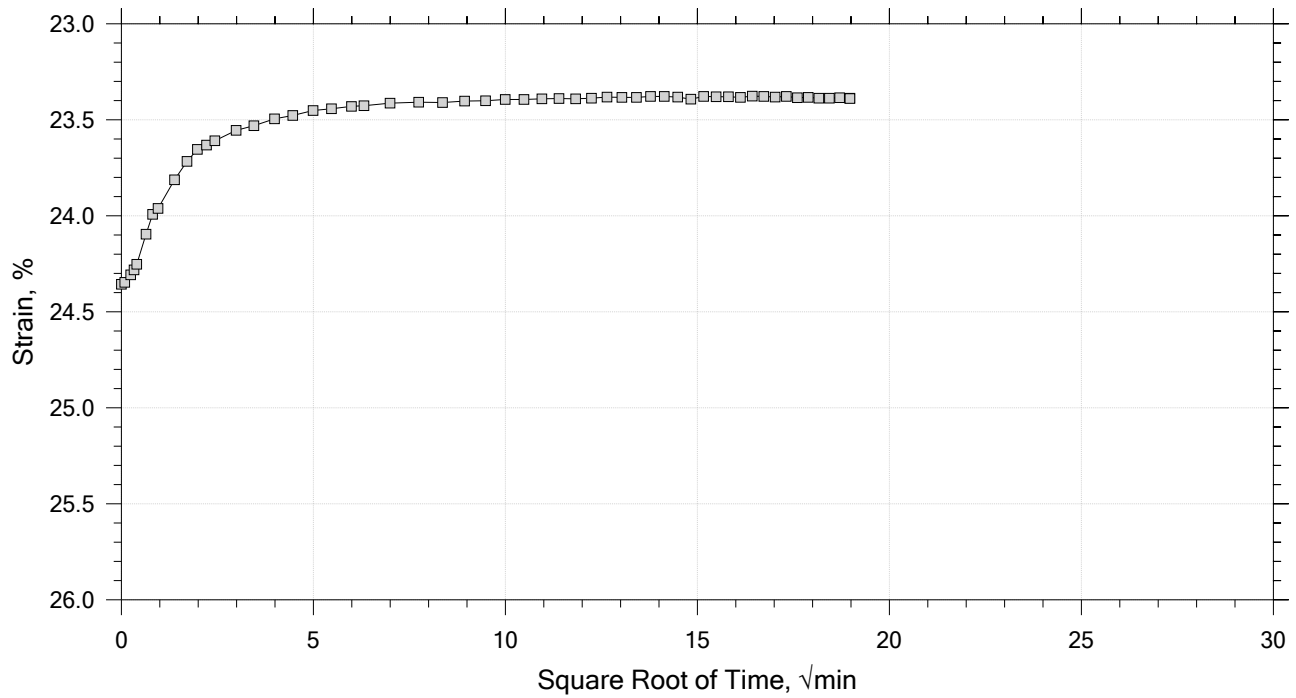
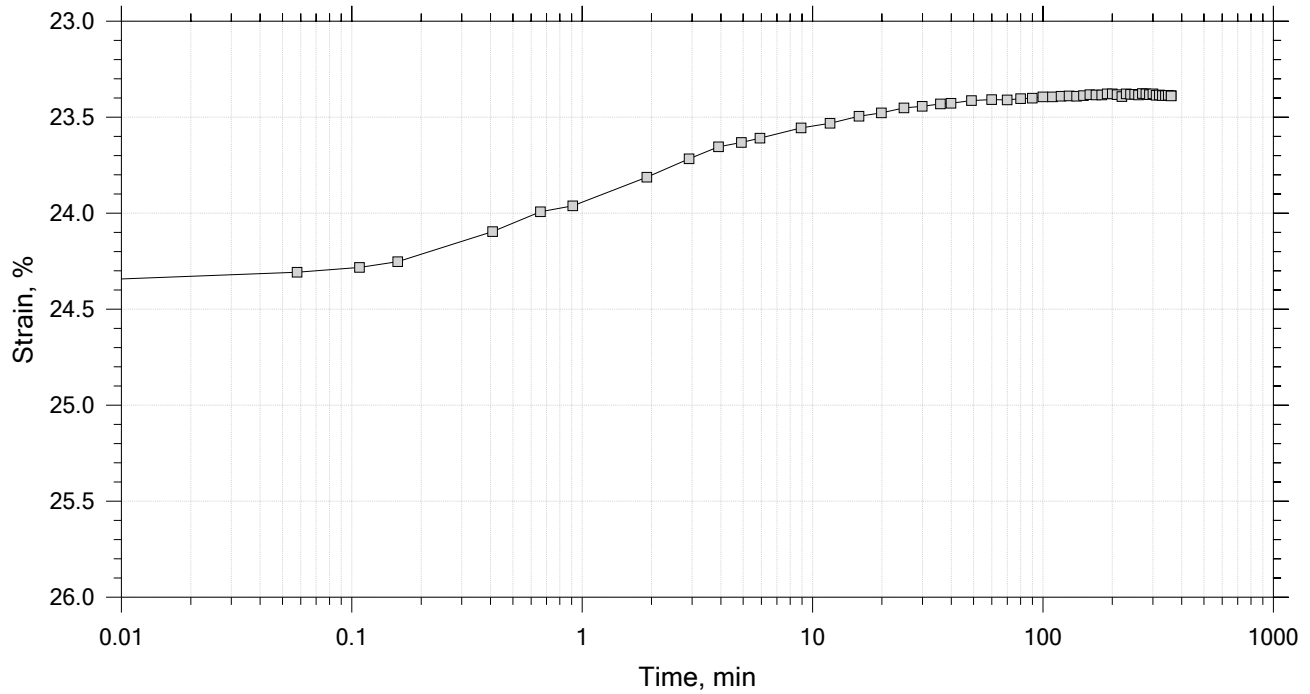
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



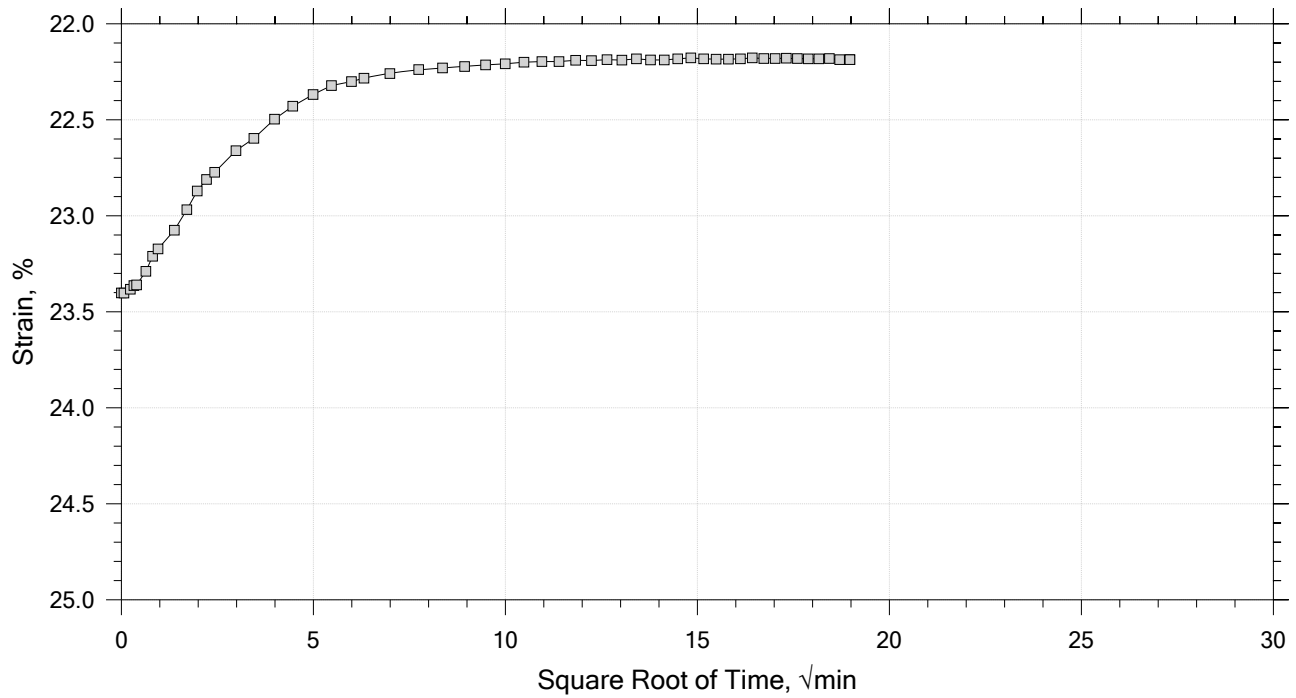
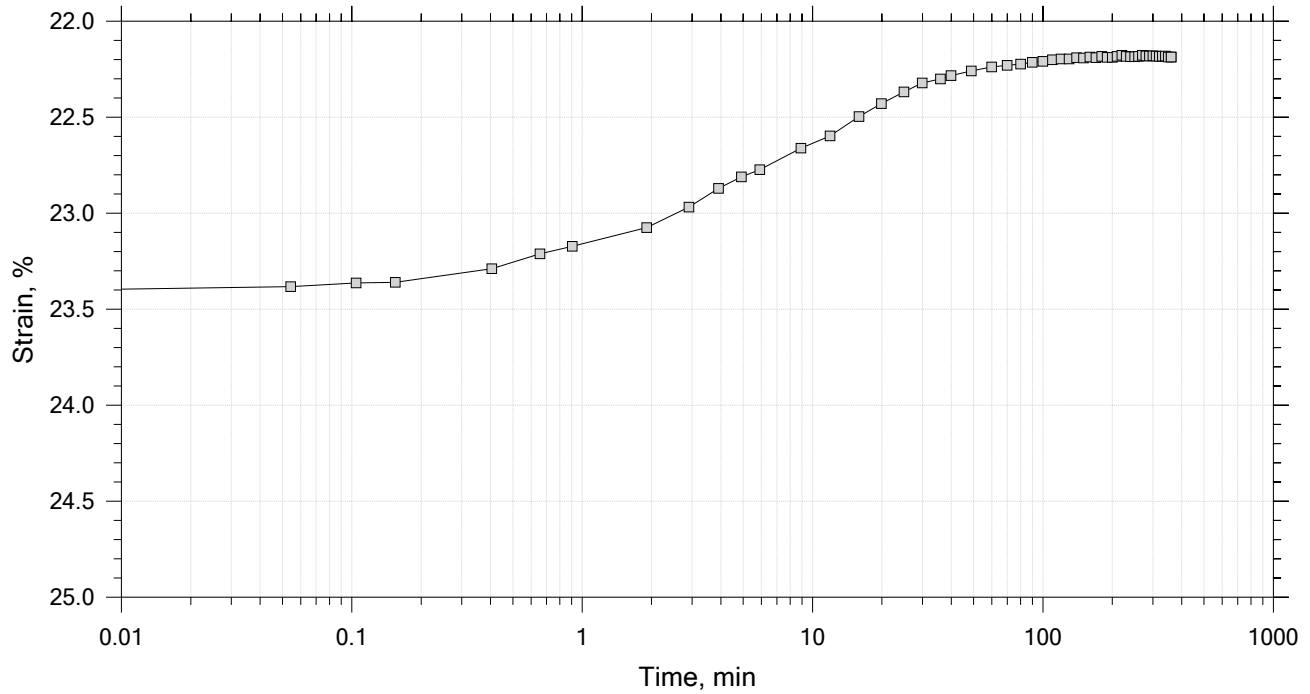
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



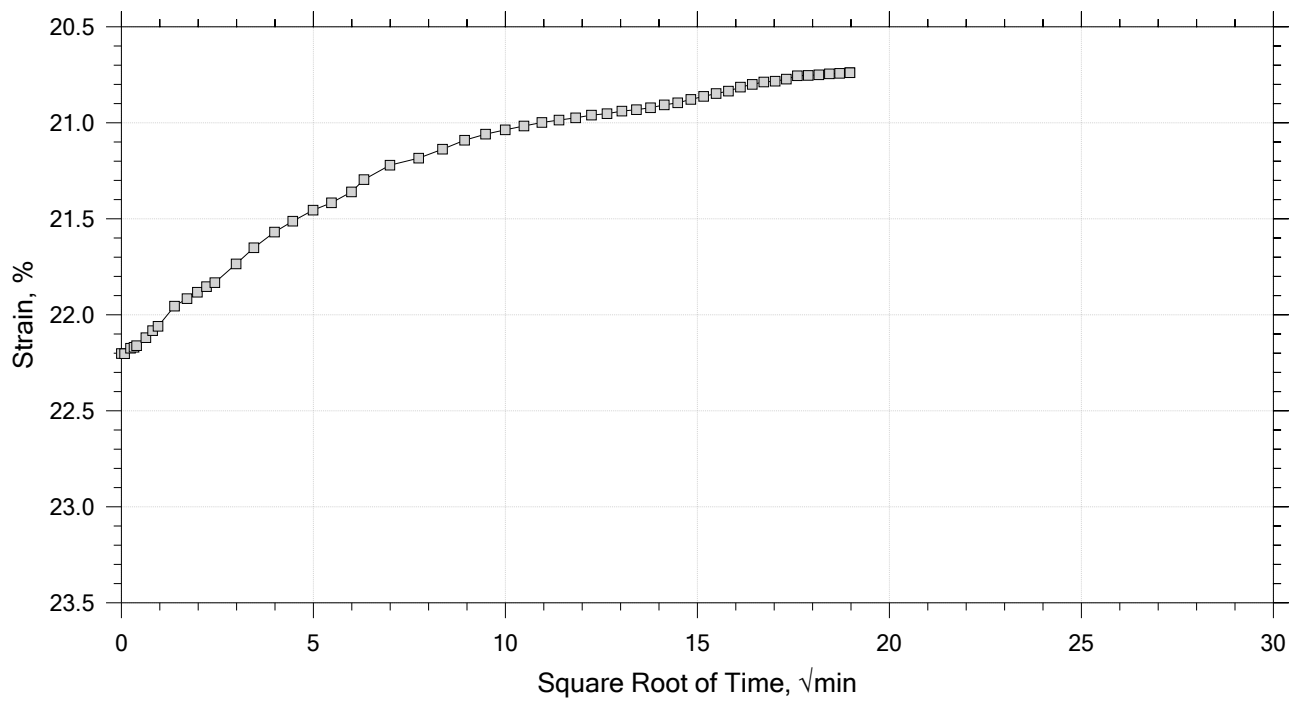
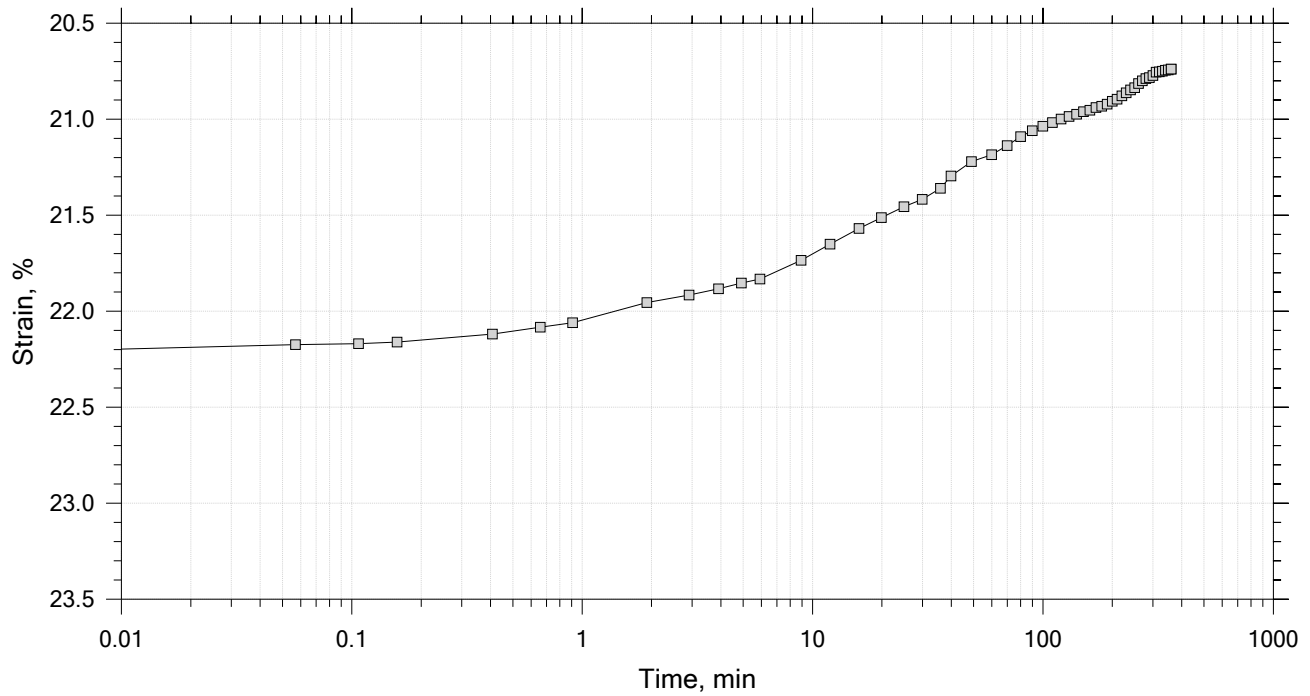
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



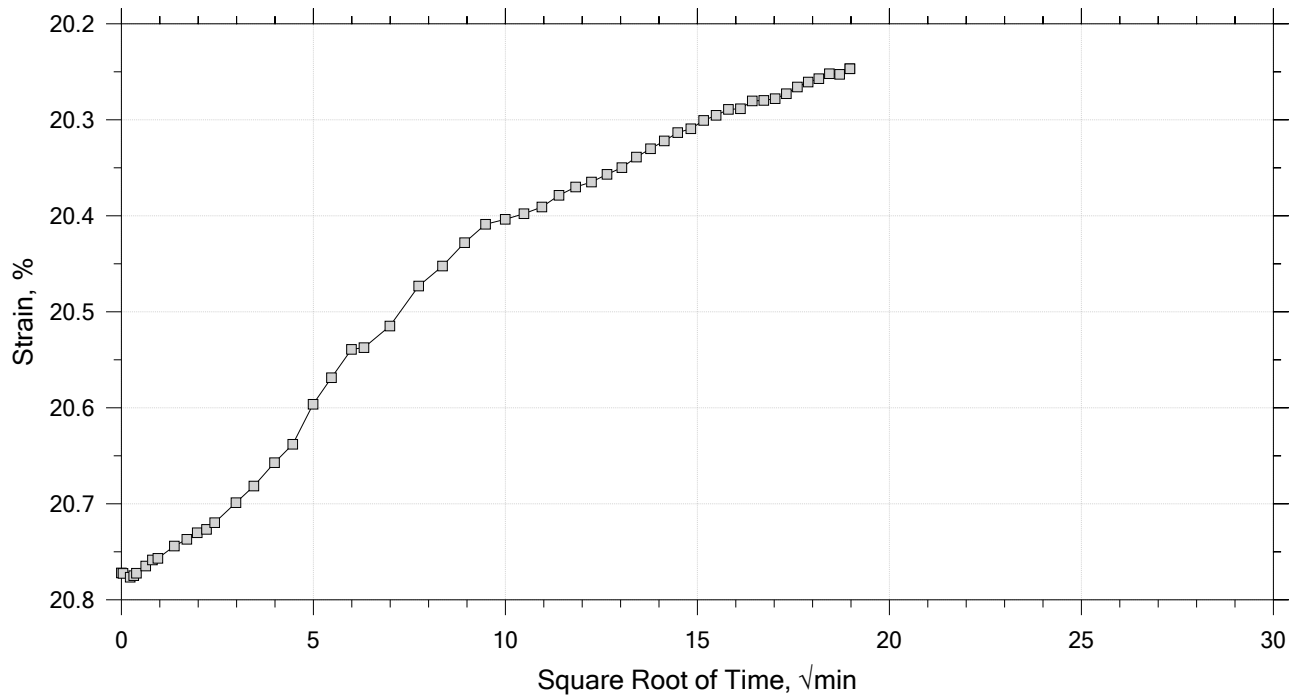
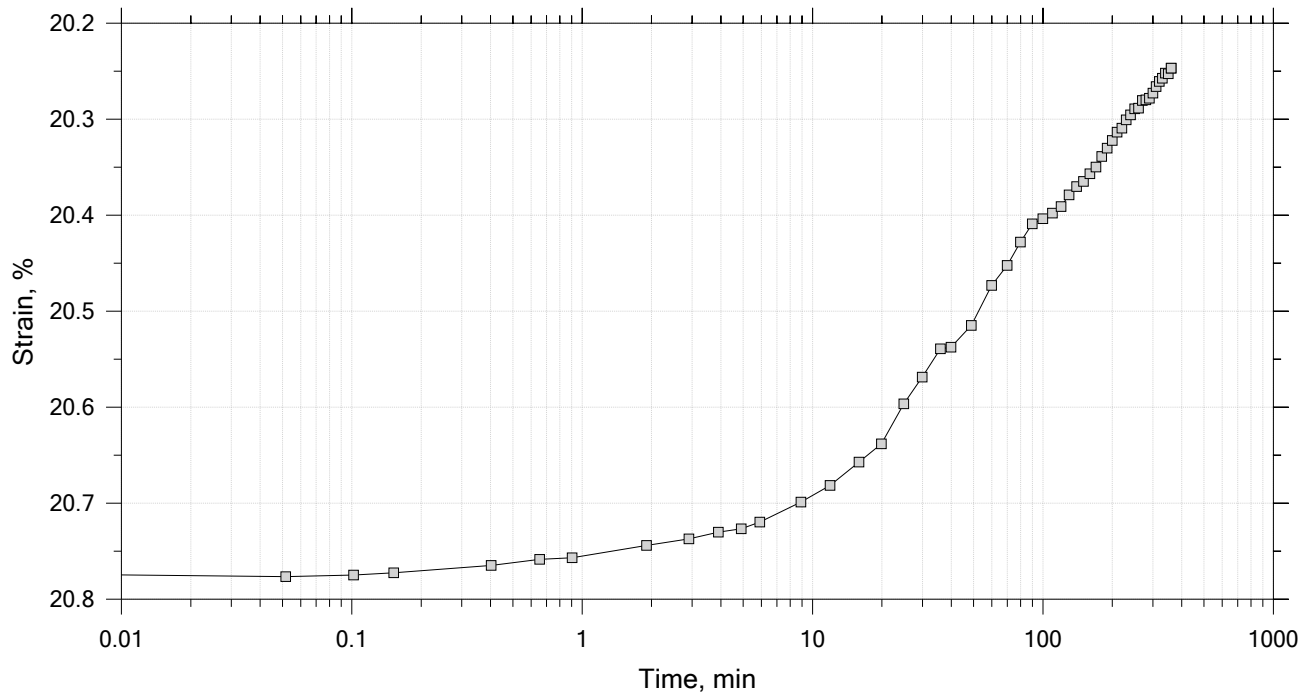
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.76	Liquid Limit: 36
Initial Height: 1.00 in	Initial Void Ratio: 0.943	Plastic Limit: 20
Final Height: 0.80 in	Final Void Ratio: 0.55	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-534	RING		E1334
Mass Container, gm	8.47	110.03	110.03	8.48
Mass Container + Wet Soil, gm	122.26	260.8	247.25	145.43
Mass Container + Dry Soil, gm	95.63	224.5	224.5	122.72
Mass Dry Soil, gm	87.16	114.47	114.47	114.24
Water Content, %	30.55	31.72	19.88	19.88
Void Ratio	---	0.94	0.55	---
Degree of Saturation, %	---	92.99	100.00	---
Dry Unit Weight, pcf	---	88.834	111.39	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

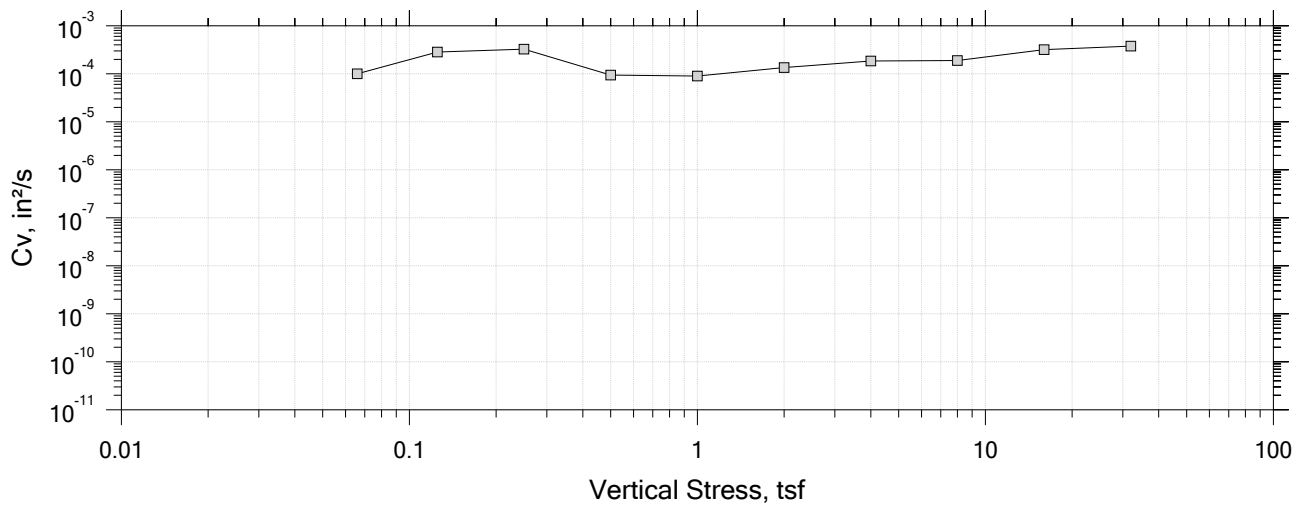
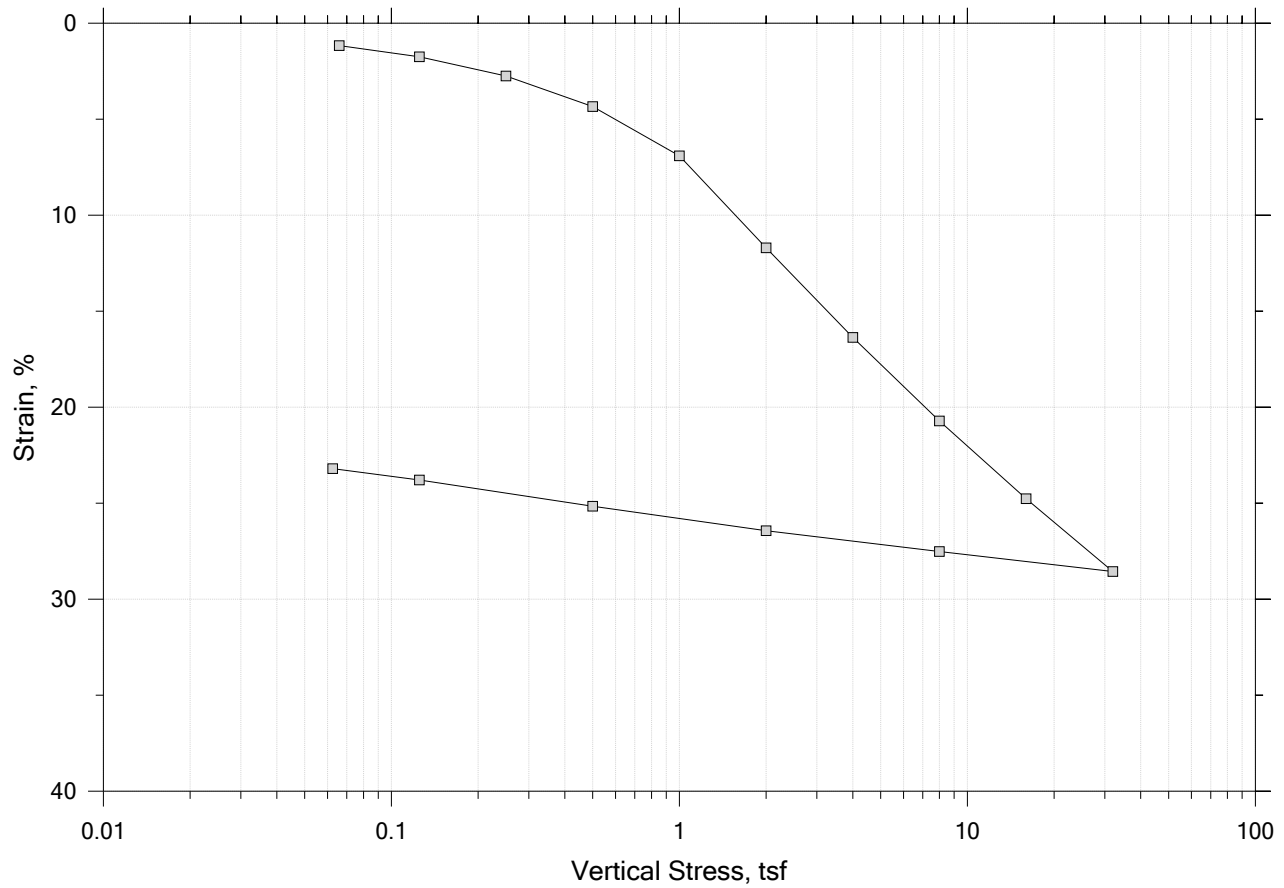
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-2	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-F, Swell Pressure = 0.069 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

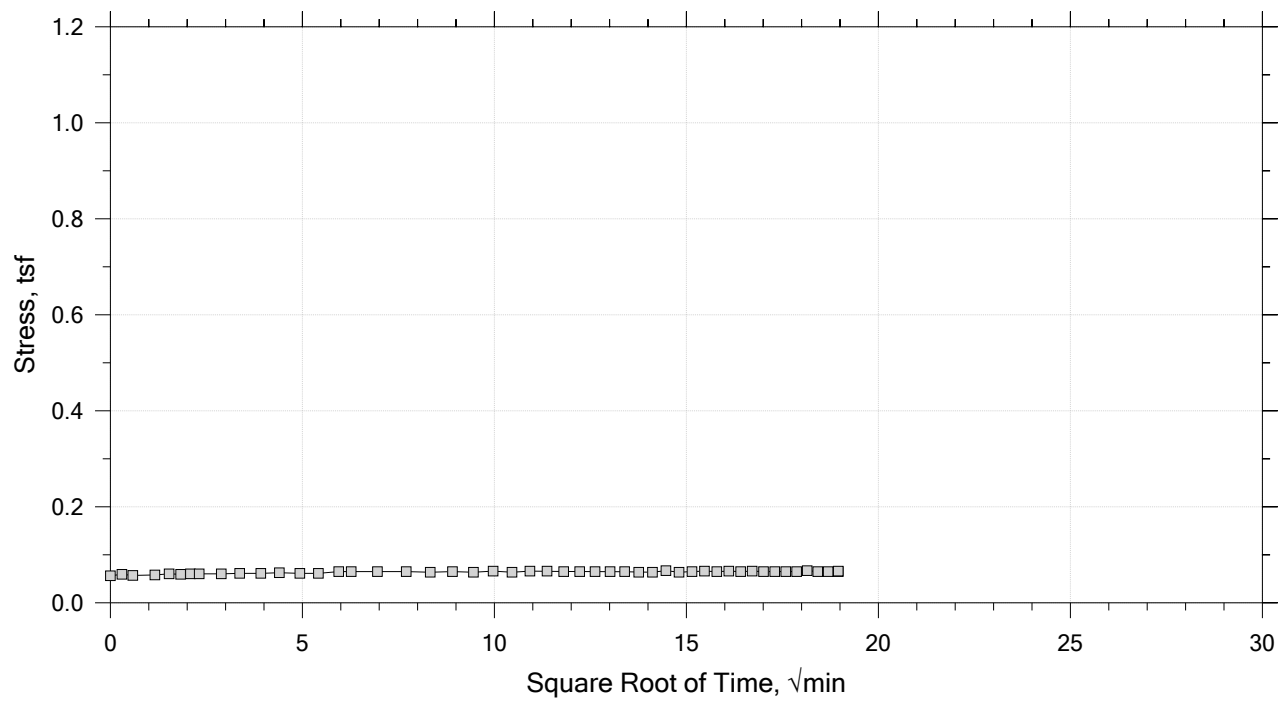
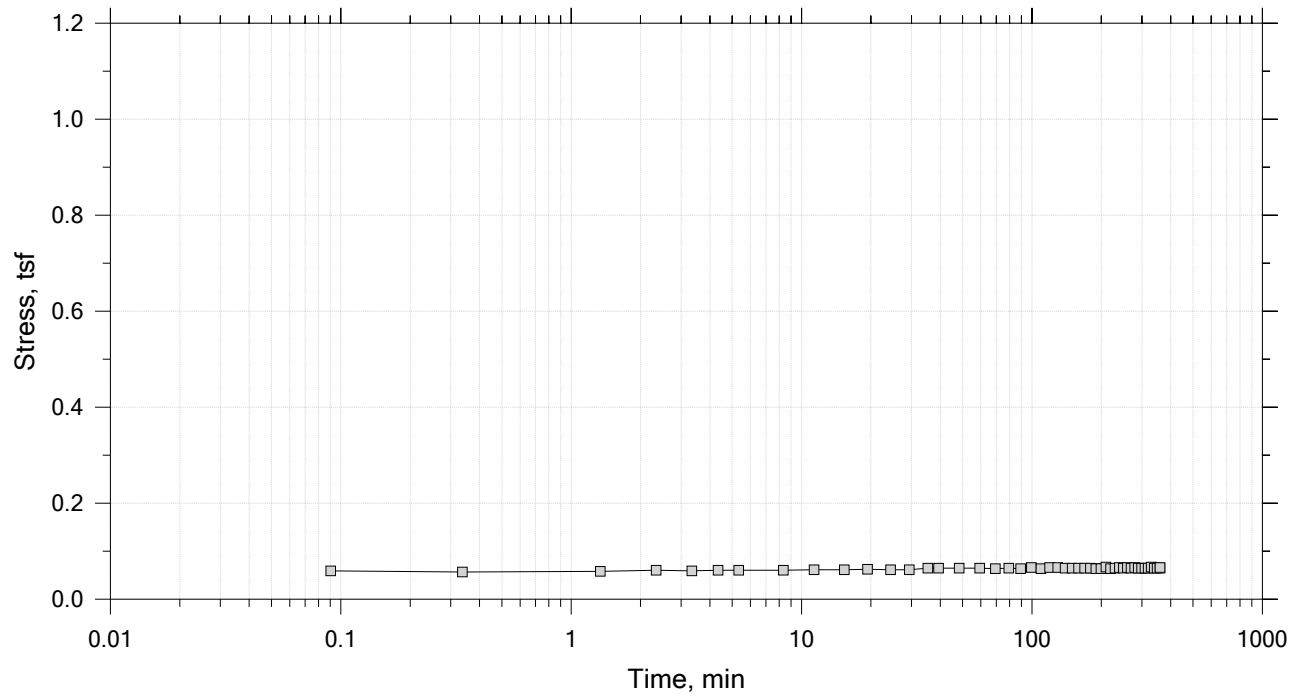
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0659 tsf



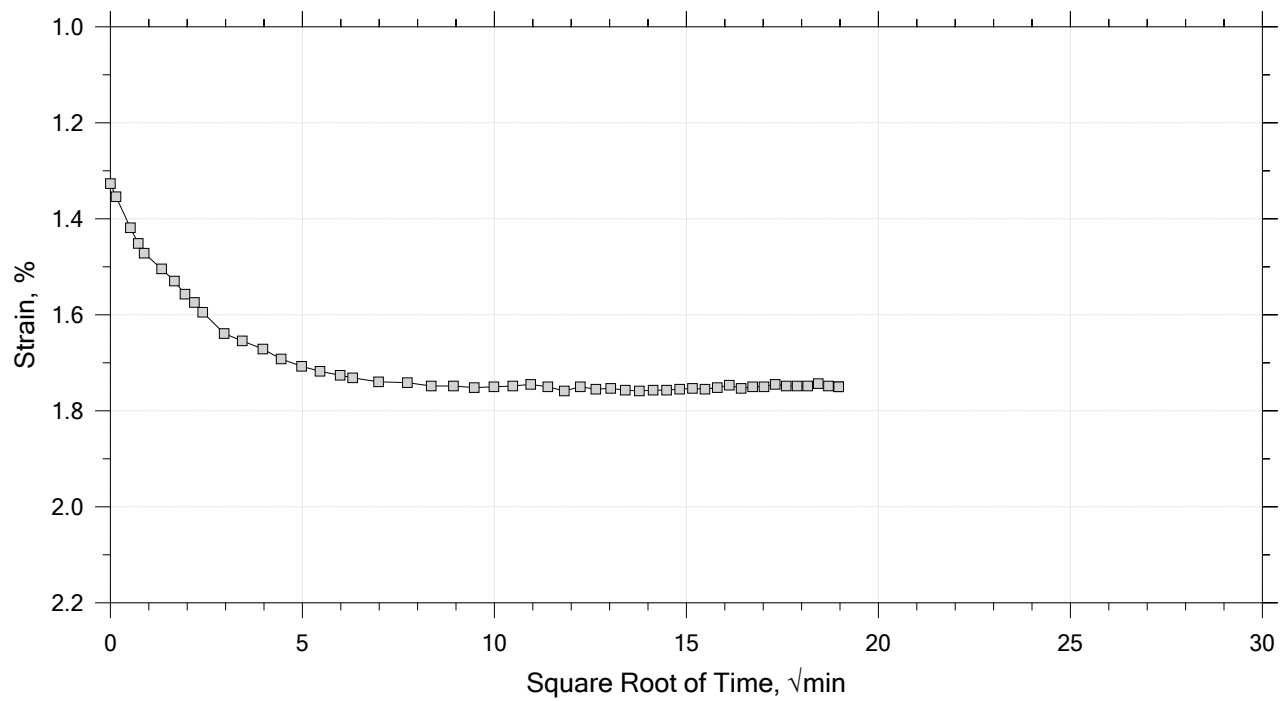
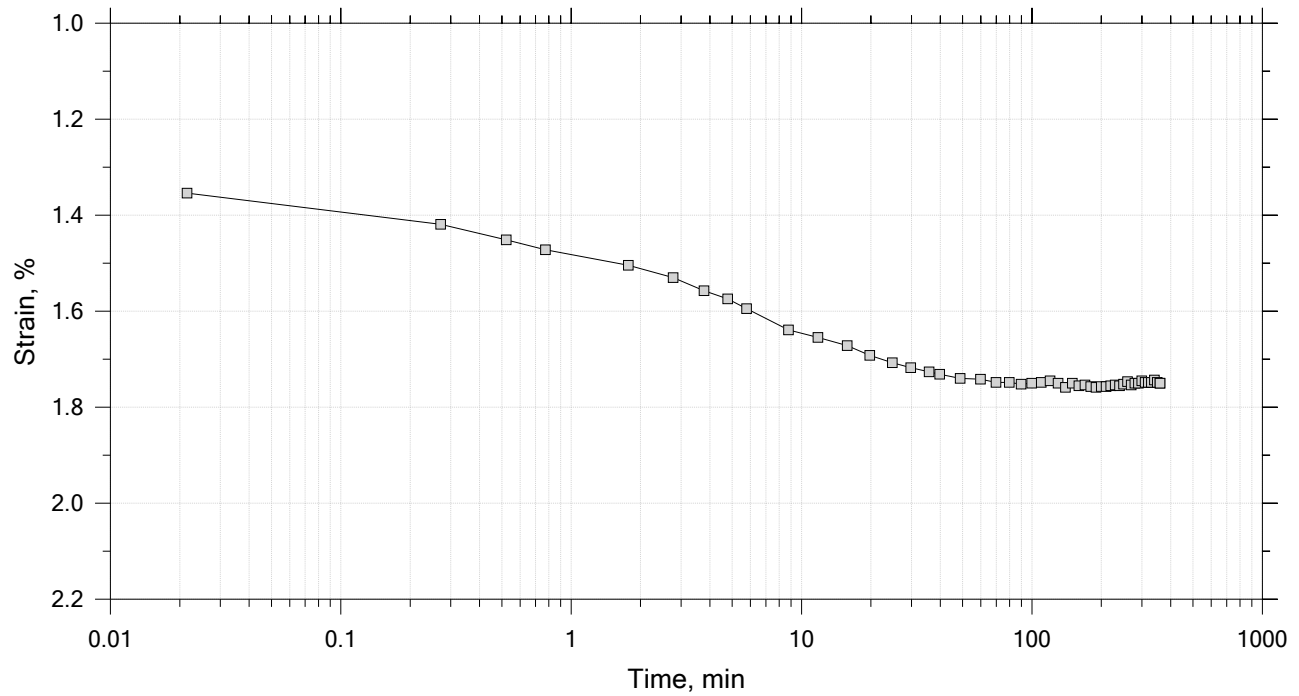
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



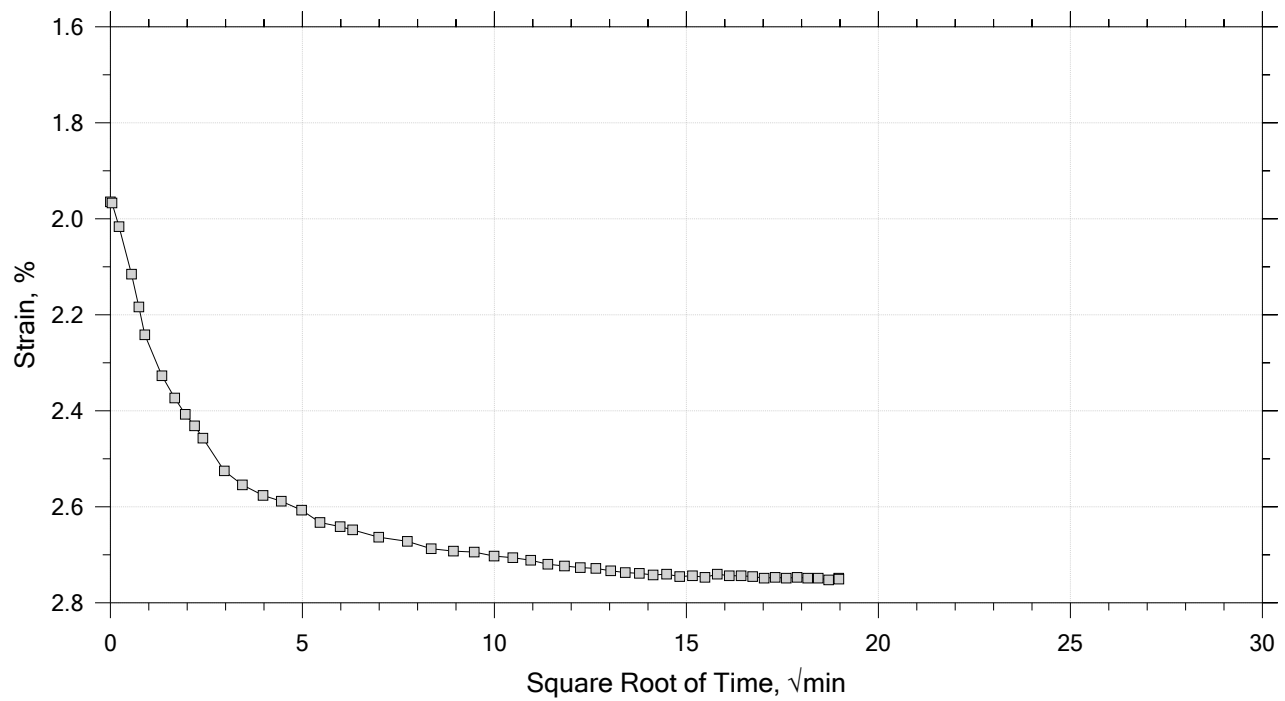
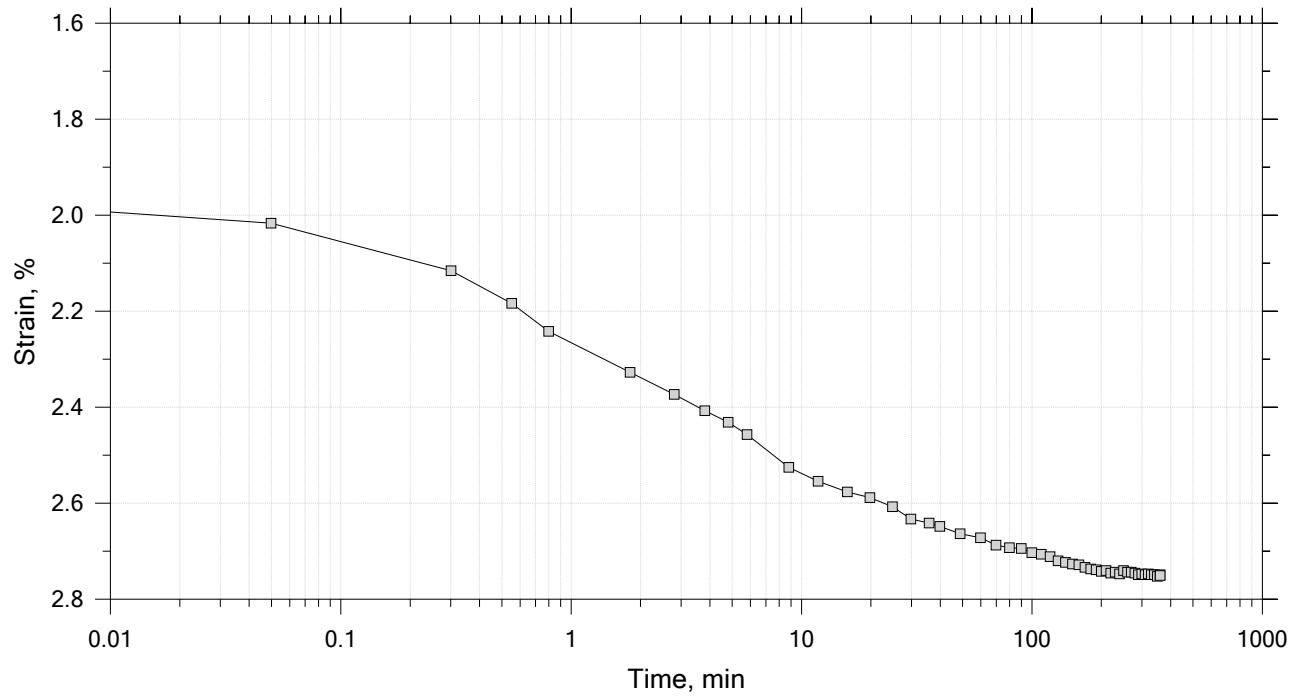
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



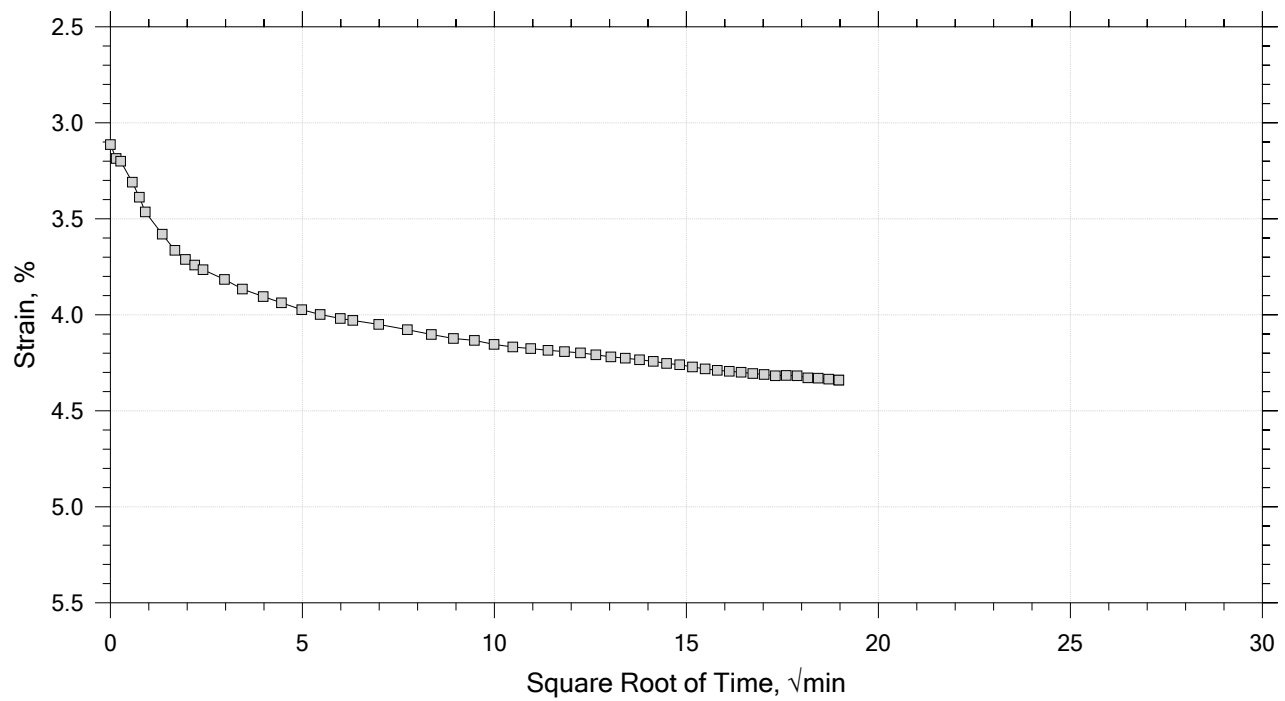
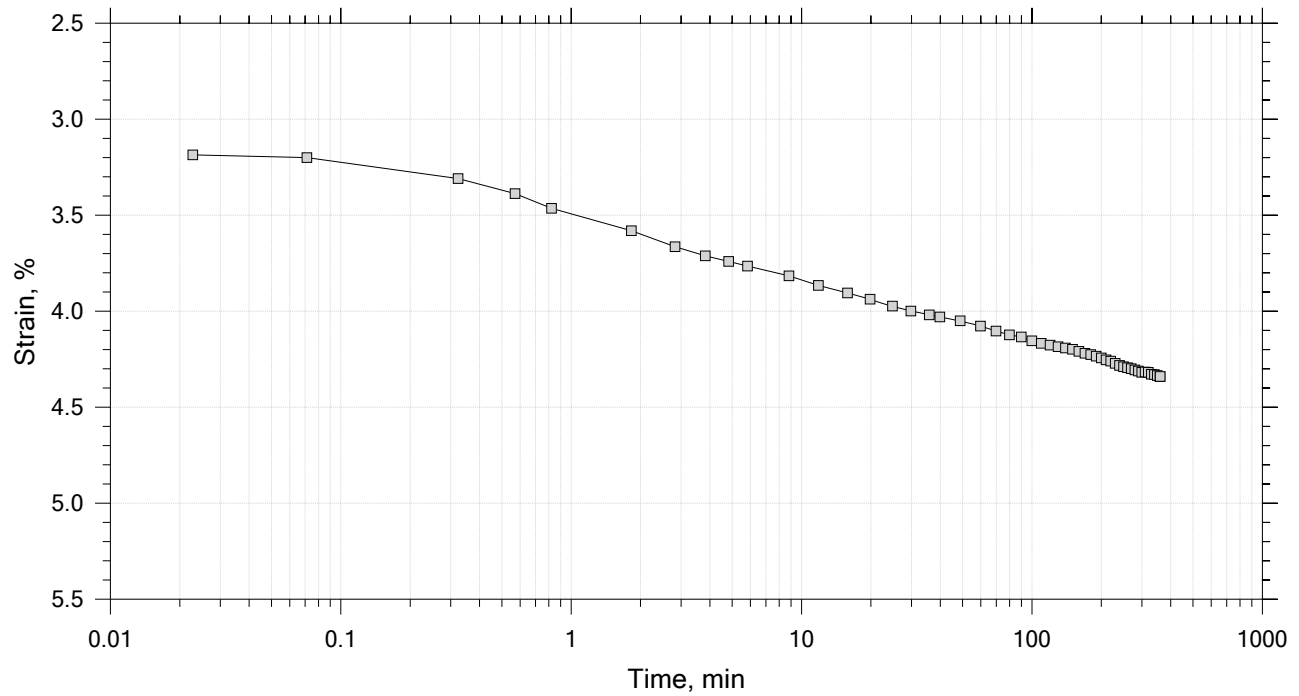
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



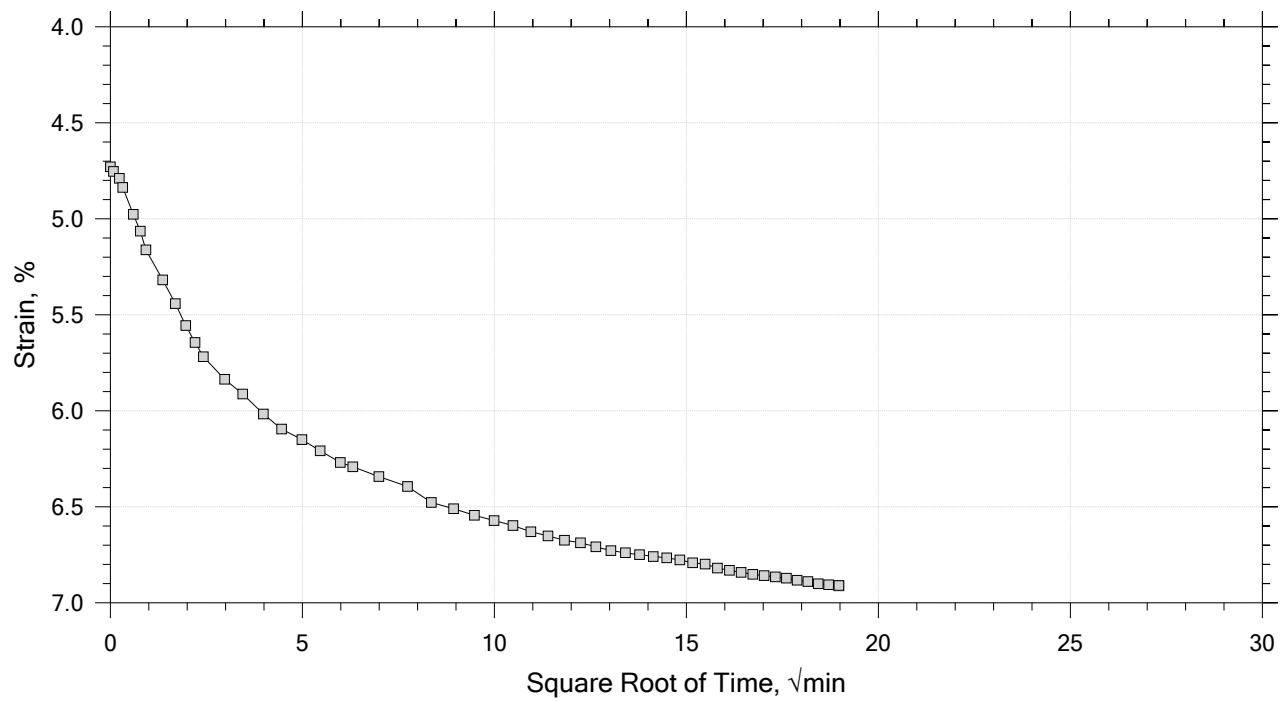
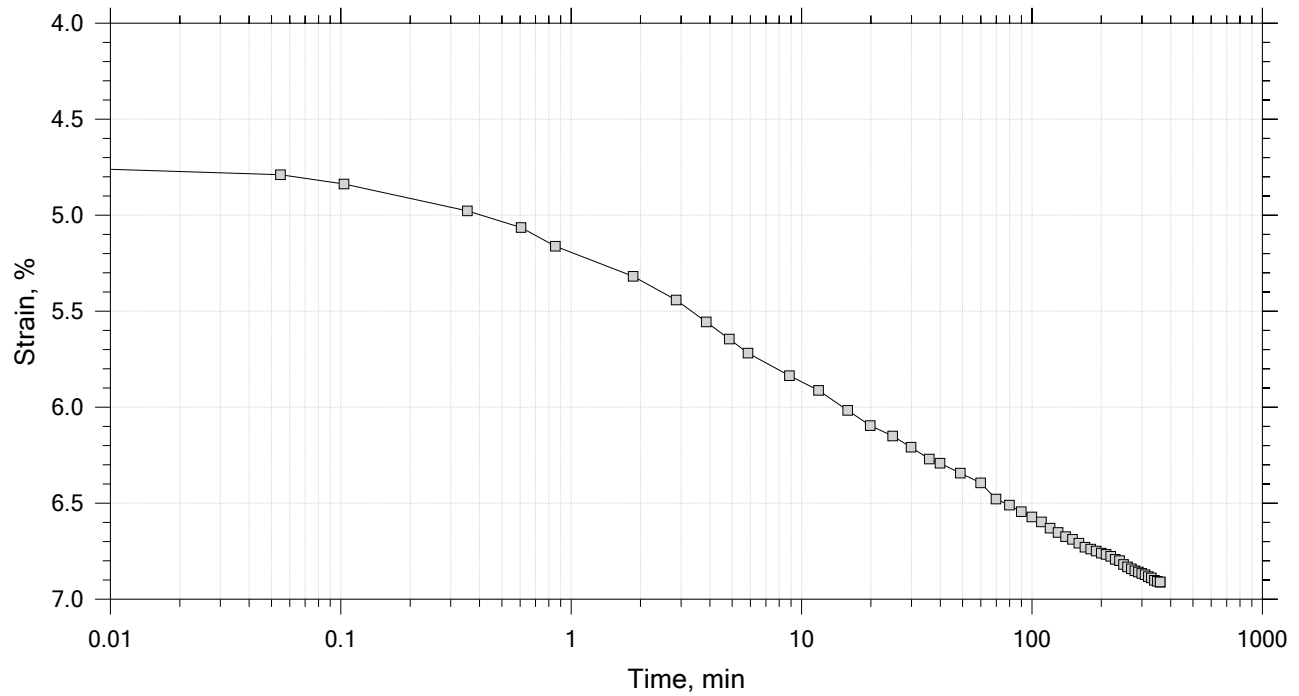
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



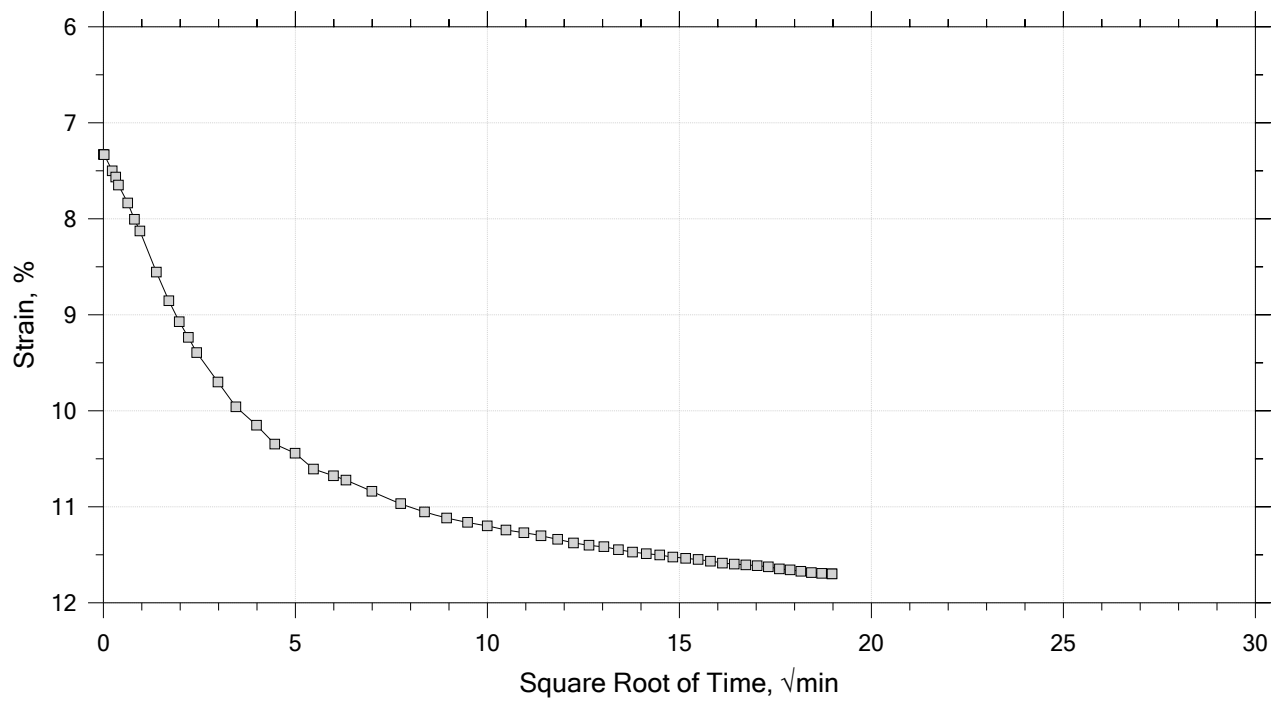
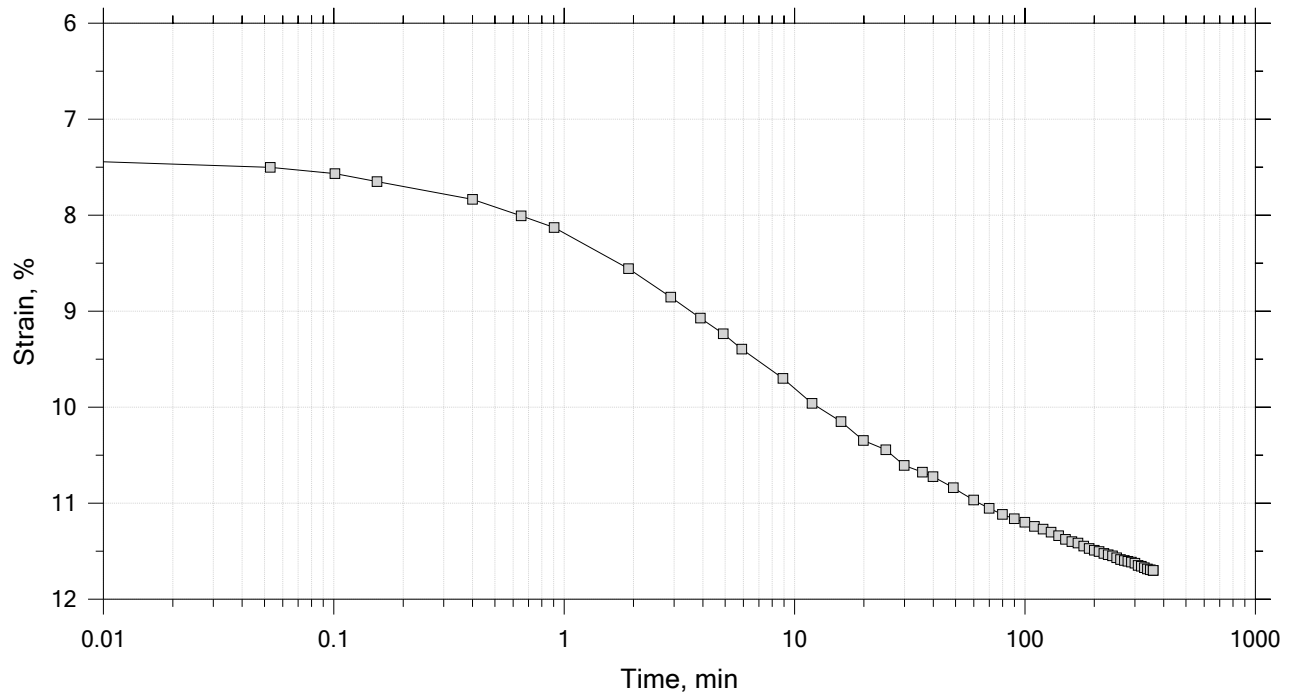
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



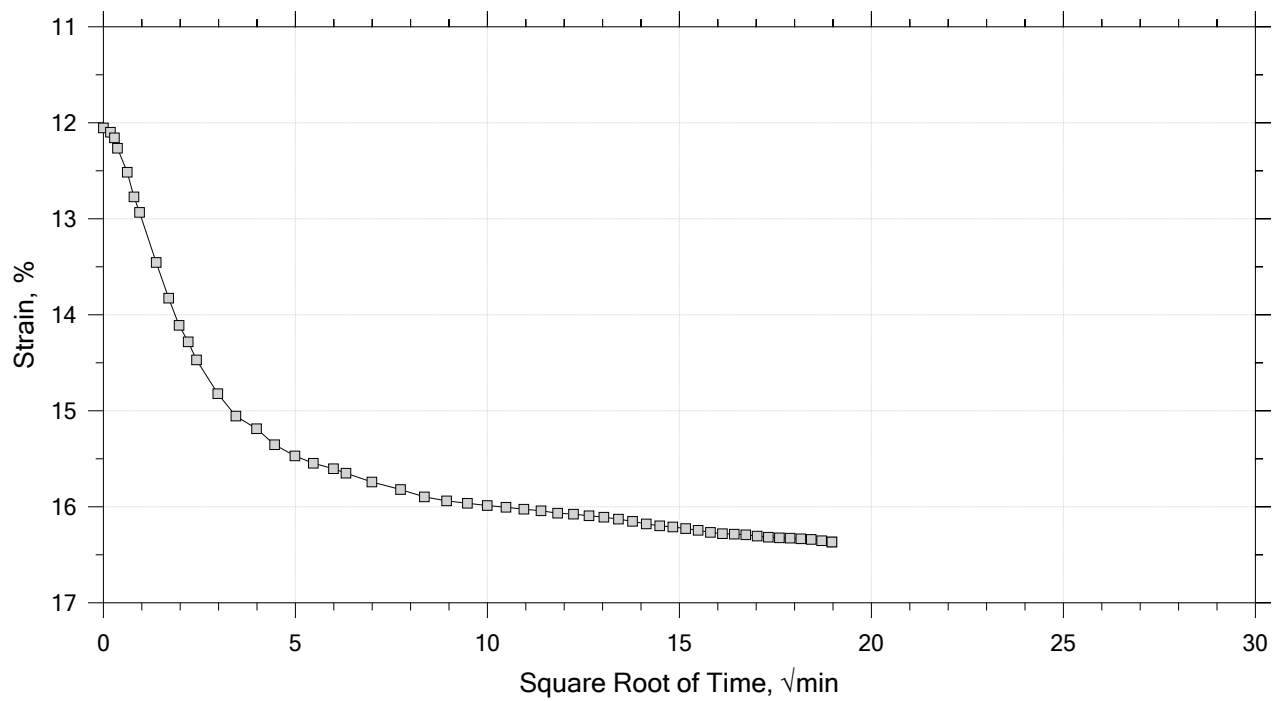
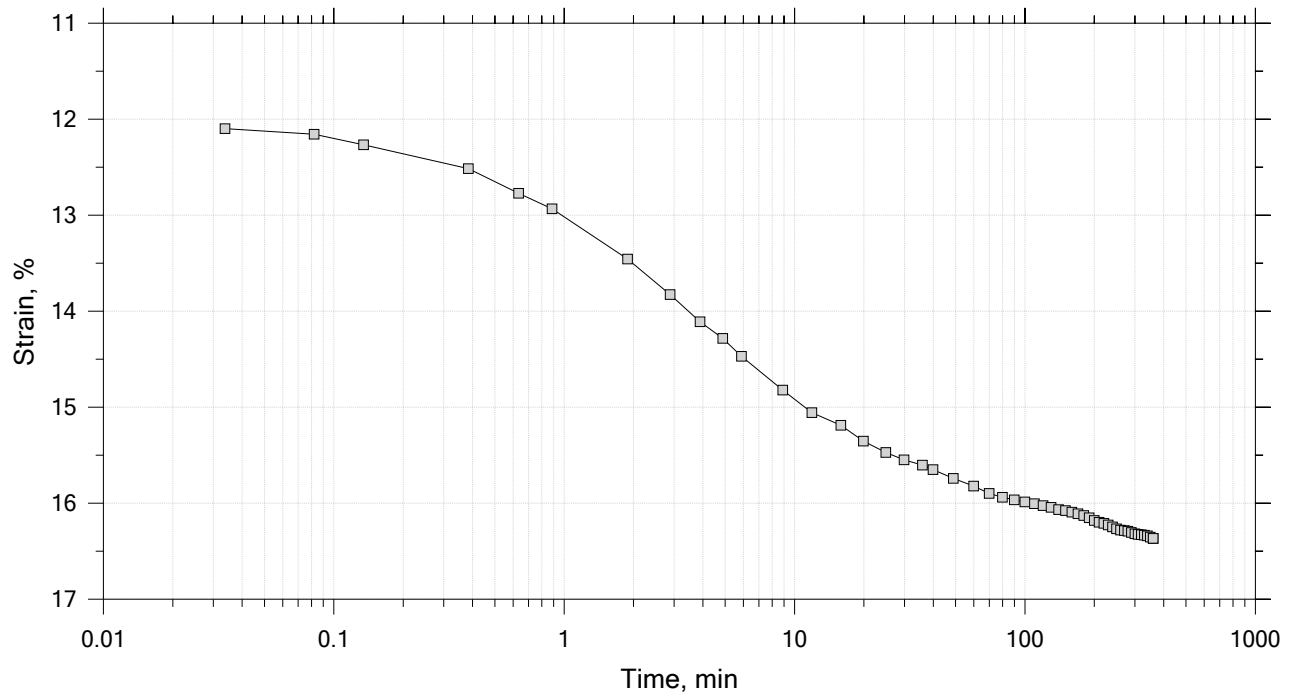
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



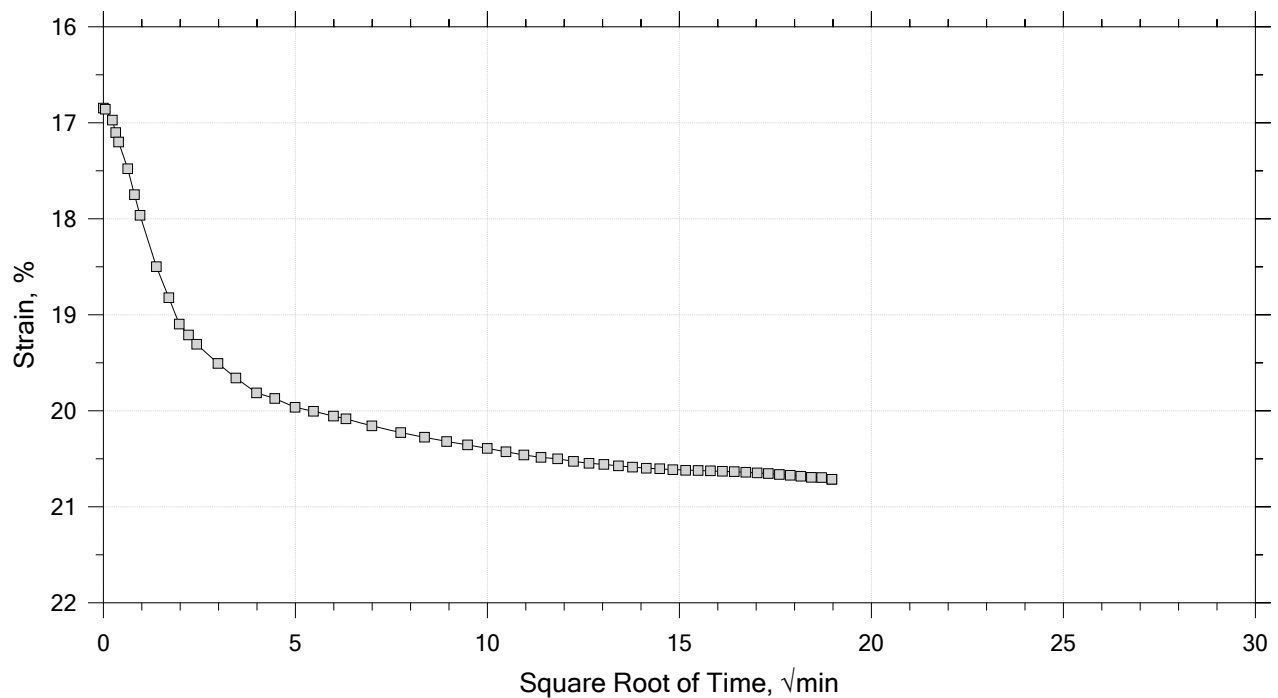
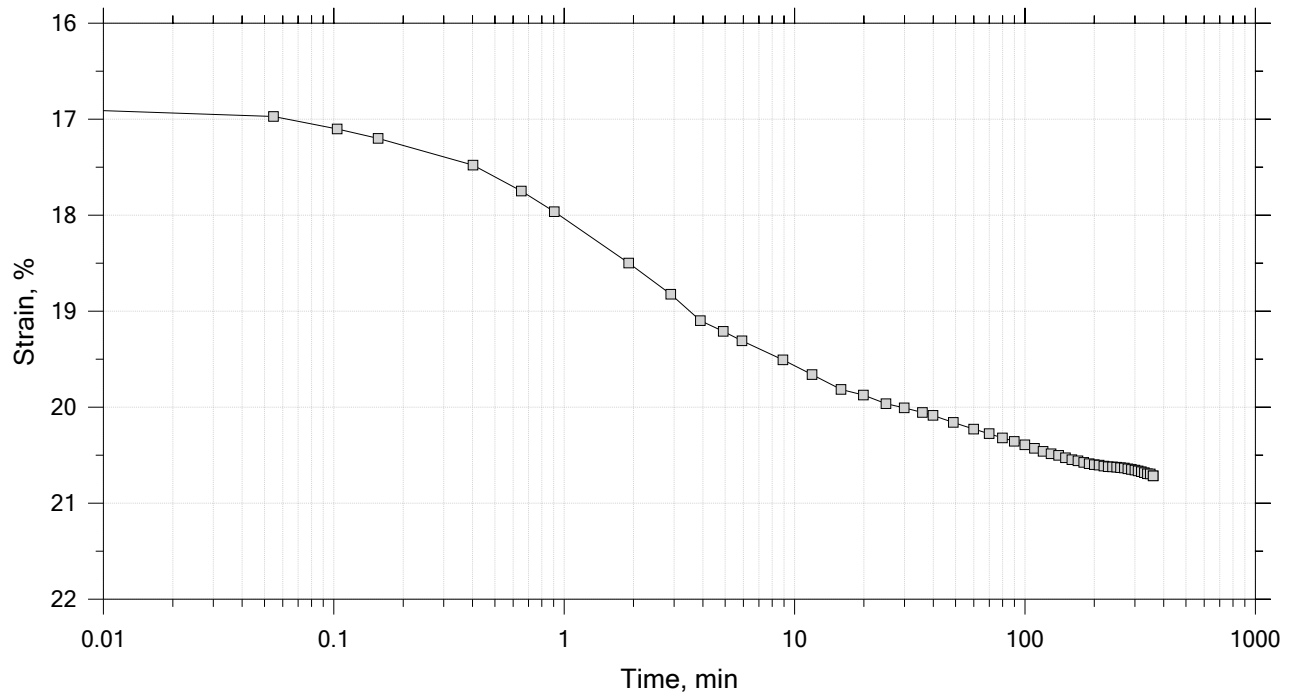
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



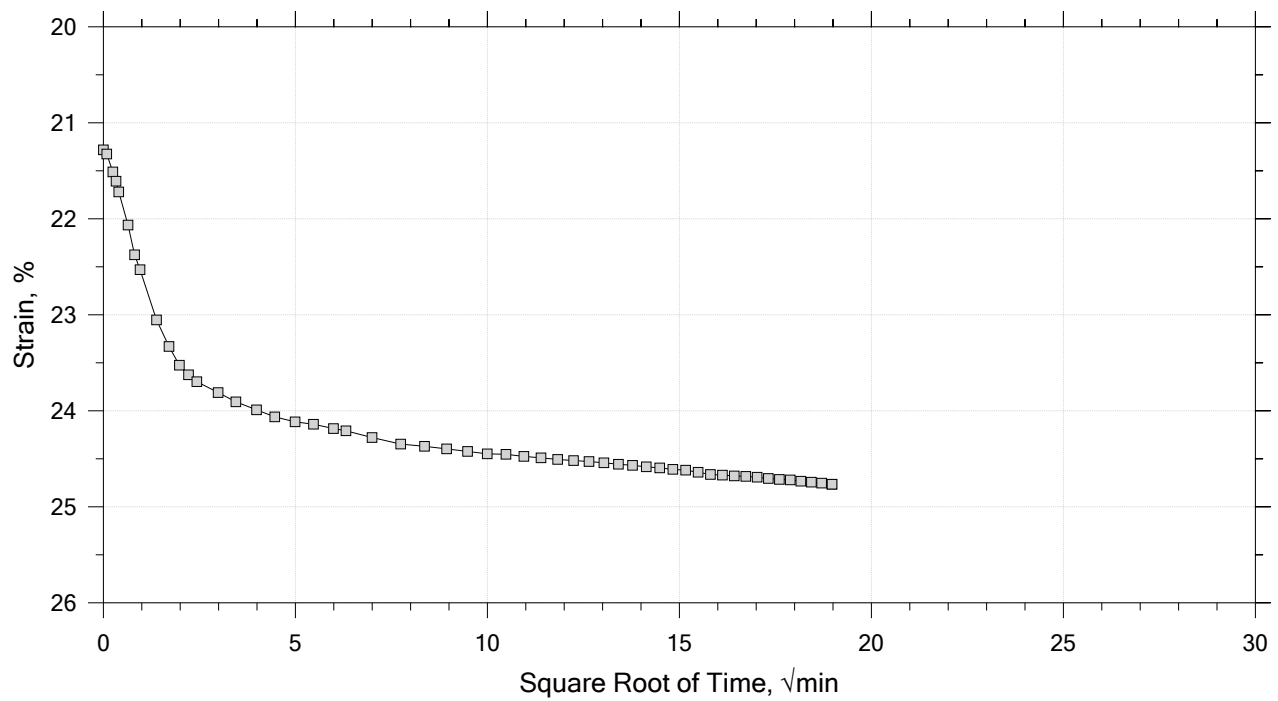
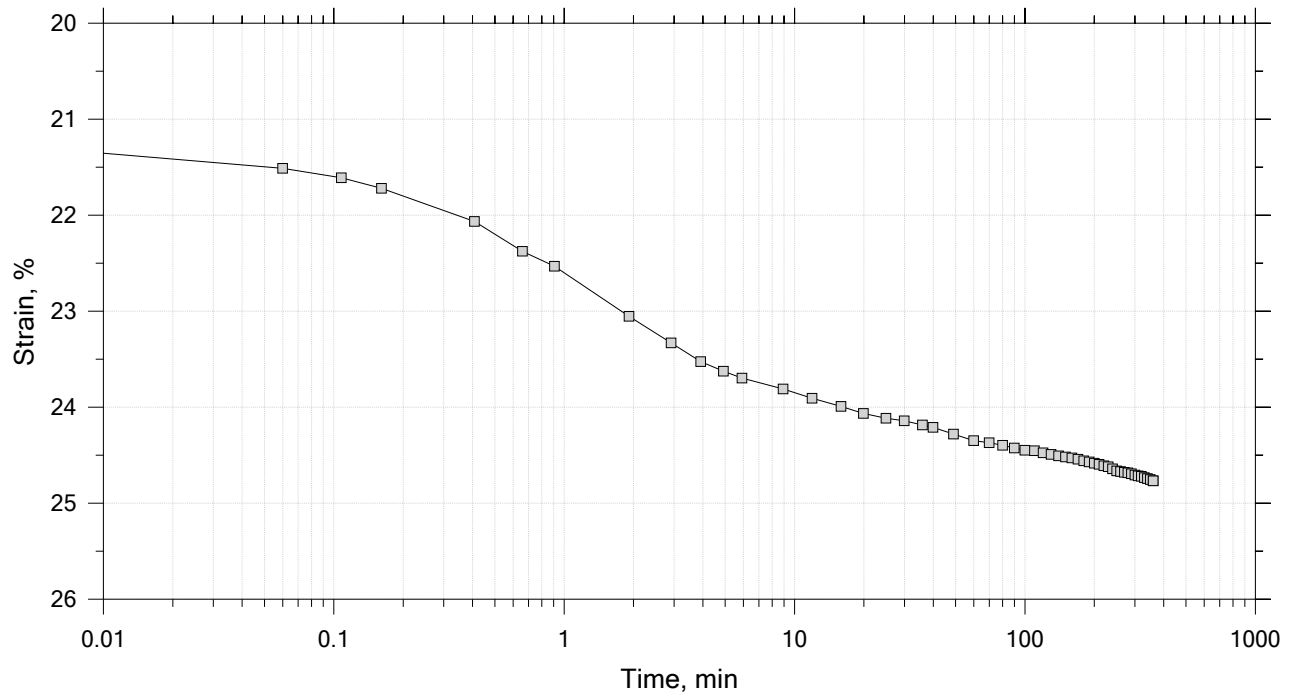
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



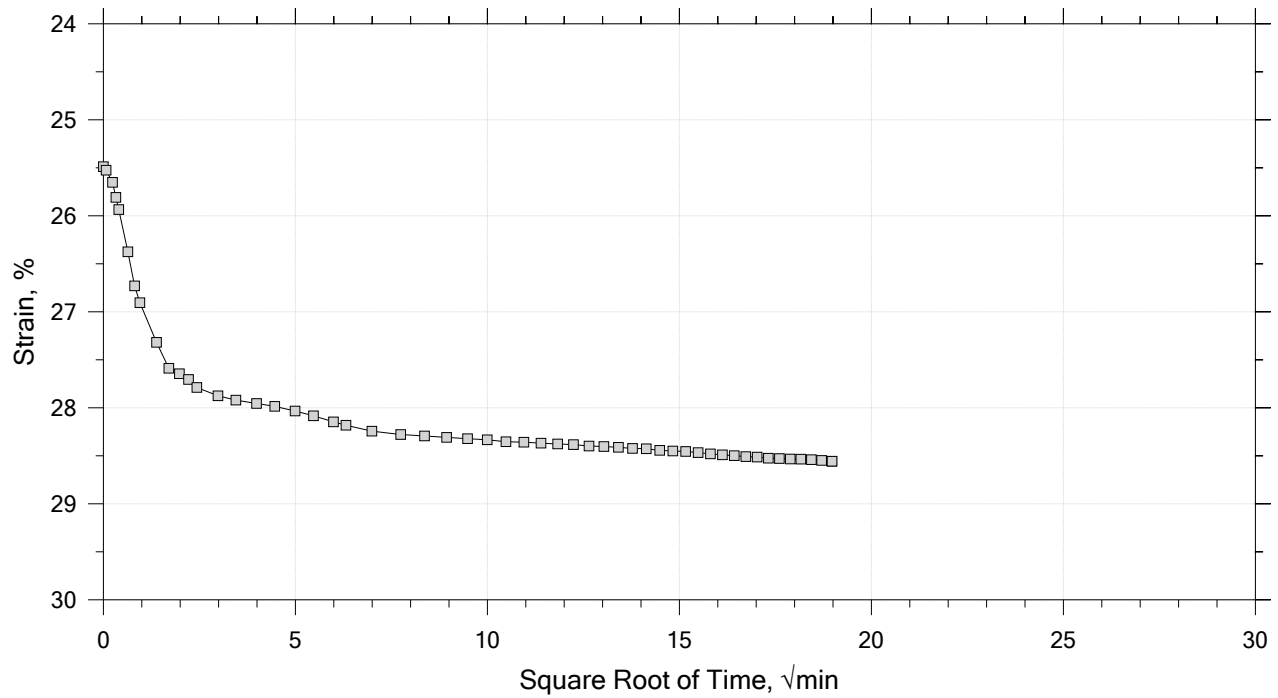
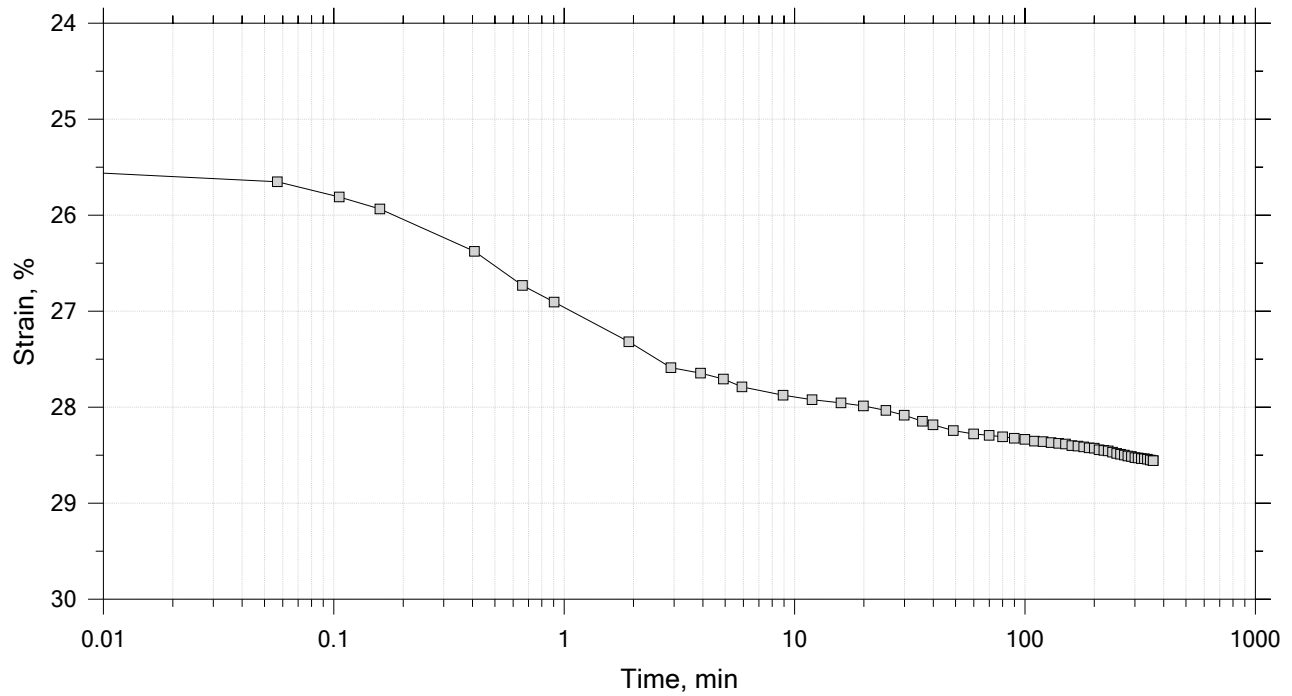
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



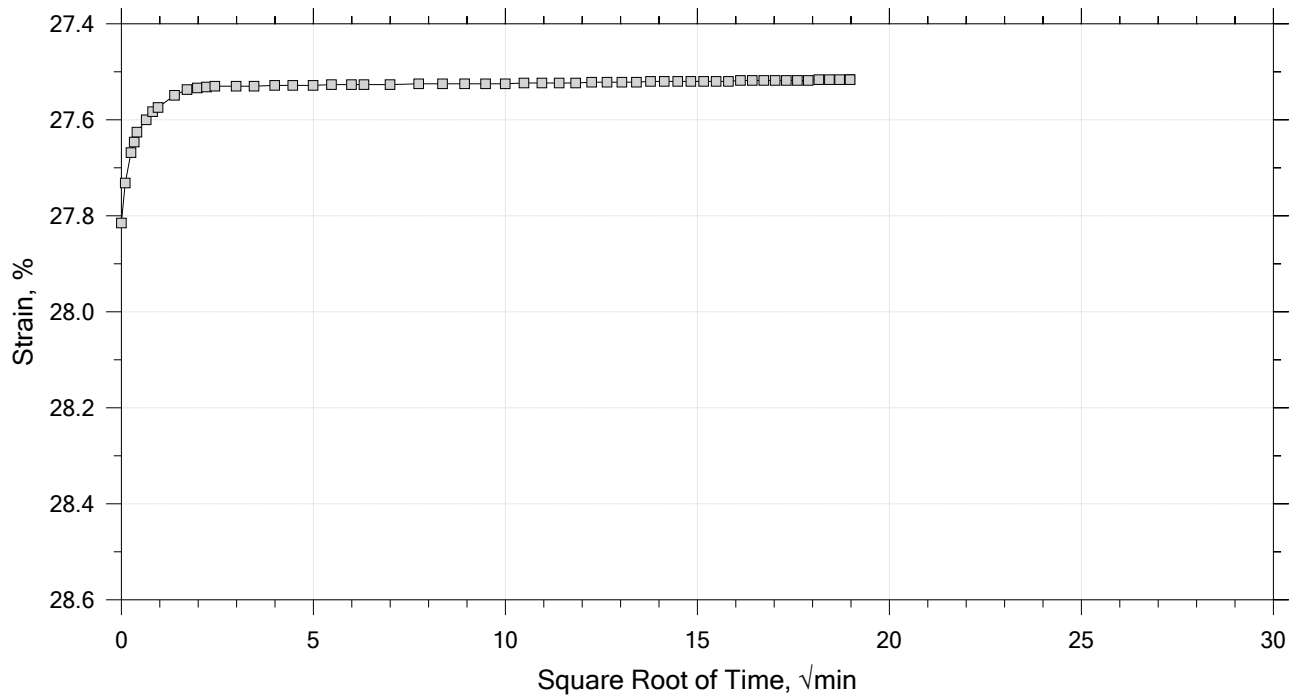
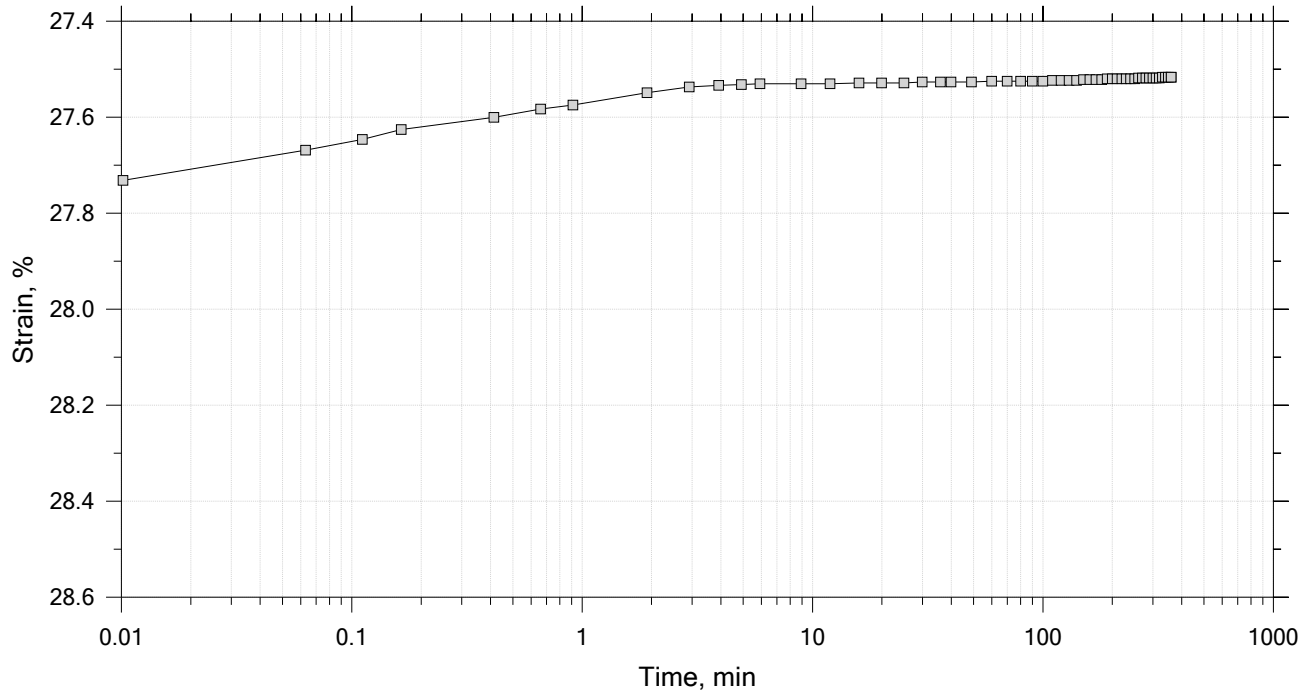
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



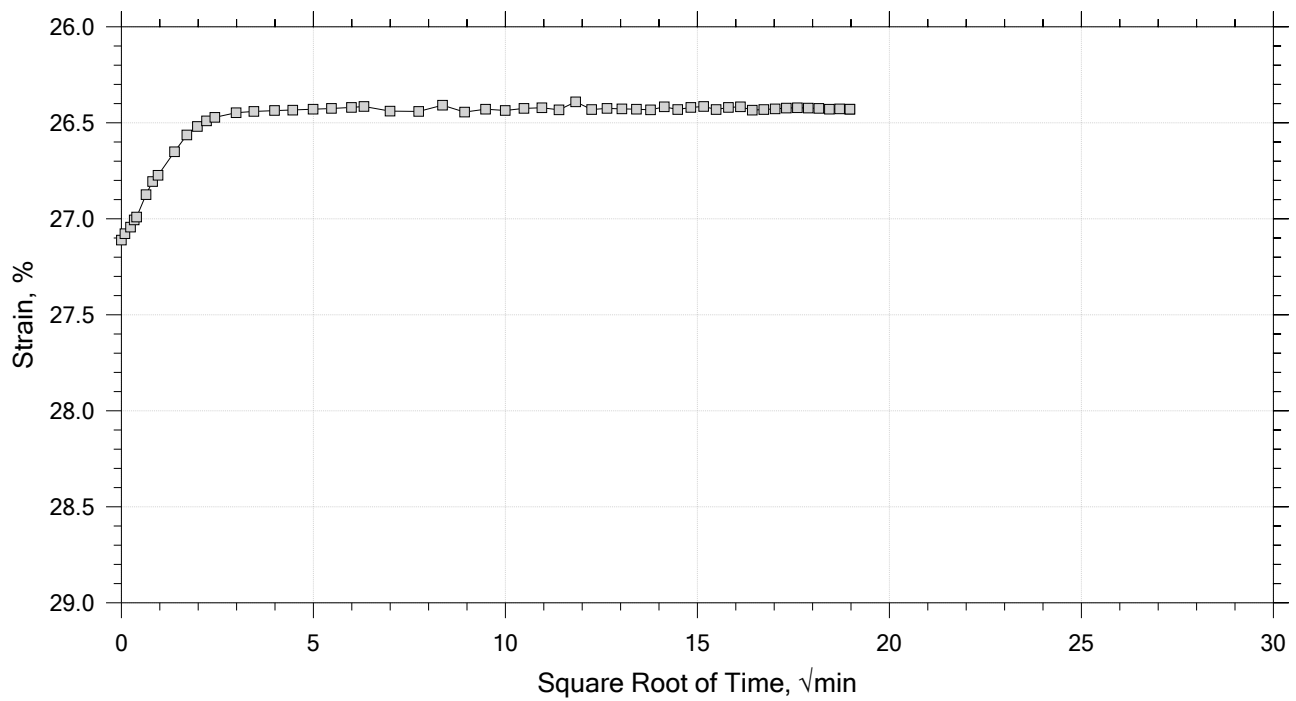
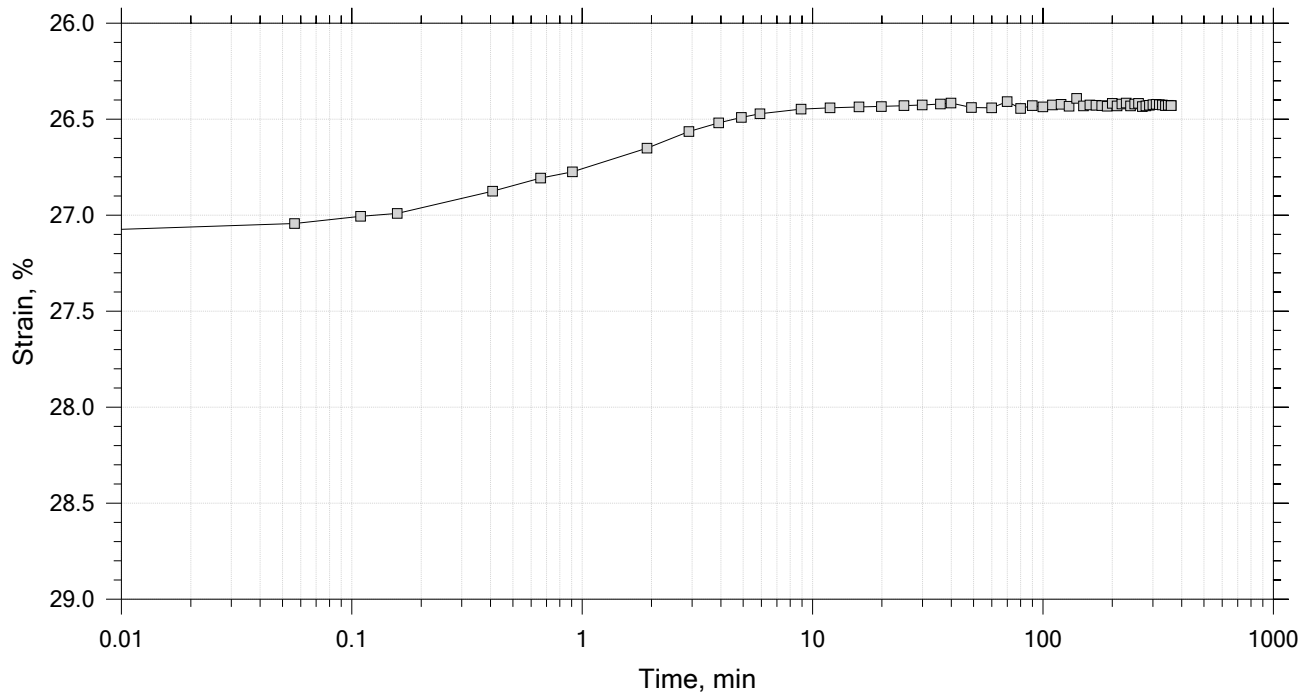
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



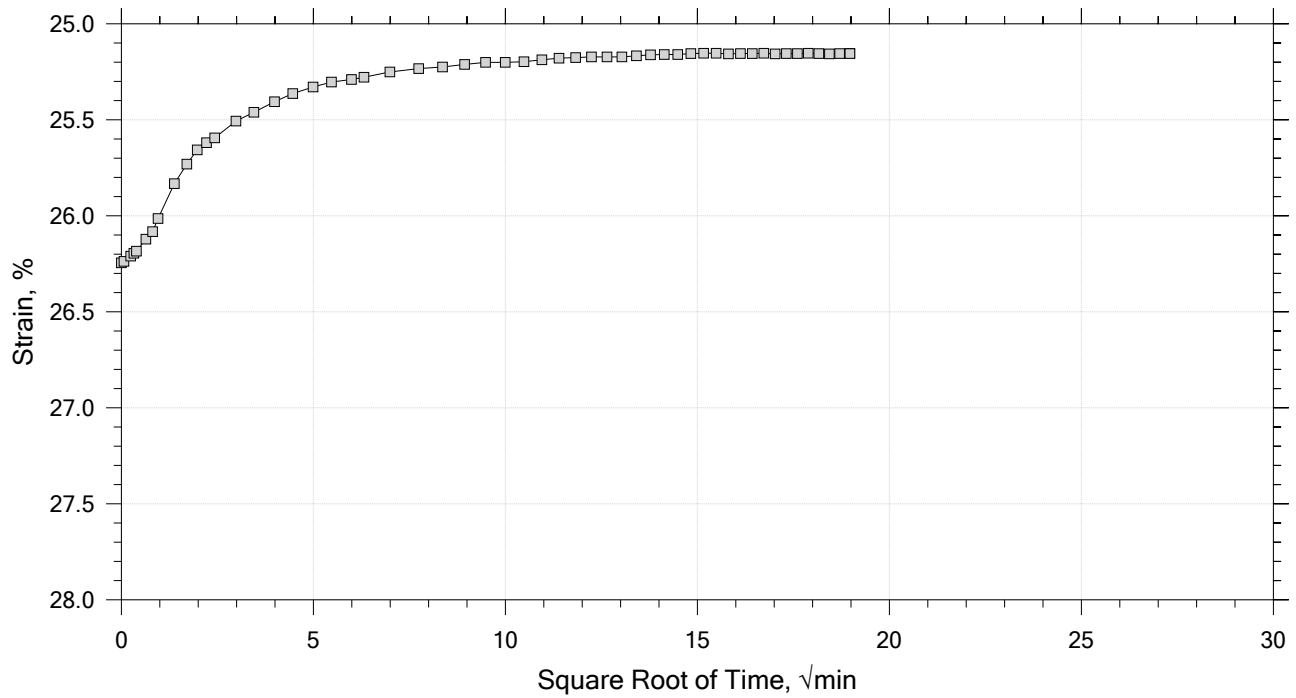
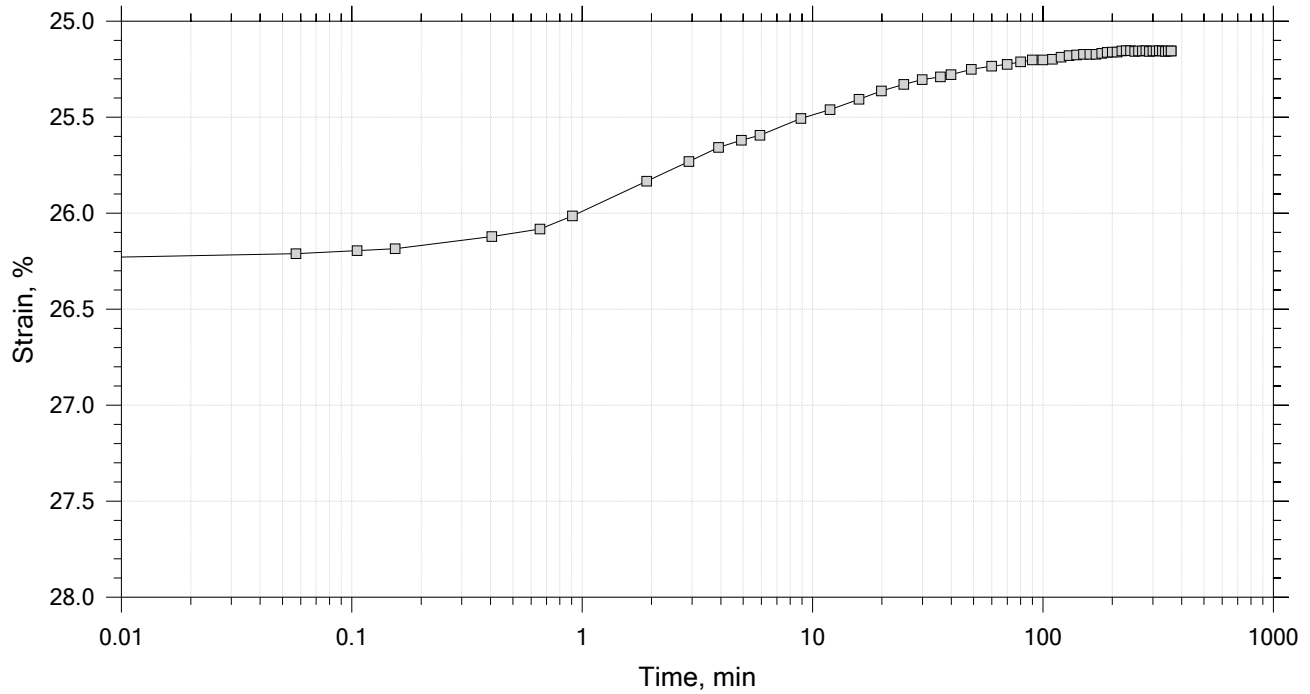
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



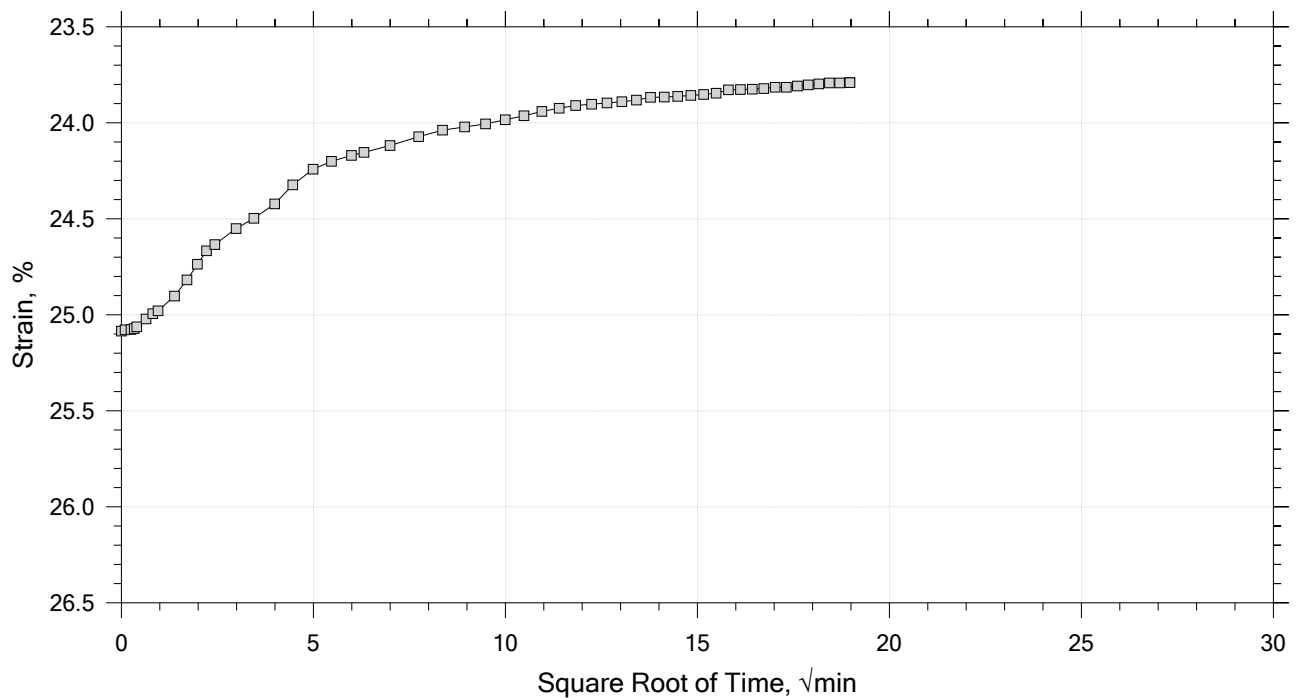
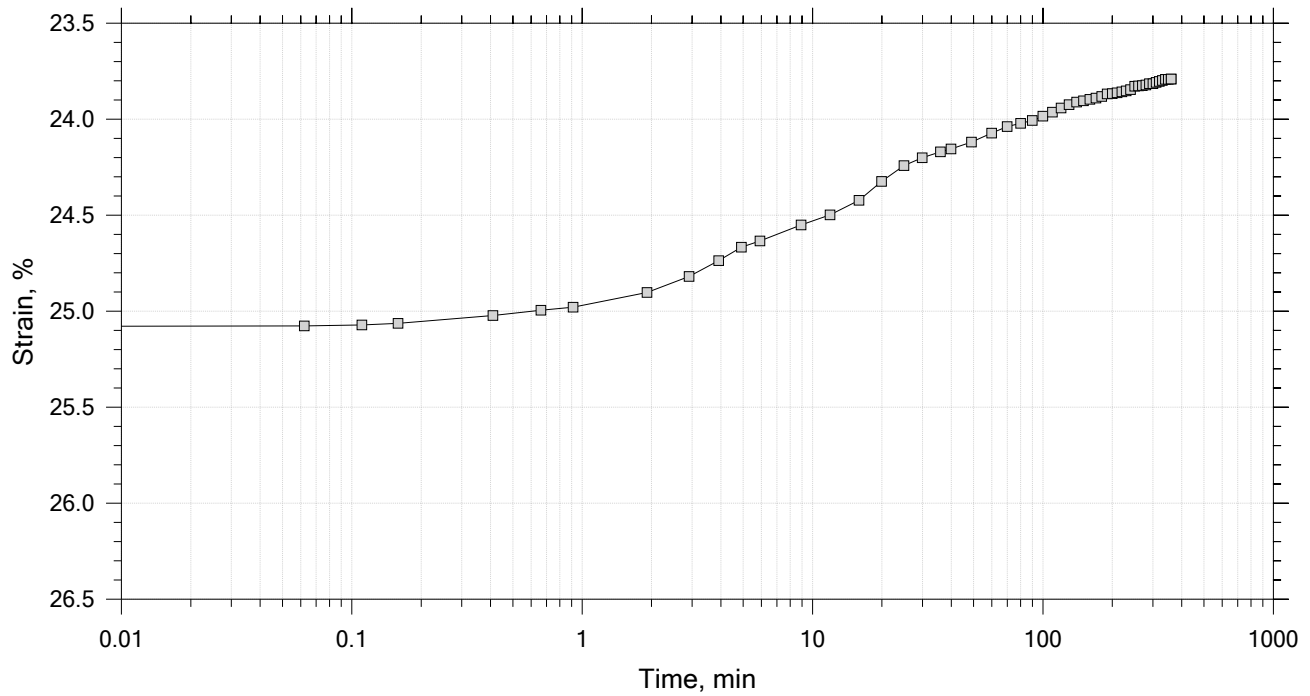
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



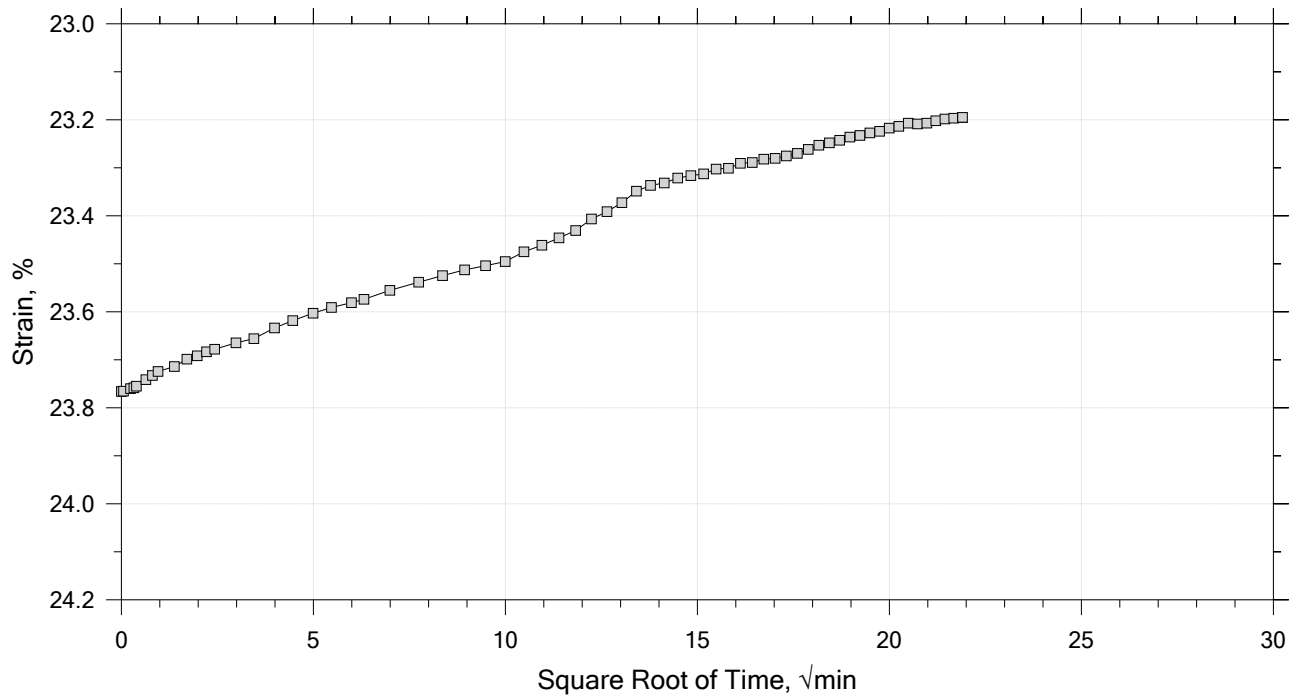
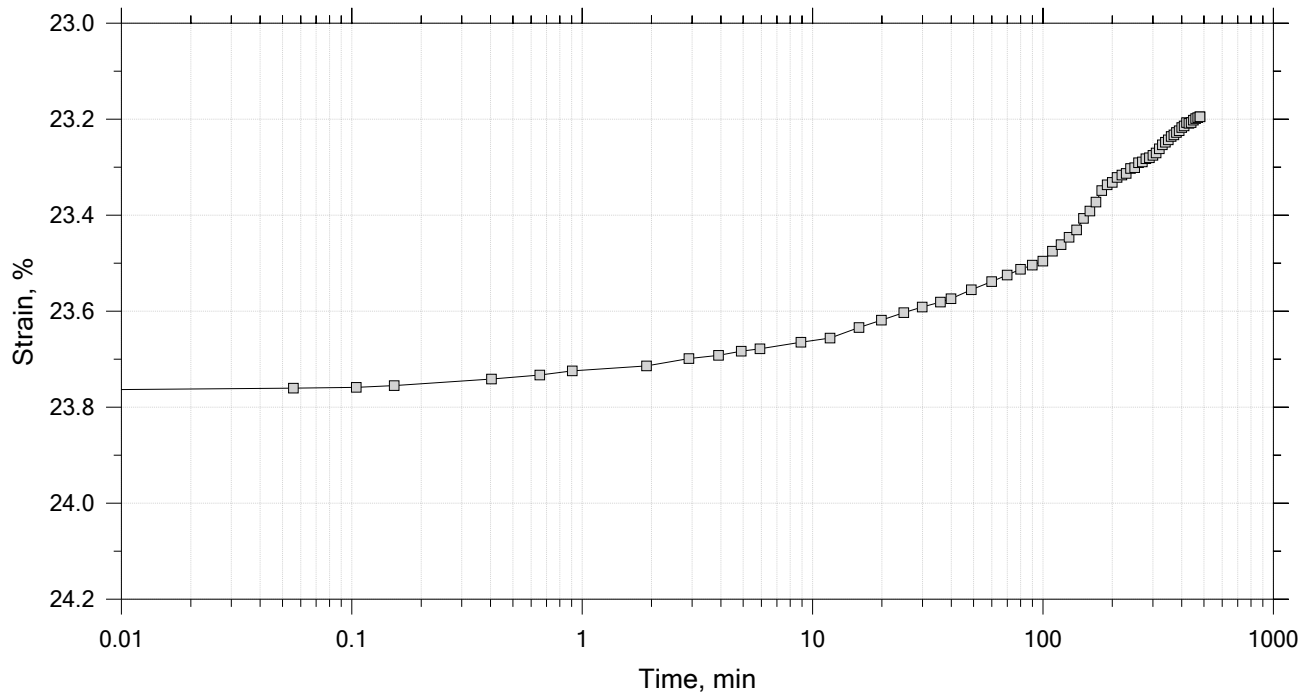
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	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.76	Liquid Limit: 37
Initial Height: 1.00 in	Initial Void Ratio: 1.02	Plastic Limit: 20
Final Height: 0.77 in	Final Void Ratio: 0.551	Plasticity Index: 17

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E-0468	RING		E2507
Mass Container, gm	8.27	109.94	109.94	8.18
Mass Container + Wet Soil, gm	84.03	258.8	241.73	139.81
Mass Container + Dry Soil, gm	63.81	219.79	219.79	117.9
Mass Dry Soil, gm	55.54	109.85	109.85	109.72
Water Content, %	36.41	35.51	19.97	19.97
Void Ratio	---	1.02	0.55	---
Degree of Saturation, %	---	96.09	100.00	---
Dry Unit Weight, pcf	---	85.255	111	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

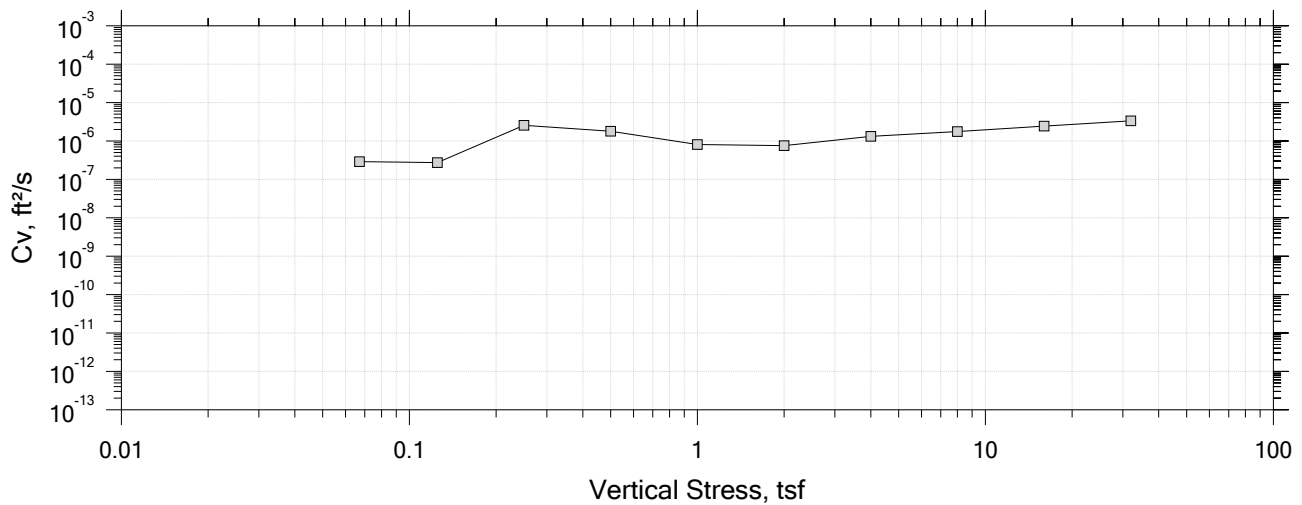
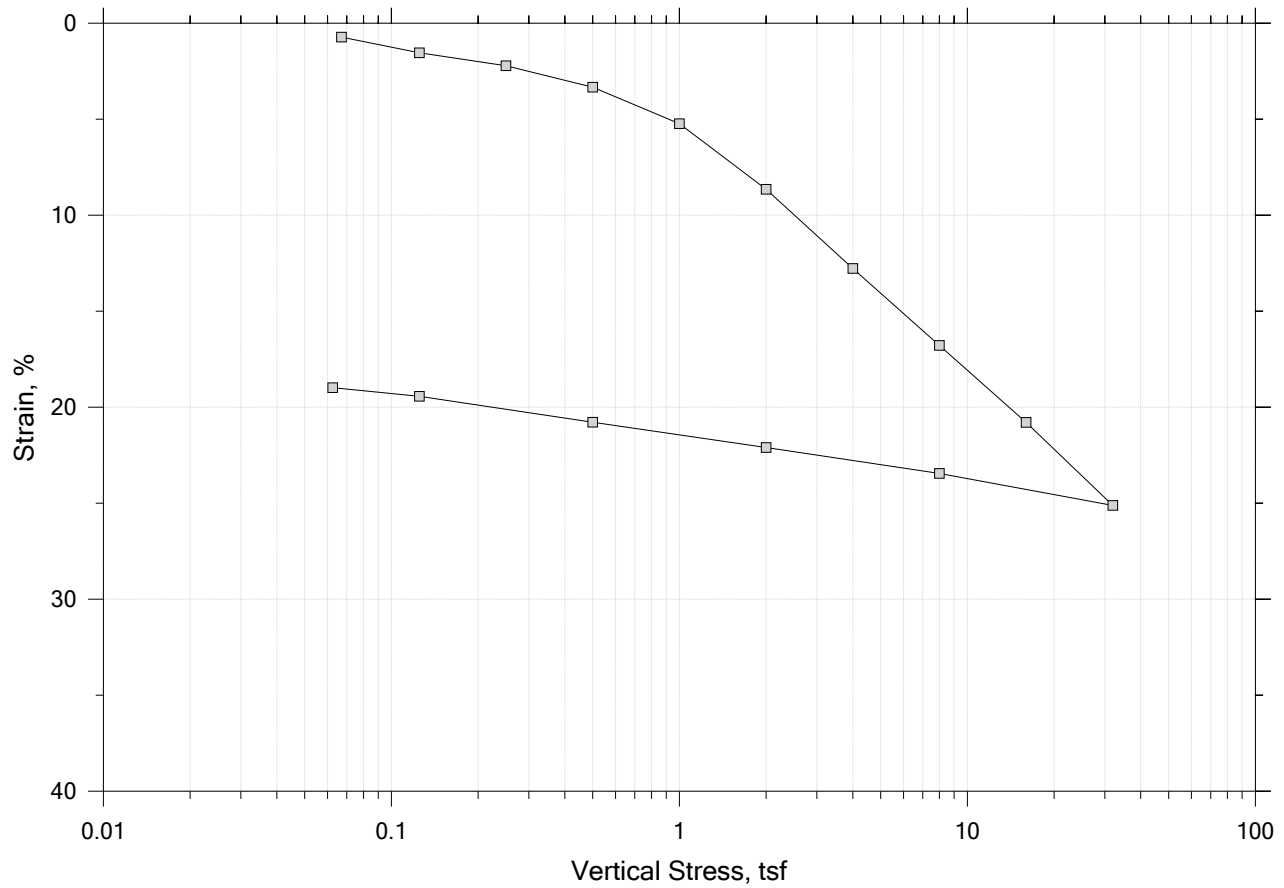
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-202	Tested By: md	Checked By: mcm
	Sample No.: U-2	Test Date: 02/23/21	Depth: 15-17 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-C, Swell Pressure = 0.0659 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

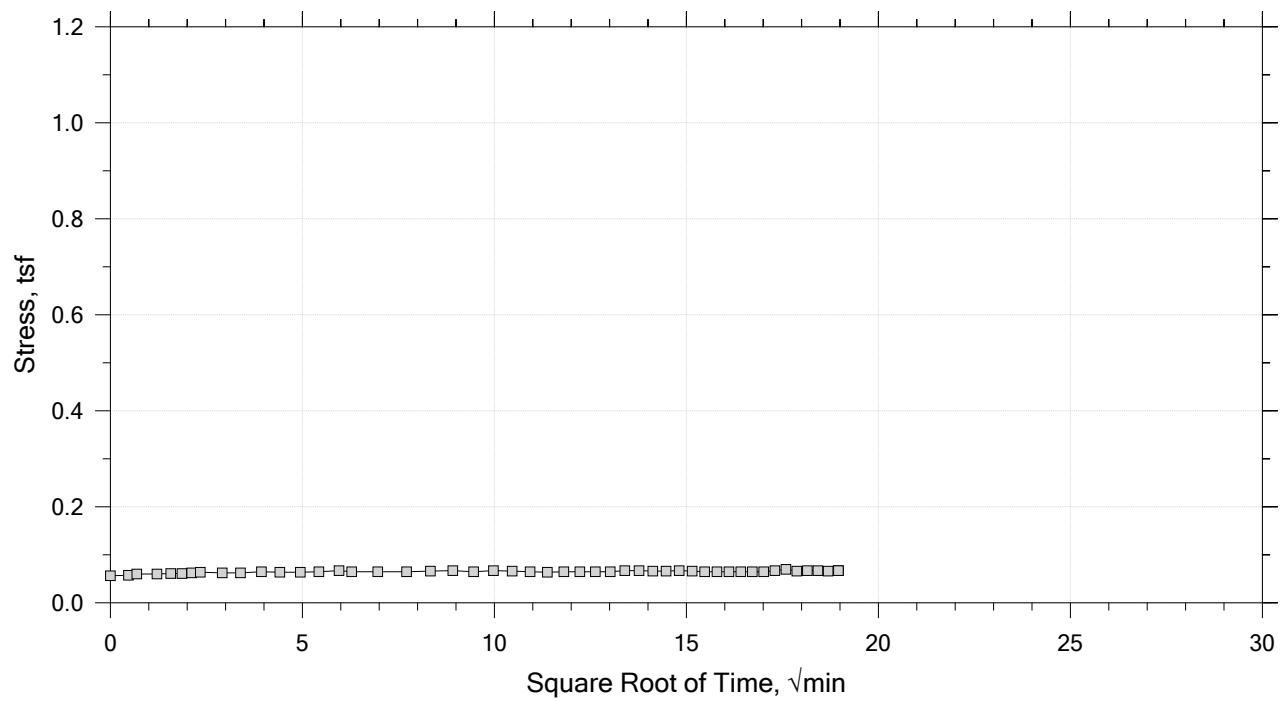
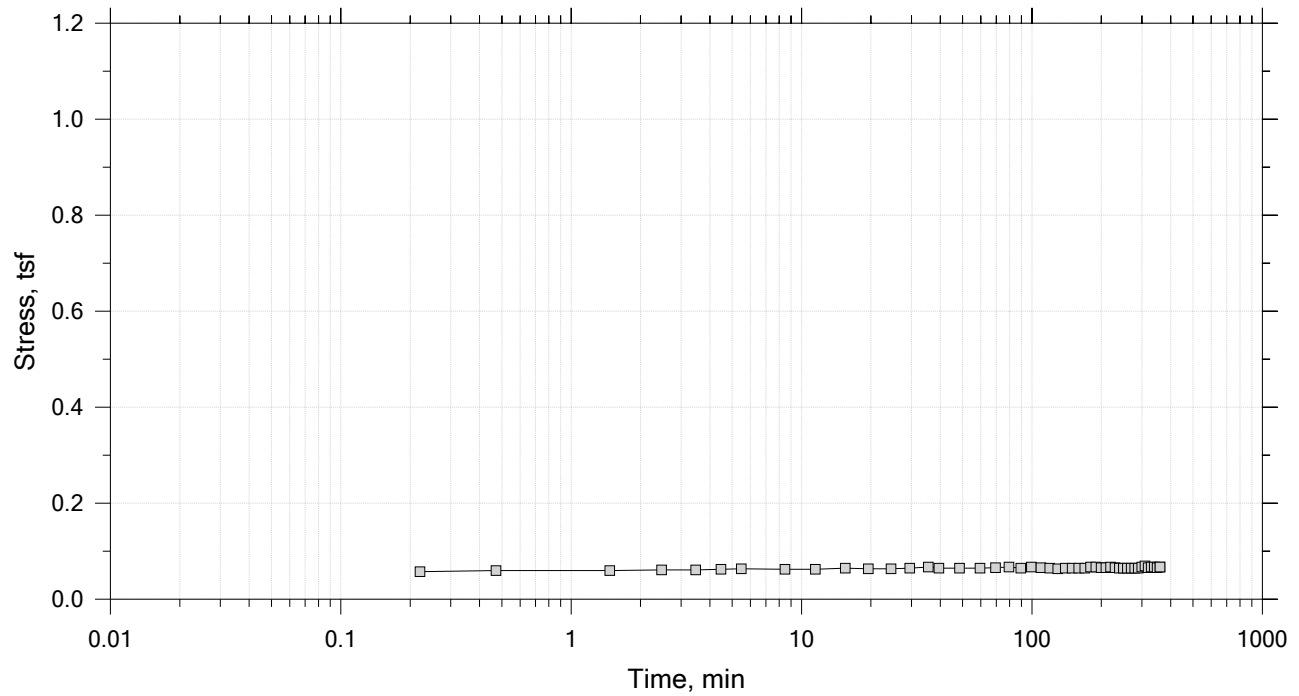
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0671 tsf



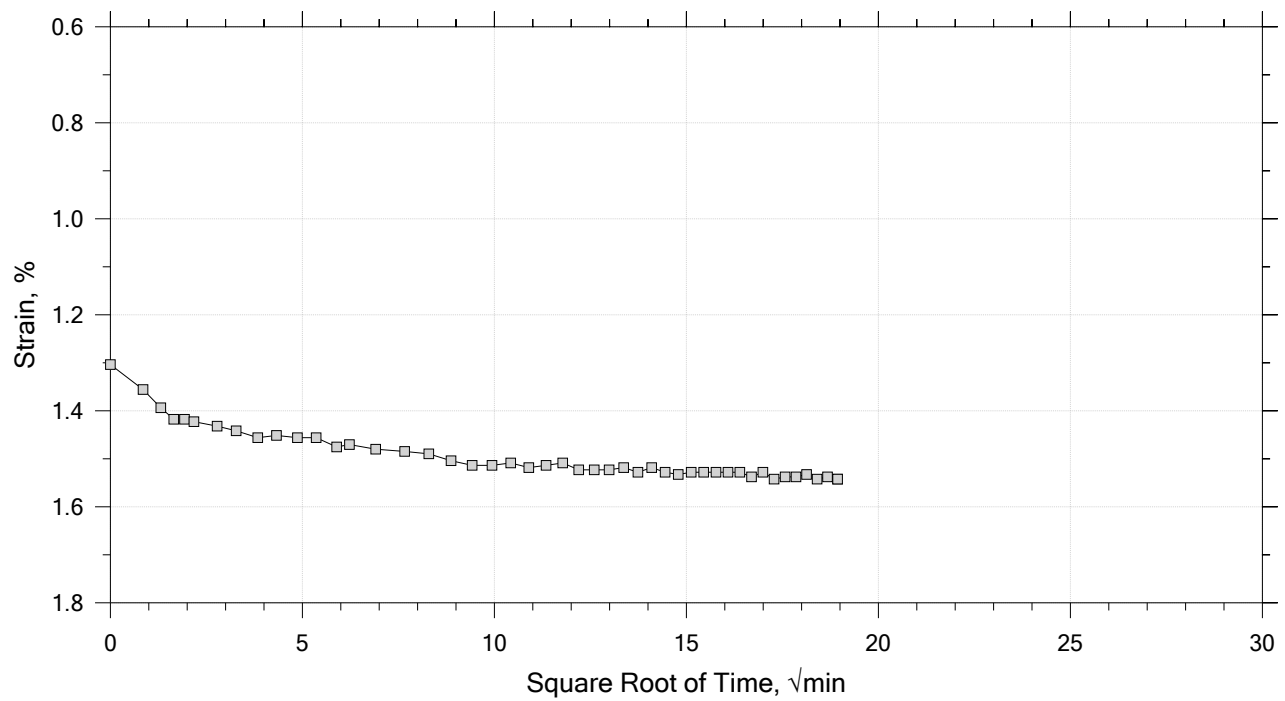
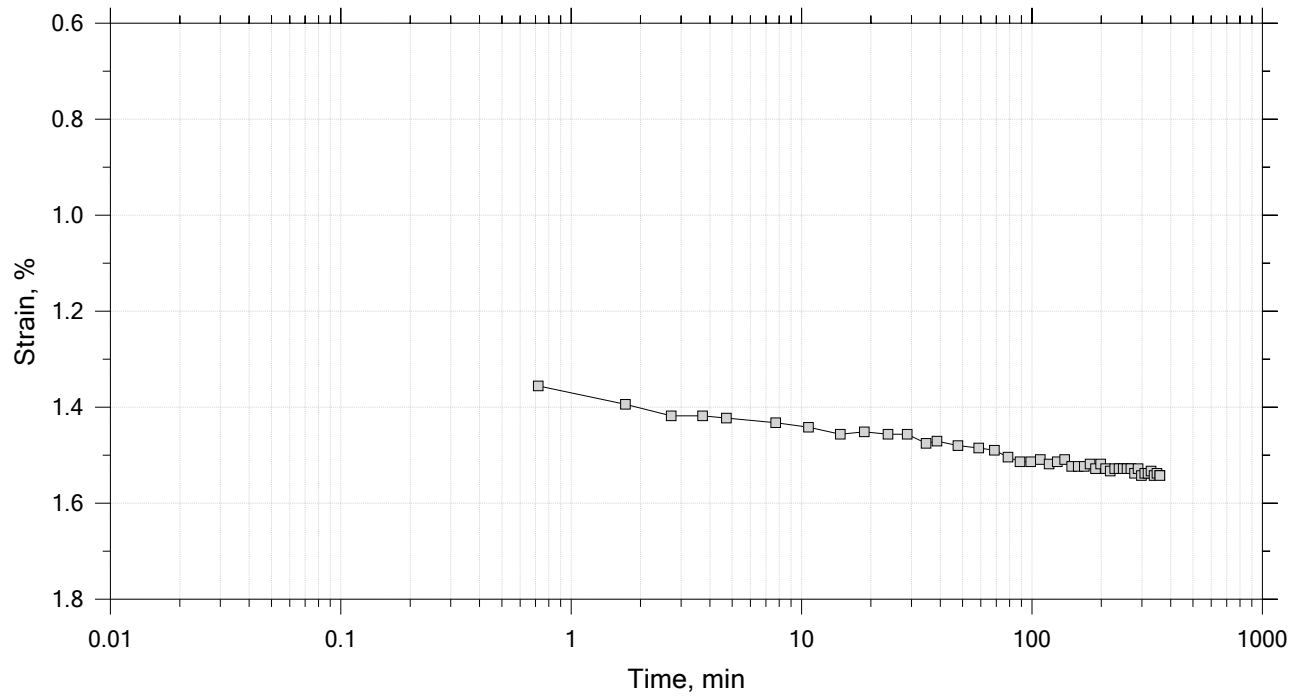
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



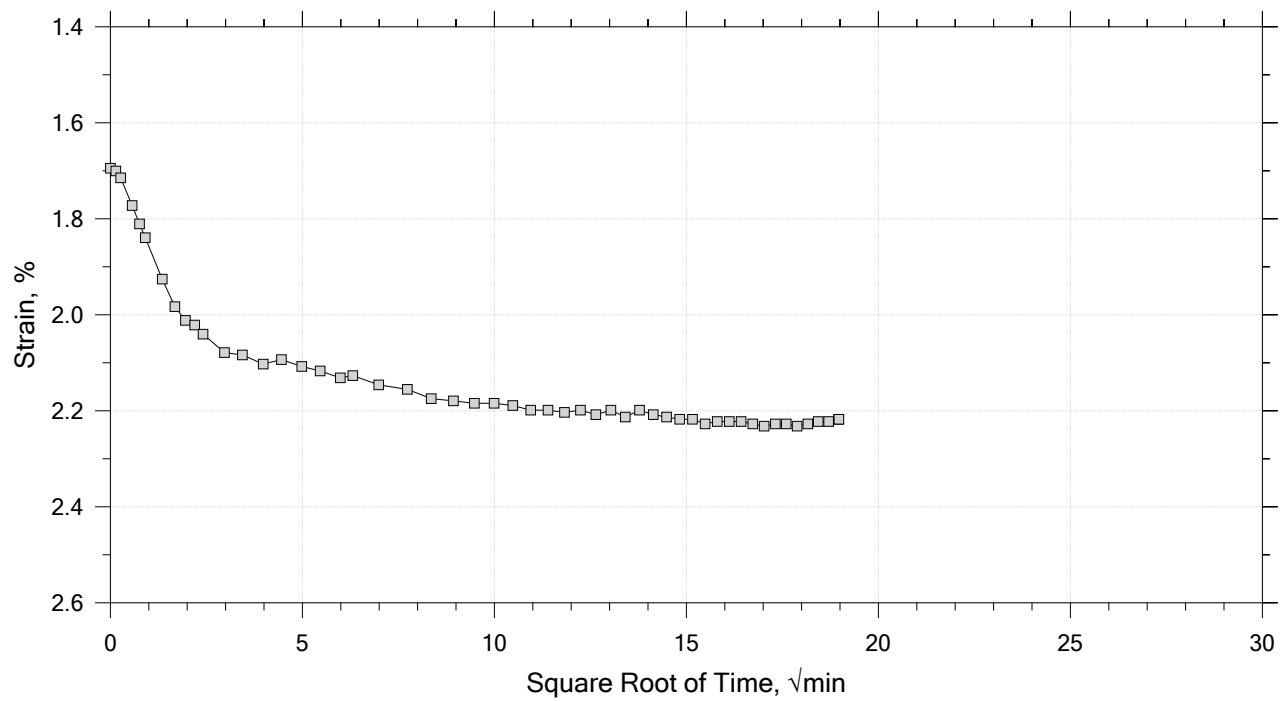
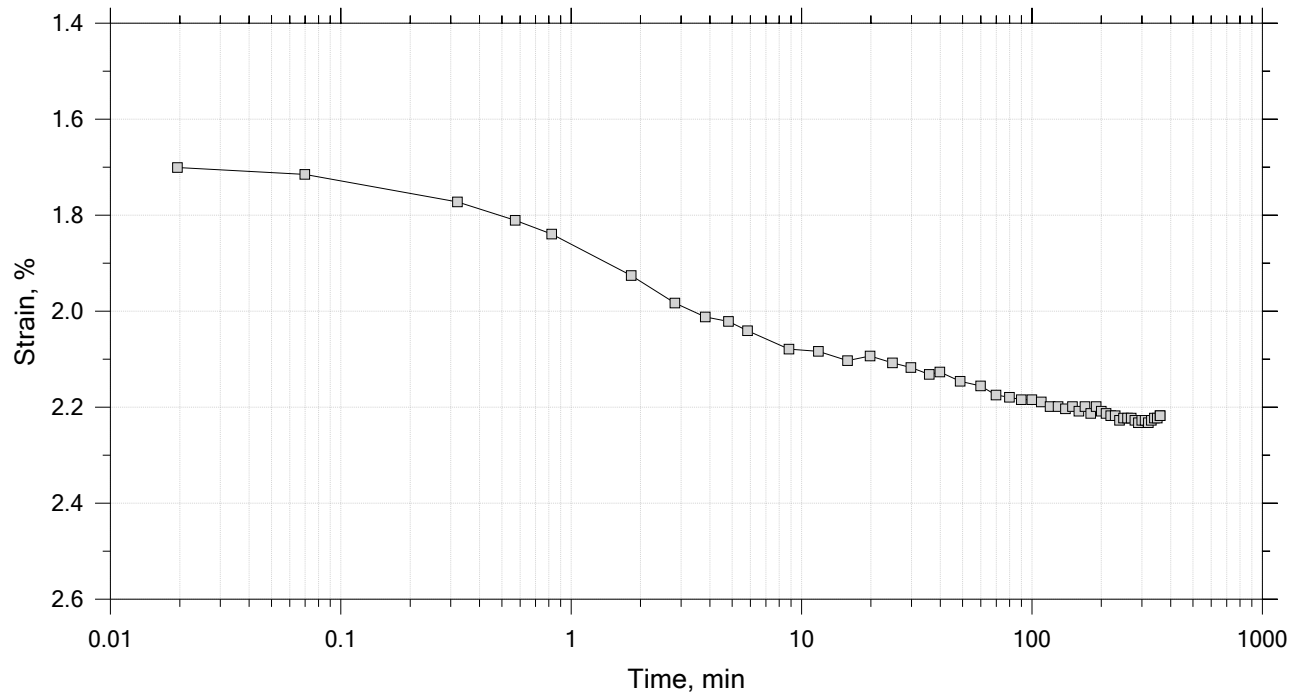
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



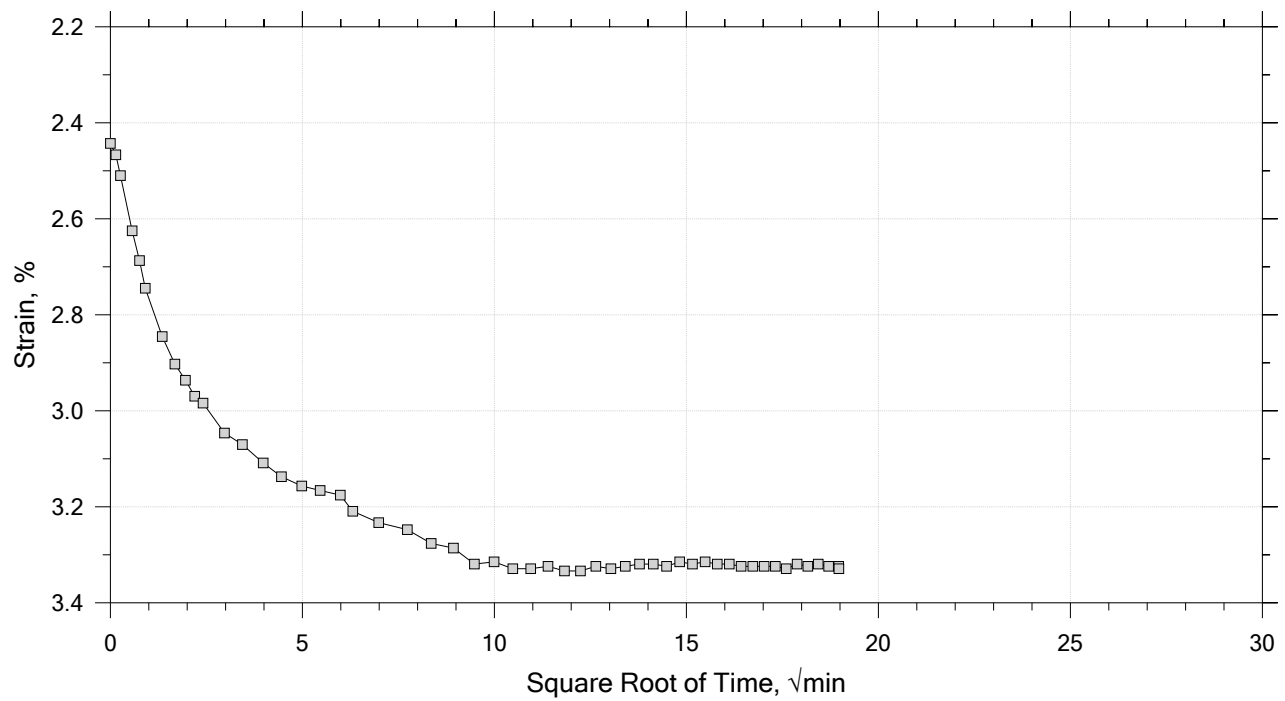
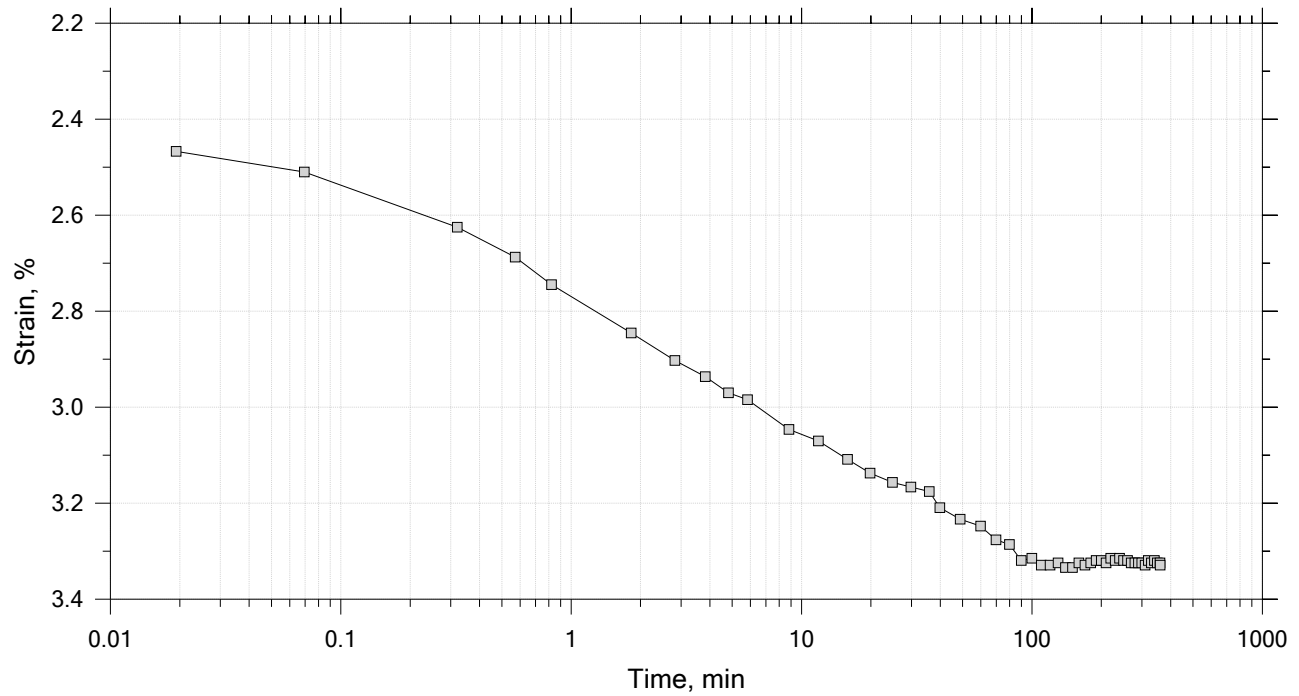
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



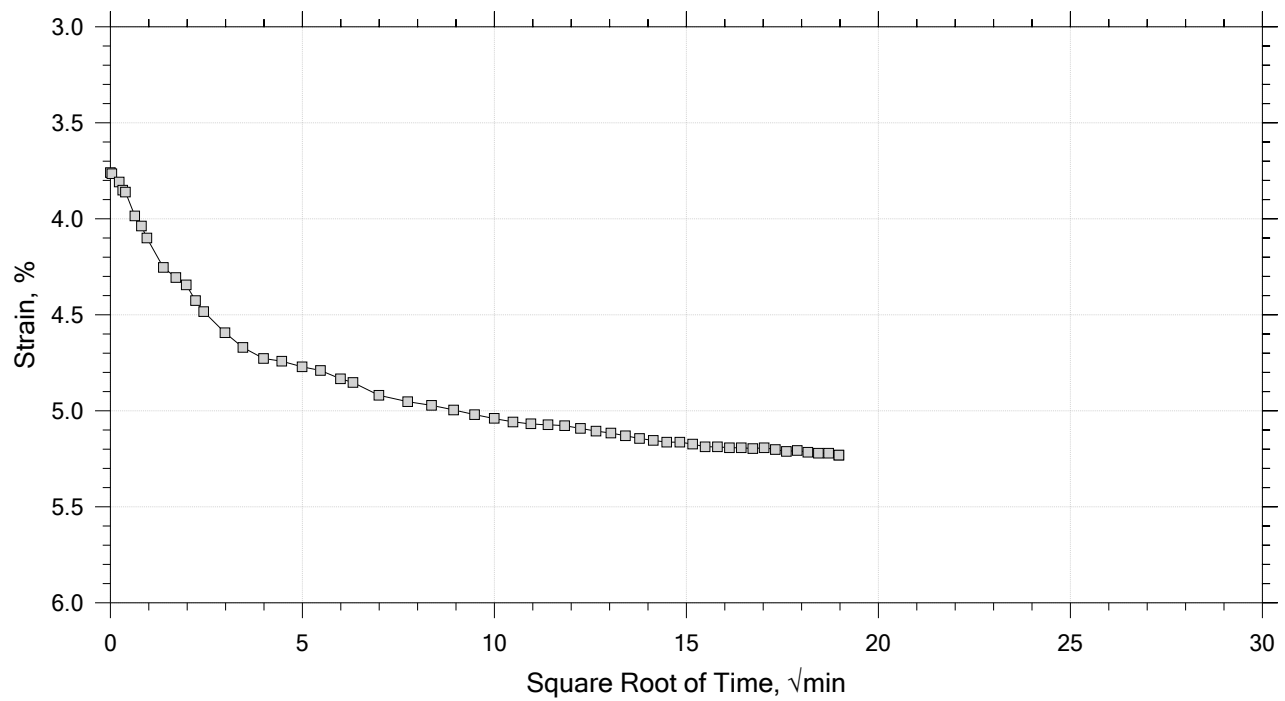
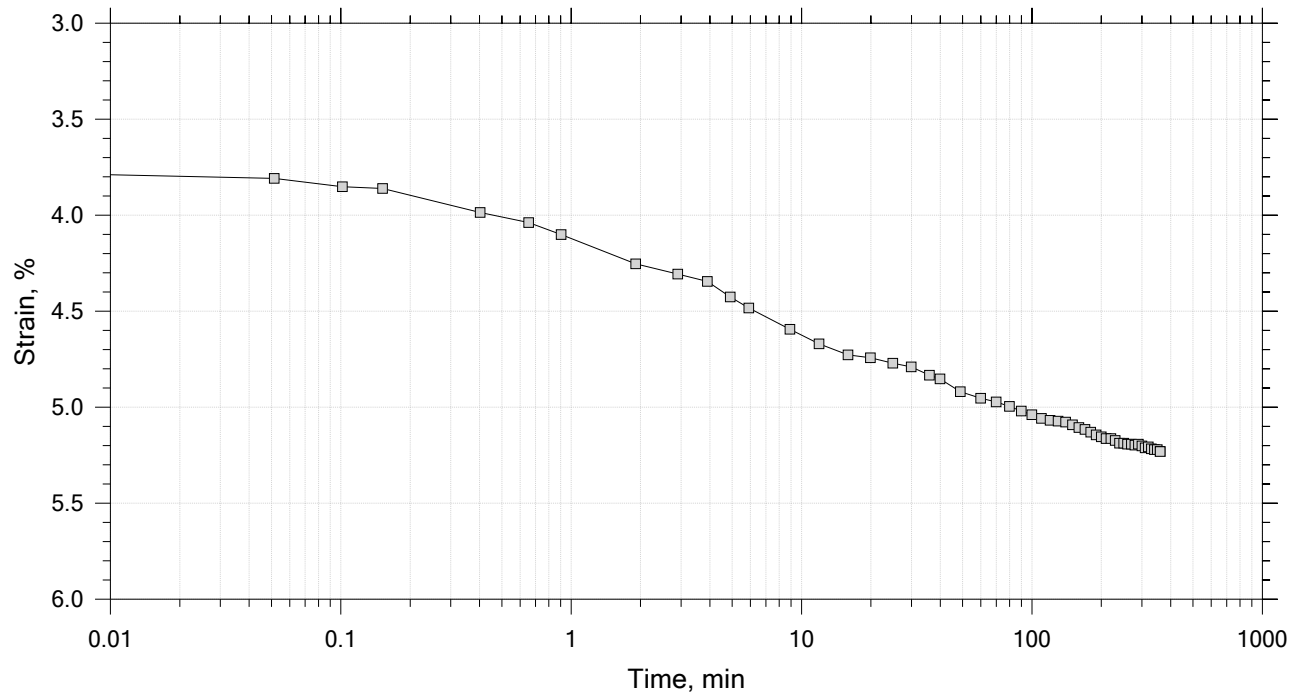
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



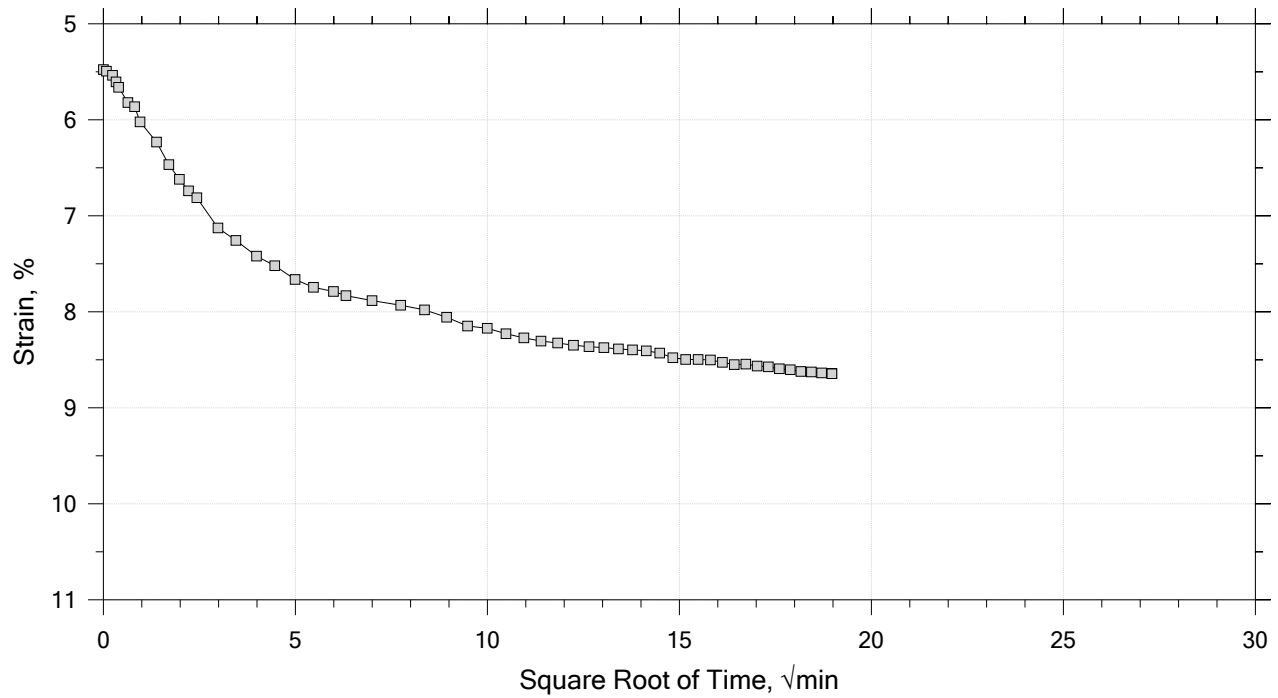
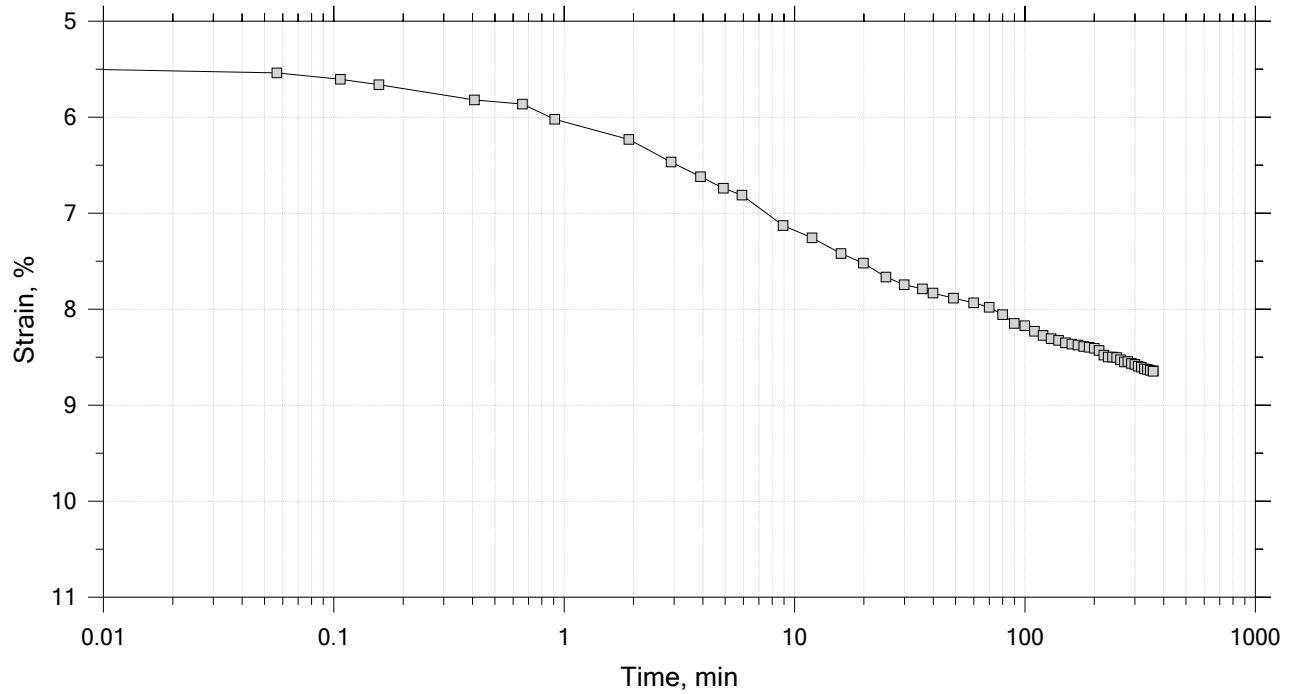
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



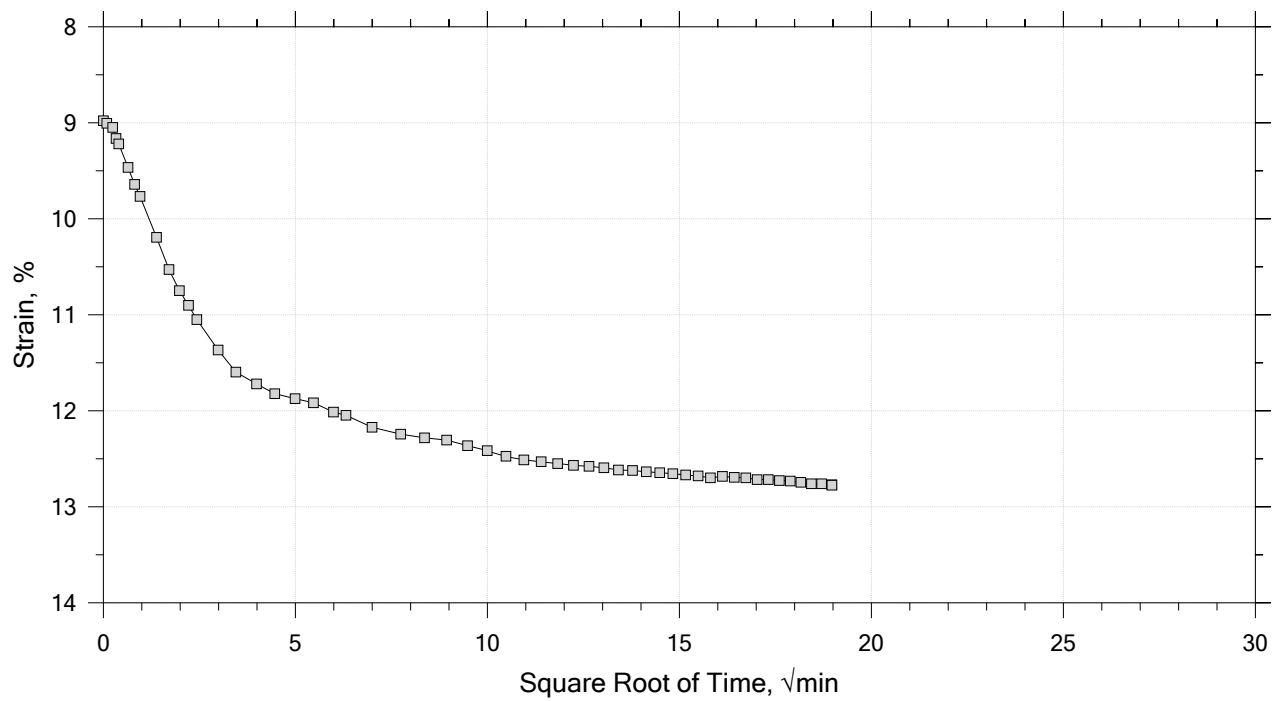
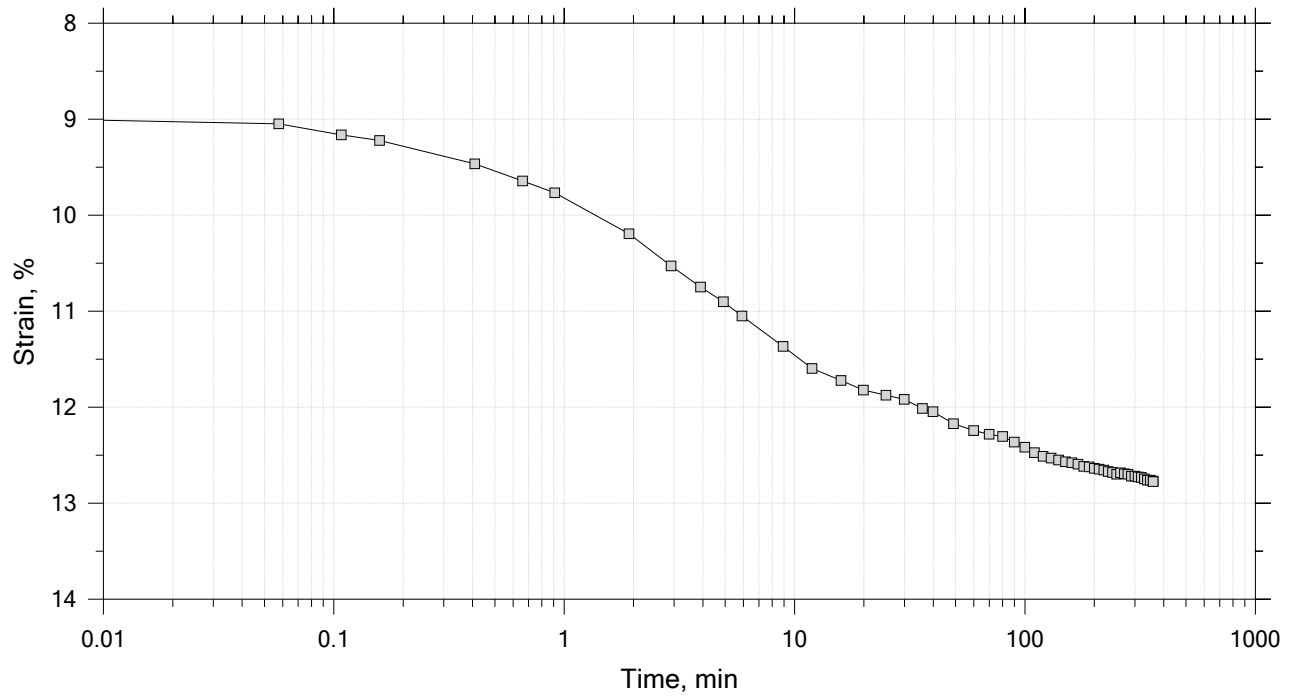
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



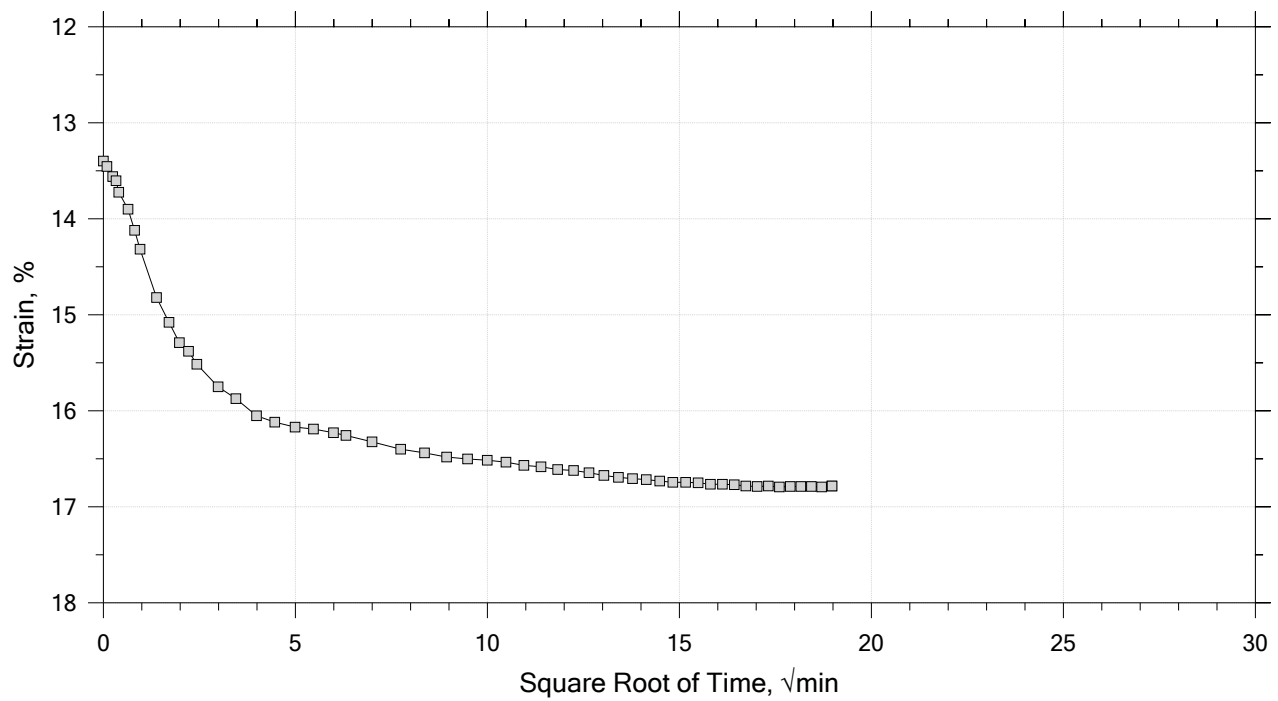
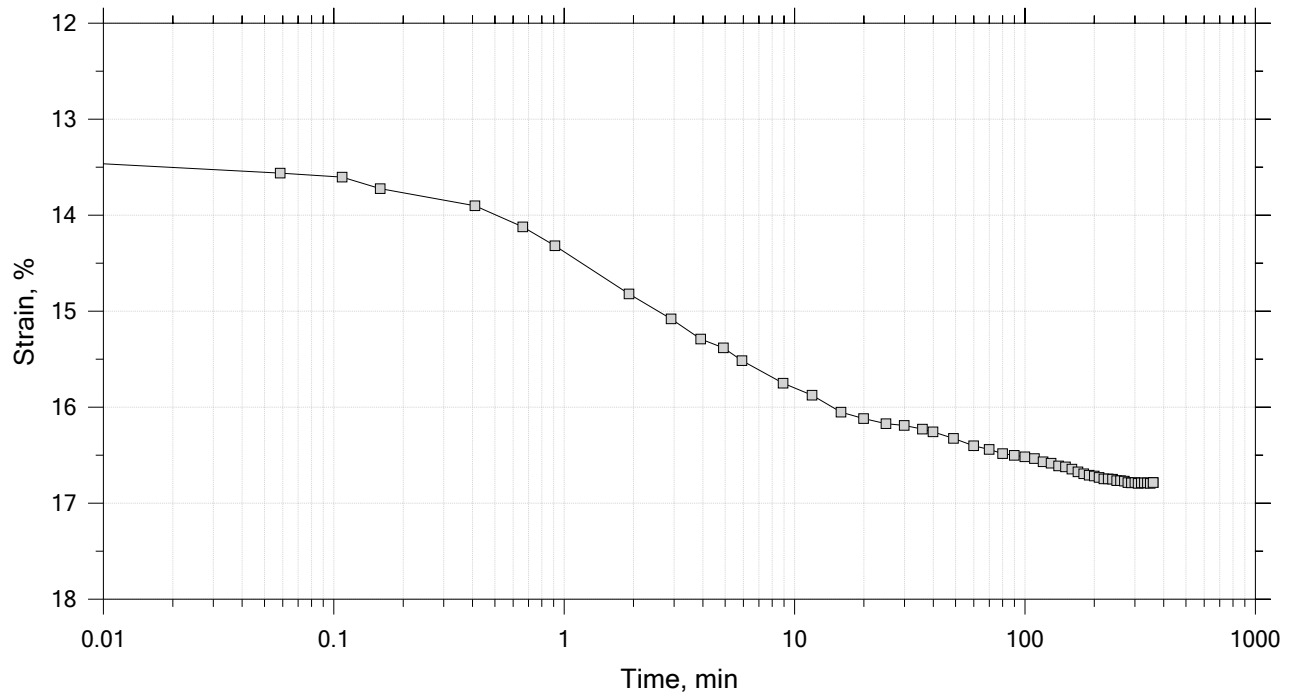
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



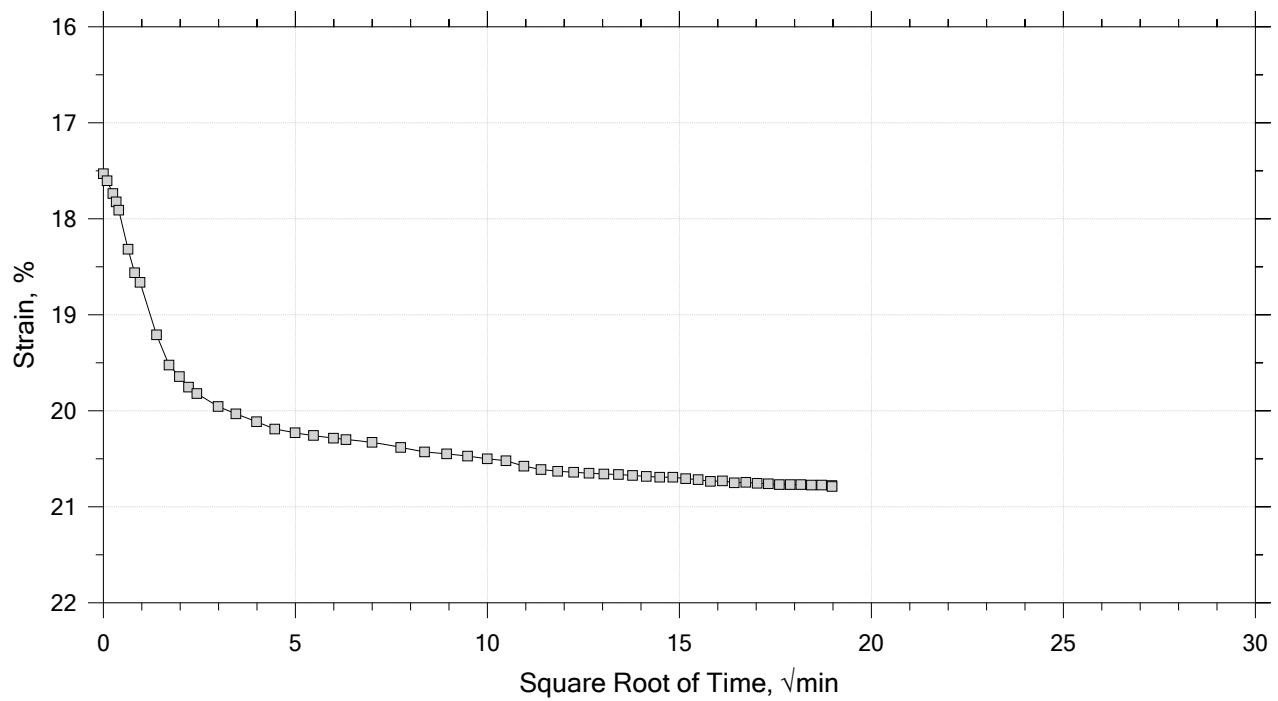
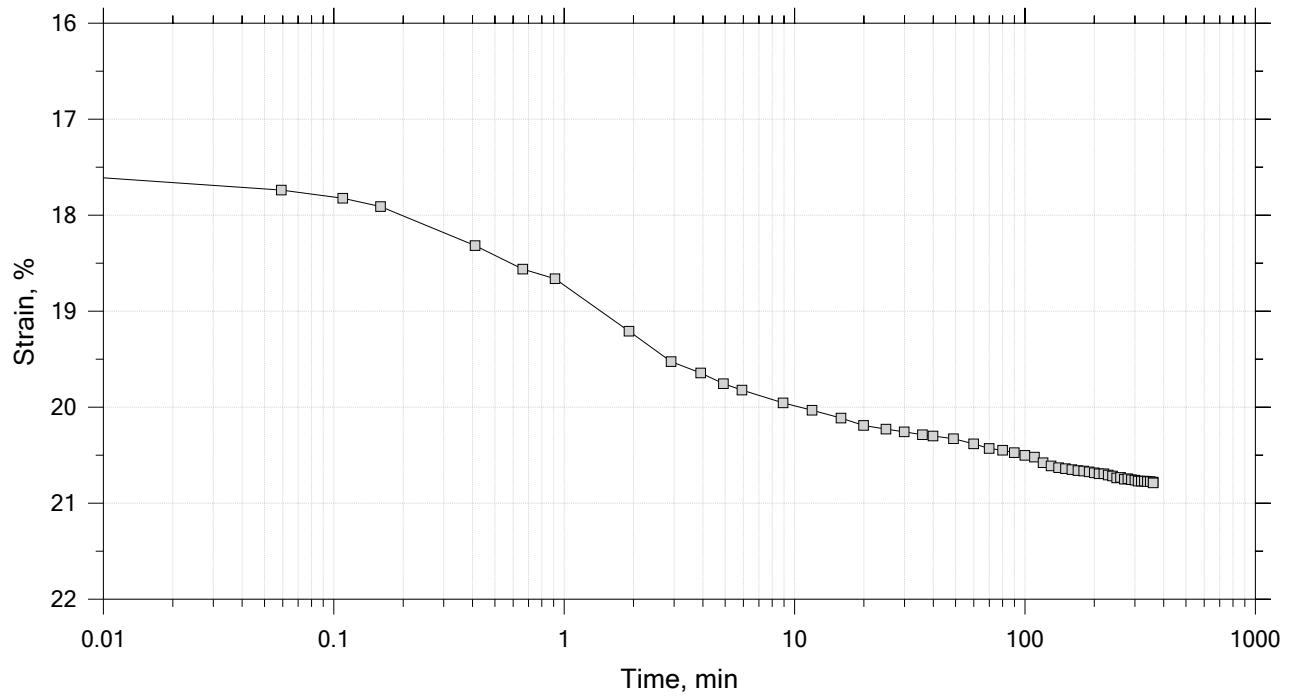
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



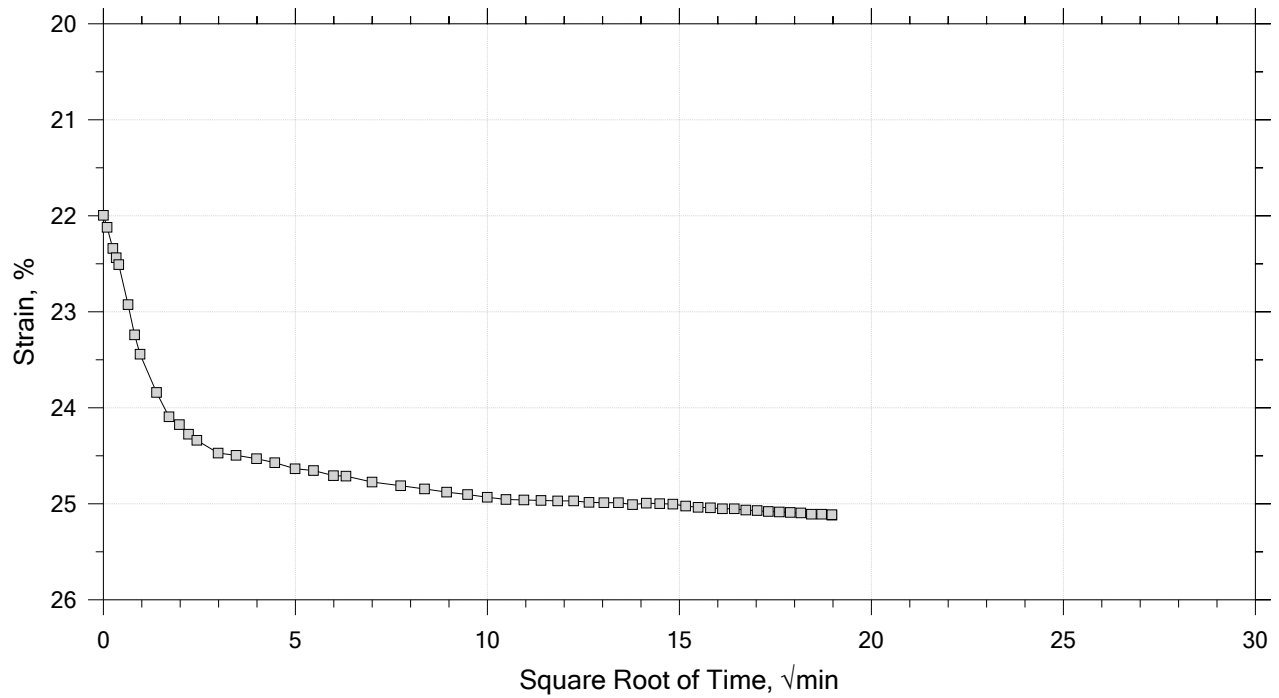
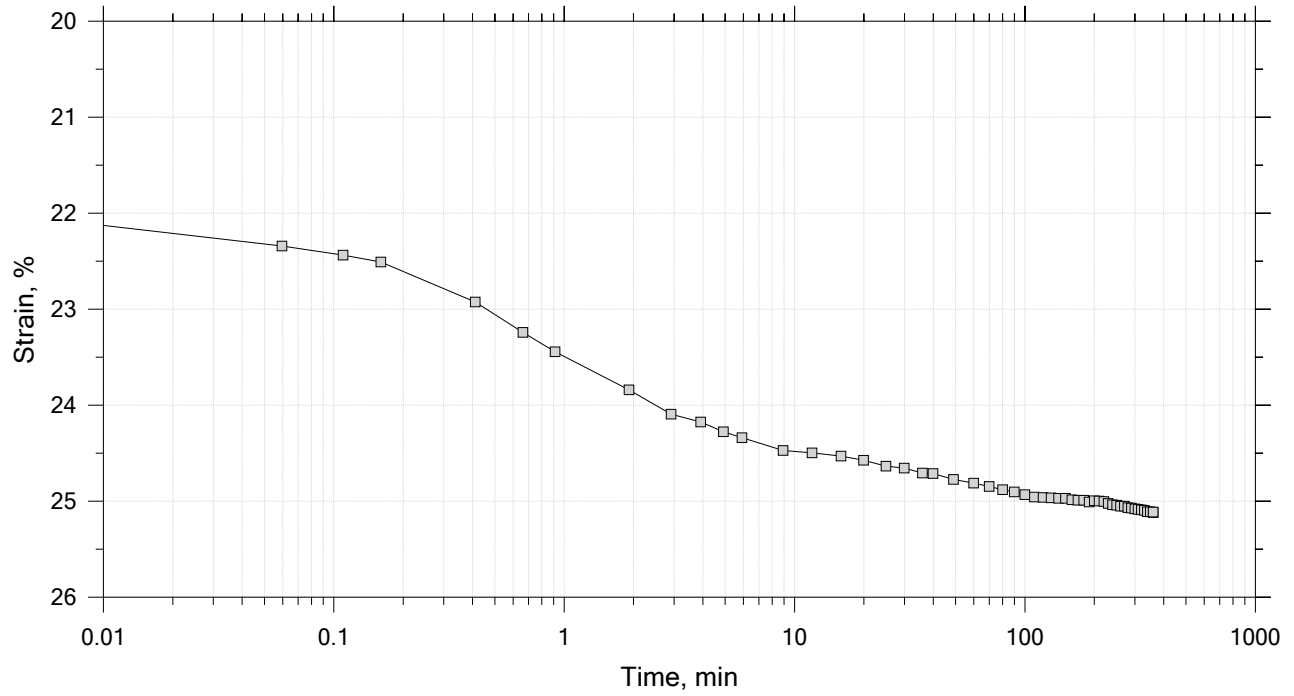
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



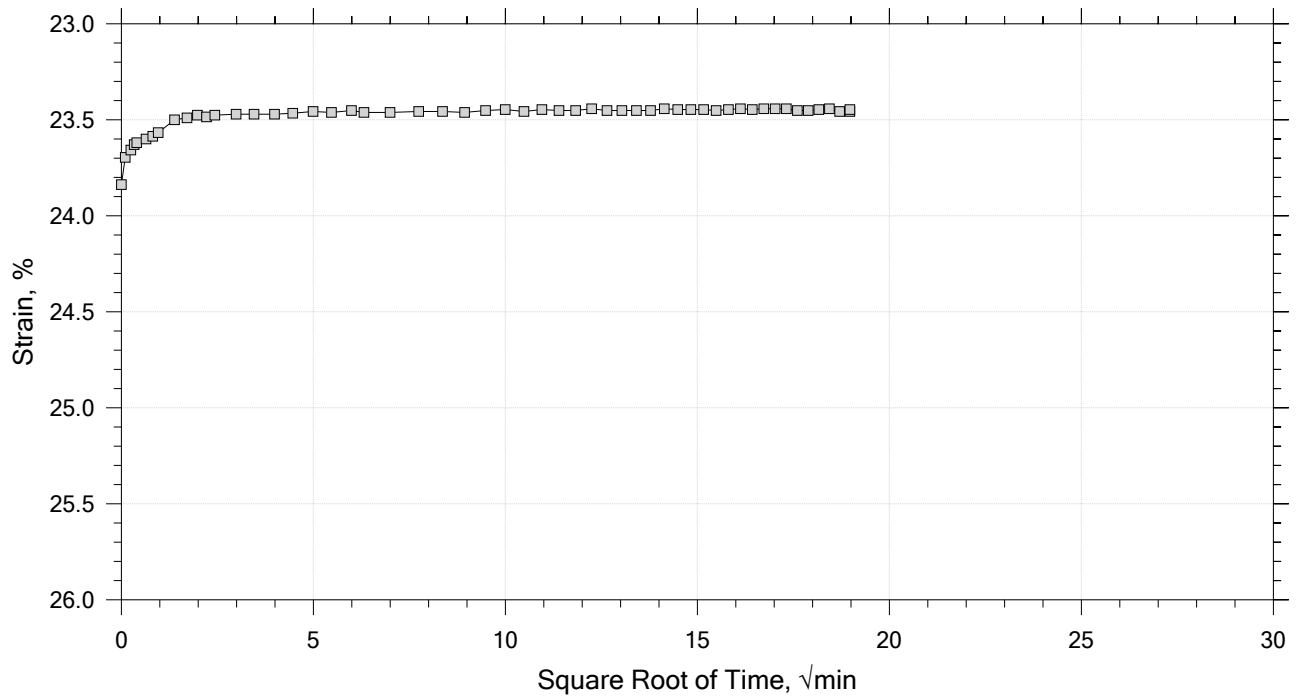
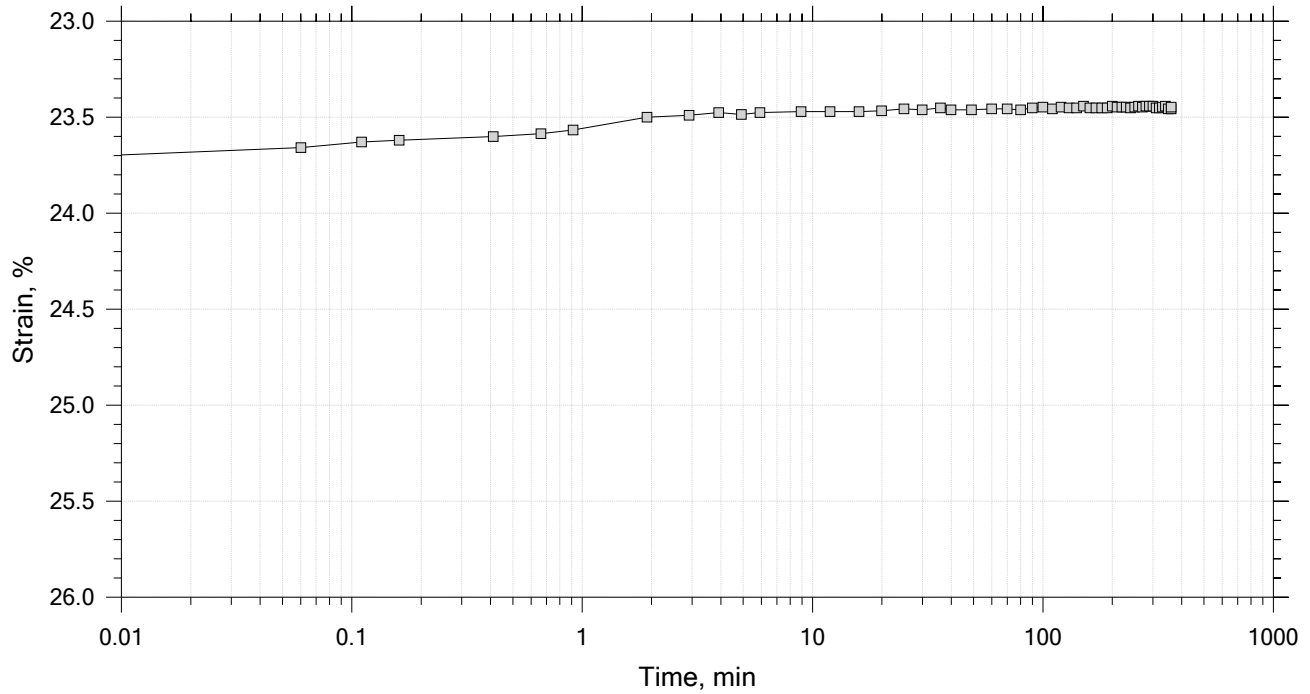
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



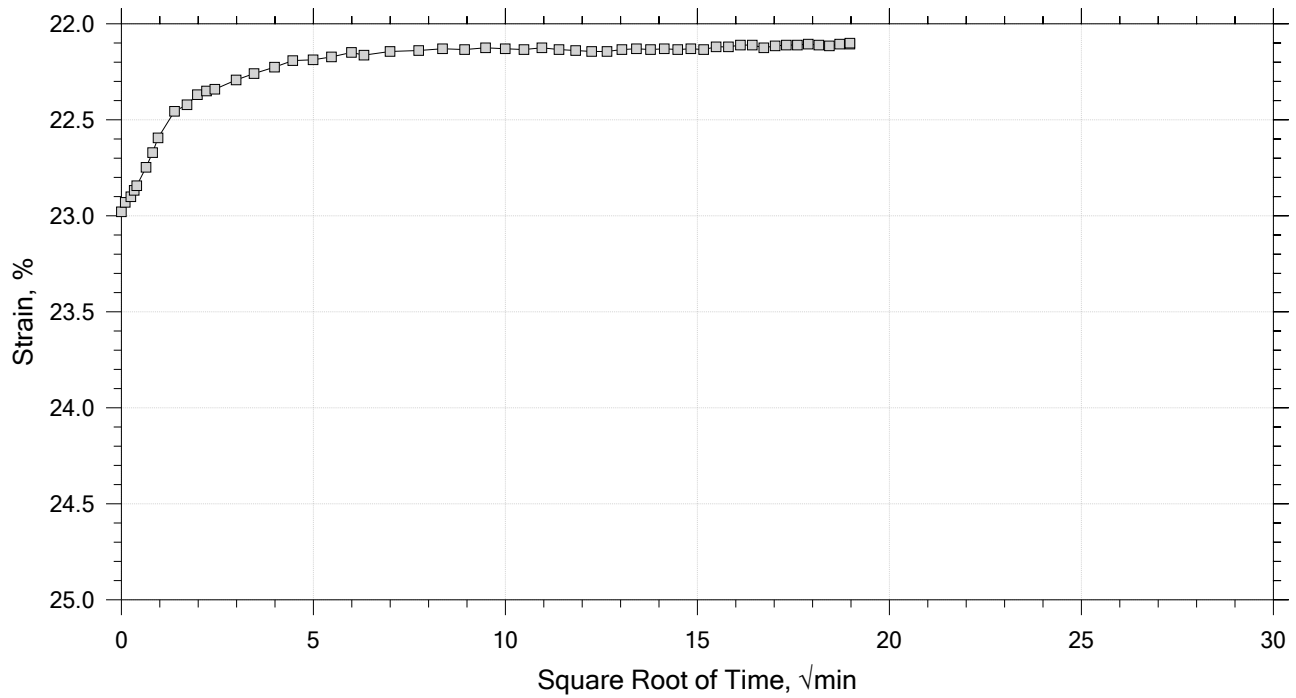
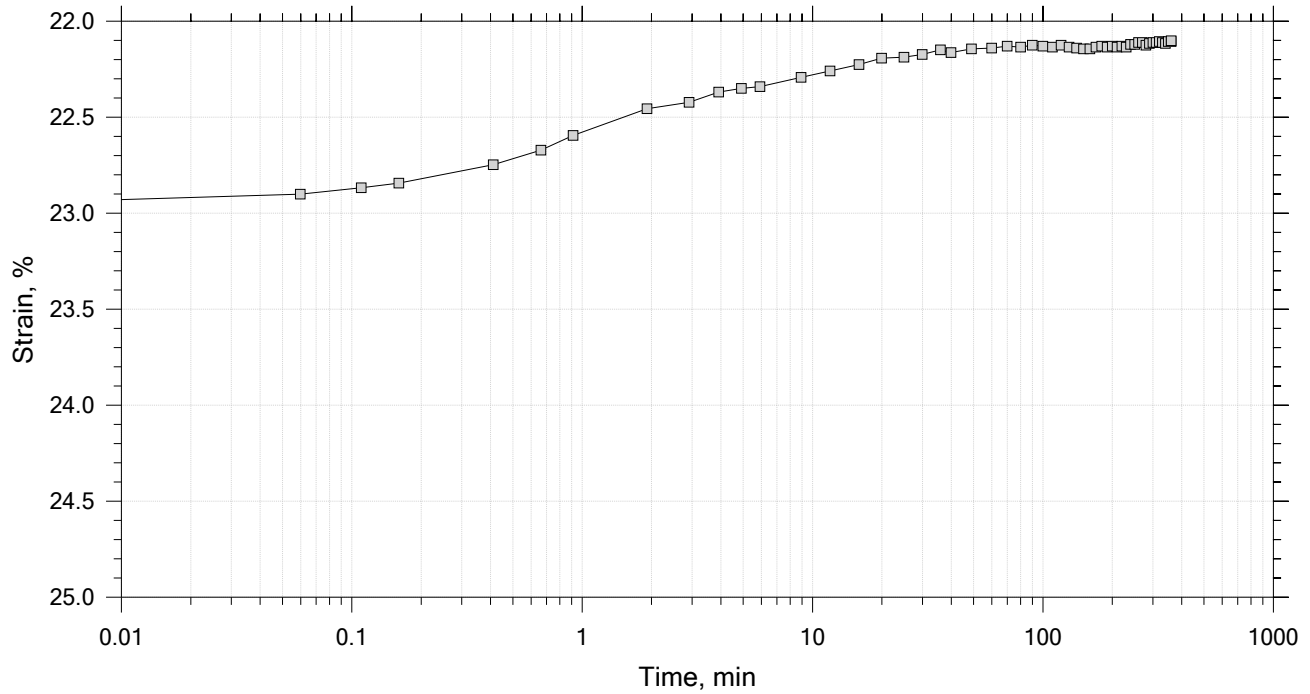
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



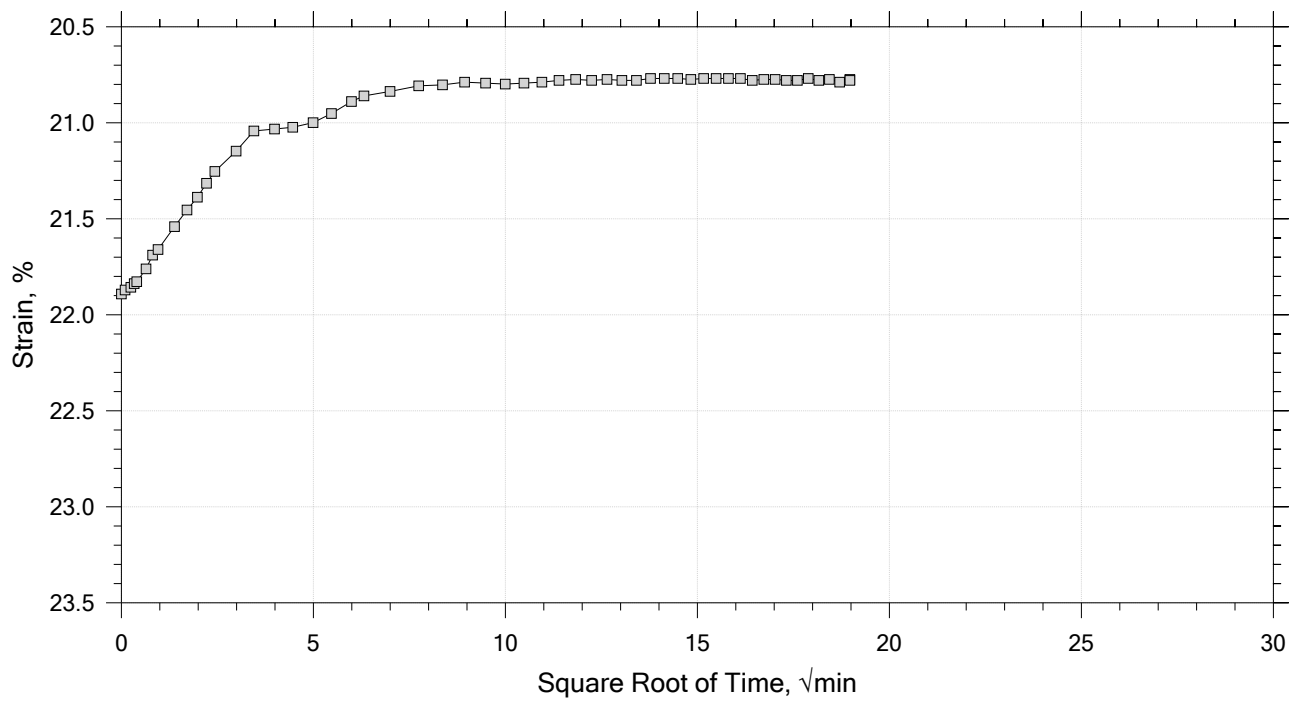
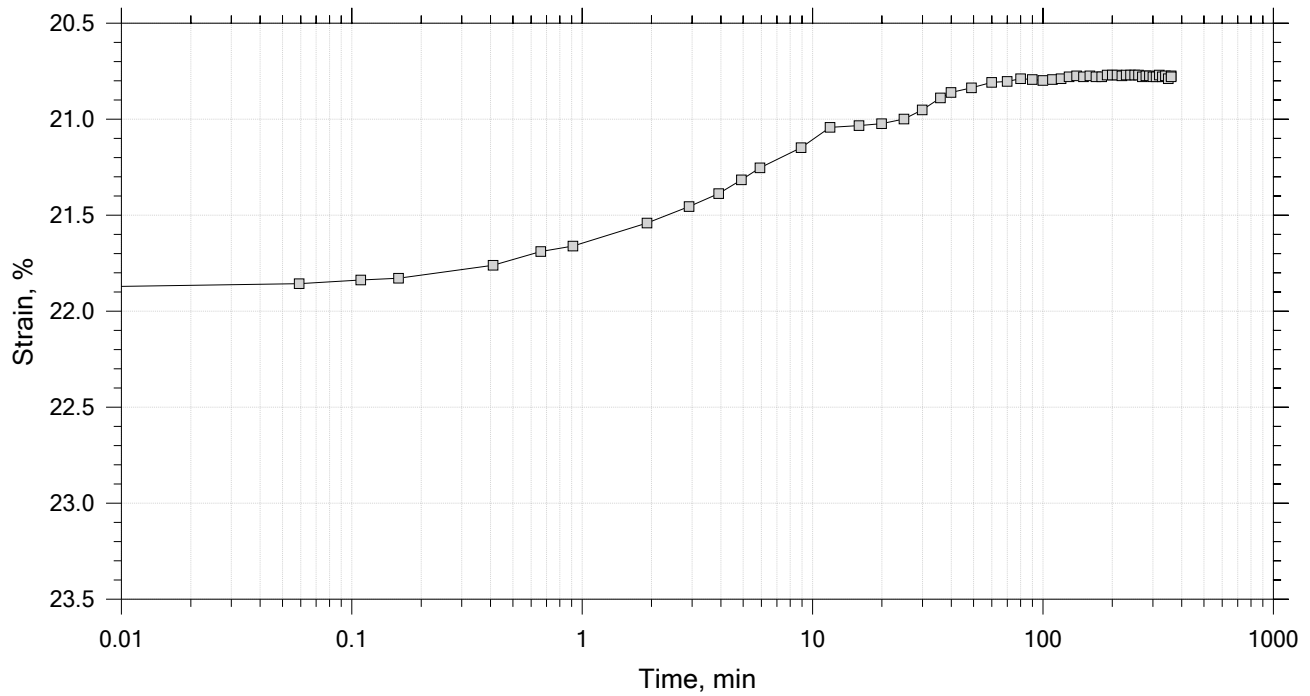
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



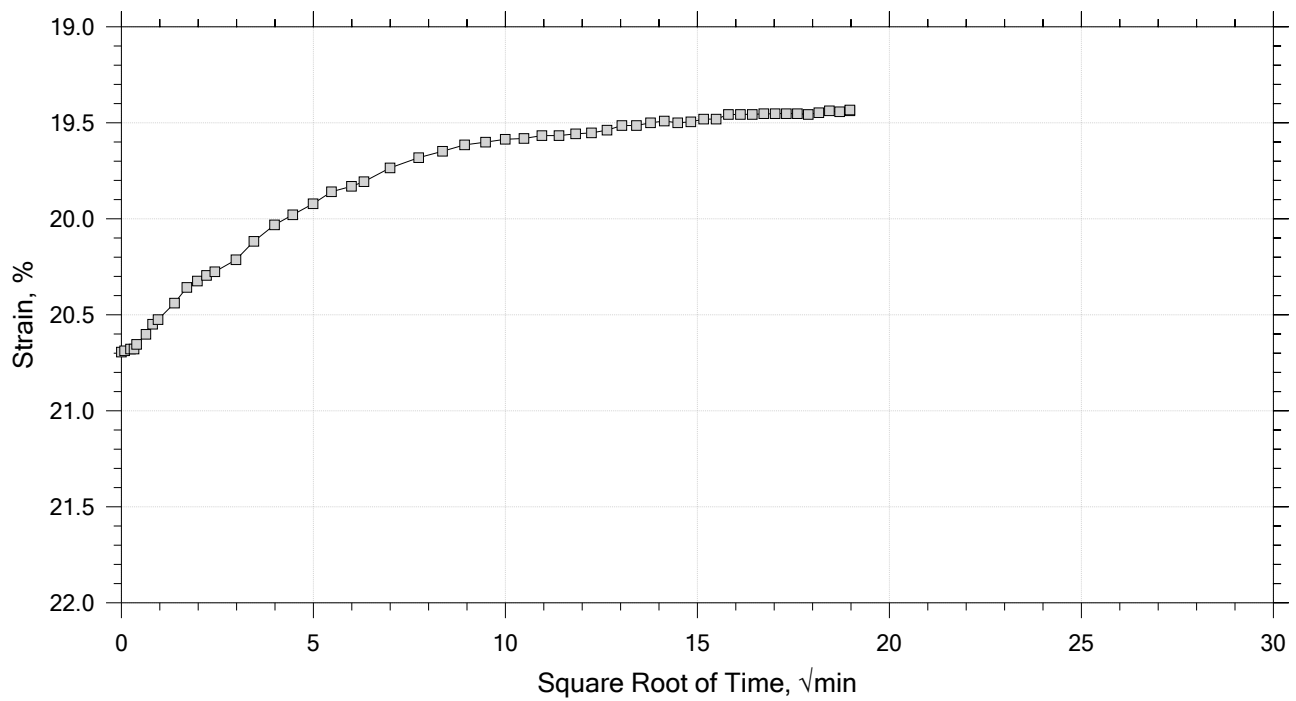
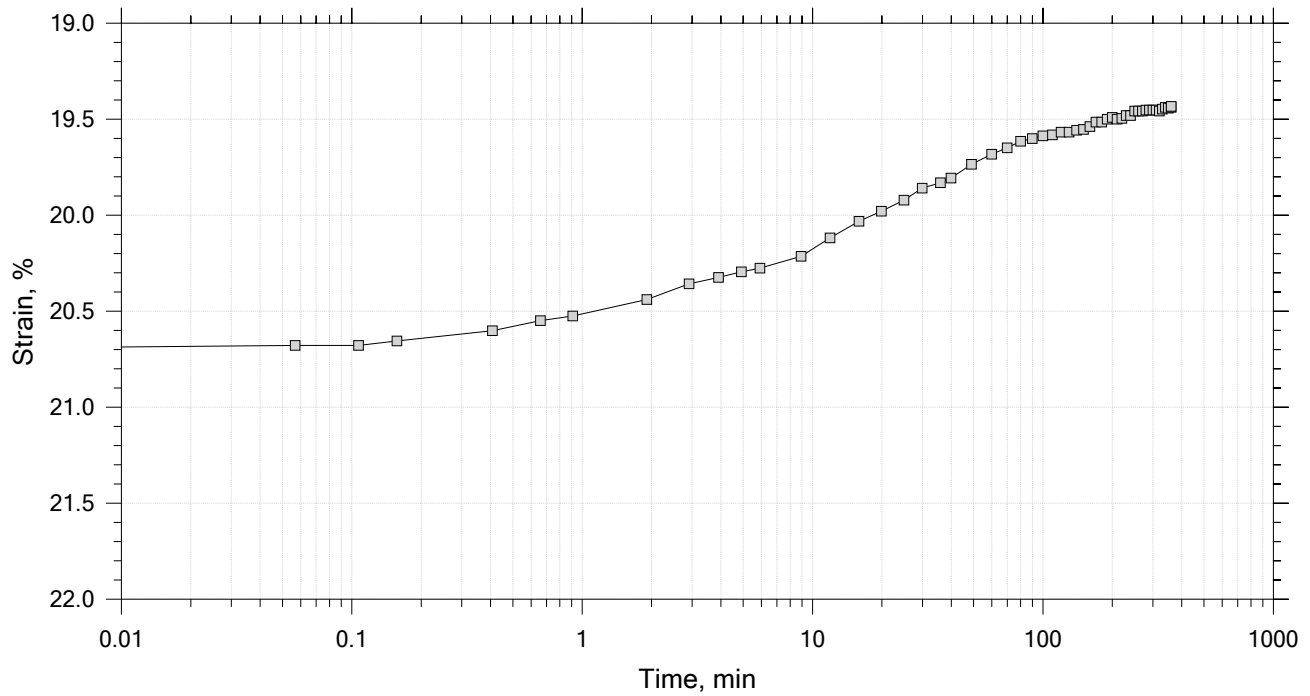
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



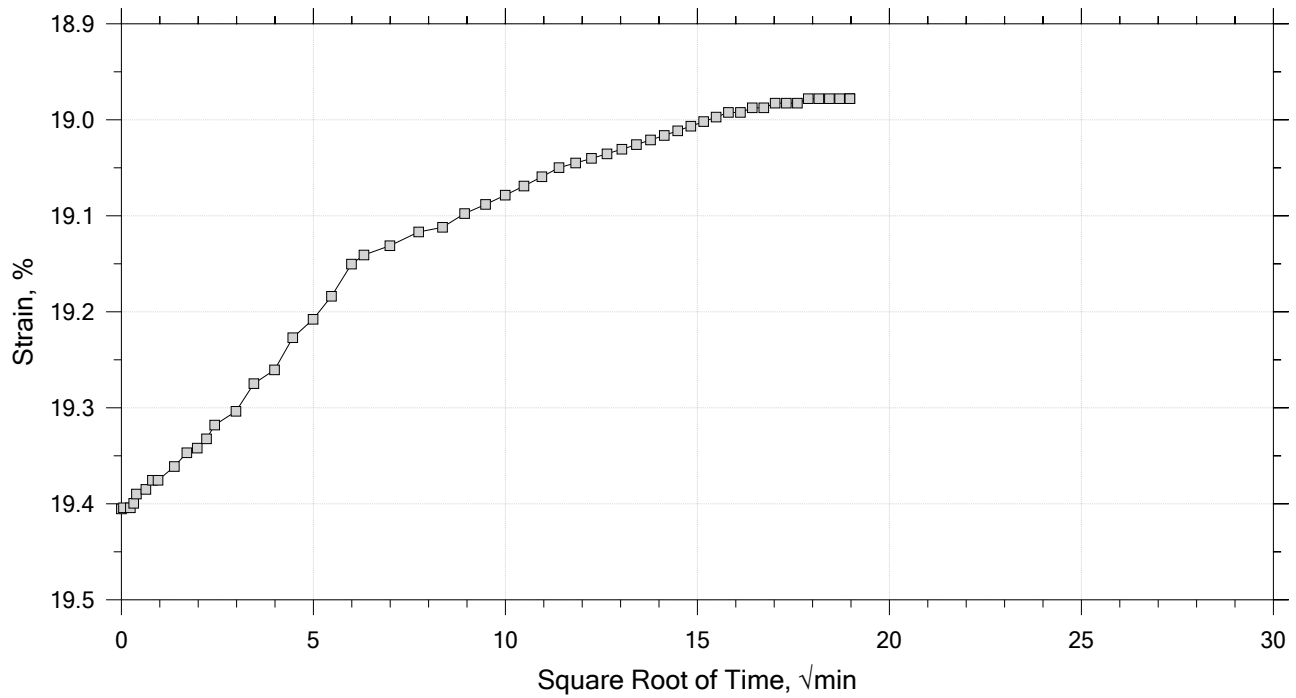
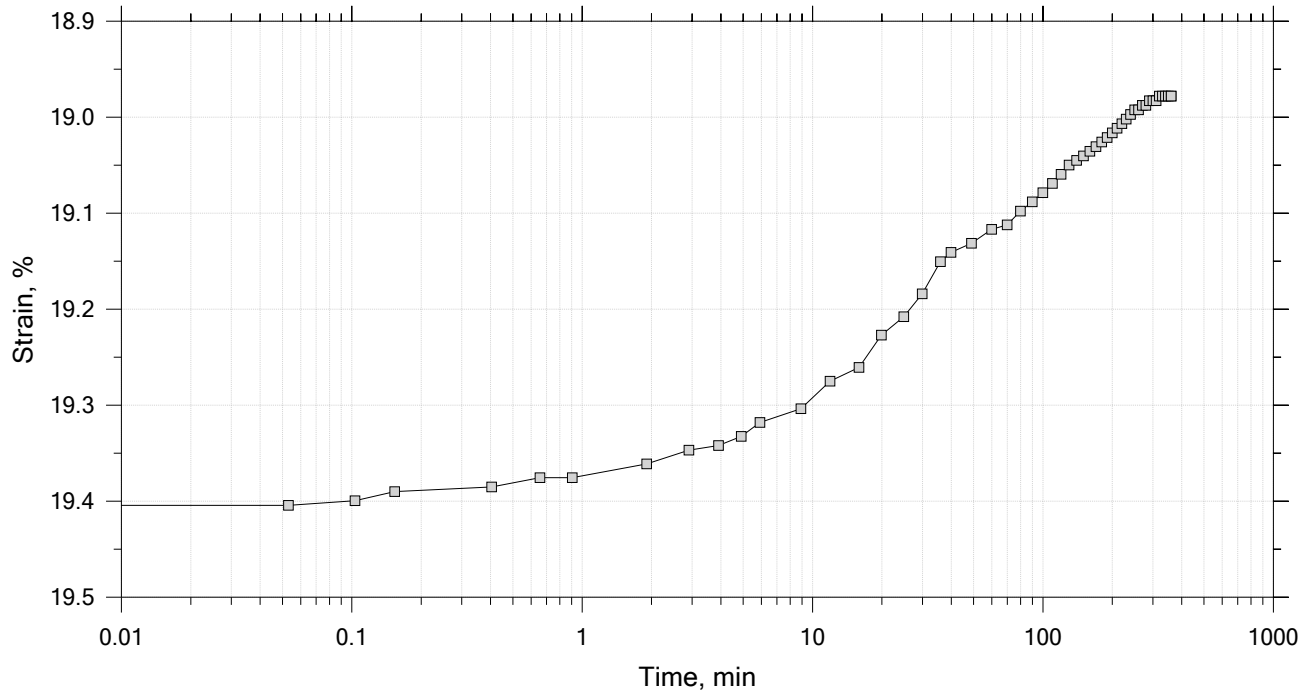
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	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.41 in	Estimated Specific Gravity: 2.75	Liquid Limit: 38
Initial Height: 1.00 in	Initial Void Ratio: 0.887	Plastic Limit: 2
Final Height: 0.82 in	Final Void Ratio: 0.555	Plasticity Index: 36

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A-1859	RING		e1401
Mass Container, gm	8.22	107.7	107.7	8.1
Mass Container + Wet Soil, gm	127.33	250.97	238.61	138.83
Mass Container + Dry Soil, gm	98.65	216.62	216.62	116.87
Mass Dry Soil, gm	90.43	108.92	108.92	108.77
Water Content, %	31.72	31.54	20.19	20.19
Void Ratio	---	0.89	0.56	---
Degree of Saturation, %	---	97.73	100.00	---
Dry Unit Weight, pcf	---	90.962	110.39	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

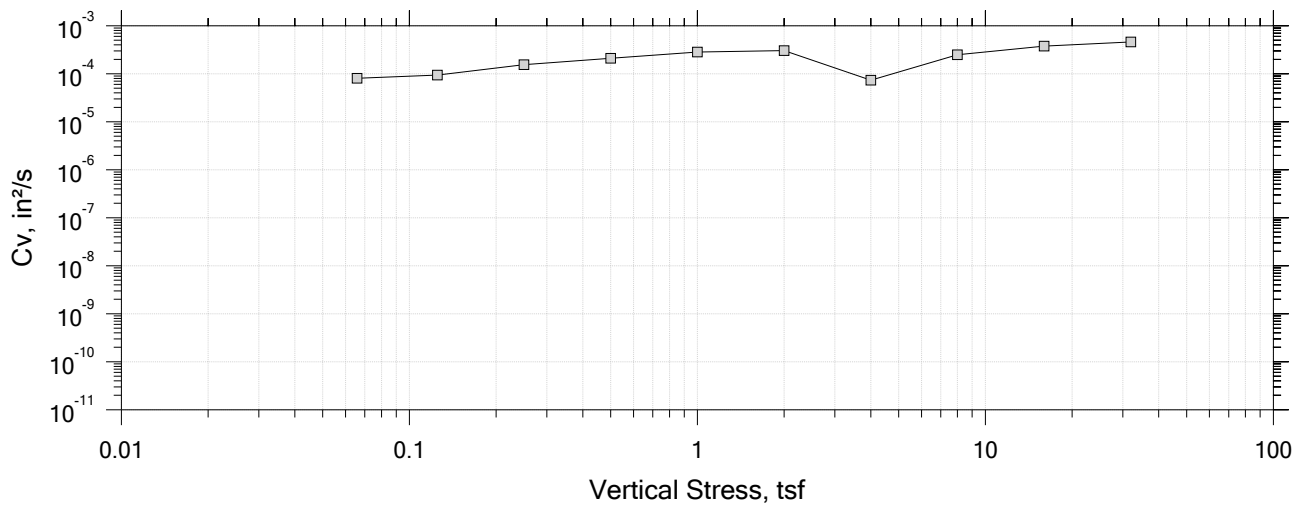
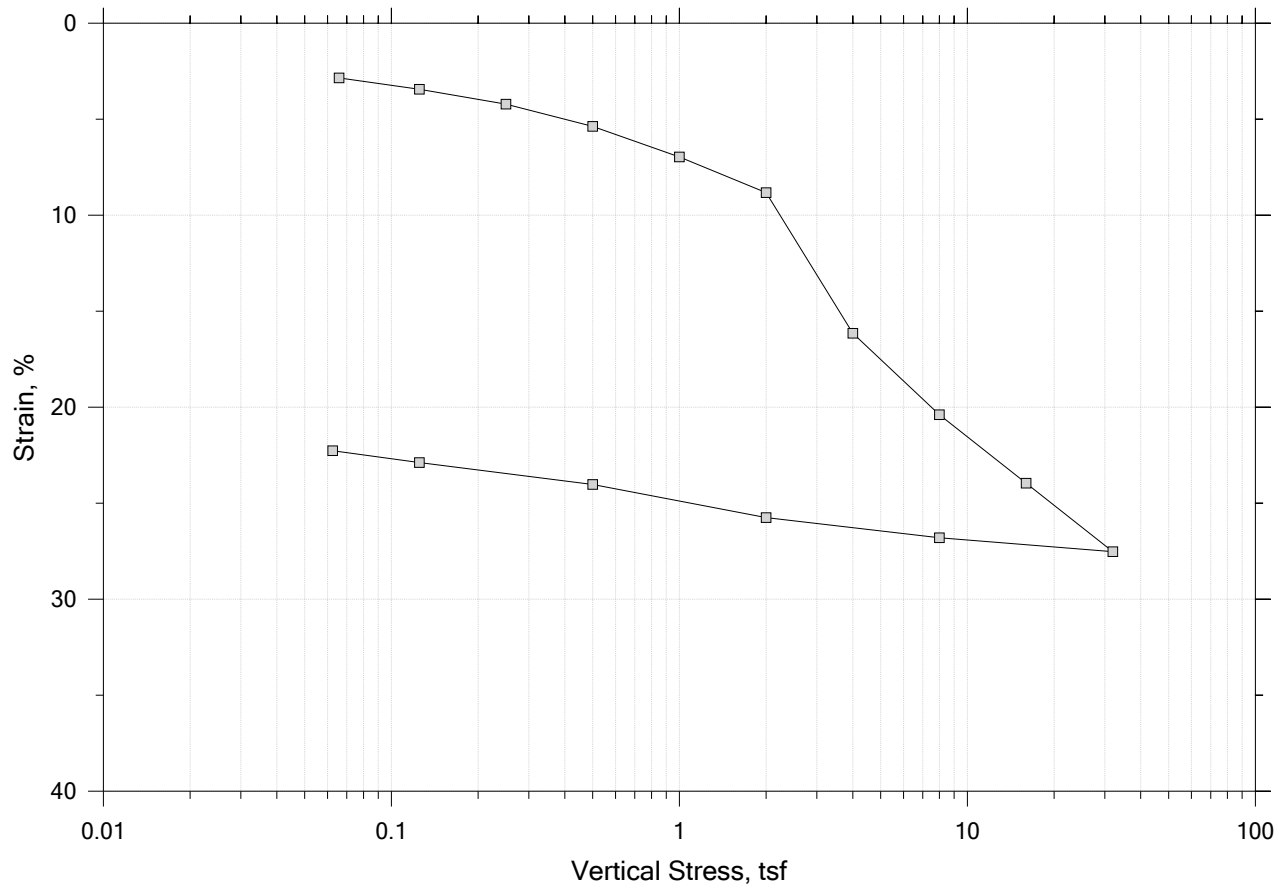
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BFB1-204	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 02/23/21	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist gray clay		
	Remarks: System TT, Swell Pressure = 0.0671 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

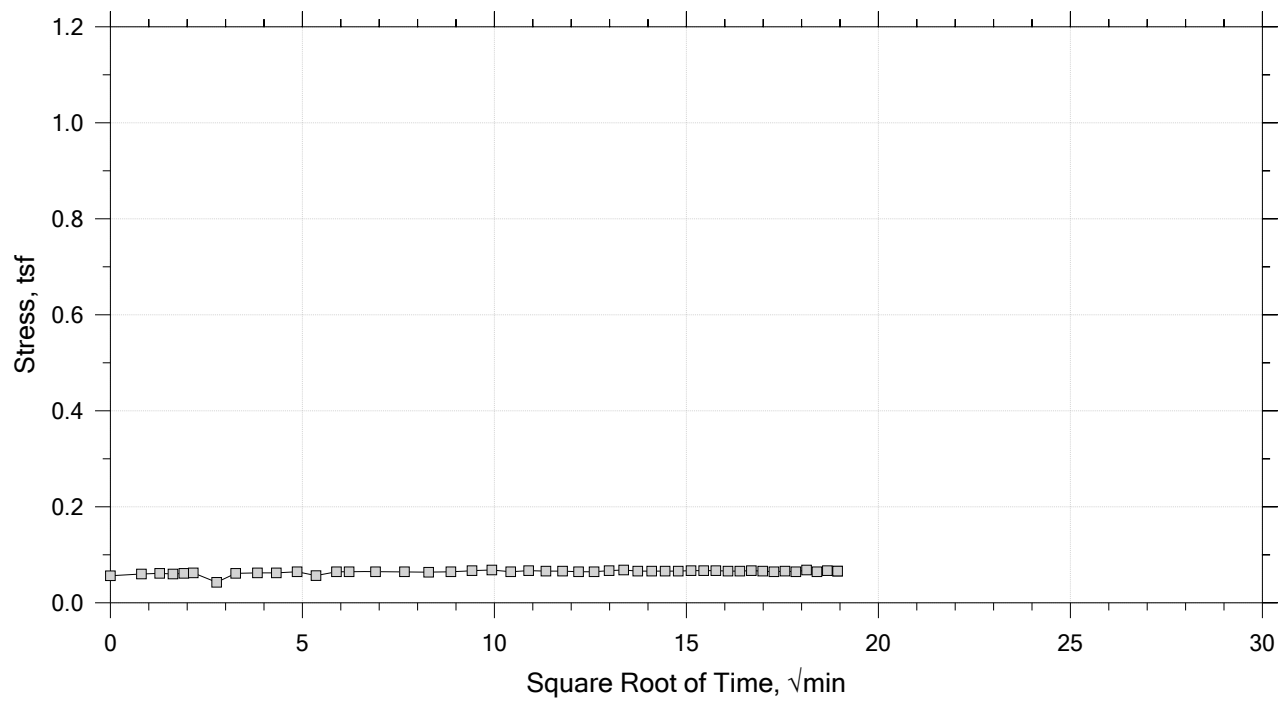
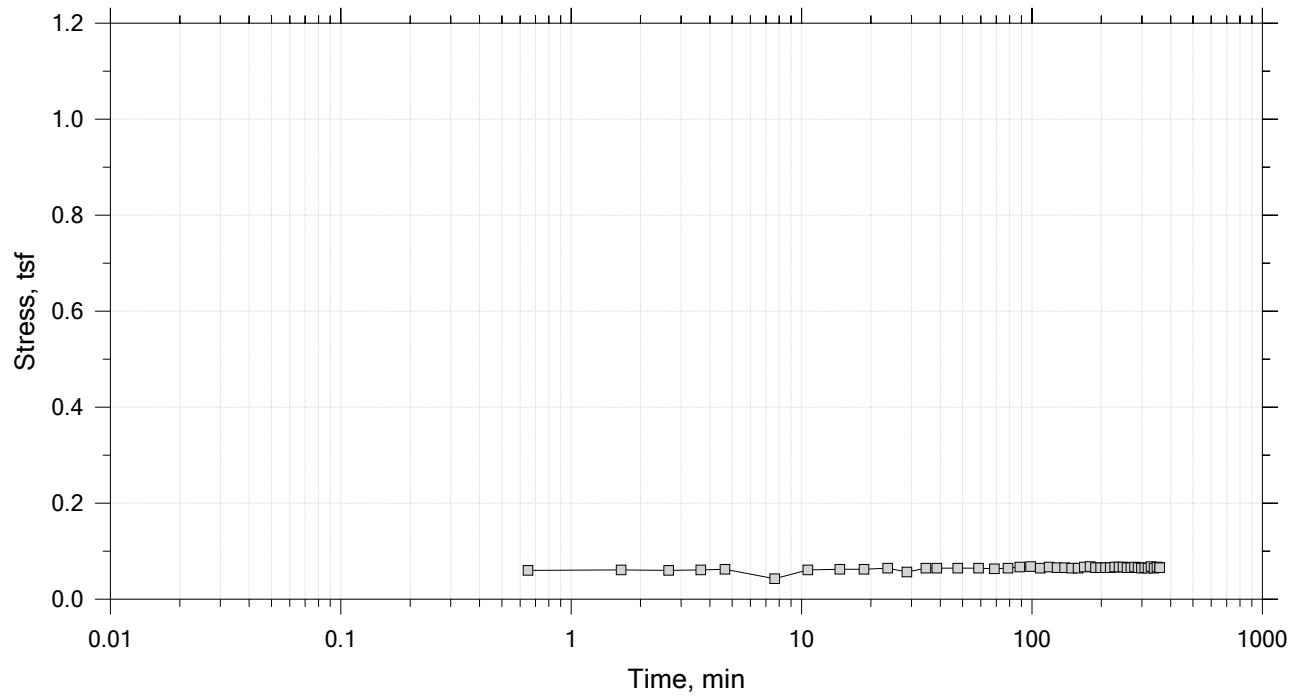
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0658 tsf



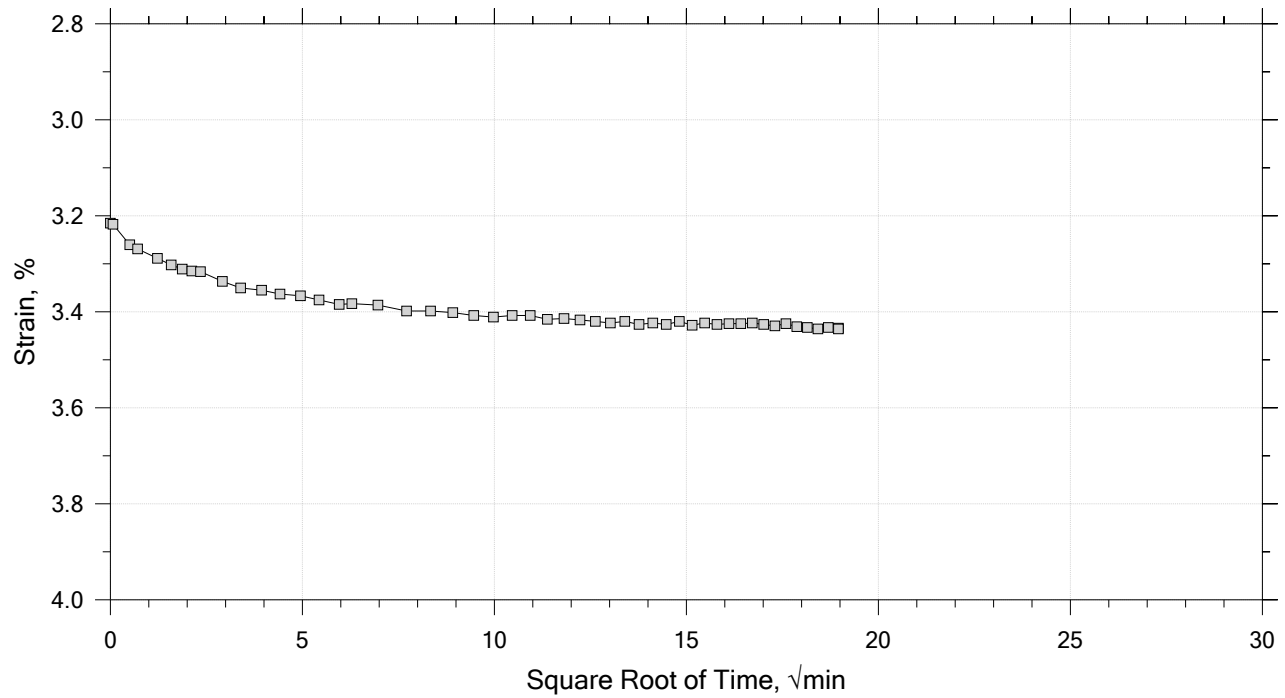
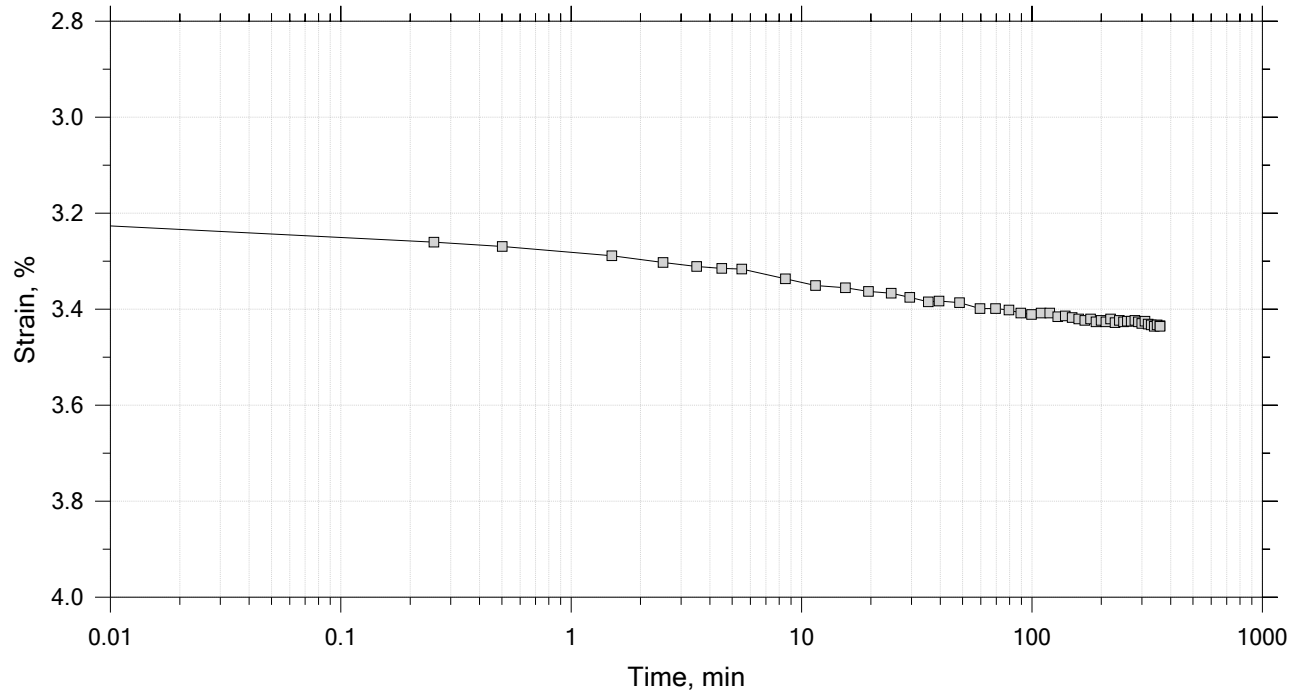
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



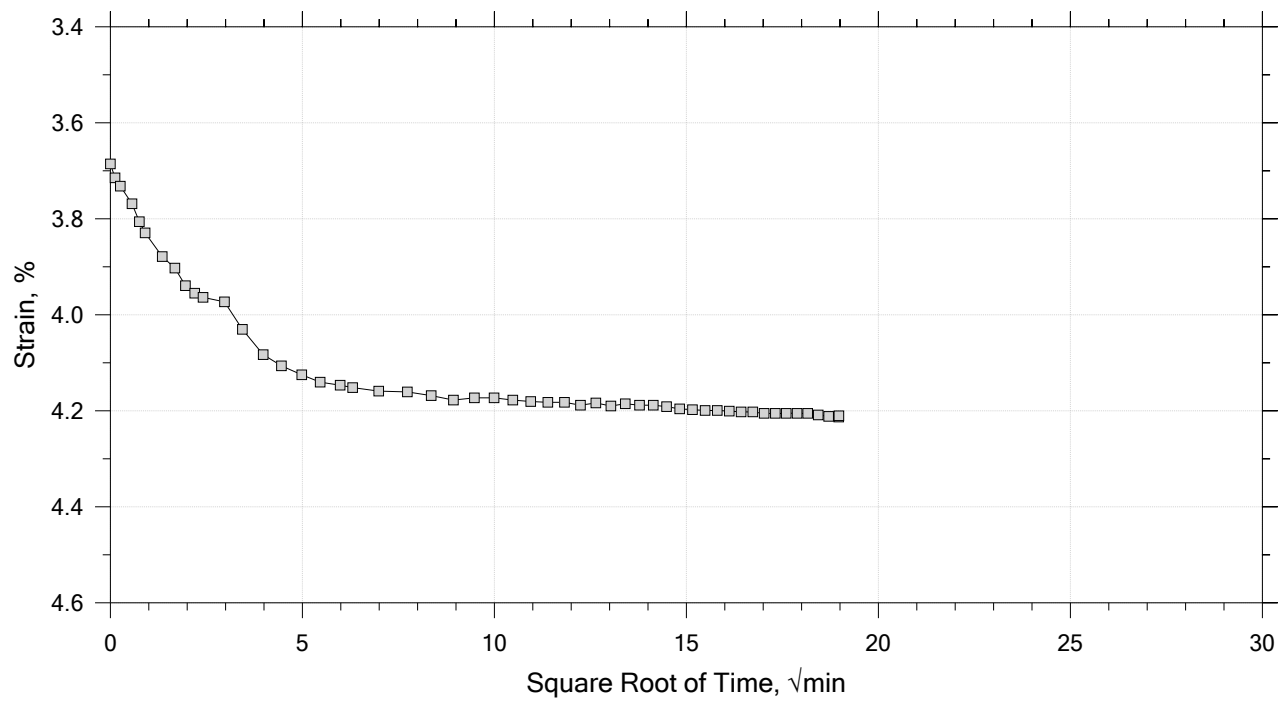
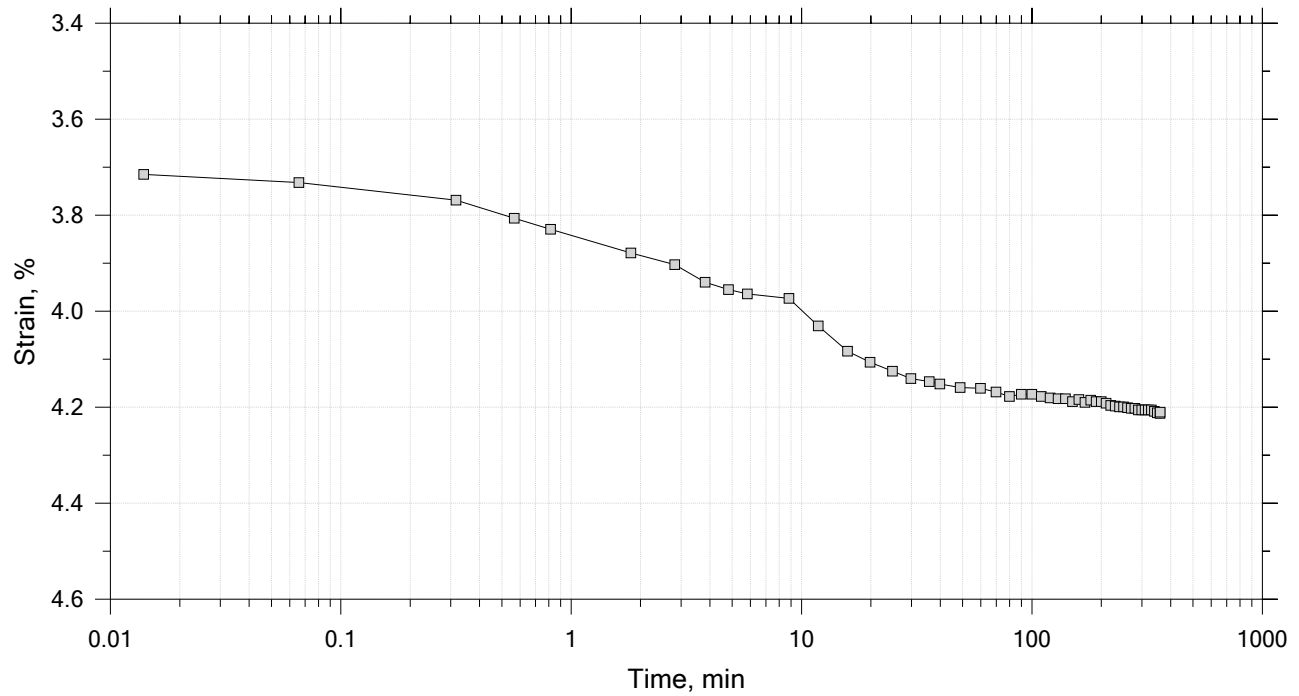
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



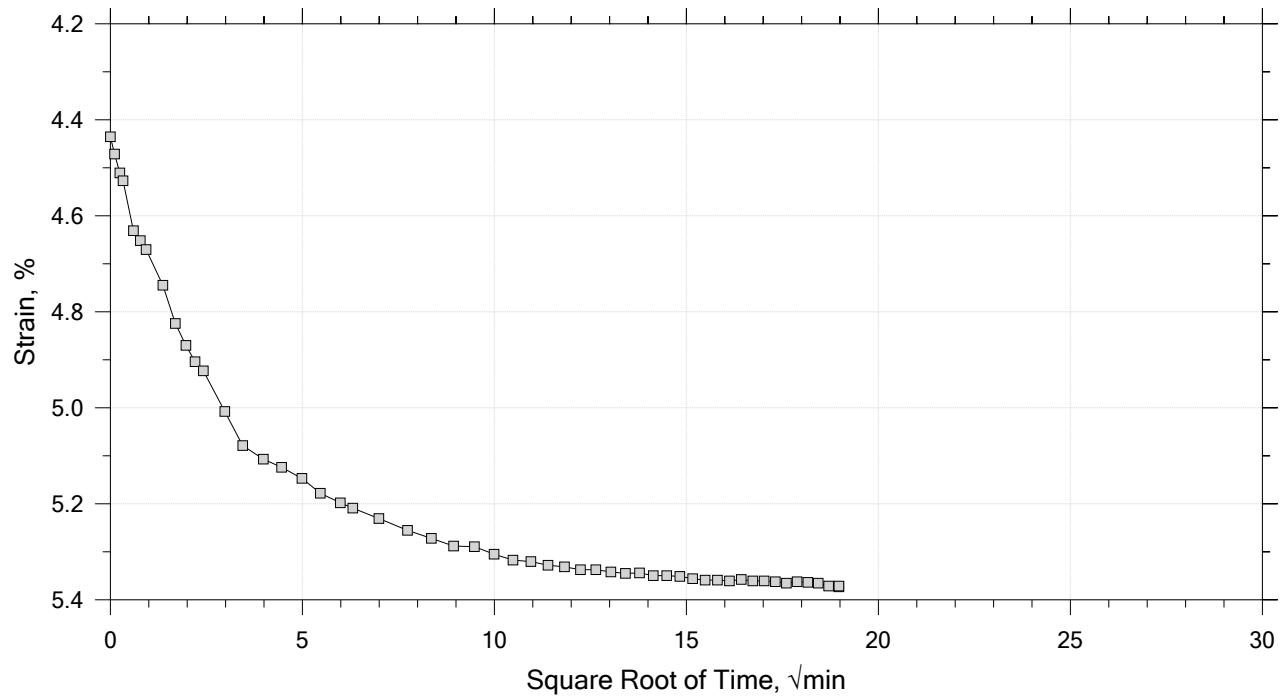
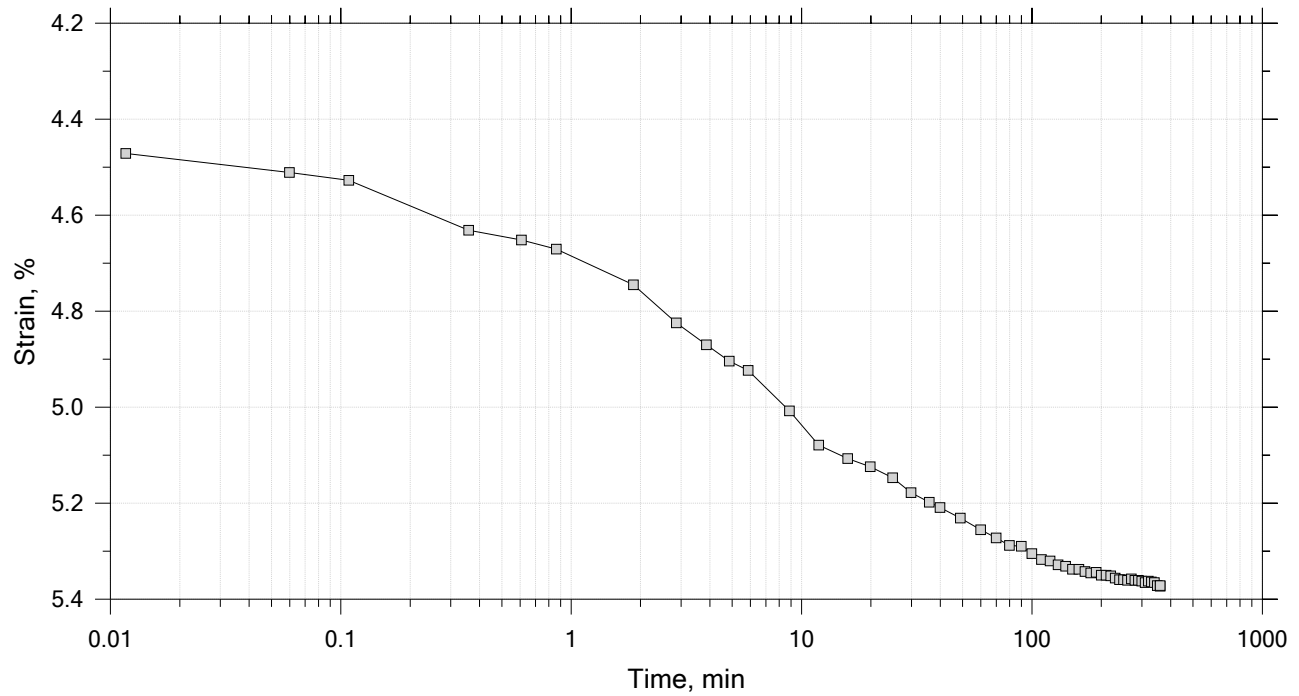
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



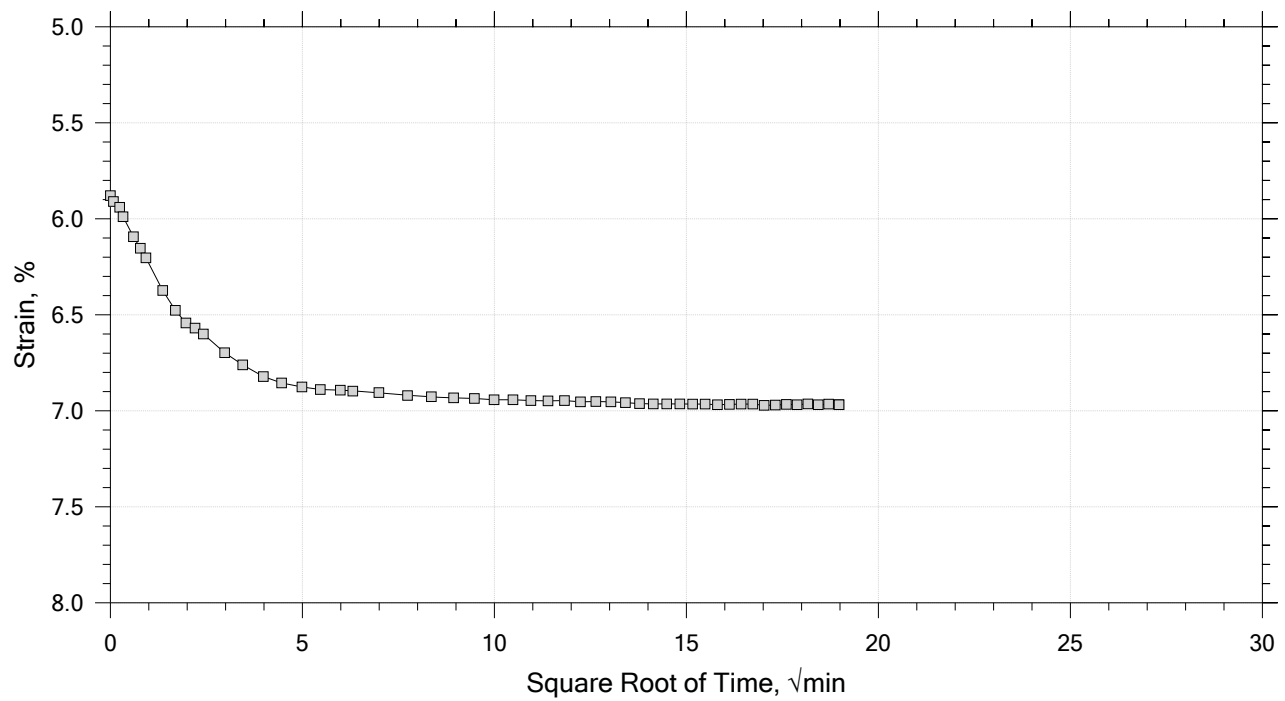
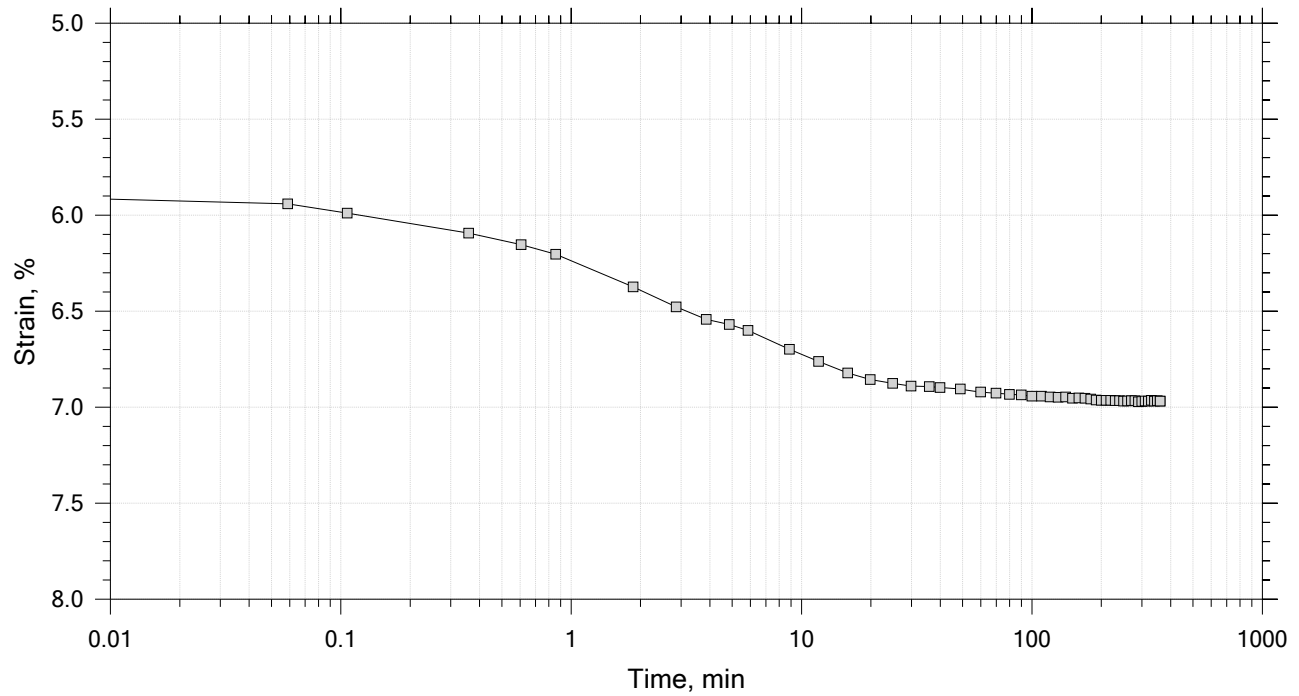
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



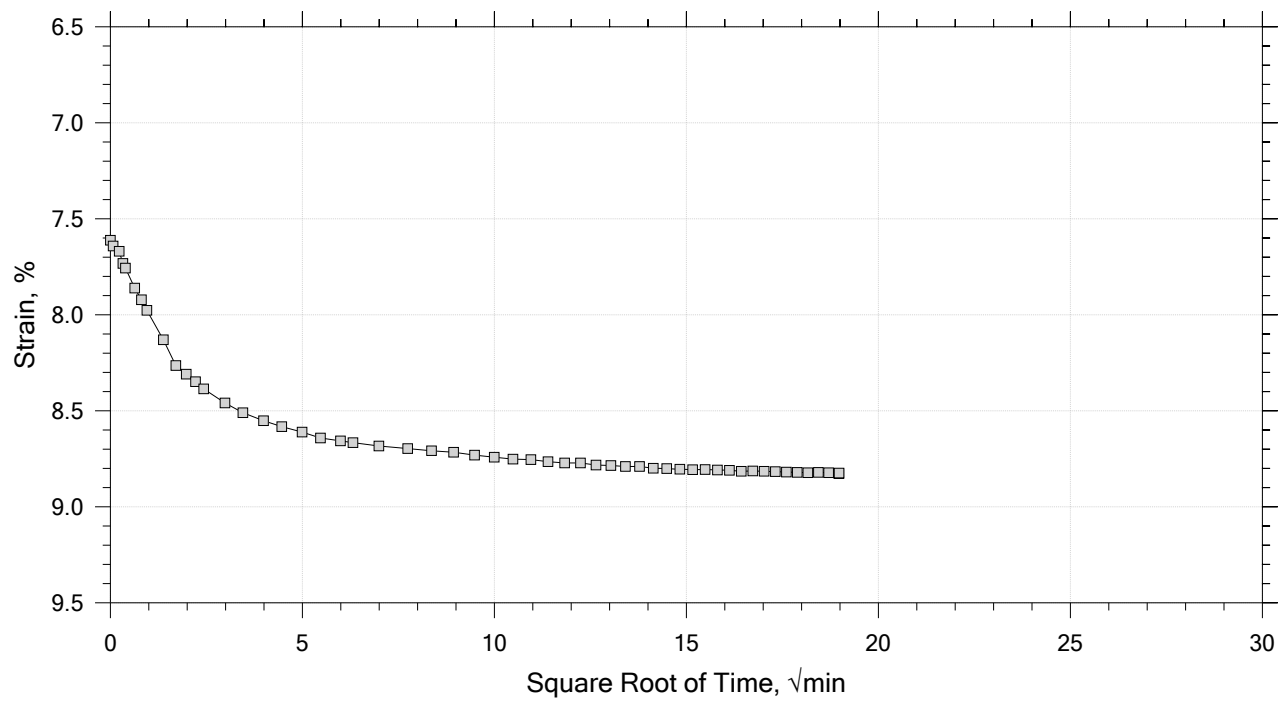
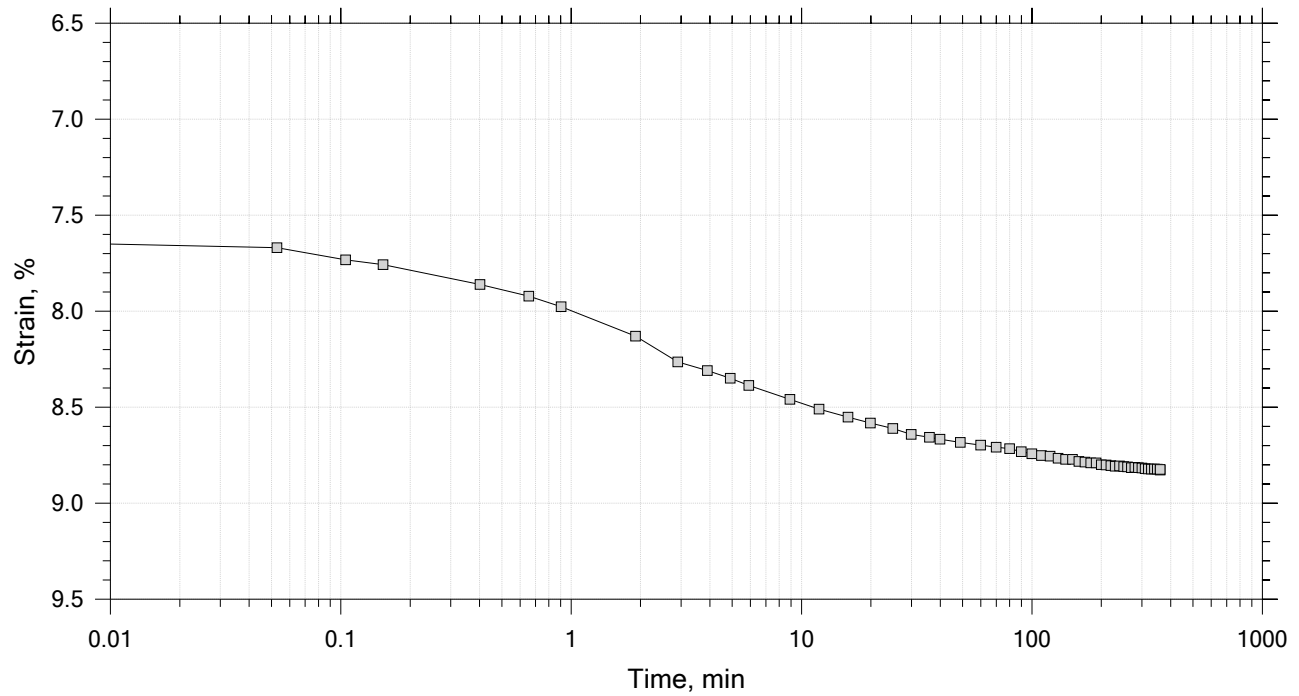
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



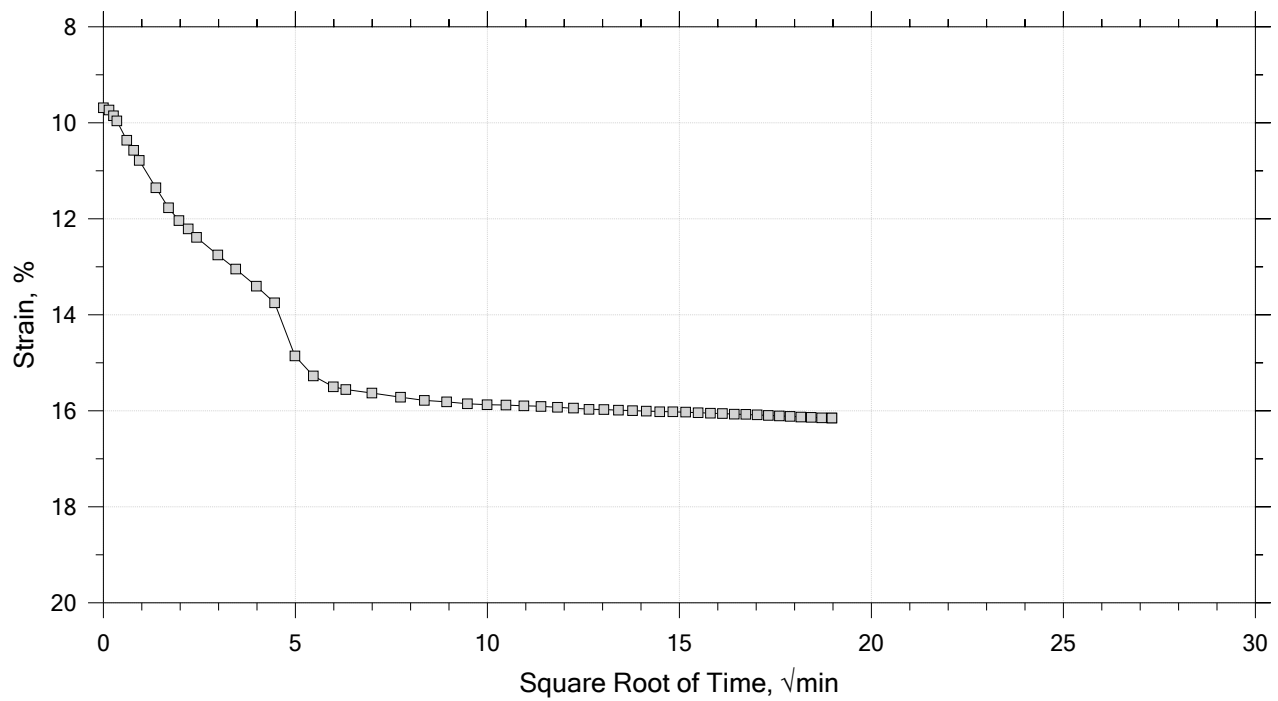
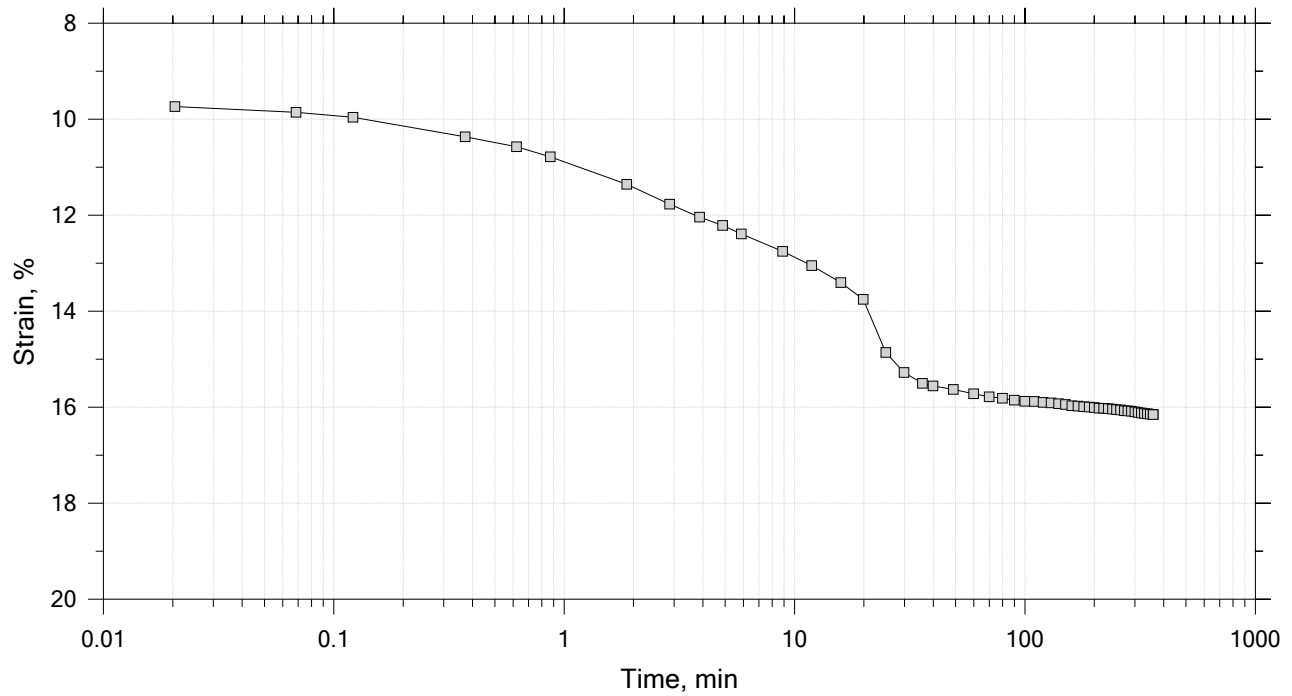
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



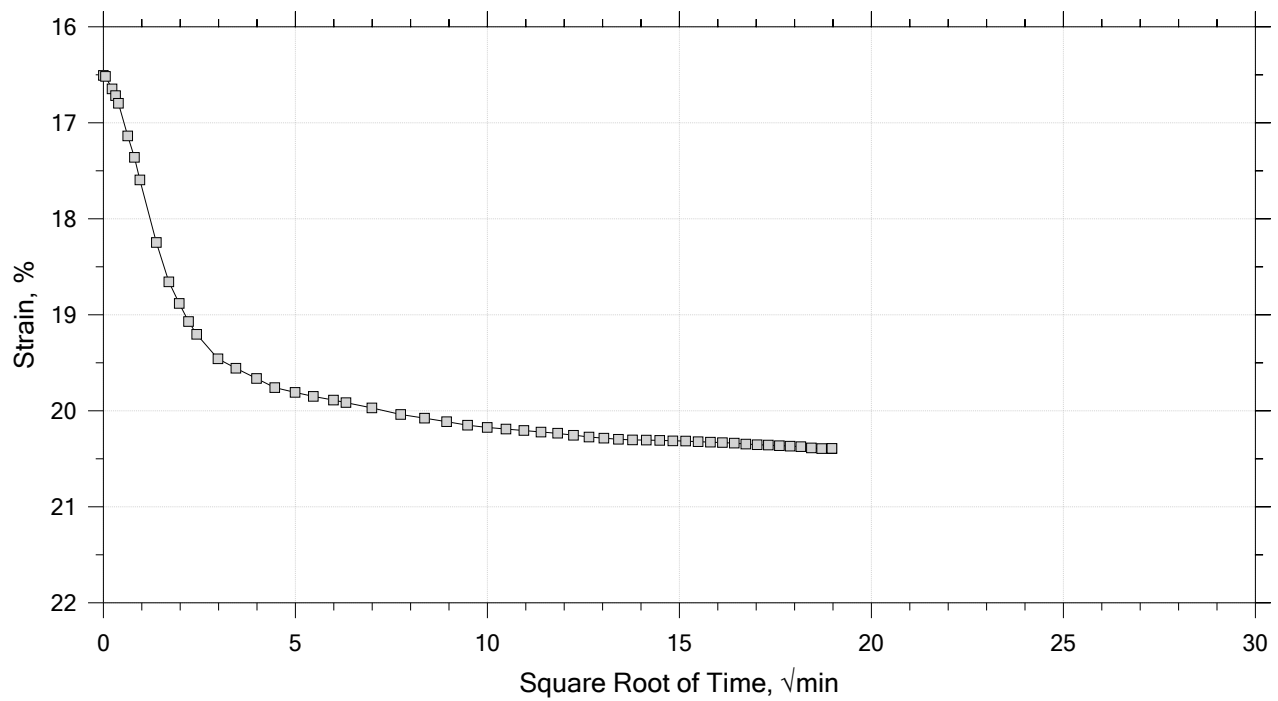
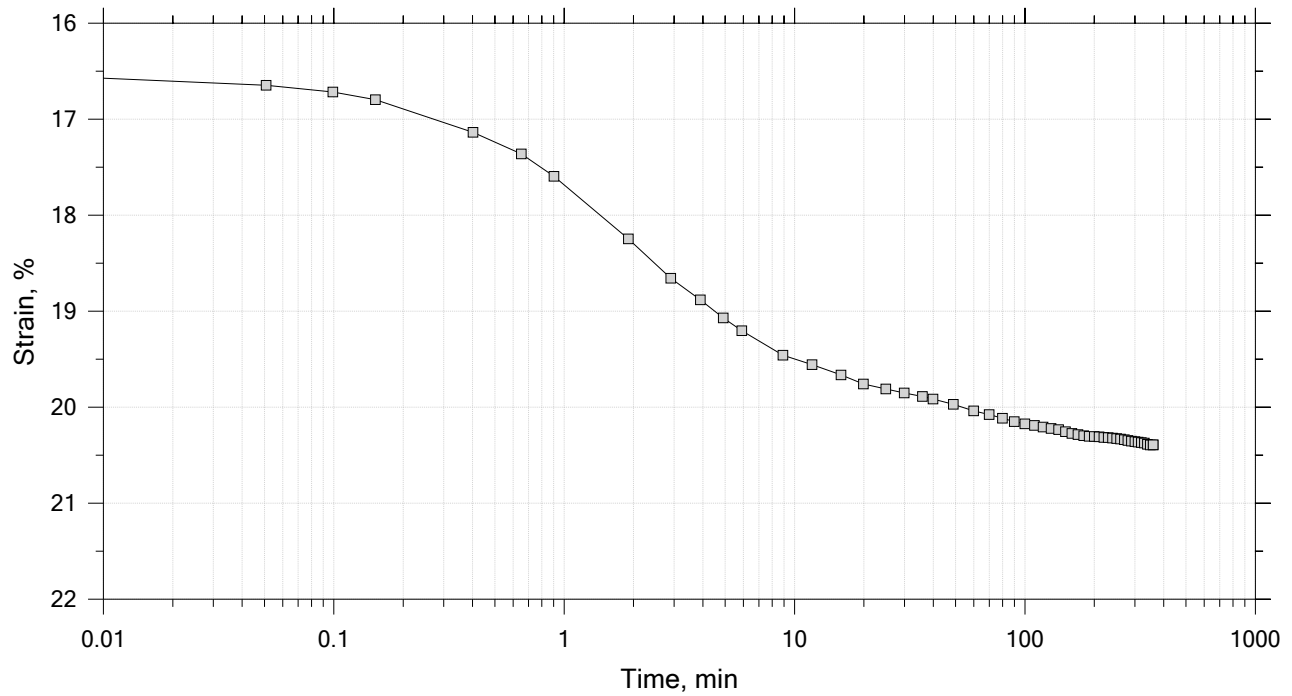
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



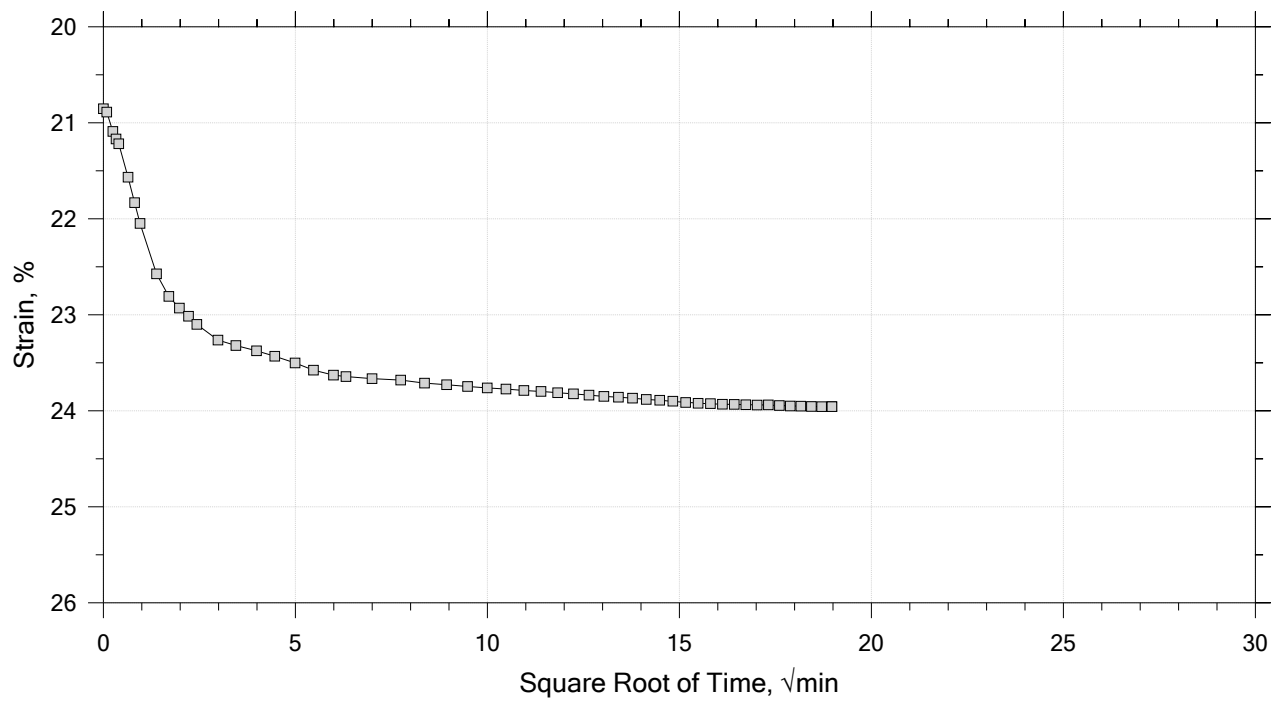
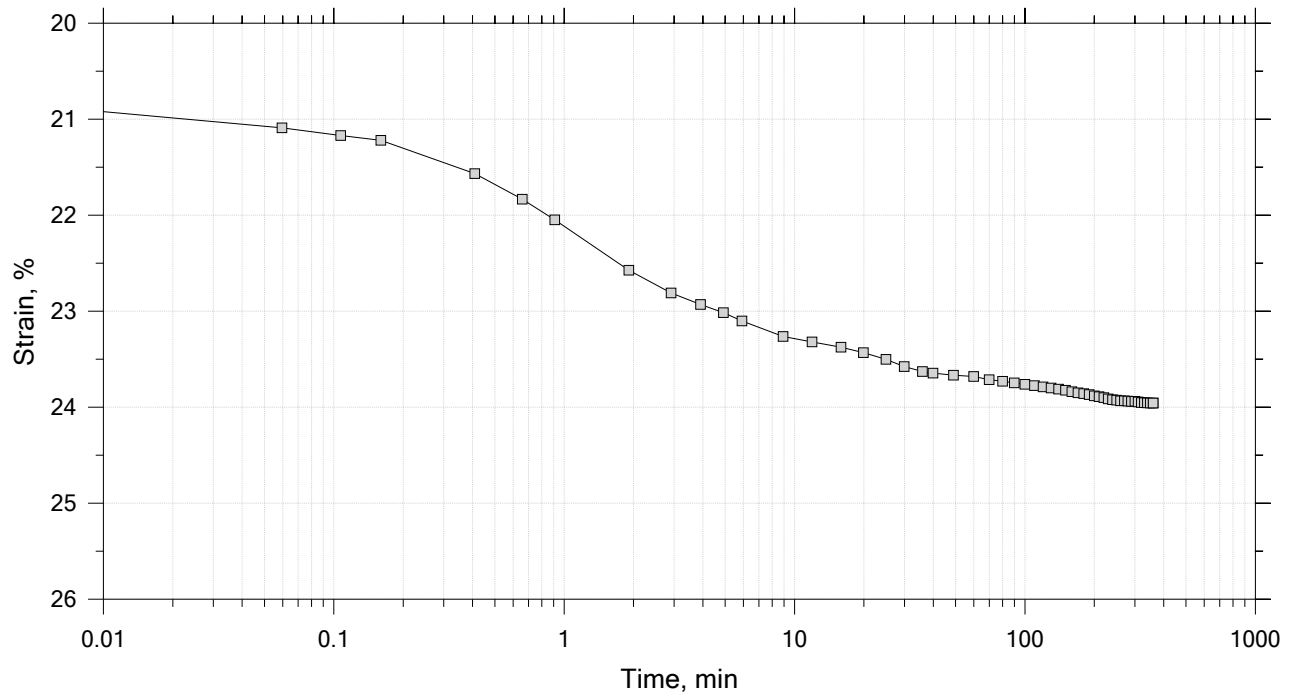
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



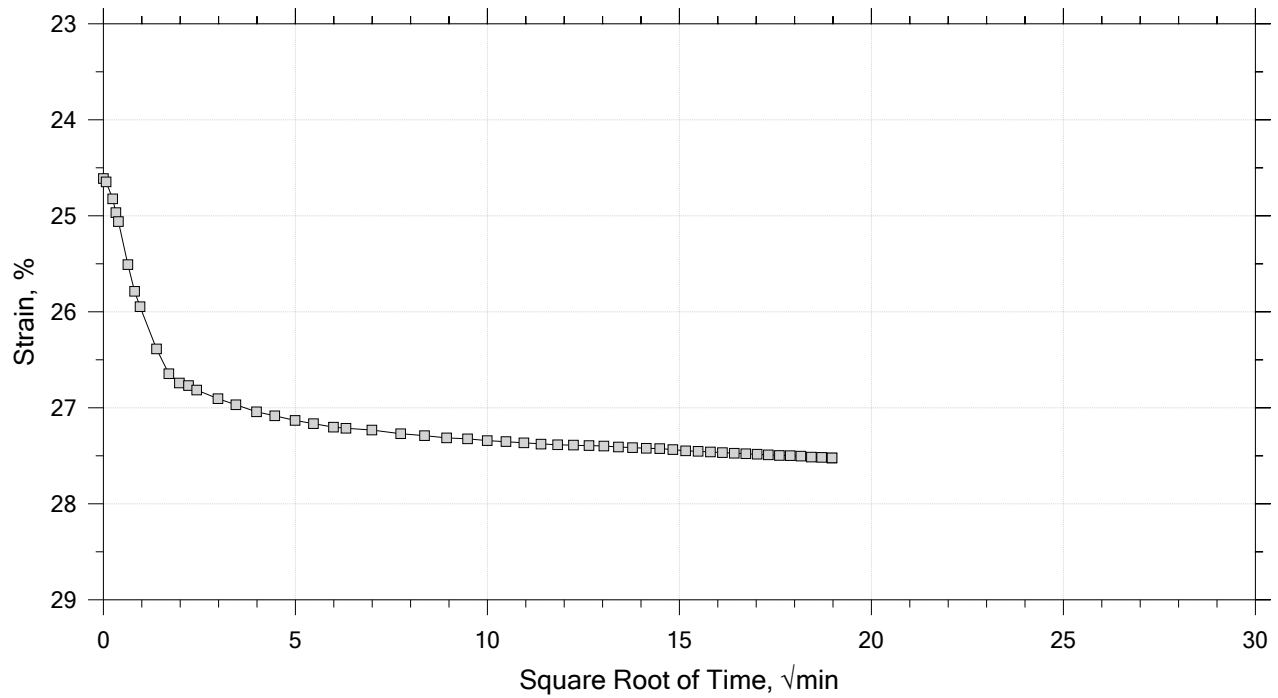
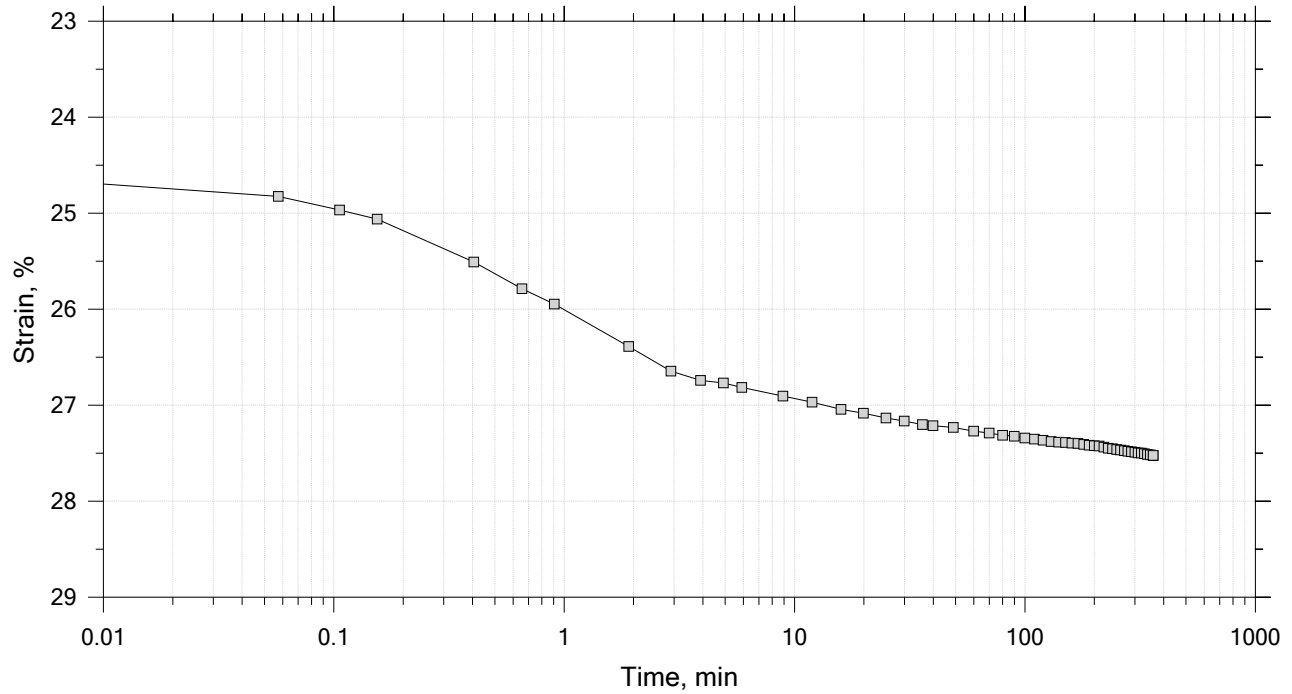
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



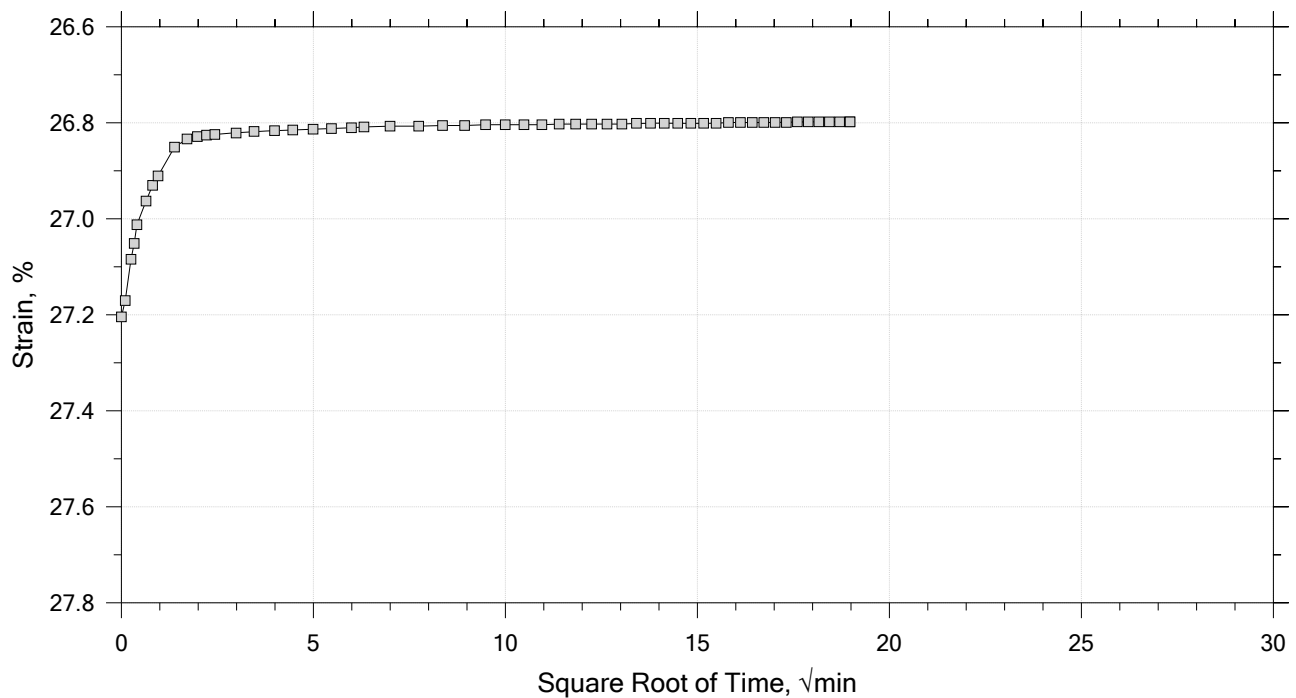
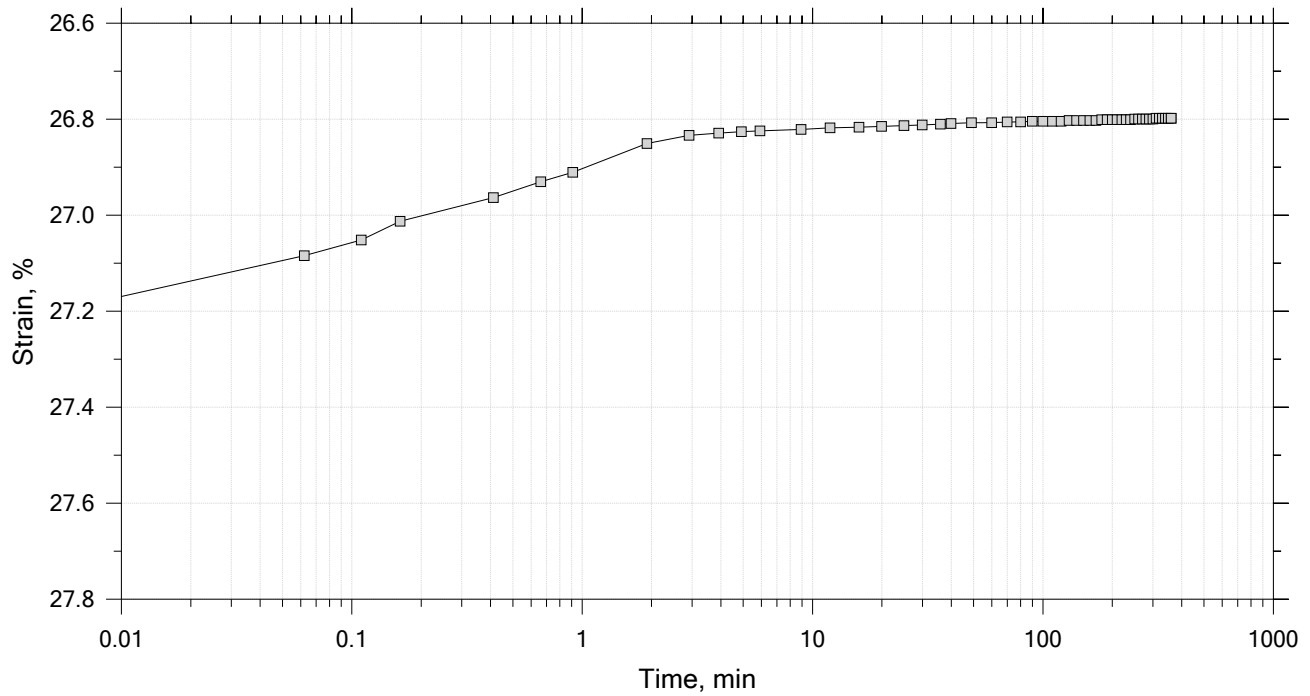
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



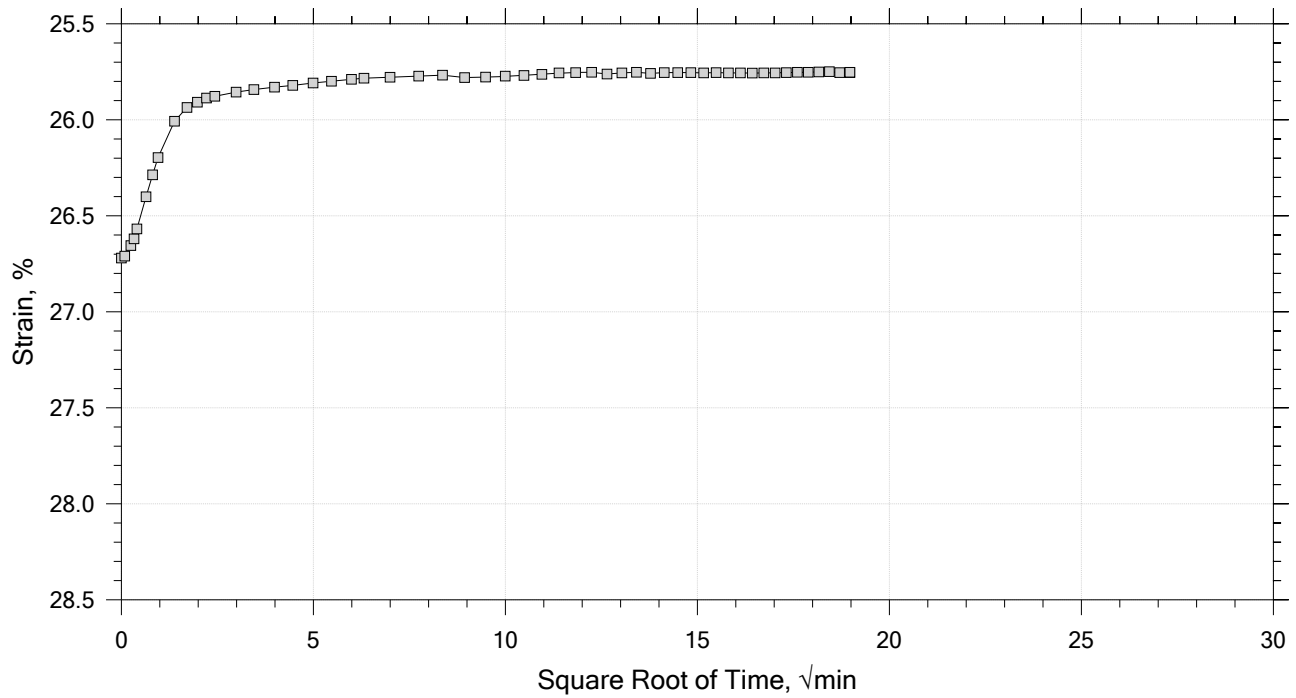
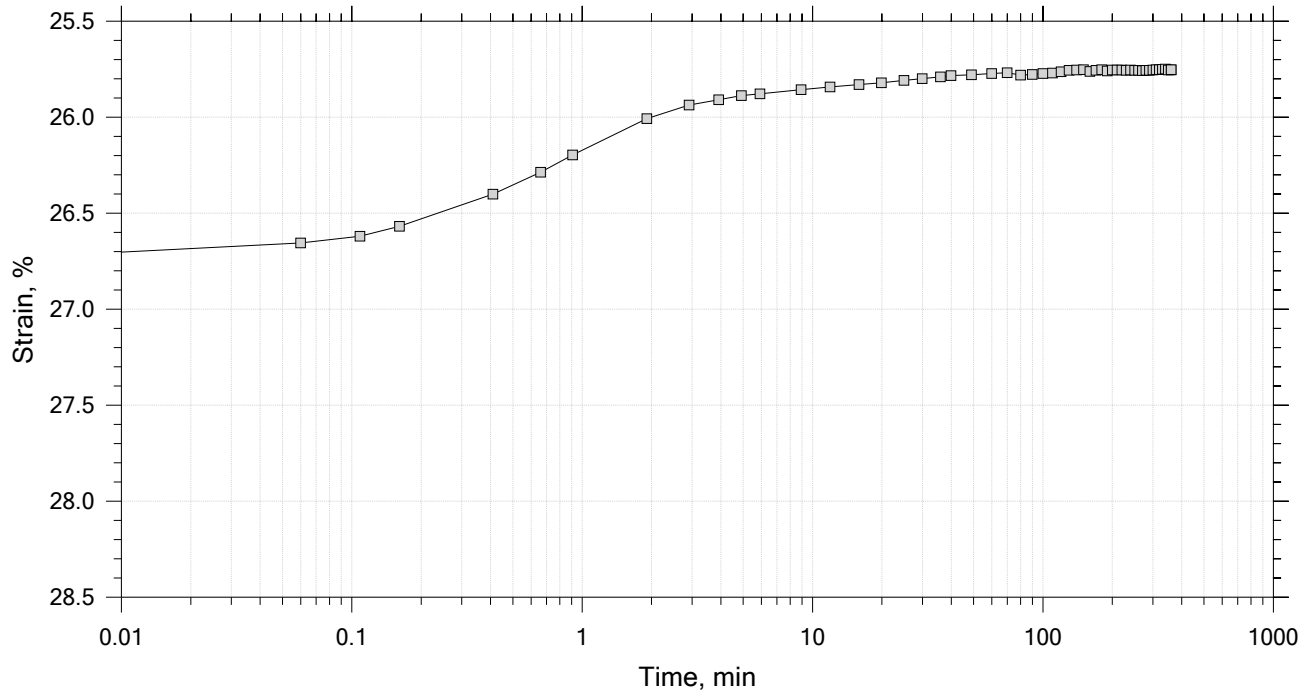
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	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



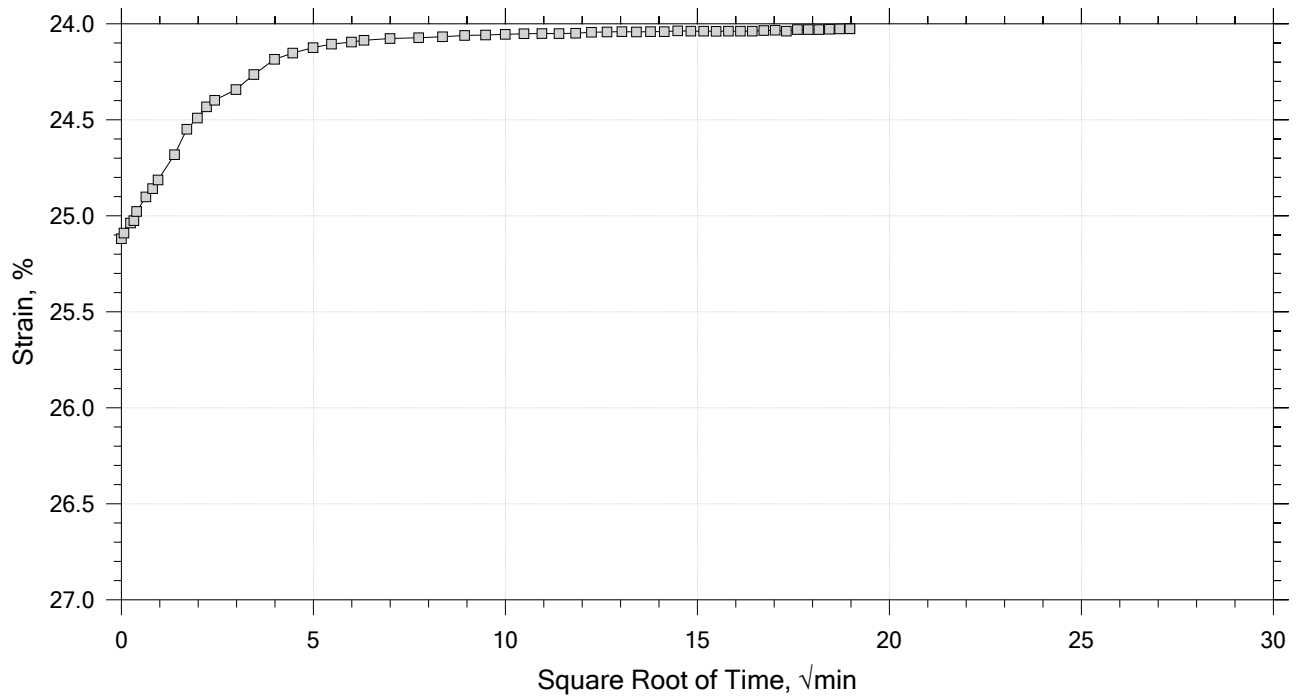
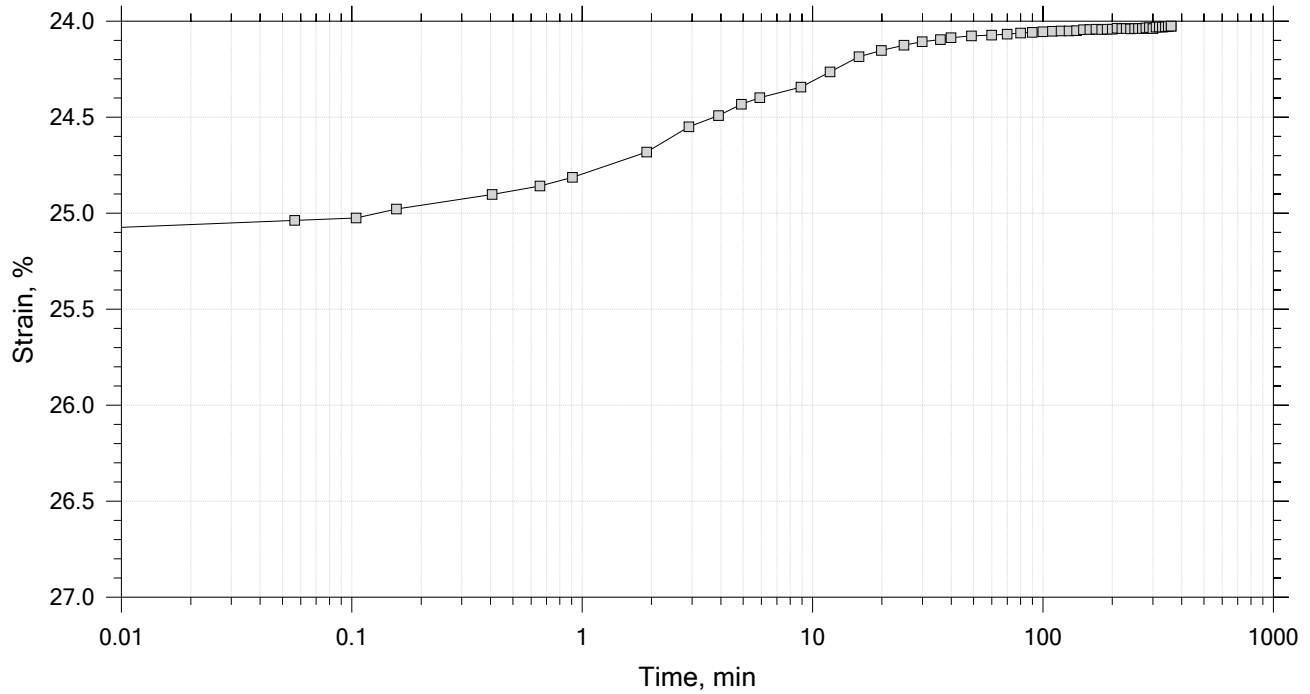
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



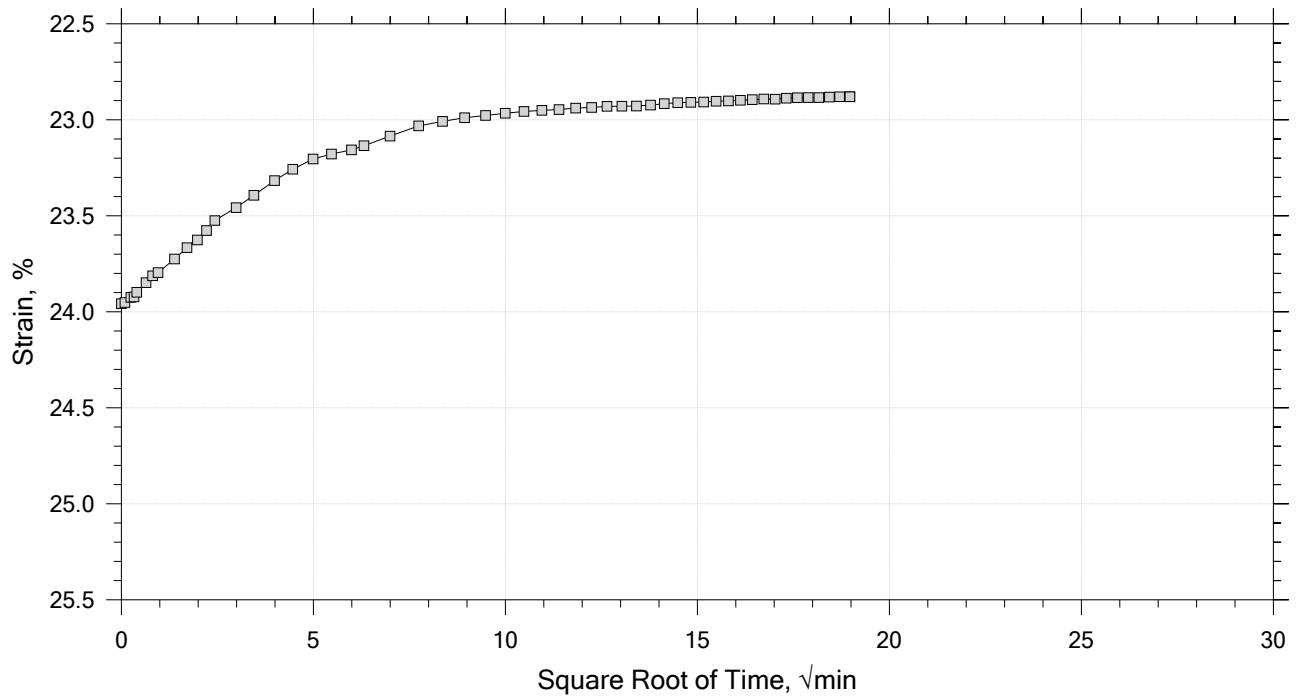
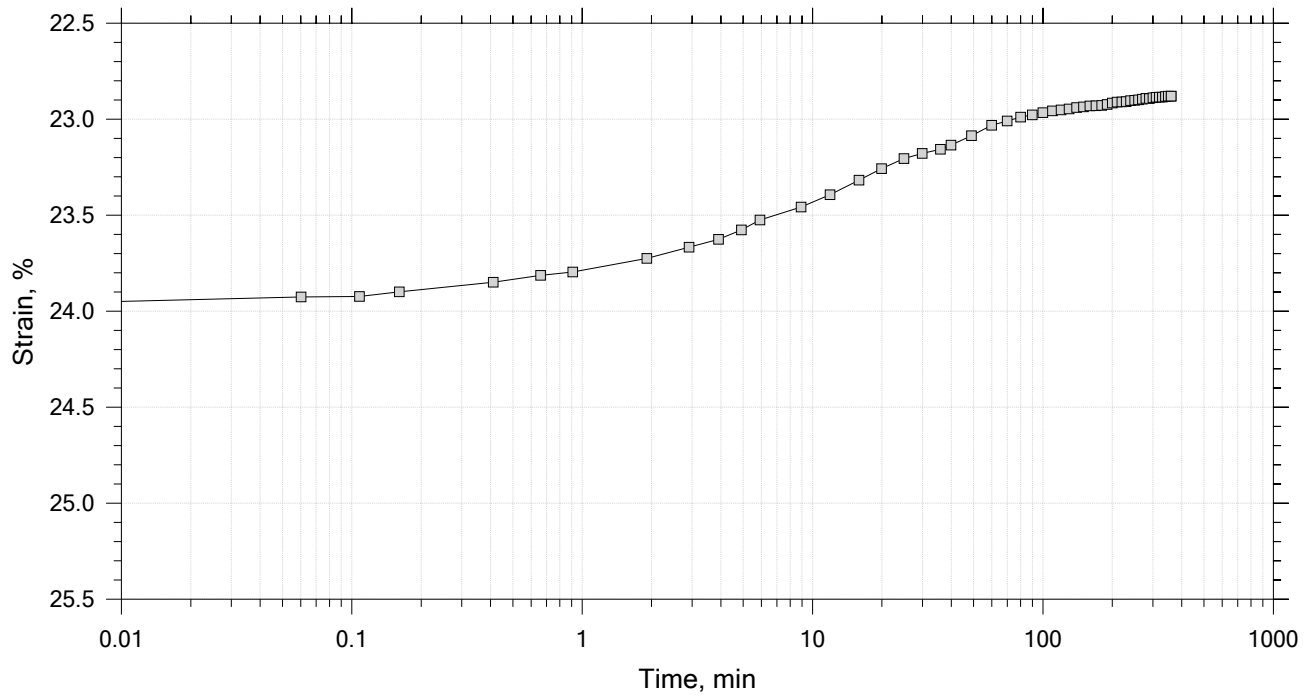
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



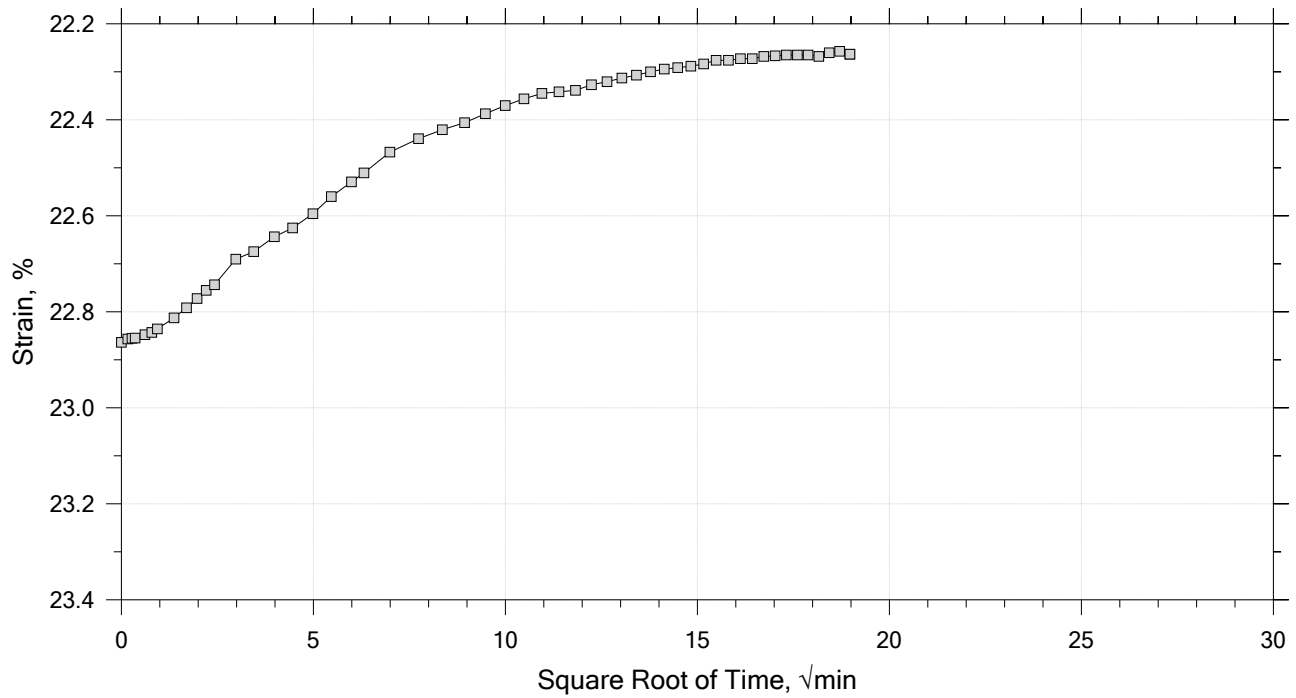
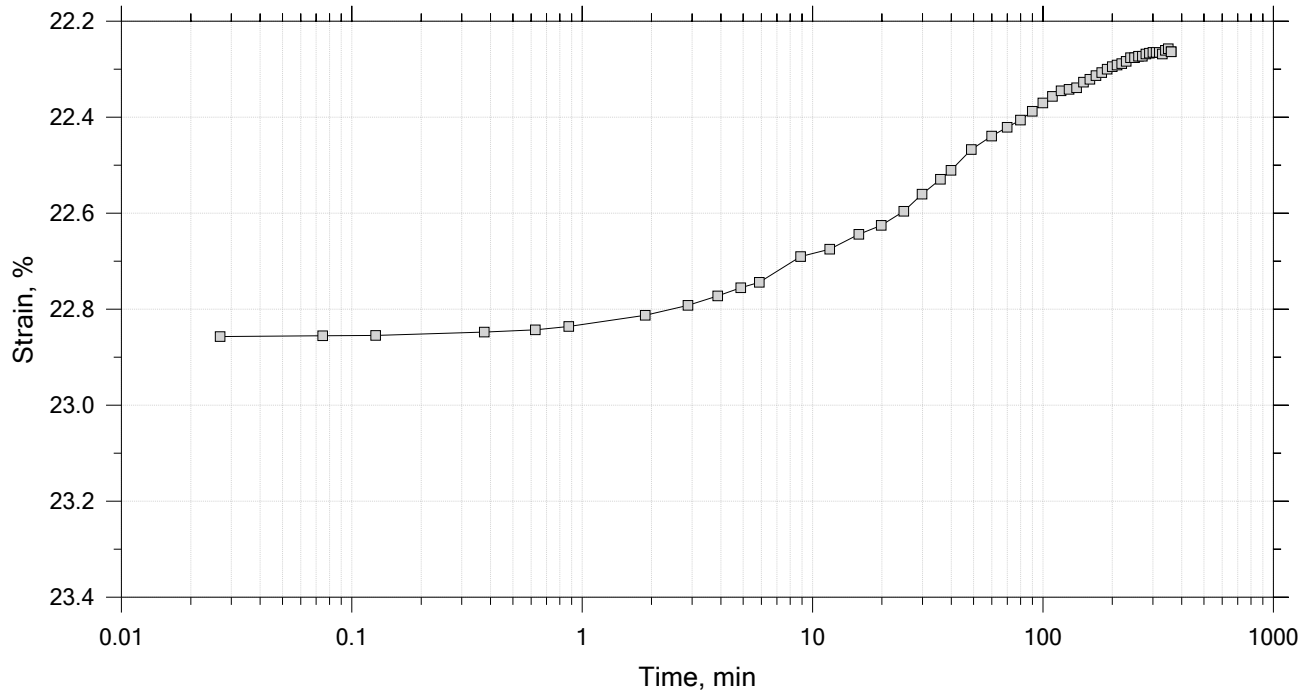
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 33
Initial Height: 1.00 in	Initial Void Ratio: 0.957	Plastic Limit: 17
Final Height: 0.82 in	Final Void Ratio: 0.605	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E-1217	RING		E1700
Mass Container, gm	8.16	110.67	110.67	8.55
Mass Container + Wet Soil, gm	217	262.76	248.39	146.27
Mass Container + Dry Soil, gm	162.33	223.53	223.53	121.41
Mass Dry Soil, gm	154.17	112.86	112.86	112.86
Water Content, %	35.46	34.76	22.03	22.03
Void Ratio	---	0.96	0.60	---
Degree of Saturation, %	---	99.72	100.00	---
Dry Unit Weight, pcf	---	87.589	106.82	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIII-0837, Swell Pressure = 0.0658 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

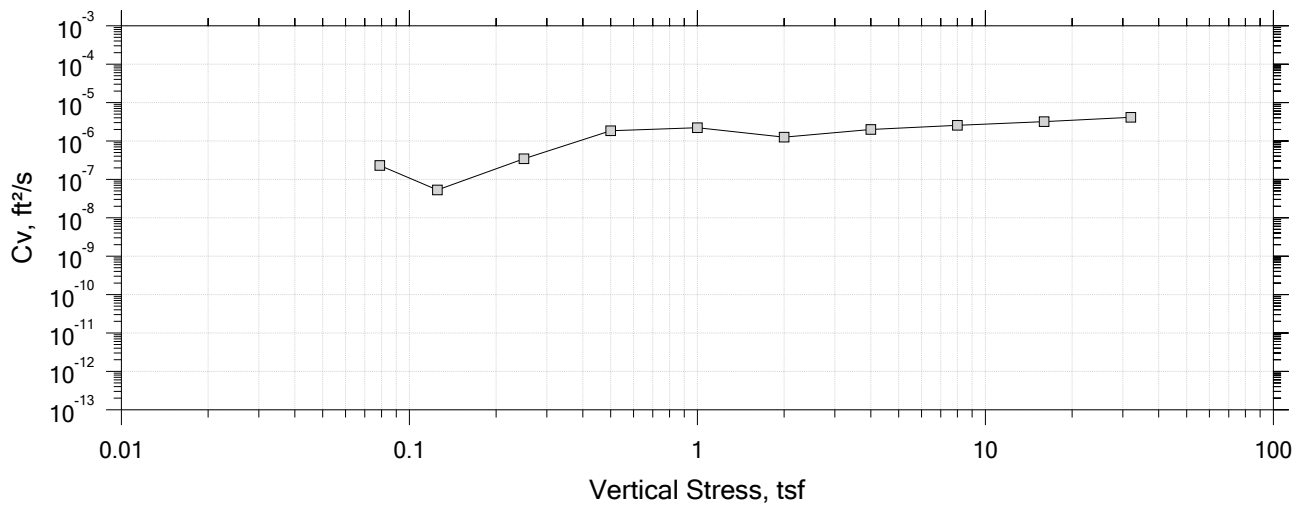
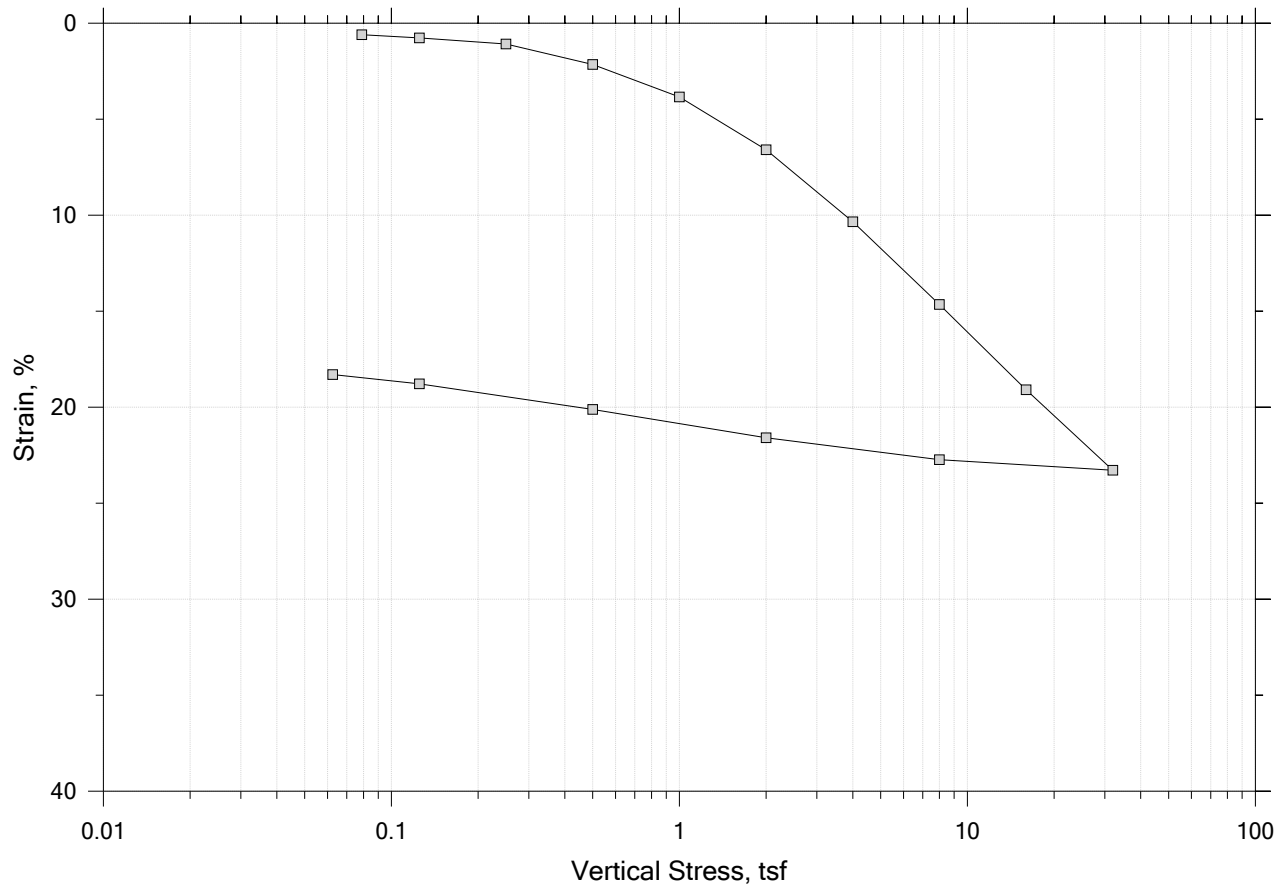
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-3132665
	Boring No.: BB-BFB2-202	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/01/21	Depth: 10-12 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Wet, gray clay		
	Remarks: System LTIH-0837, Swell Pressure = 0.0658 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

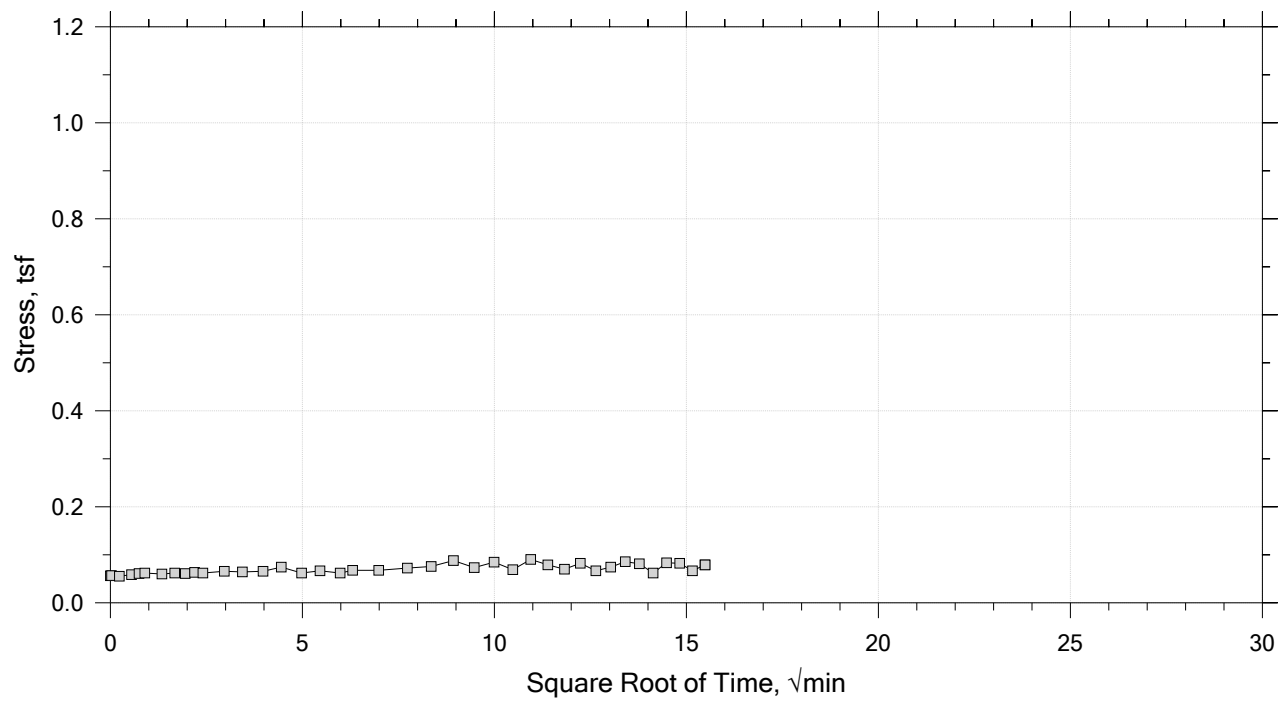
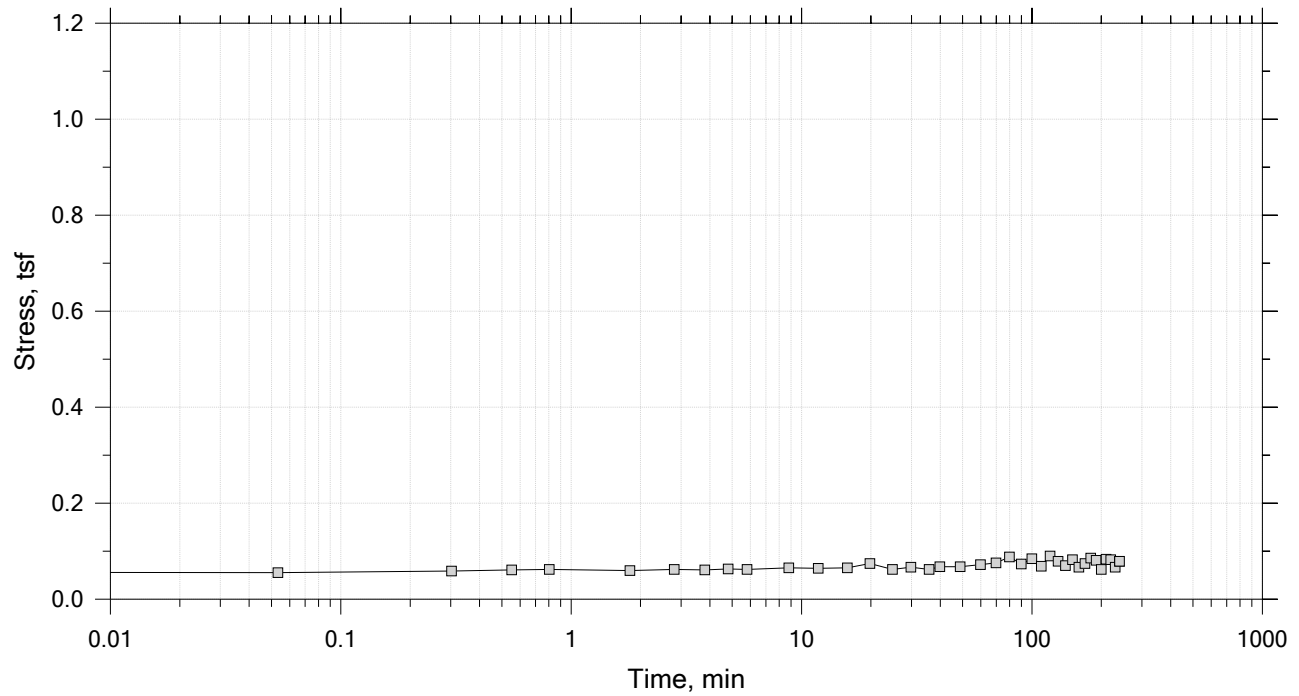
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0789 tsf



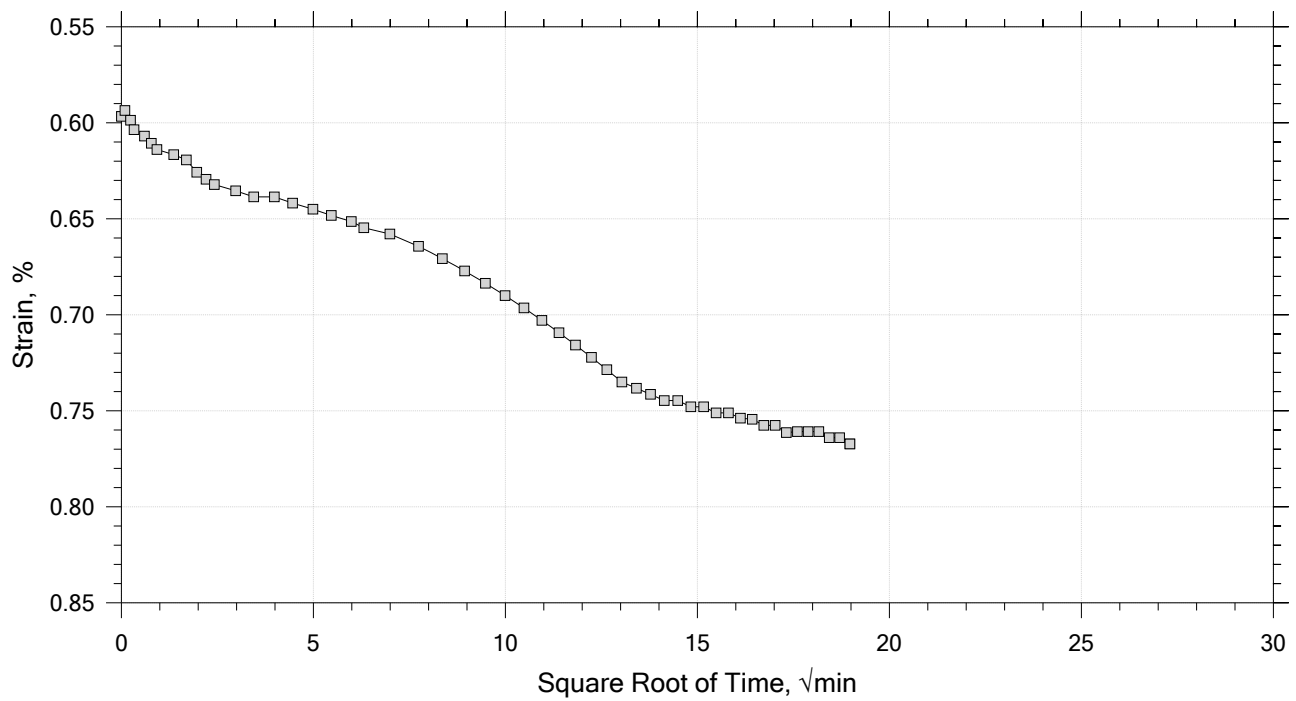
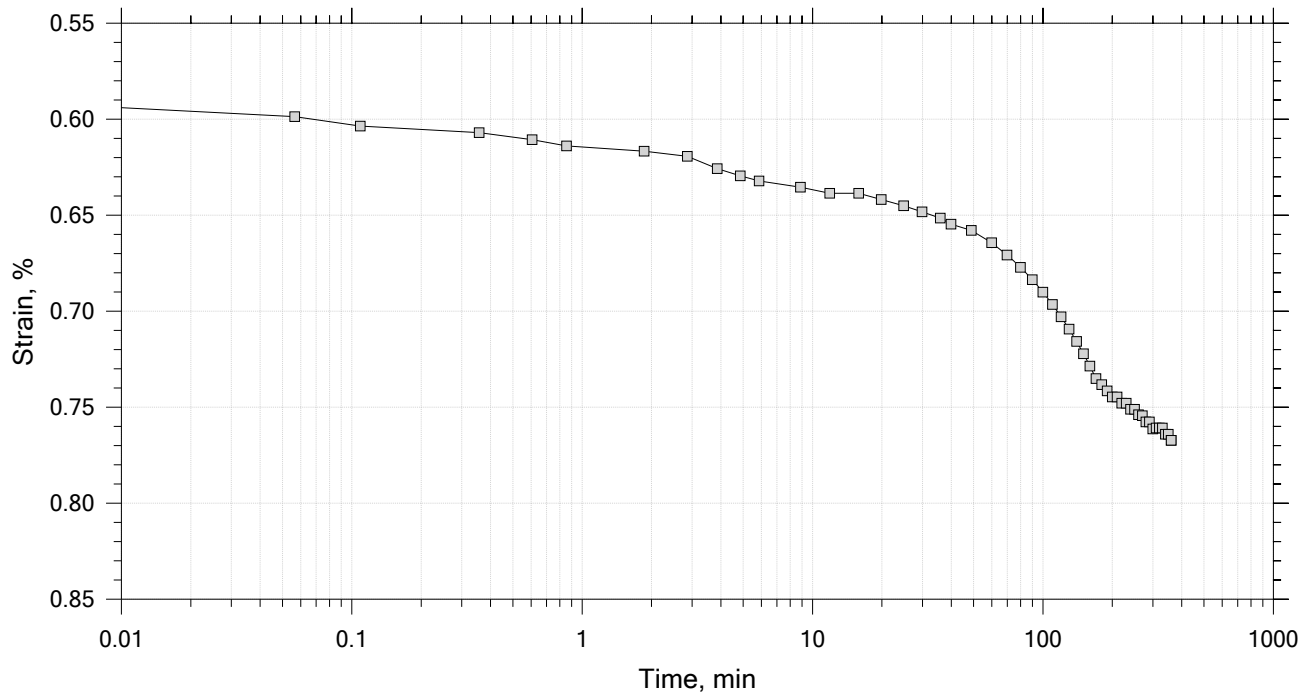
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



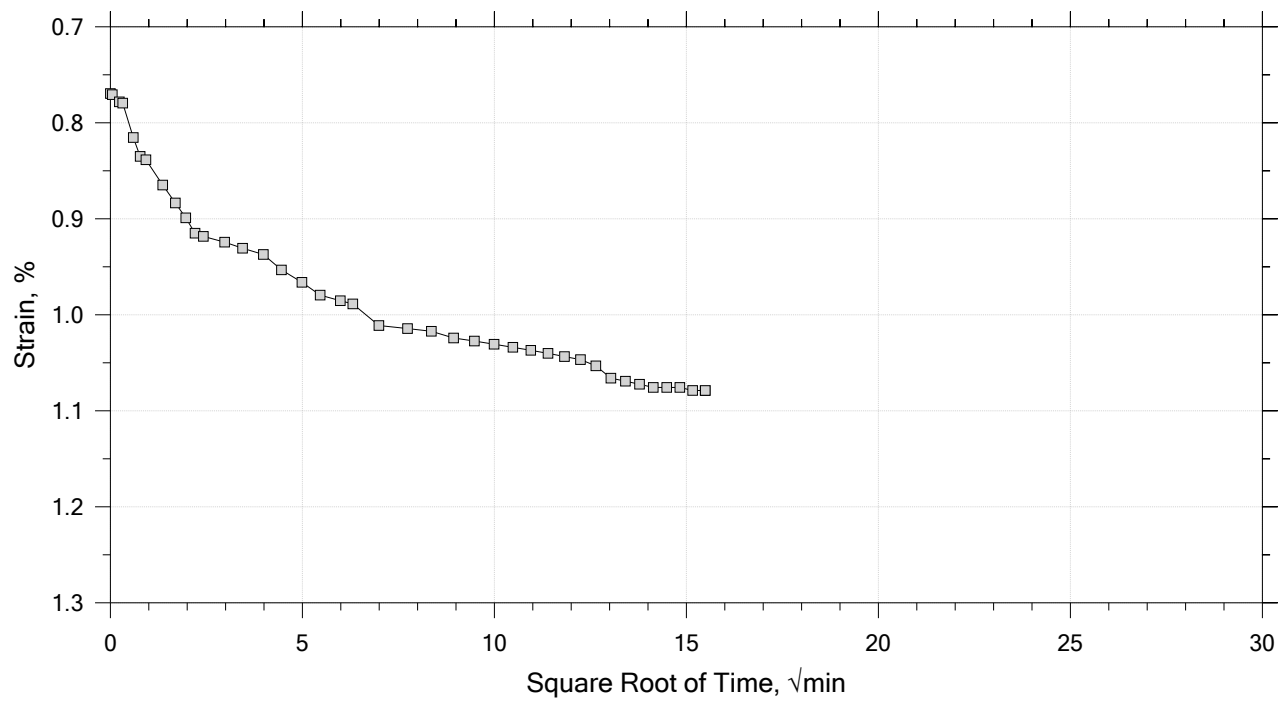
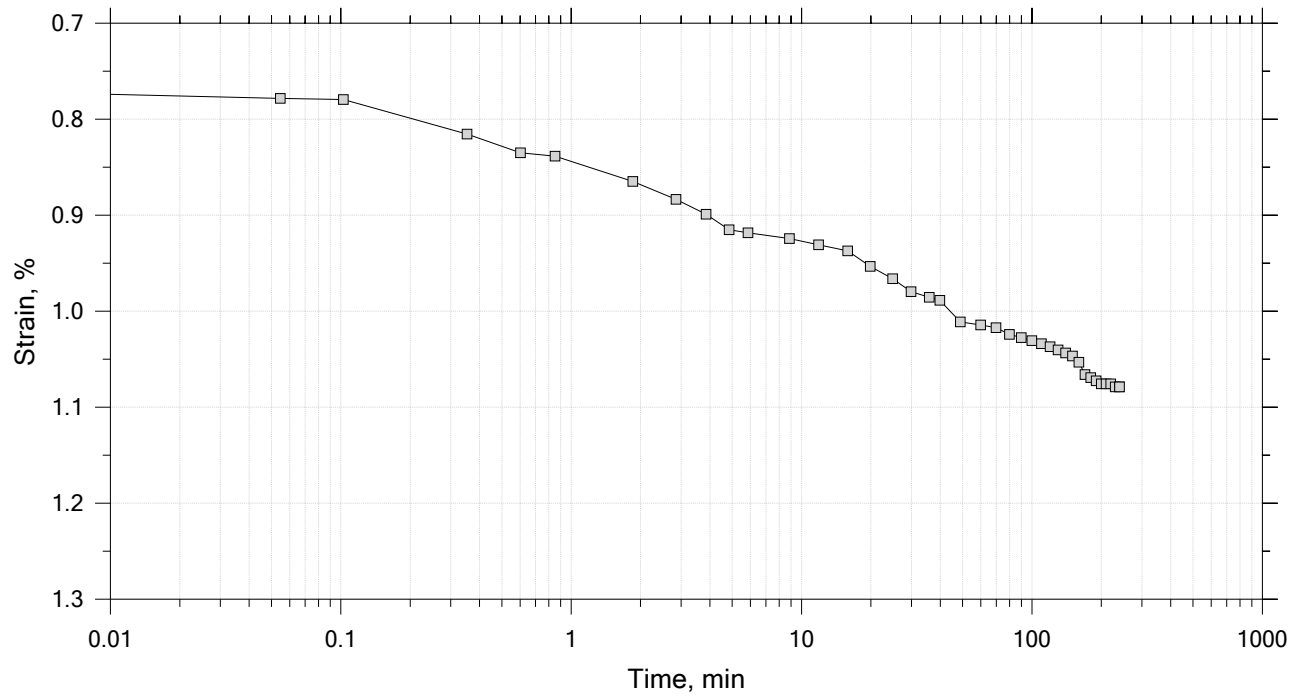
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



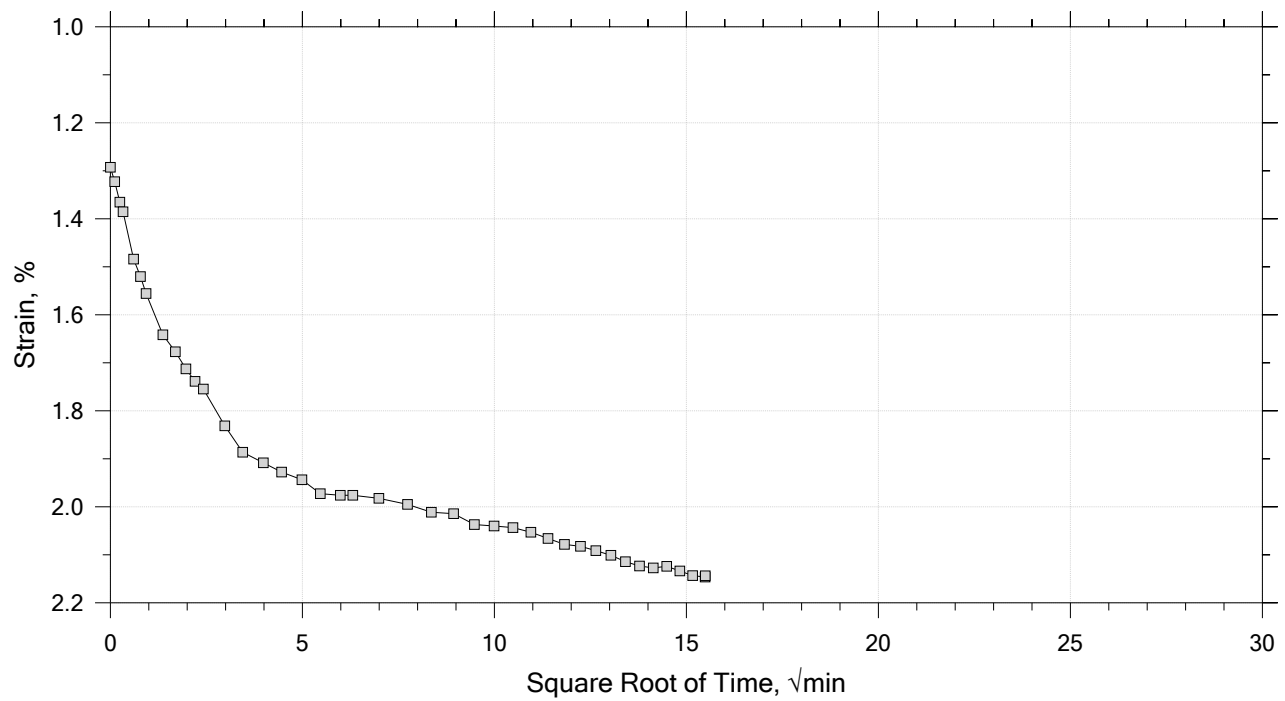
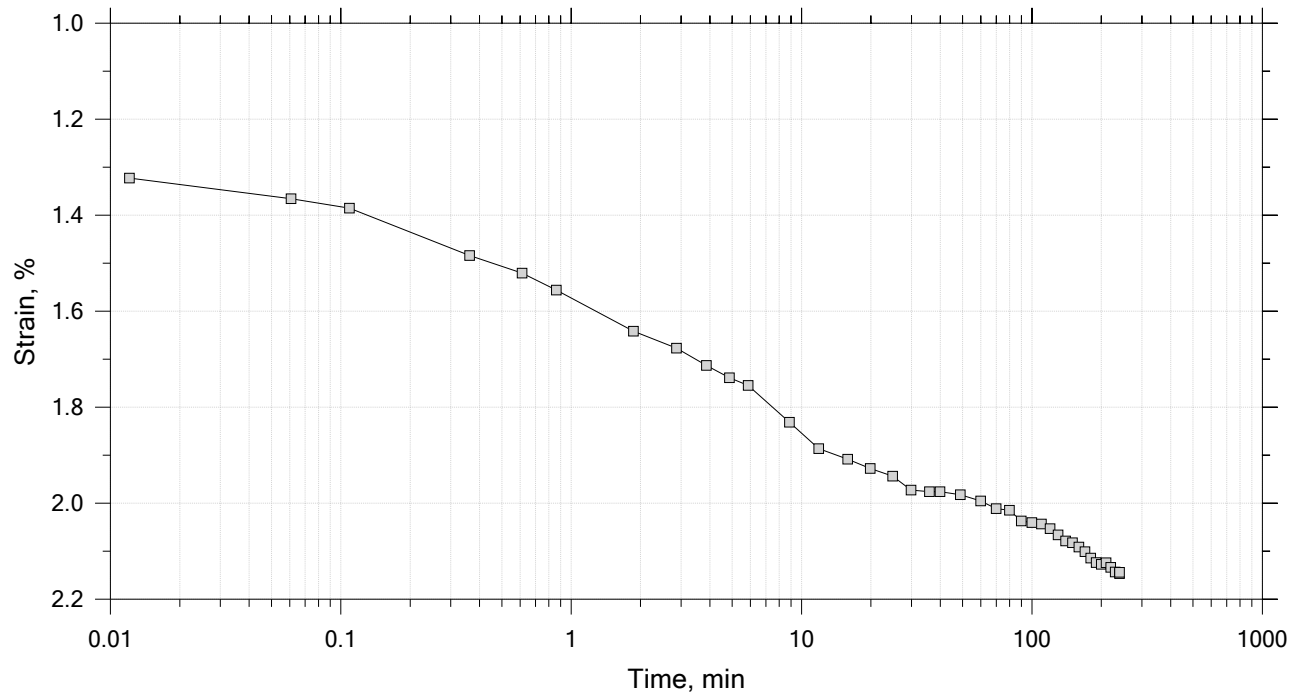
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



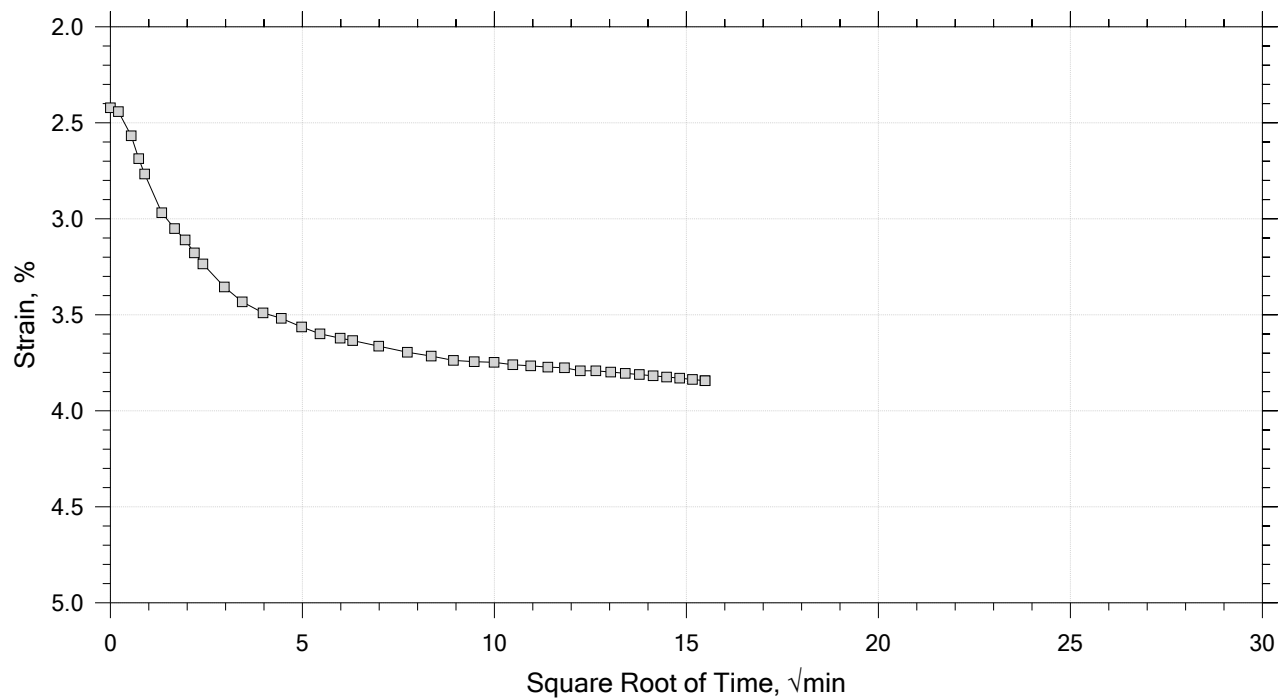
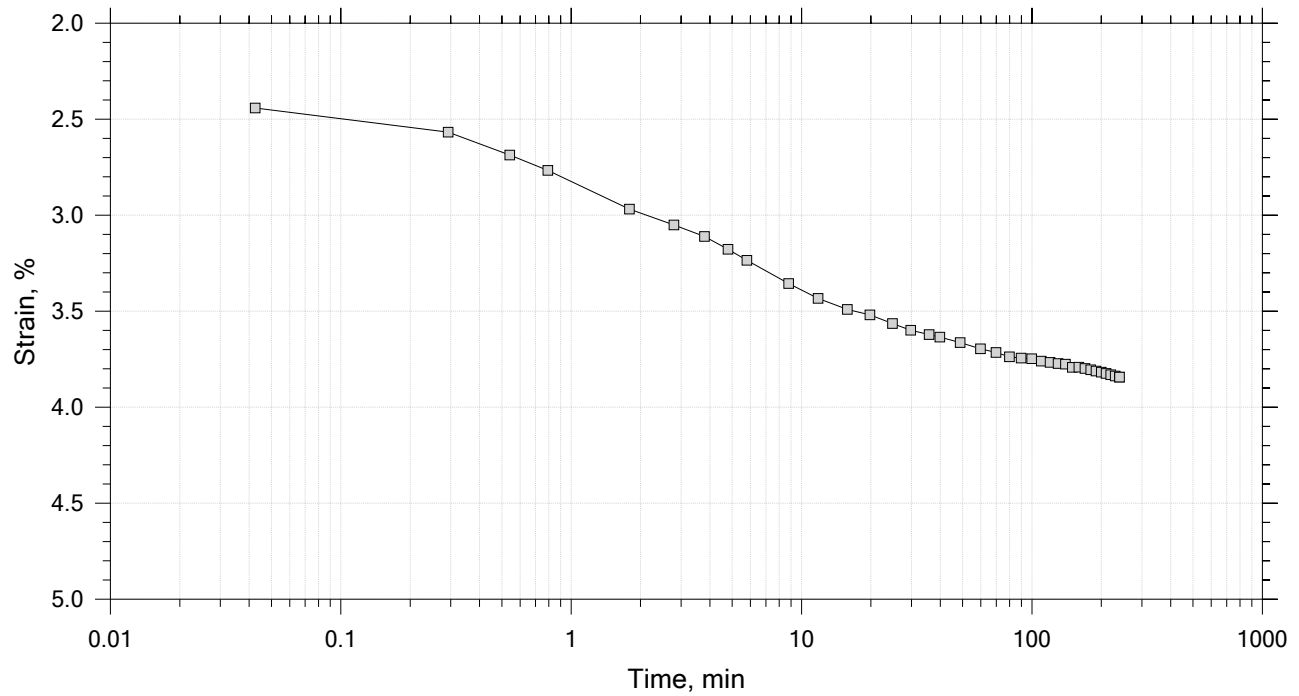
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



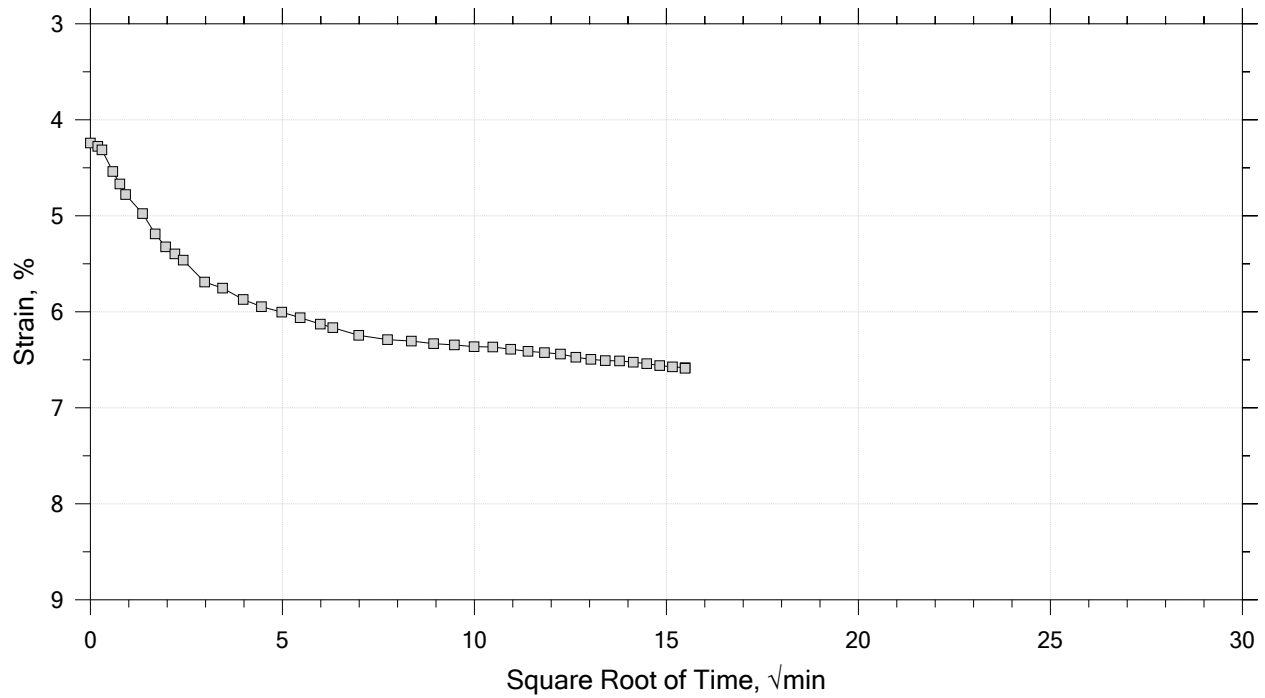
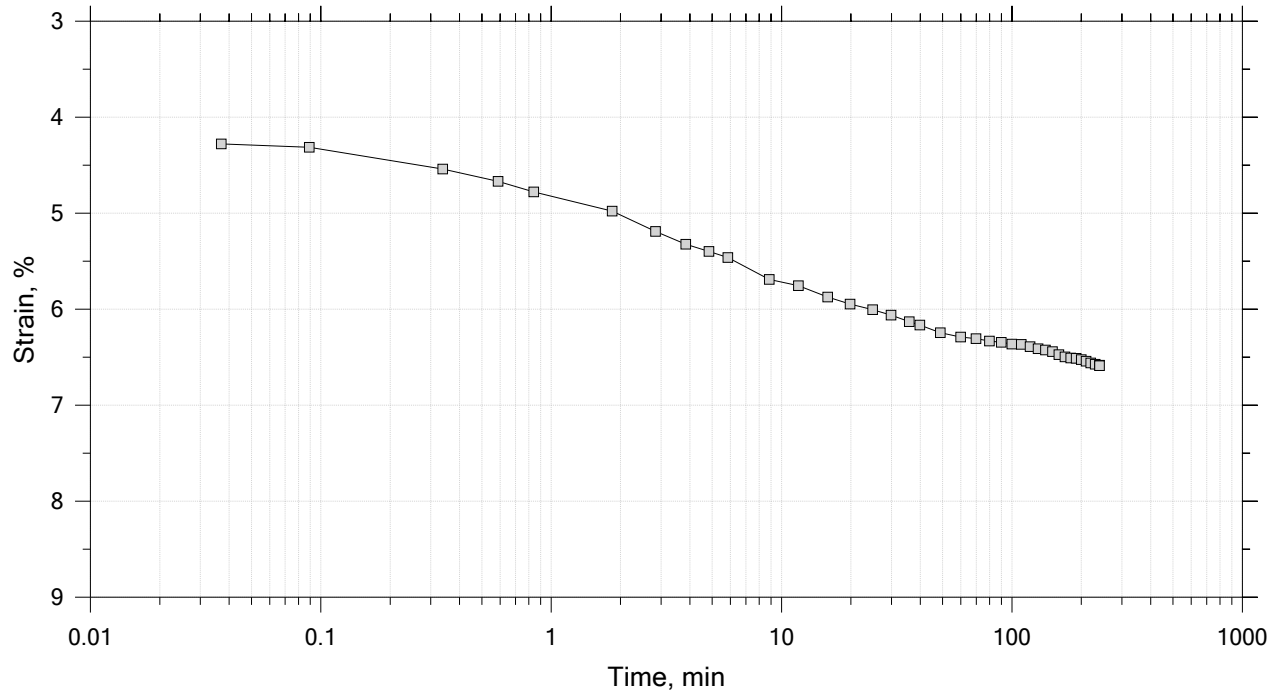
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



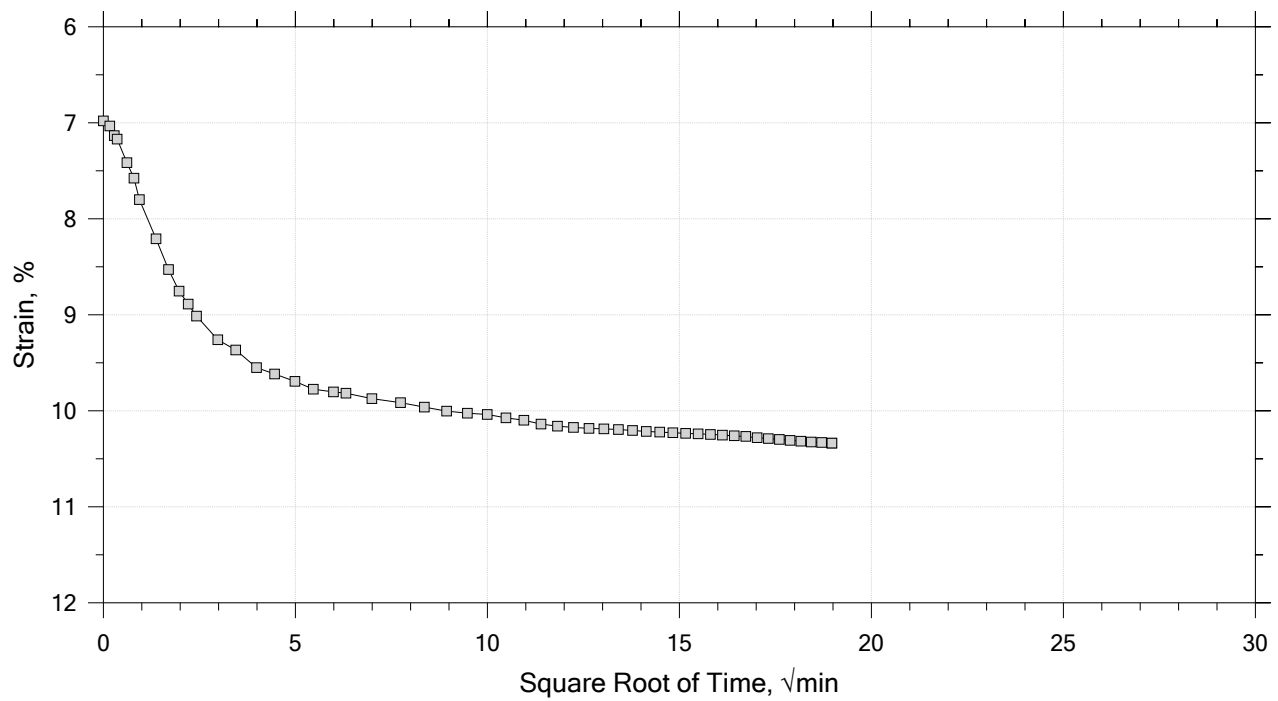
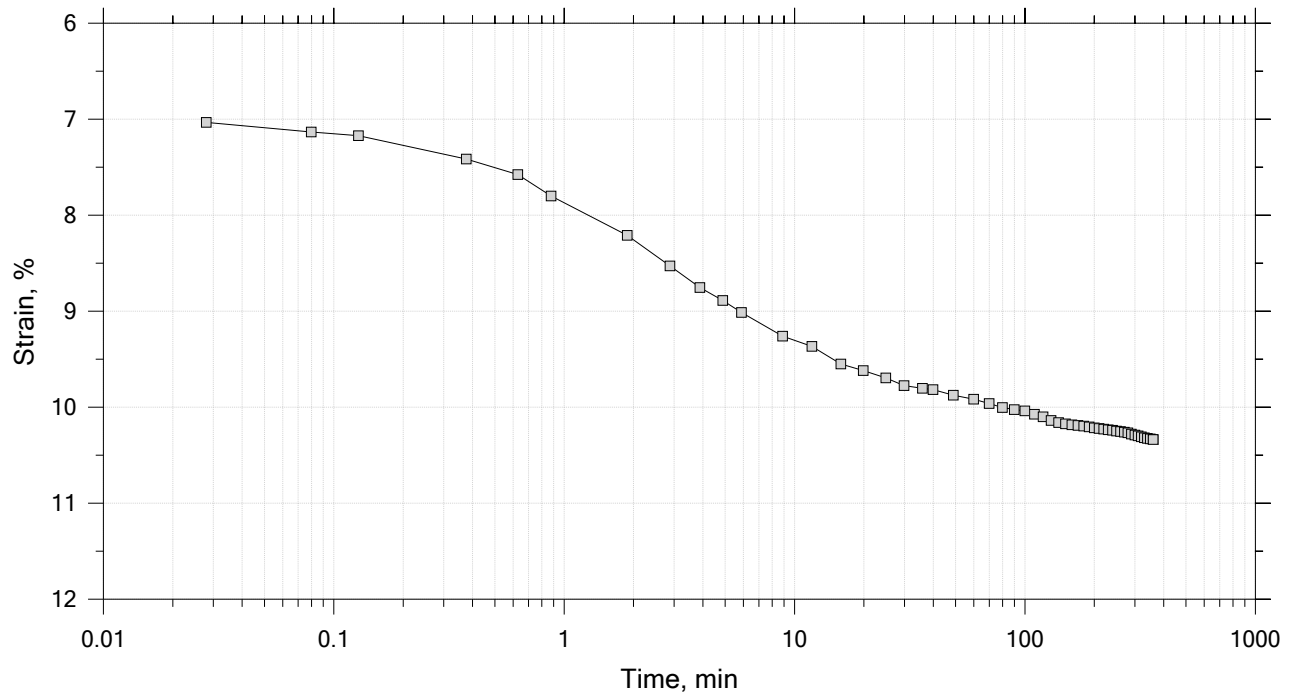
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



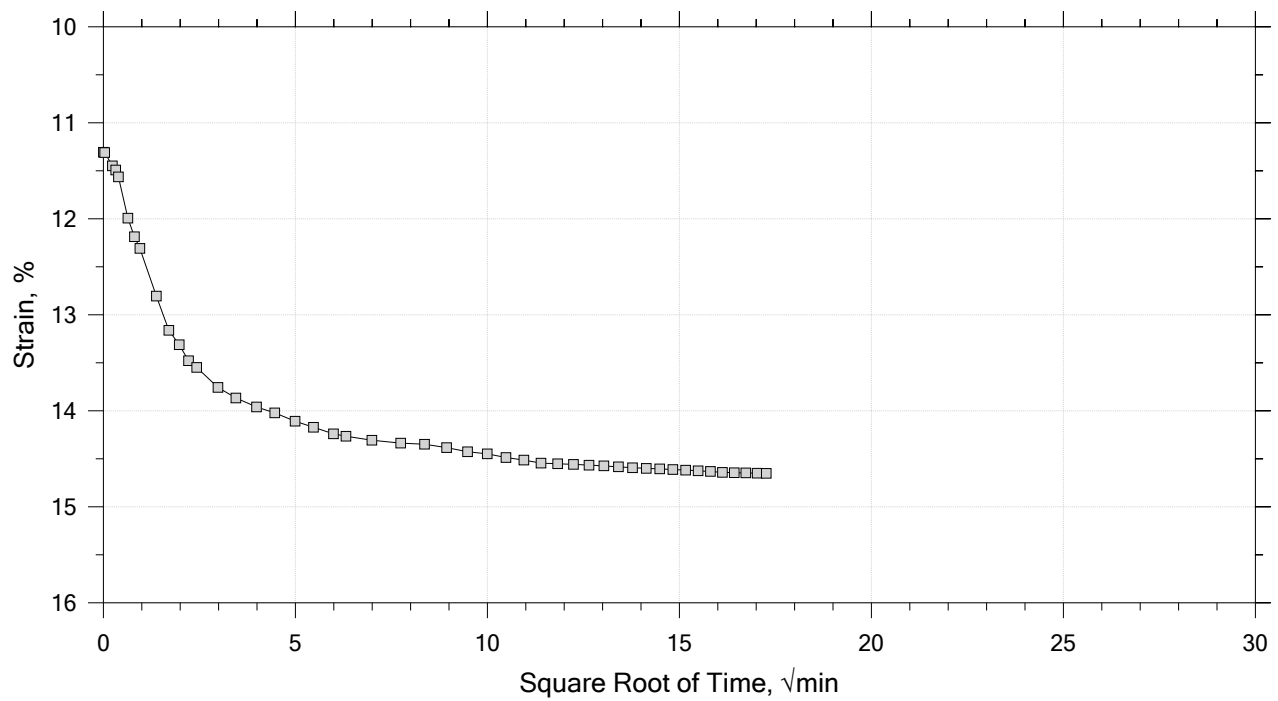
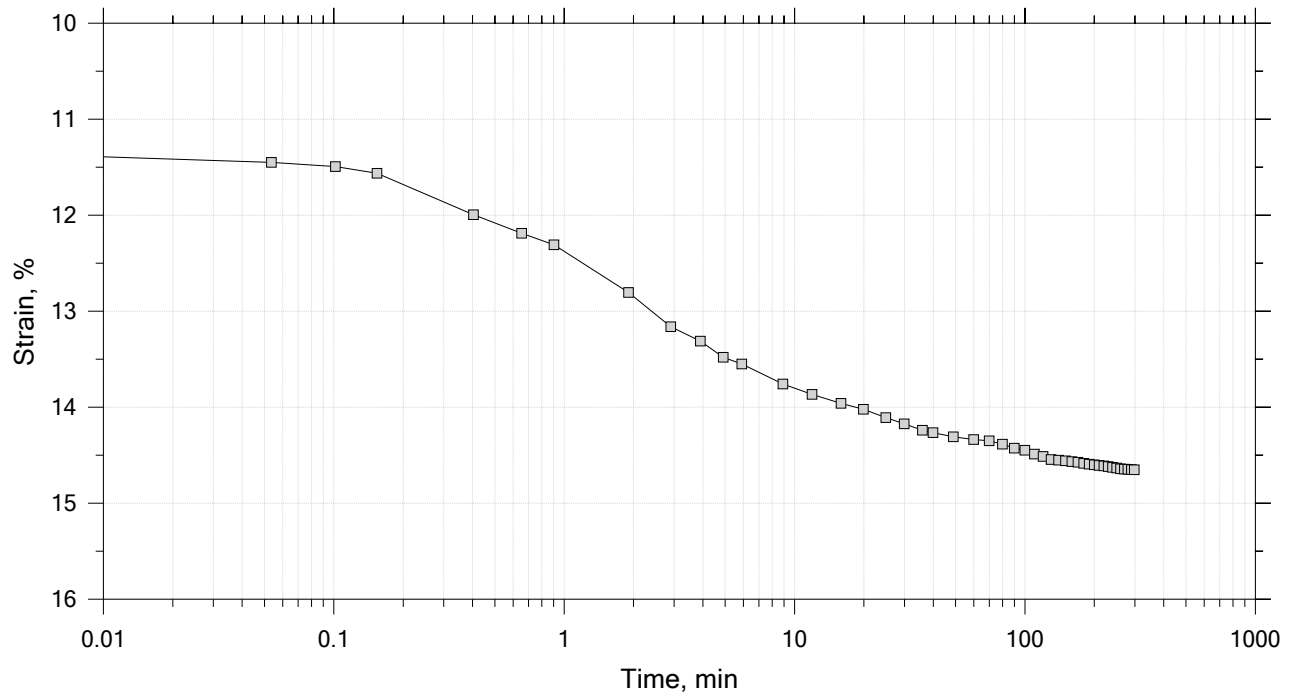
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



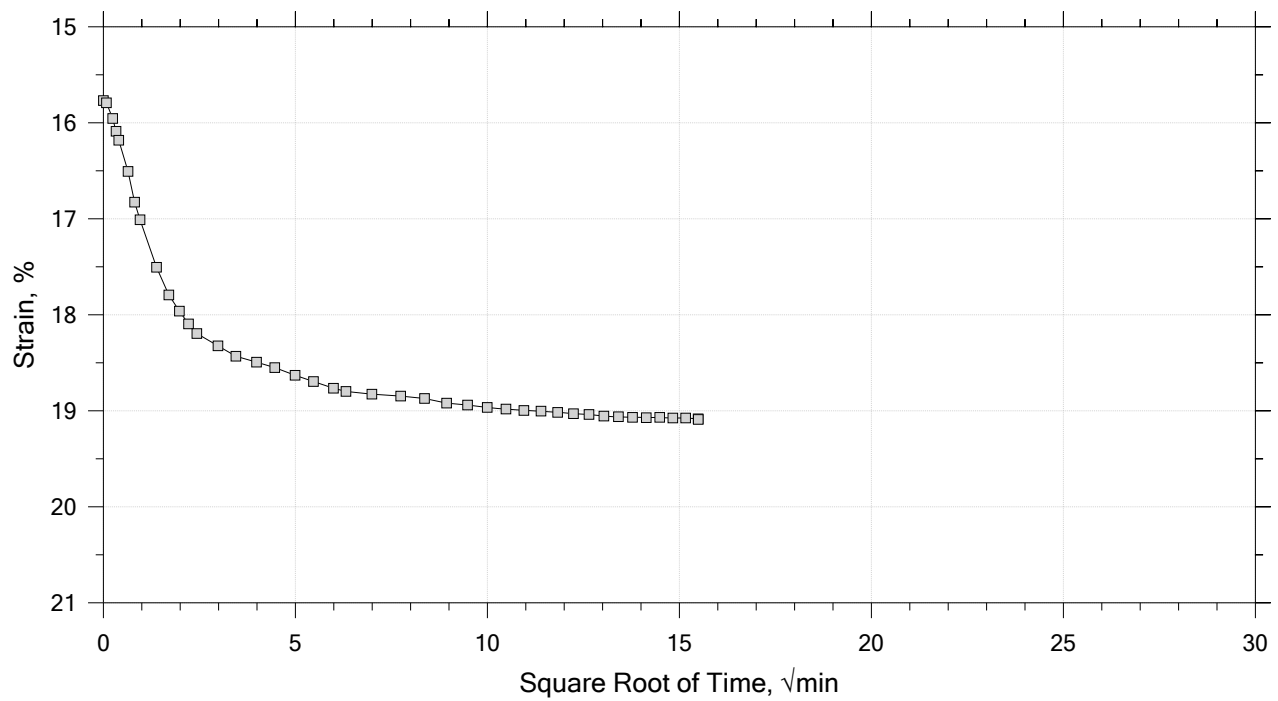
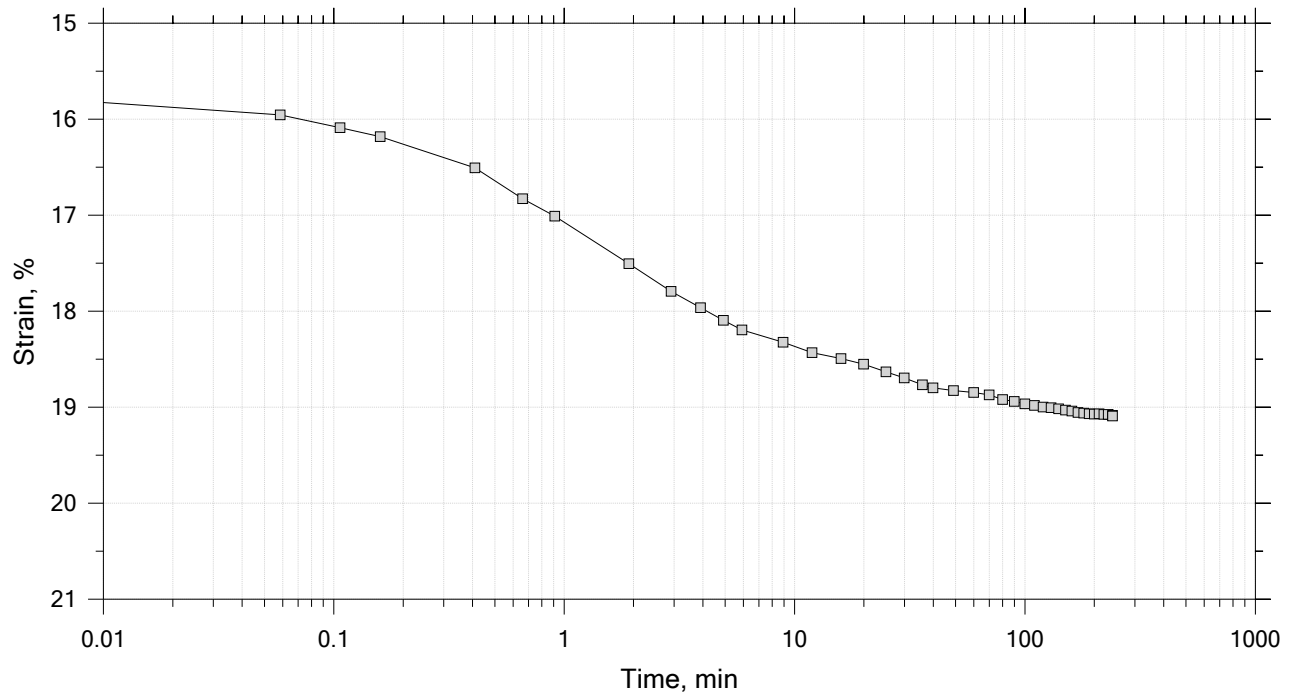
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



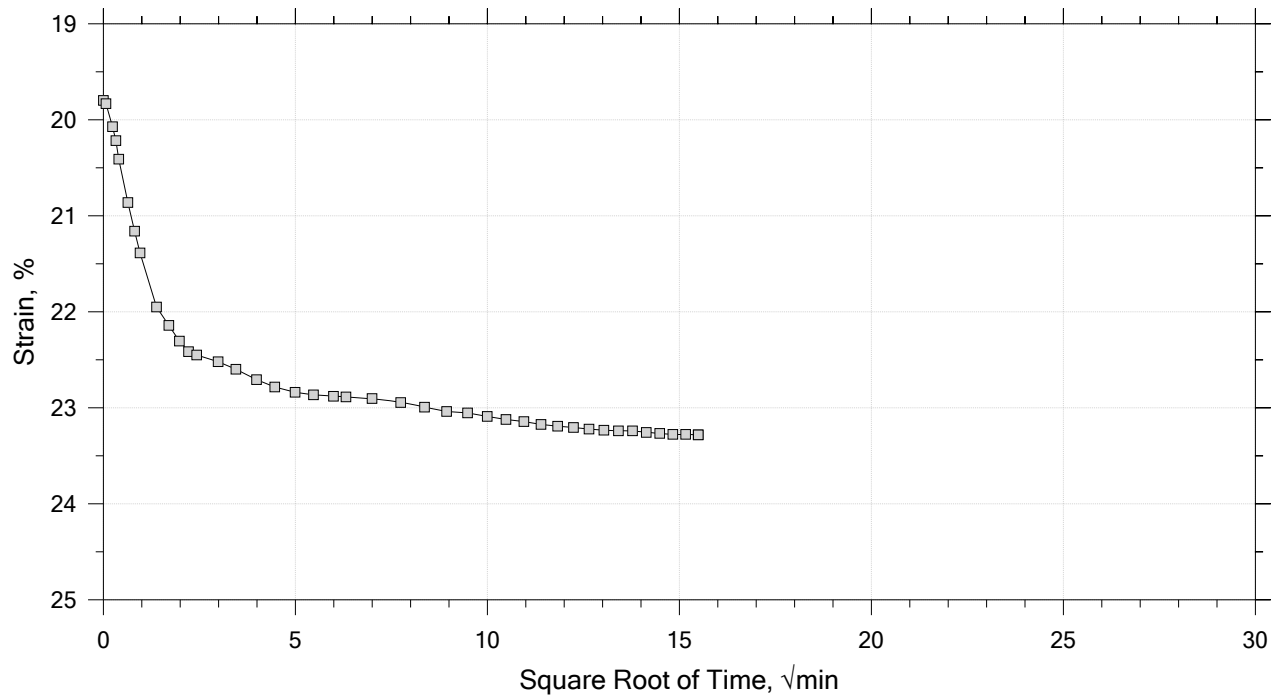
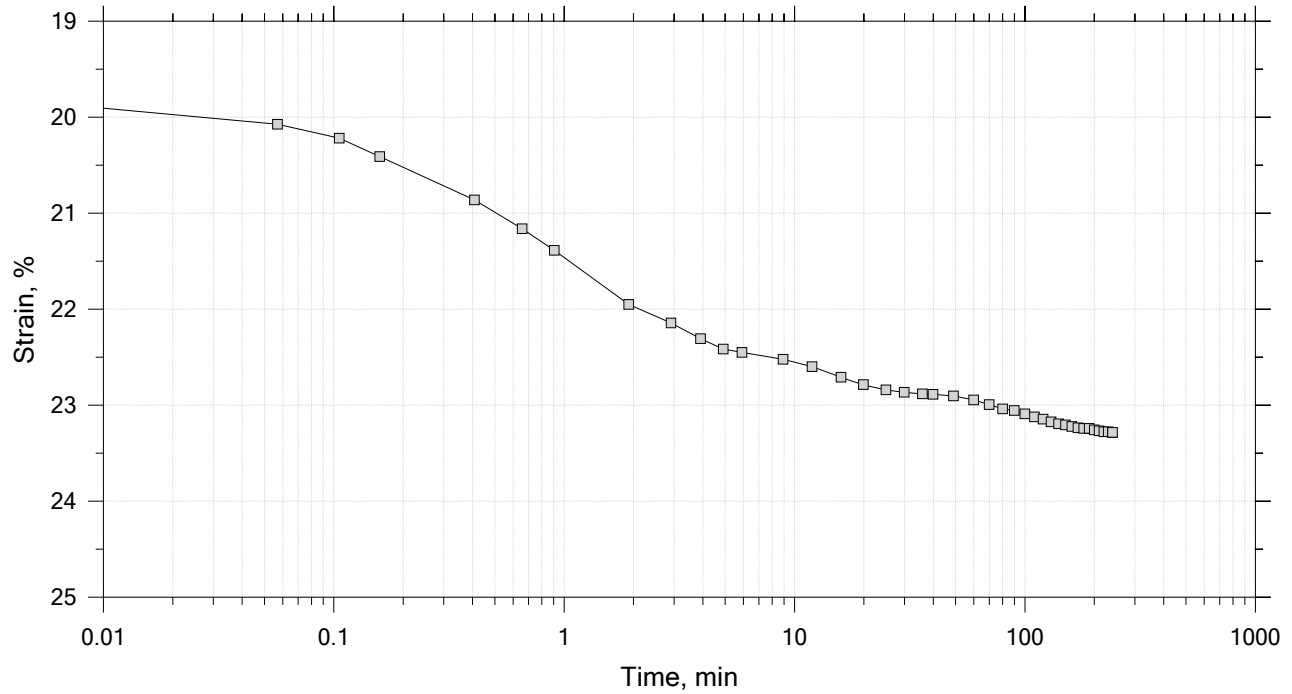
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



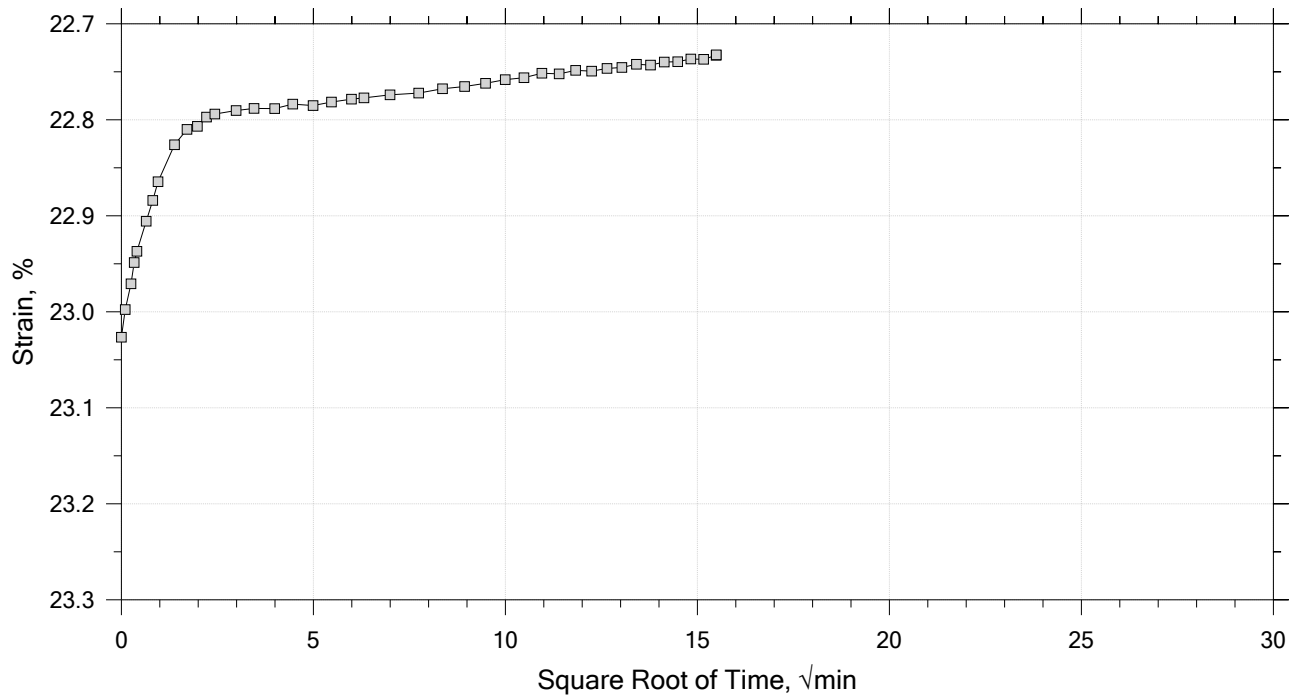
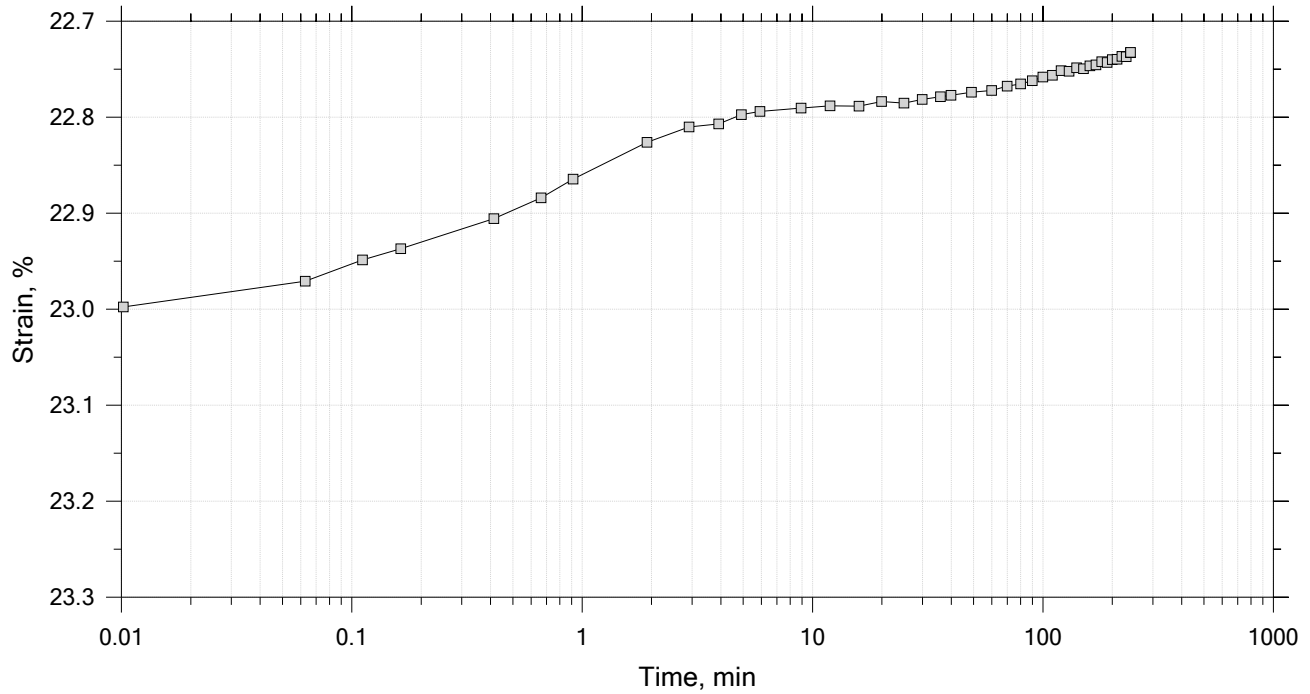
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



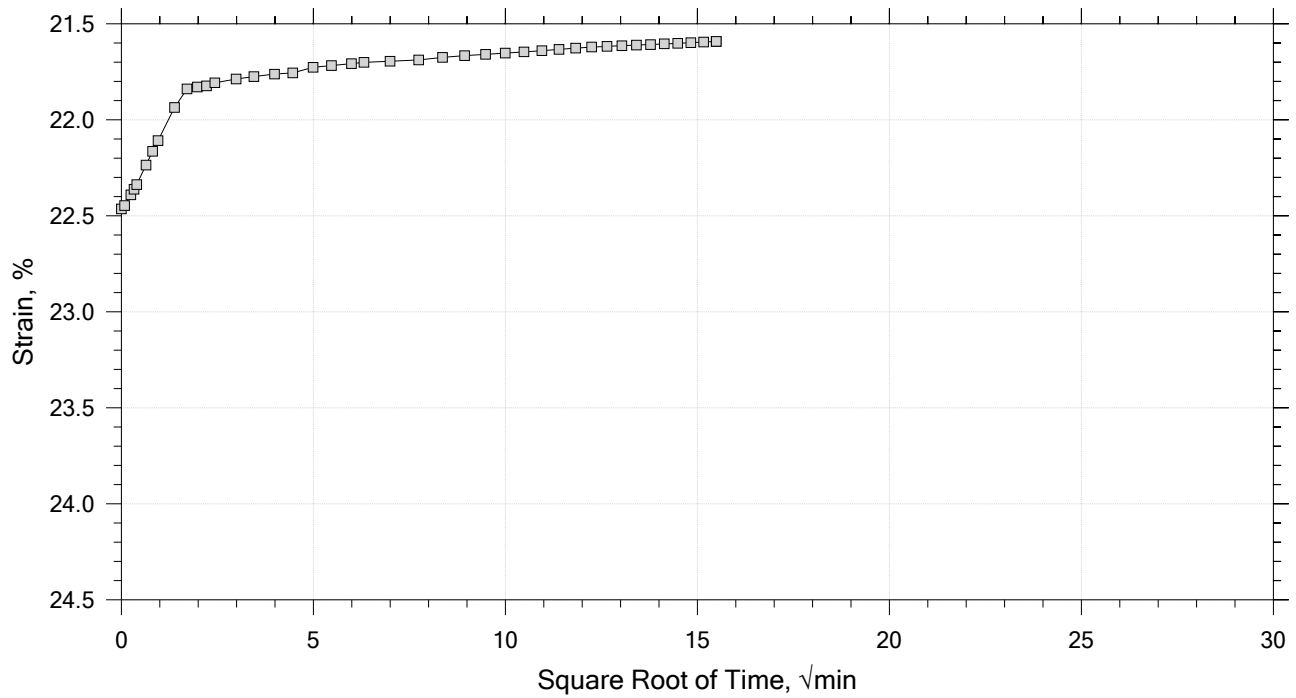
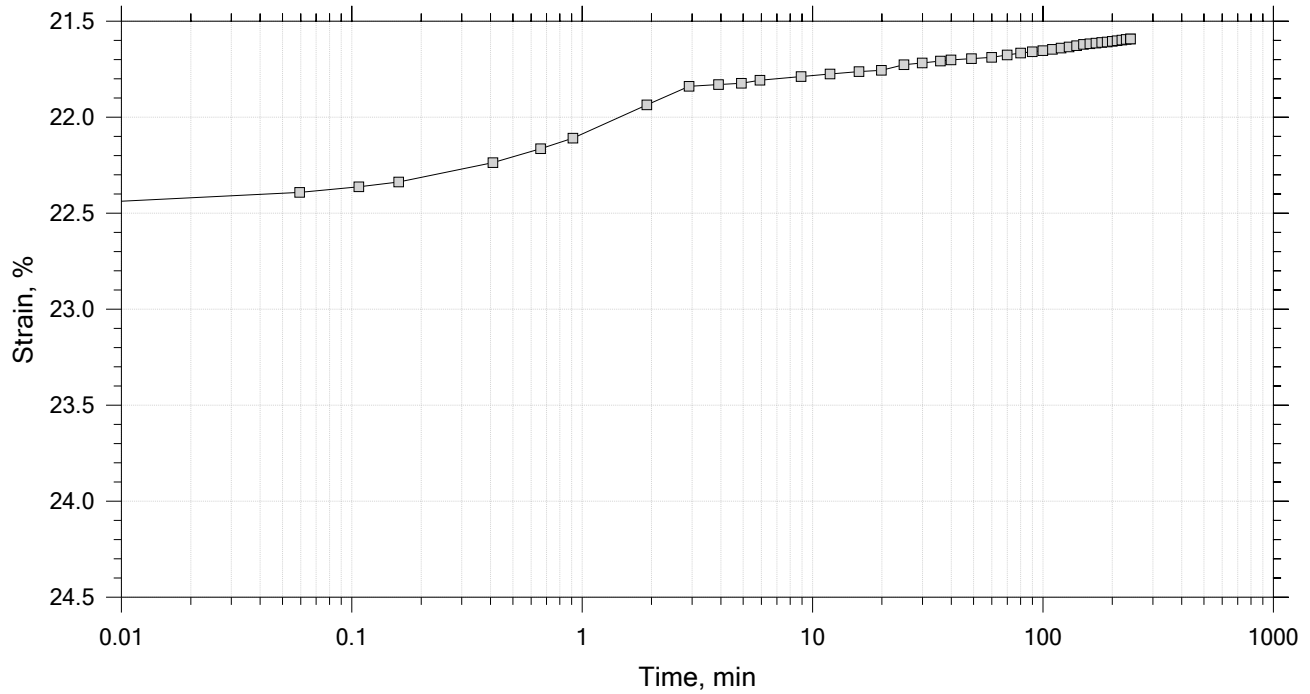
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	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



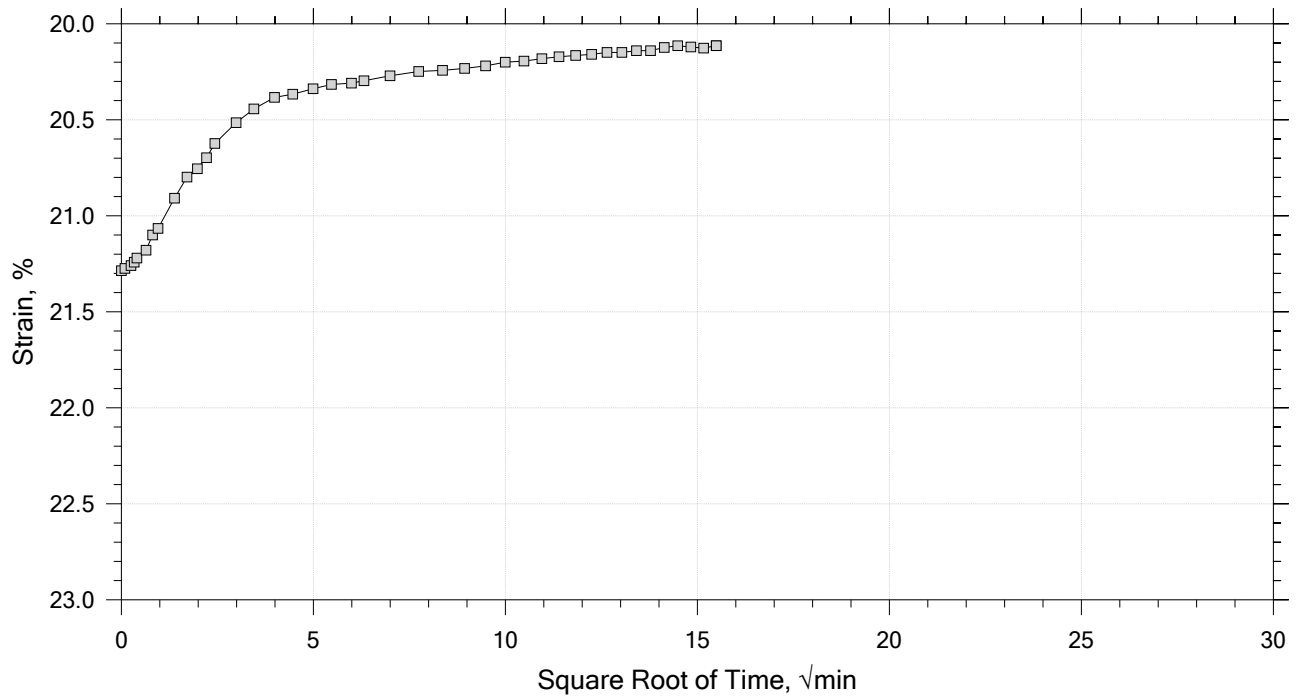
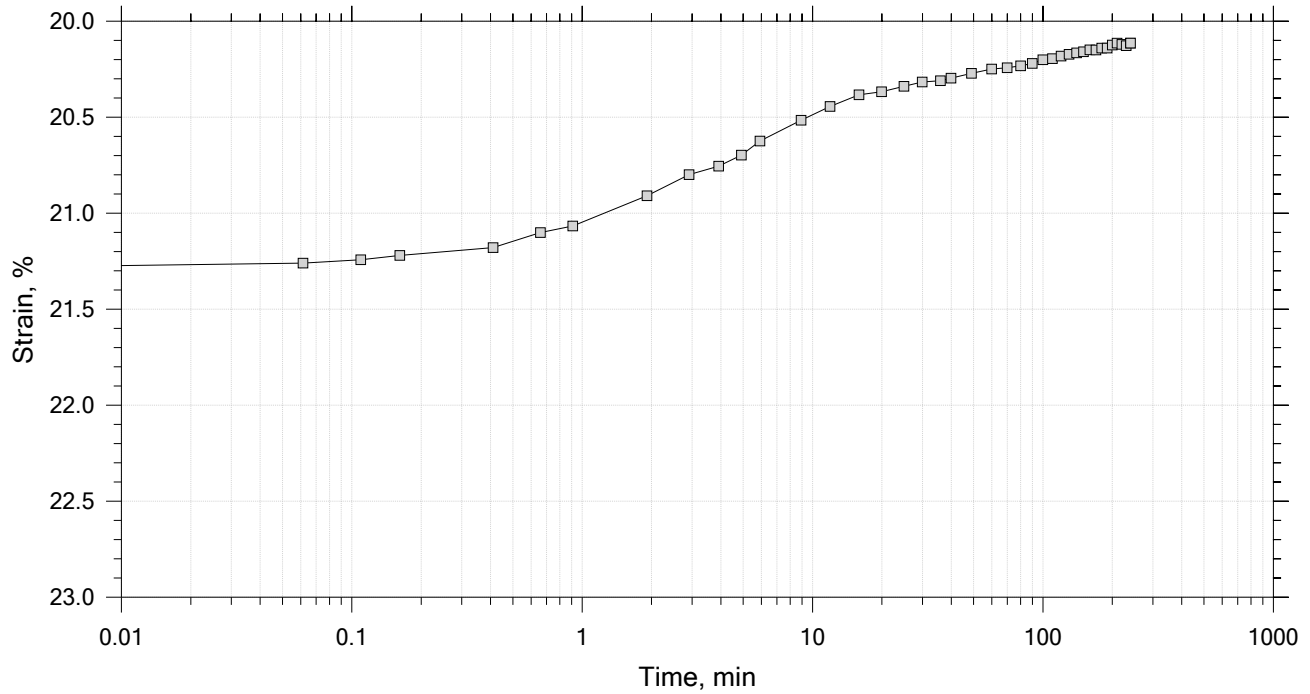
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



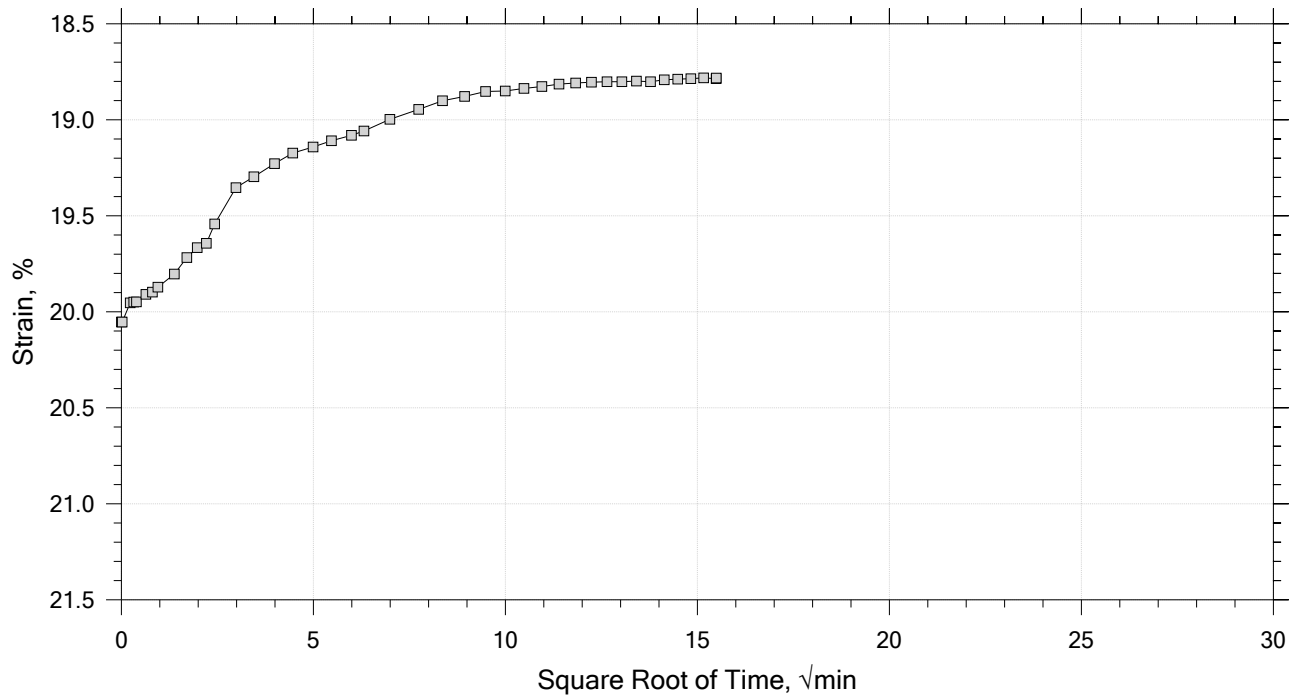
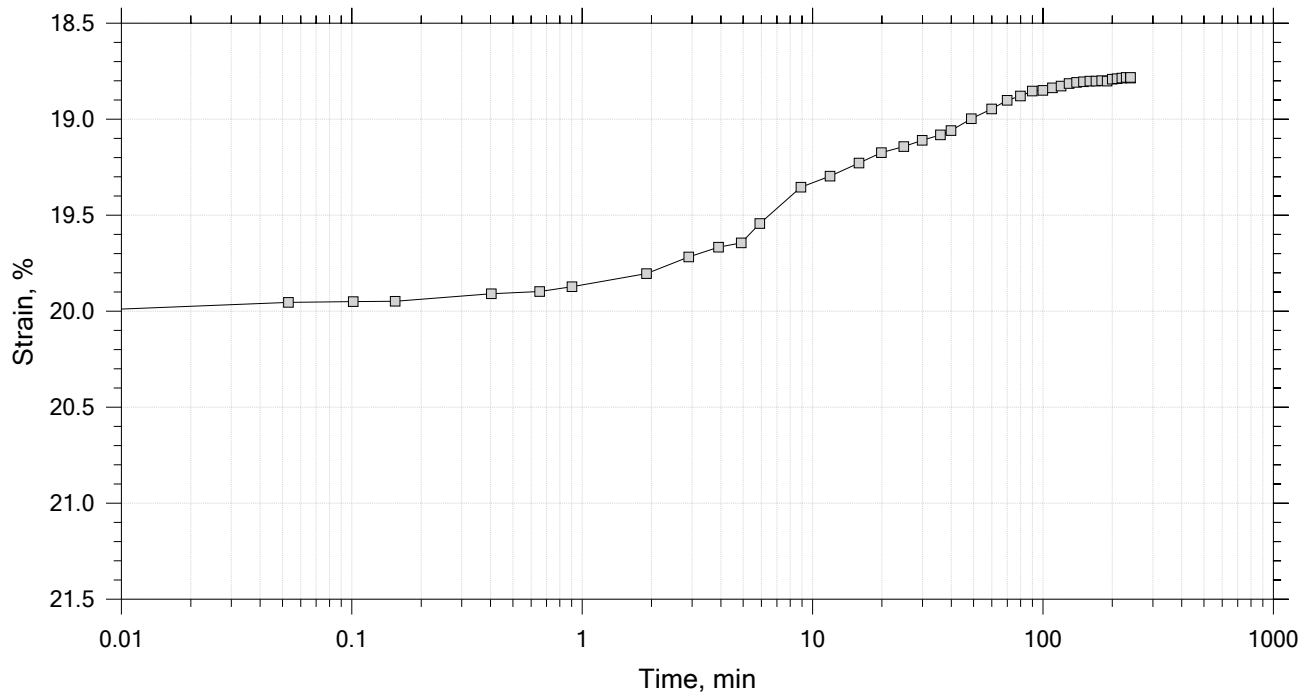
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



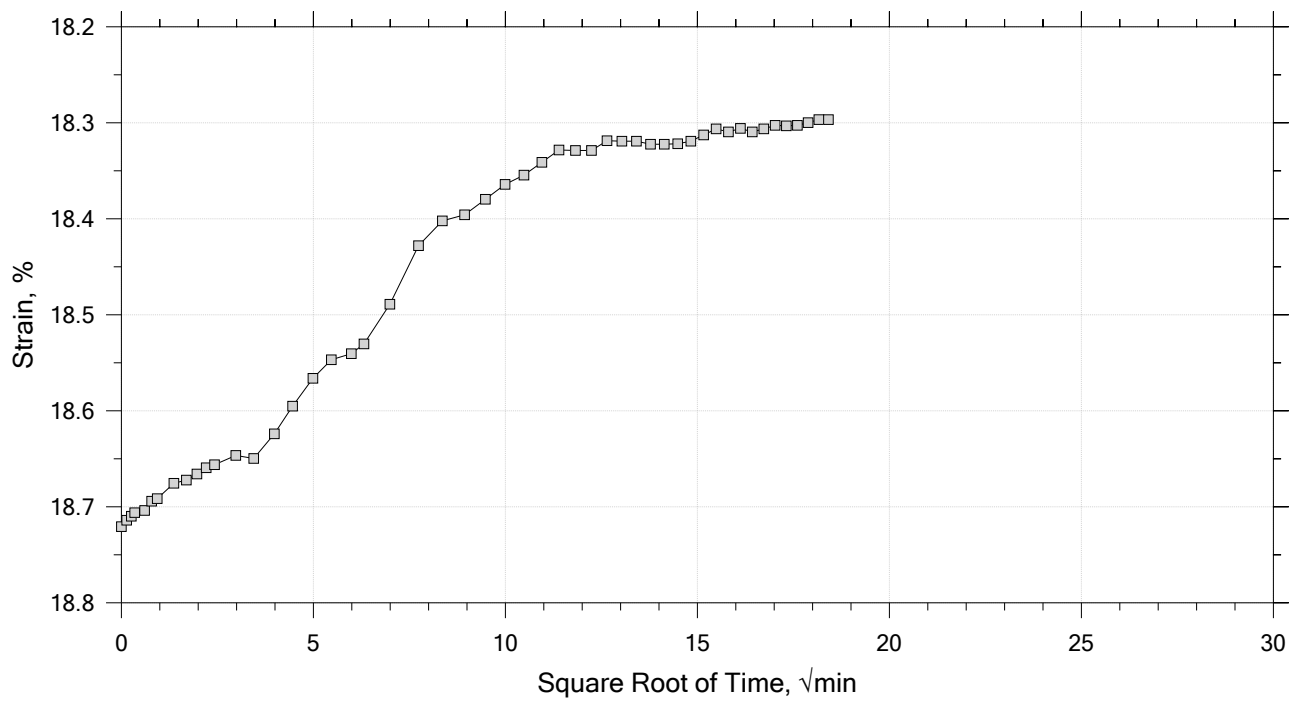
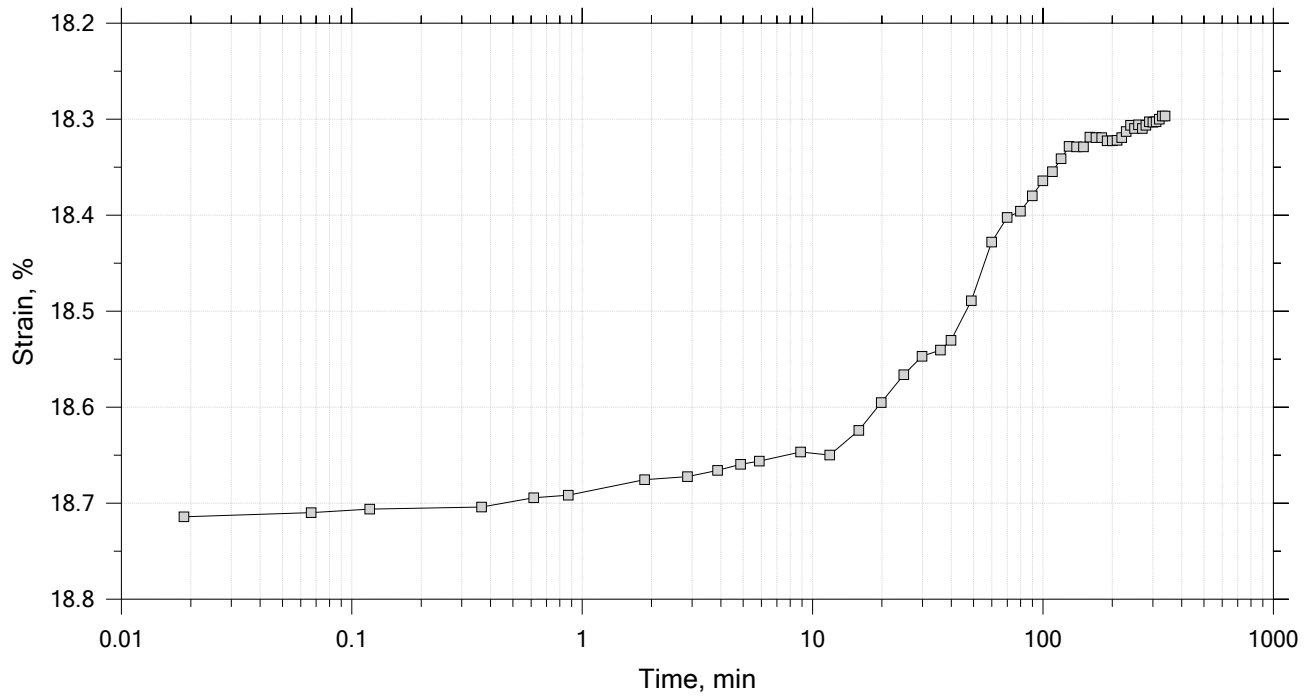
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.76	Liquid Limit: 37
Initial Height: 1.00 in	Initial Void Ratio: 0.933	Plastic Limit: 20
Final Height: 0.82 in	Final Void Ratio: 0.579	Plasticity Index: 17

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-968	RING		A-3022
Mass Container, gm	8.24	109.56	109.56	8.64
Mass Container + Wet Soil, gm	155.83	260.03	248.39	147.12
Mass Container + Dry Soil, gm	118.13	224.29	224.29	123.08
Mass Dry Soil, gm	109.89	114.73	114.73	114.44
Water Content, %	34.31	31.15	21.01	21.01
Void Ratio	---	0.93	0.58	---
Degree of Saturation, %	---	92.07	100.00	---
Dry Unit Weight, pcf	---	89.039	108.98	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

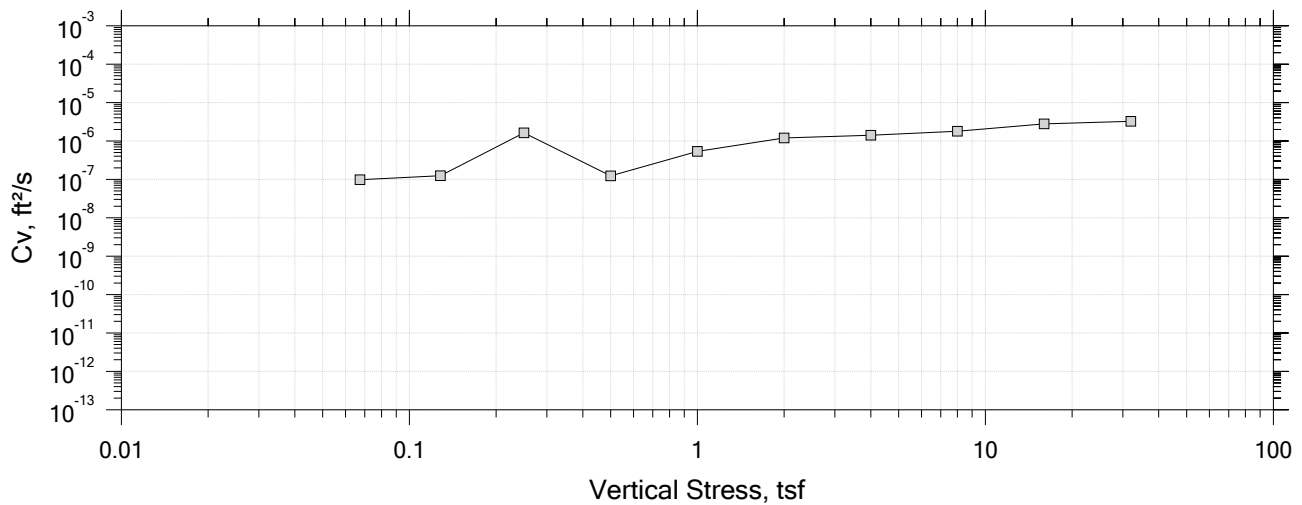
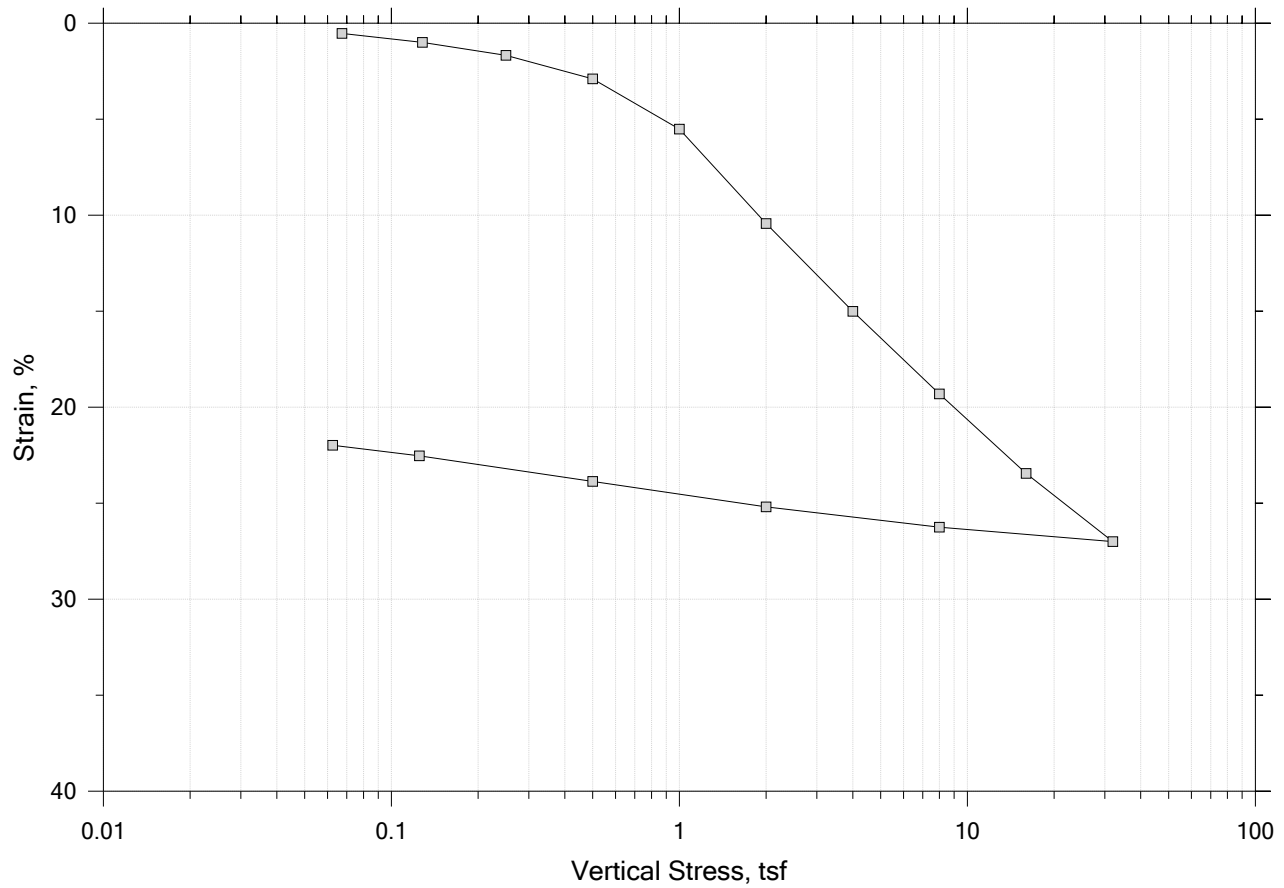
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-BST1-101	Tested By: md	Checked By: mcm
	Sample No.: 1-U	Test Date: 07/22/19	Depth: 10-12 ft
	Test No.: IP-16	Sample Type: Intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0789 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

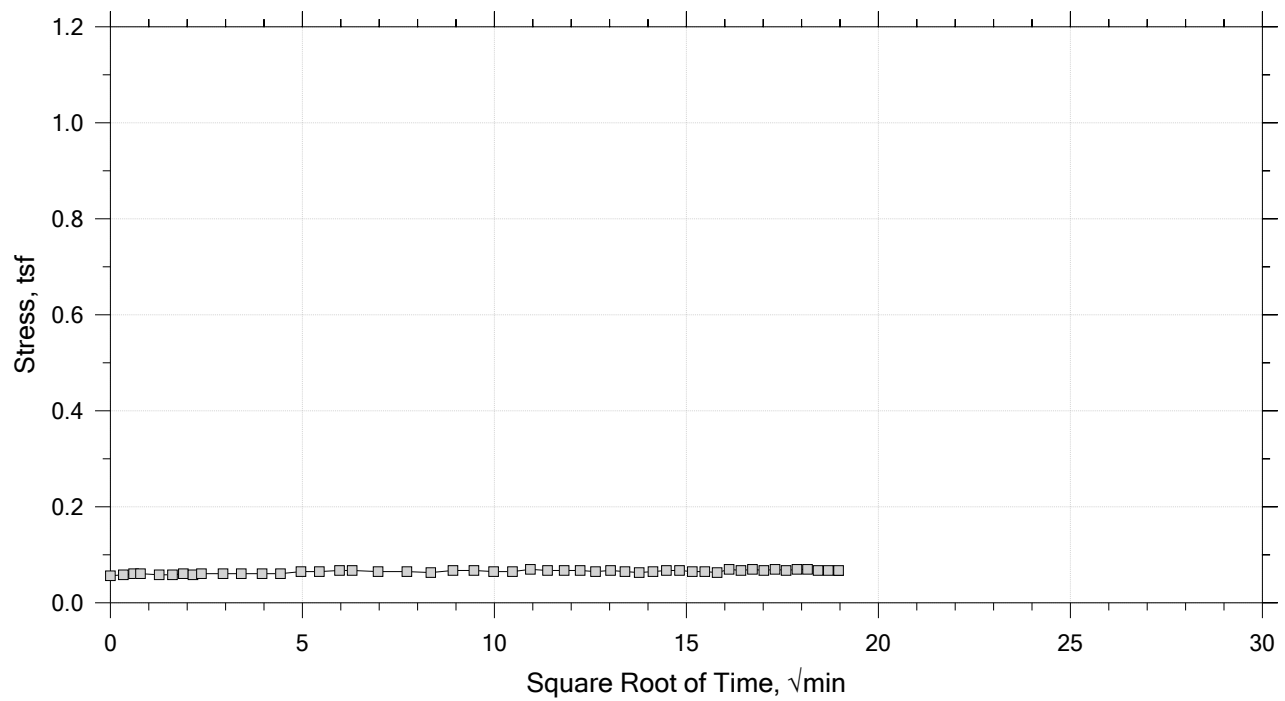
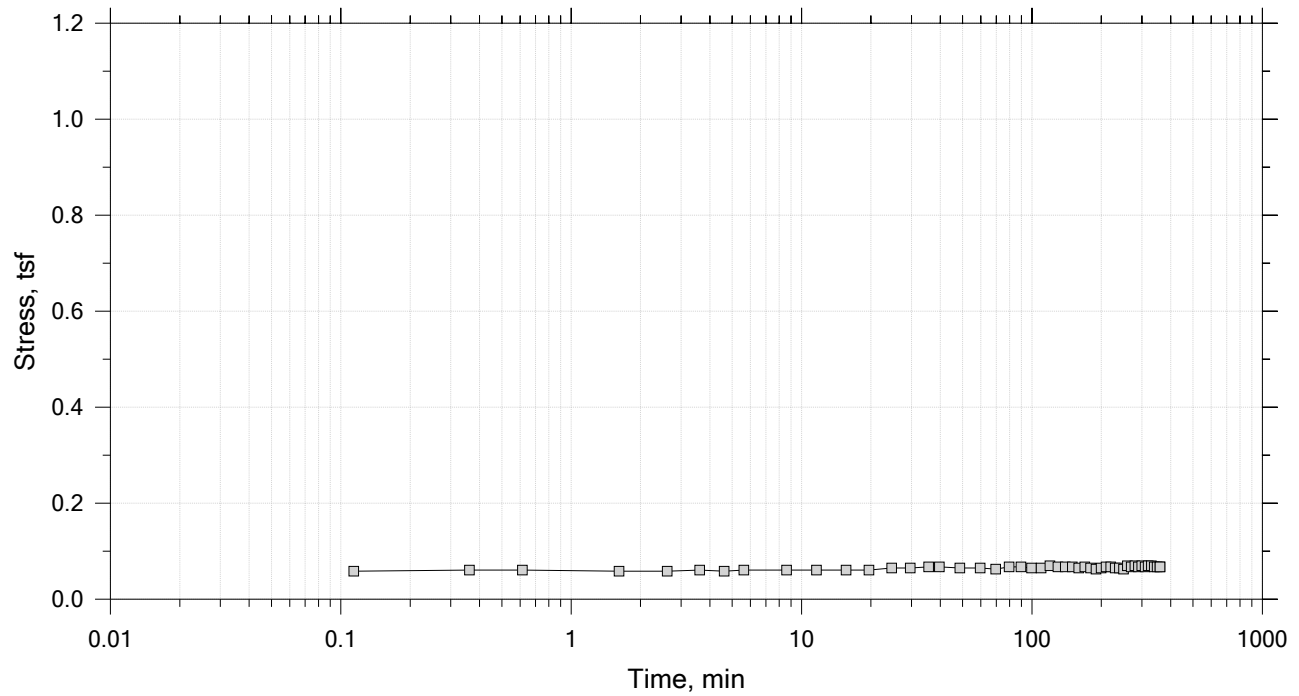
Summary Report




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

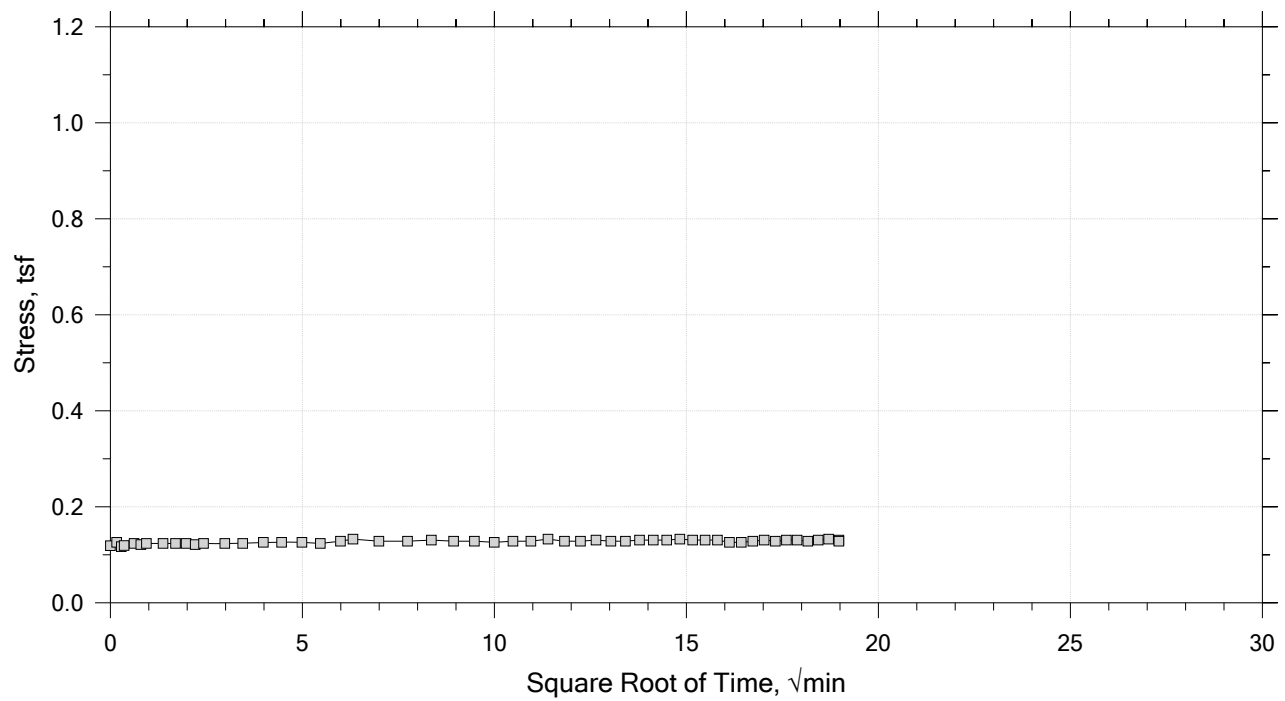
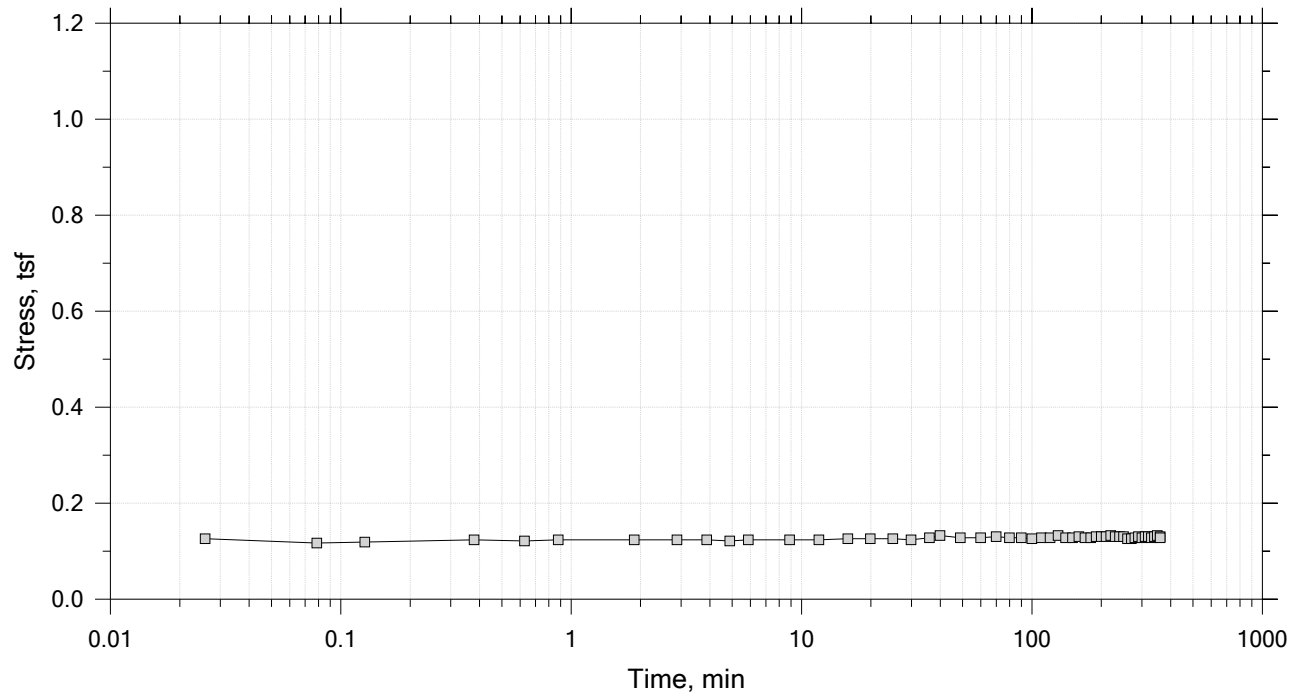
Time Curve 1 of 15
Constant Volume Step
Stress: 0.0673 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15
Constant Volume Step
Stress: 0.128 tsf



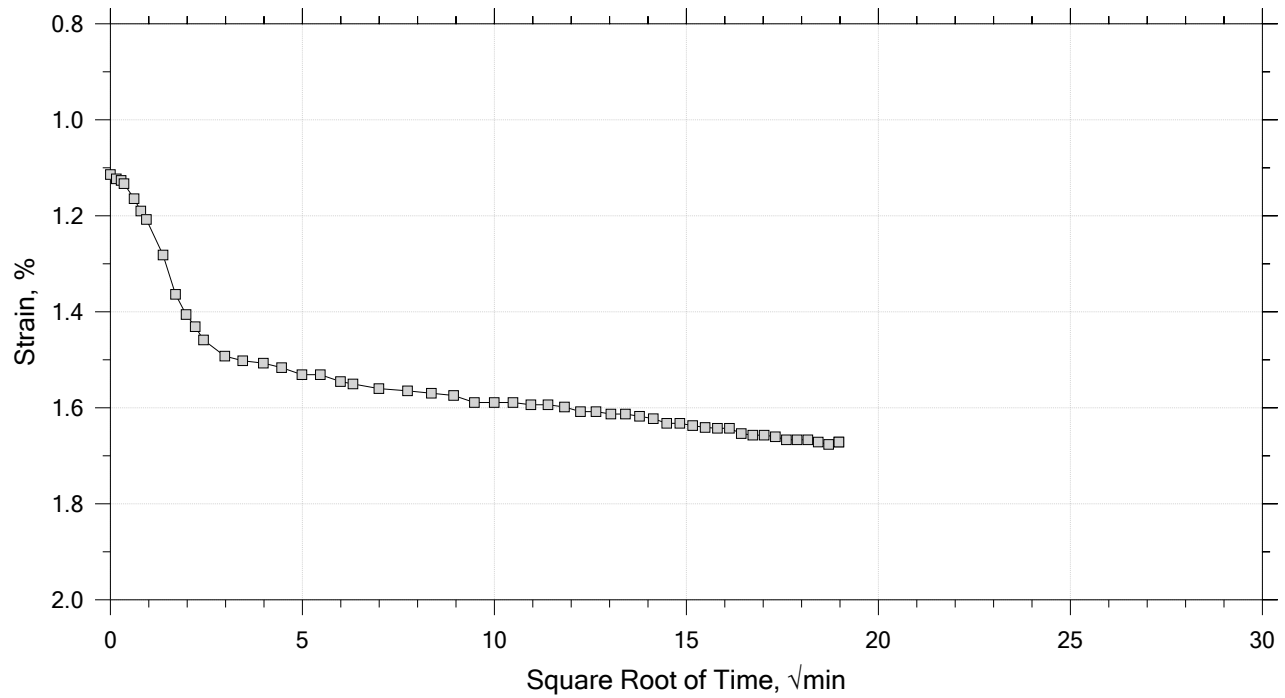
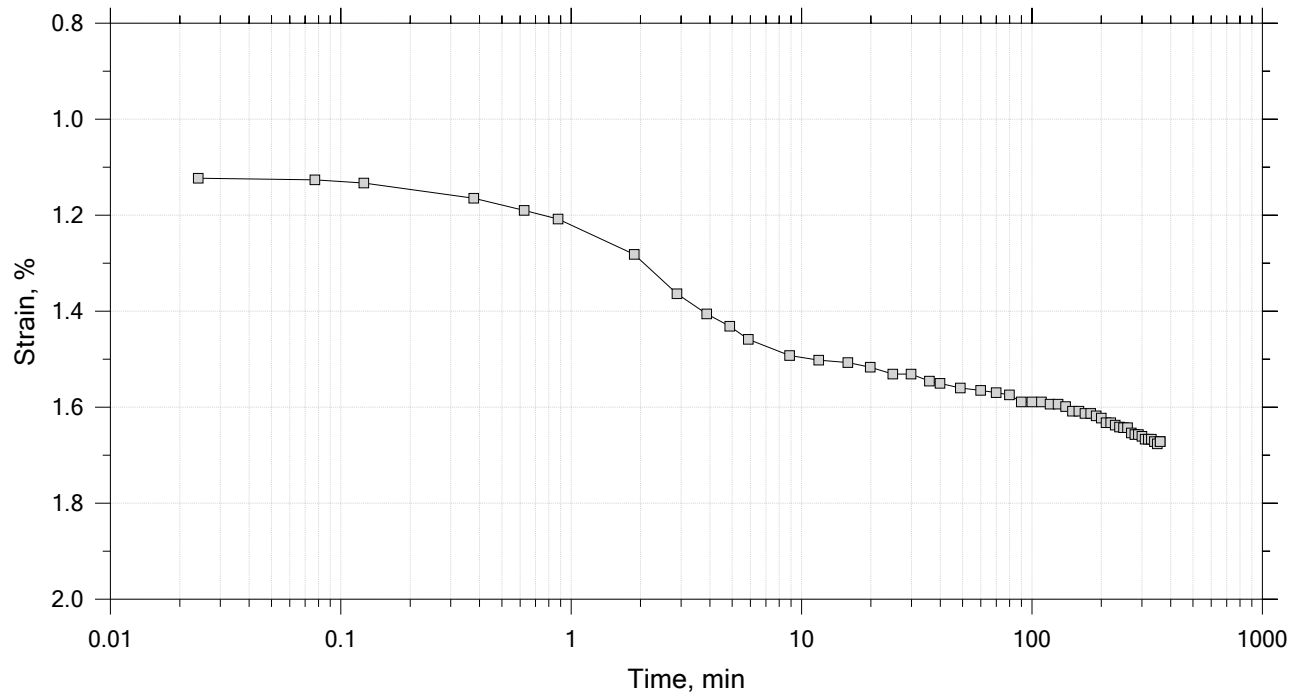
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



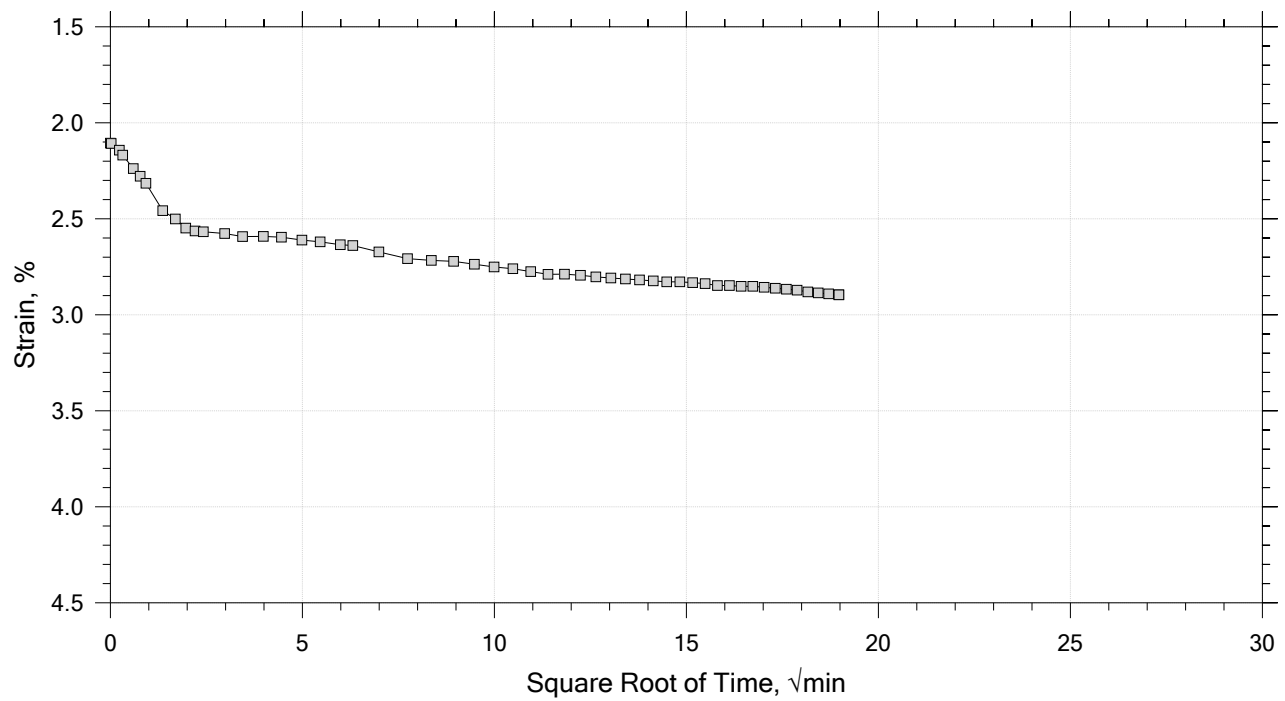
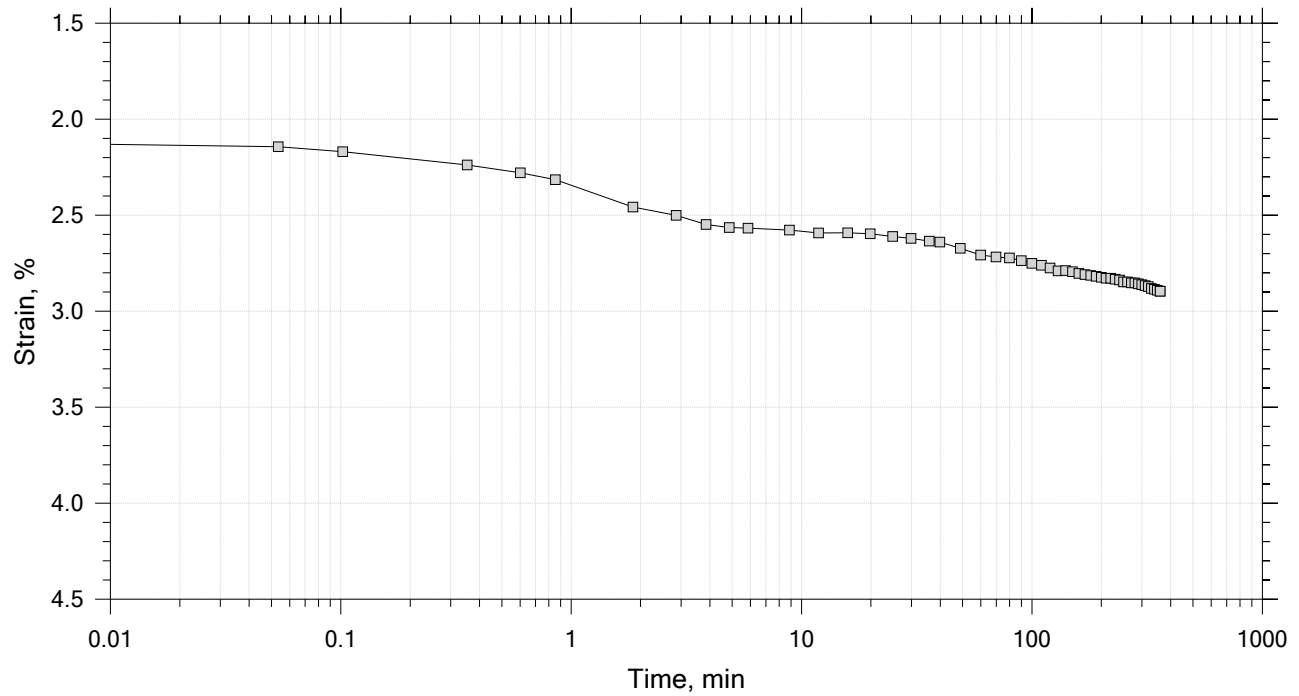
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



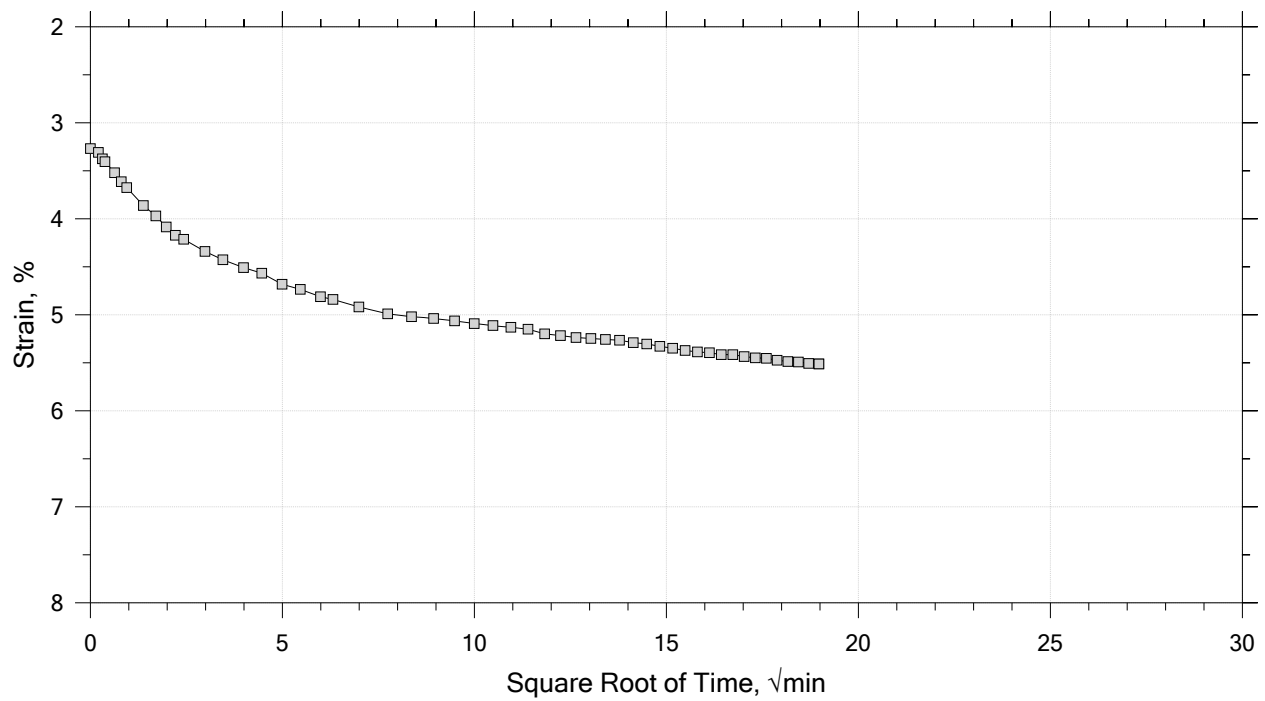
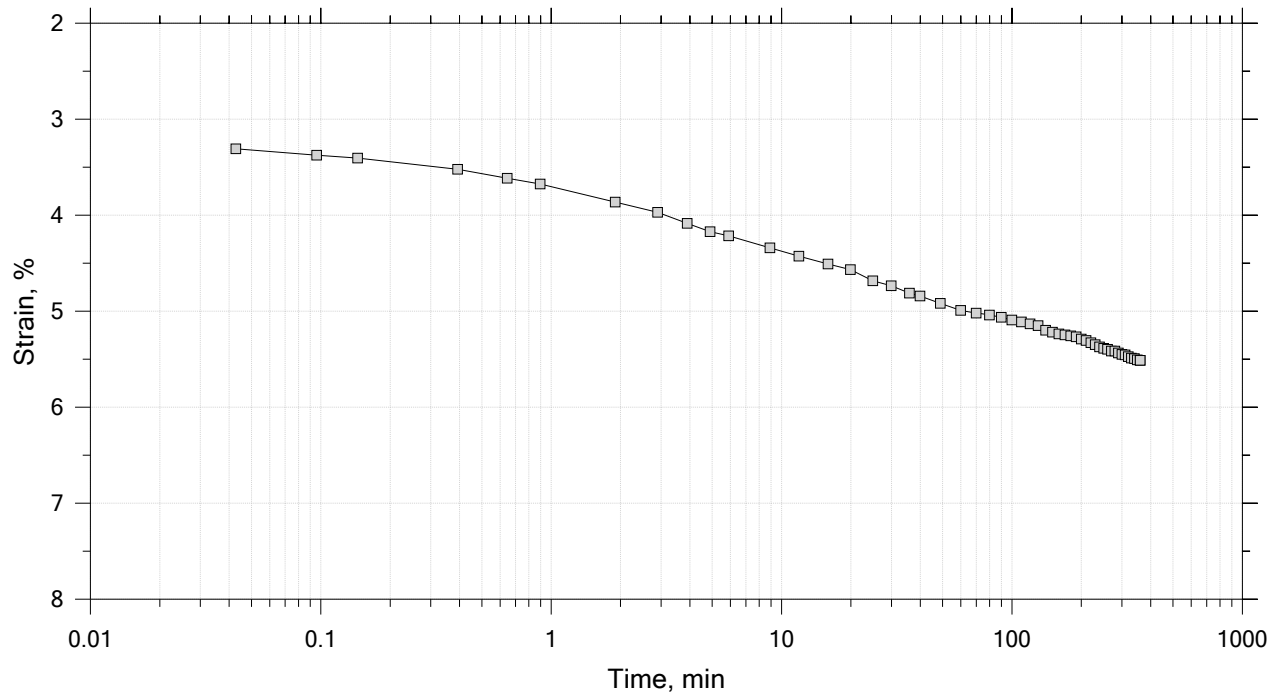
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



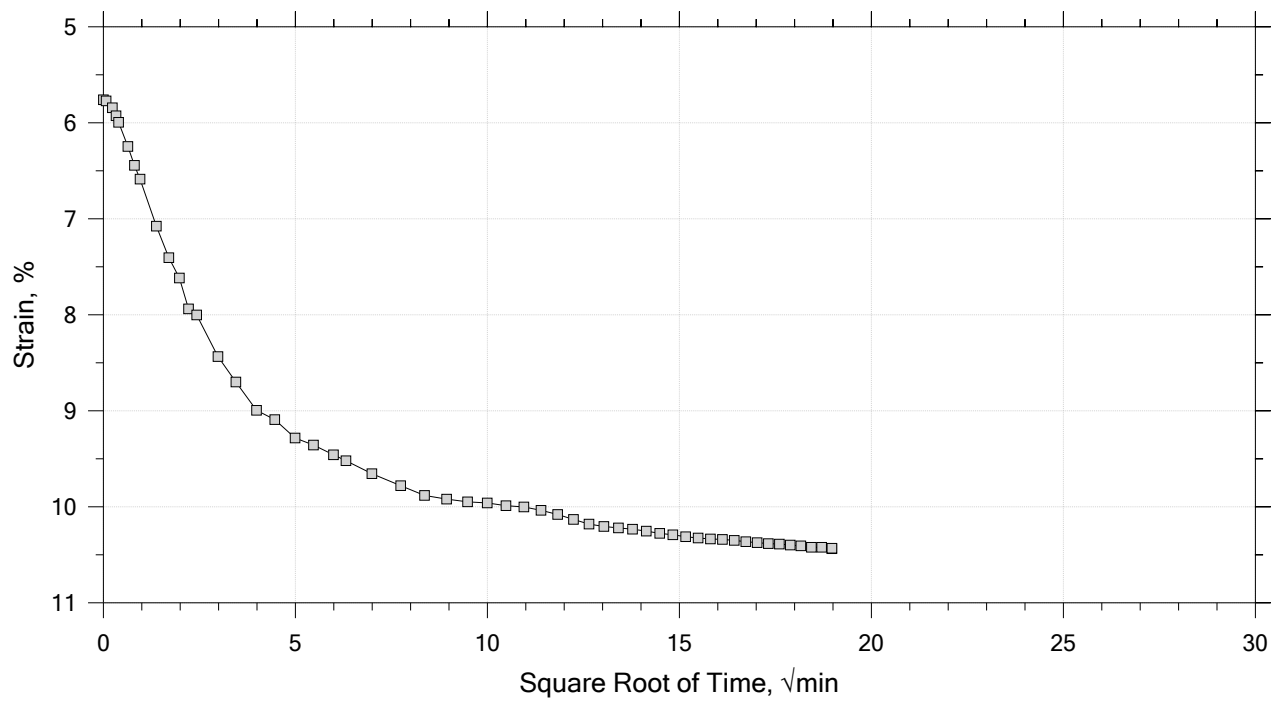
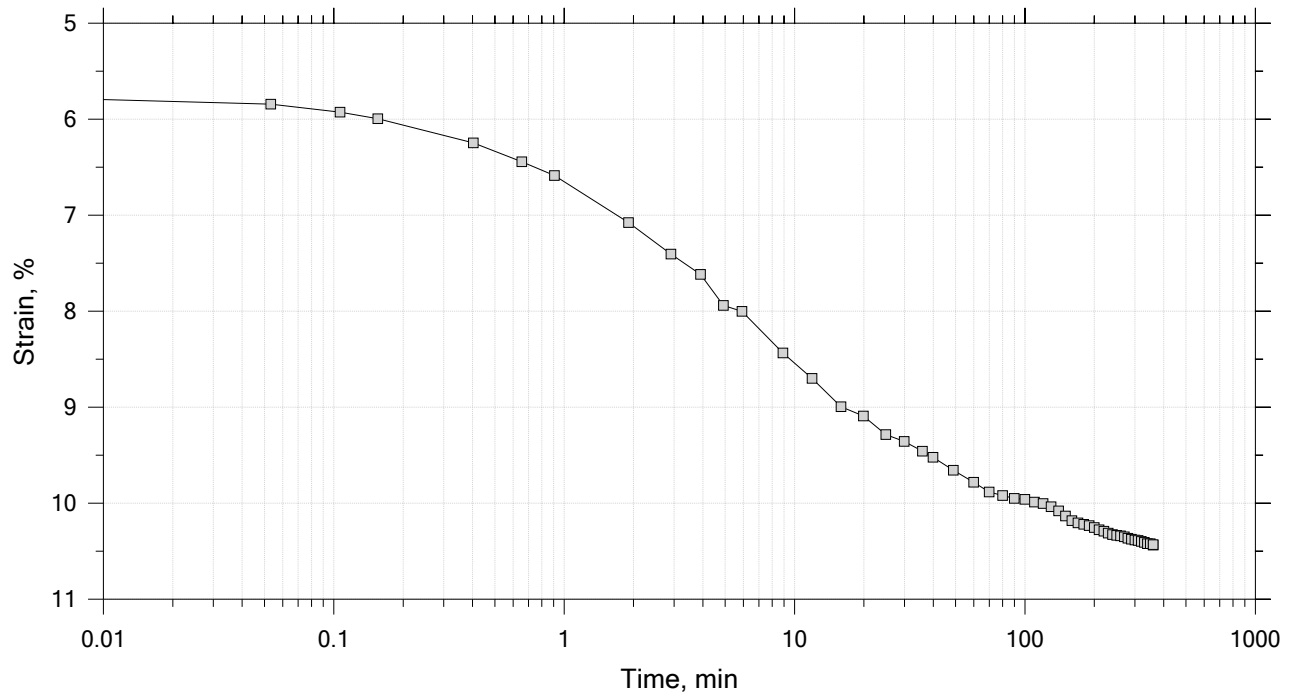
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



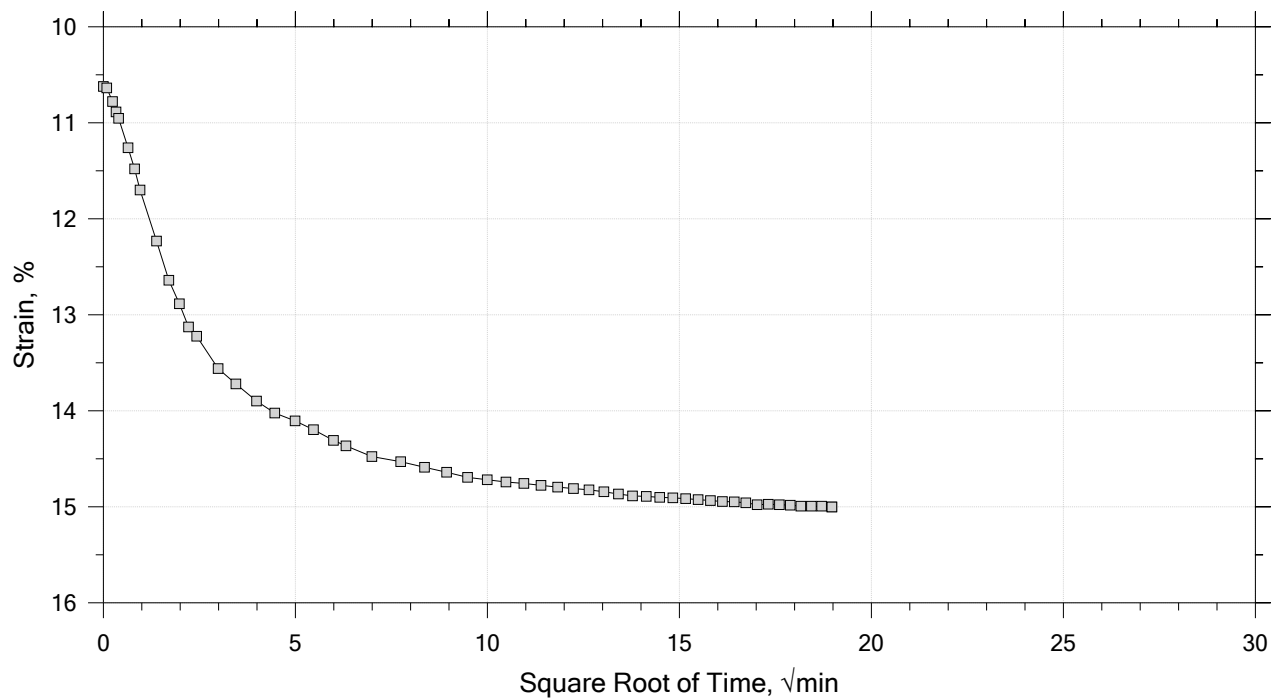
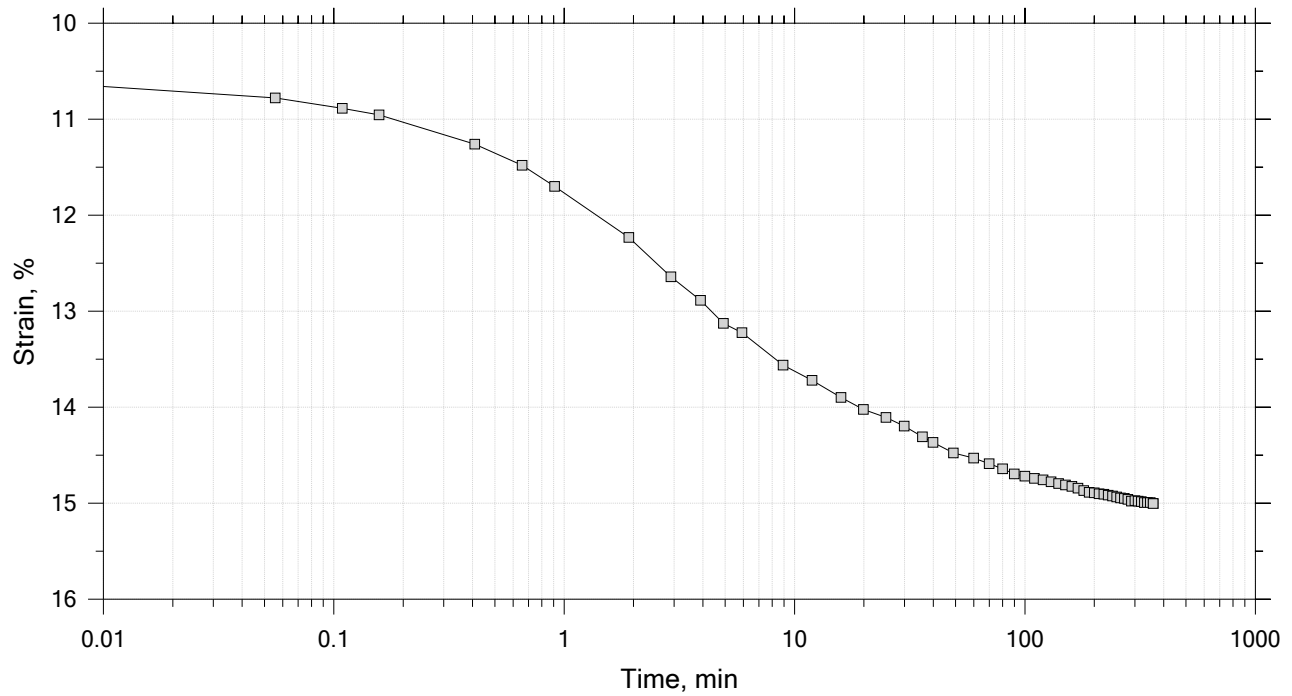
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



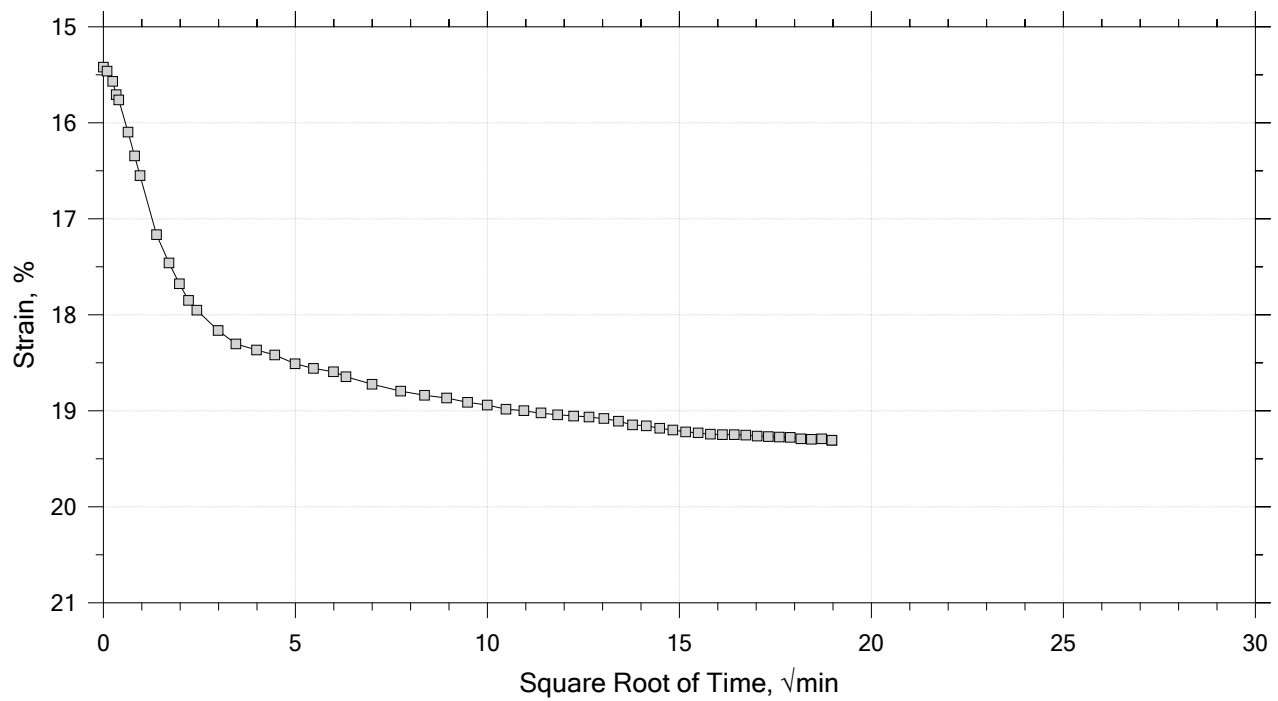
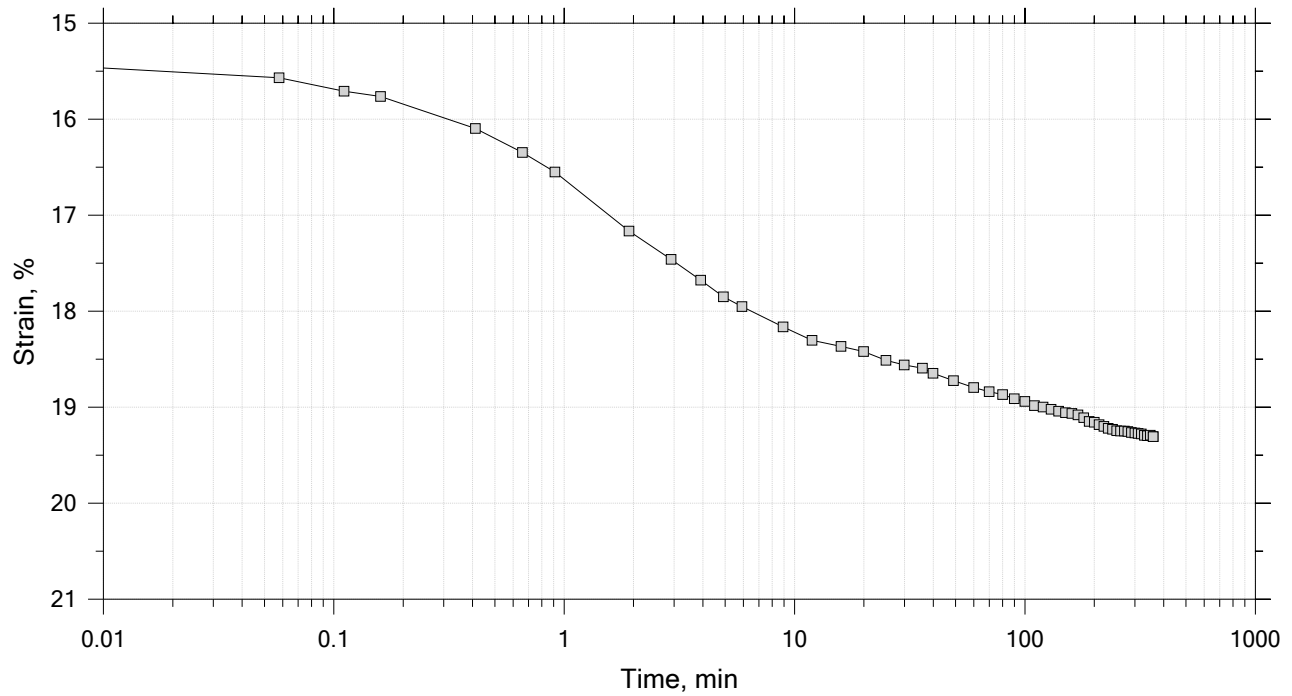
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



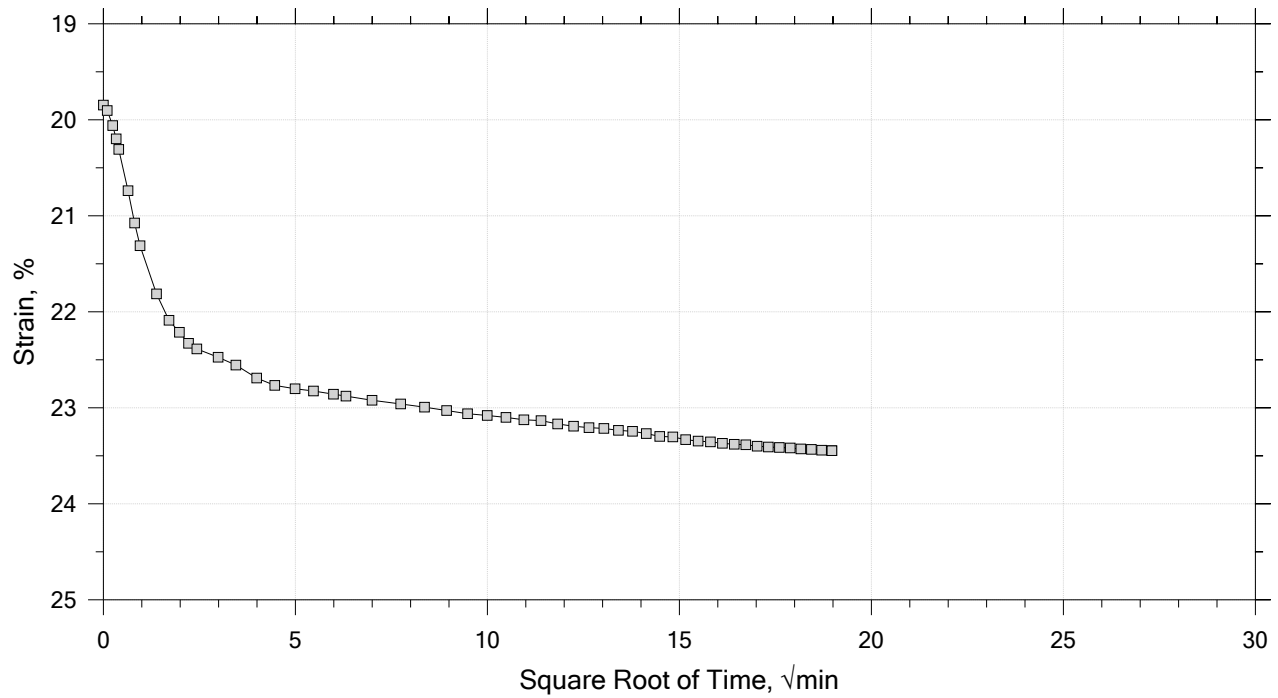
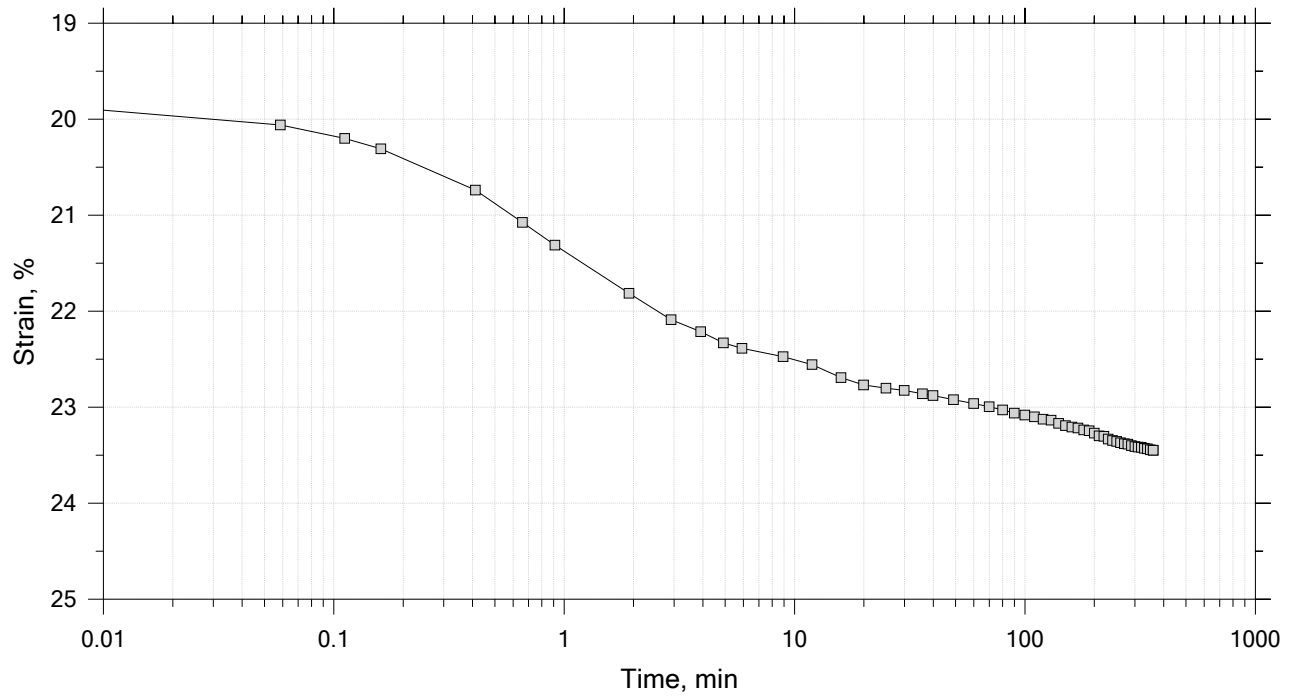
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	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



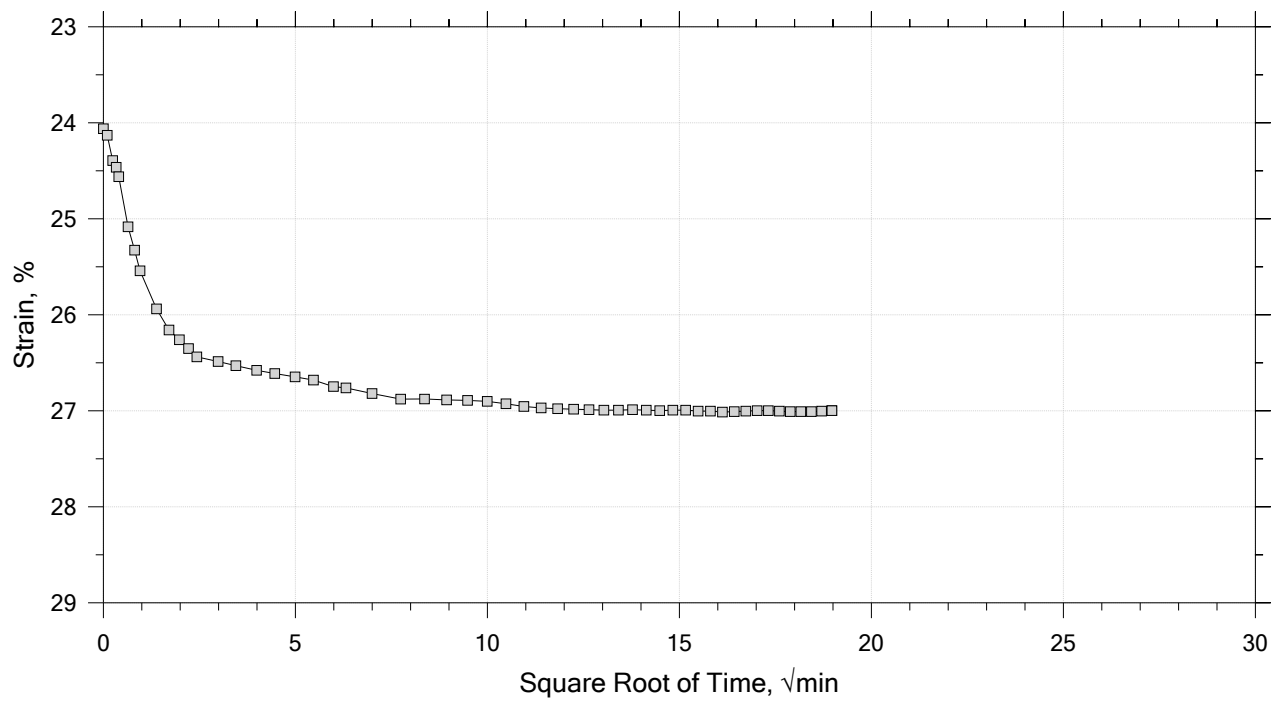
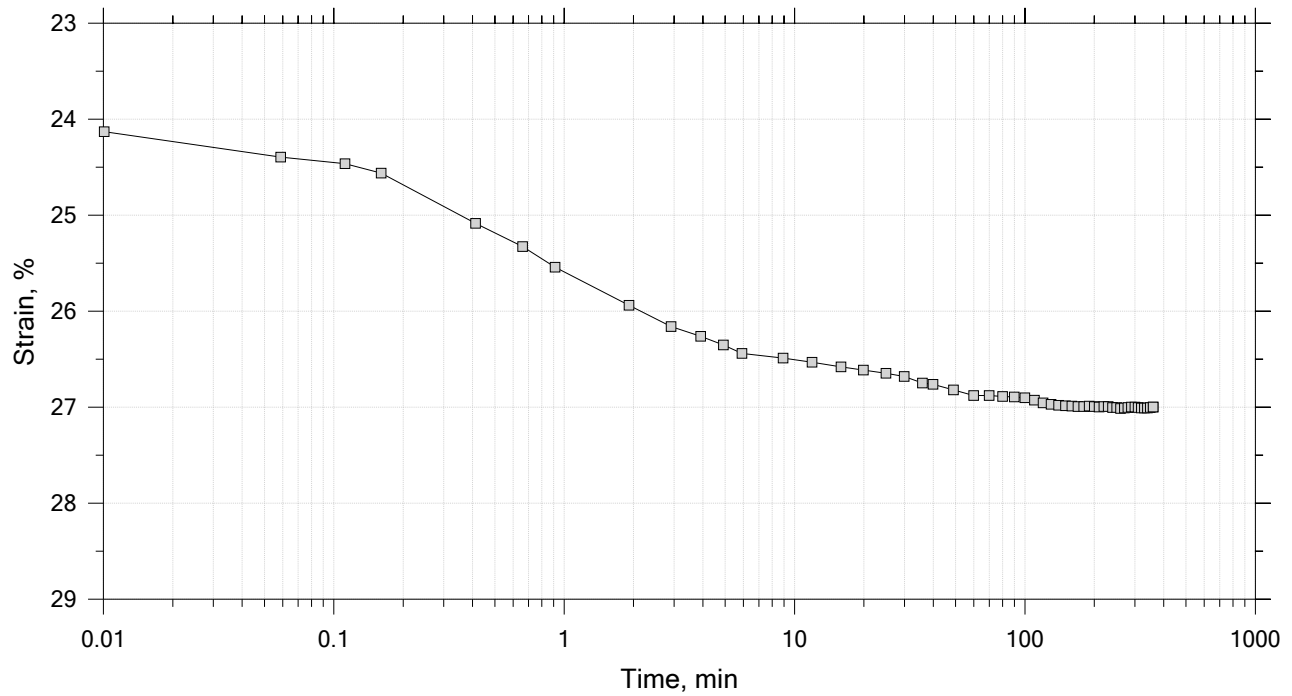
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



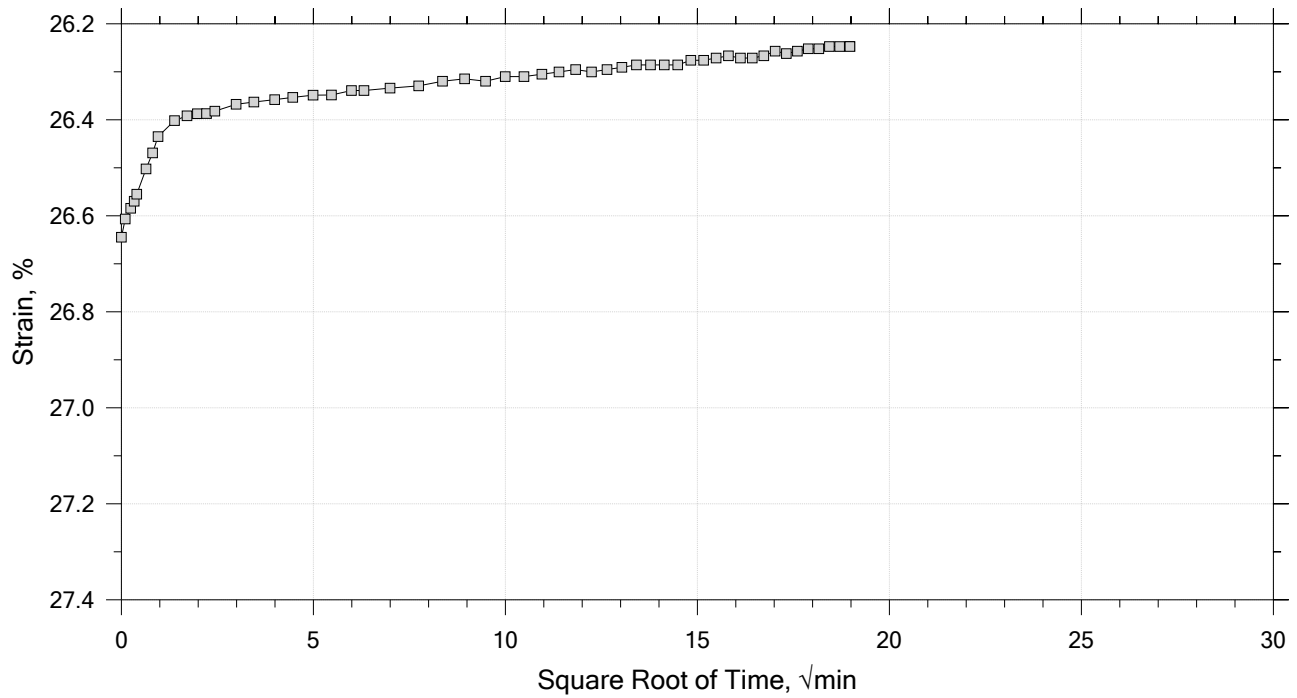
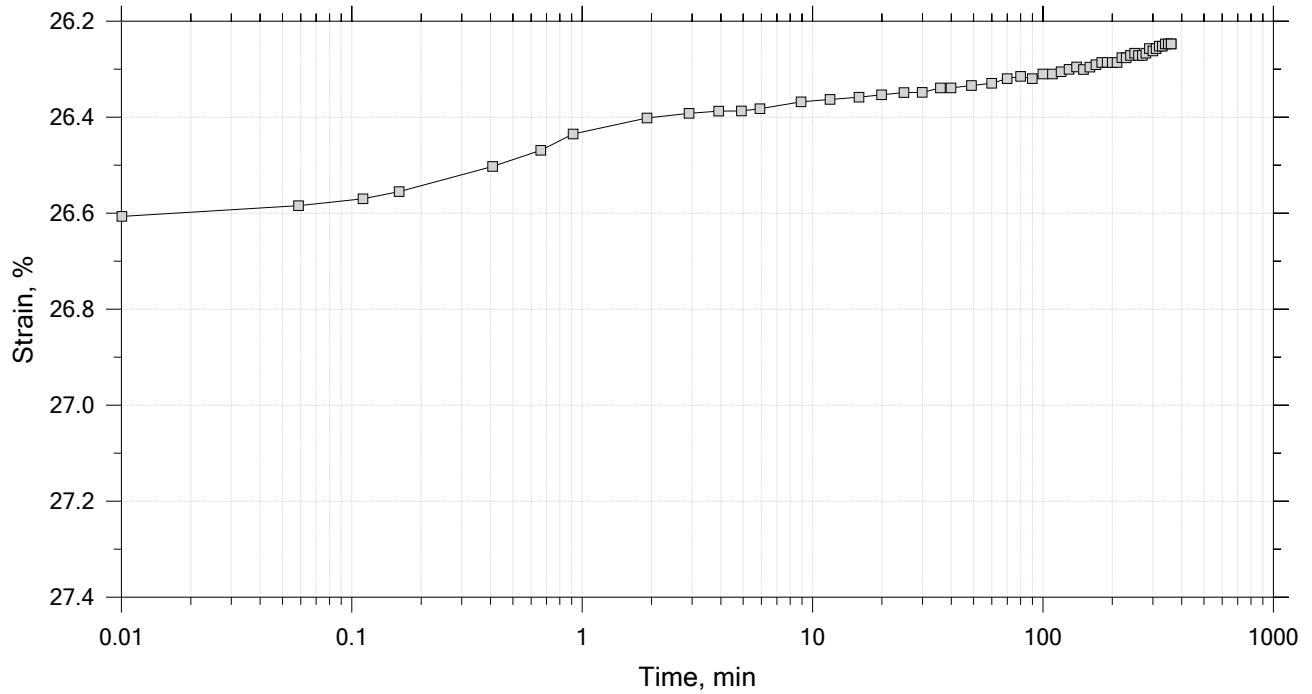
	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



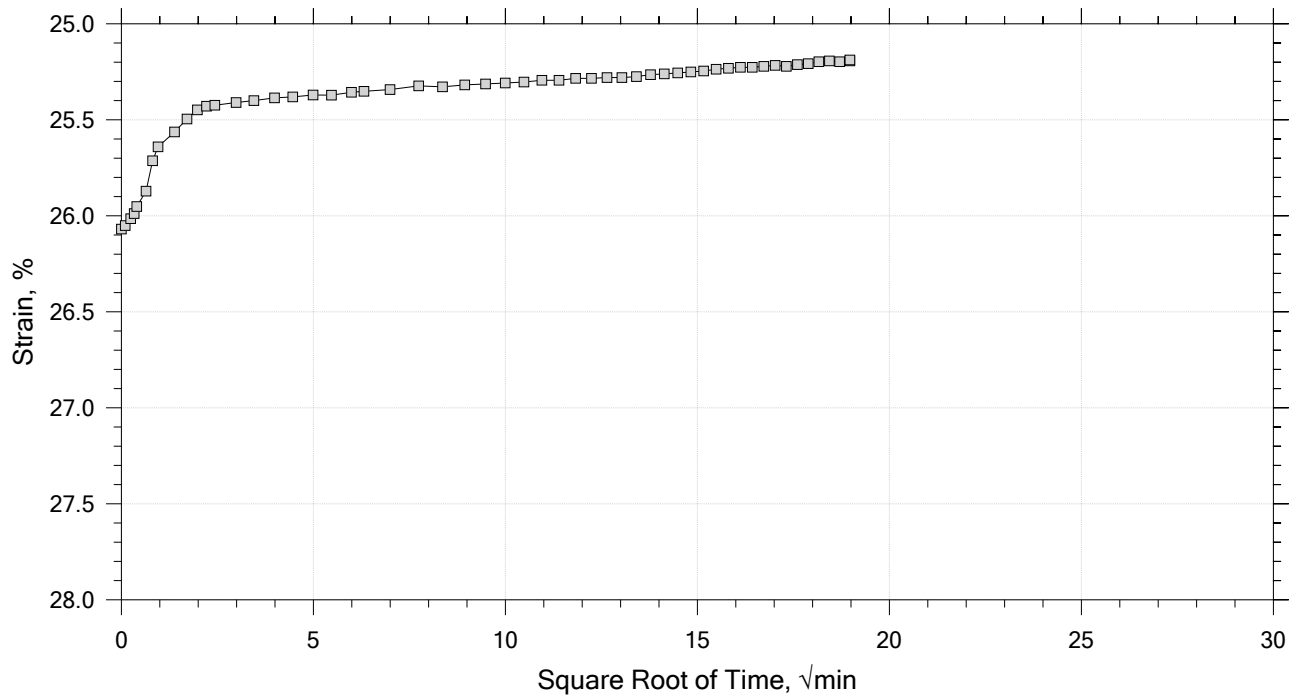
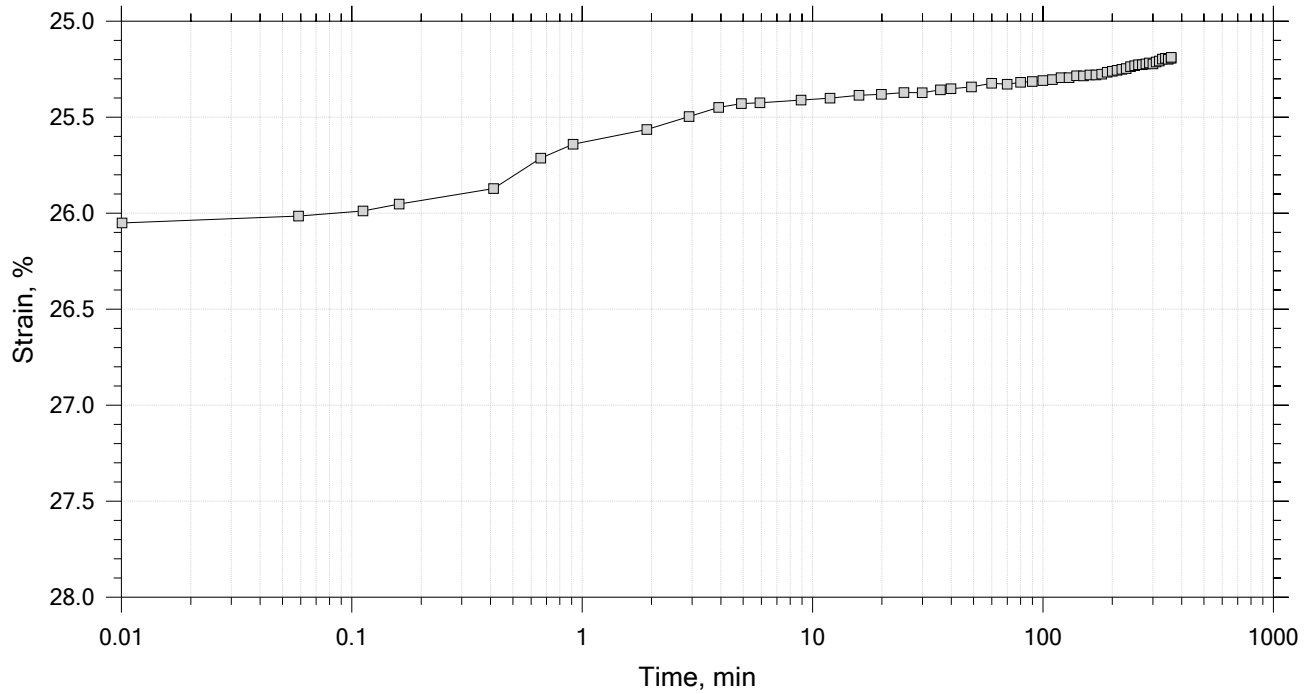
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	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



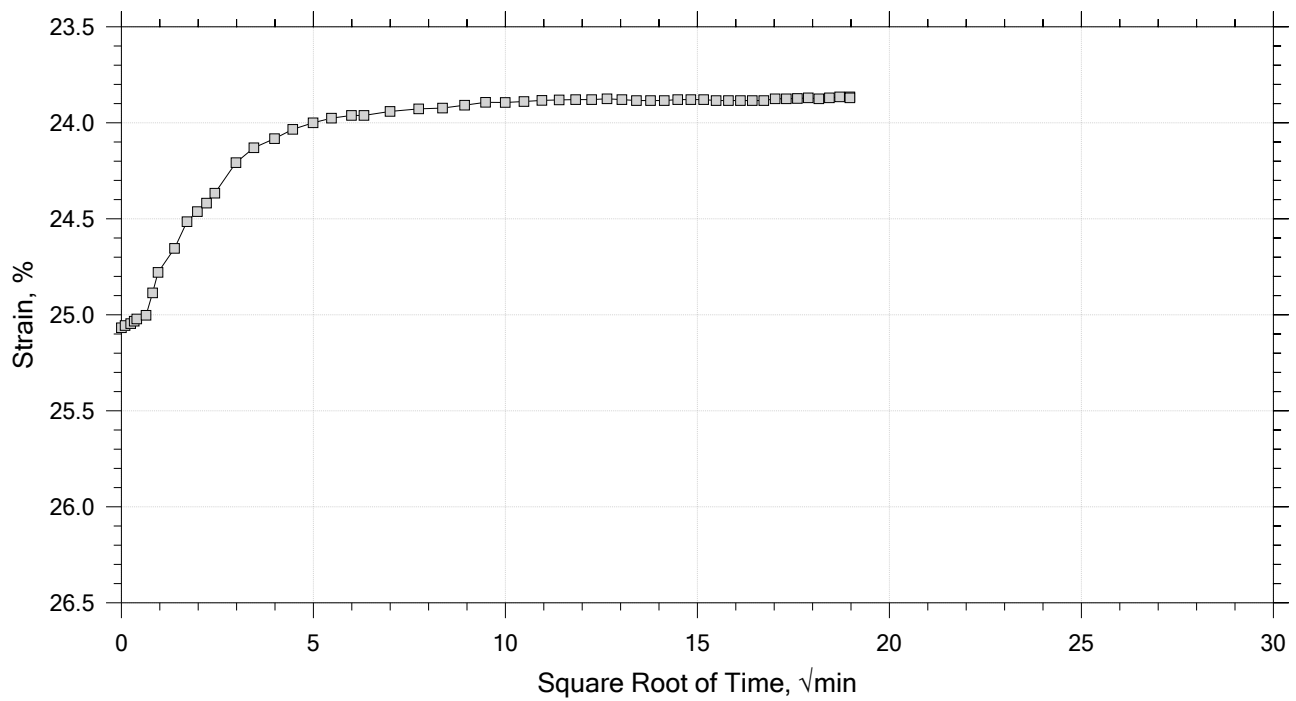
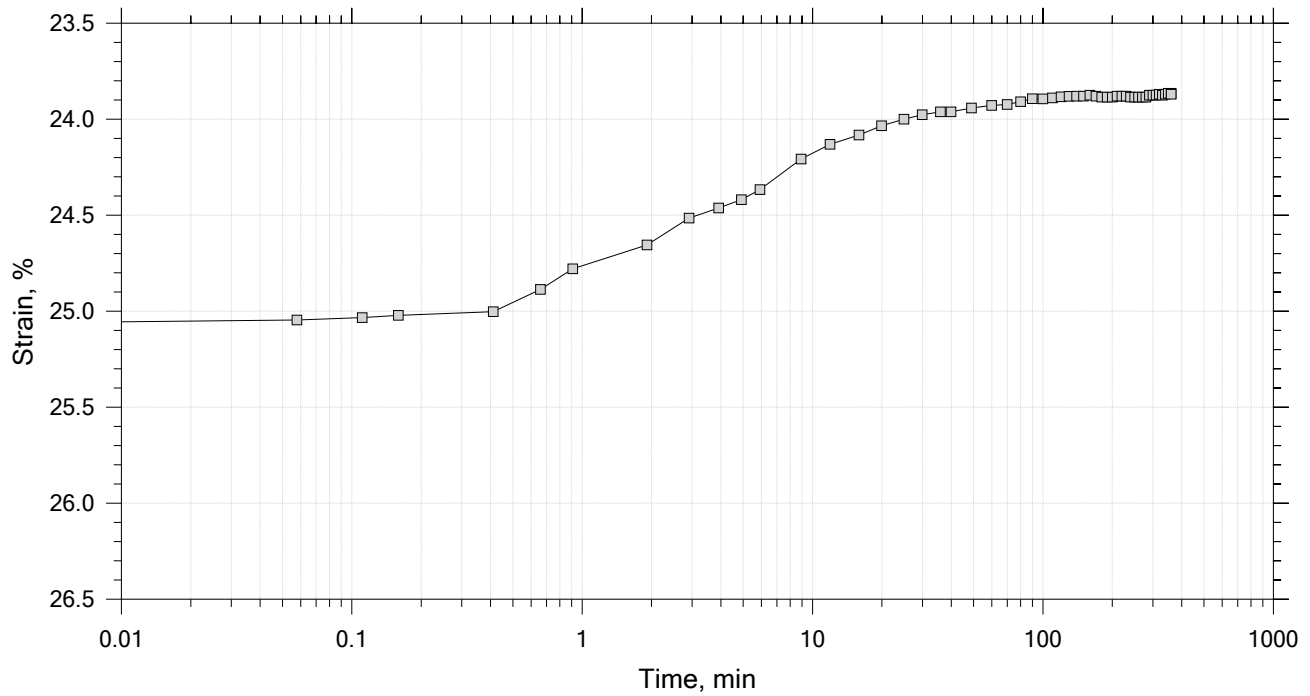
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	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



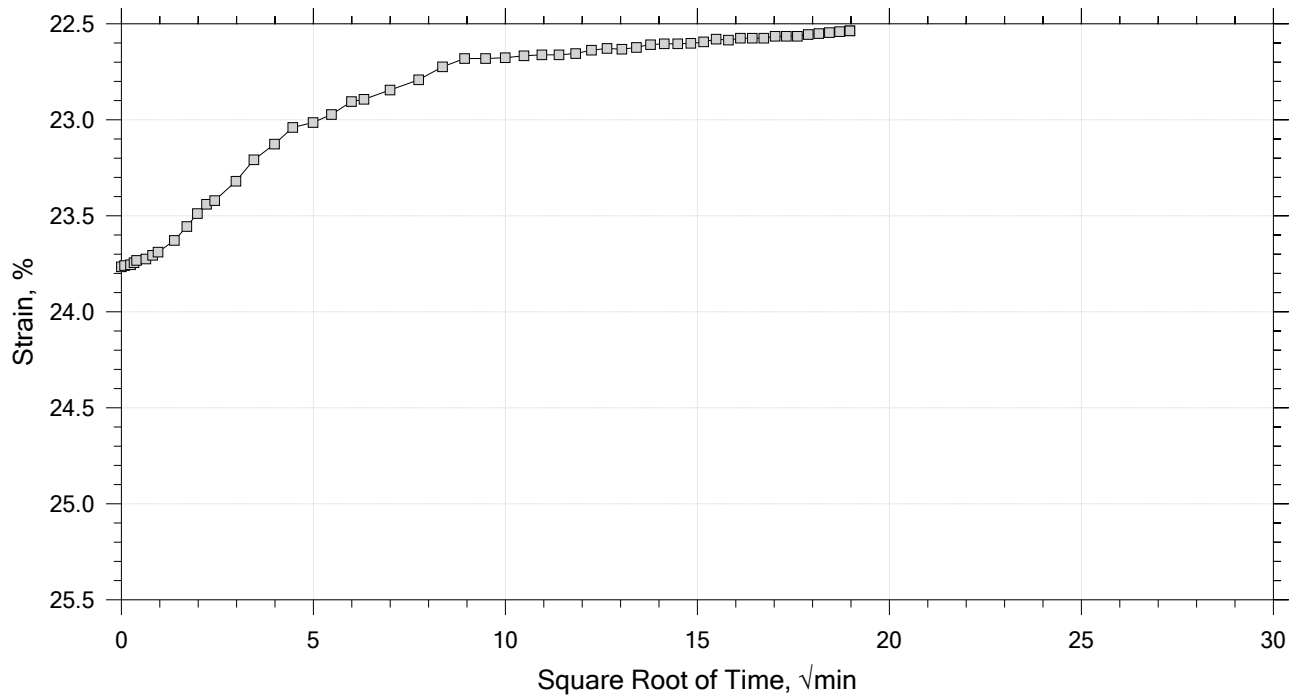
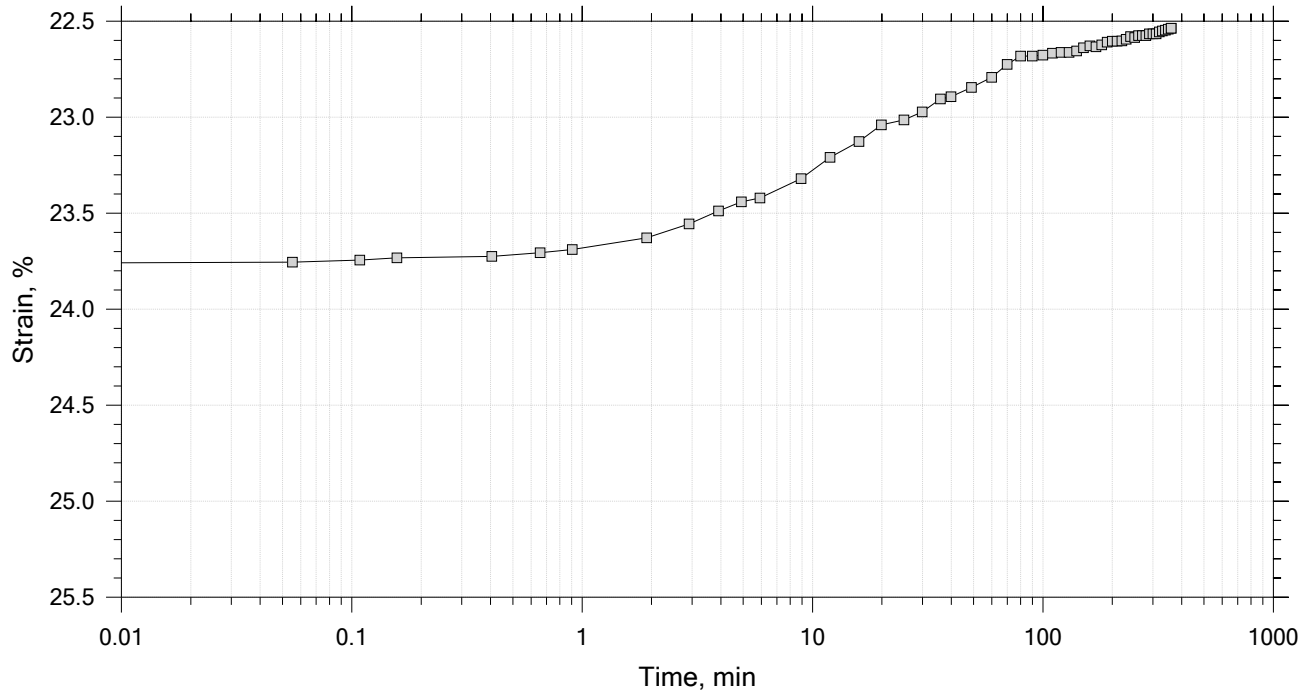
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	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



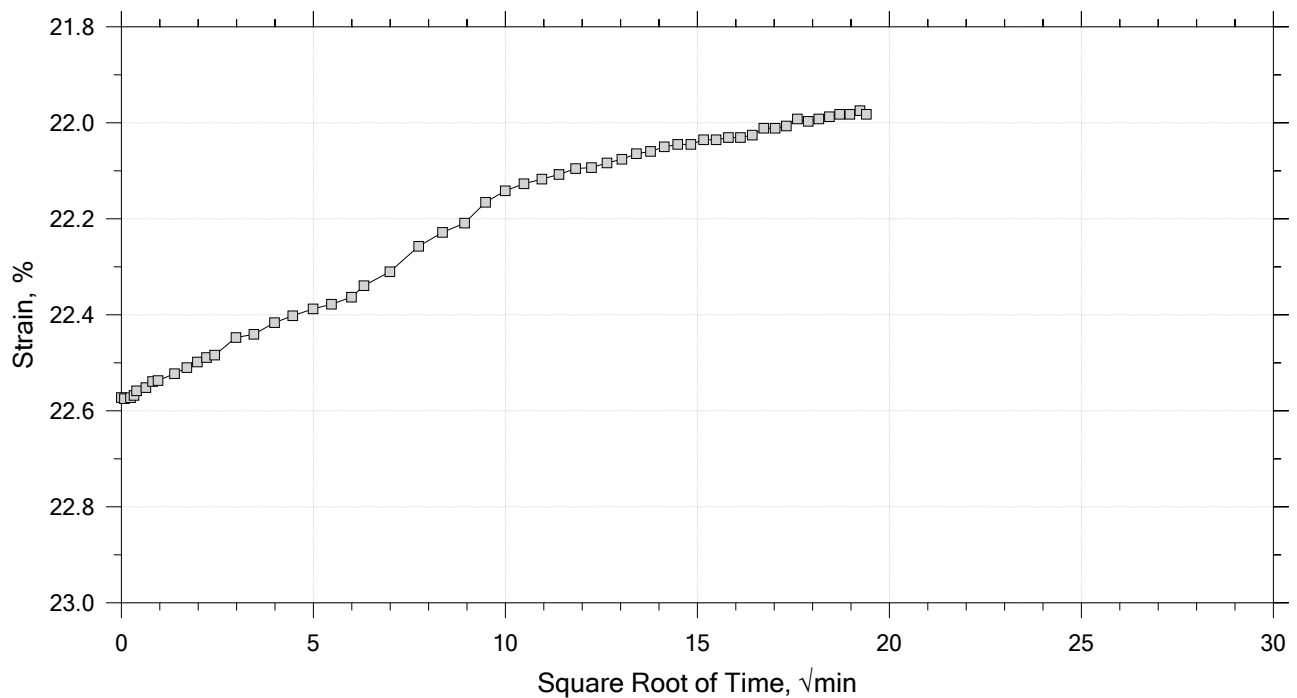
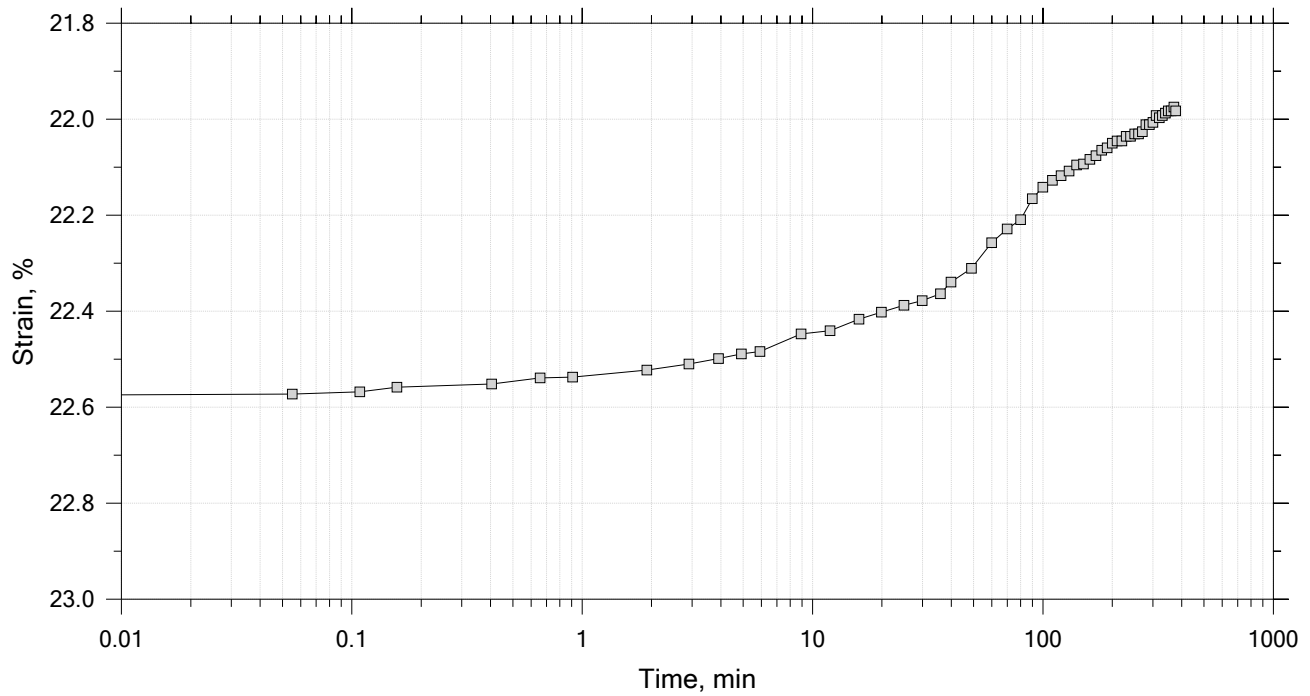
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	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 36
Initial Height: 1.00 in	Initial Void Ratio: 1.02	Plastic Limit: 19
Final Height: 0.79 in	Final Void Ratio: 0.596	Plasticity Index: 17

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	E-1411	RING		E0995
Mass Container, gm	8.12	110.89	110.89	8.46
Mass Container + Wet Soil, gm	155.63	260.31	244.24	141.54
Mass Container + Dry Soil, gm	115.43	220.51	220.51	117.86
Mass Dry Soil, gm	107.31	109.62	109.62	109.4
Water Content, %	37.46	36.30	21.65	21.65
Void Ratio	---	1.02	0.60	---
Degree of Saturation, %	---	97.98	100.00	---
Dry Unit Weight, pcf	---	85.076	107.69	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

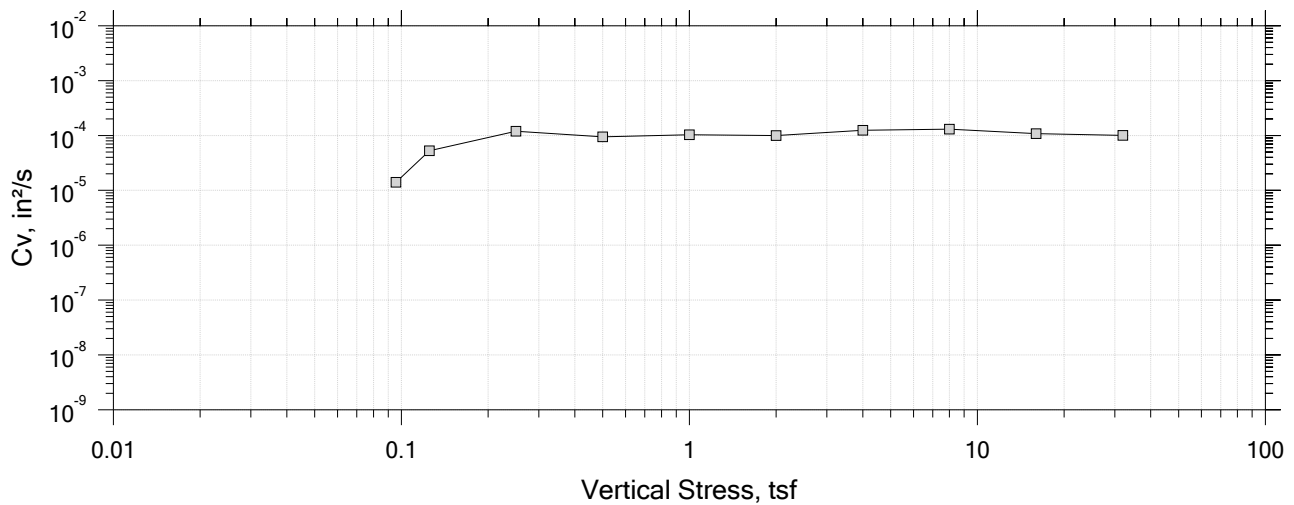
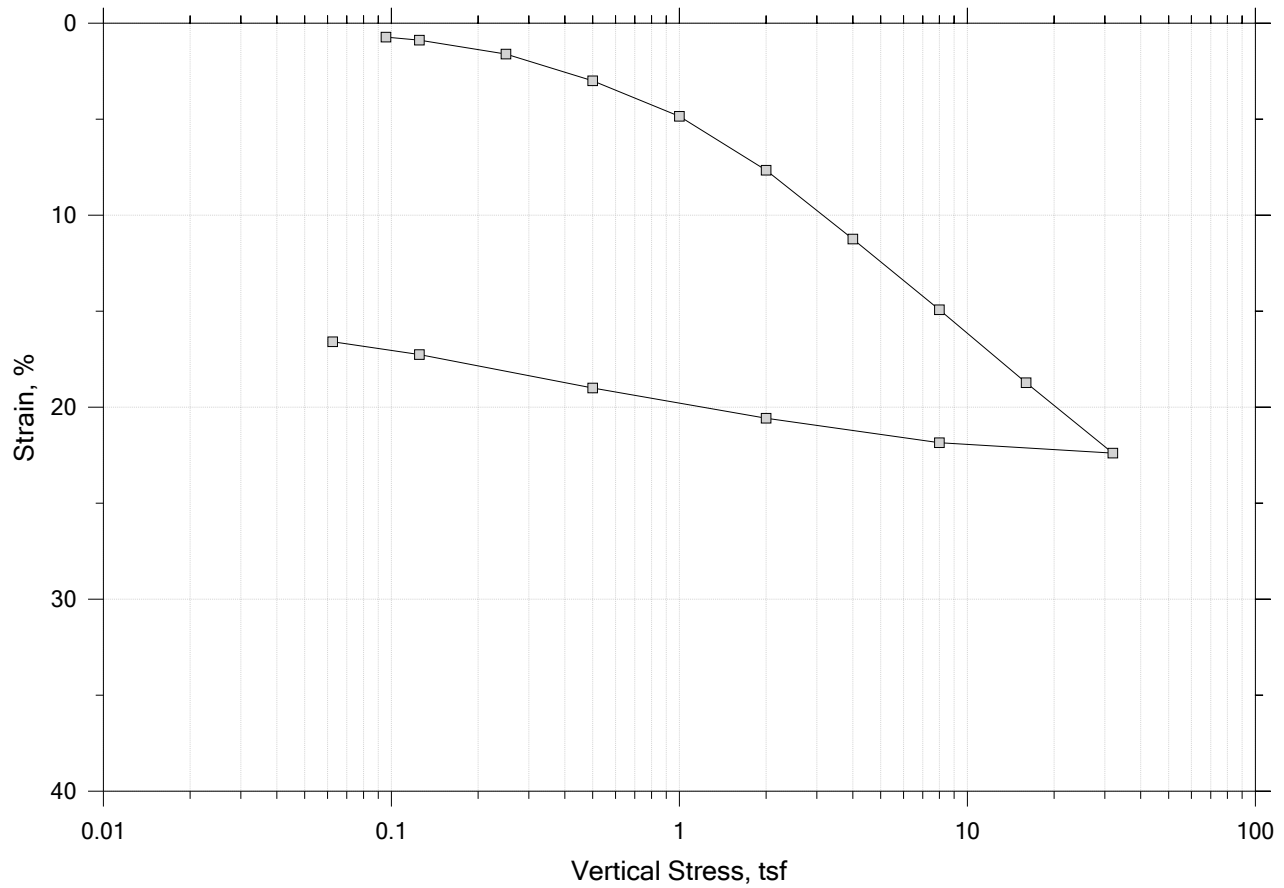
Square Root of Time Coefficients


[illegible]

	Project: I-395/Rte 9 Connector (Area 1)	Location: Brewer-Eddington, ME	Project No.: GTX-312665
	Boring No.: BB-BST1-201	Tested By: md	Checked By: mcm
	Sample No.: U1	Test Date: 03/24/21	Depth: 15-17 ft
	Test No.: IP-7A-	Sample Type: intact	Elevation: ---
	Description: Moist, olive clay		
	Remarks: System X, Swell Pressure = 0.128 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

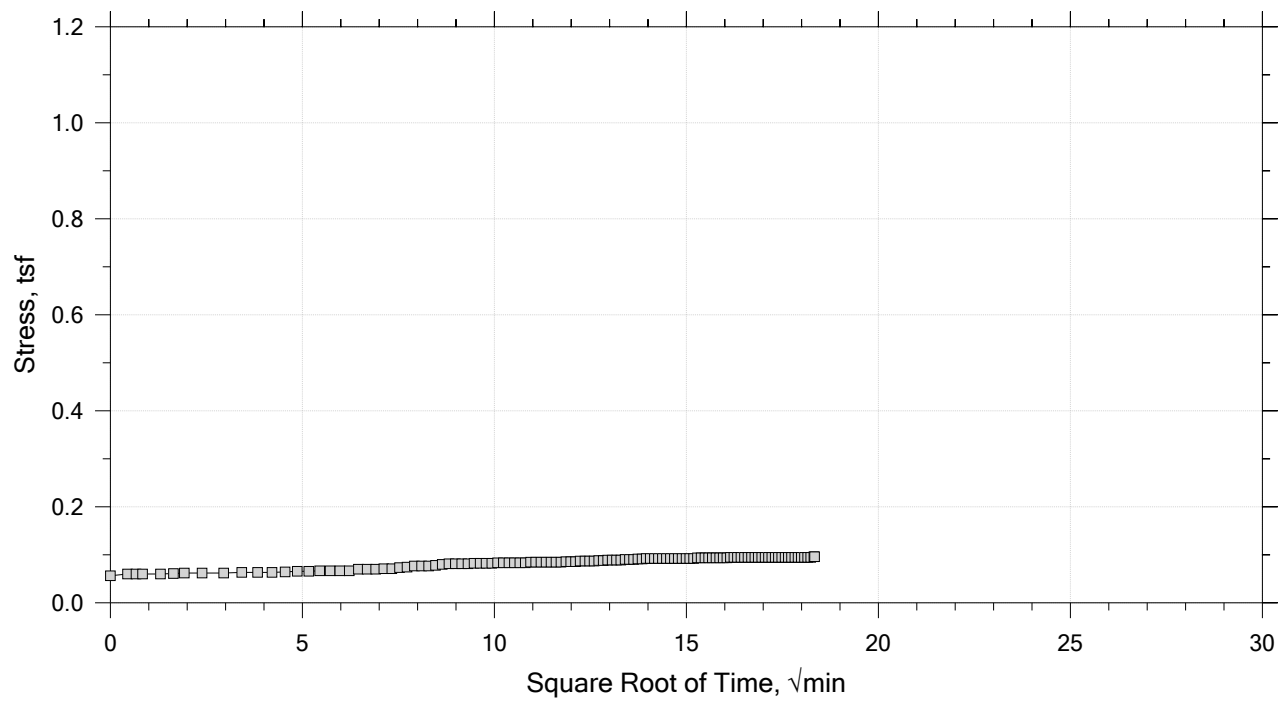
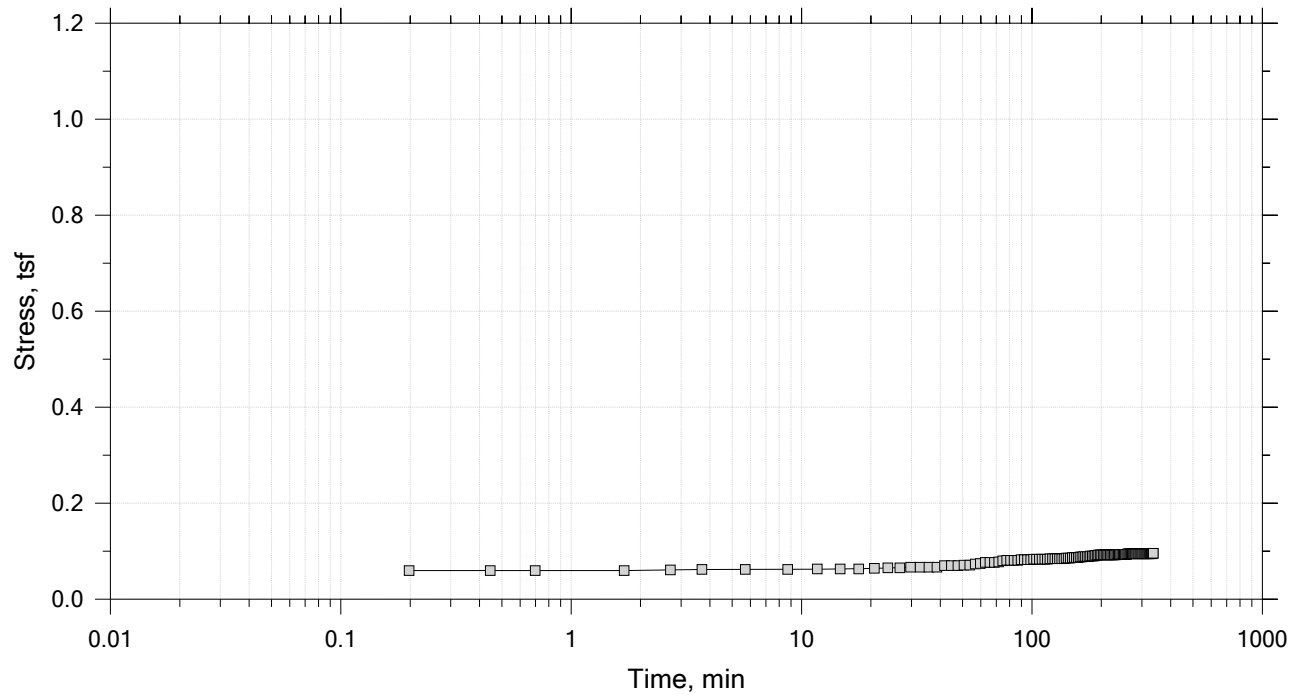
Summary Report




	Project: RT9/I-395 Connector-Wilson St.	Location: Brewer & Eddington, ME	Project No.: GTX-311345
	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0958 tsf



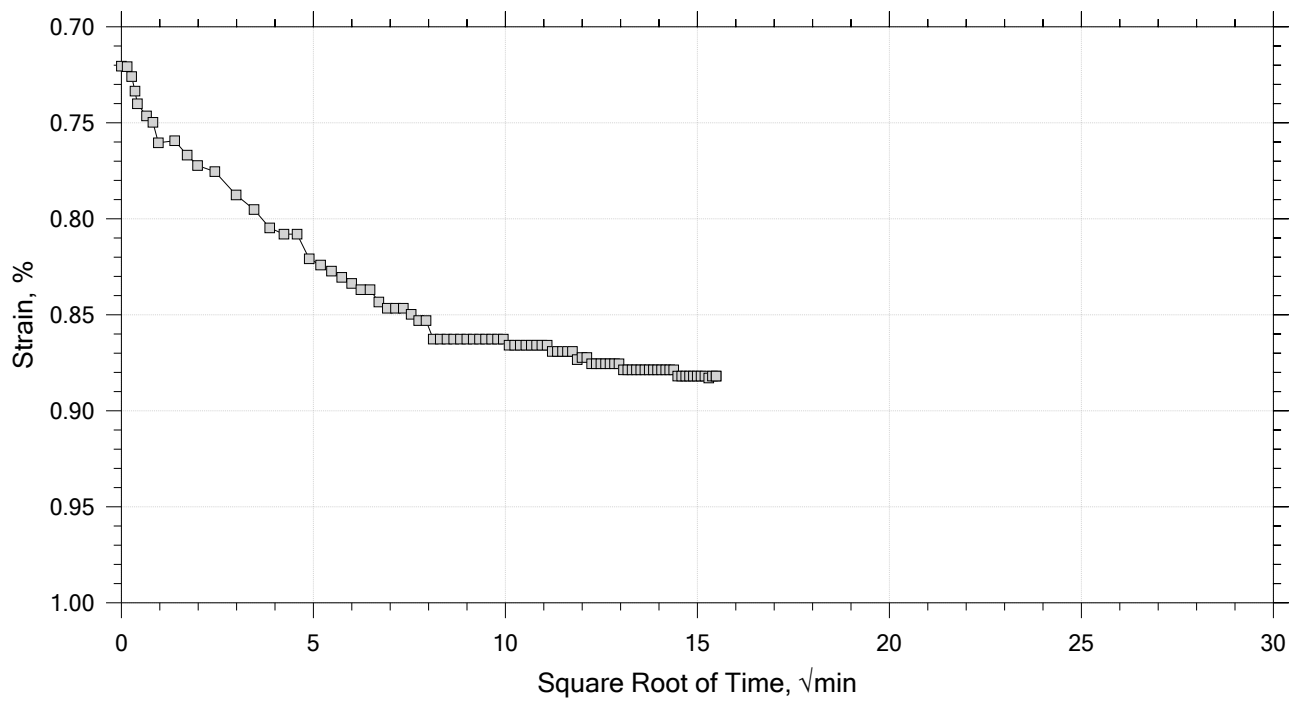
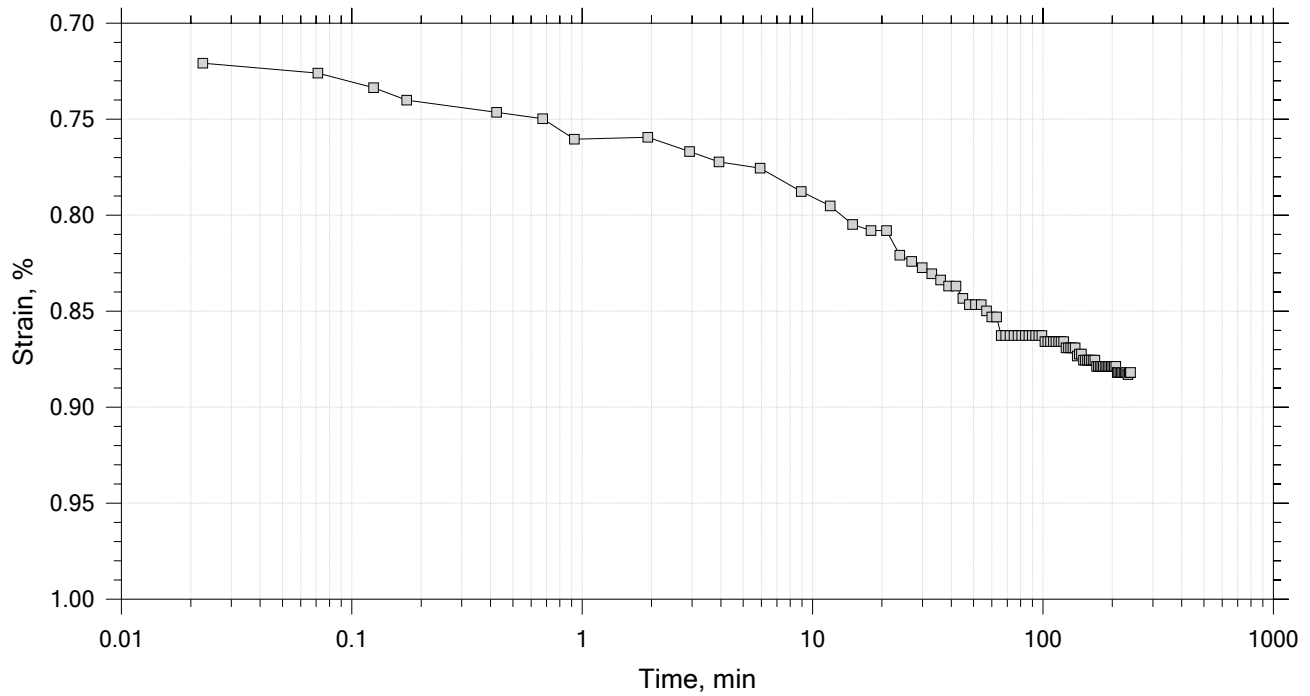
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



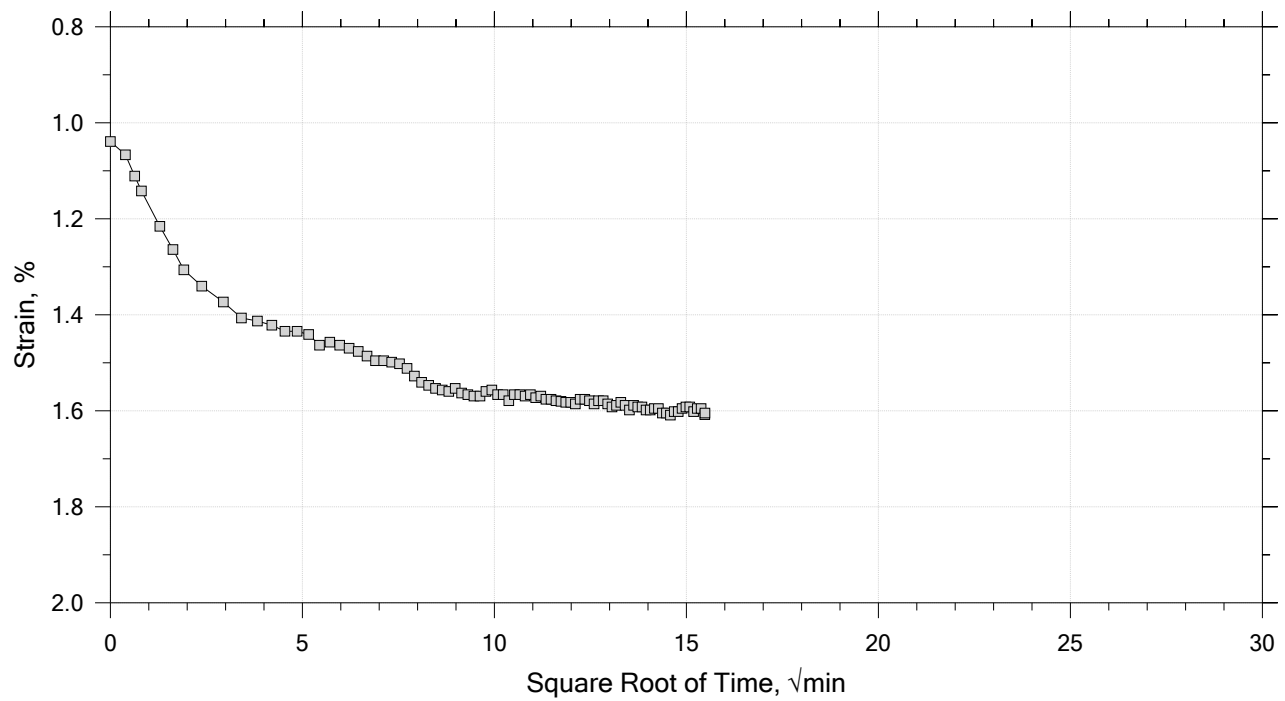
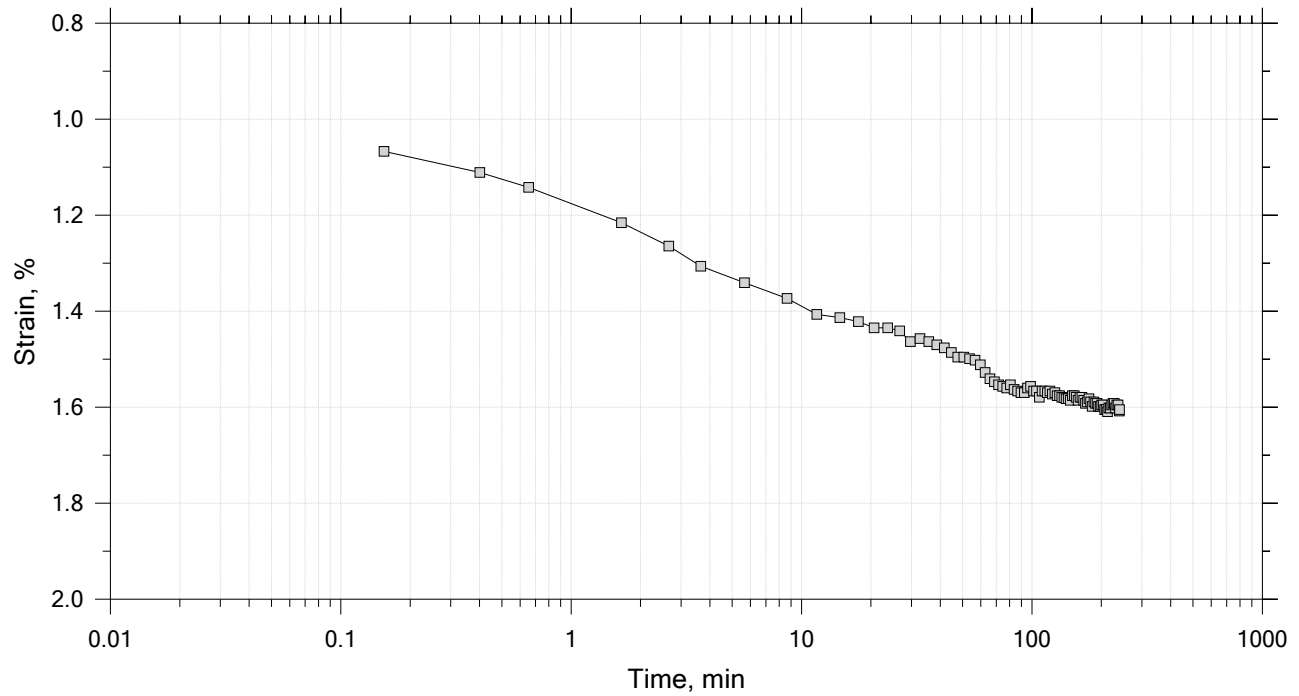
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



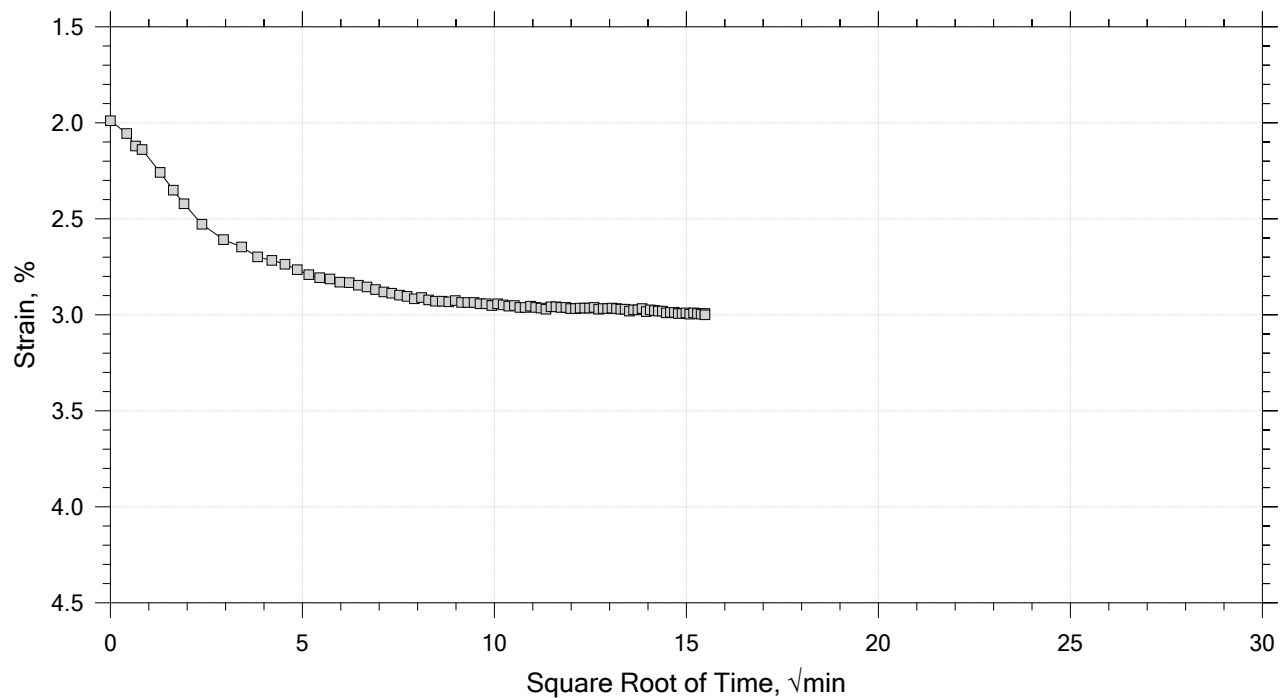
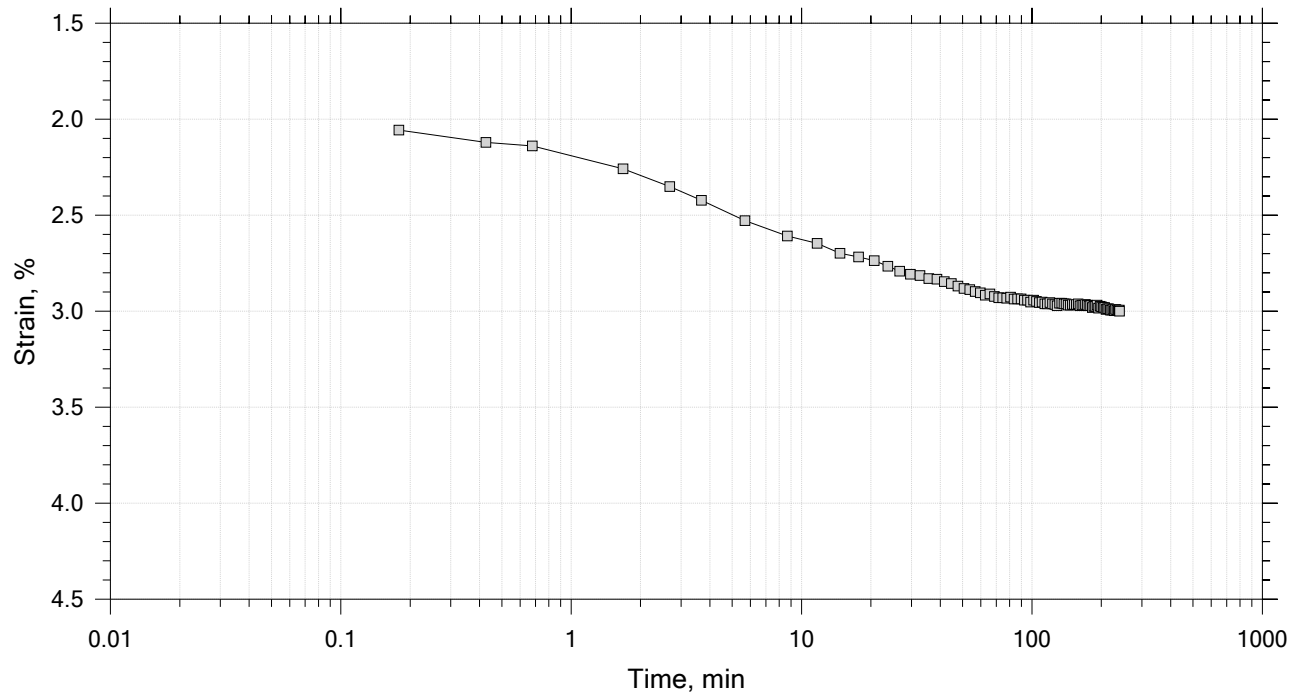
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



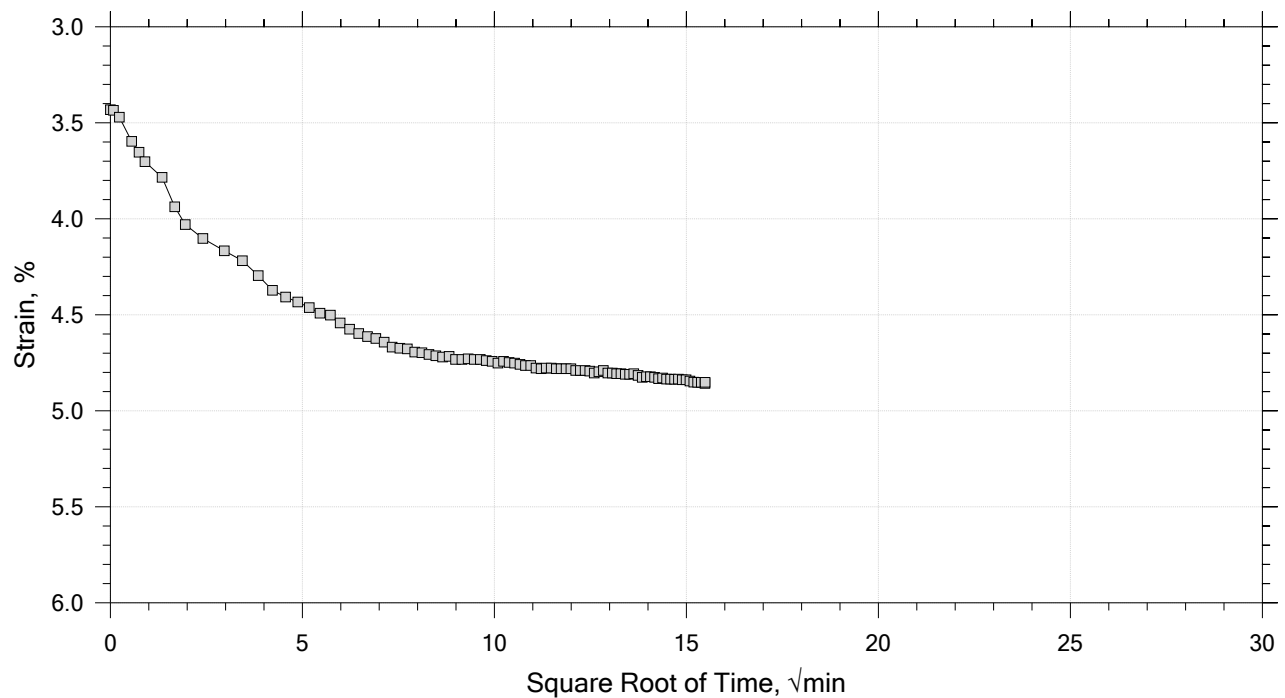
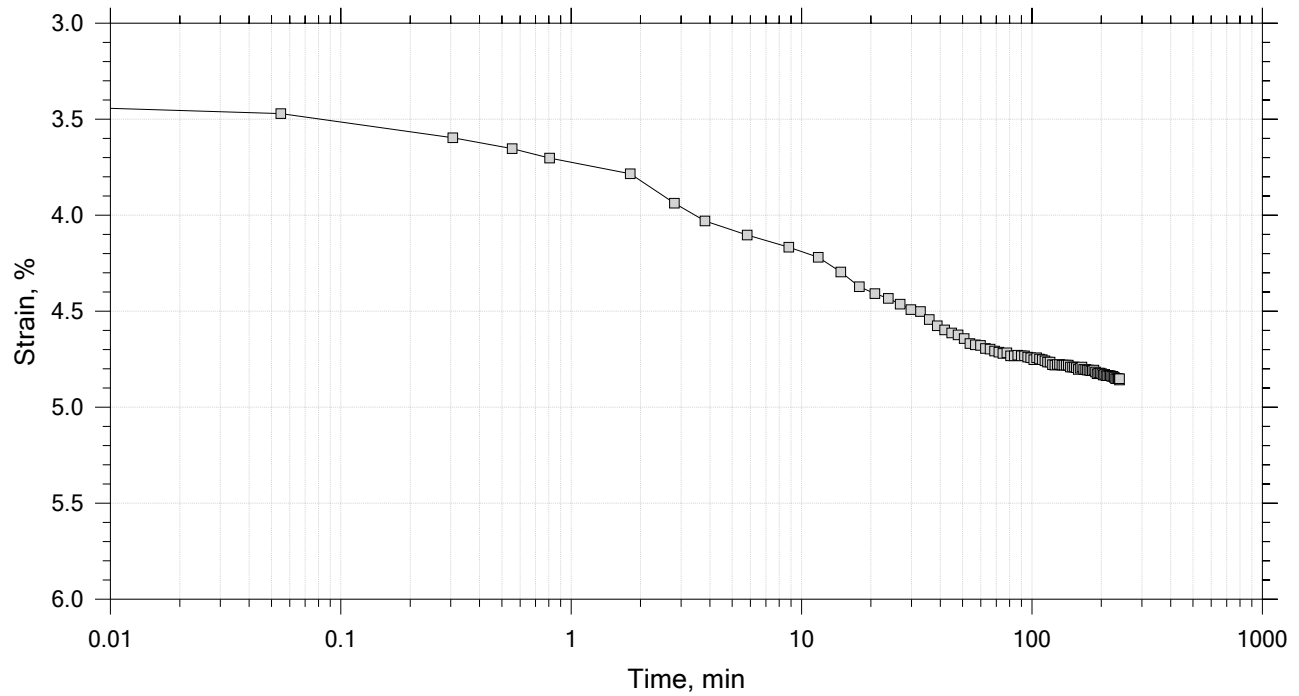
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



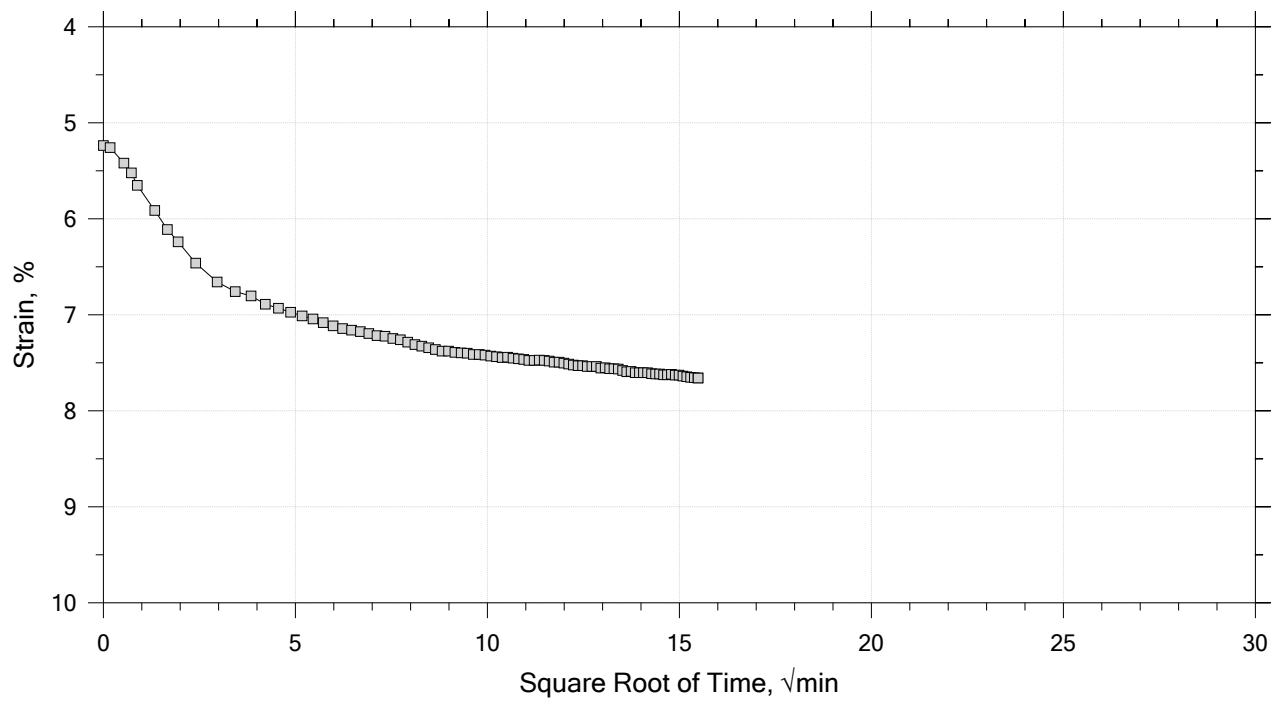
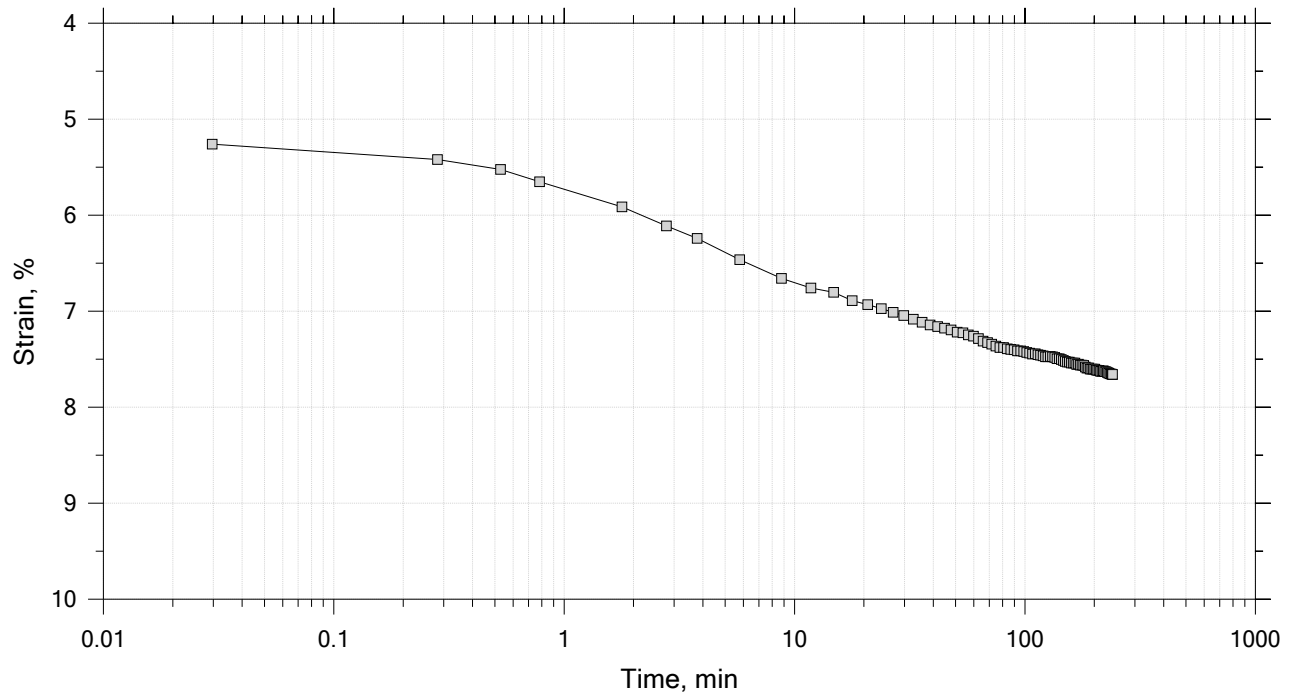
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



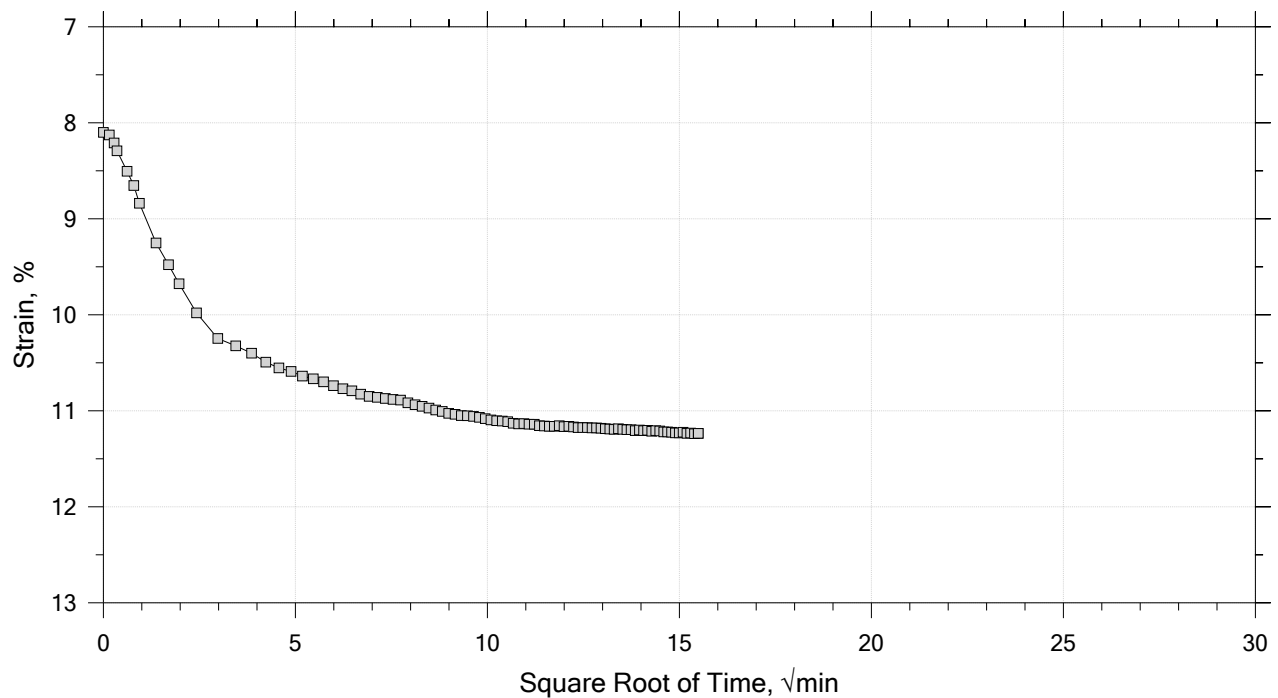
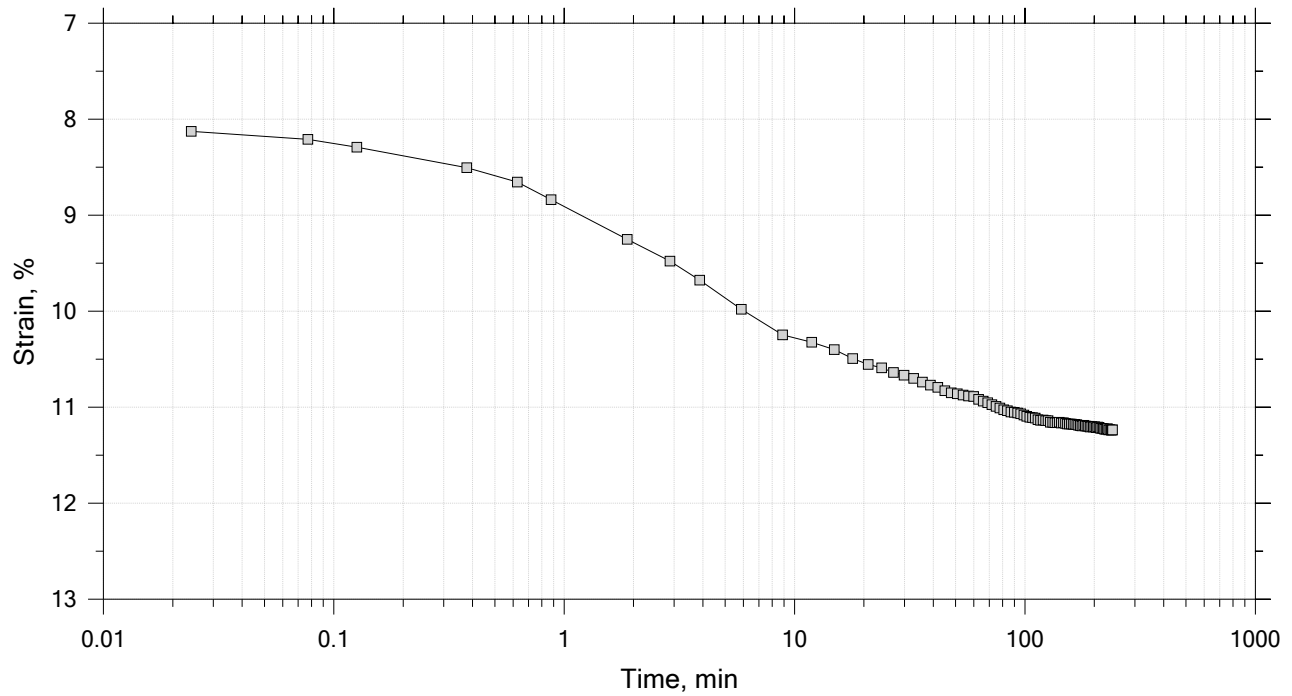
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



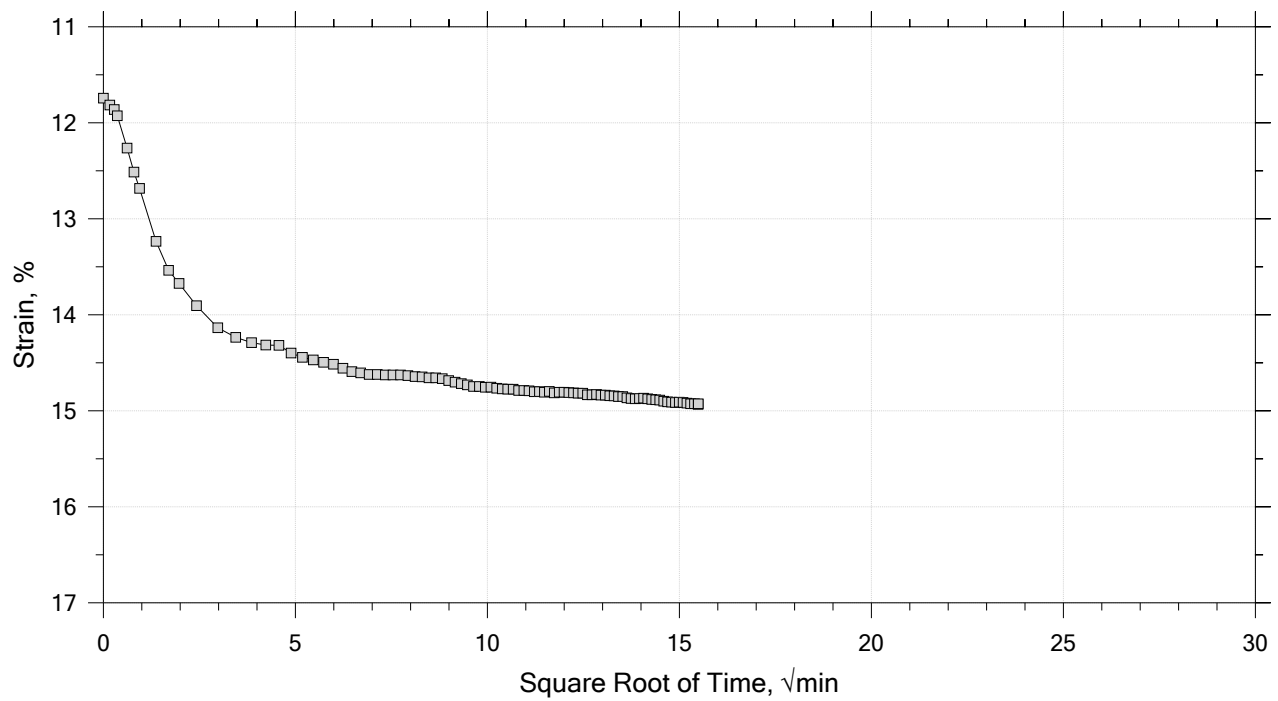
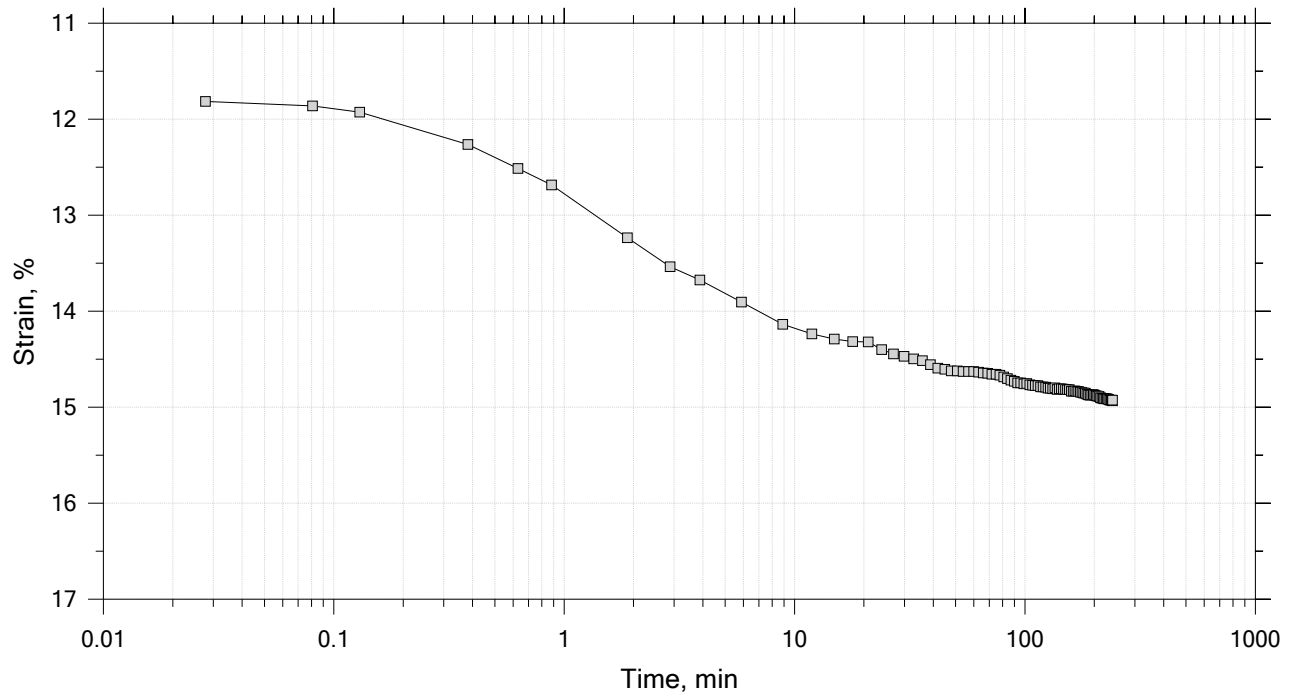
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



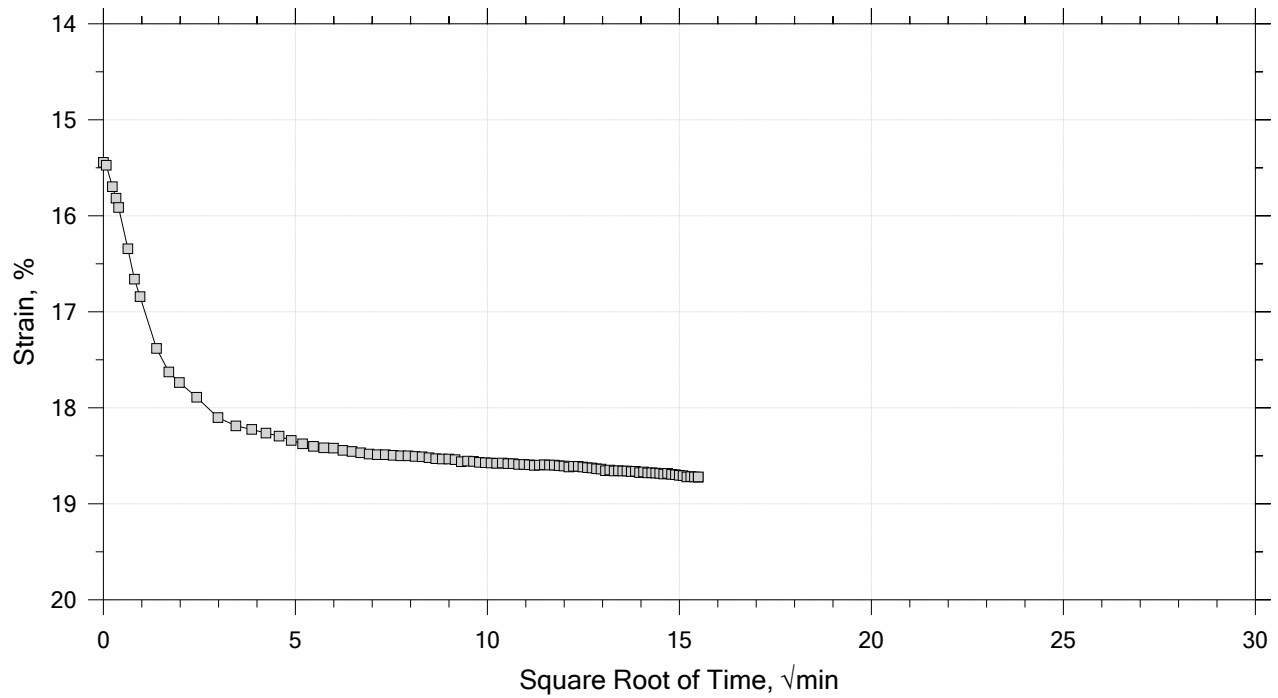
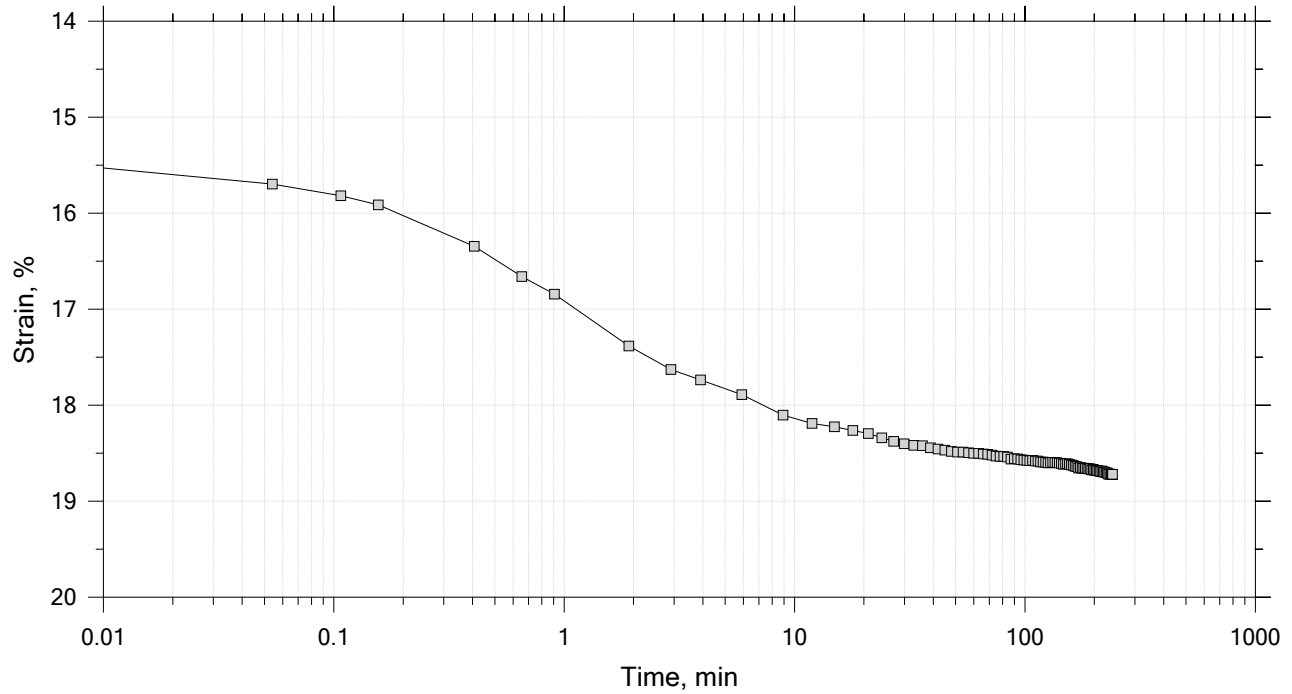
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



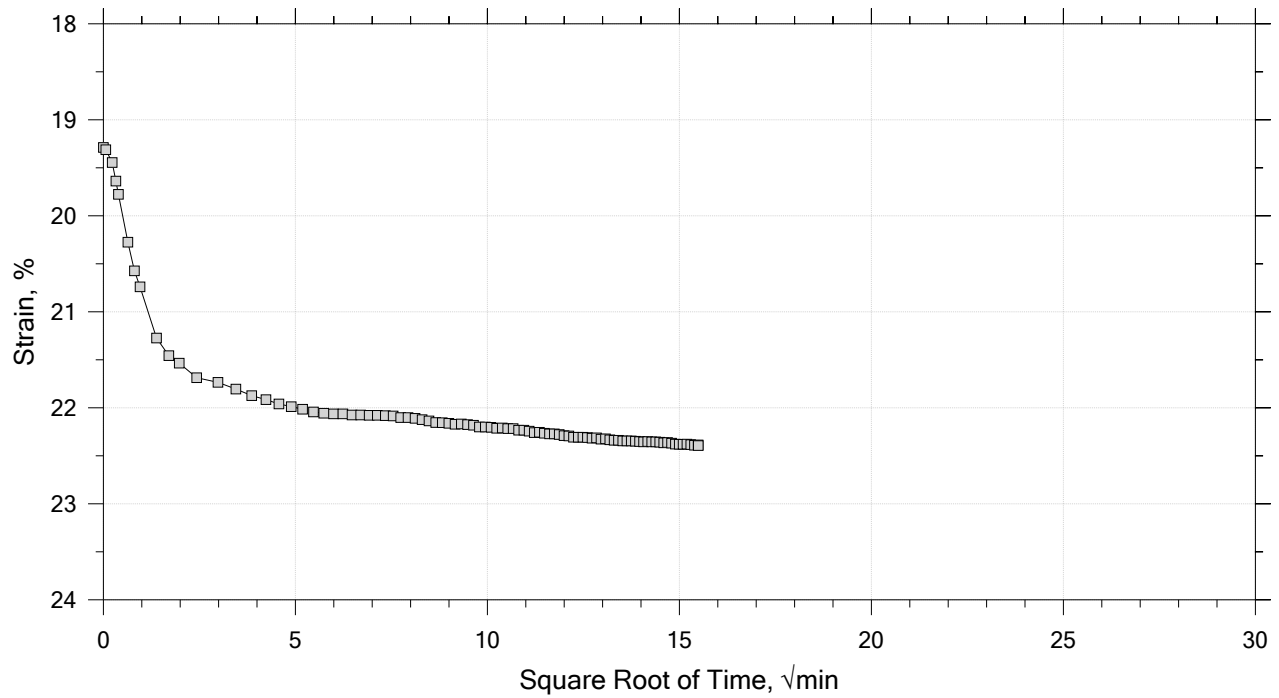
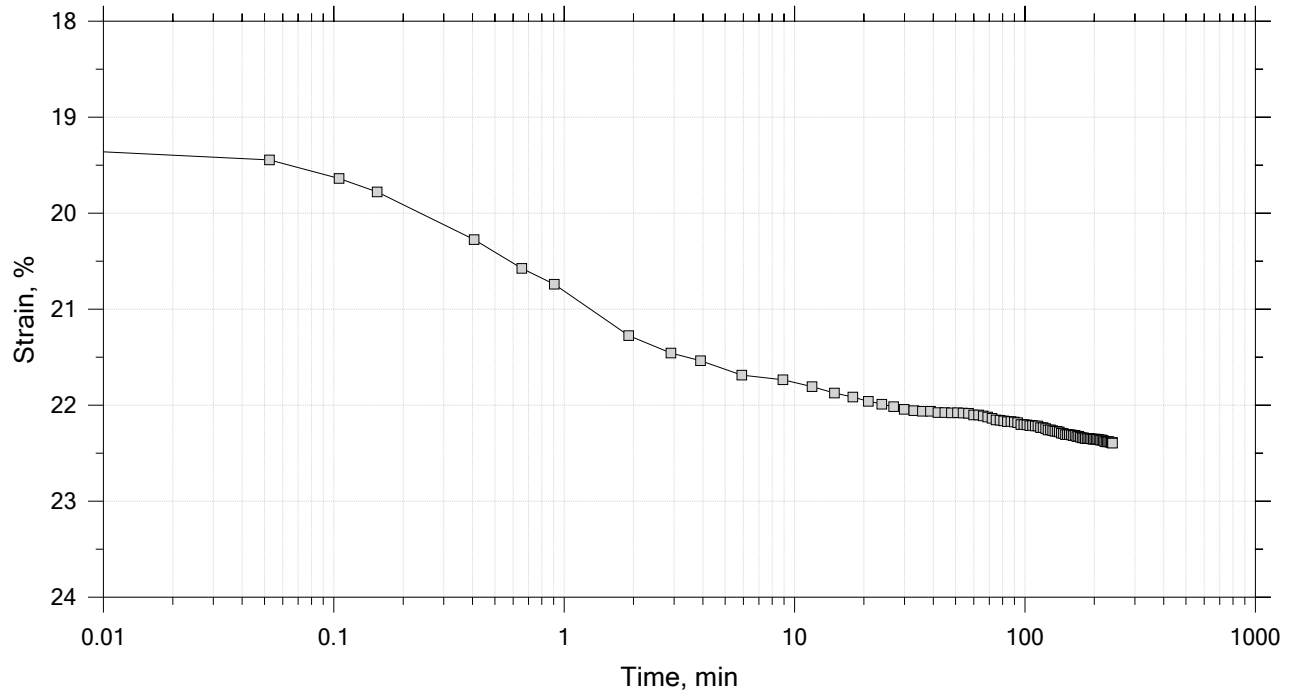
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



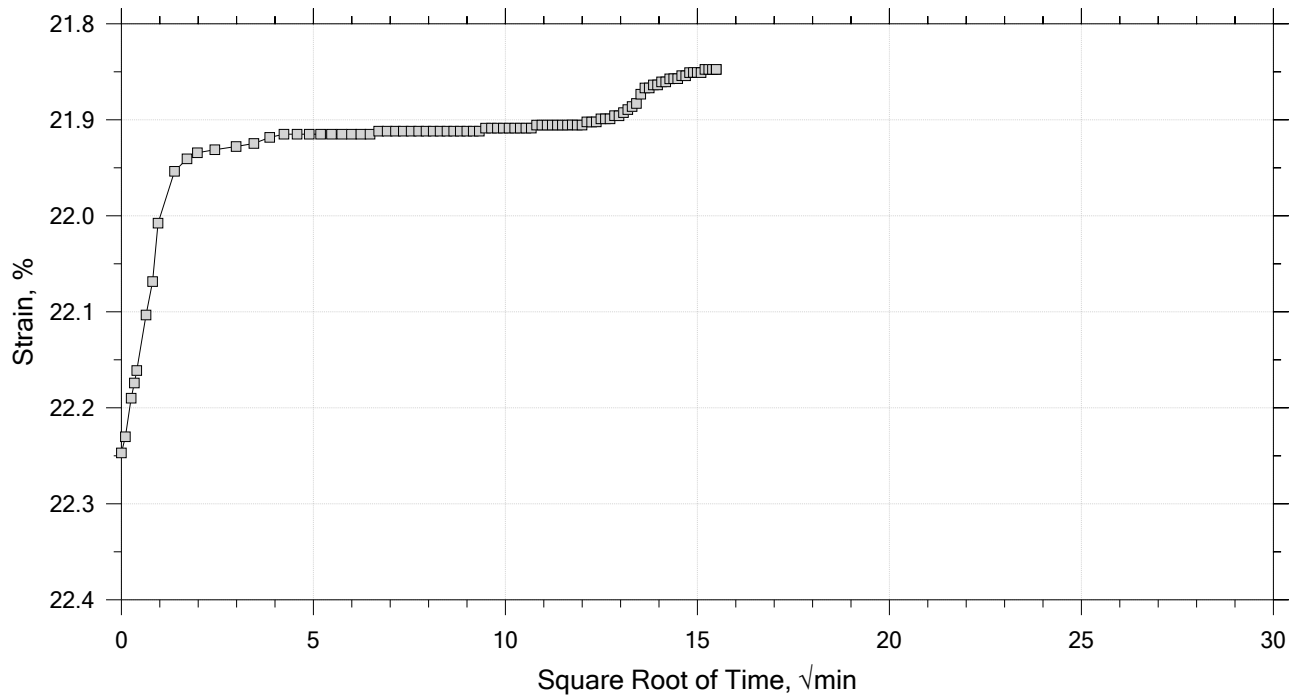
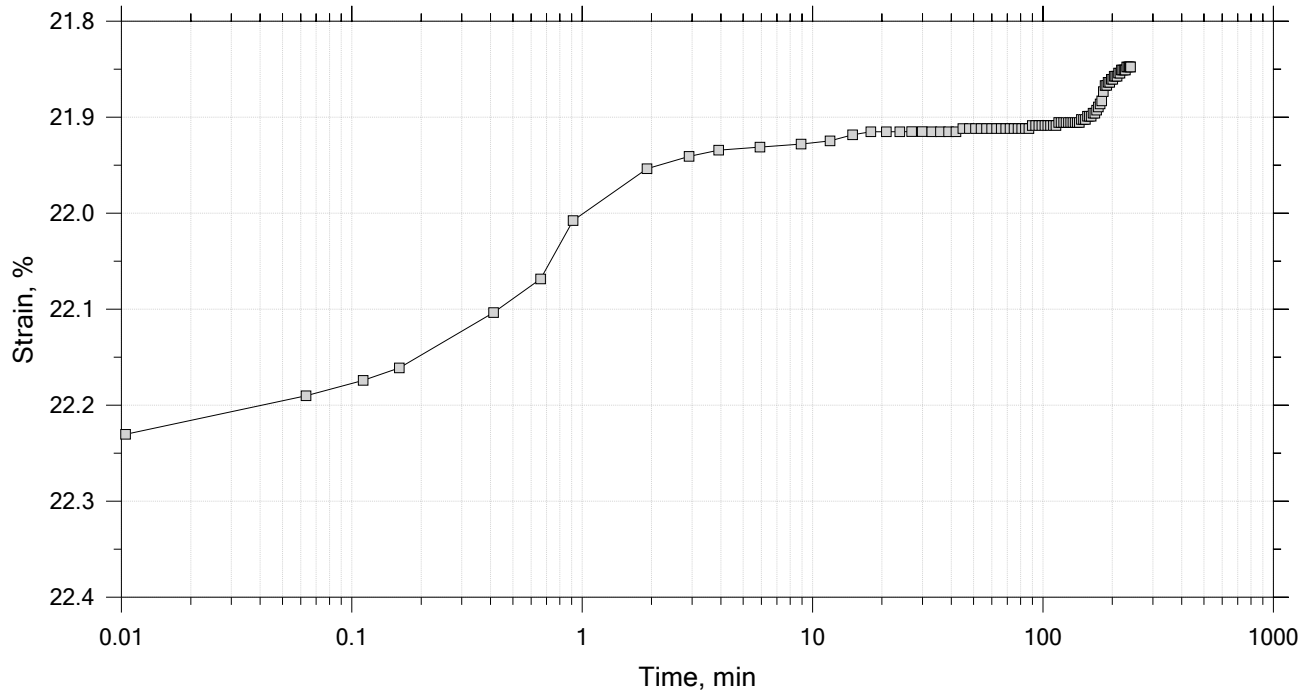
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



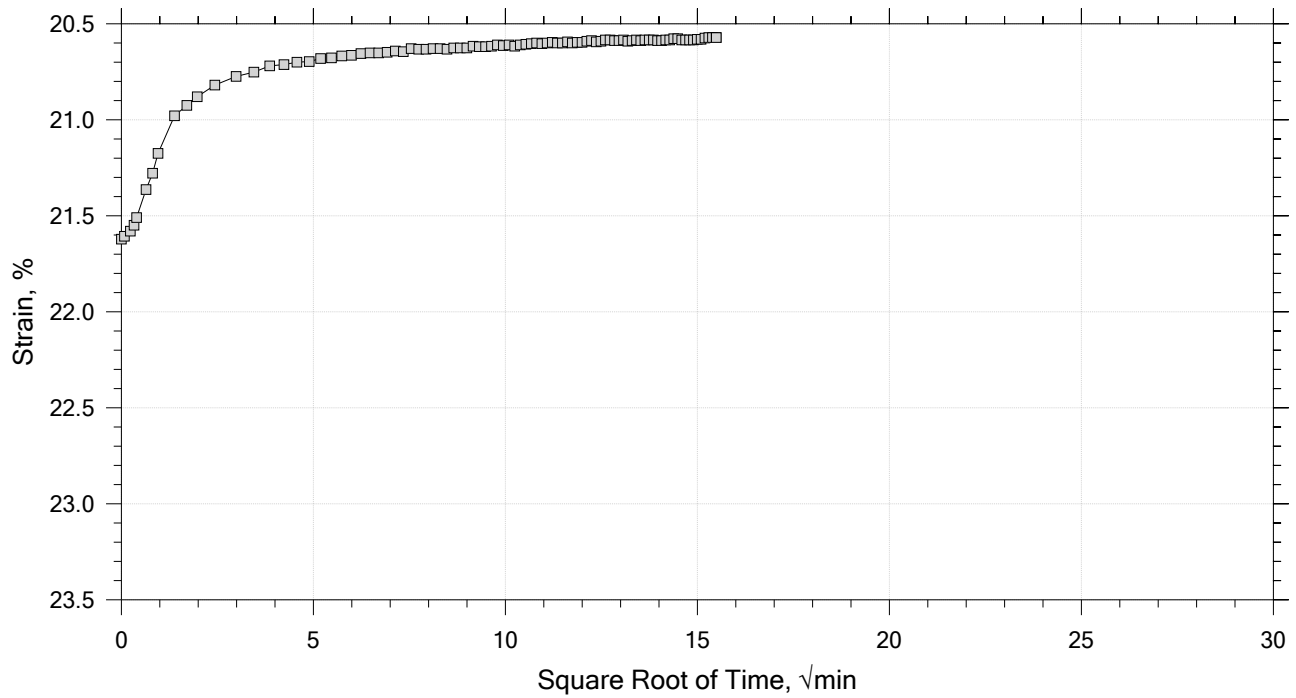
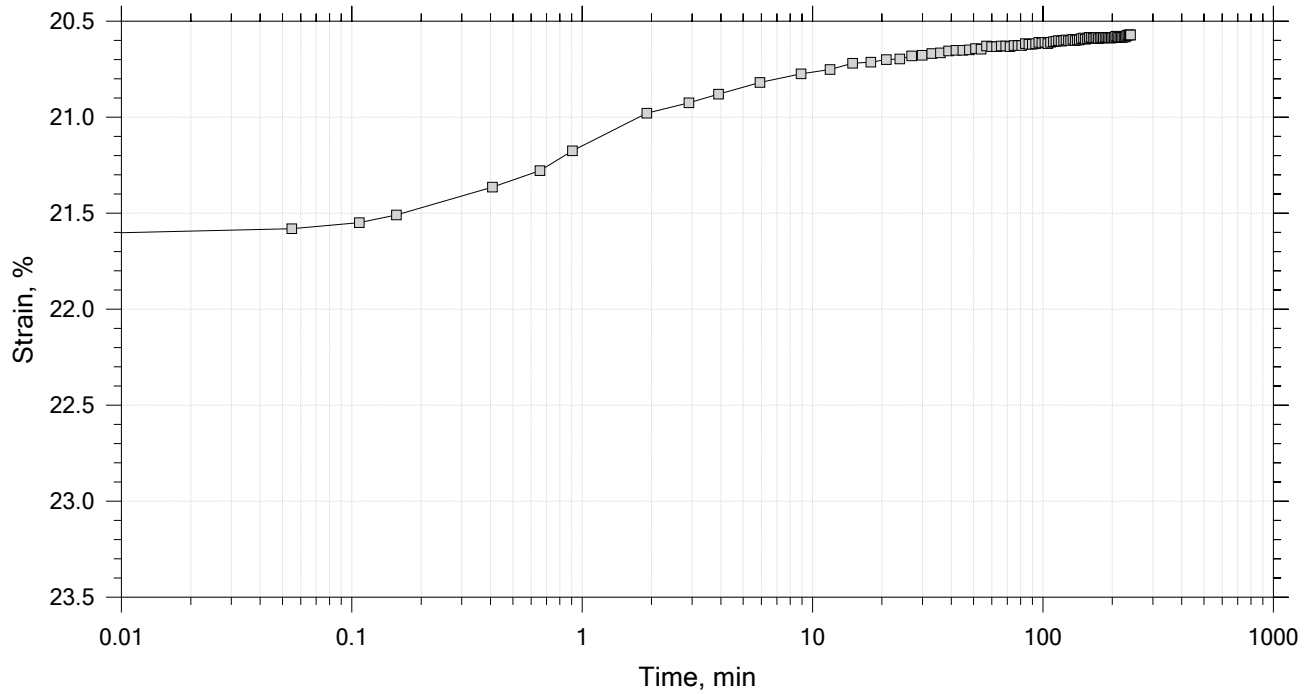
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



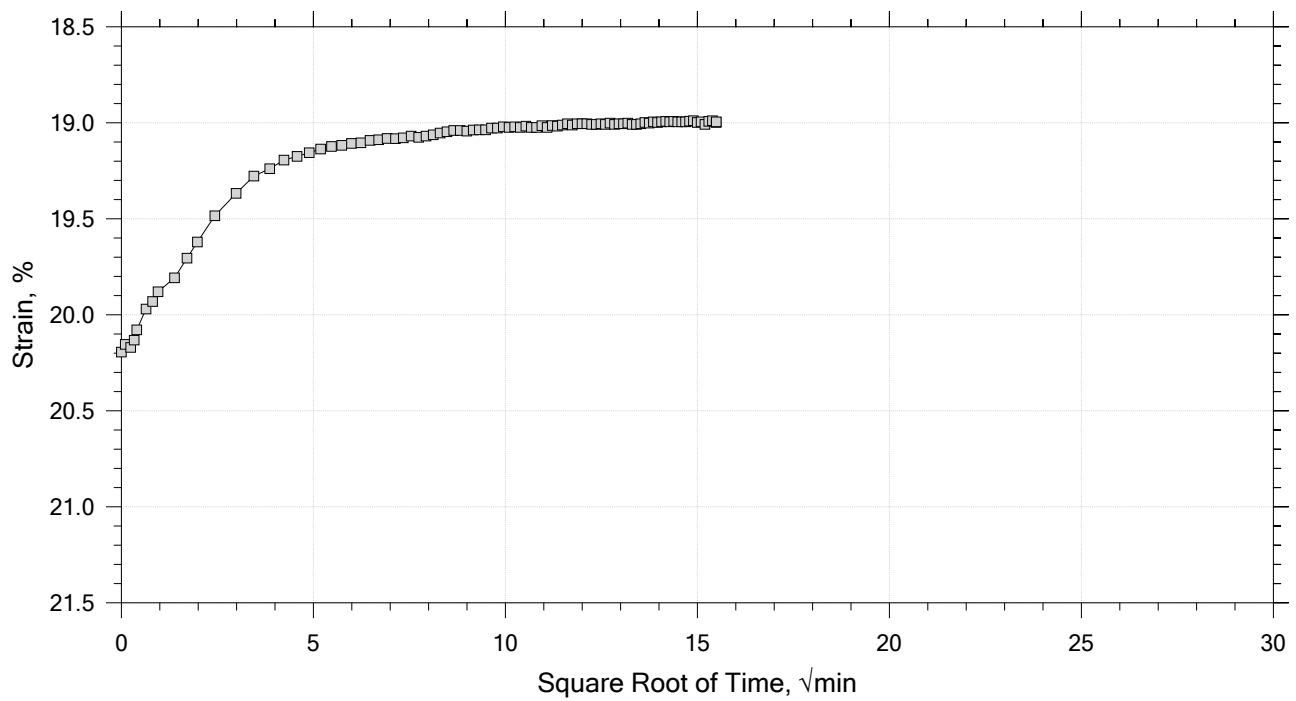
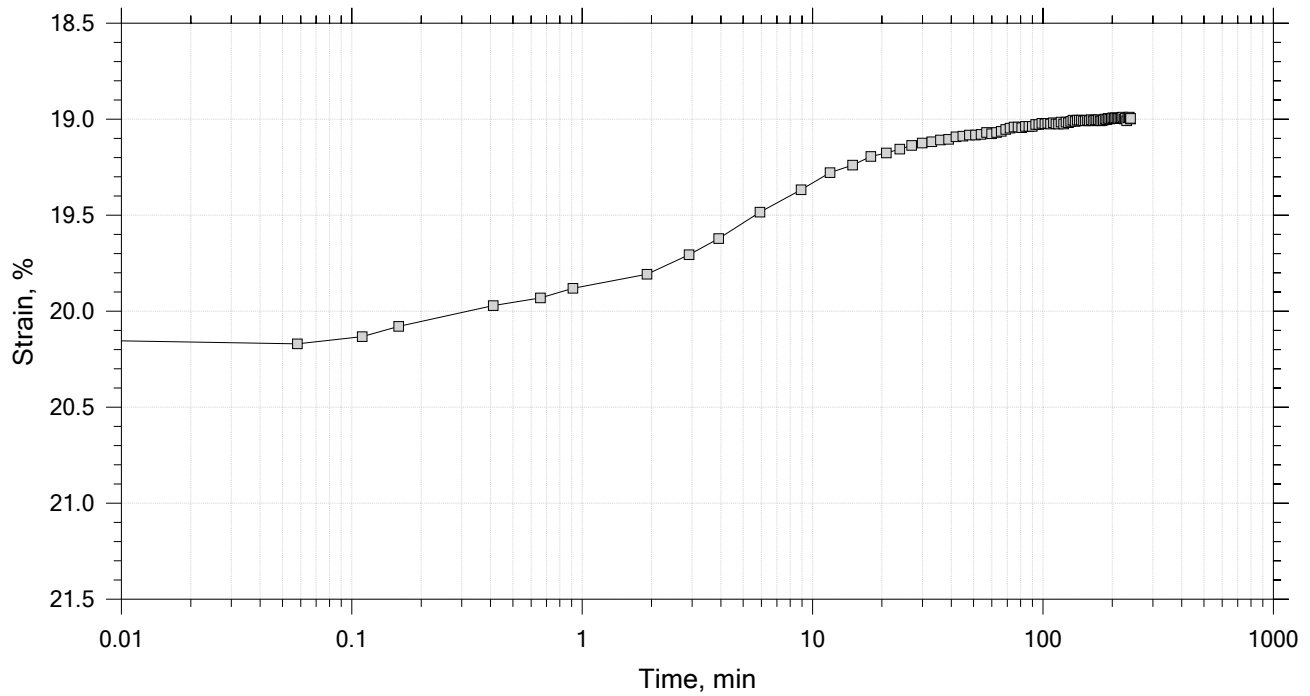
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



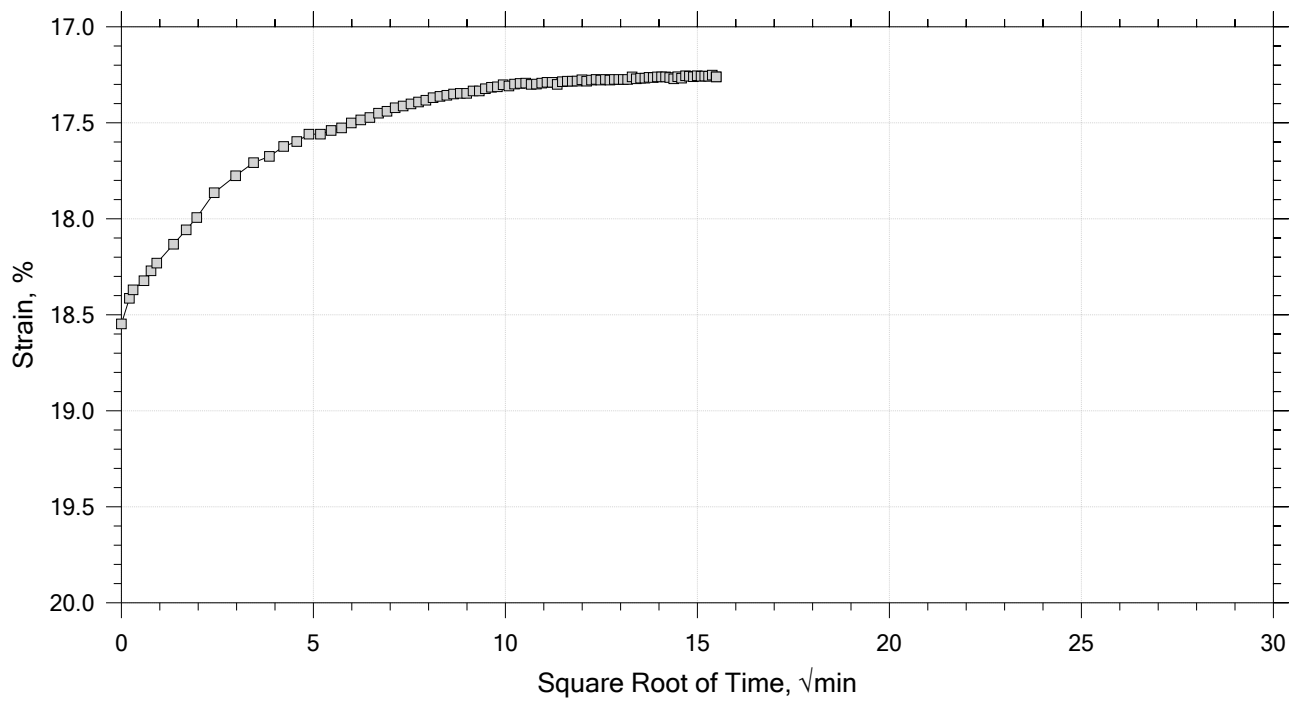
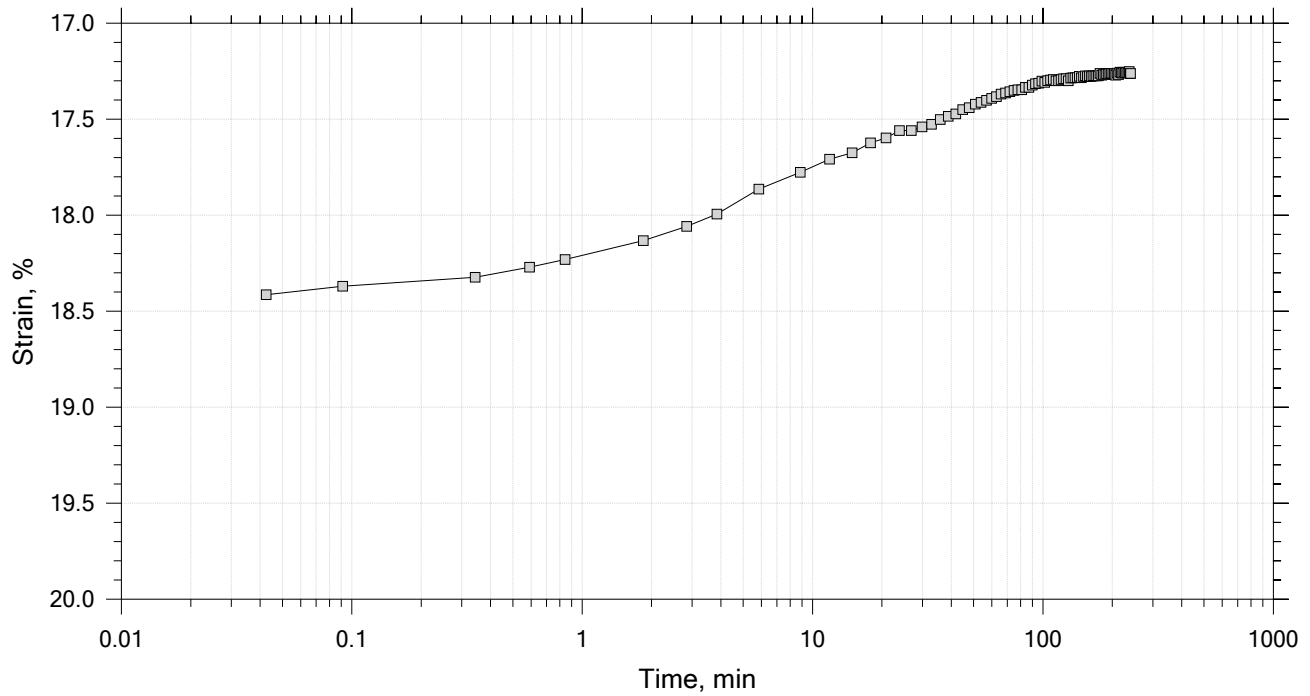
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



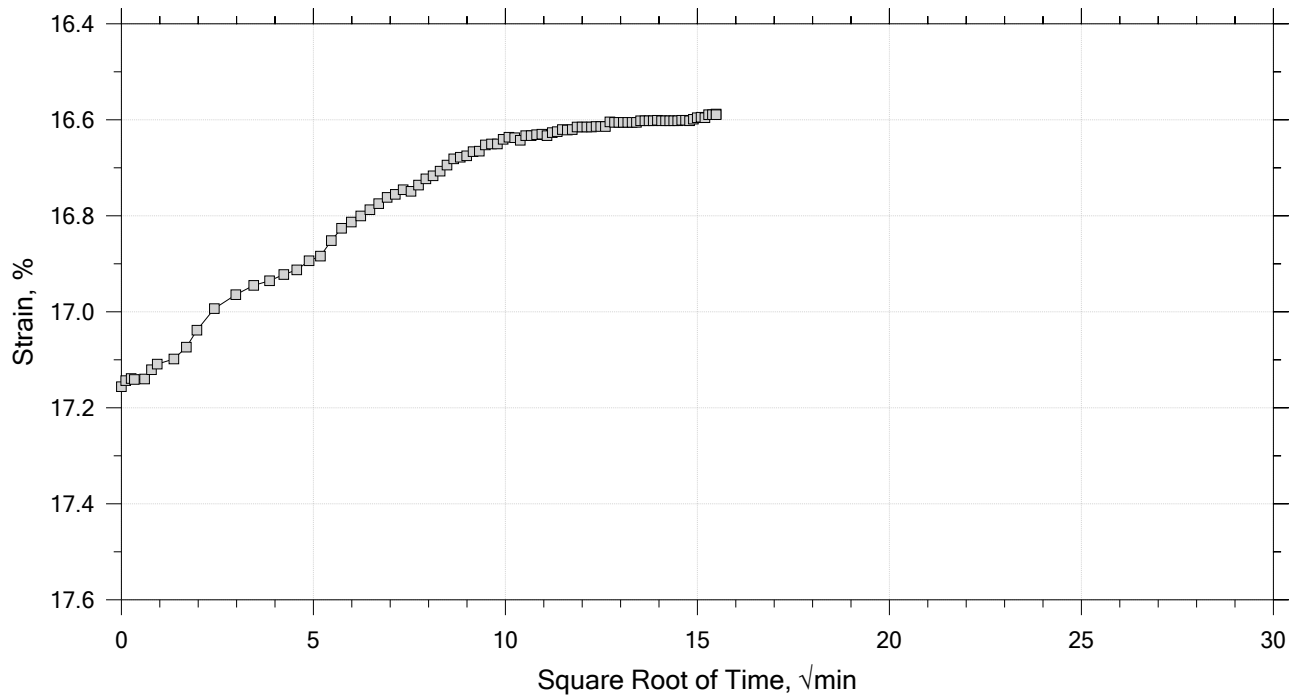
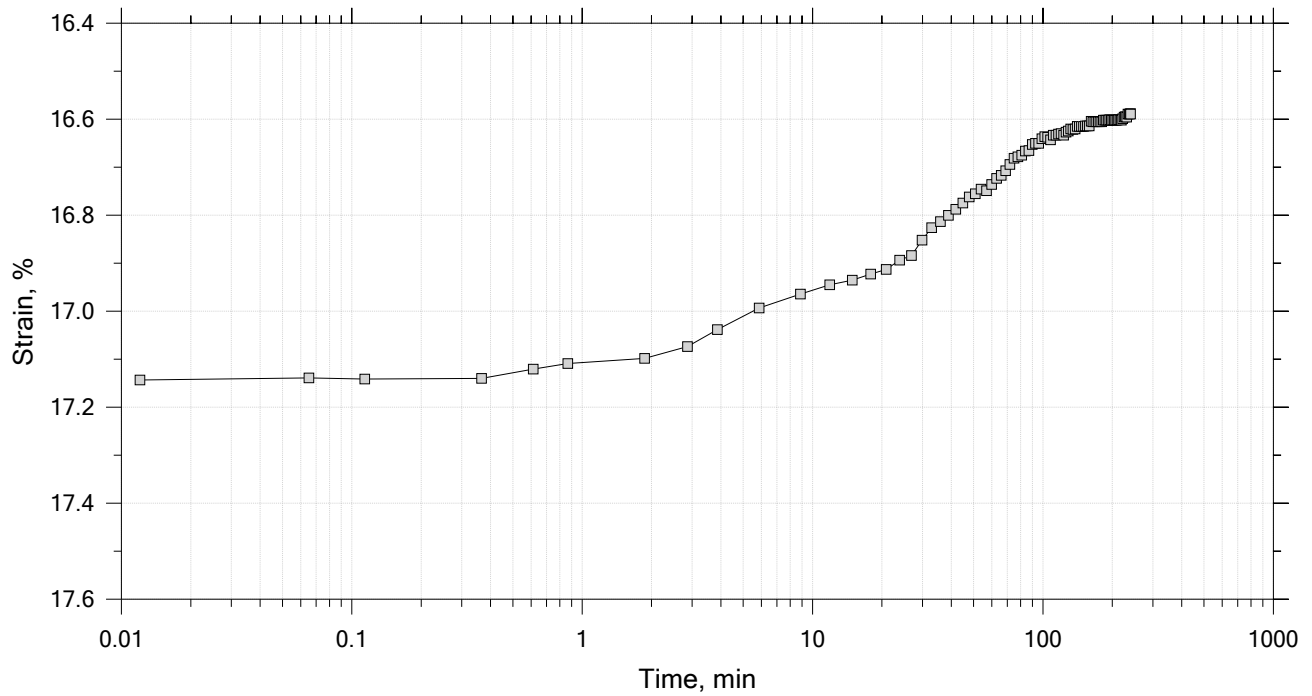
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	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: RT9/I-395 Connector-Wilson St.	Location: Brewer & Eddington, ME	Project No.: GTX-311345
	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.77	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 0.9	Plastic Limit: 19
Final Height: 0.85 in	Final Void Ratio: 0.615	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-745	RING		D-1069
Mass Container, gm	8.38	109.71	109.71	8.29
Mass Container + Wet Soil, gm	336.34	264.28	253	150.91
Mass Container + Dry Soil, gm	256.25	226.97	226.97	125
Mass Dry Soil, gm	247.87	117.26	117.26	116.71
Water Content, %	32.31	31.82	22.20	22.20
Void Ratio	---	0.90	0.61	---
Degree of Saturation, %	---	97.94	100.00	---
Dry Unit Weight, pcf	---	91.002	107.06	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: RT9/I-395 Connector-Wilson St.	Location: Brewer & Eddington, ME	Project No.: GTX-311345
	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

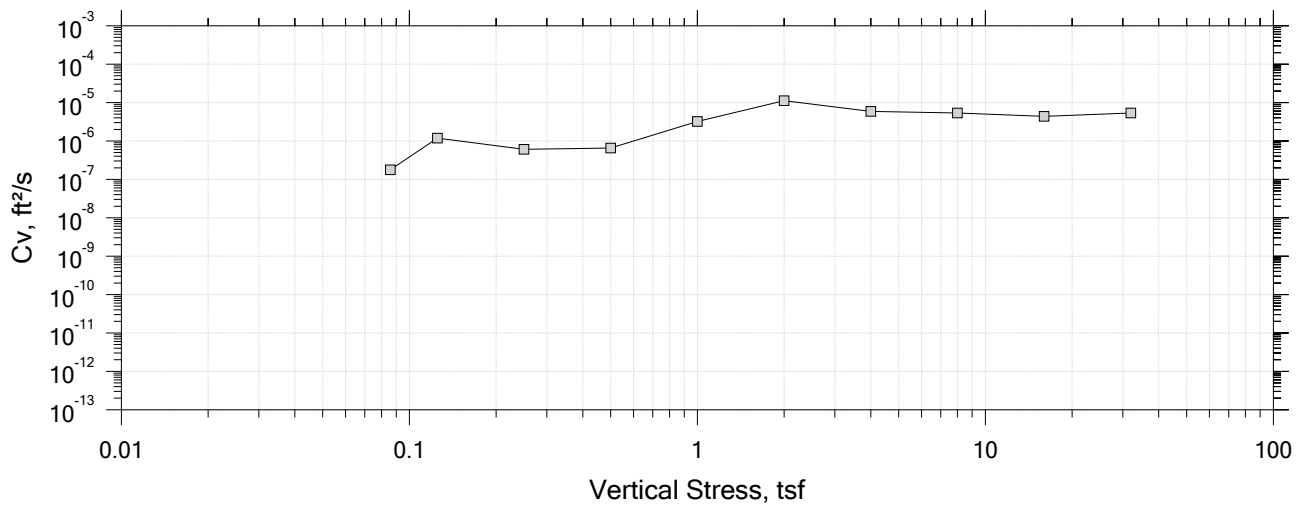
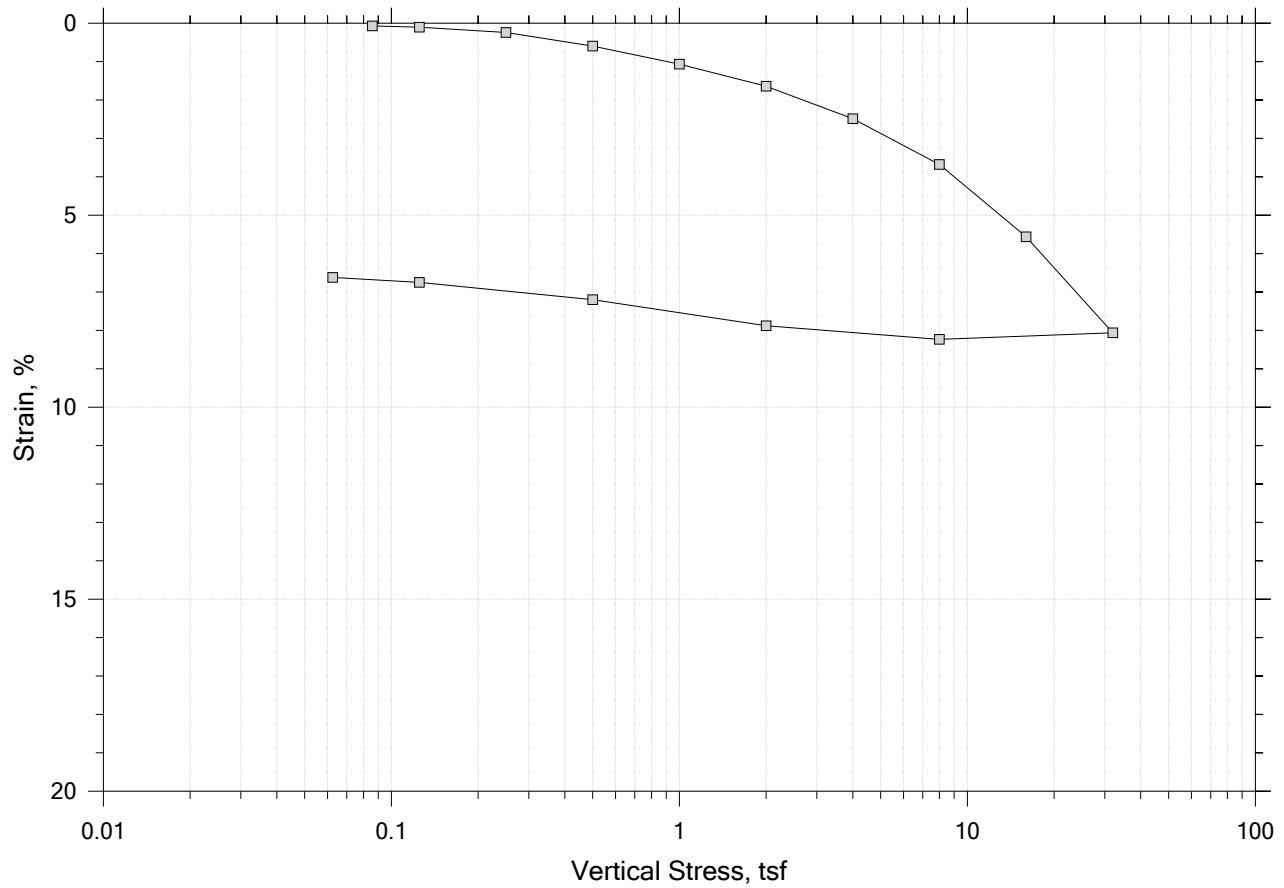
Square Root of Time Coefficients


[illegible]

	Project: RT9/I-395 Connector-Wilson St.	Location: Brewer & Eddington, ME	Project No.: GTX-311345
	Boring No.: BB-BWS-301	Tested By: md	Checked By: anm
	Sample No.: 3U	Test Date: 02/18/20	Depth: 29-31 ft
	Test No.: IP-1 C	Sample Type: tube	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.0958 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

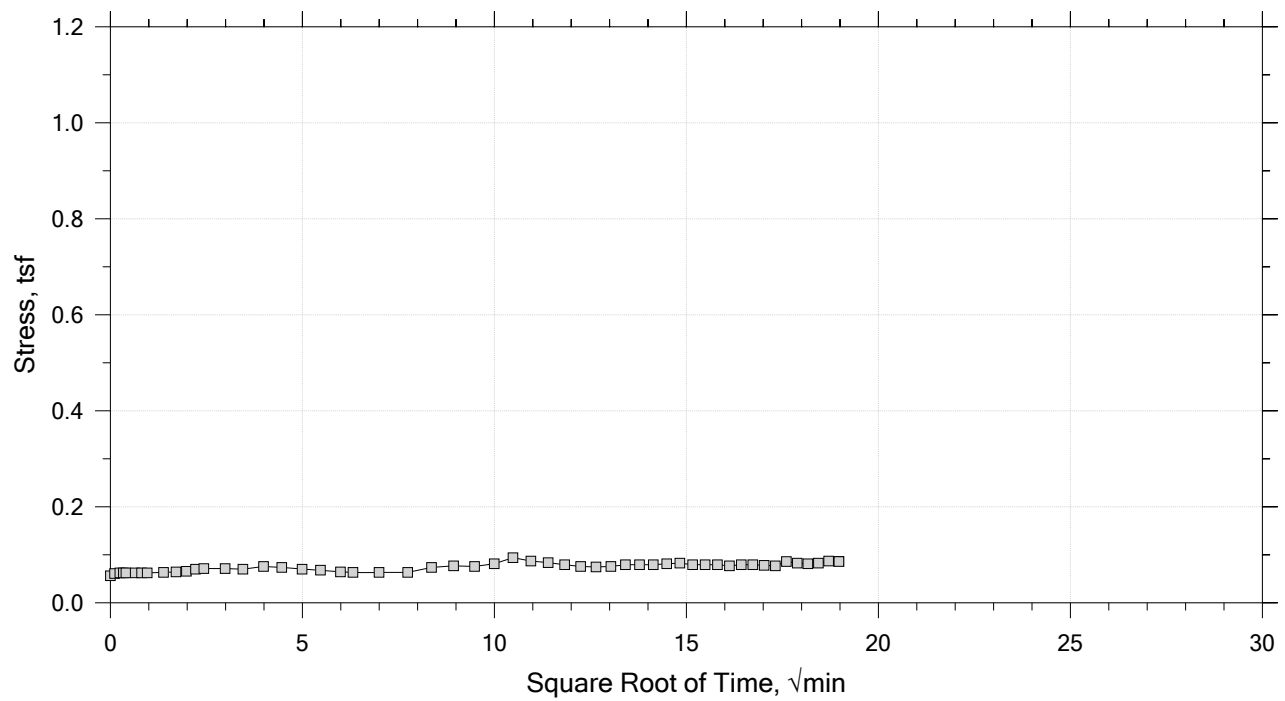
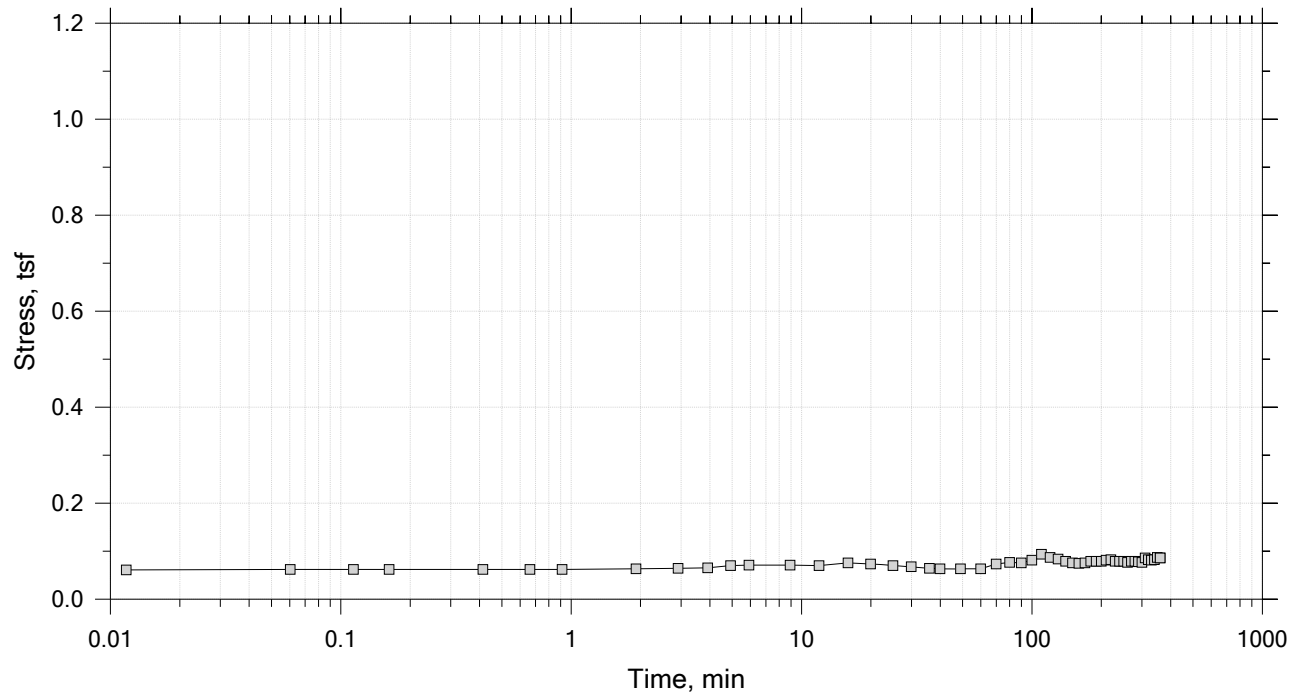
Summary Report




	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0859 tsf



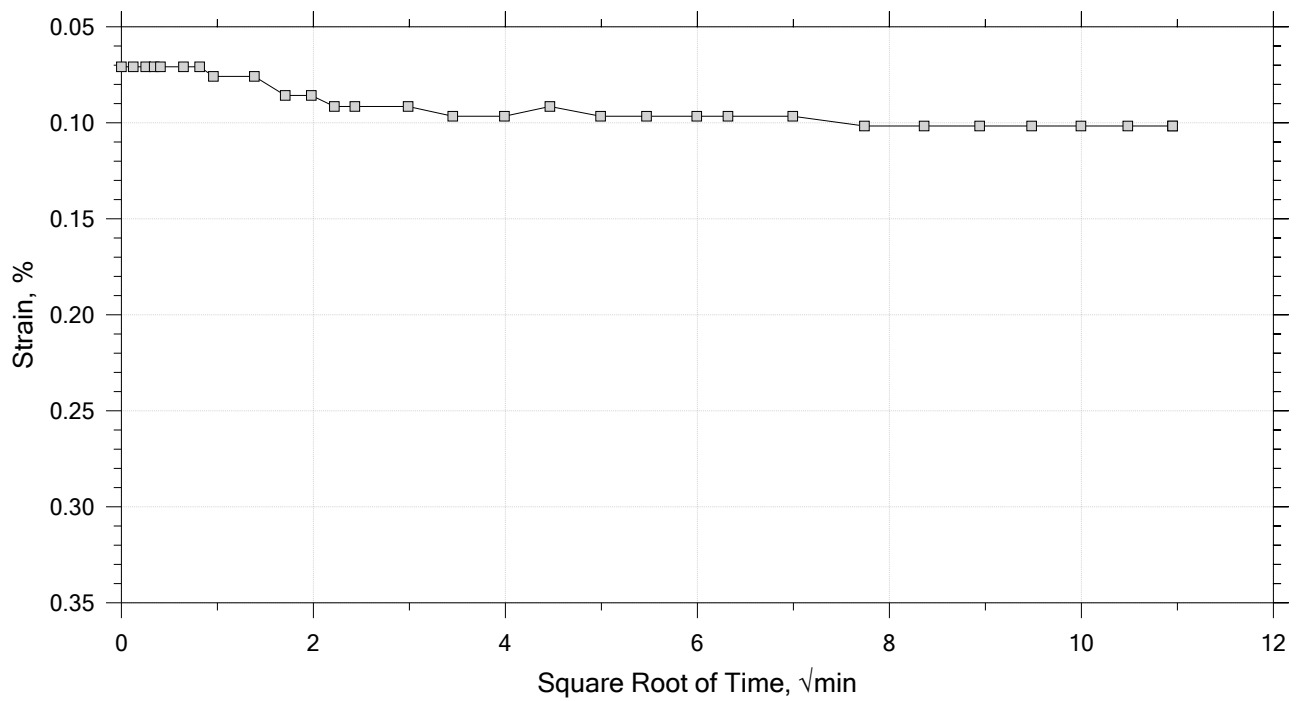
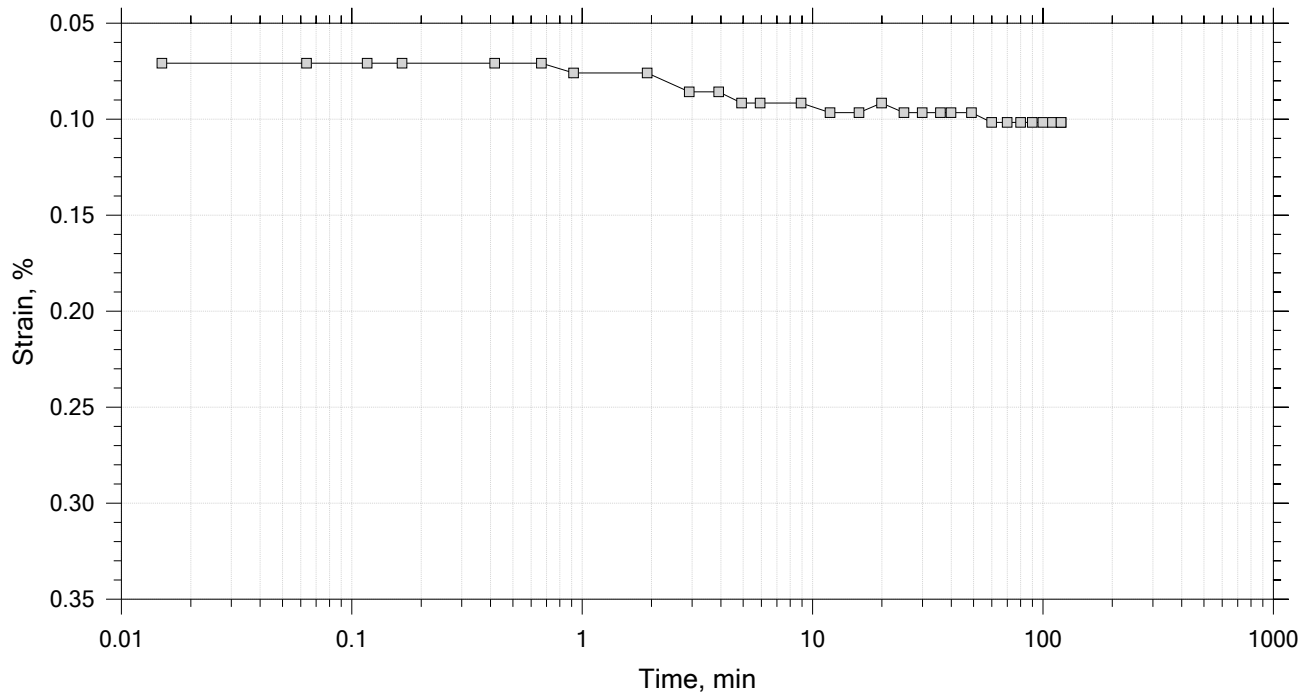
	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



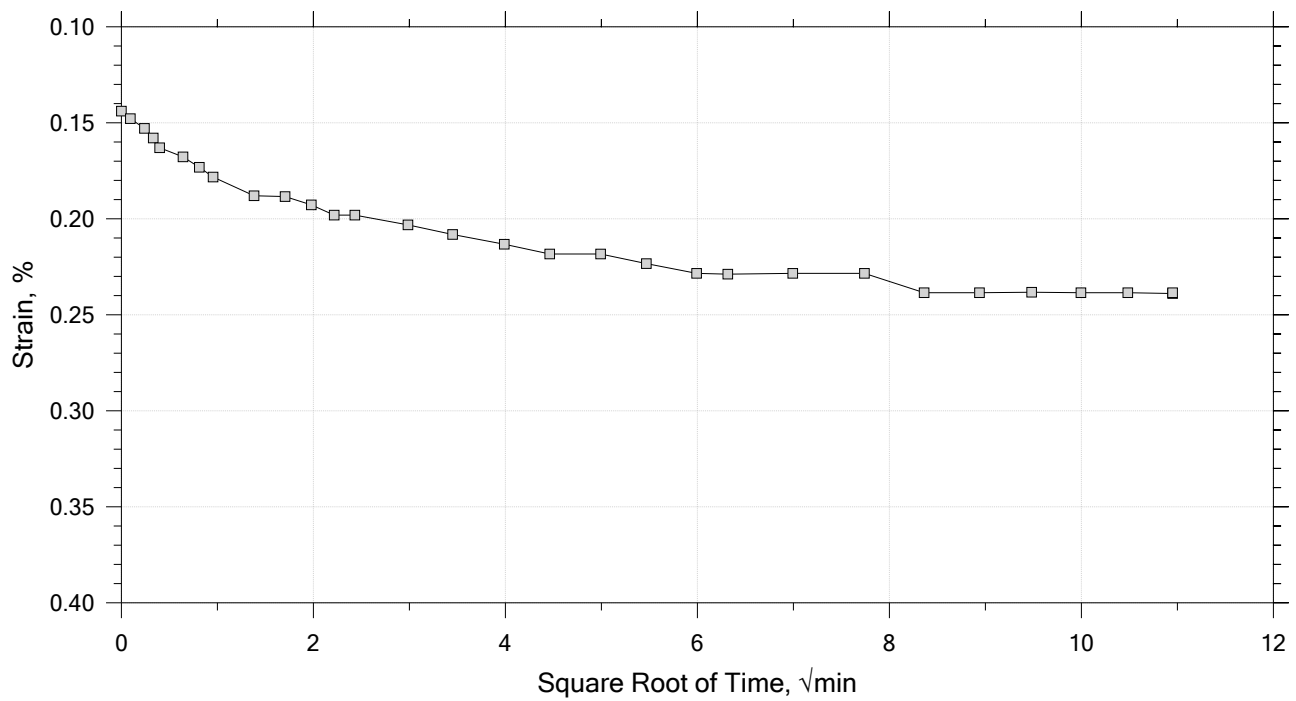
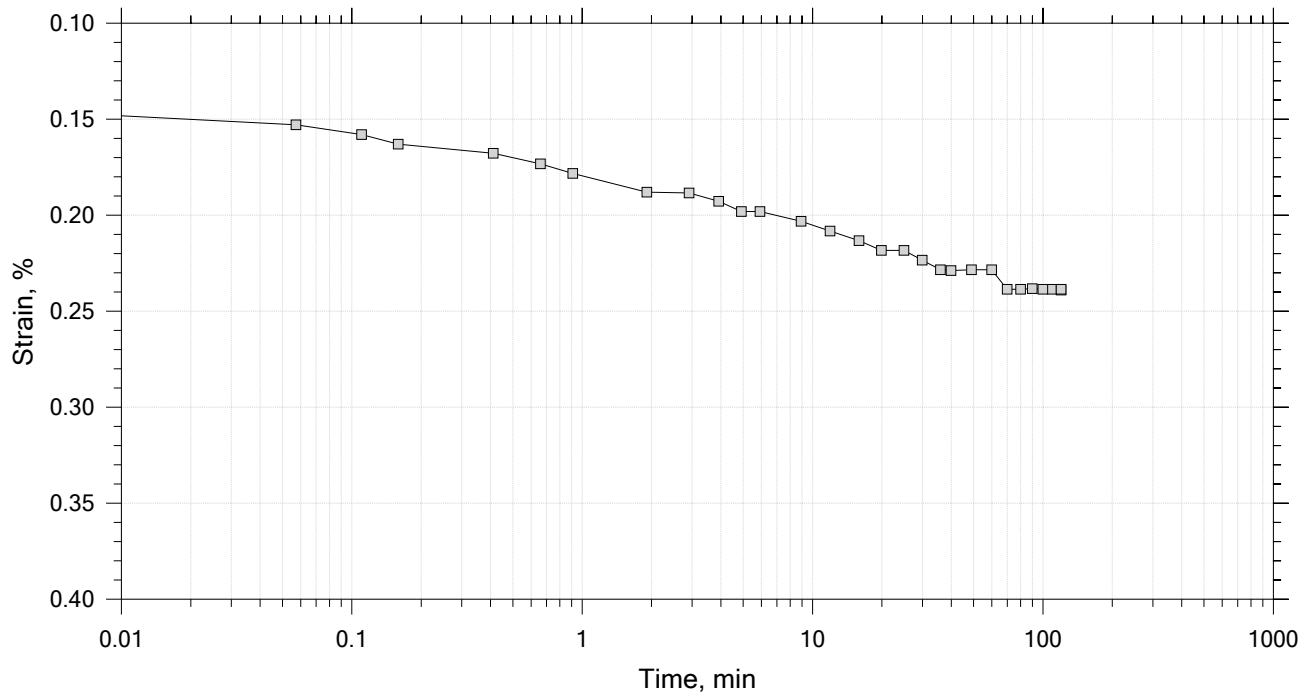
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



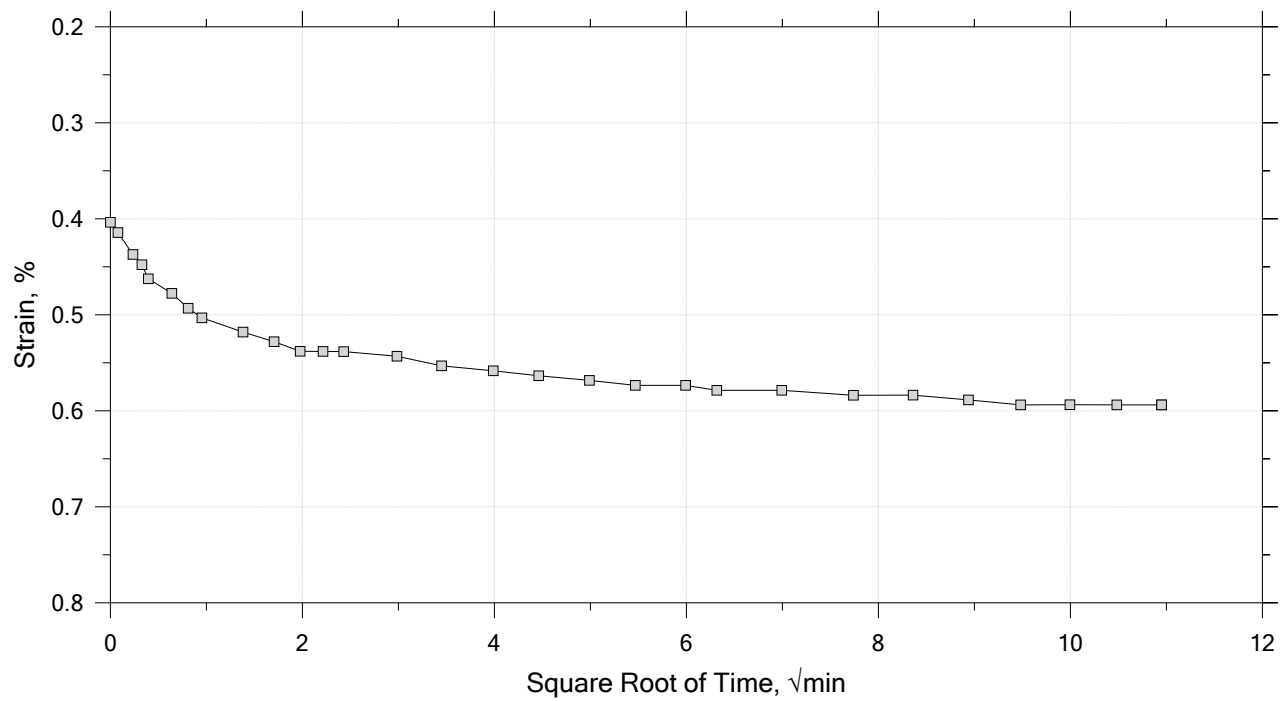
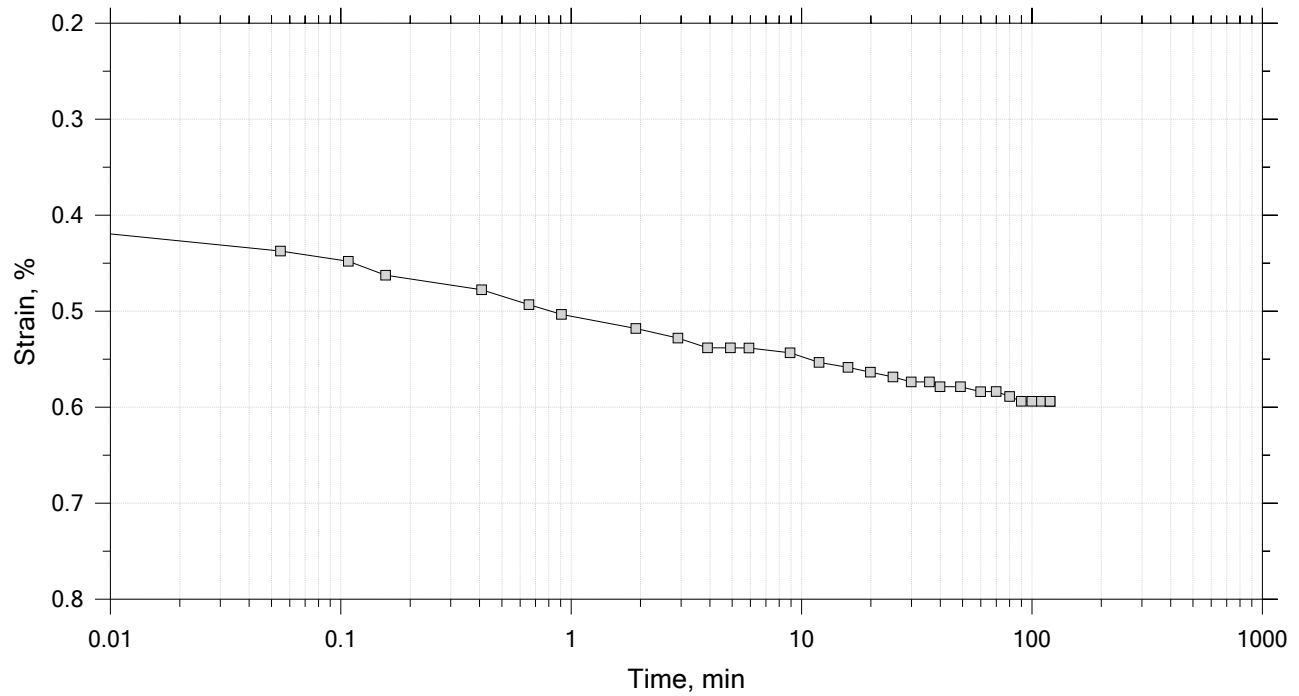
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



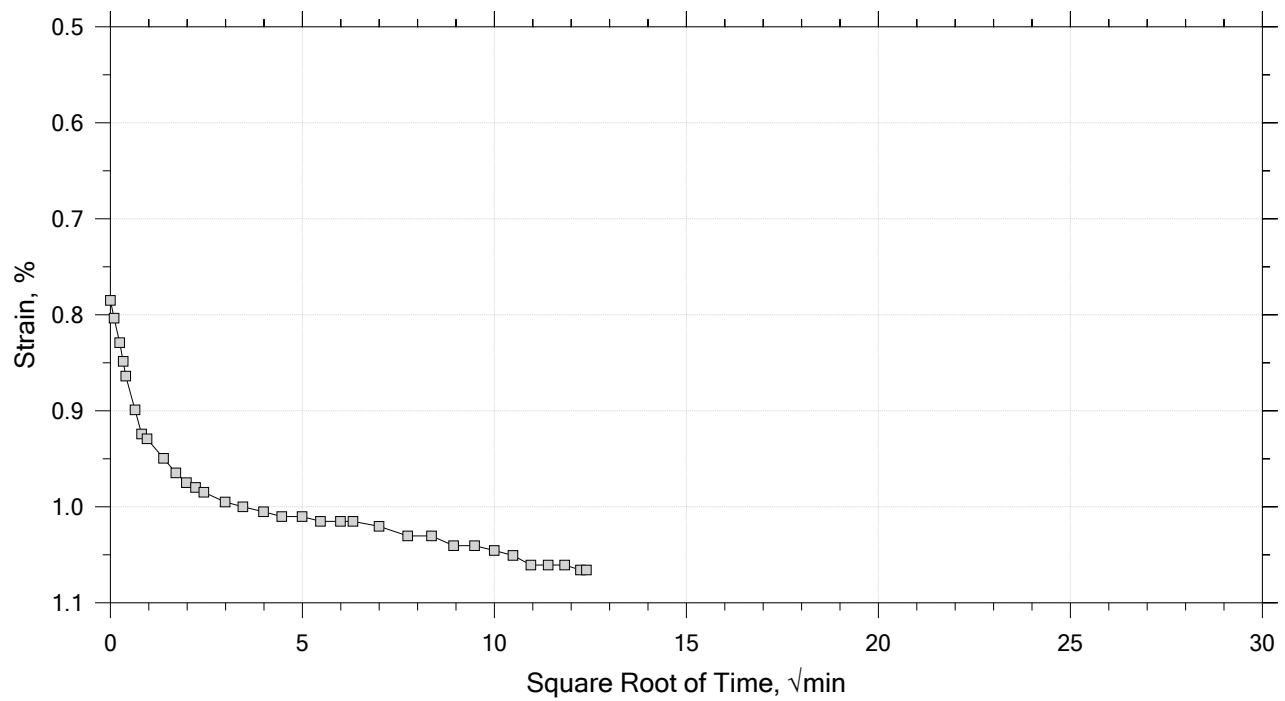
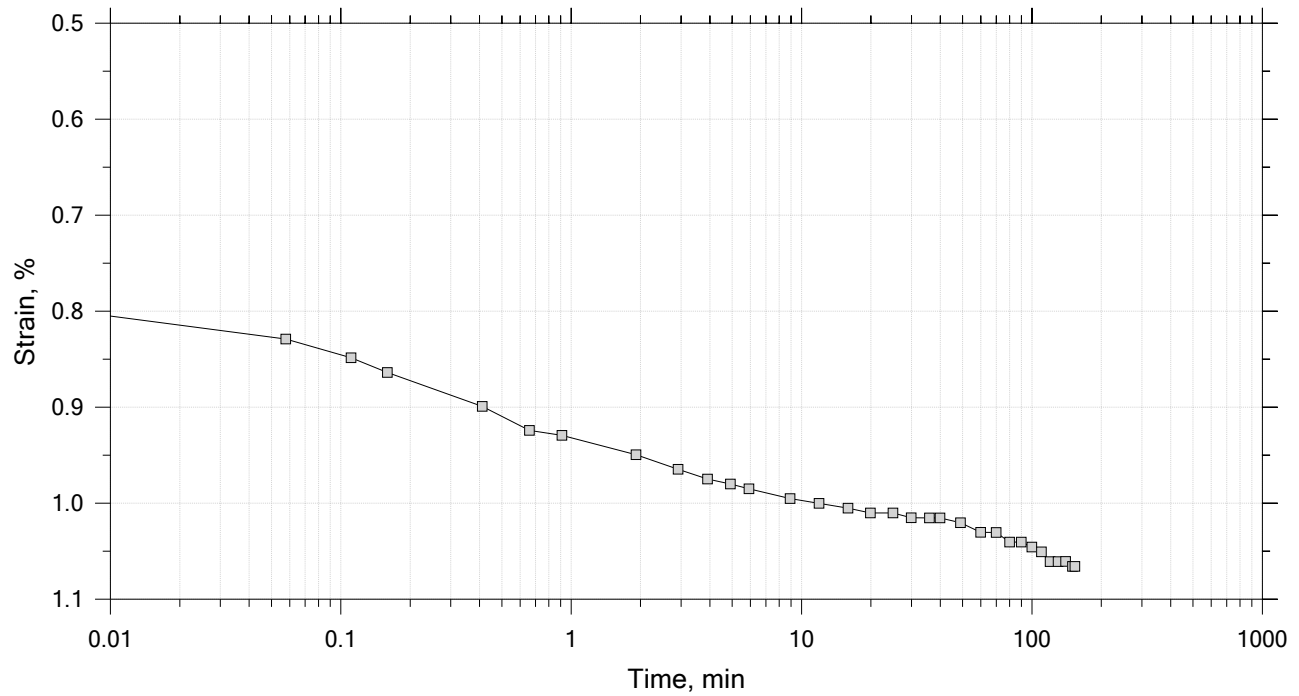
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



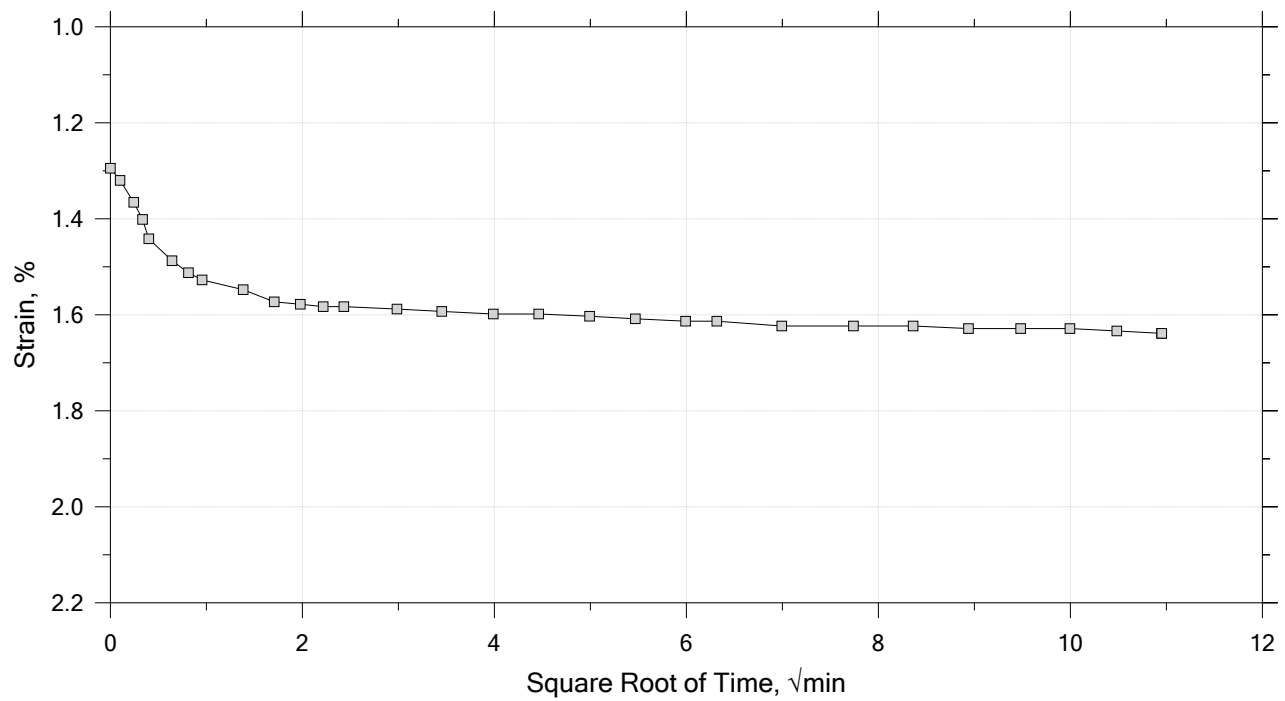
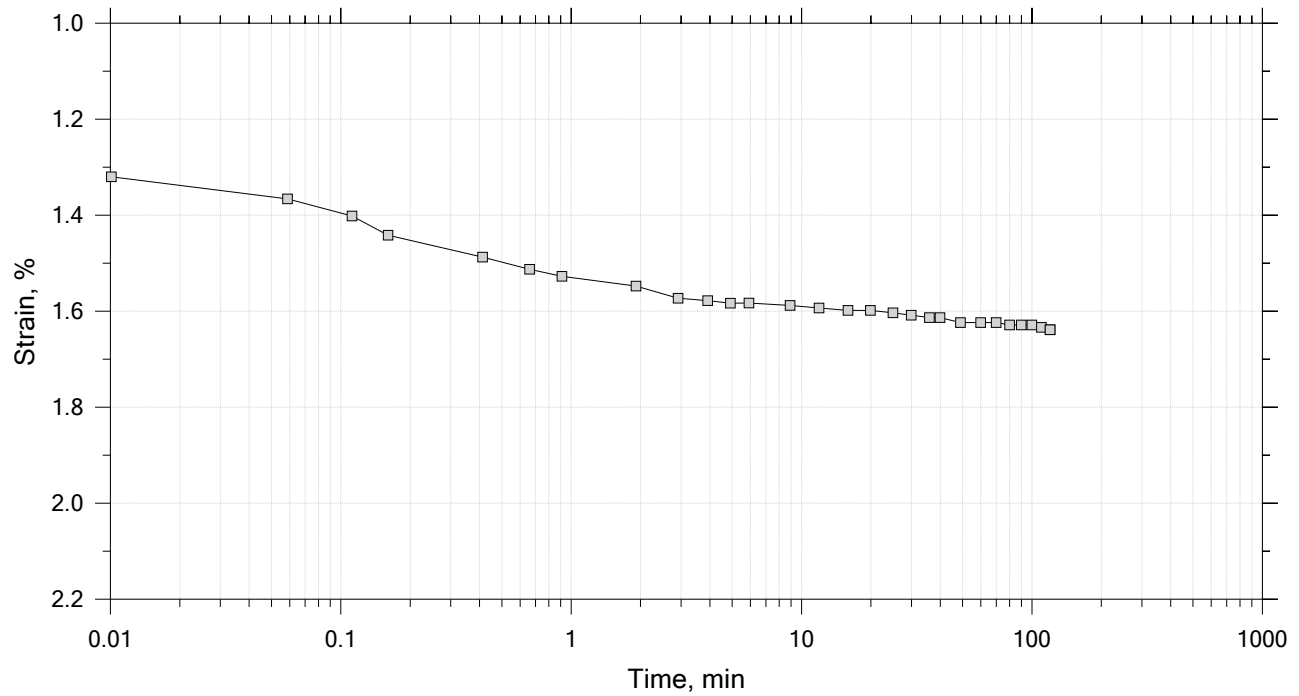
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



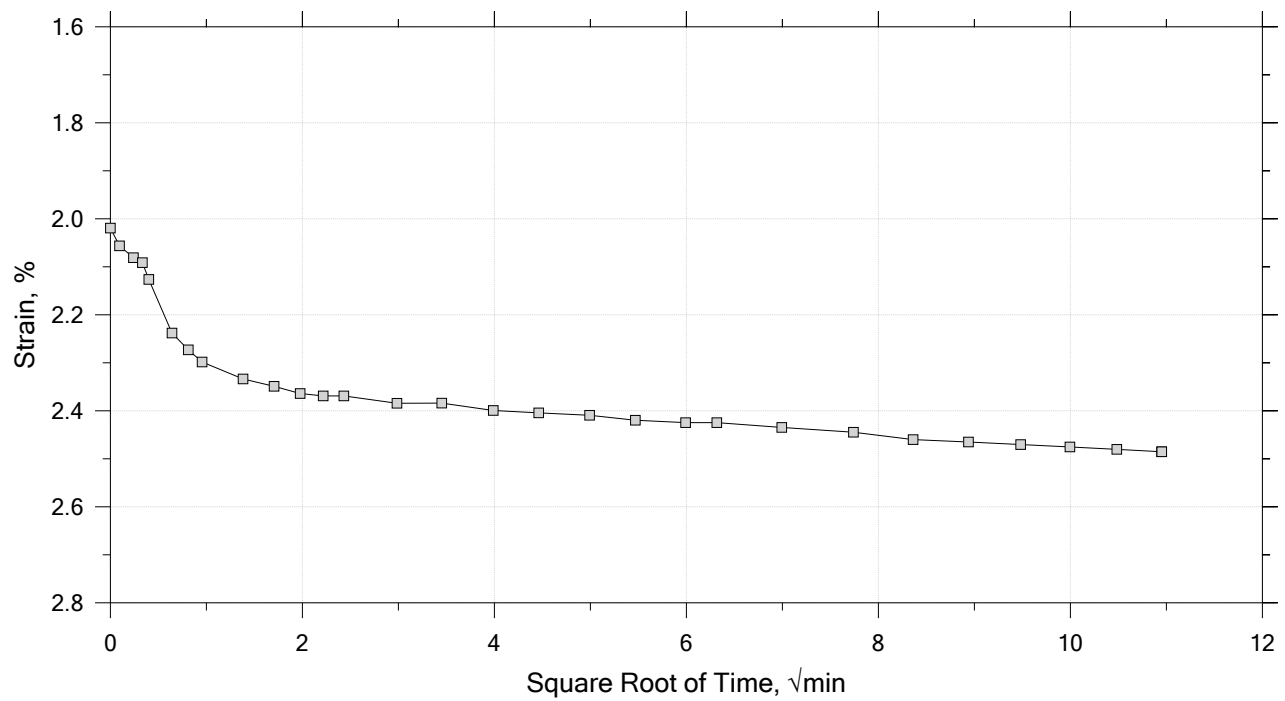
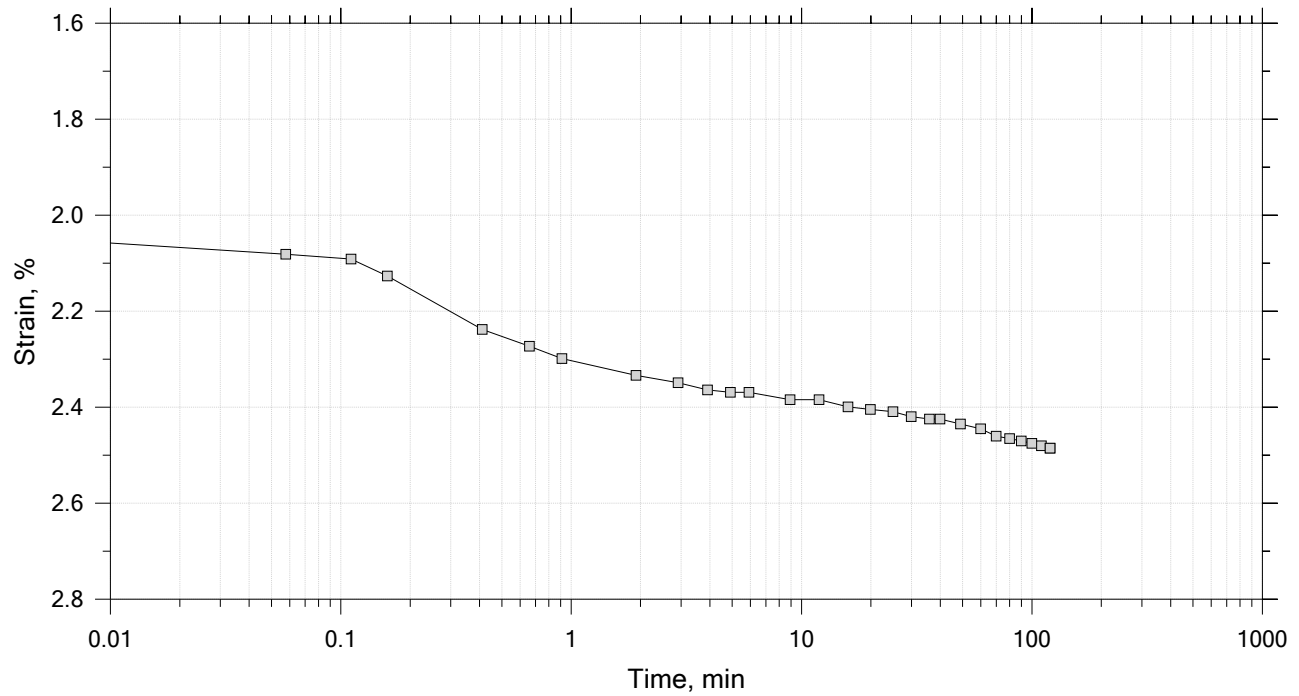
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



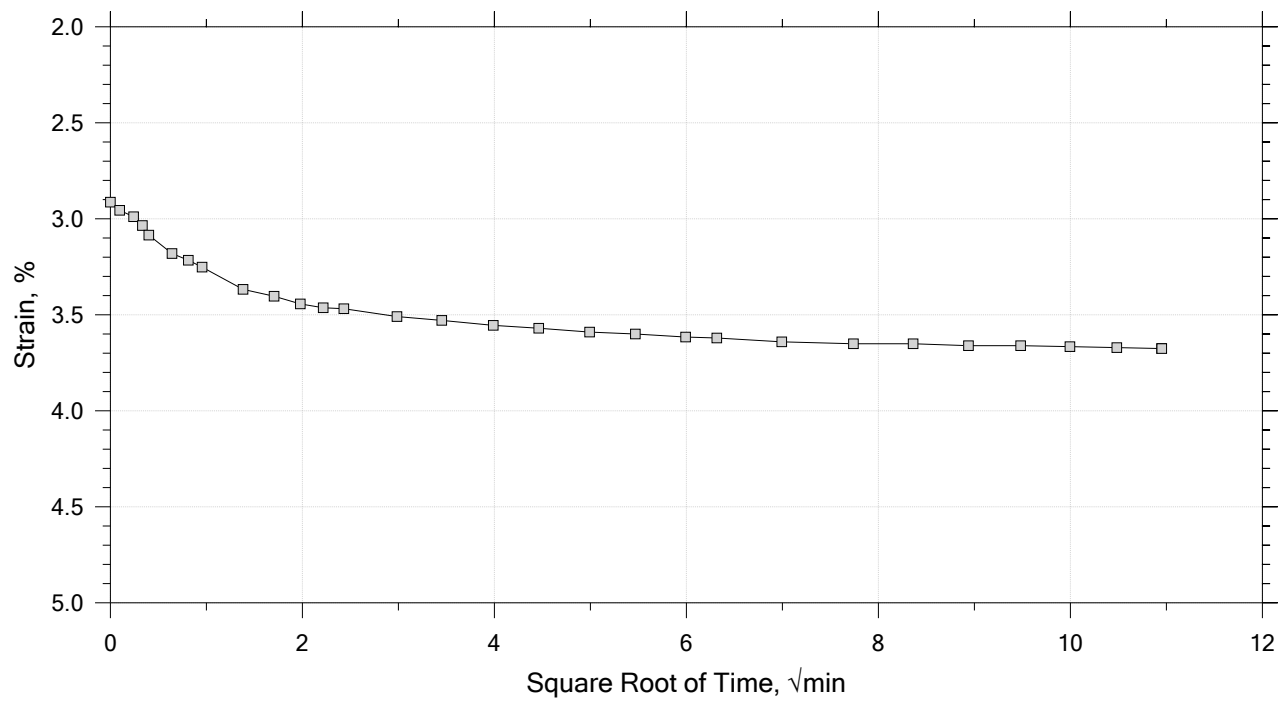
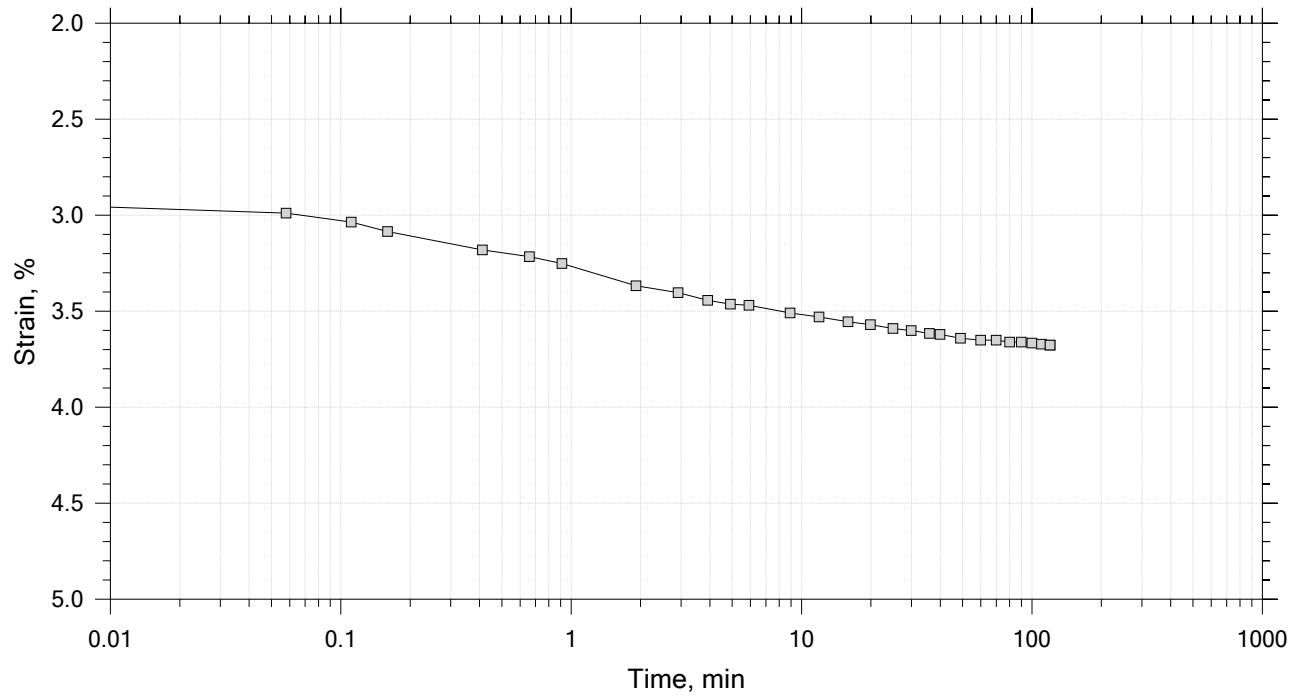
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



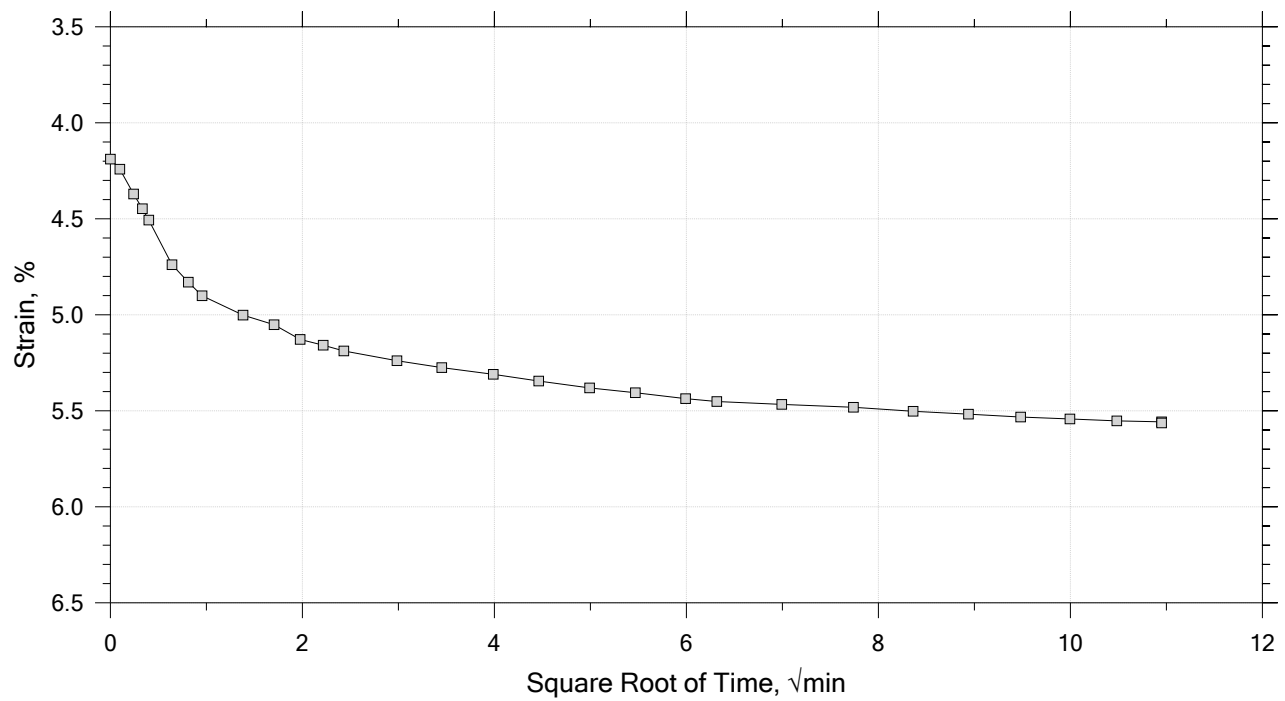
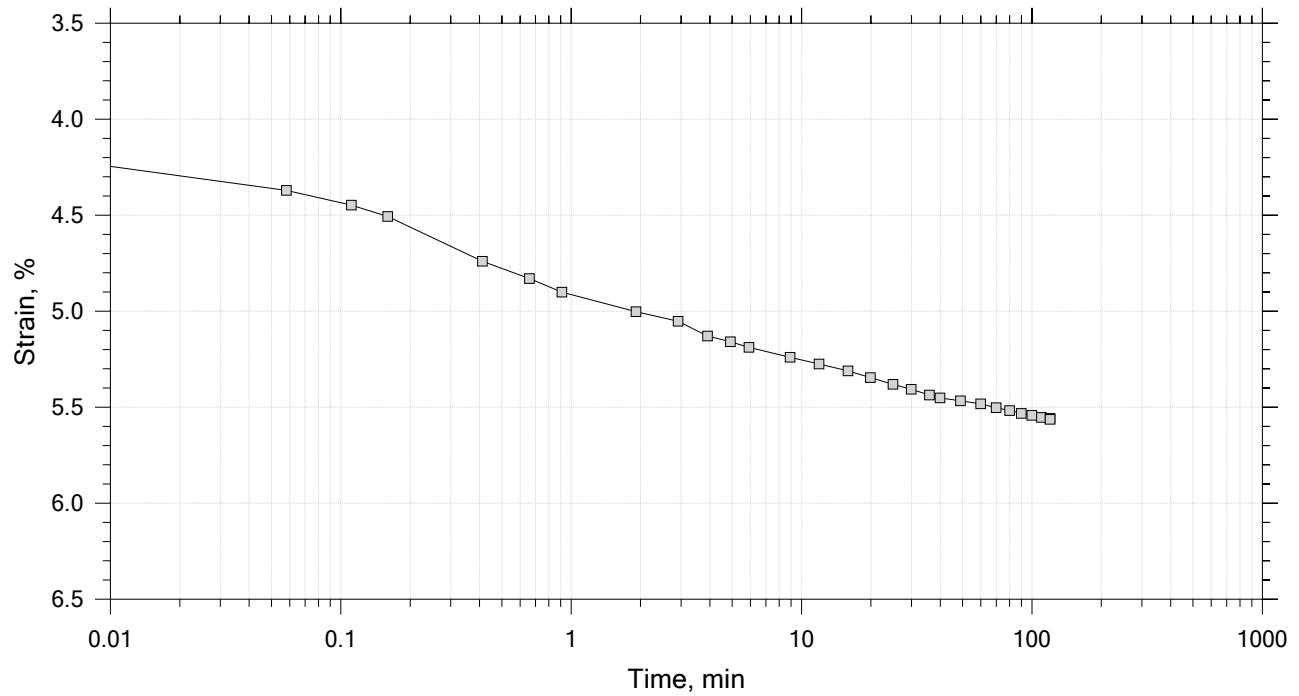
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



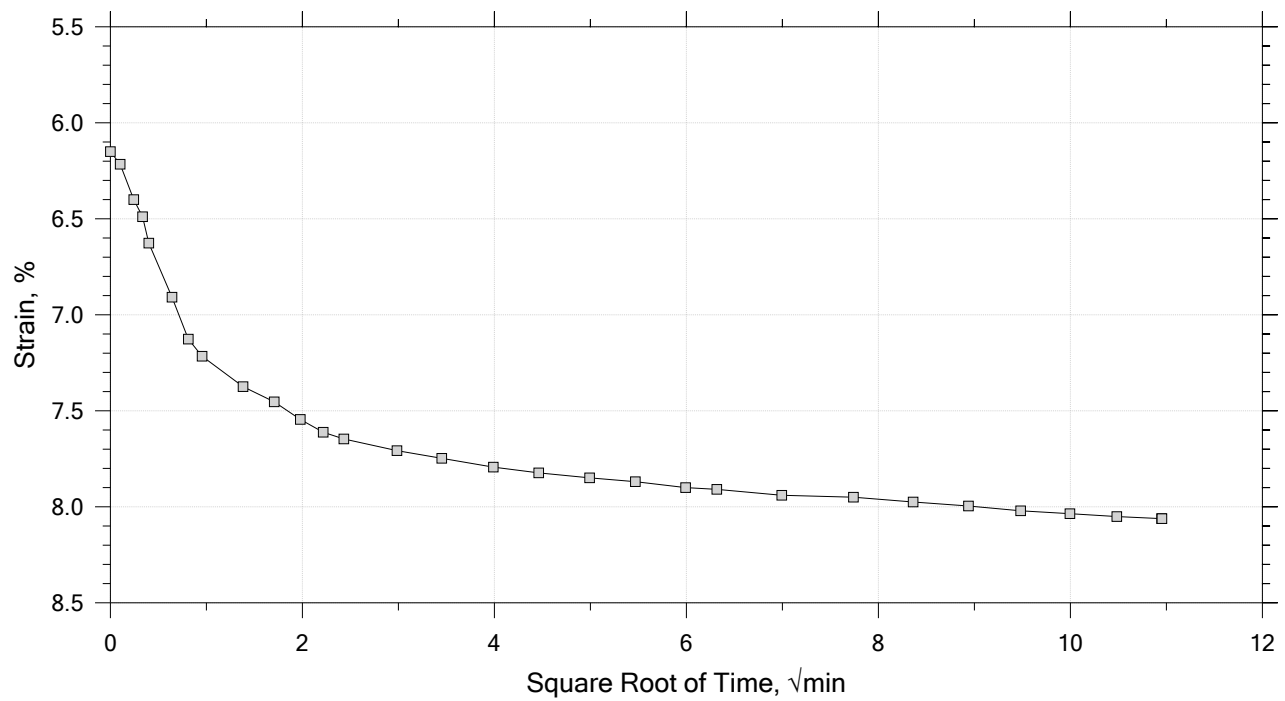
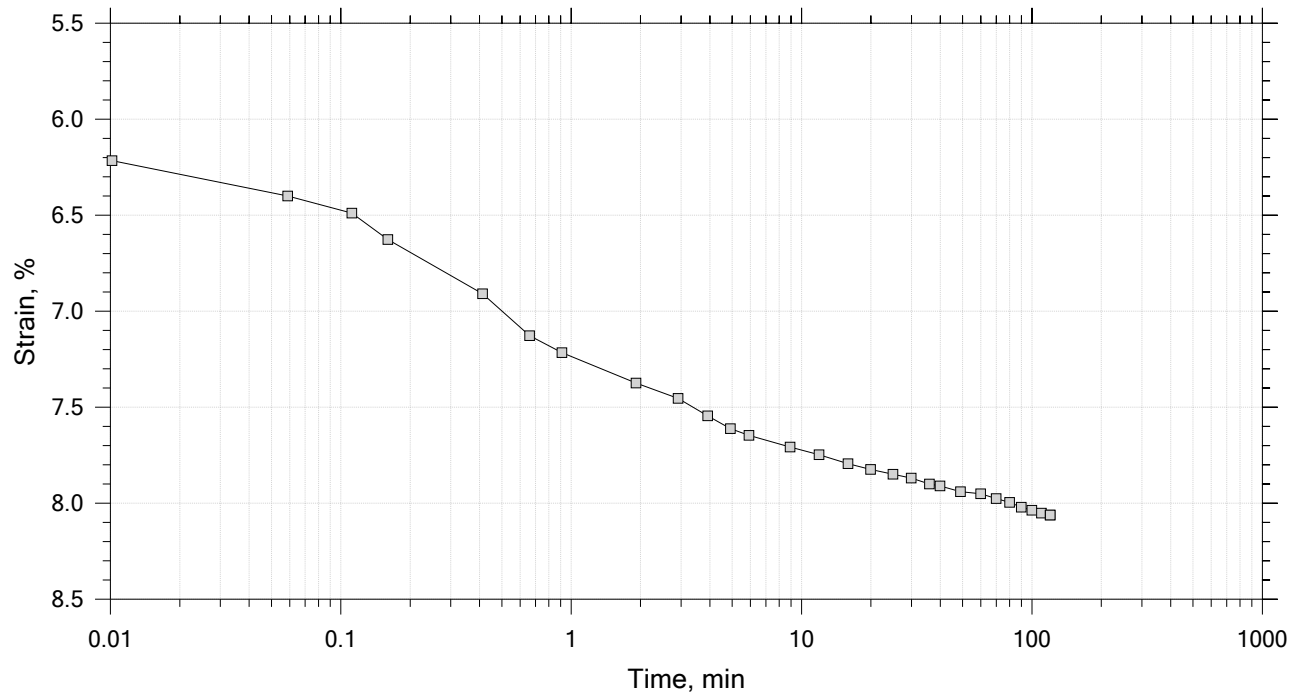
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



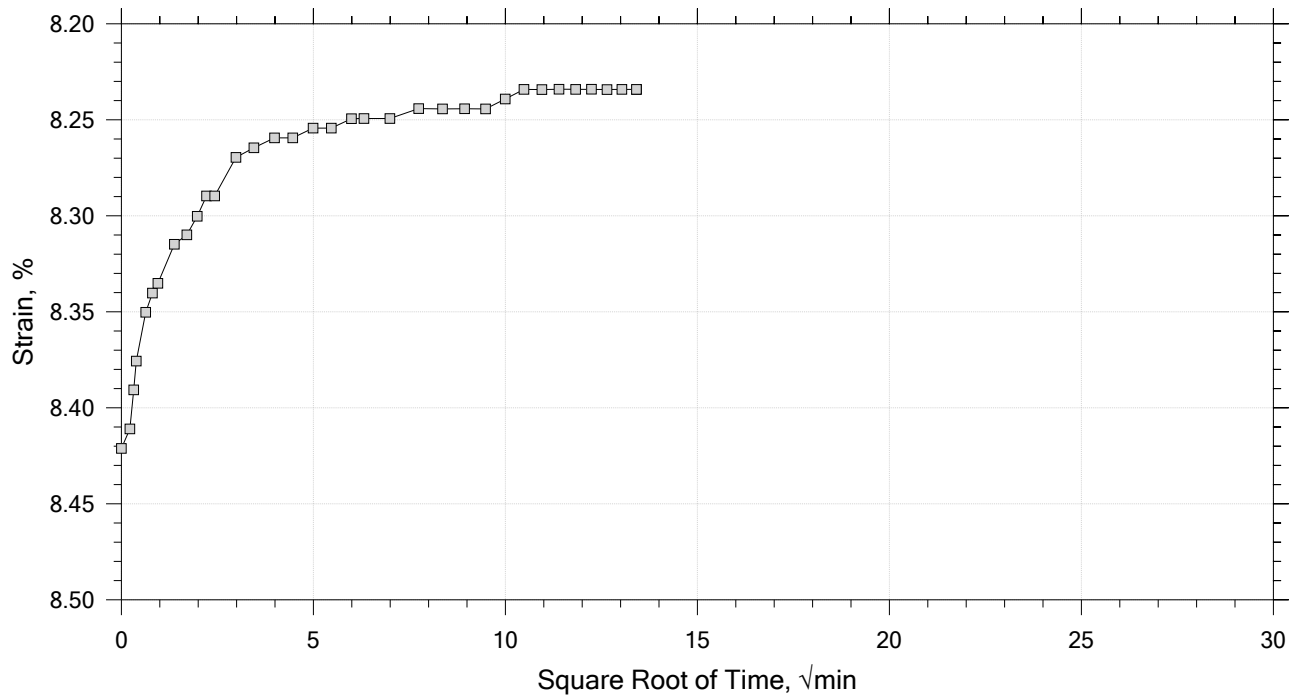
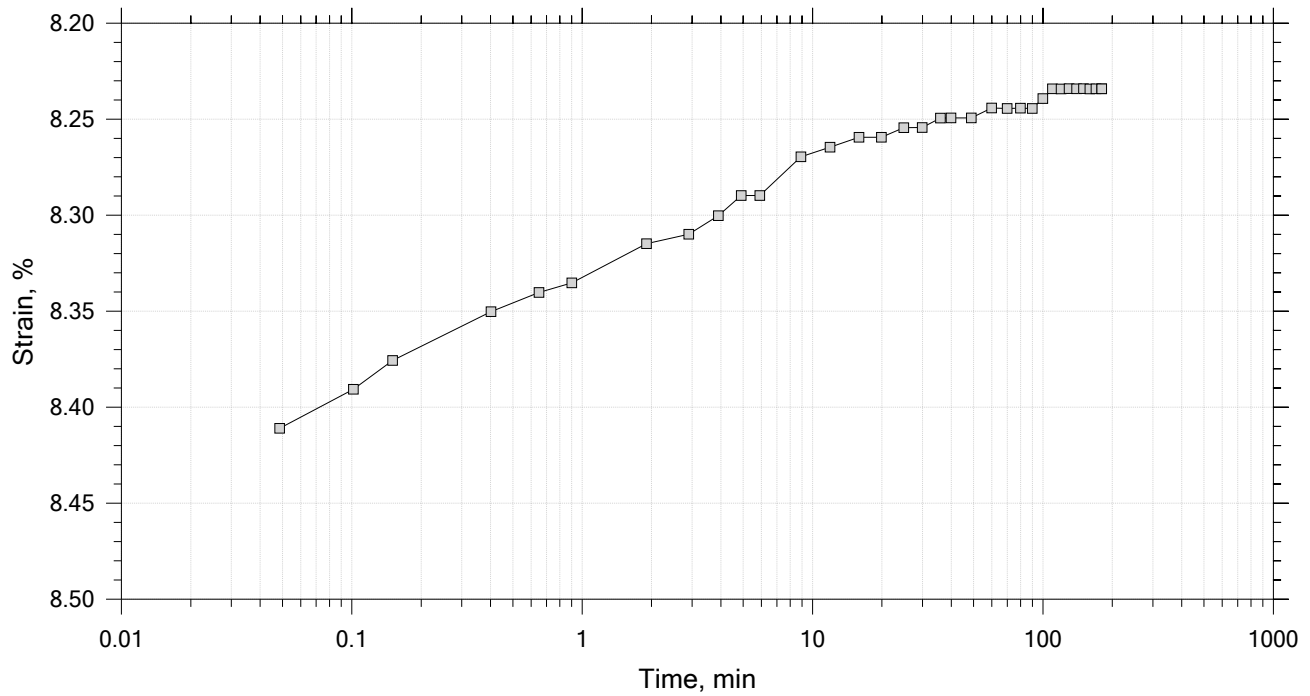
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



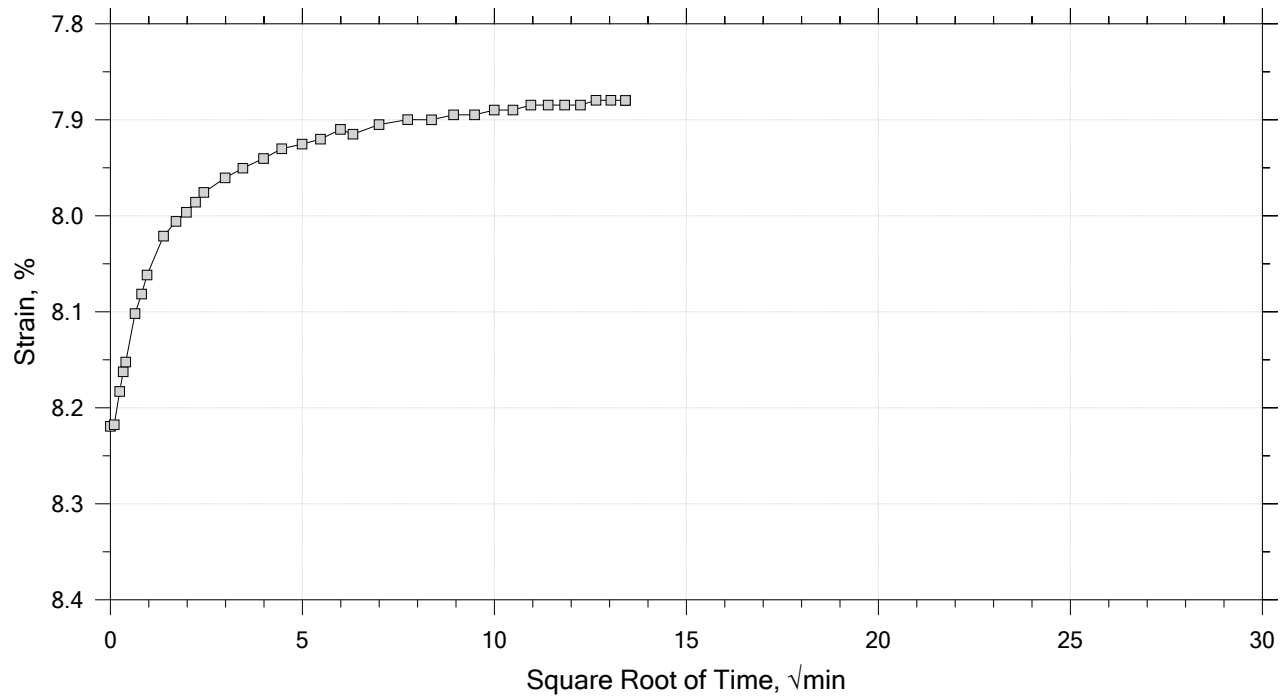
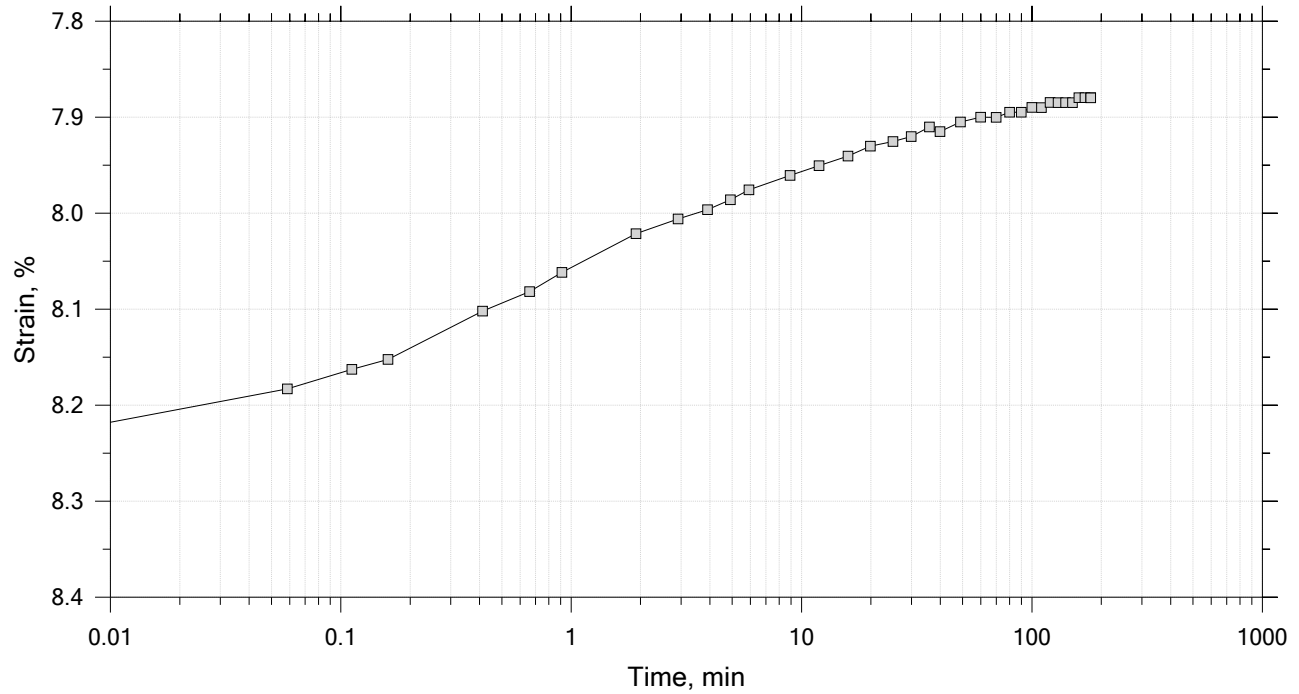
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



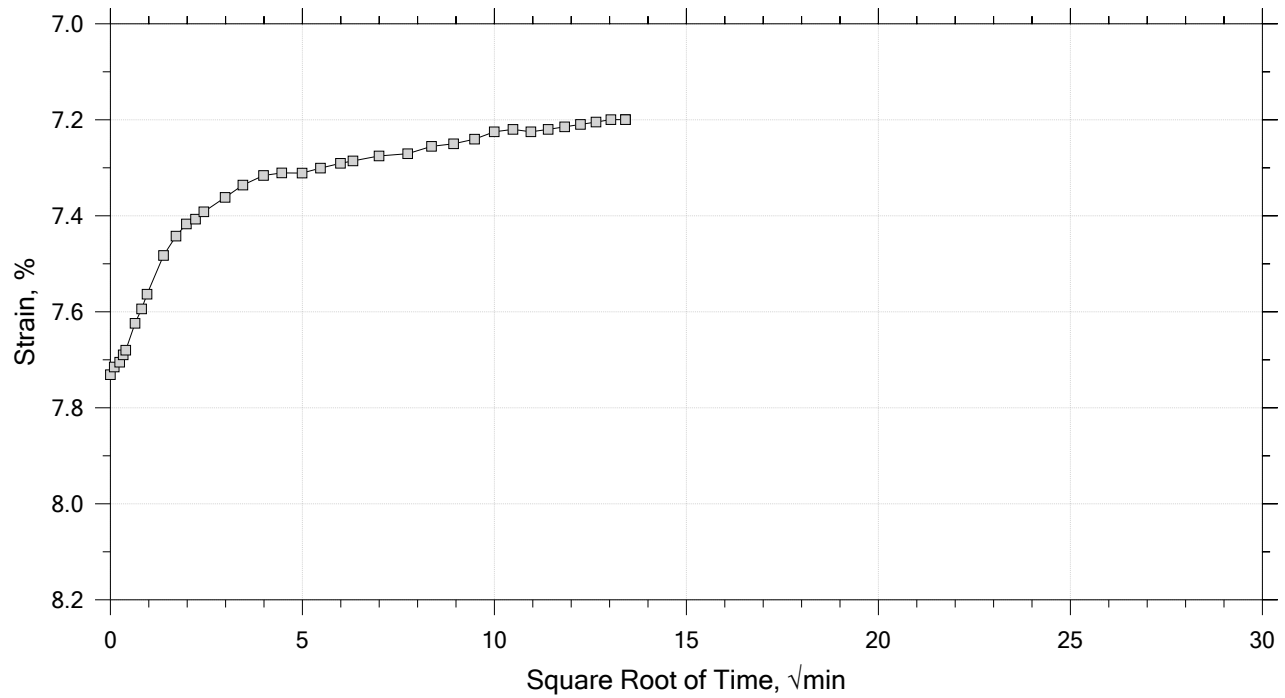
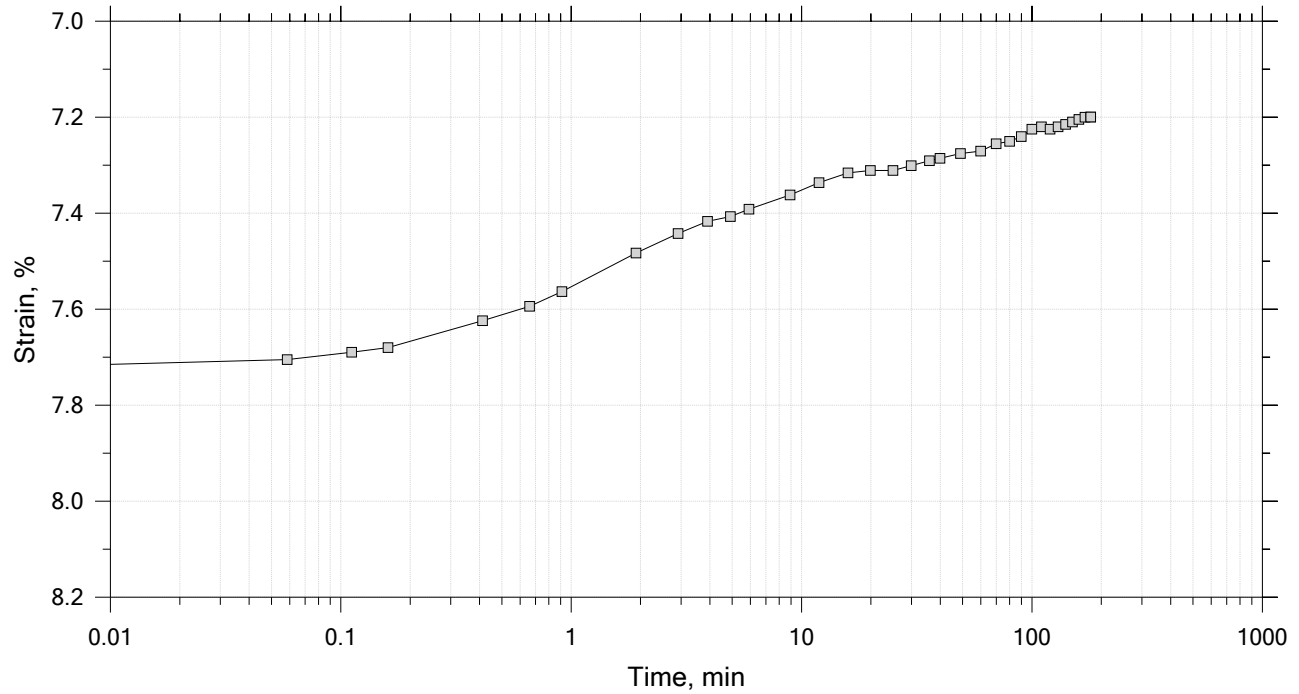
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	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



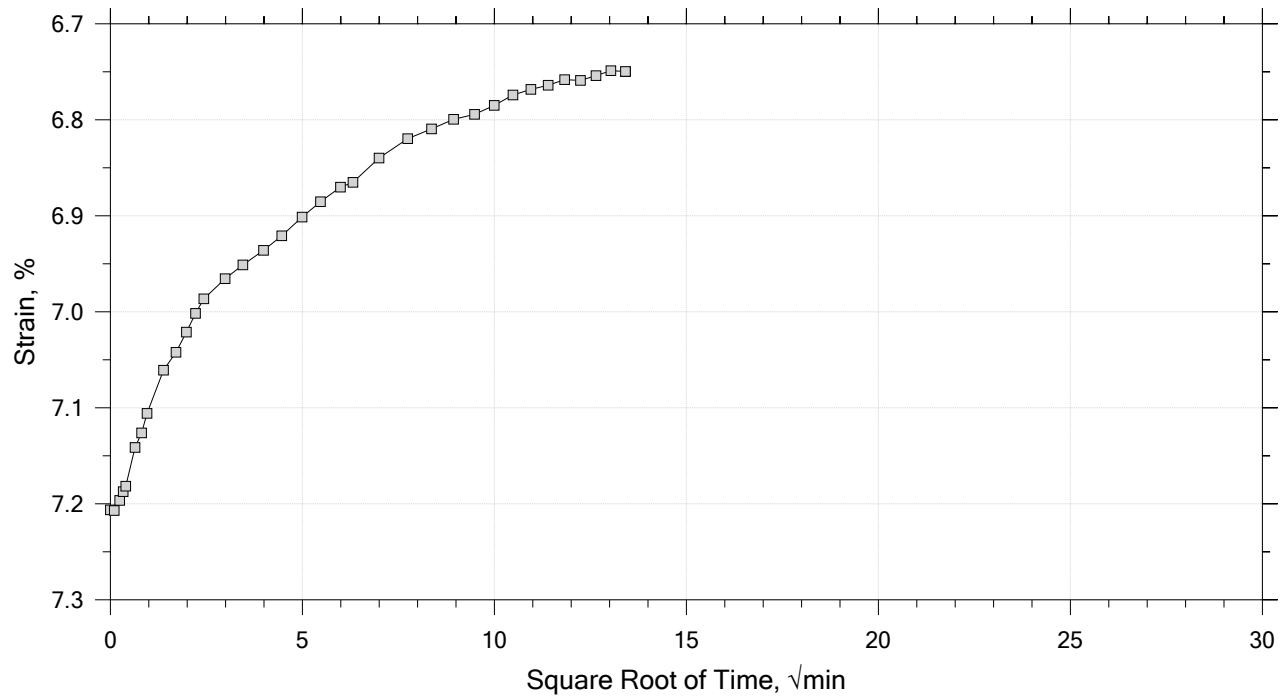
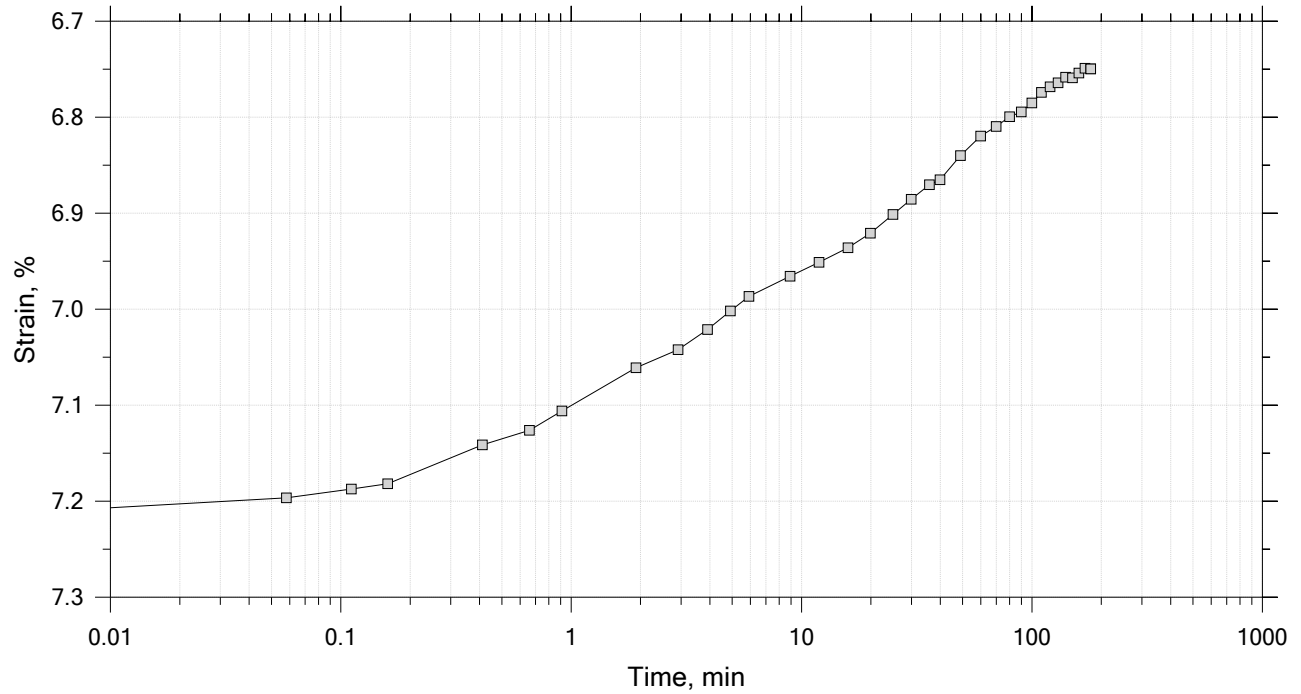
	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



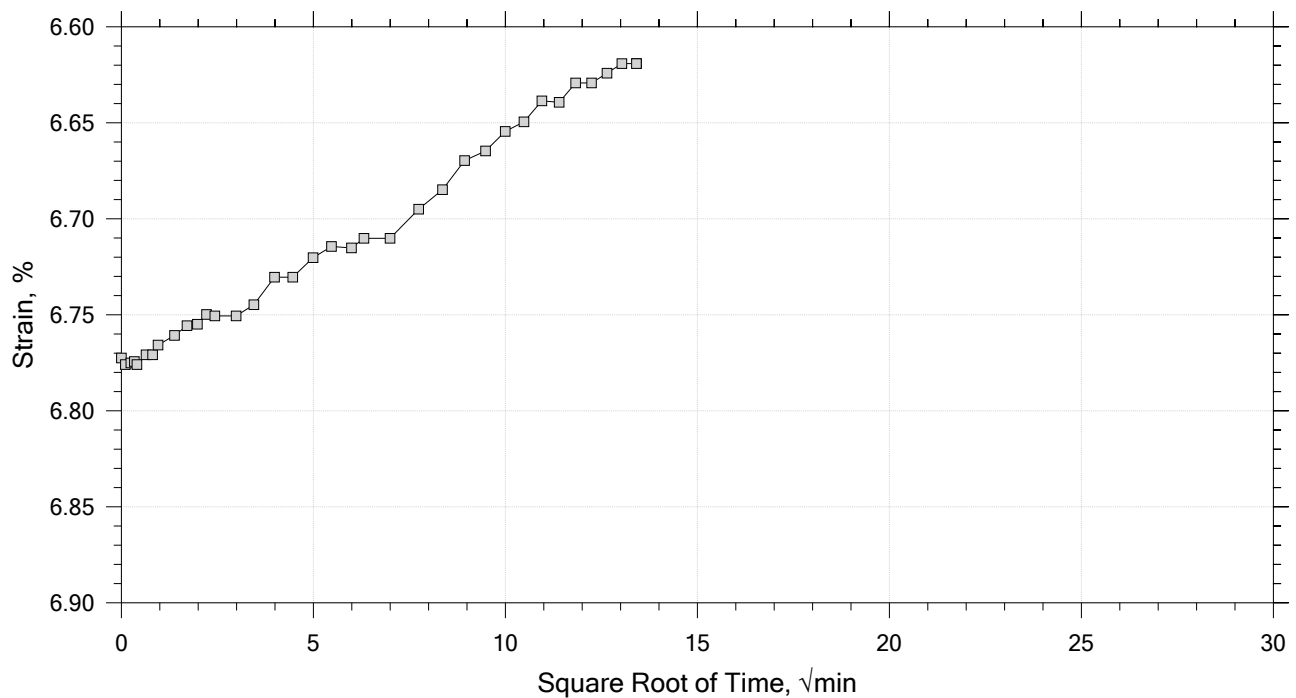
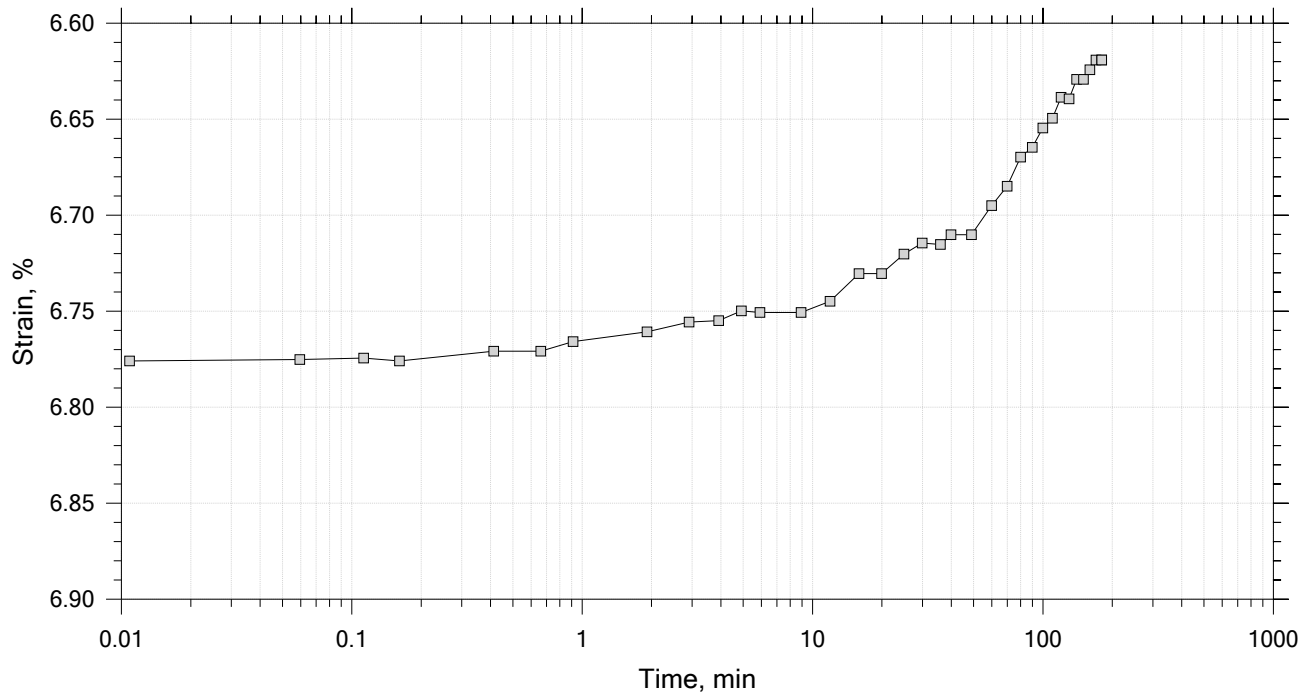
	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 0.847	Plastic Limit: 18
Final Height: 0.93 in	Final Void Ratio: 0.725	Plasticity Index: 17

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	B-722	RING		B-2443
Mass Container, gm	8.67	109.1	109.1	9.1
Mass Container + Wet Soil, gm	143.93	264.35	261.67	161.7
Mass Container + Dry Soil, gm	113.13	230.11	230.11	130.13
Mass Dry Soil, gm	104.46	121.01	121.01	121.03
Water Content, %	29.48	28.30	26.08	26.08
Void Ratio	---	0.85	0.72	---
Degree of Saturation, %	---	92.83	100.00	---
Dry Unit Weight, pcf	---	93.911	100.57	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

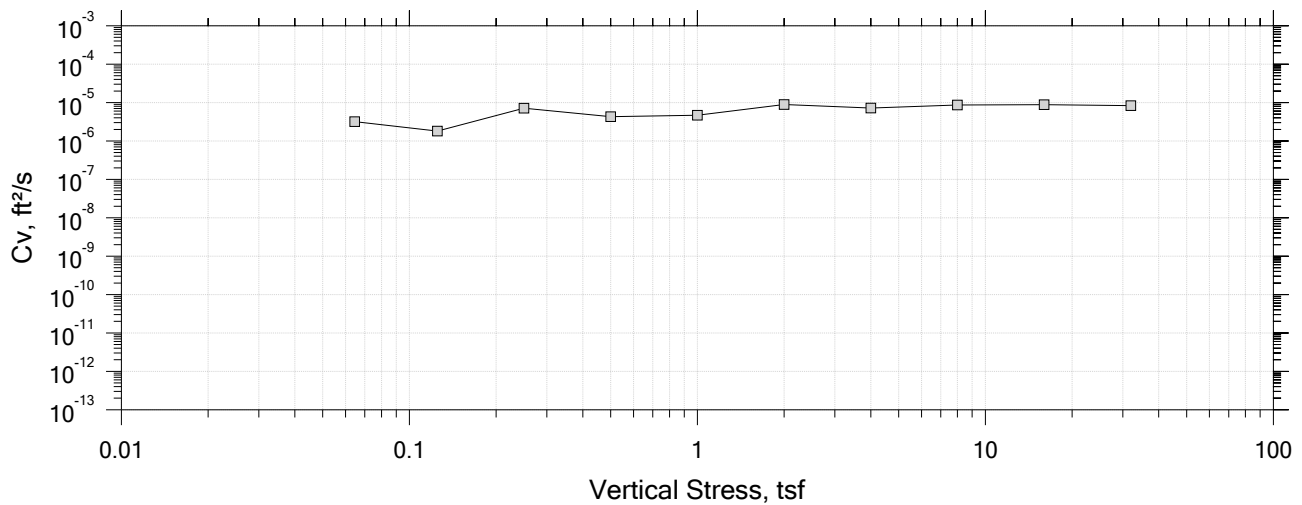
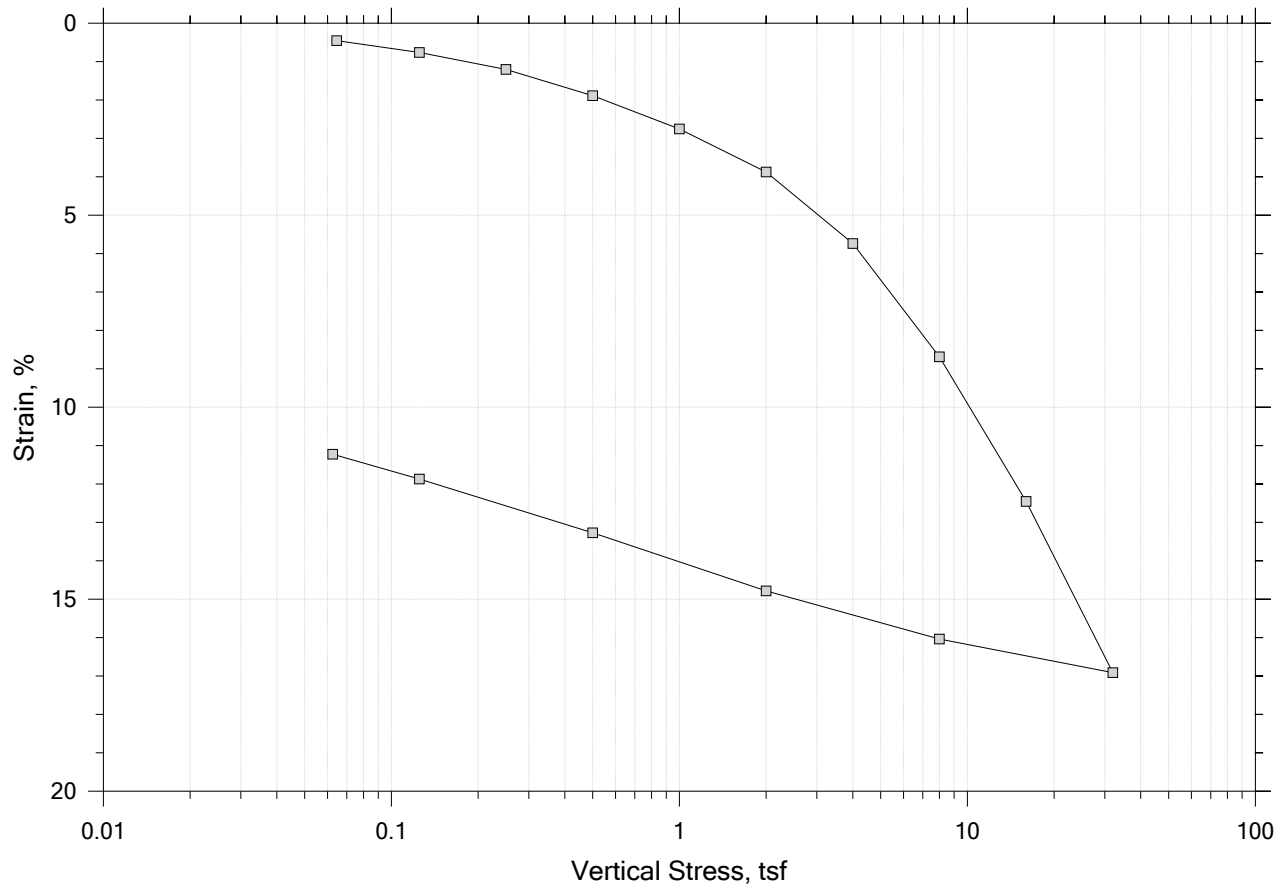
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: BB-EEBT2-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/13/19	Depth: 5-7 ft
	Test No.: IP-10	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System O, Swell Pressure = 0.0859 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

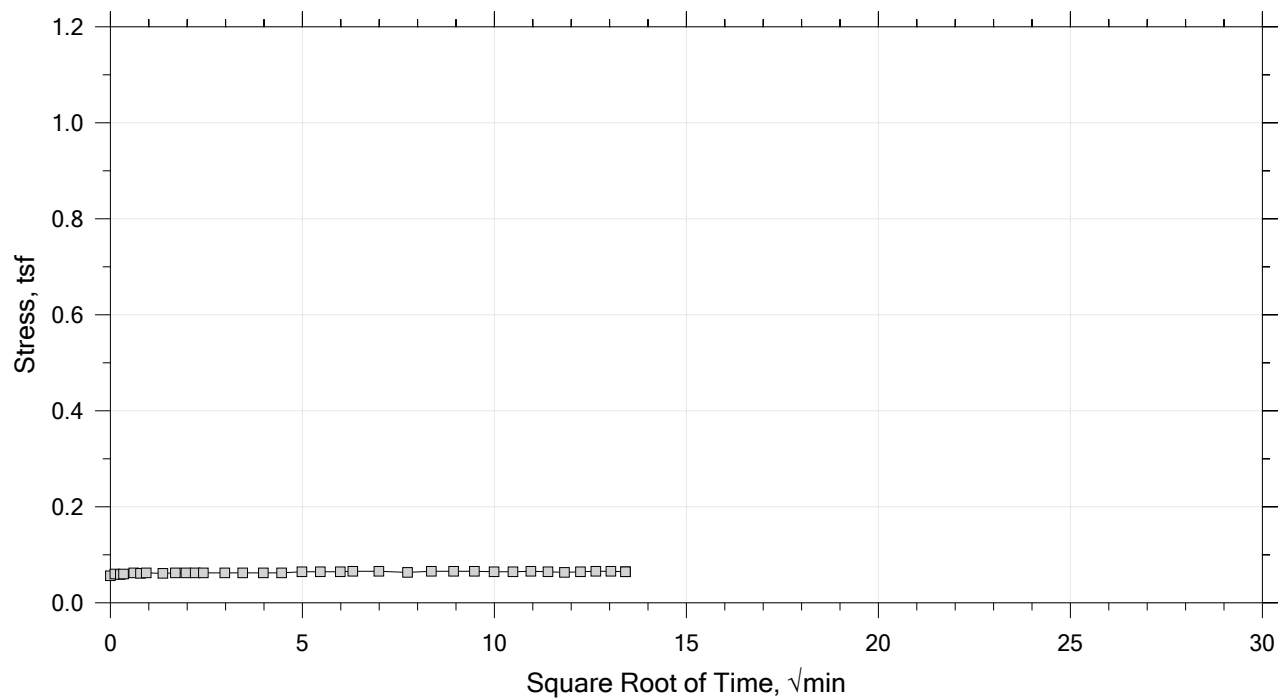
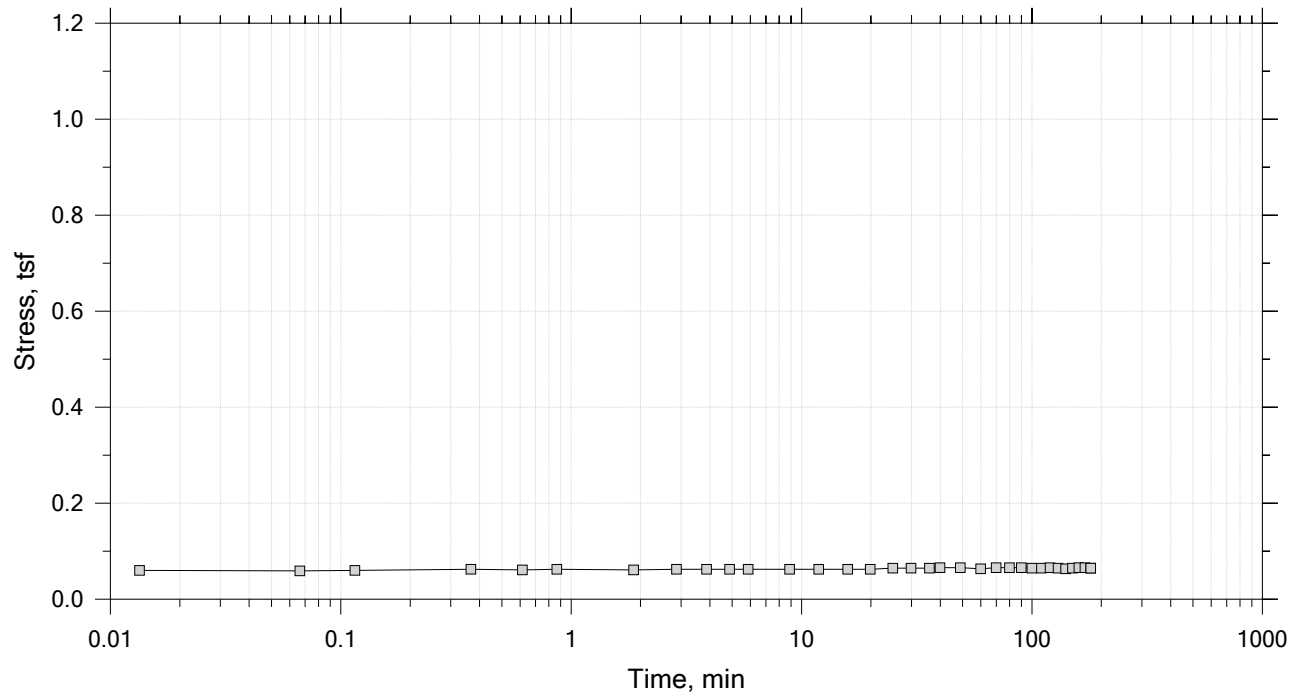
Summary Report




	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0645 tsf



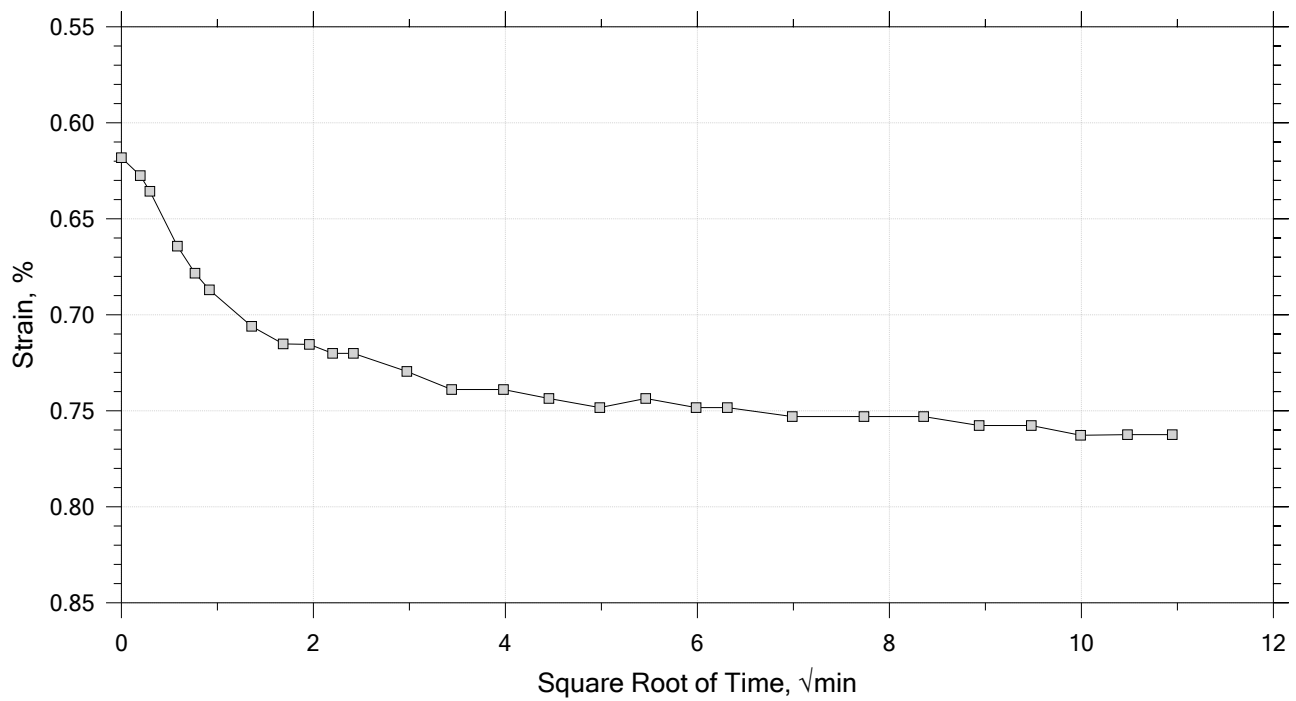
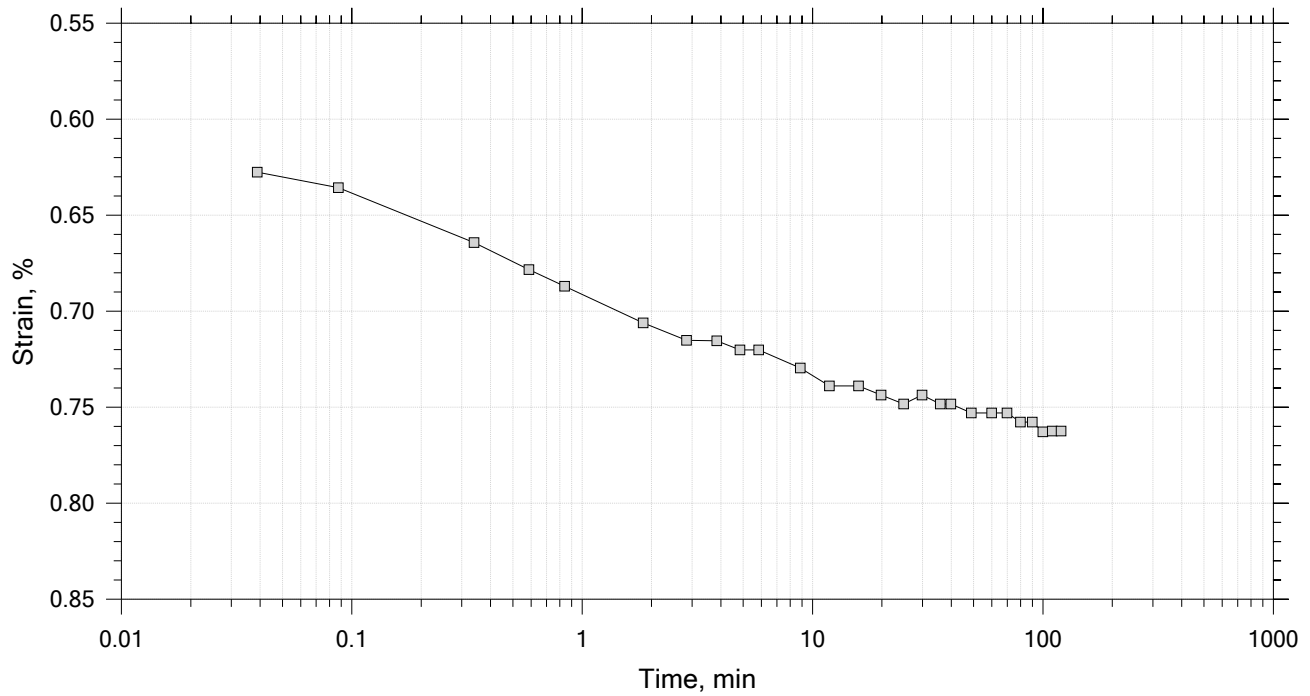
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	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



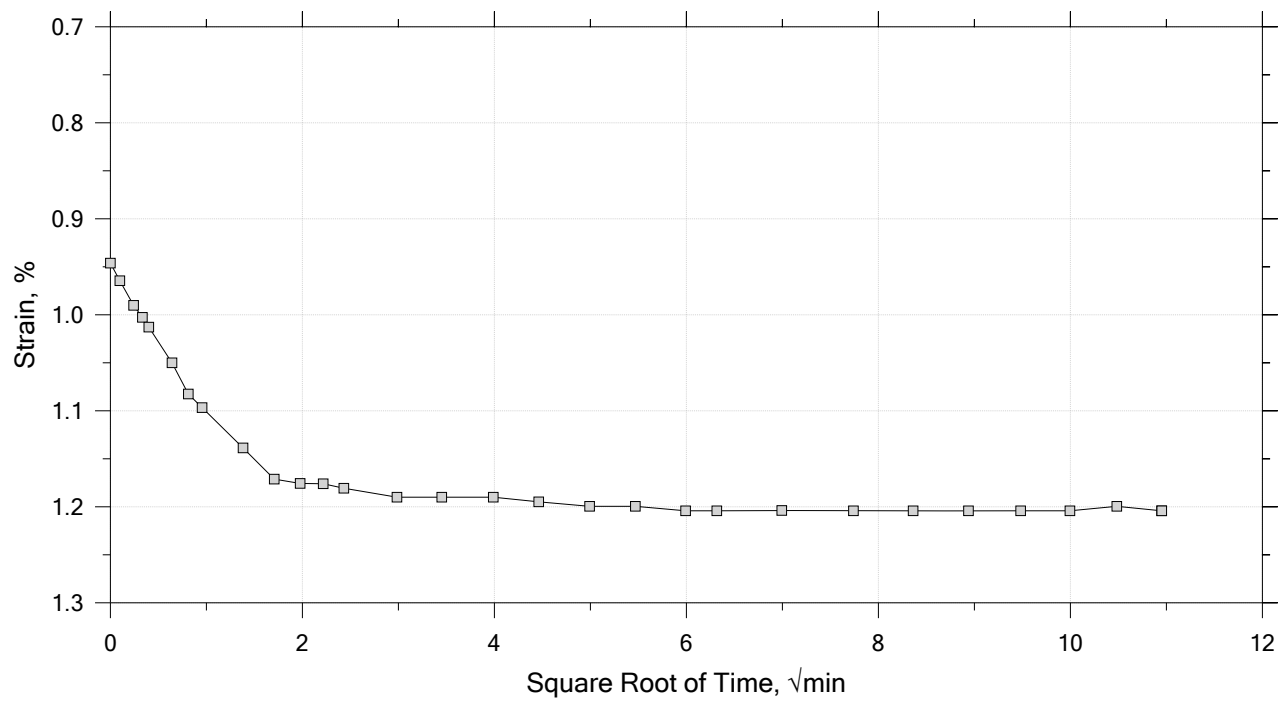
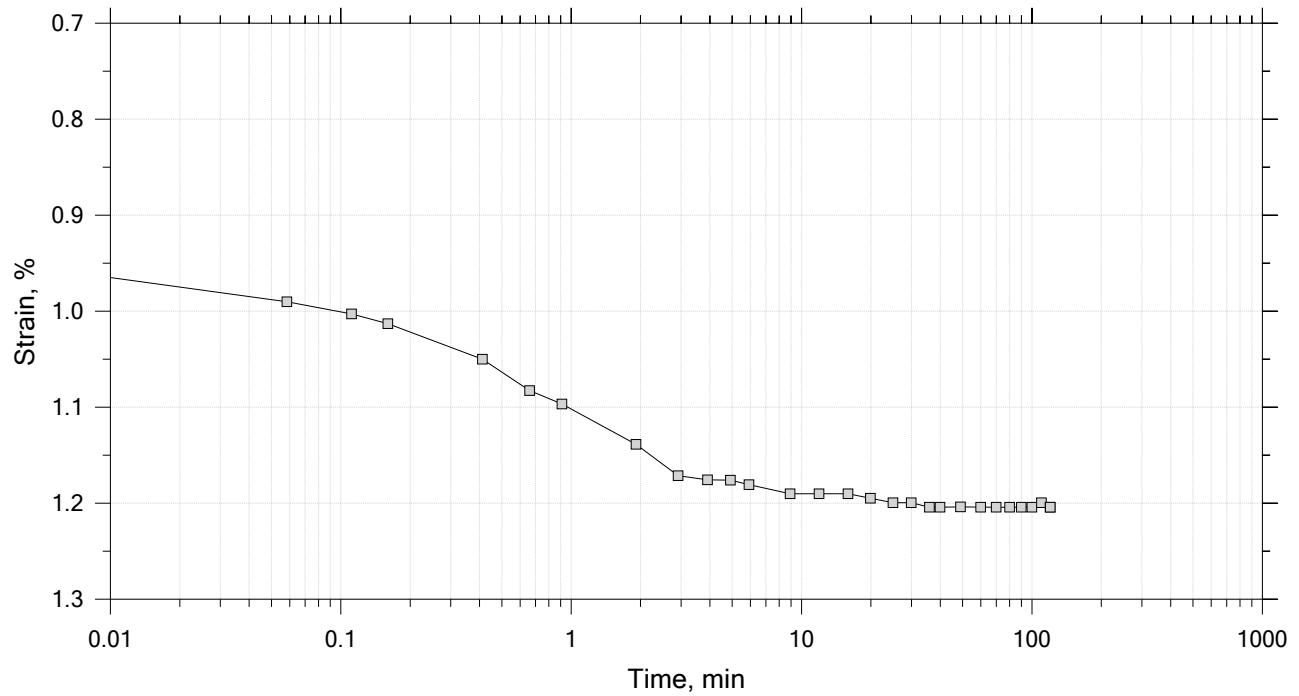
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	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



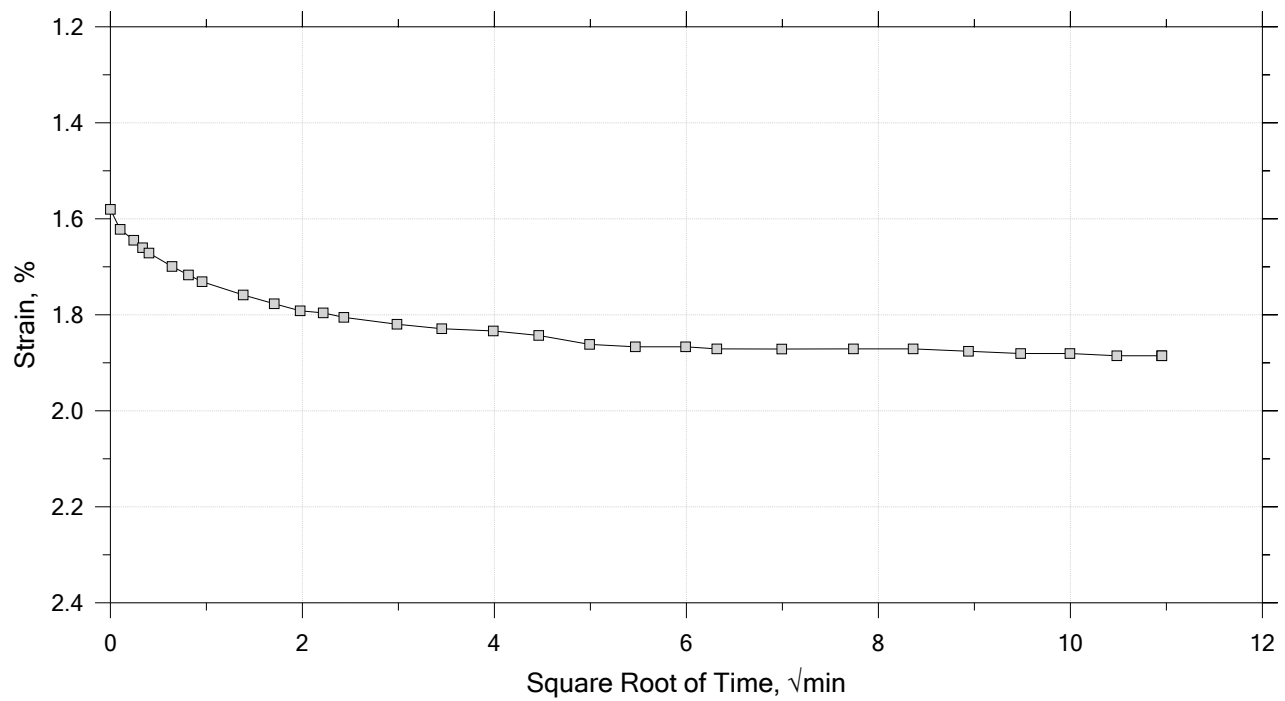
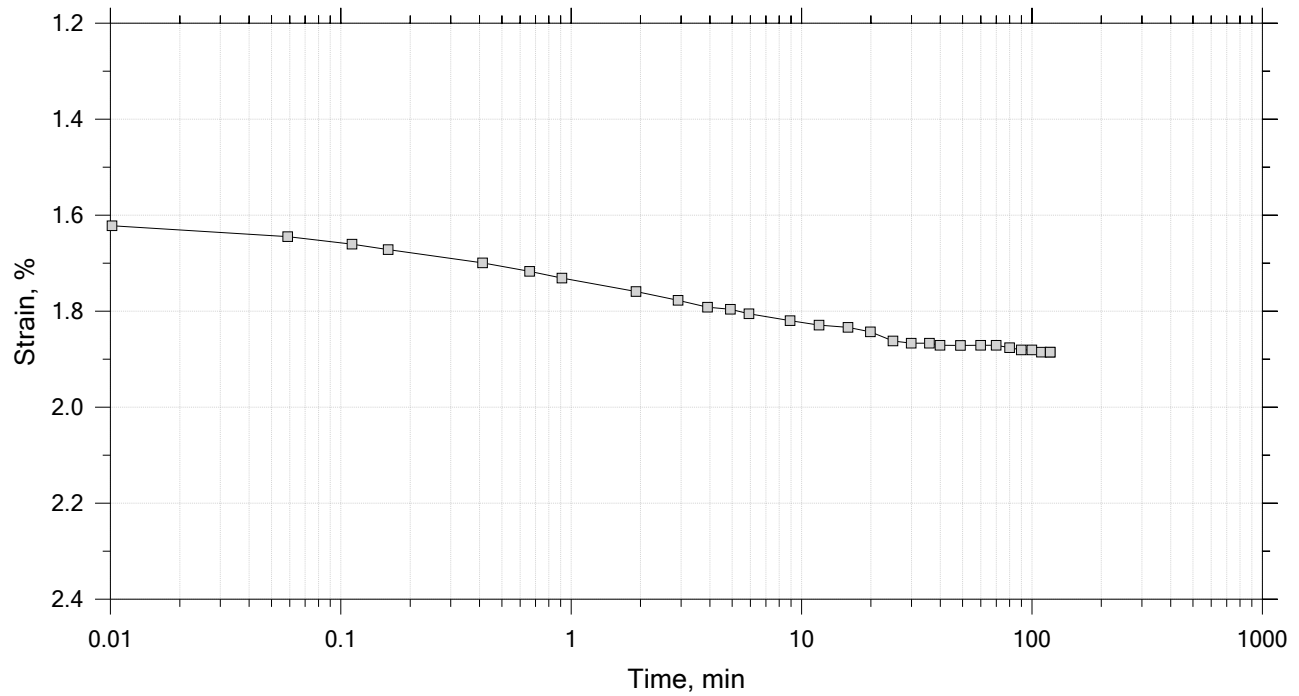
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



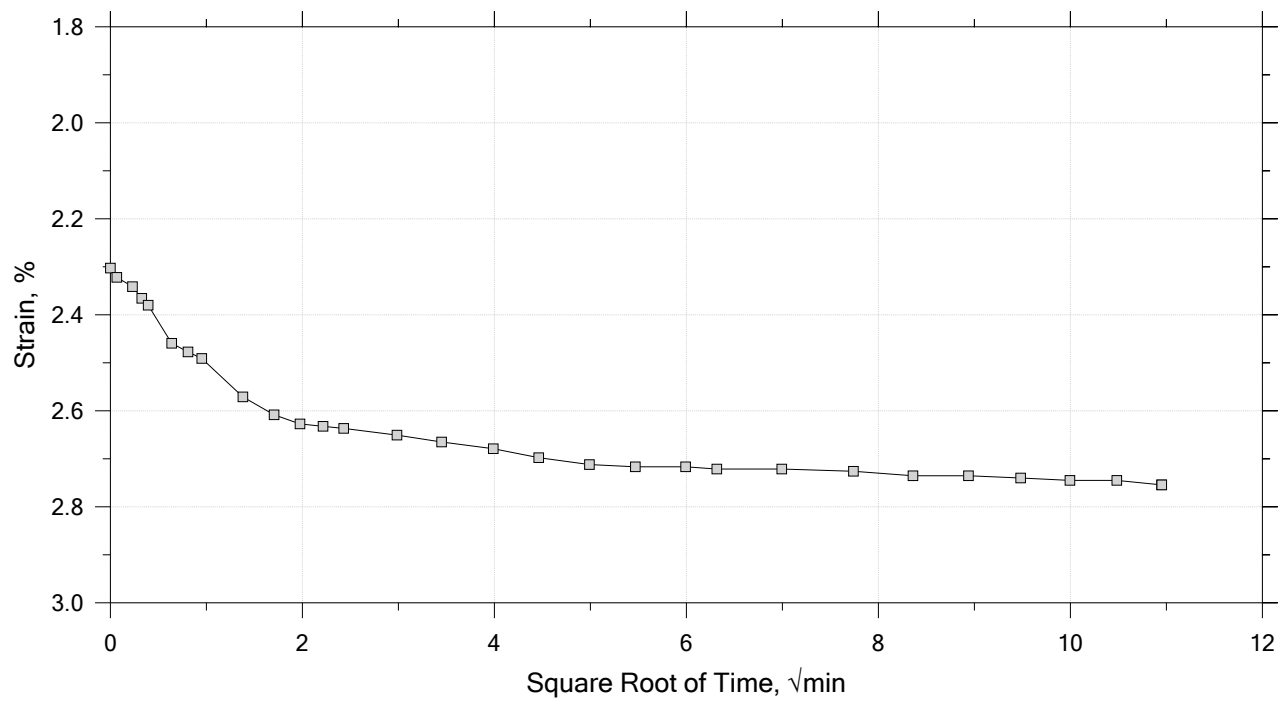
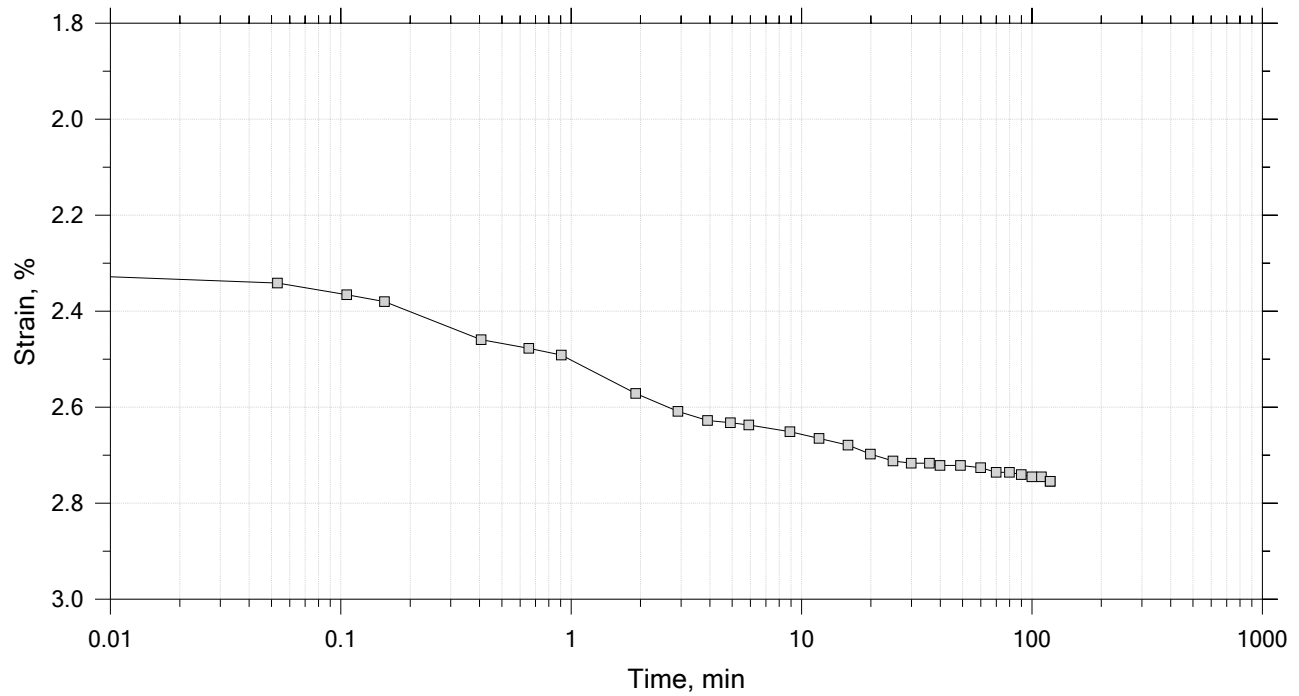
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



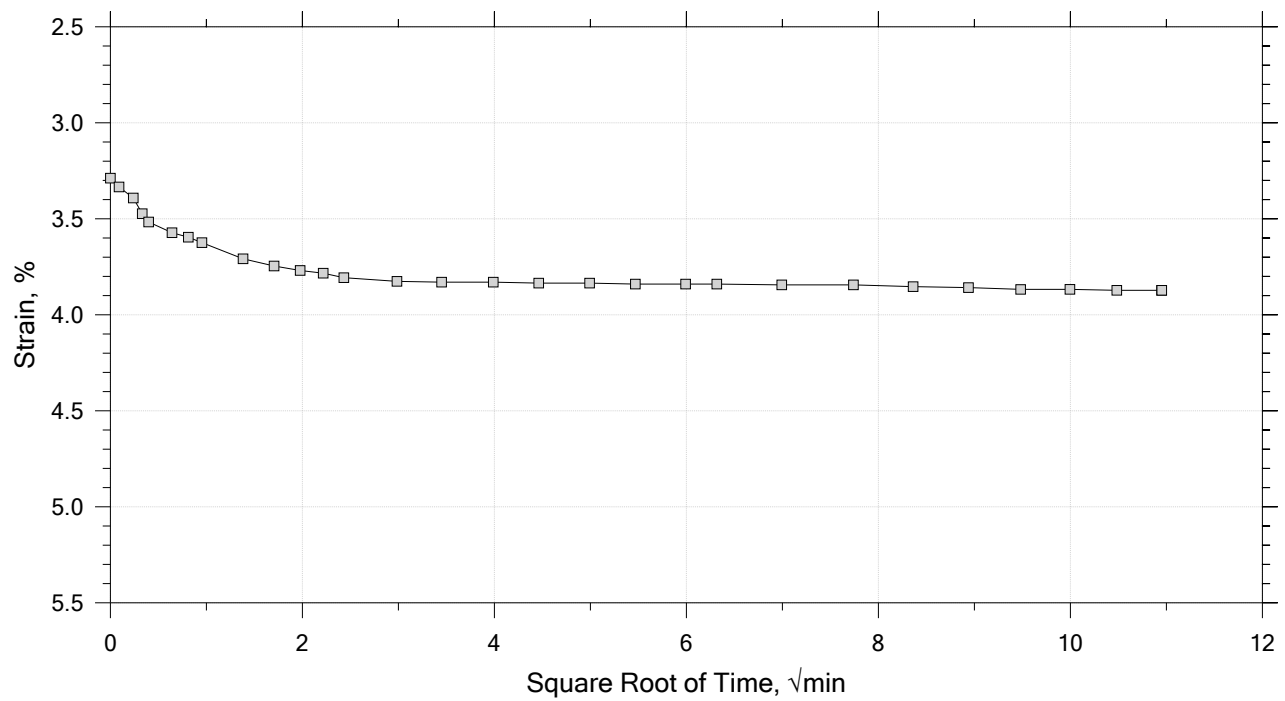
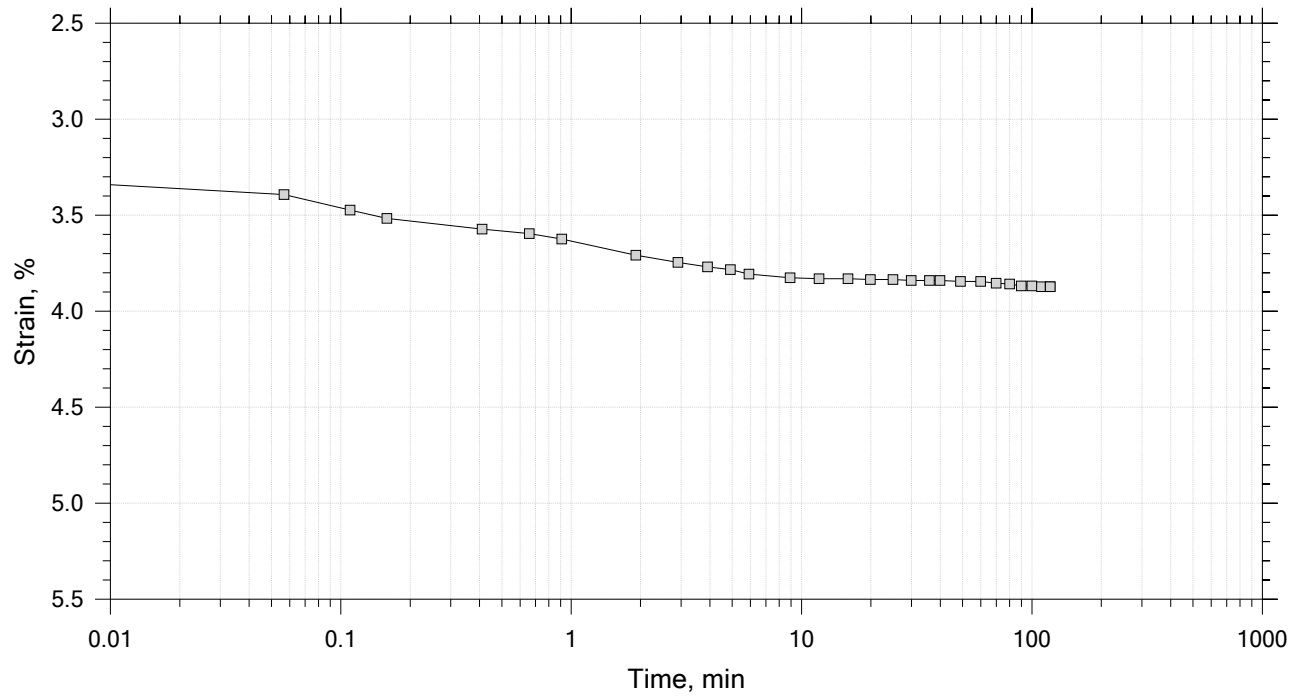
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



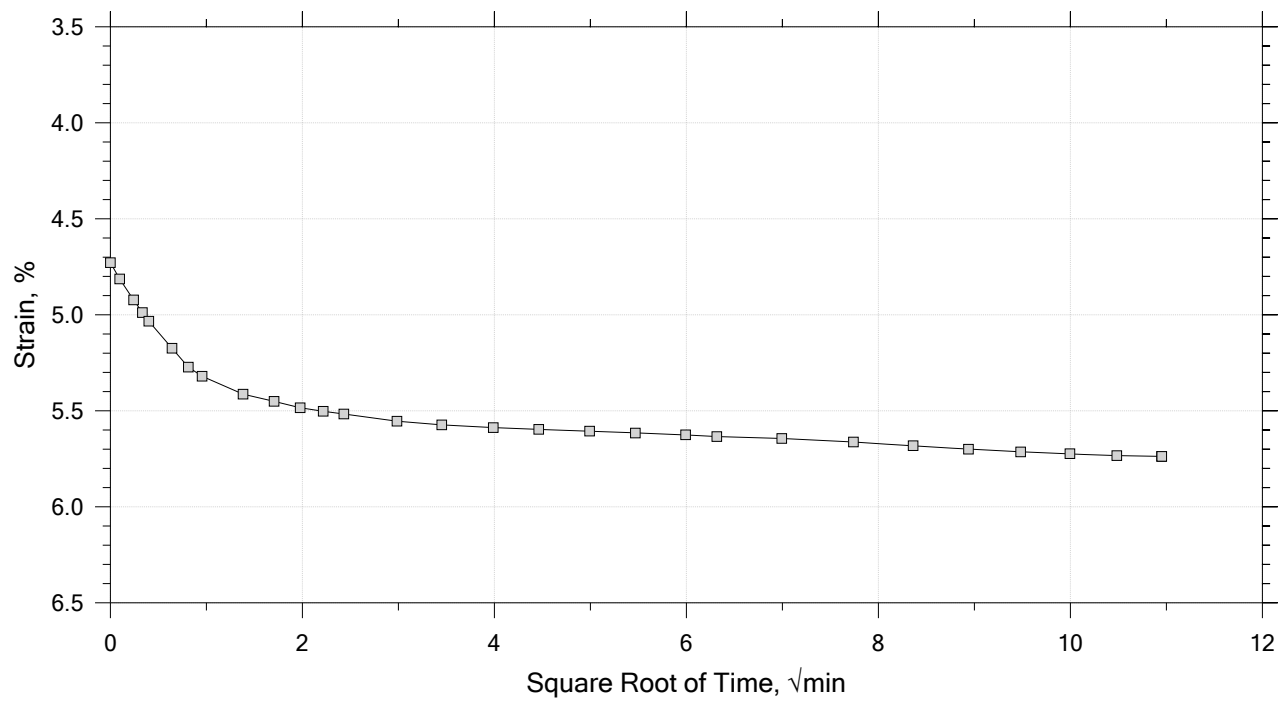
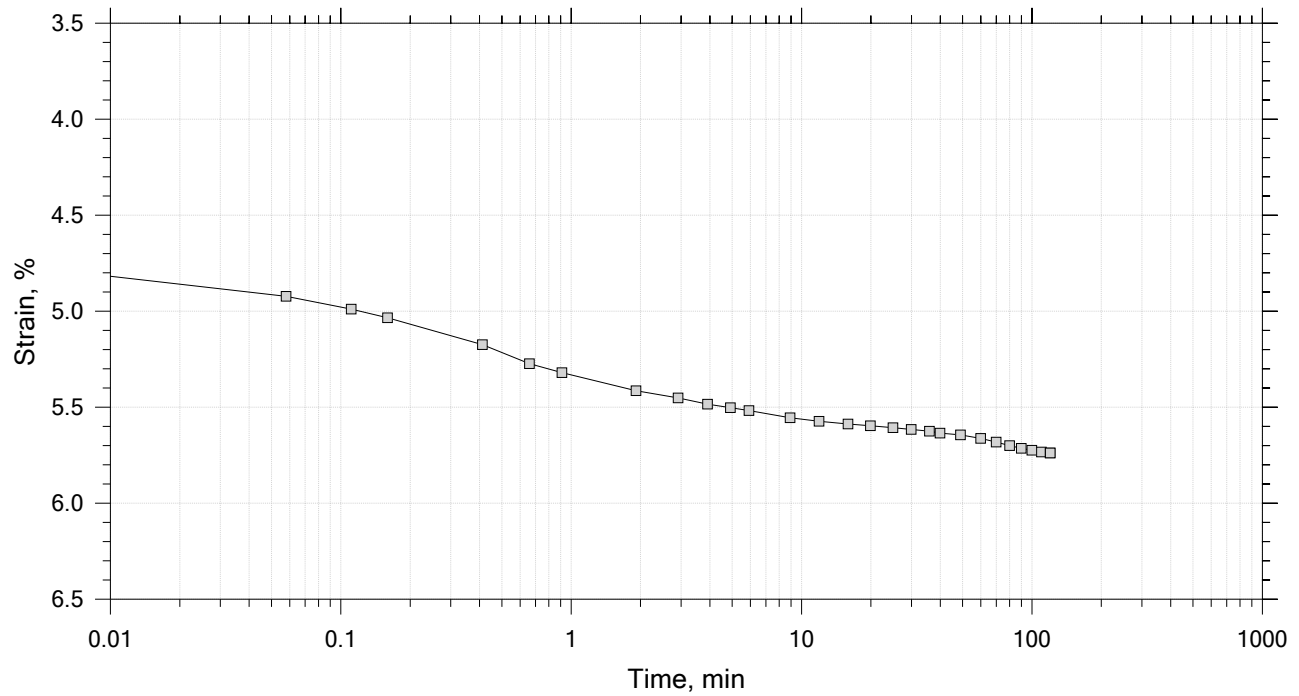
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



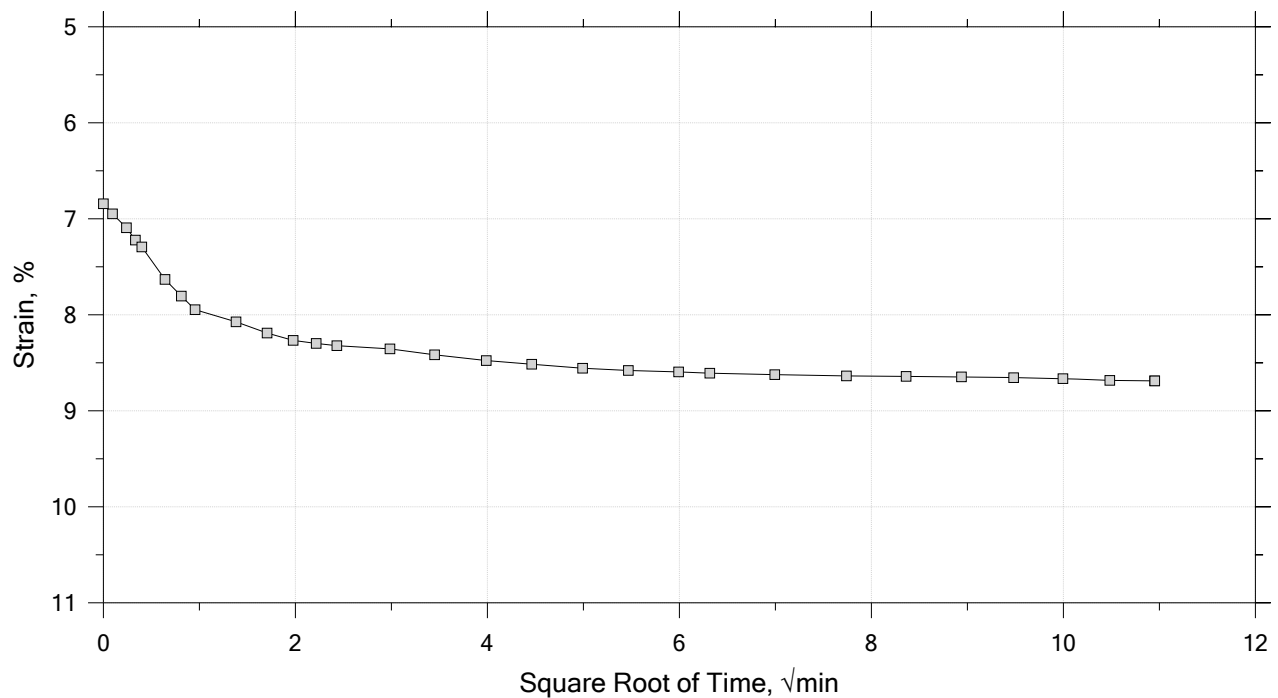
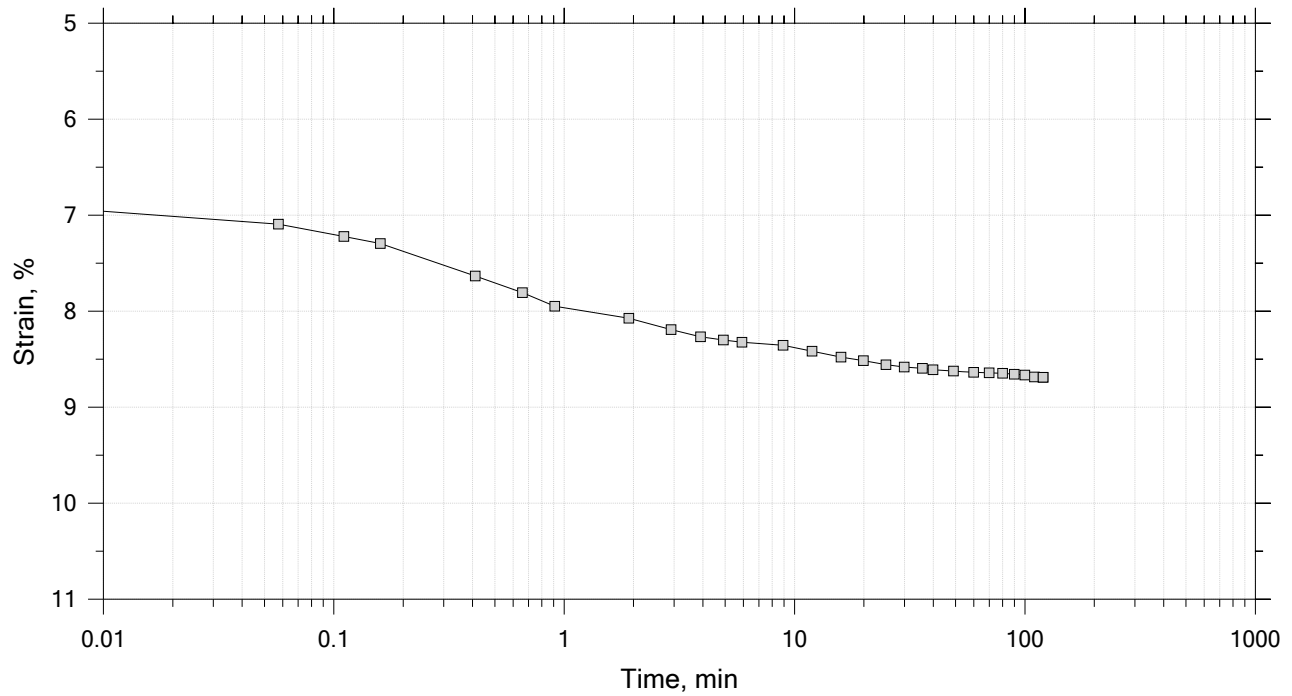
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	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



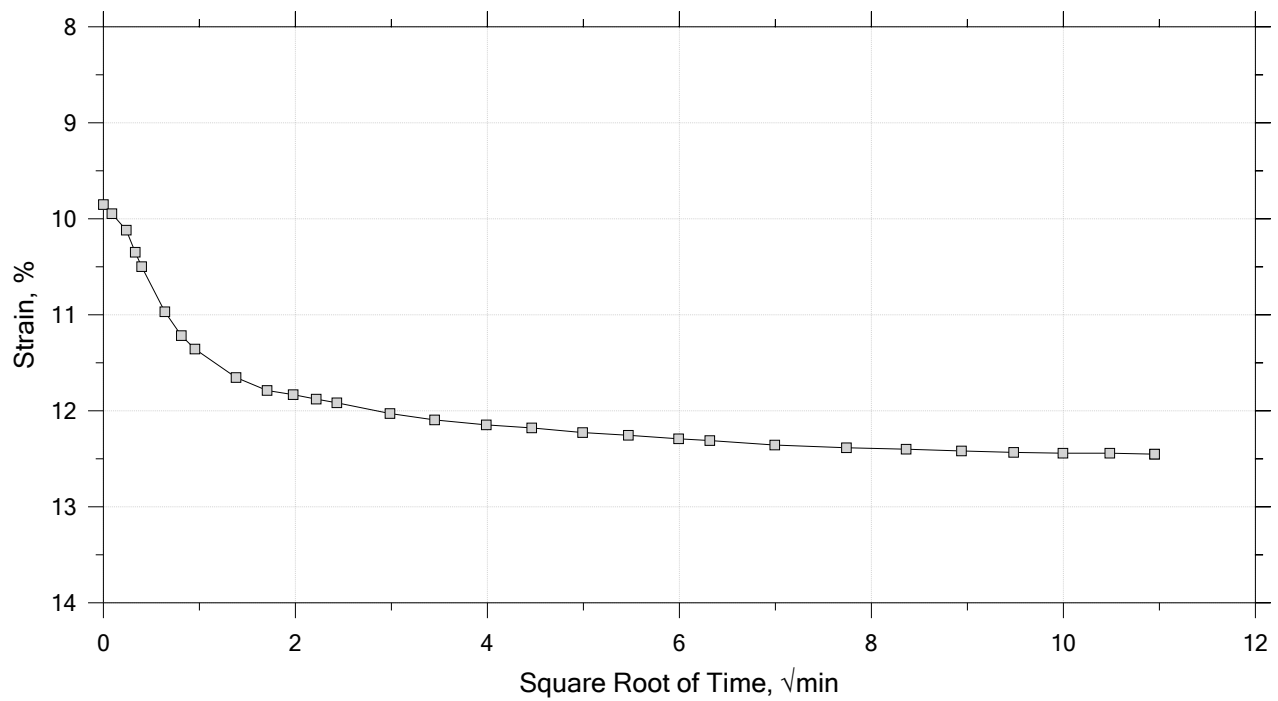
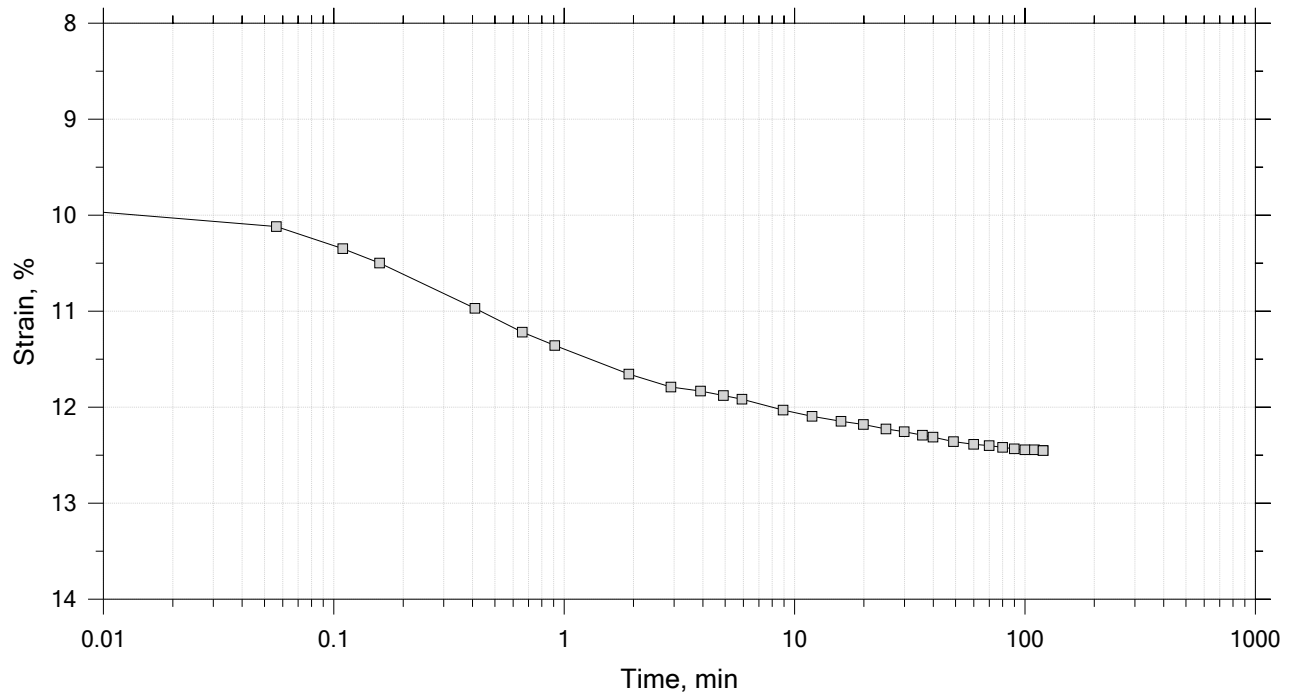
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



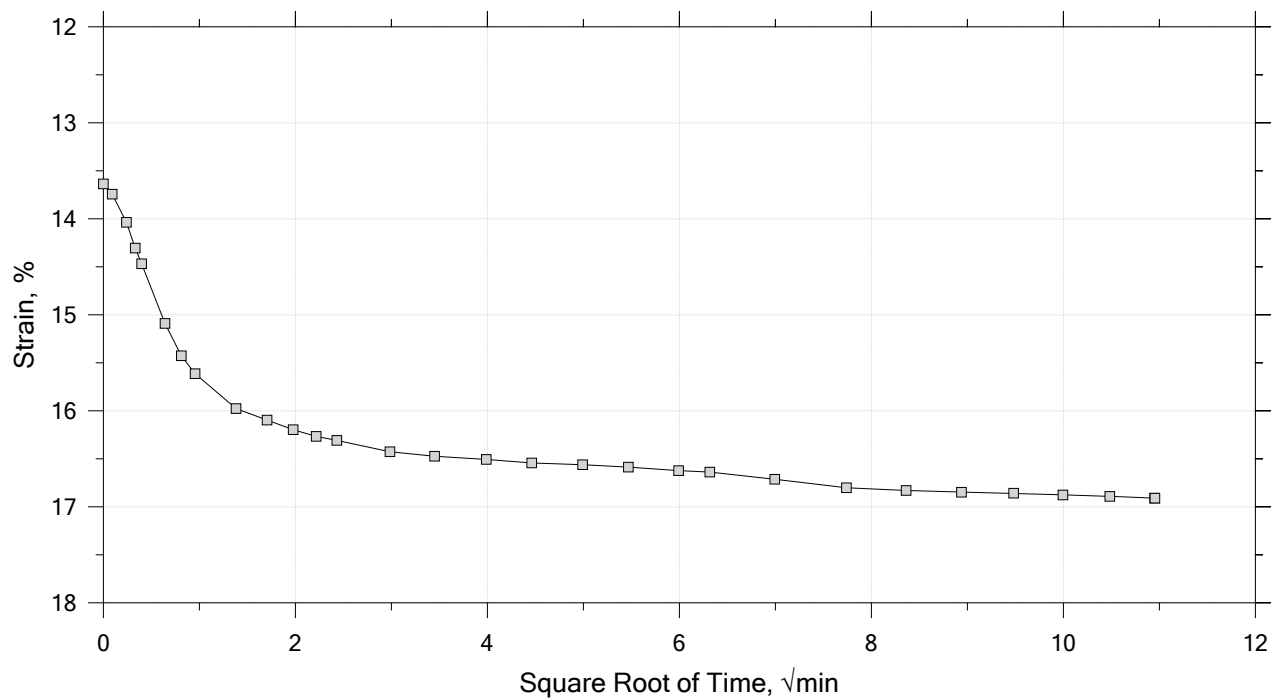
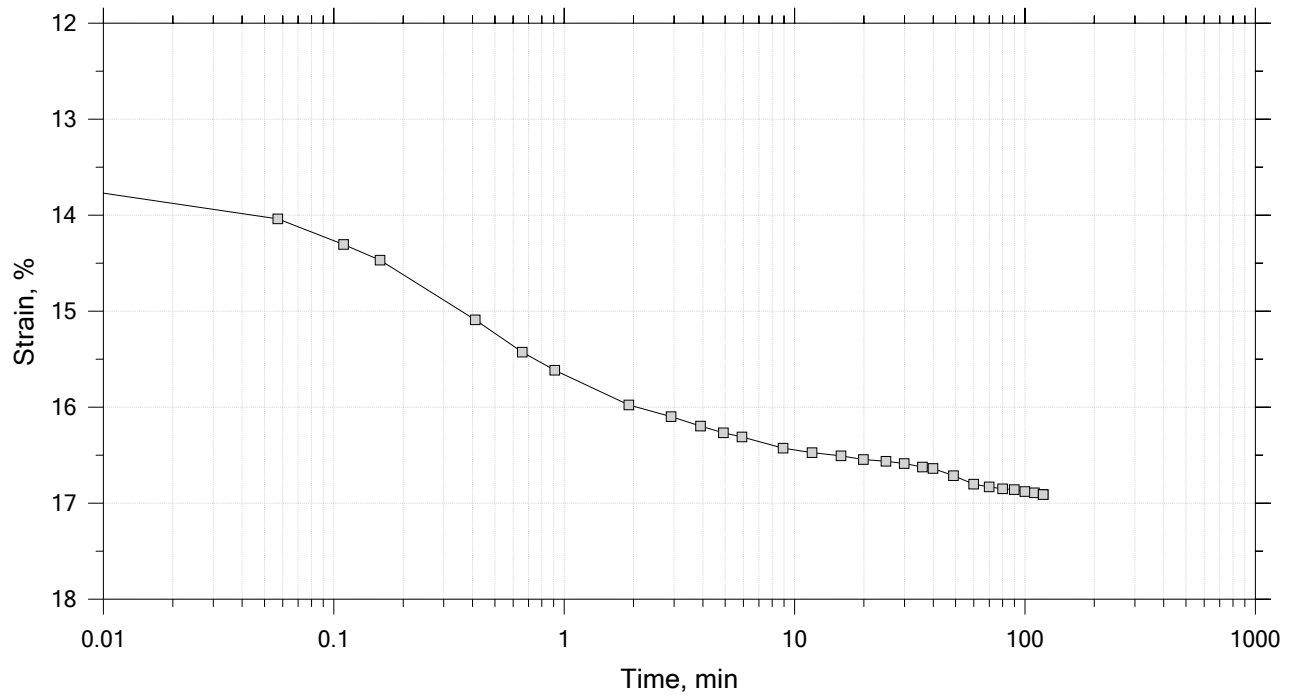
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



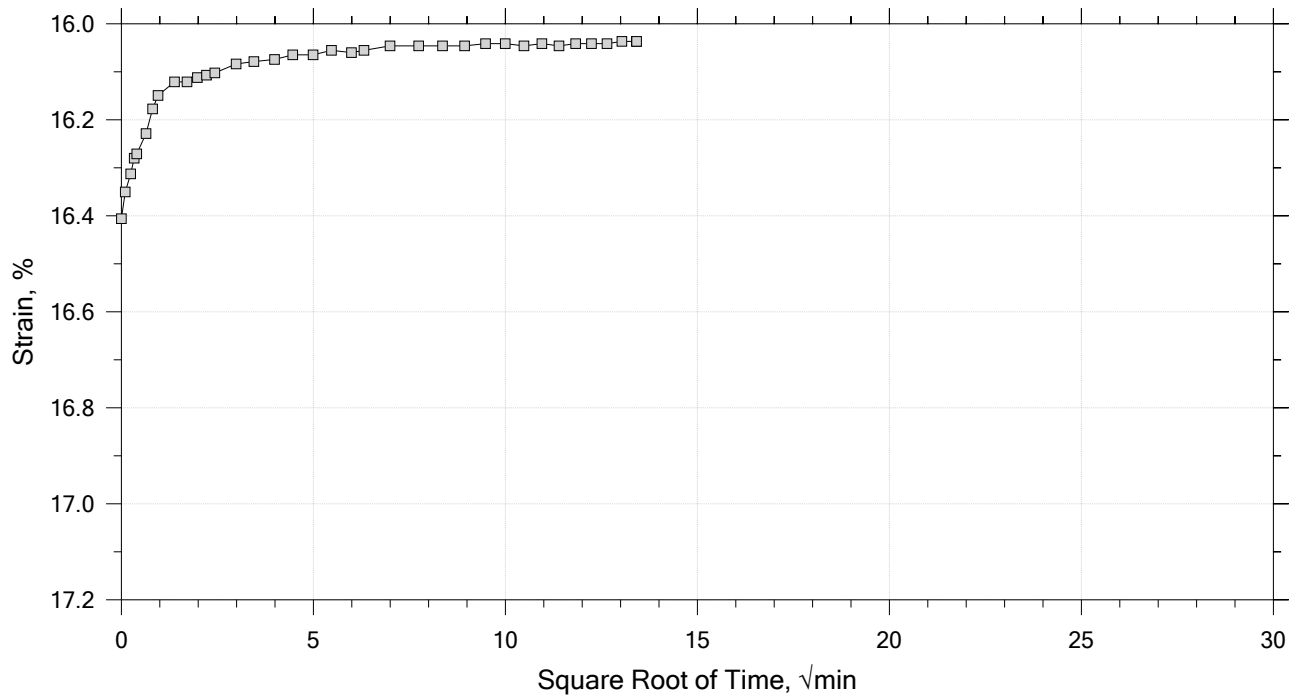
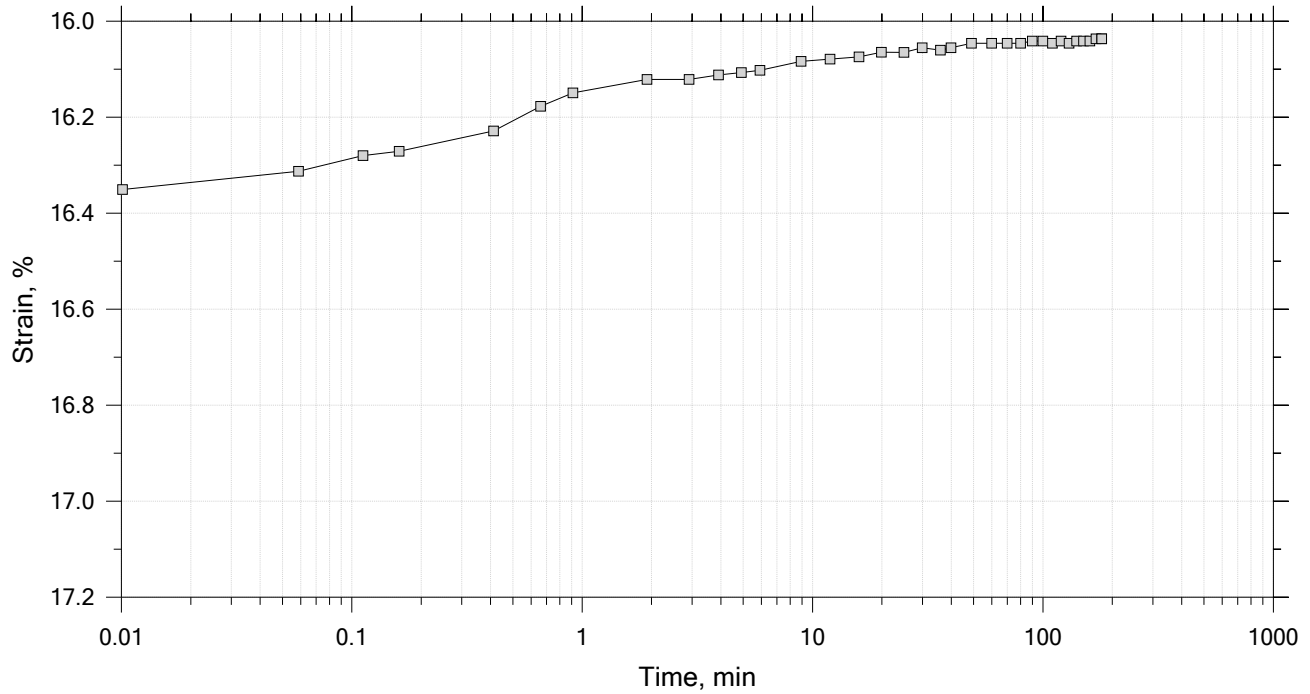
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



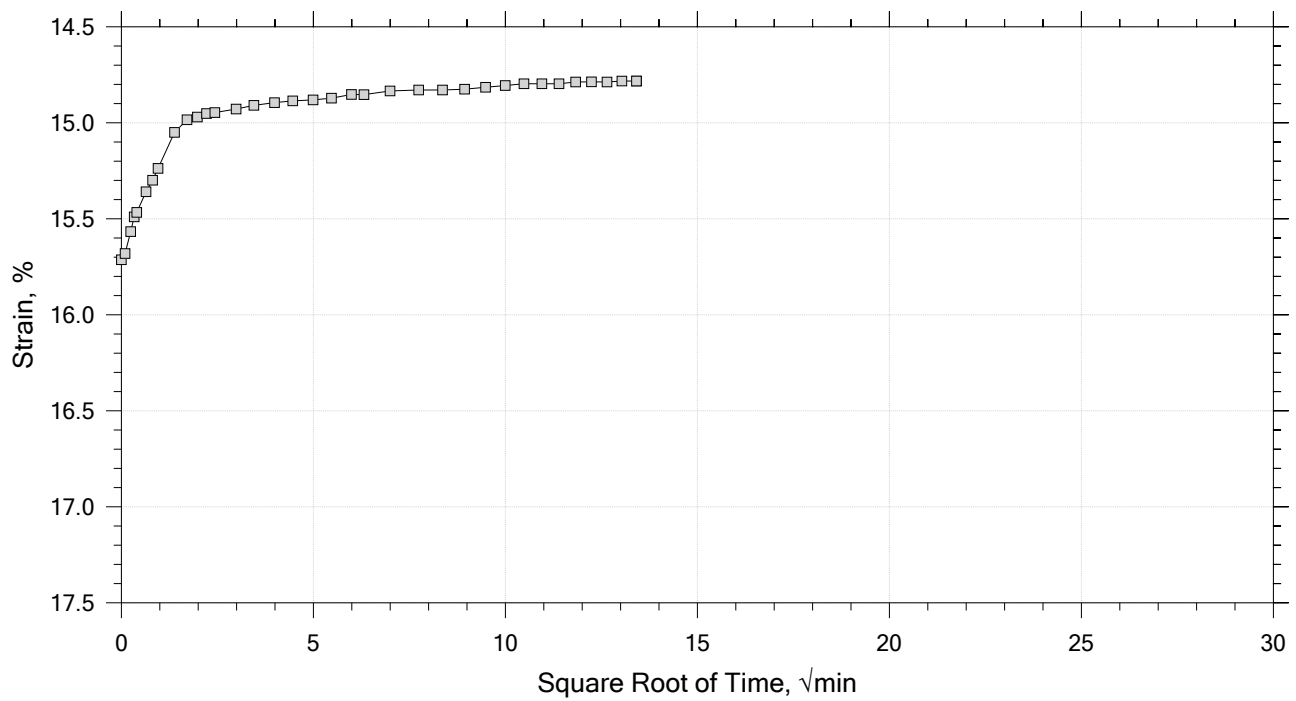
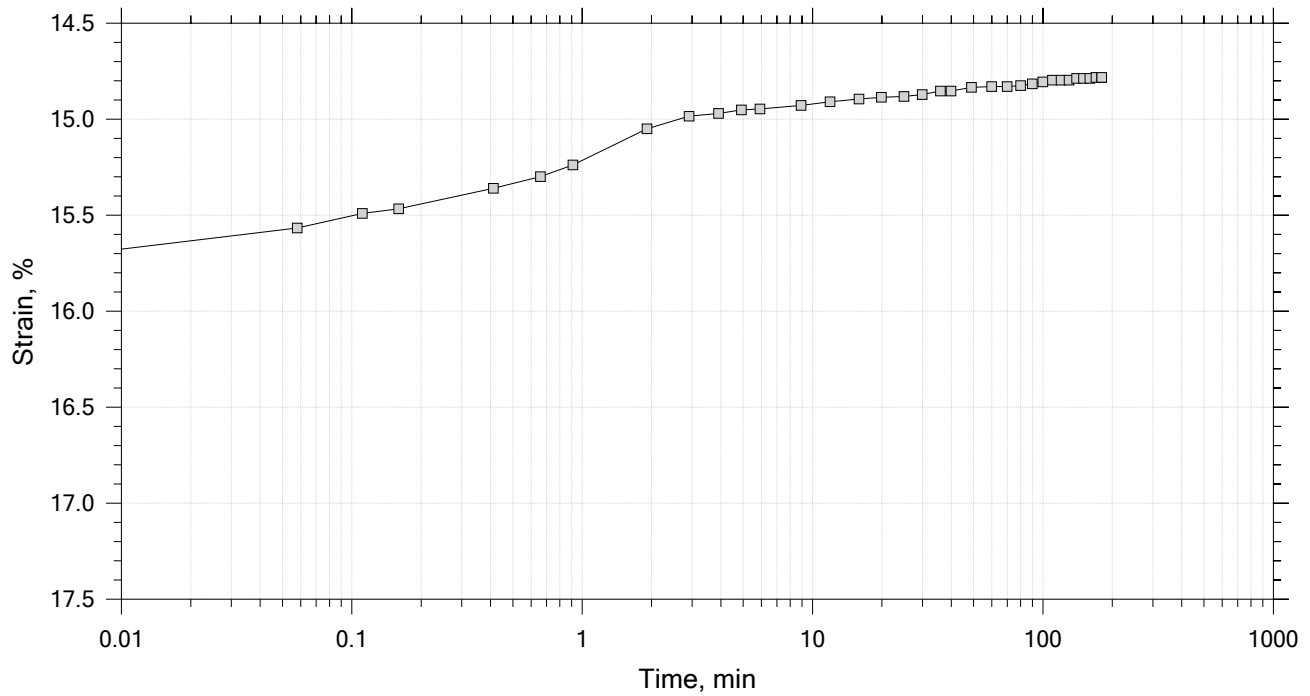
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



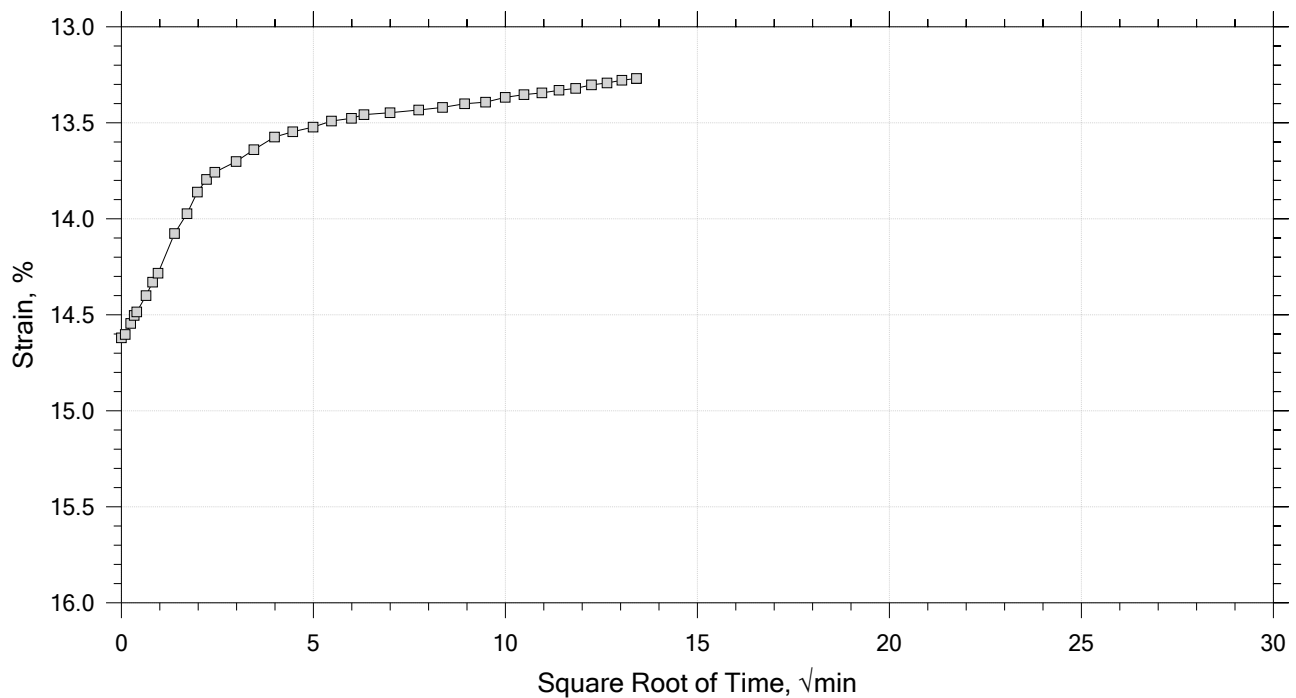
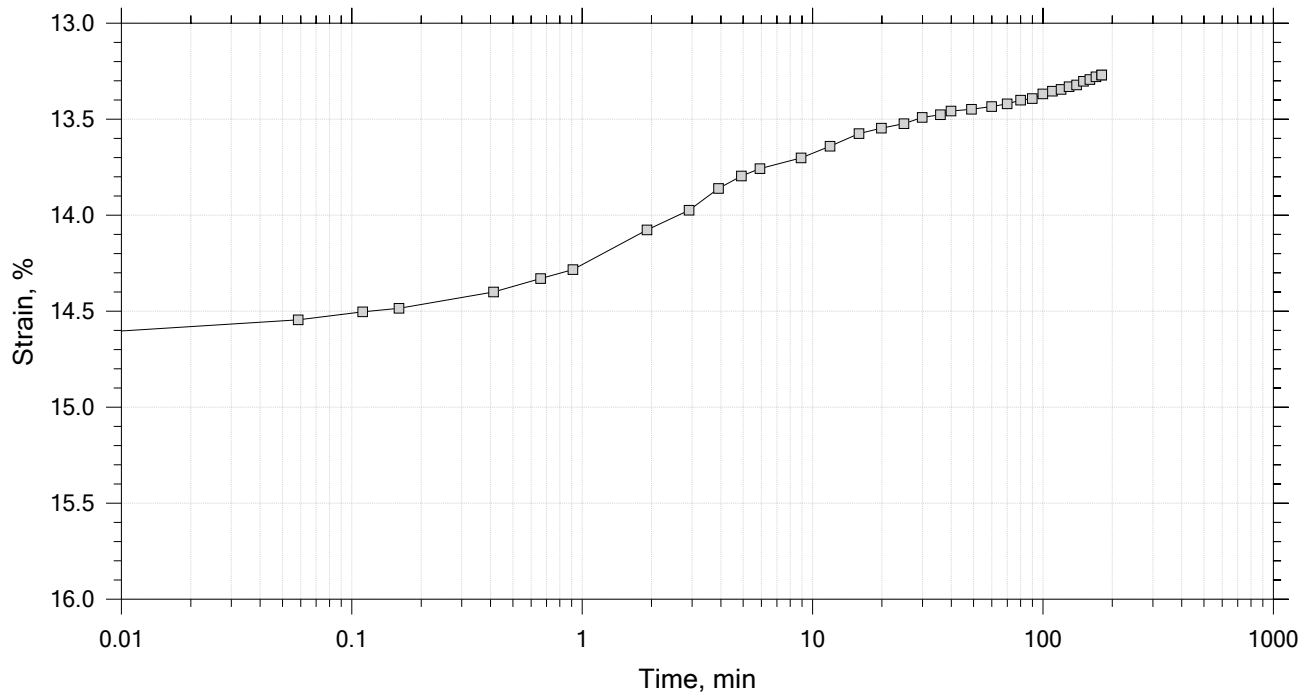
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



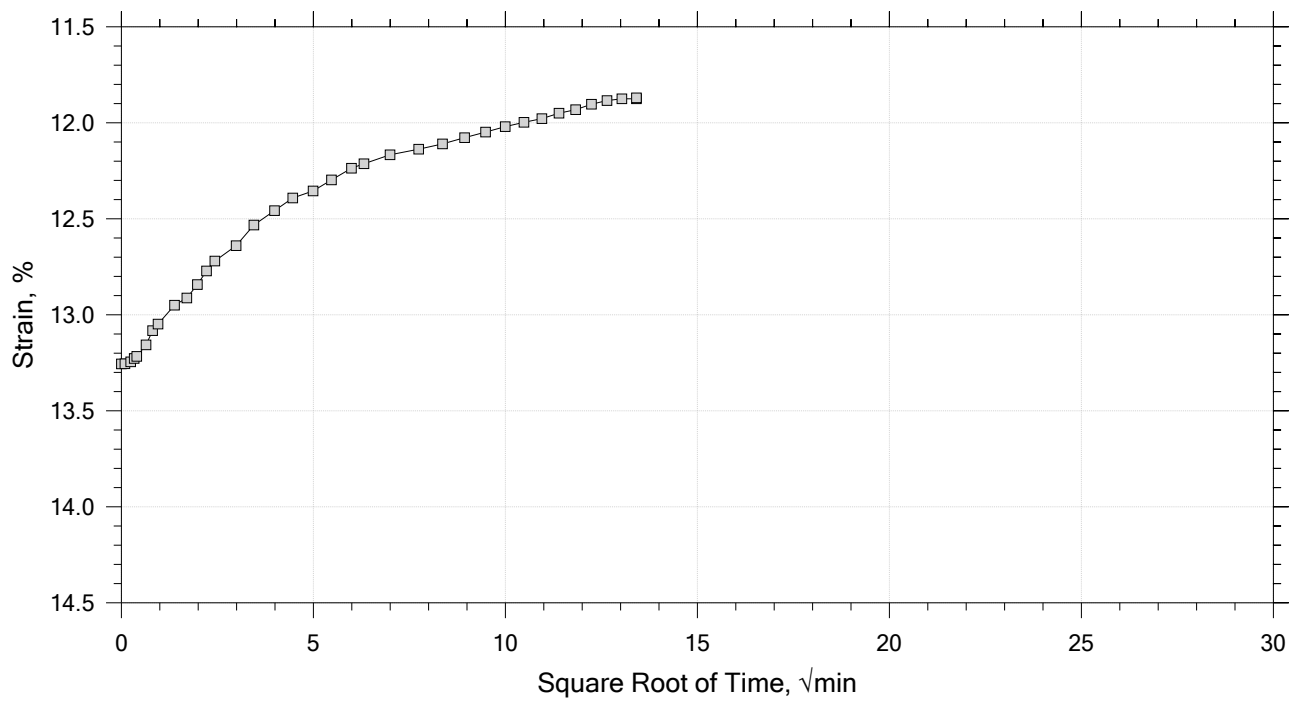
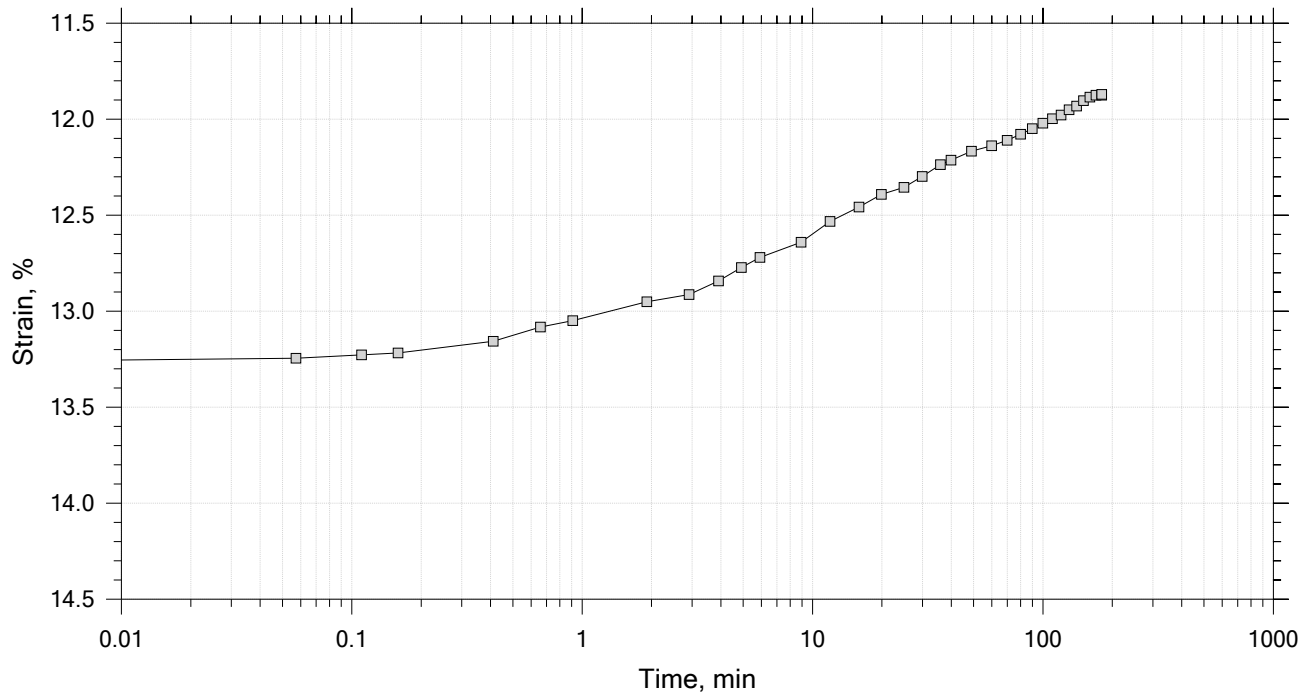
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	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



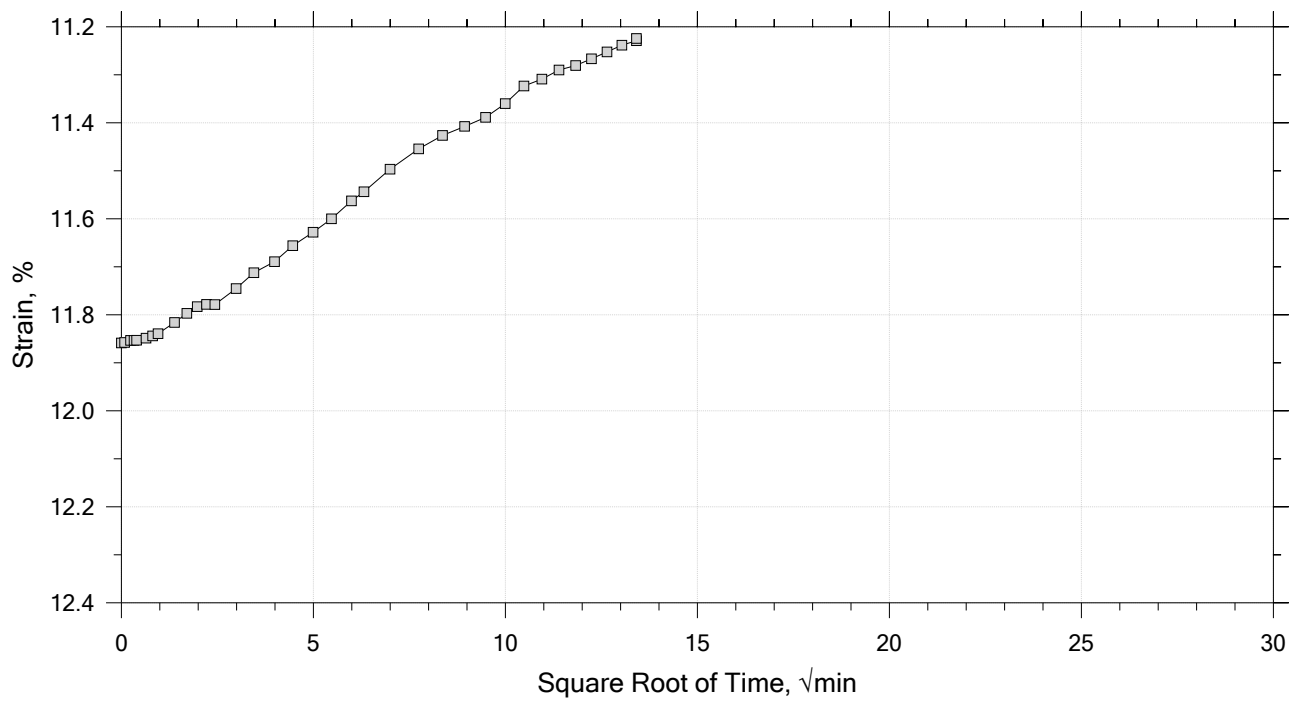
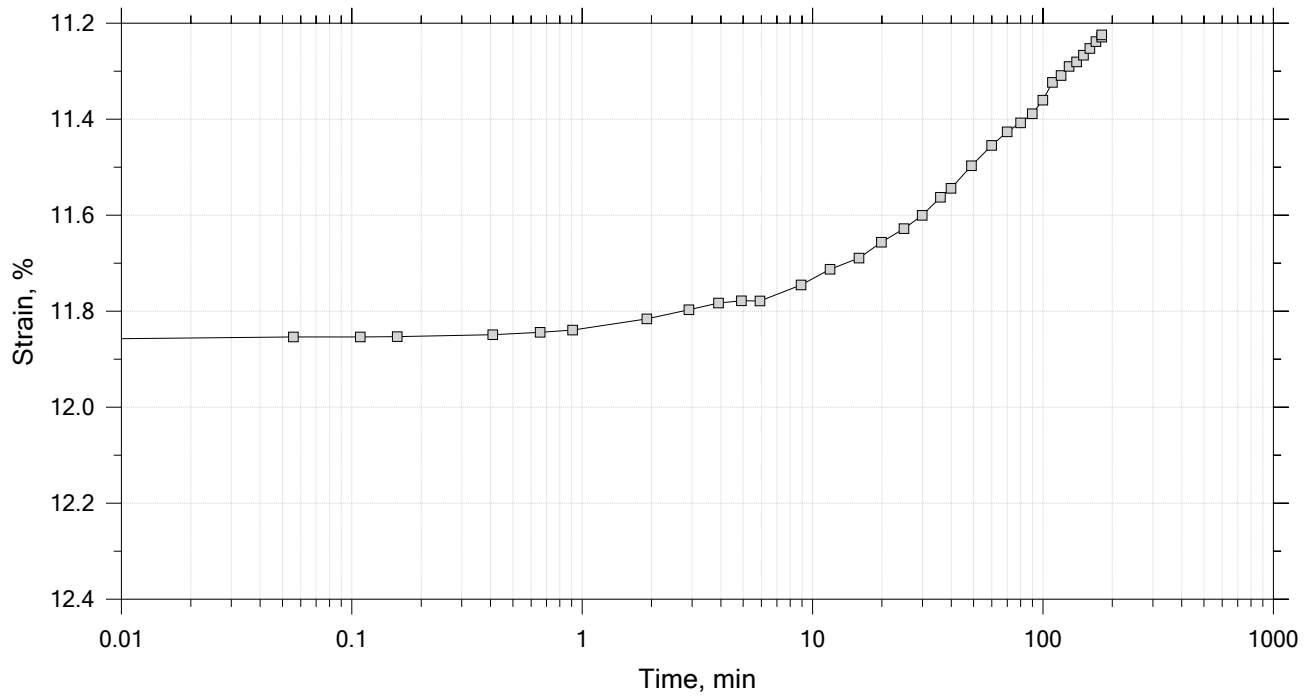
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 45
Initial Height: 1.00 in	Initial Void Ratio: 0.998	Plastic Limit: 24
Final Height: 0.92 in	Final Void Ratio: 0.838	Plasticity Index: 21

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	C-1725	RING		A1264
Mass Container, gm	8.52	108.84	108.84	8.42
Mass Container + Wet Soil, gm	142.75	258.5	253.17	152.55
Mass Container + Dry Soil, gm	107.85	219.43	219.43	118.86
Mass Dry Soil, gm	99.33	110.59	110.59	110.44
Water Content, %	35.14	35.32	30.51	30.51
Void Ratio	---	1.00	0.84	---
Degree of Saturation, %	---	97.25	100.00	---
Dry Unit Weight, pcf	---	85.83	93.293	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

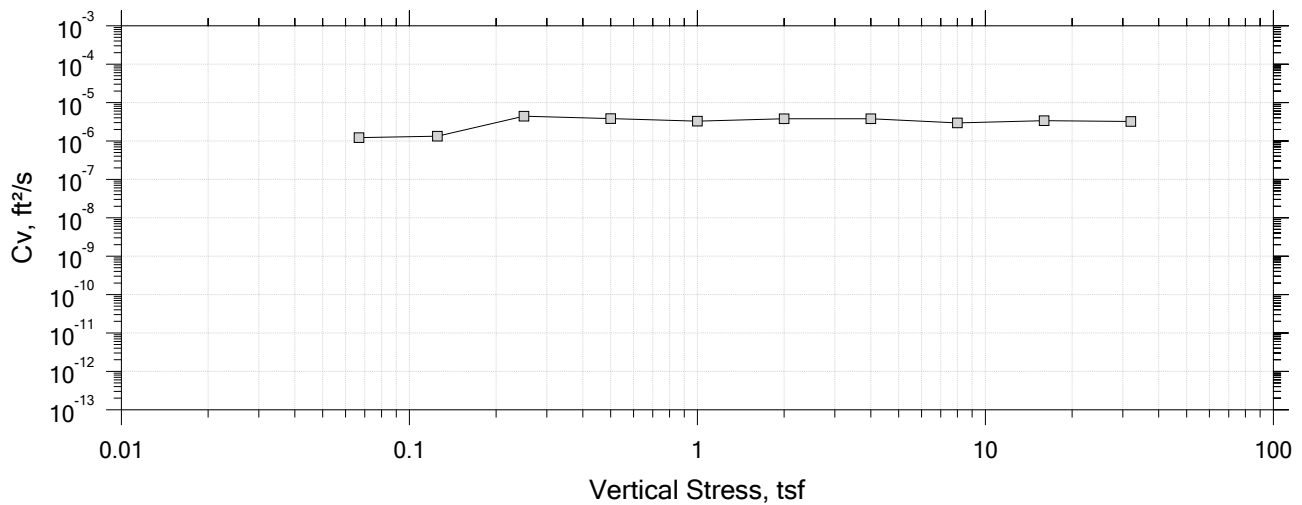
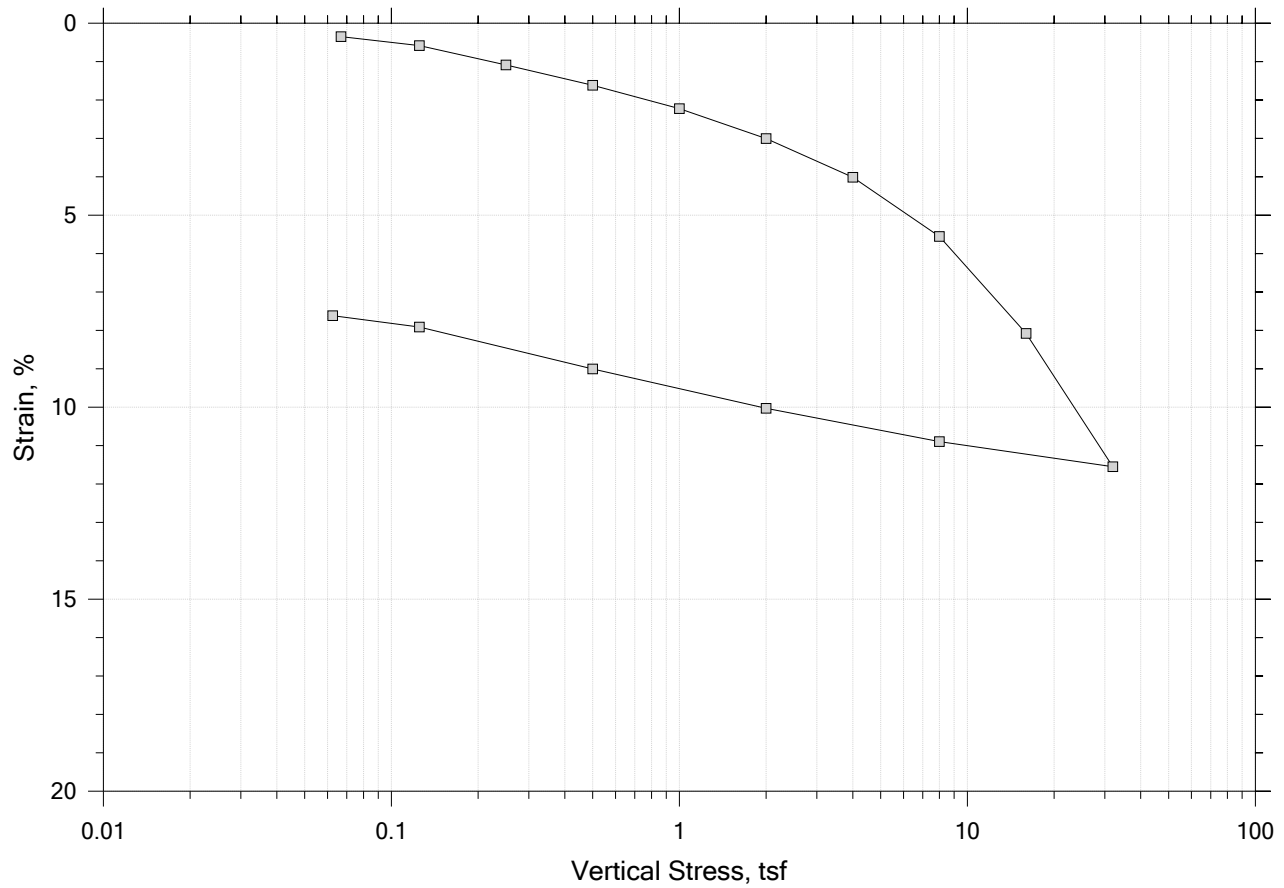
Square Root of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-101	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 09/27/18	Depth: 5-7 ft
	Test No.: IP-3	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

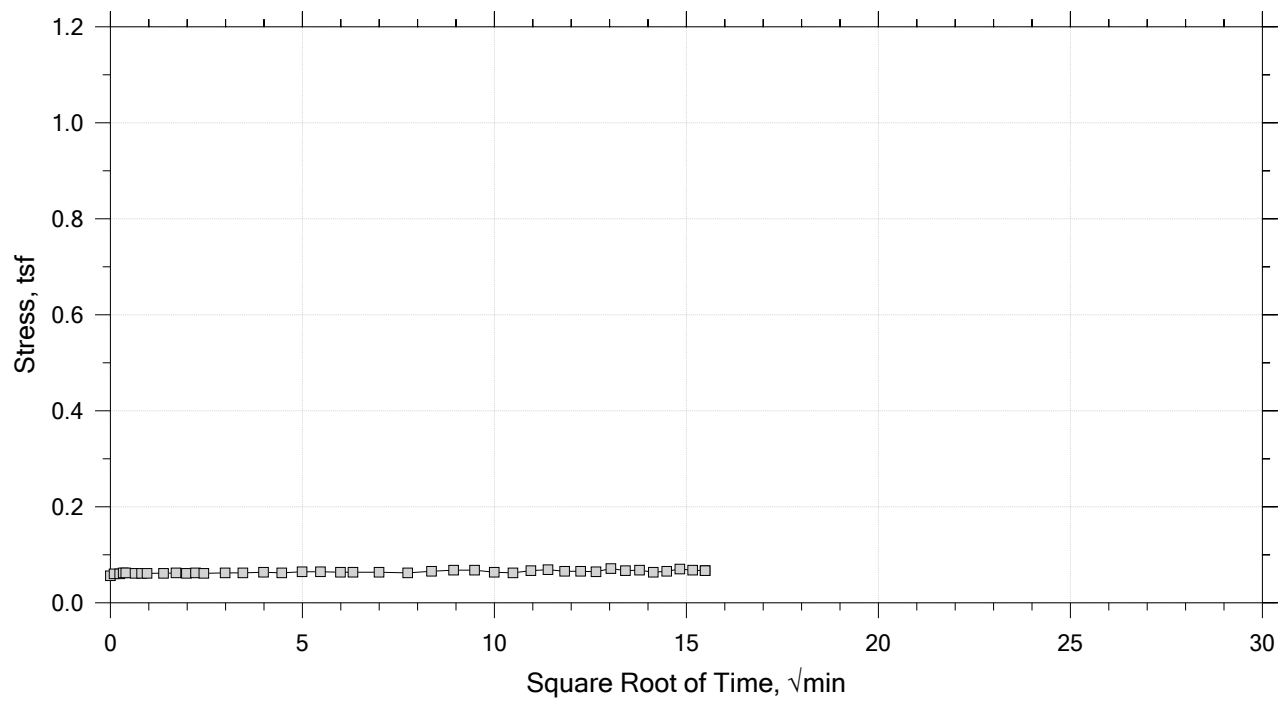
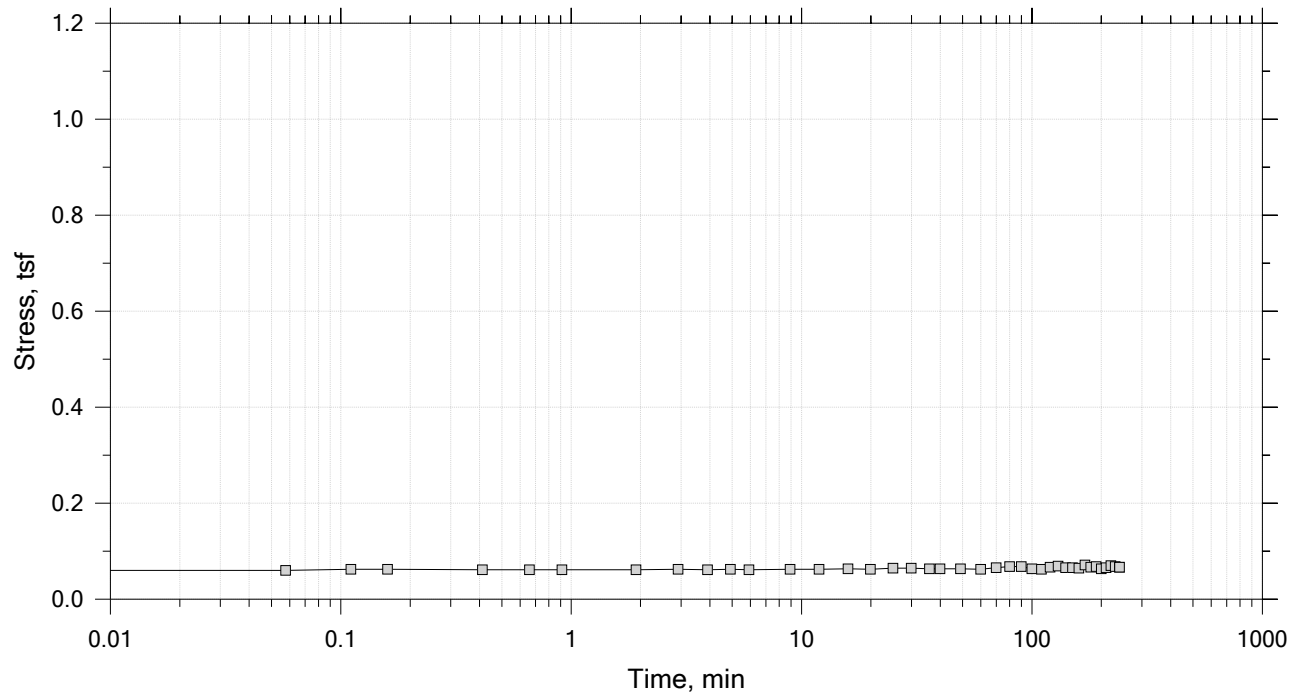
Summary Report




	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0668 tsf



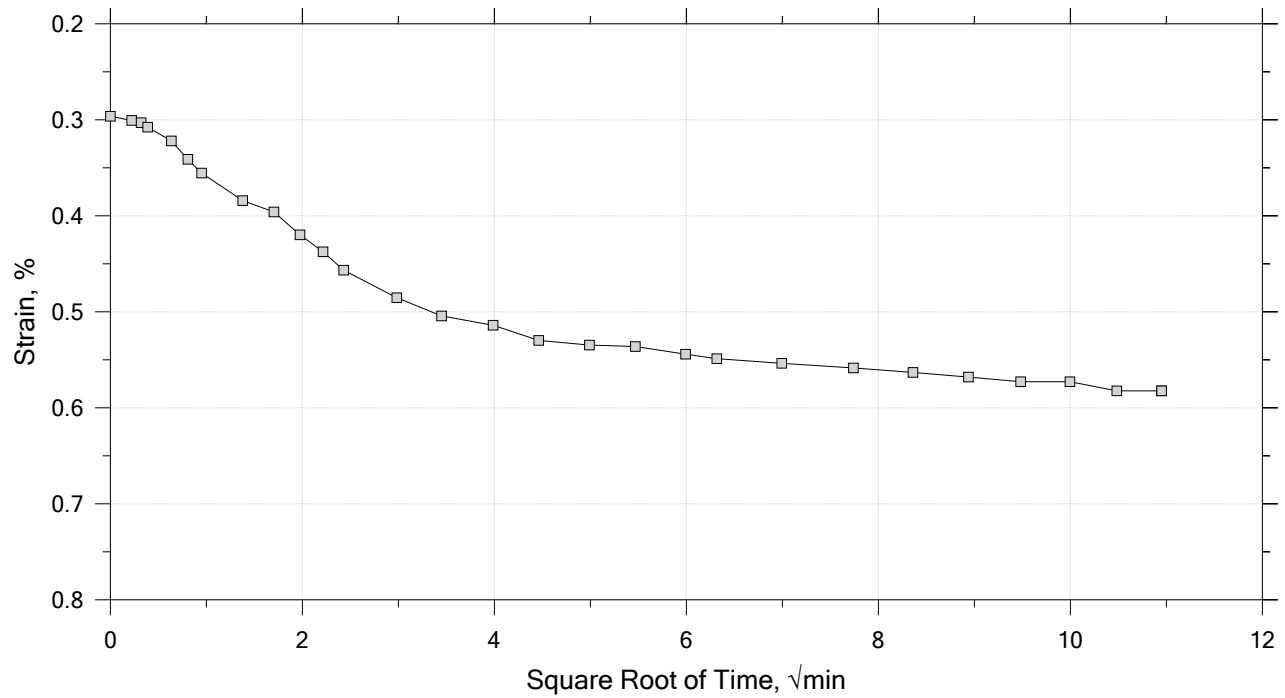
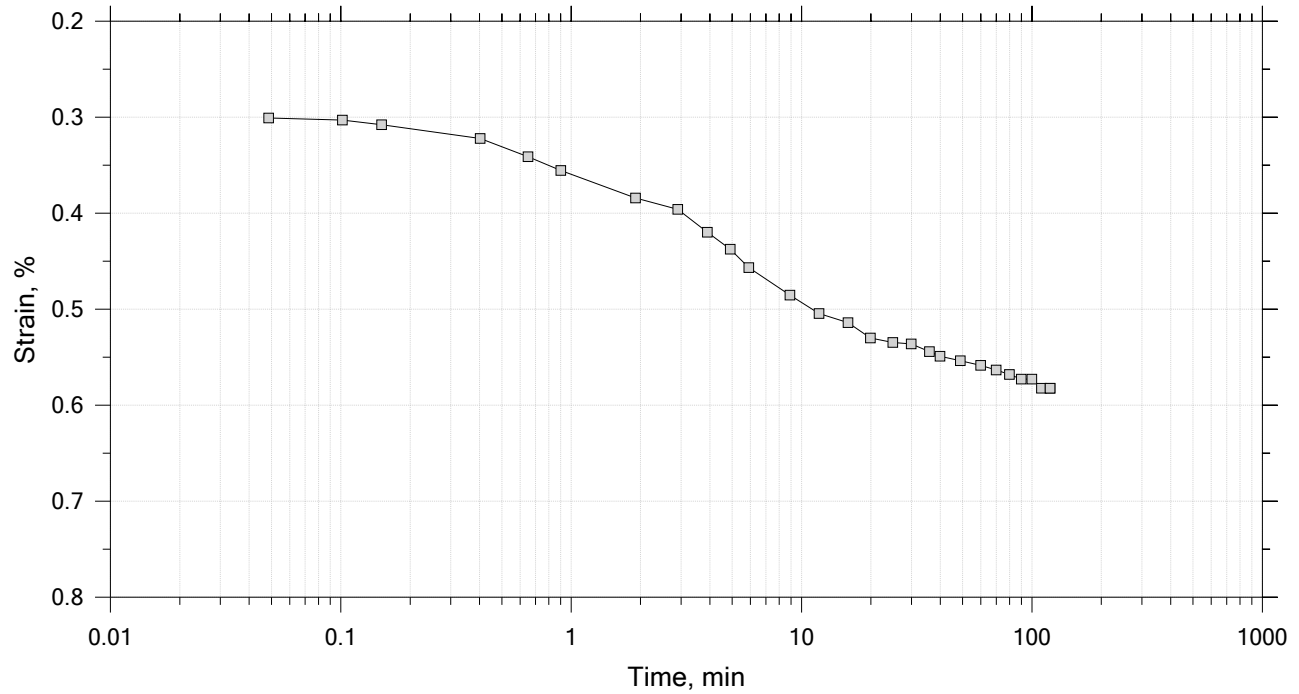
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



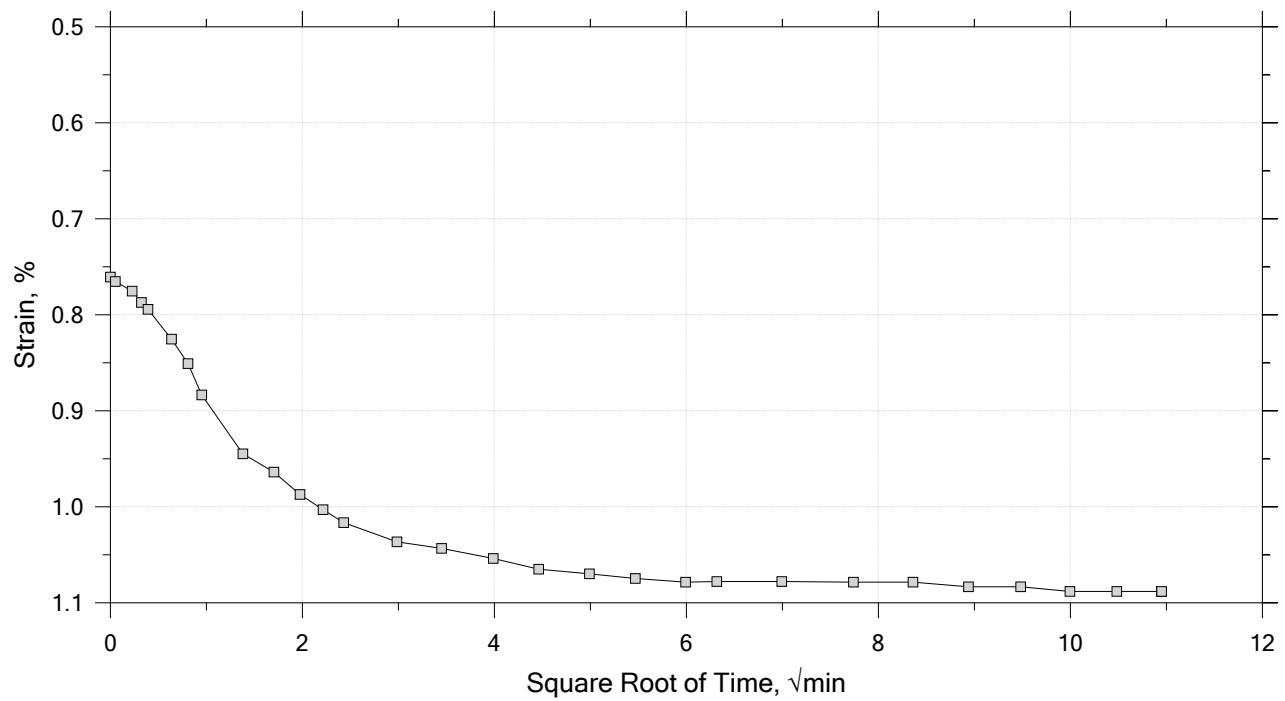
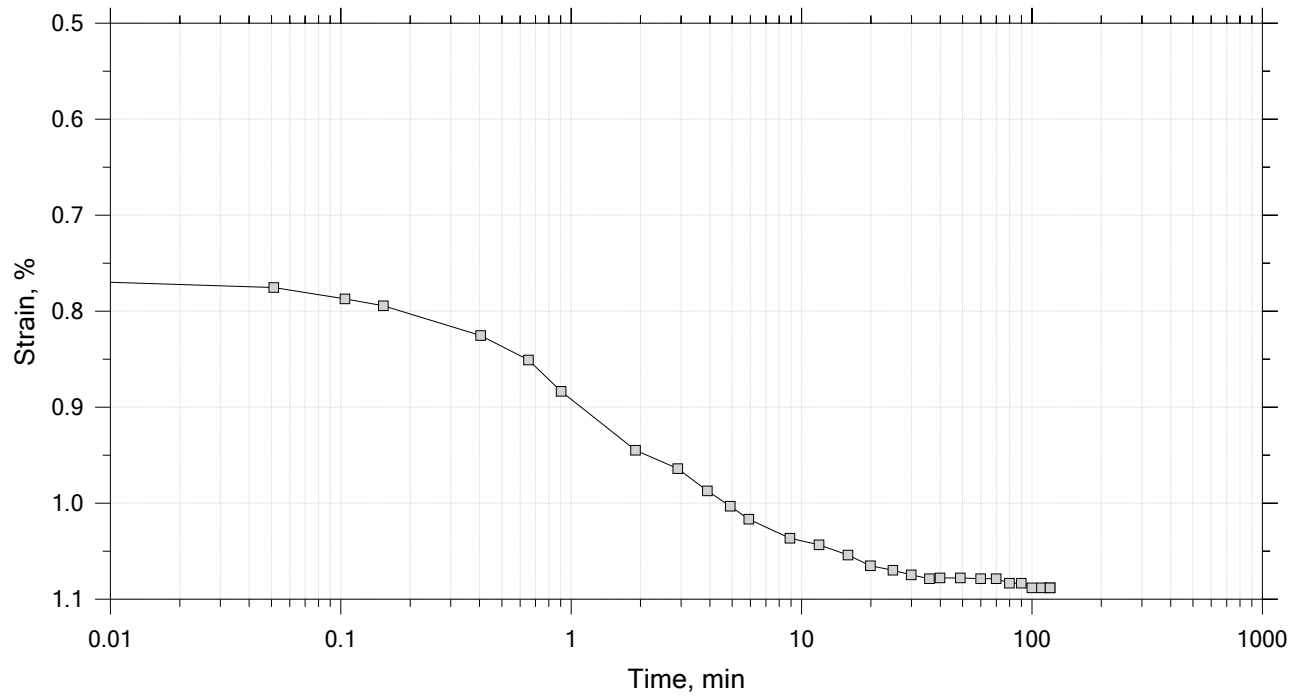
	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



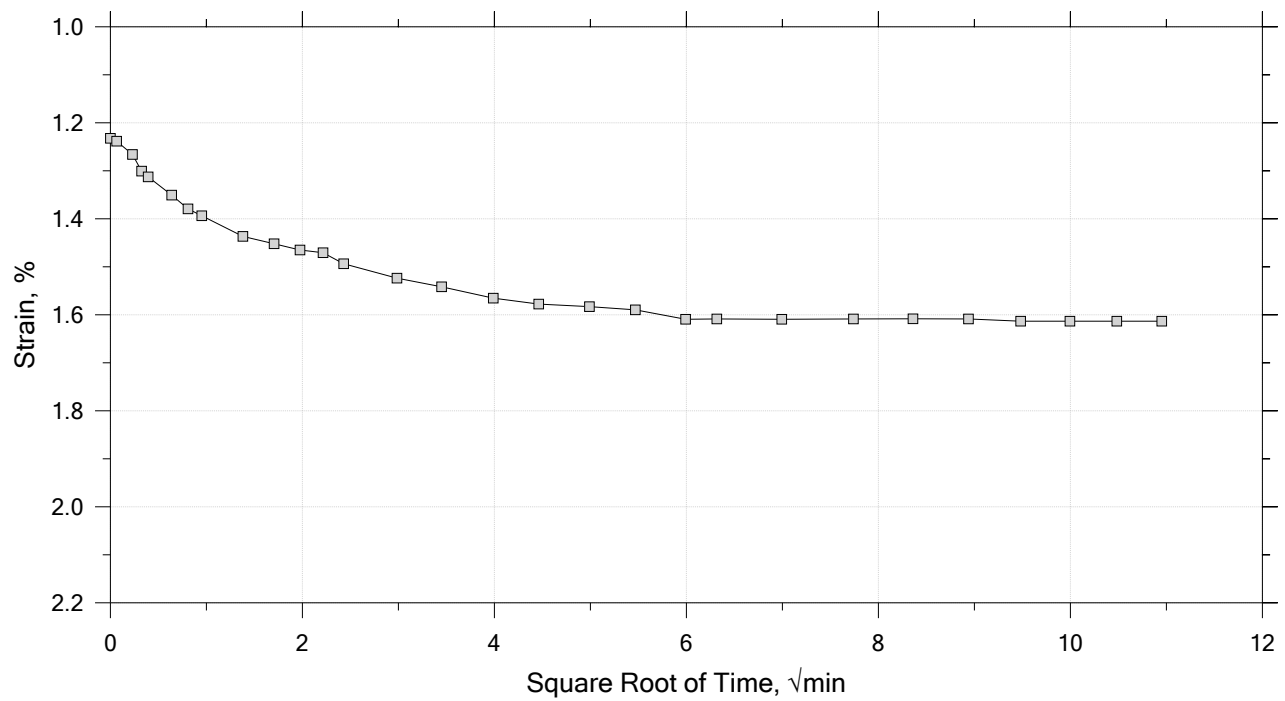
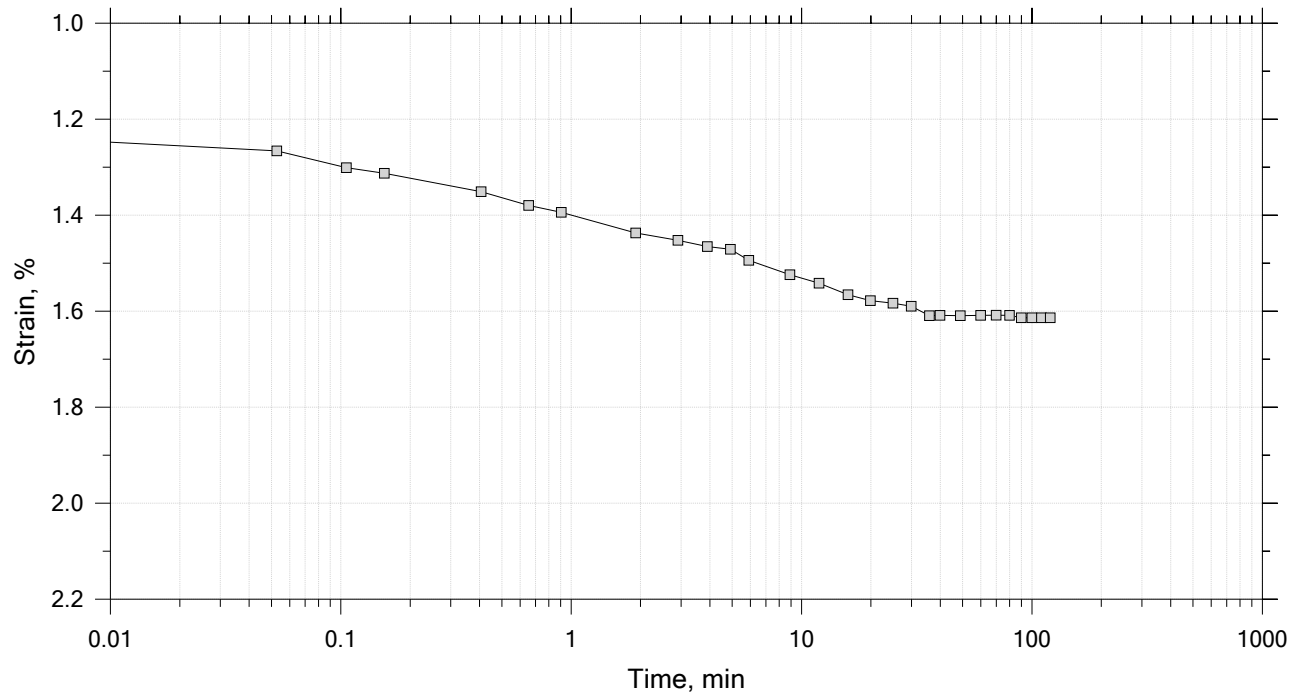
	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



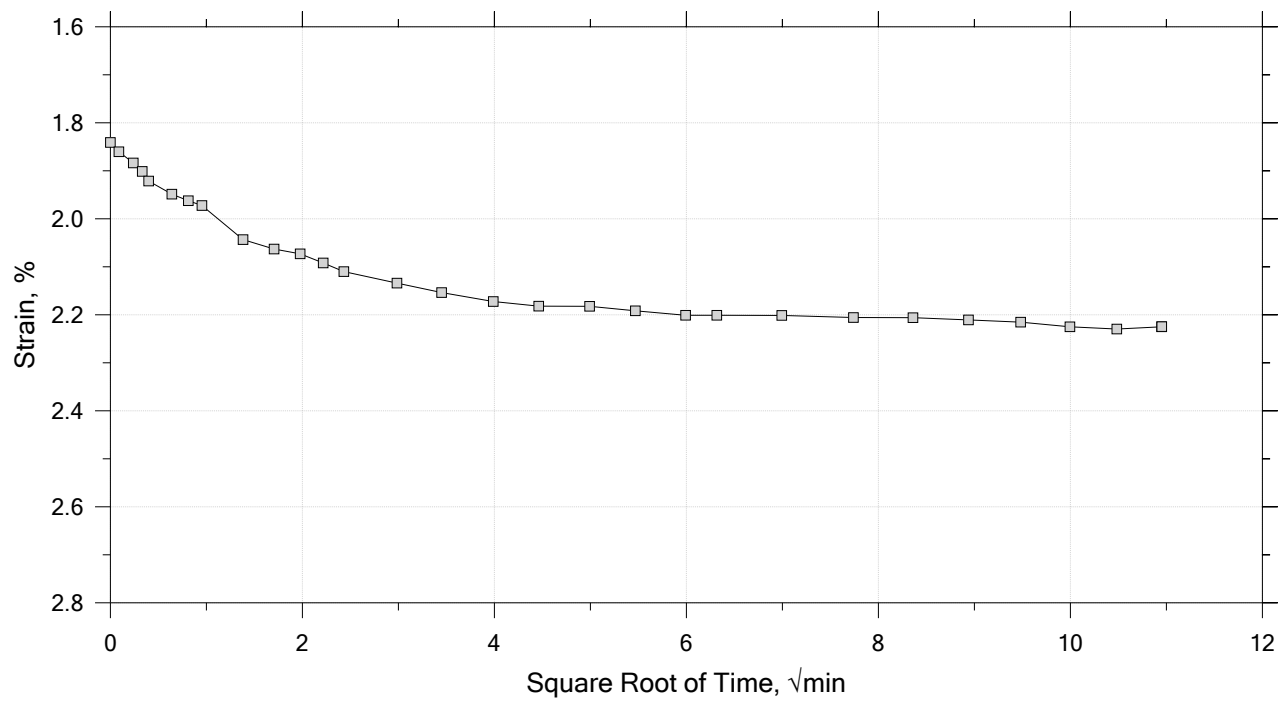
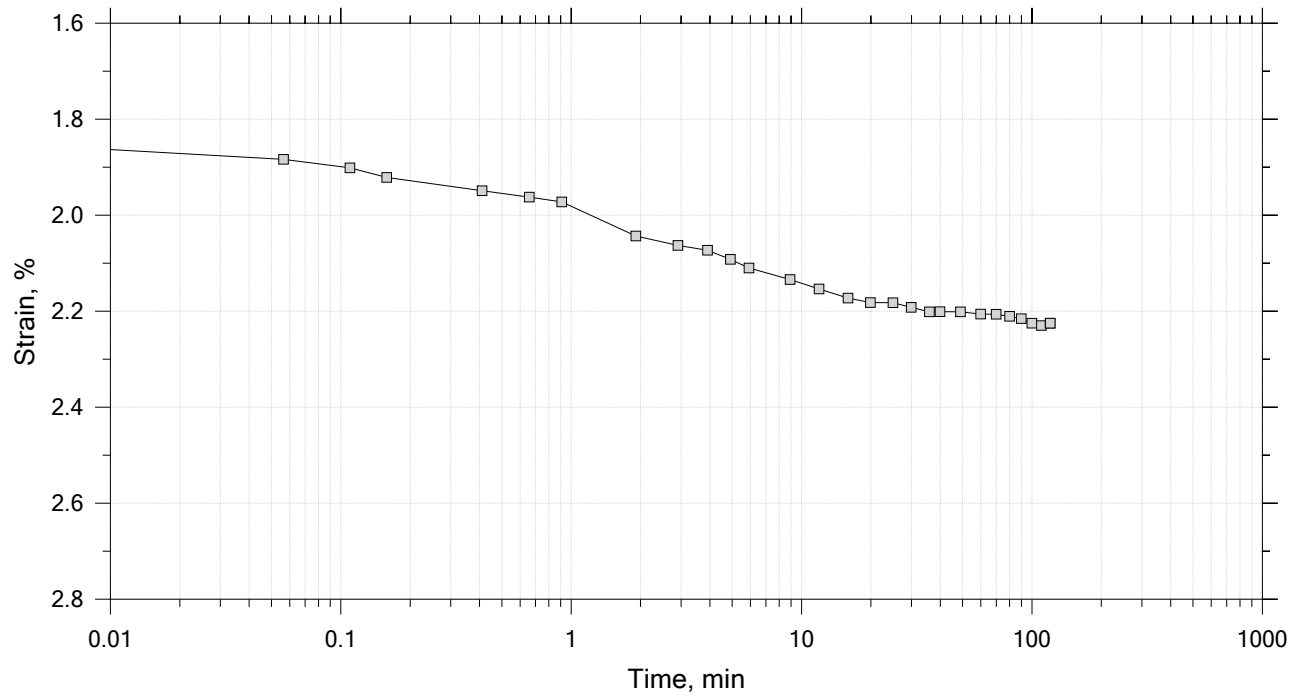
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



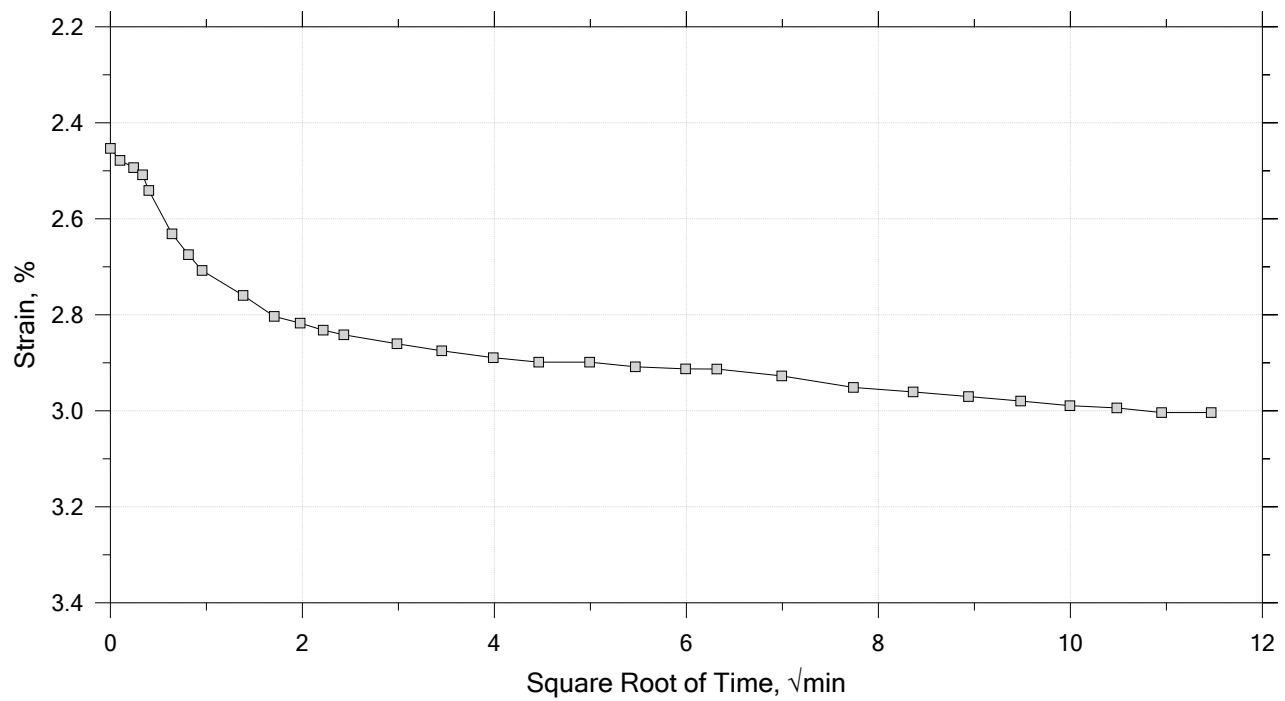
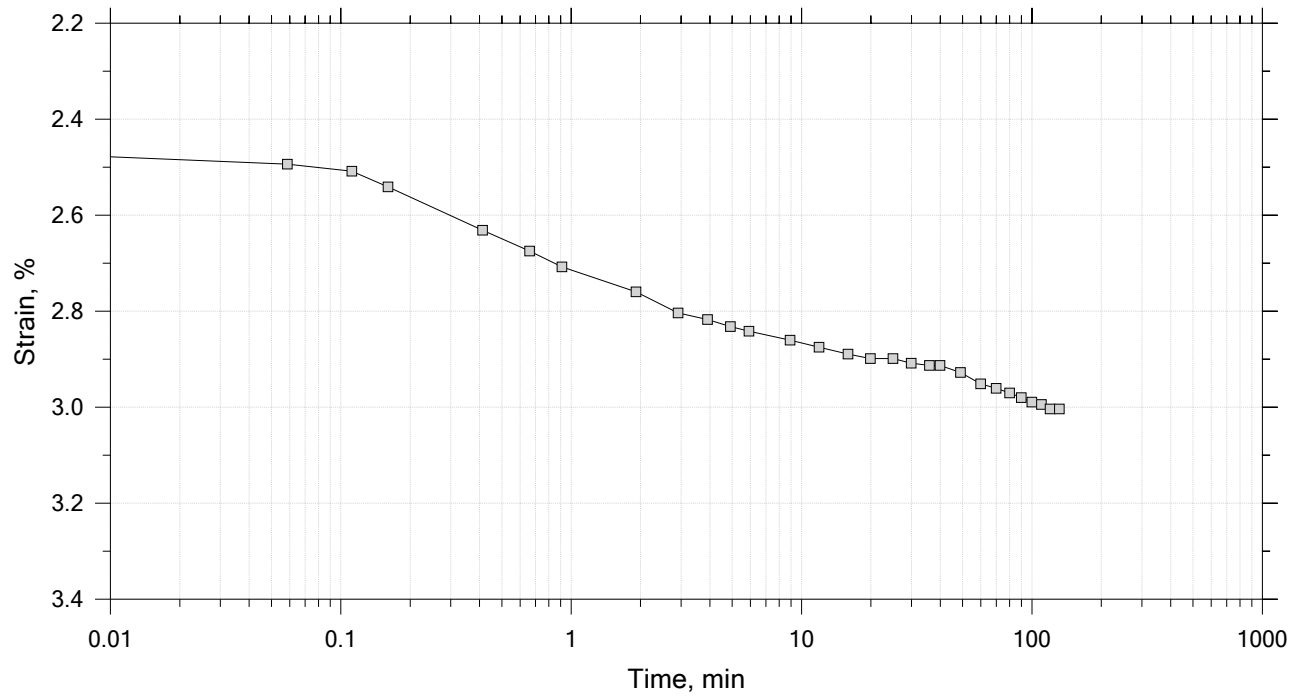
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



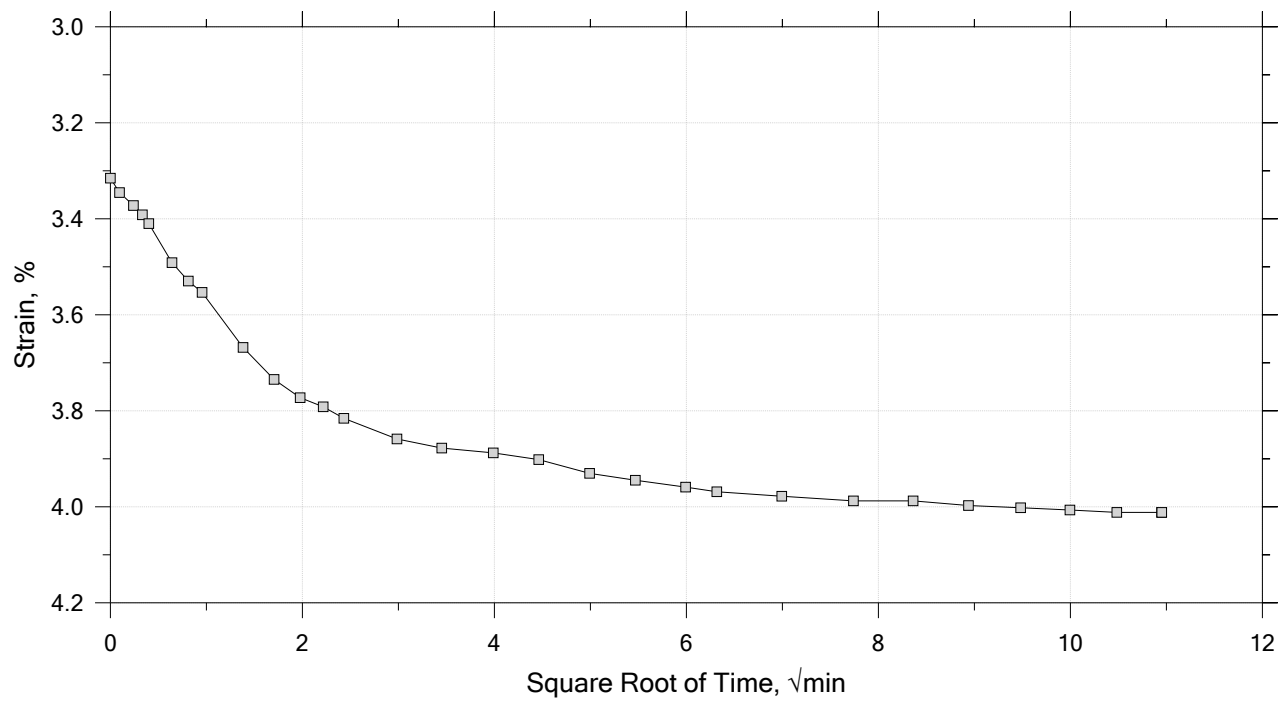
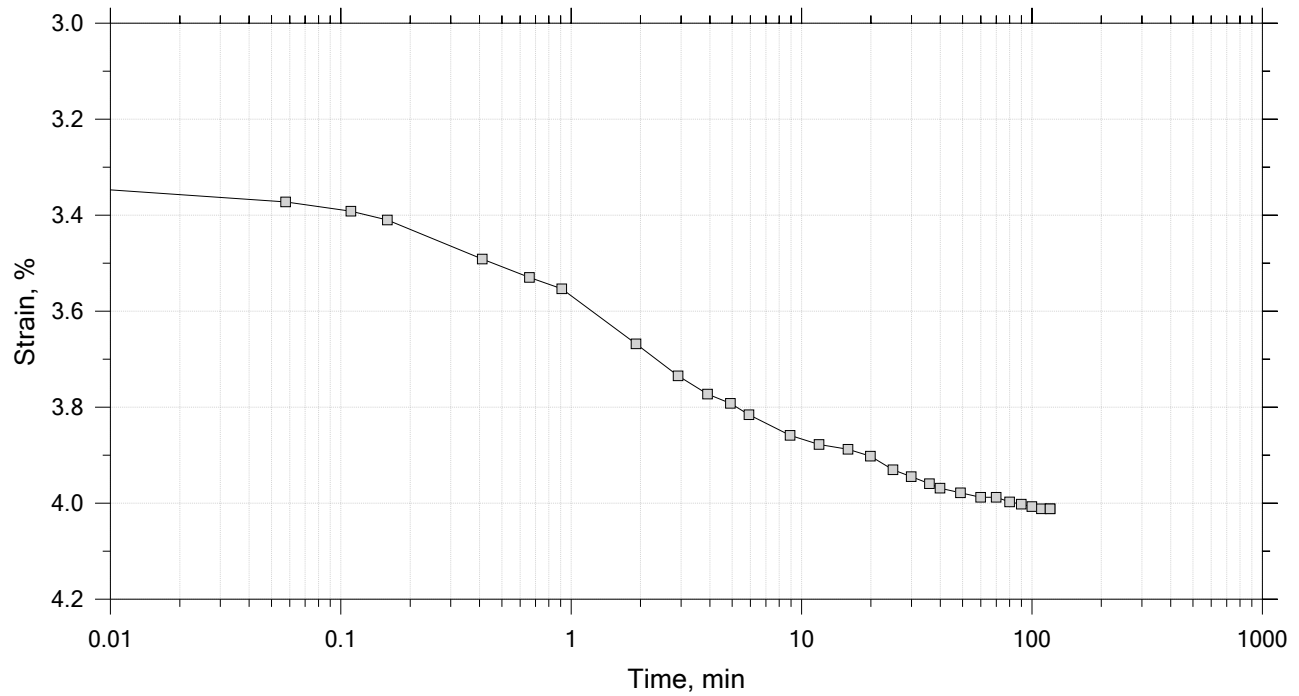
	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



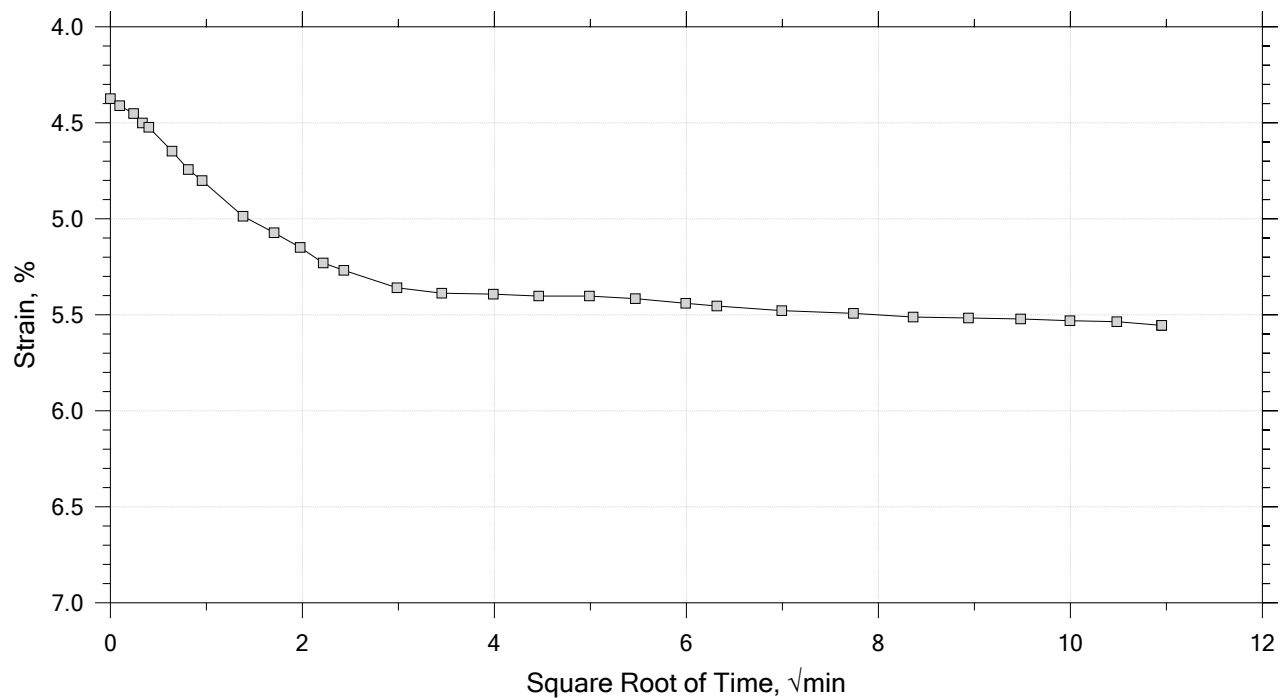
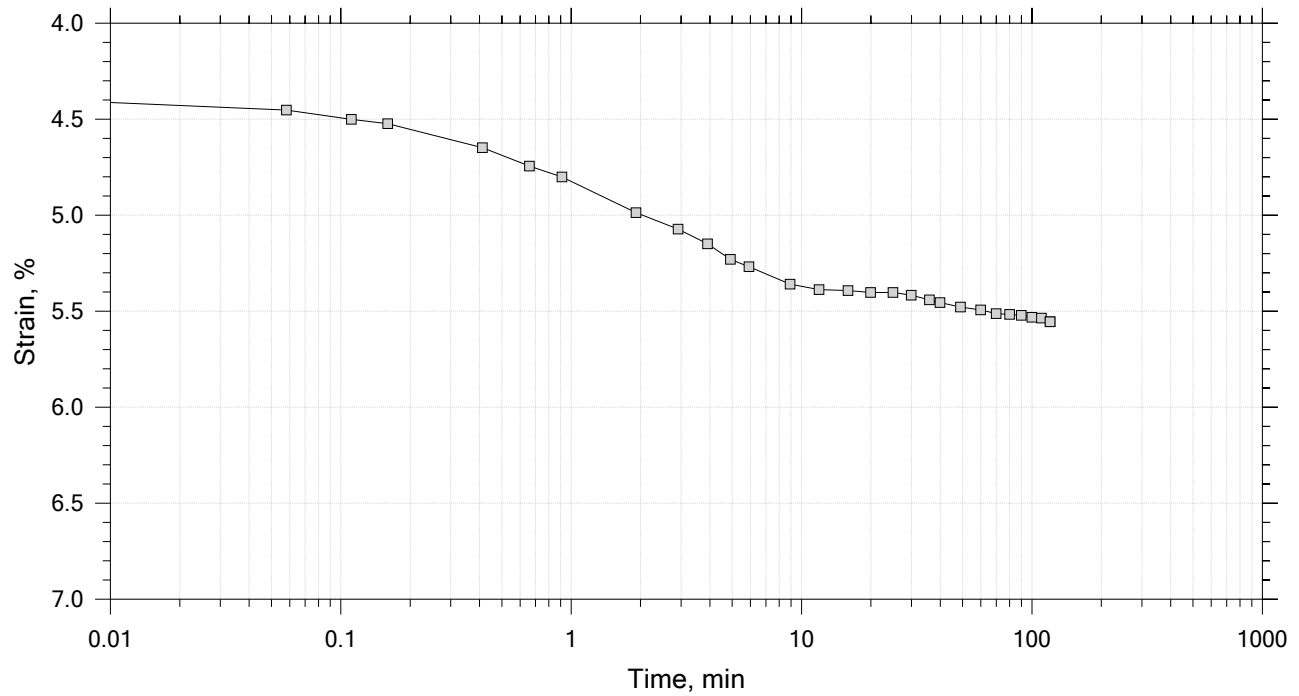
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



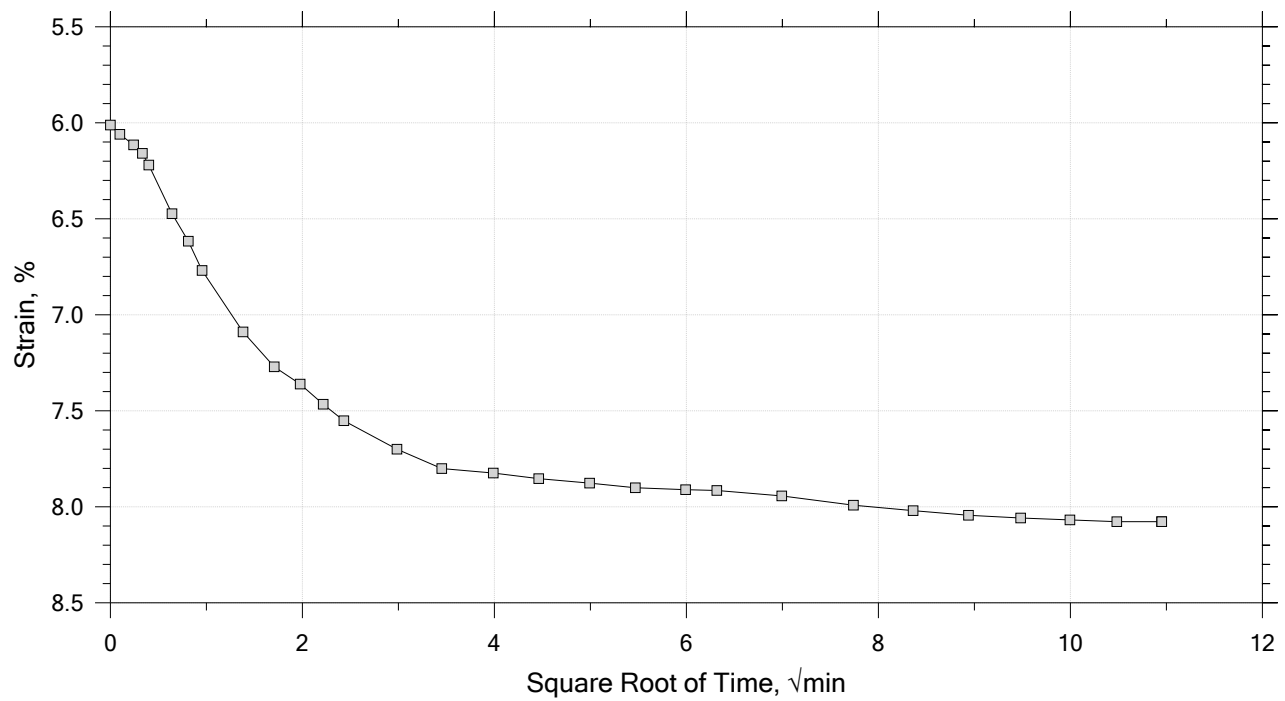
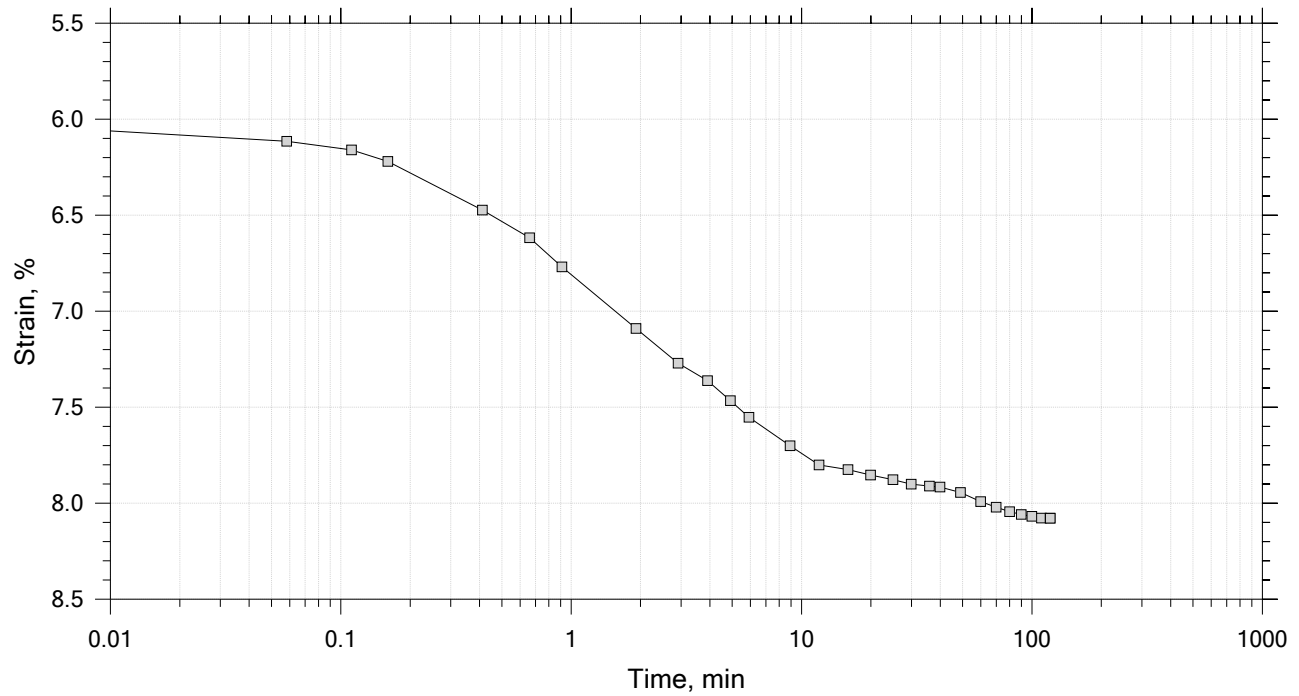
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



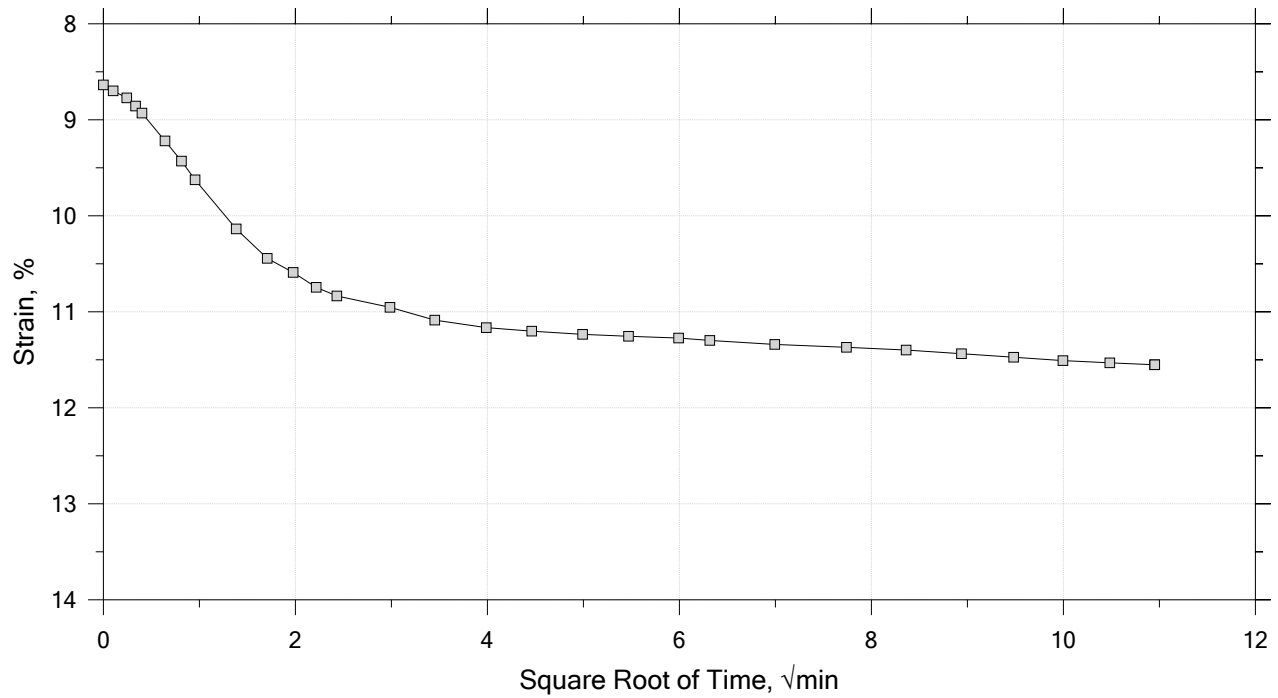
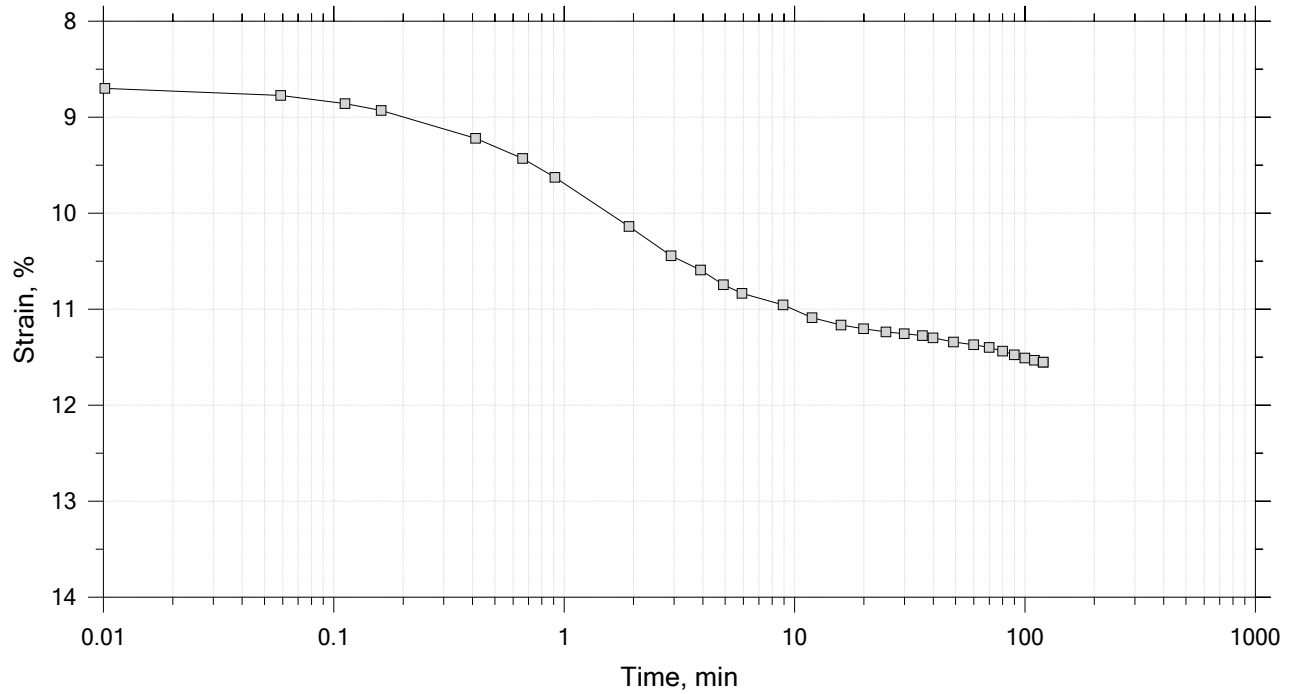
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



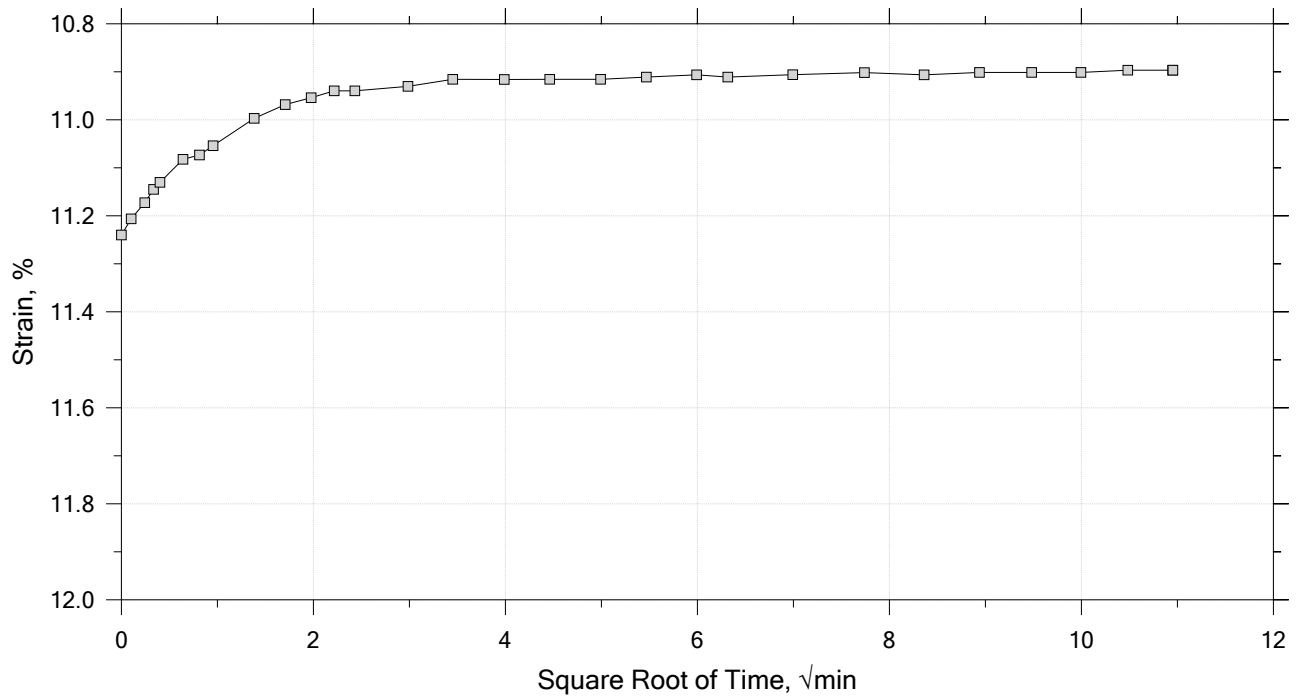
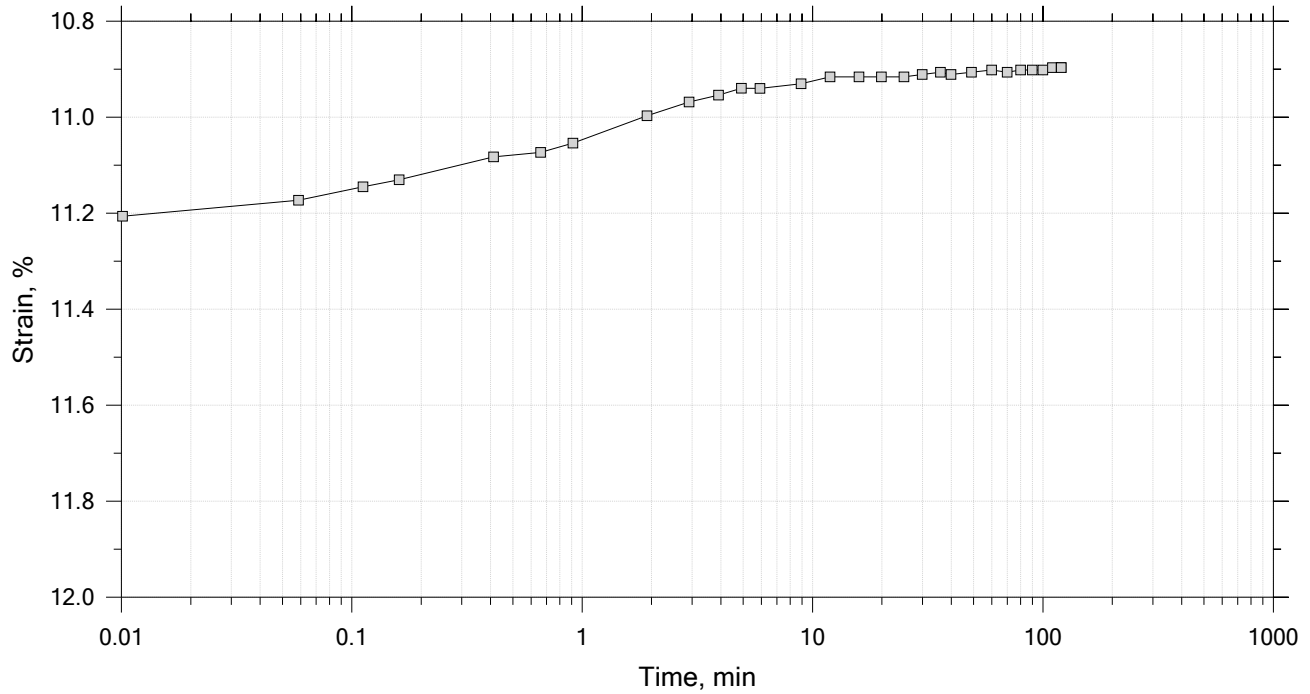
	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



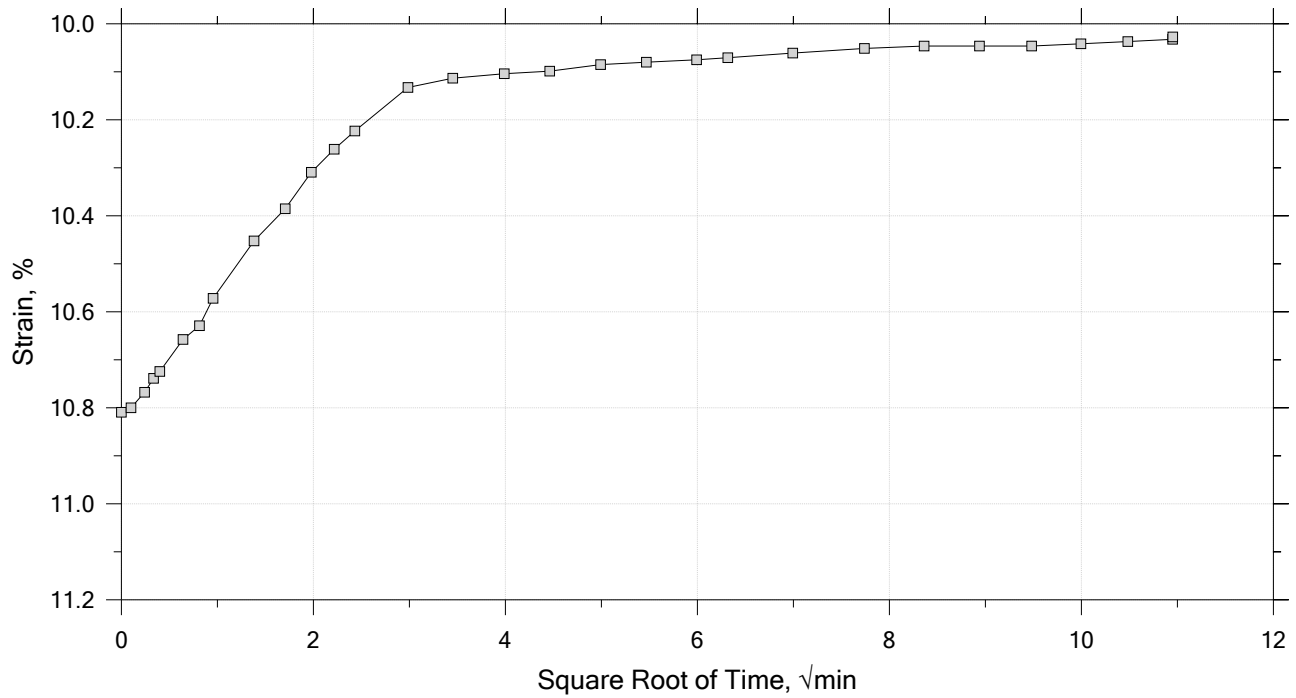
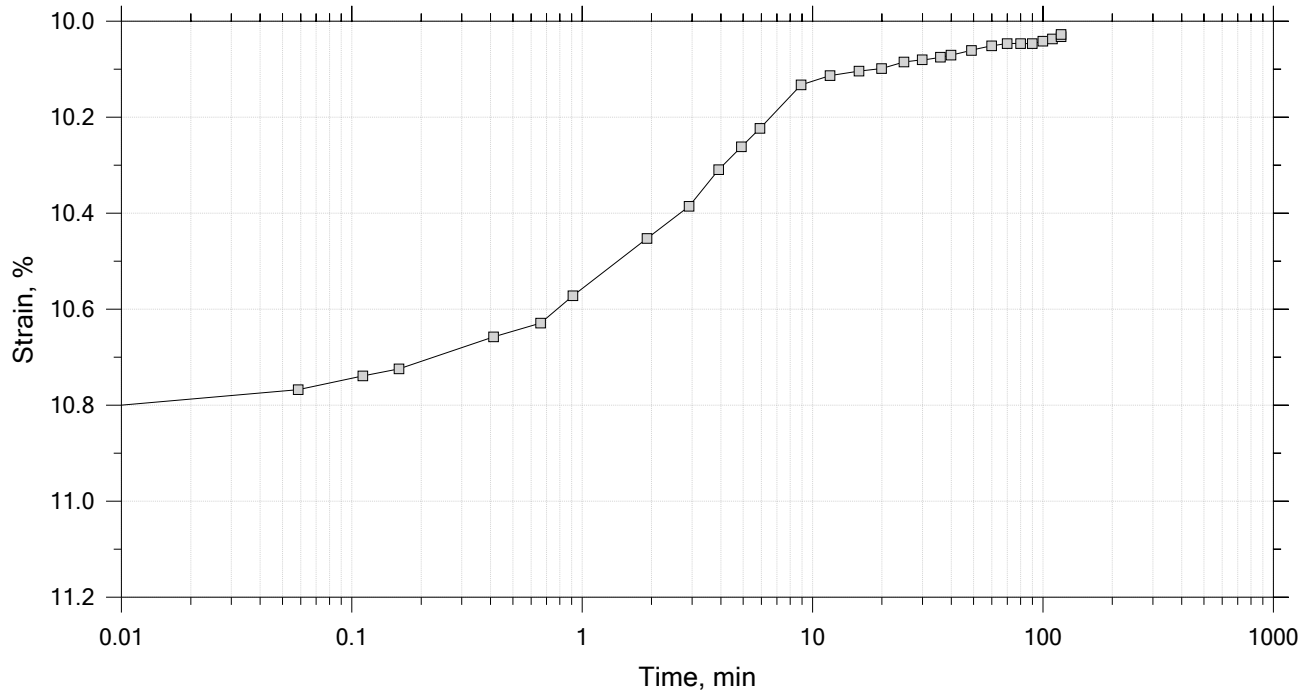
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



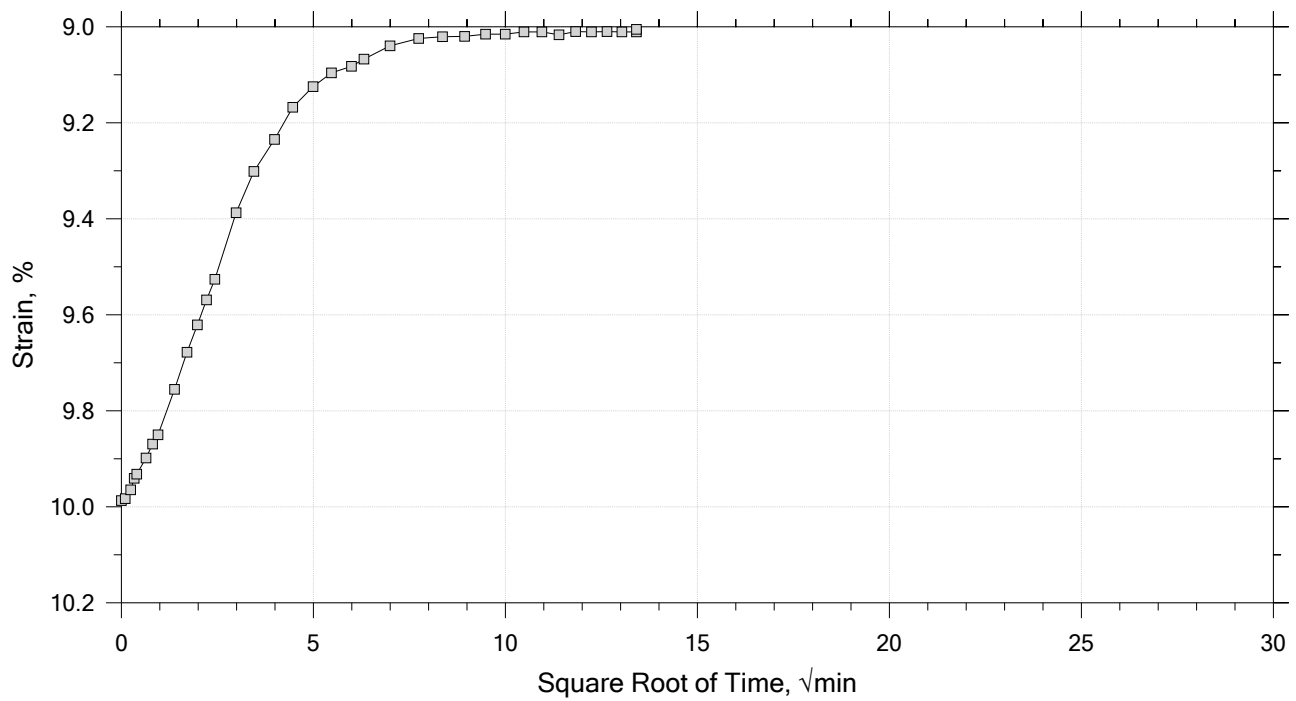
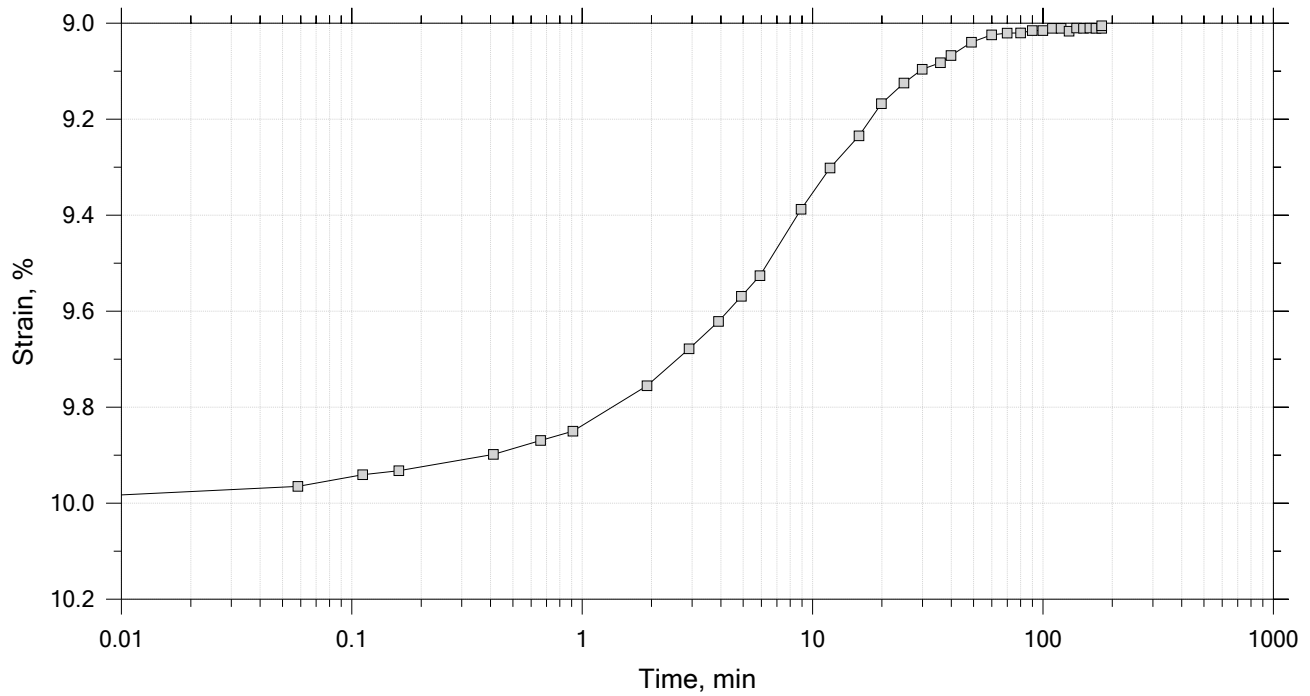
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



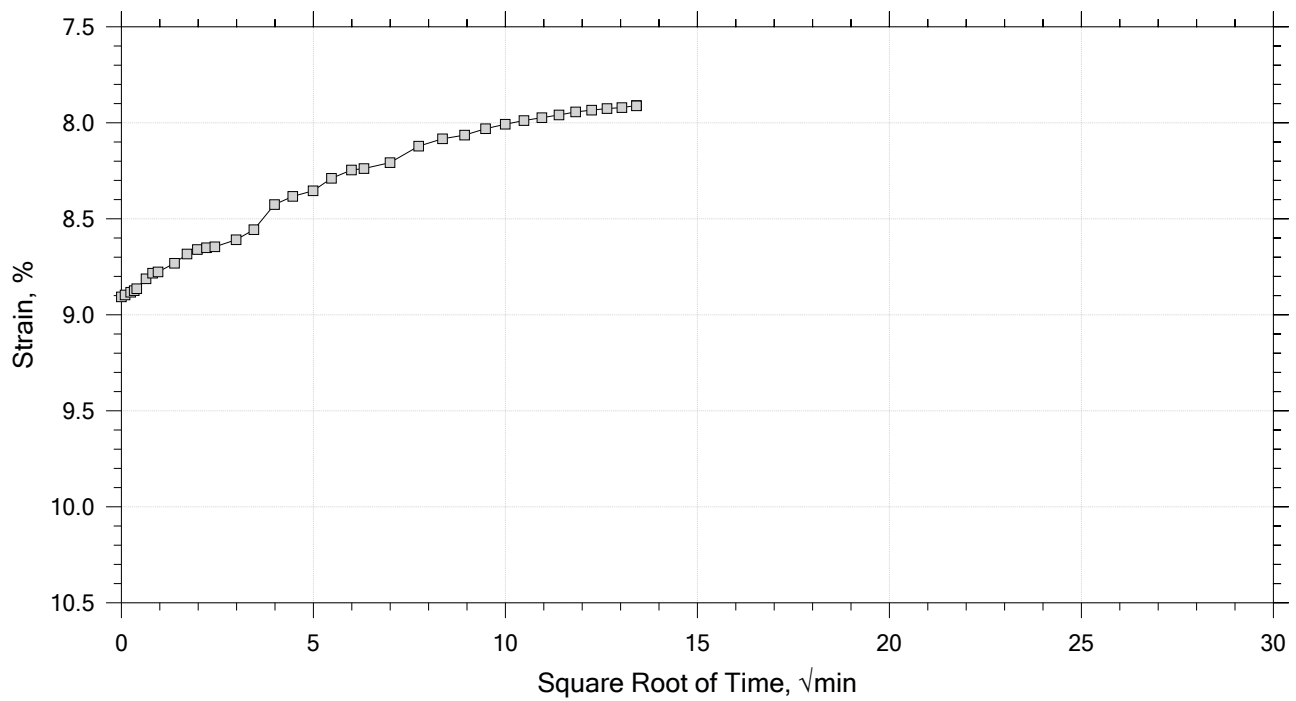
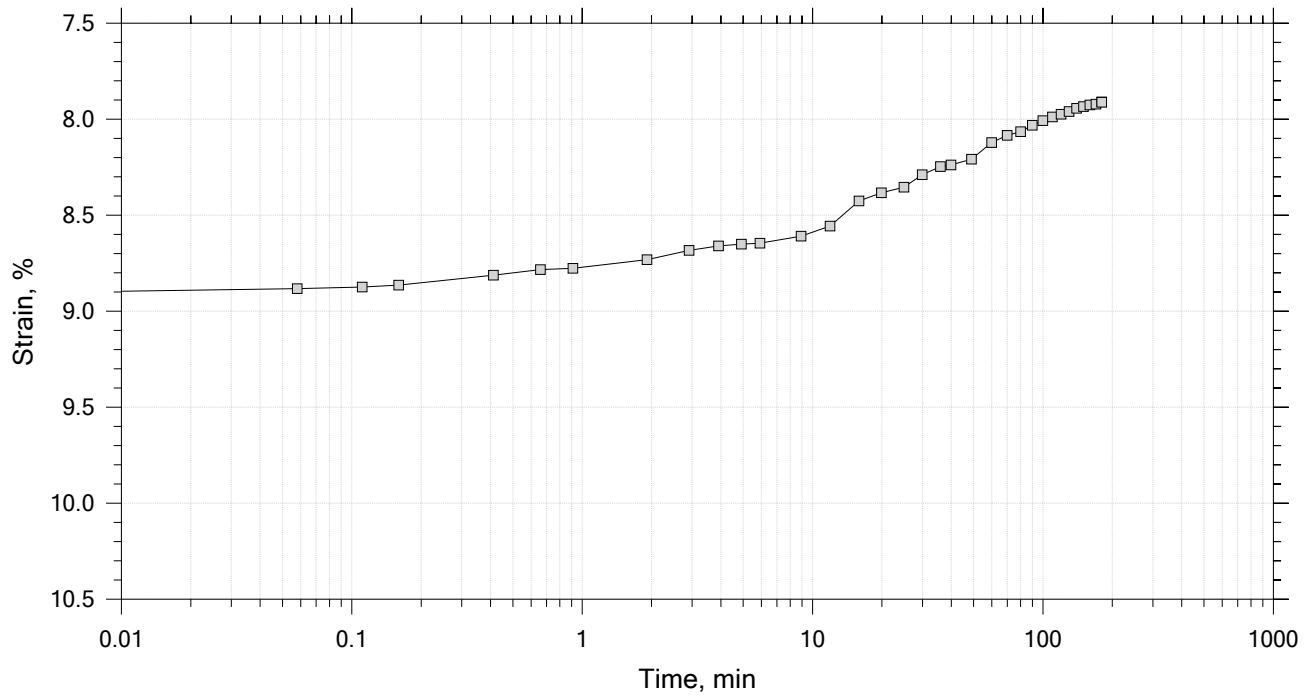
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



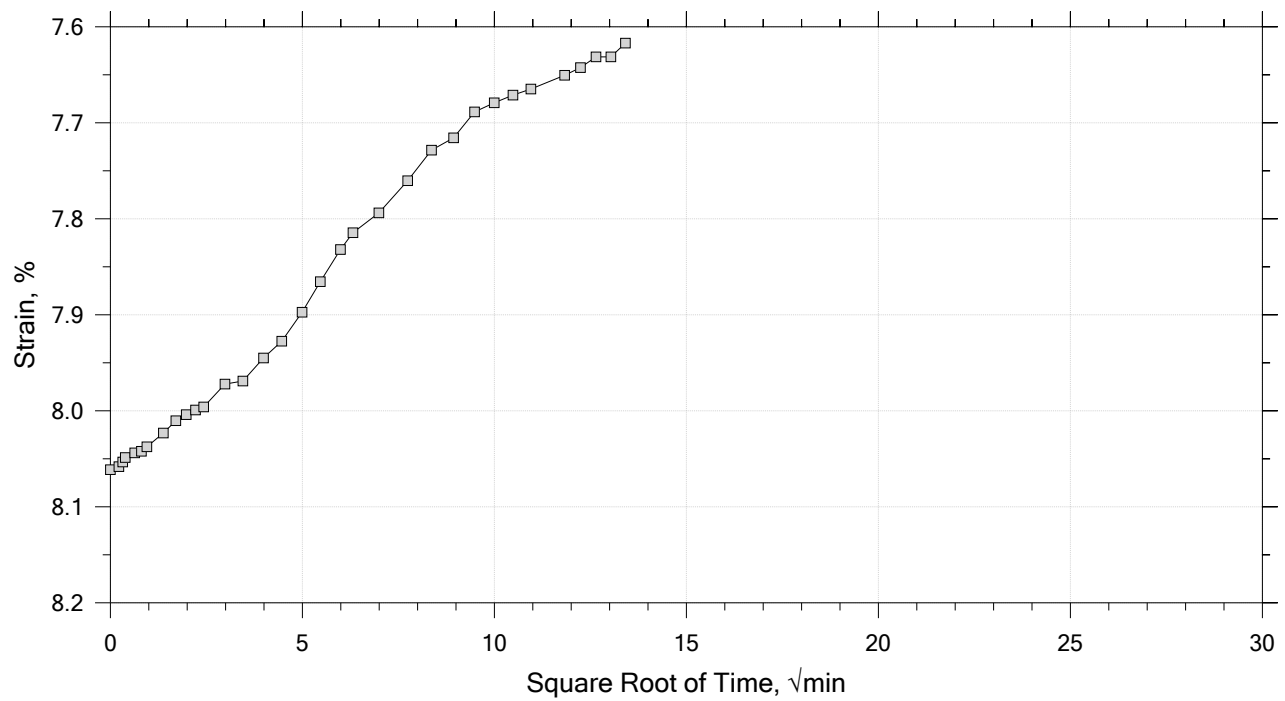
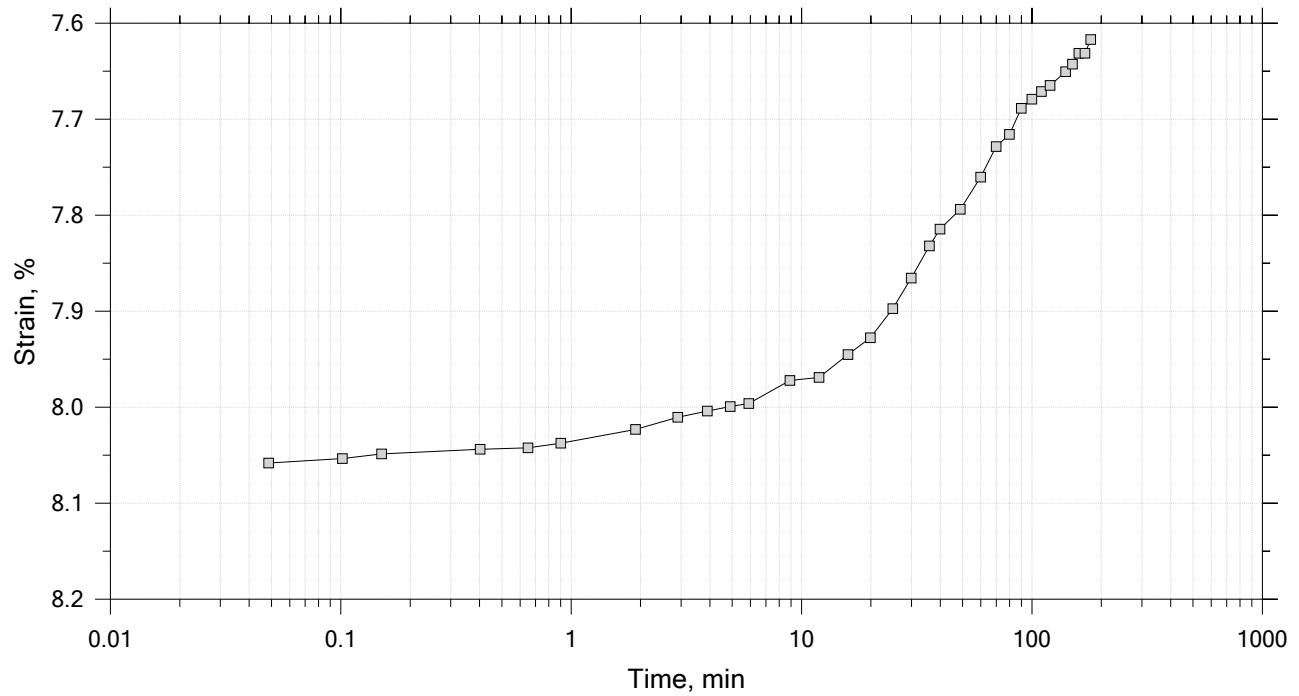
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	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 32
Initial Height: 1.00 in	Initial Void Ratio: 0.812	Plastic Limit: 19
Final Height: 0.93 in	Final Void Ratio: 0.692	Plasticity Index: 13

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A1412	RING		C348
Mass Container, gm	8.15	108.97	108.97	9.04
Mass Container + Wet Soil, gm	220.69	267.73	263.22	162.34
Mass Container + Dry Soil, gm	174.74	232.49	232.49	131.8
Mass Dry Soil, gm	166.59	123.52	123.52	122.76
Water Content, %	27.58	28.53	24.88	24.88
Void Ratio	---	0.81	0.69	---
Degree of Saturation, %	---	97.78	100.00	---
Dry Unit Weight, pcf	---	95.862	102.64	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

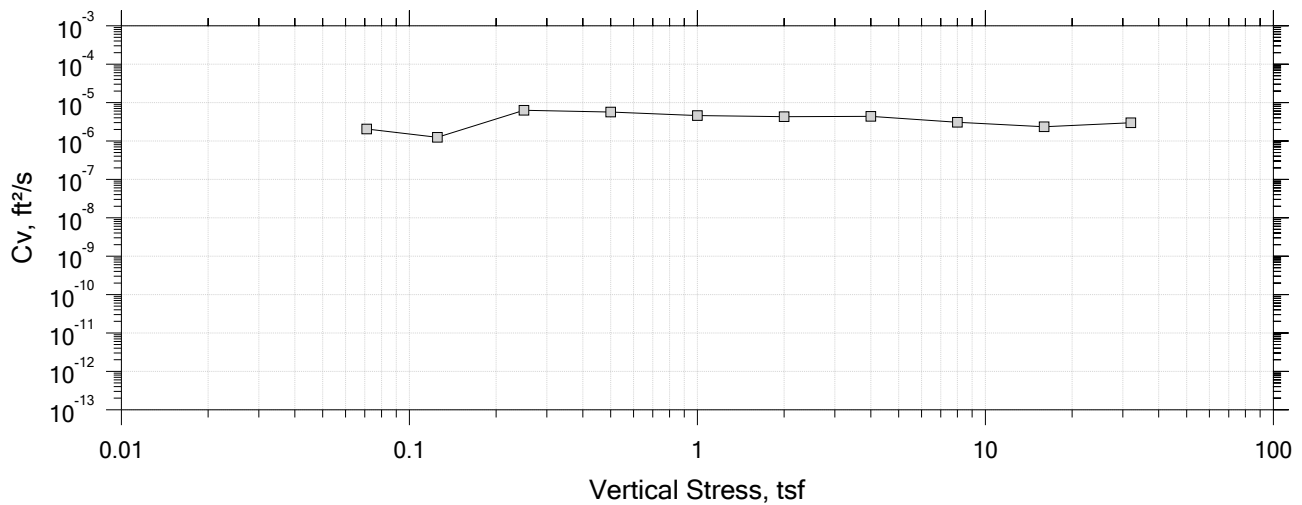
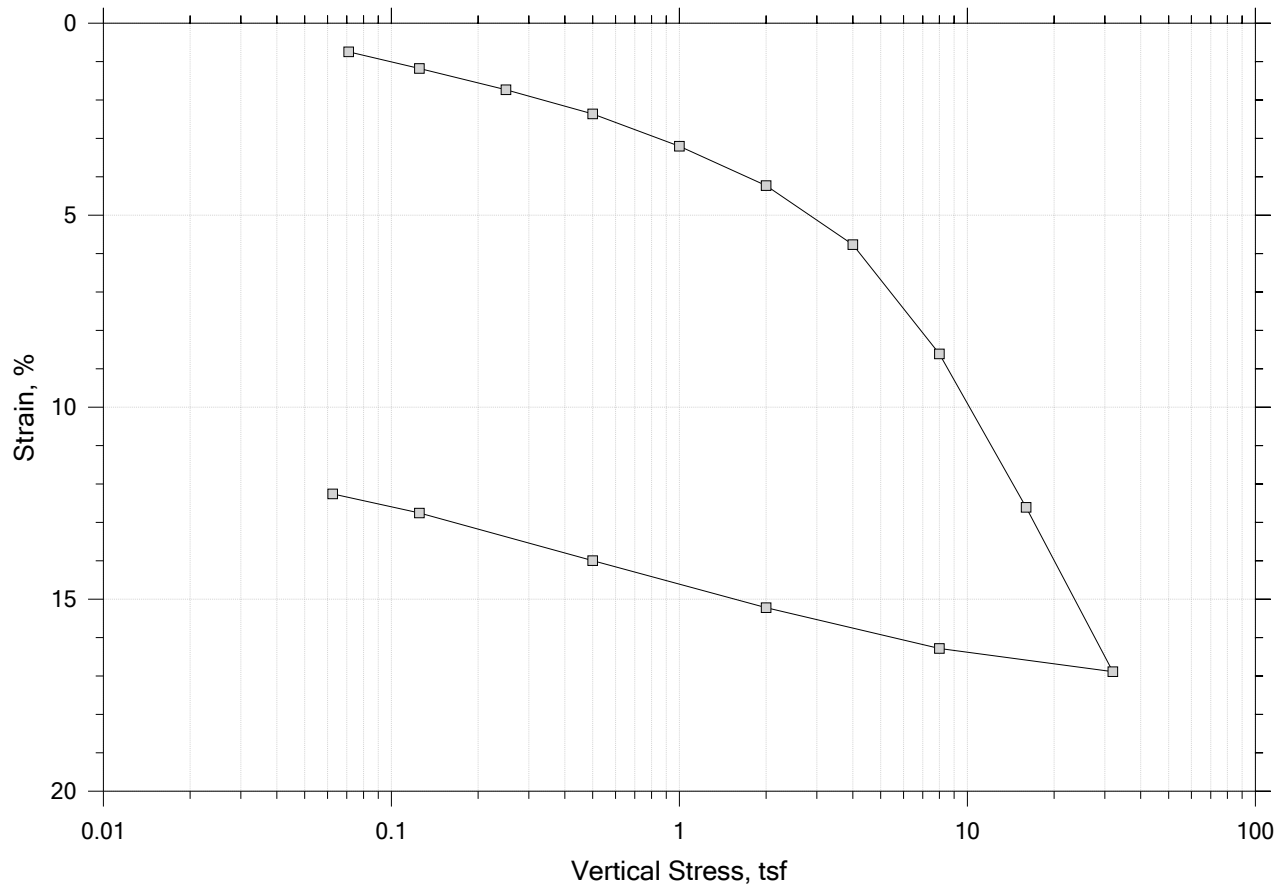
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-102	Tested By: trm	Checked By:
	Sample No.: 1U	Test Date: 9/29/18	Depth: 10-11.3 ft
	Test No.: IP-8	Sample Type: intact	Elevation: ---
	Description: Moist, dark olive gray clay		
	Remarks: System E, Swell Pressure = 0.0668 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

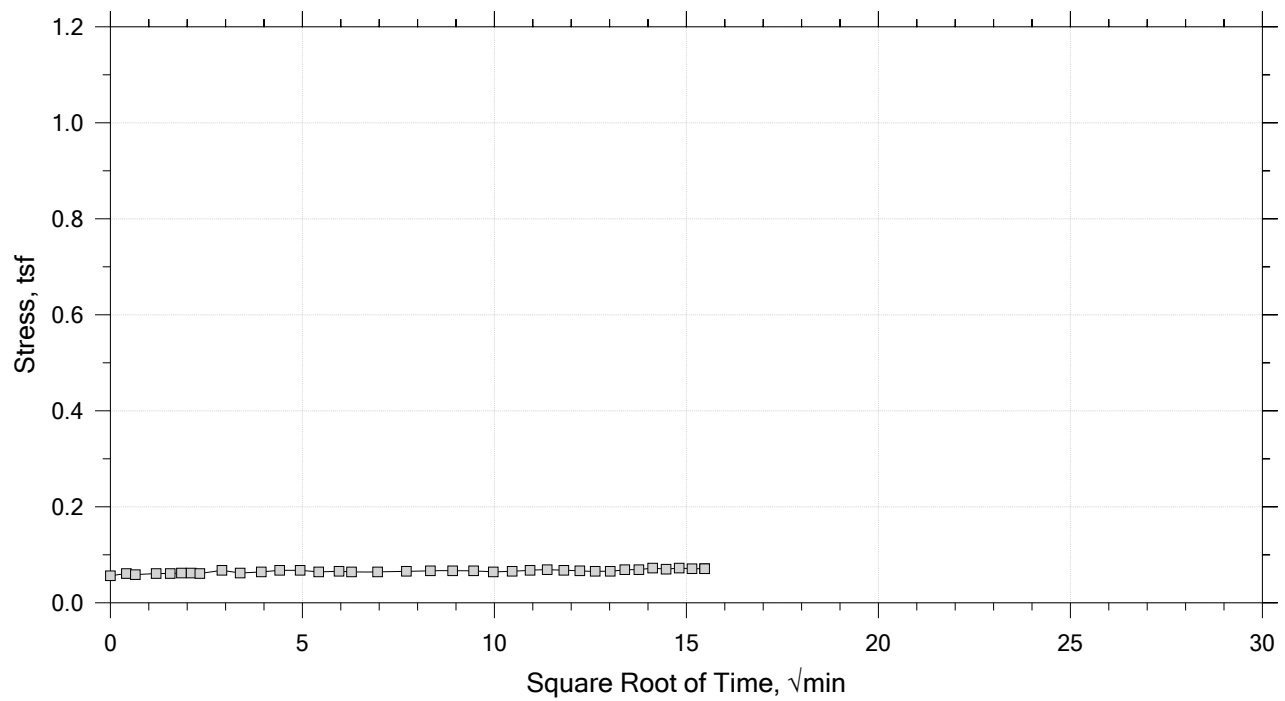
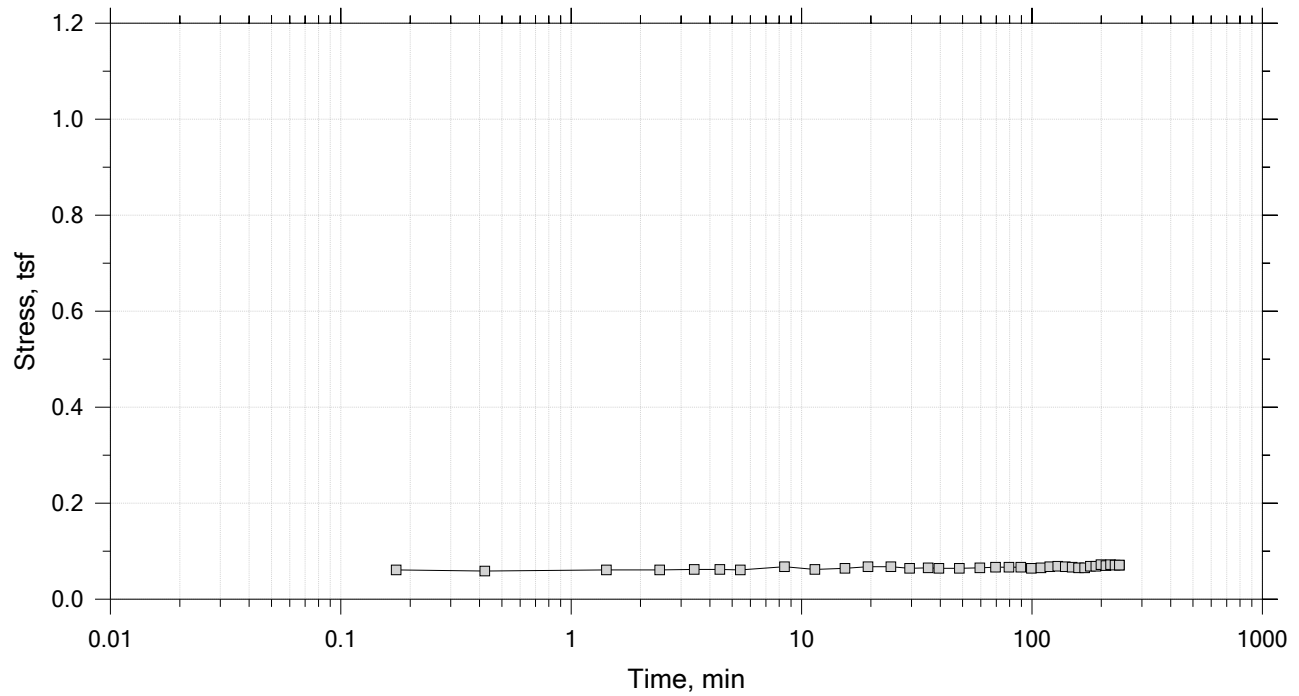
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.071 tsf



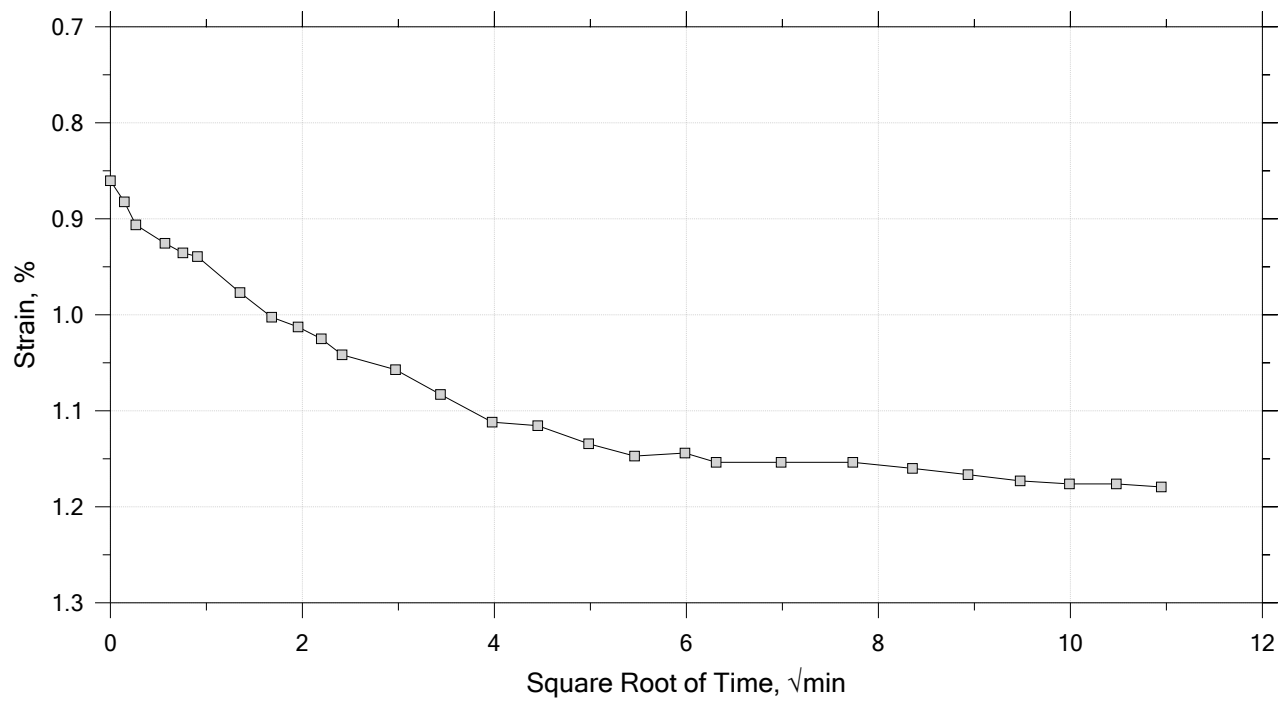
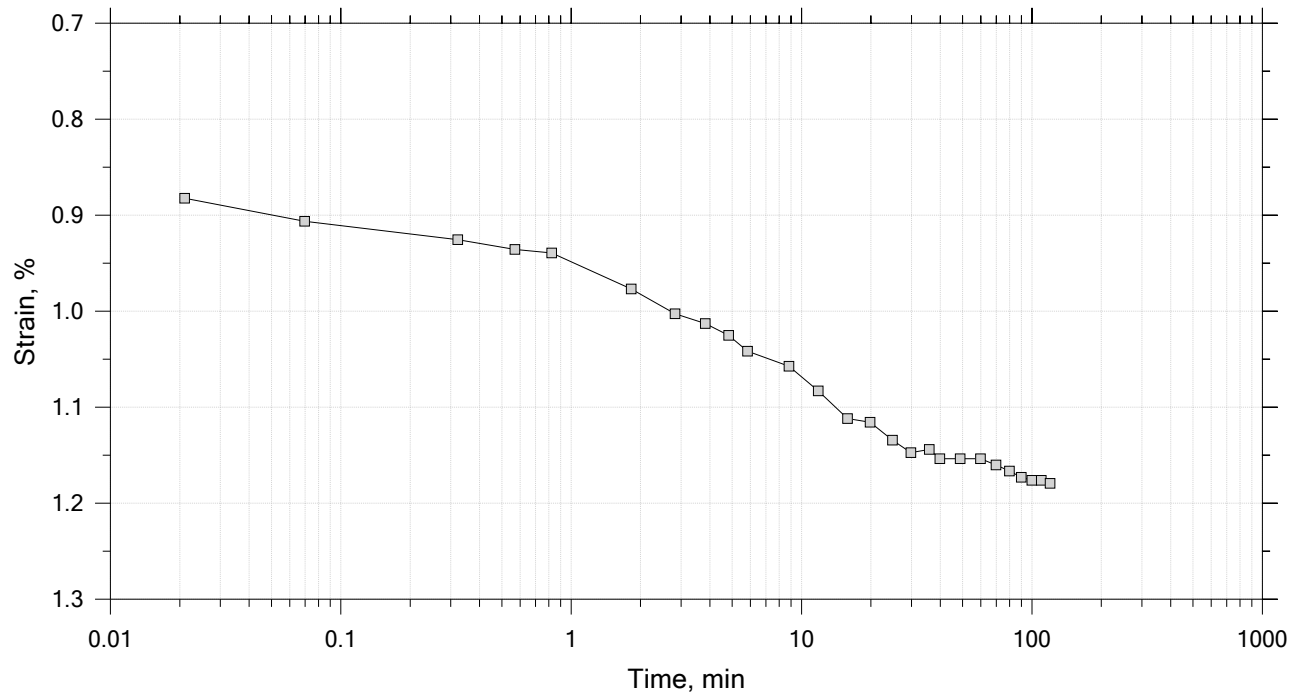
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



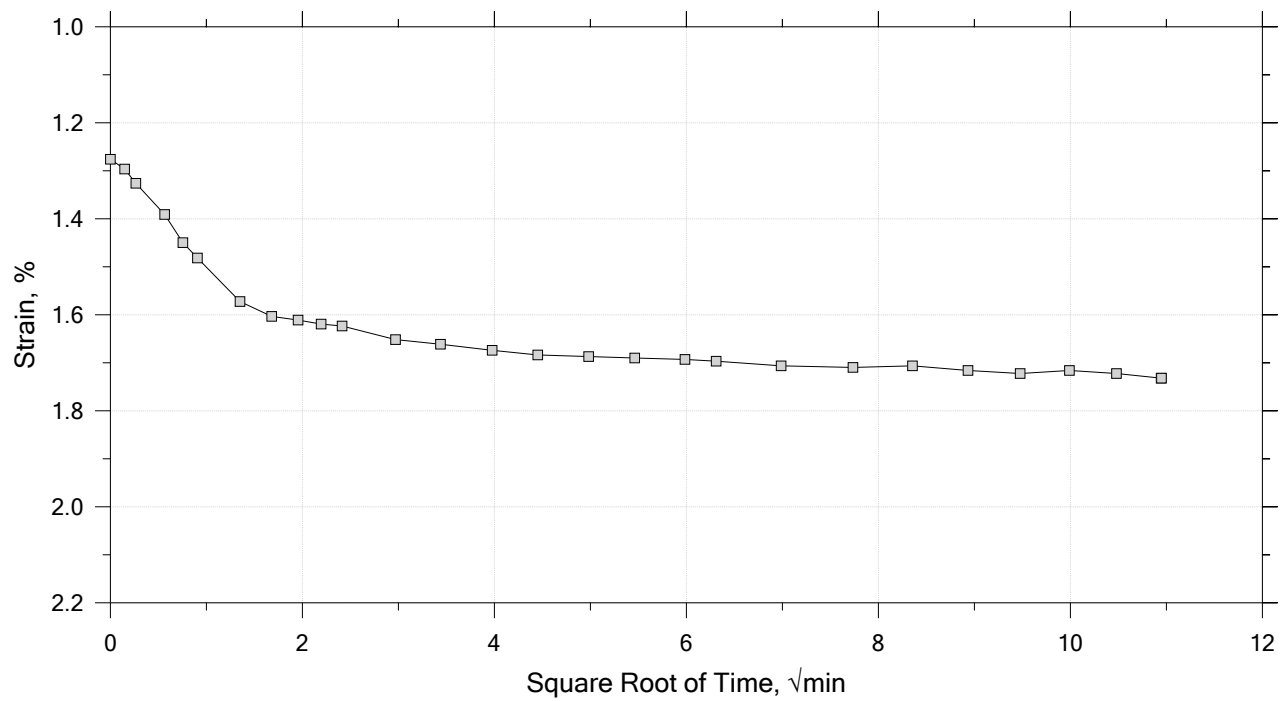
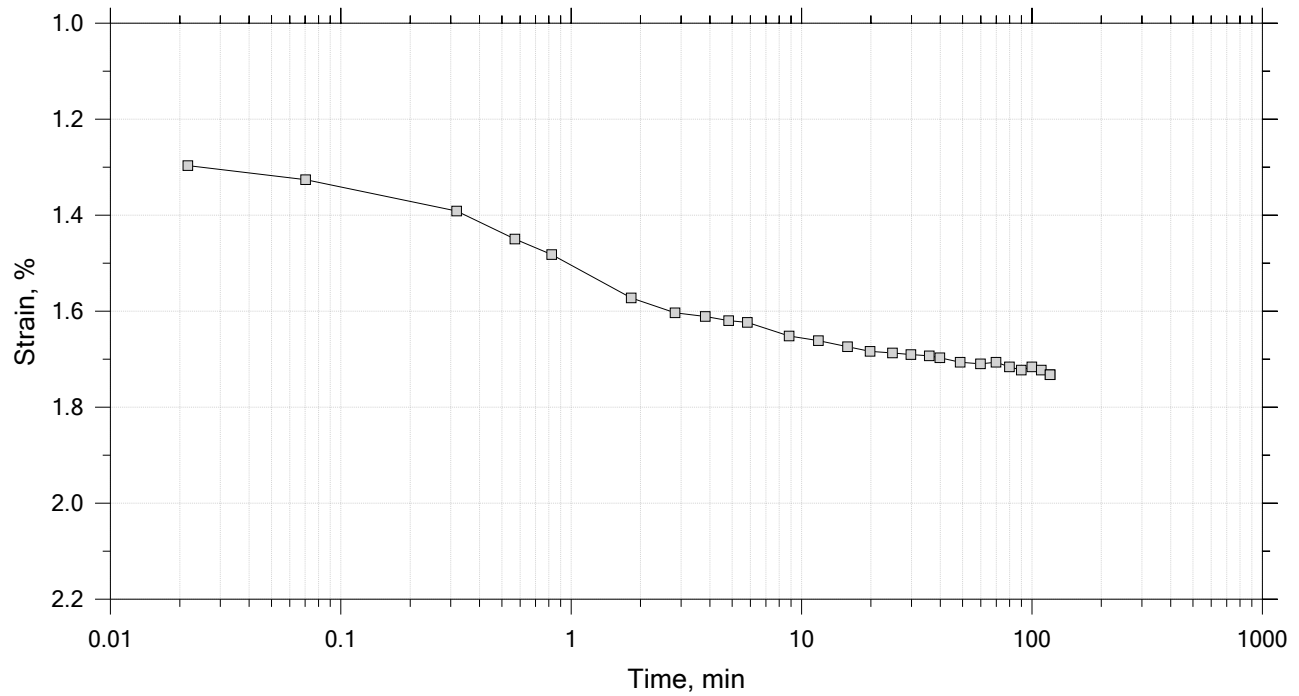
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



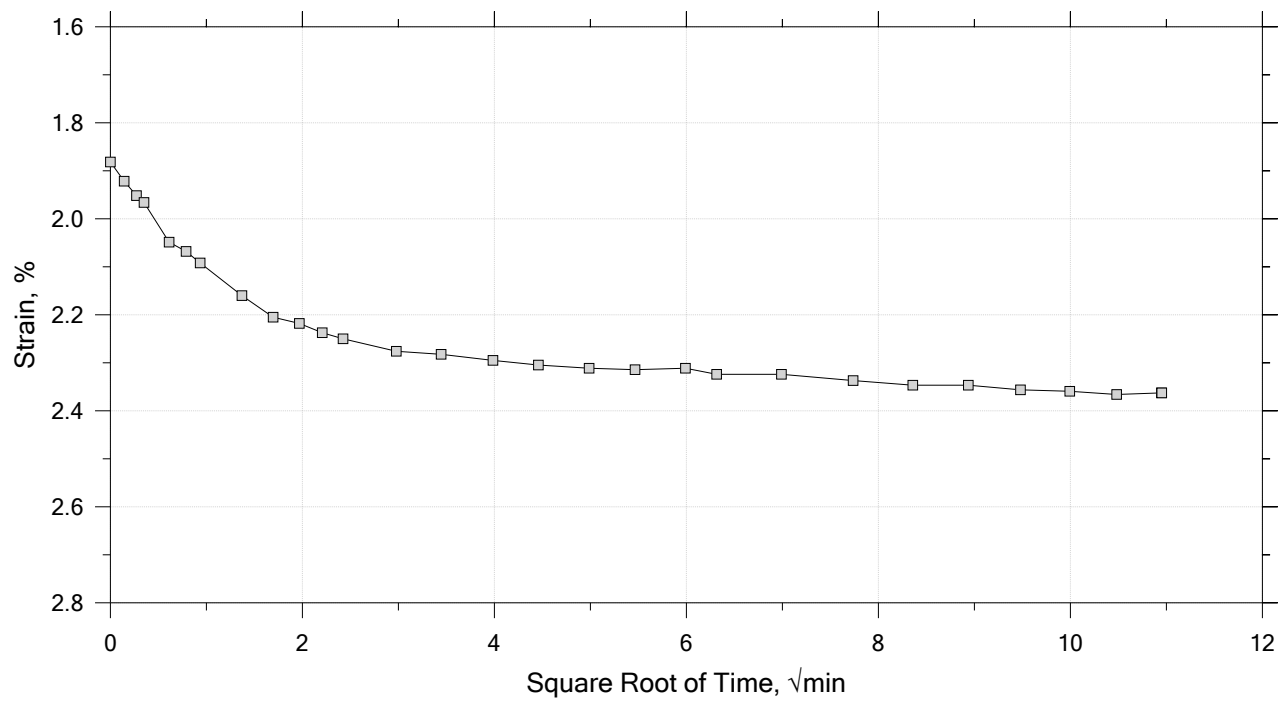
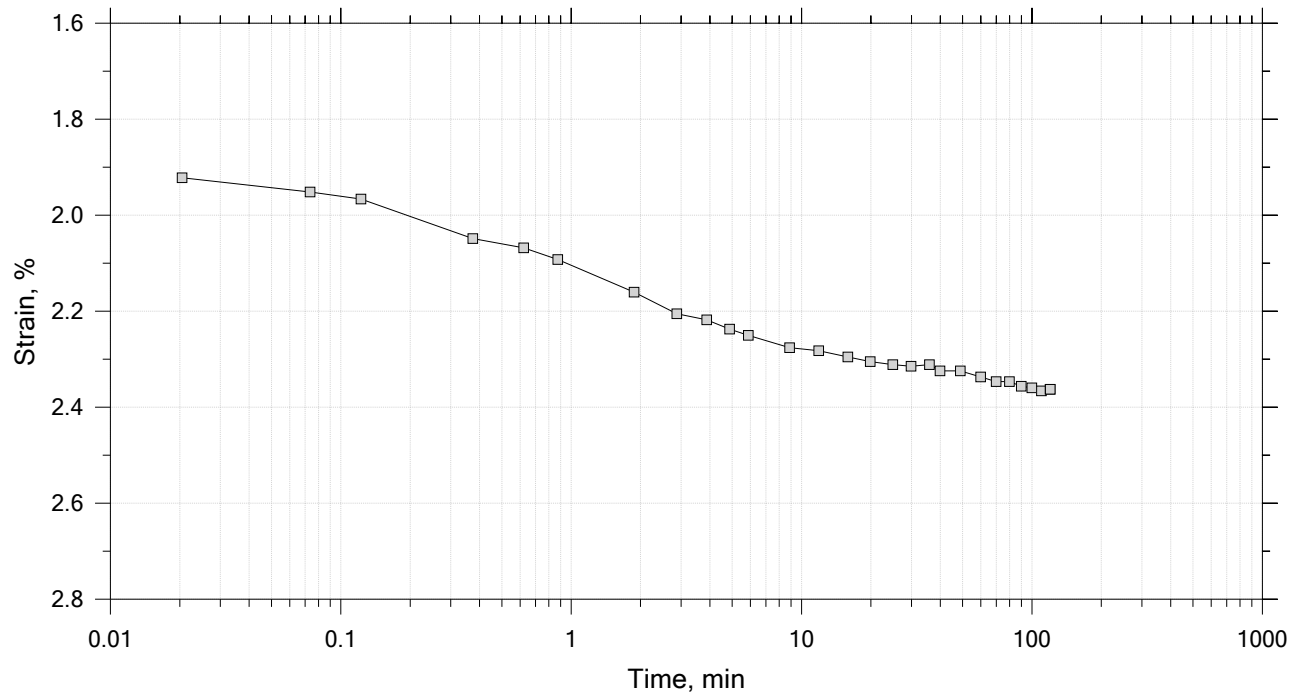
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



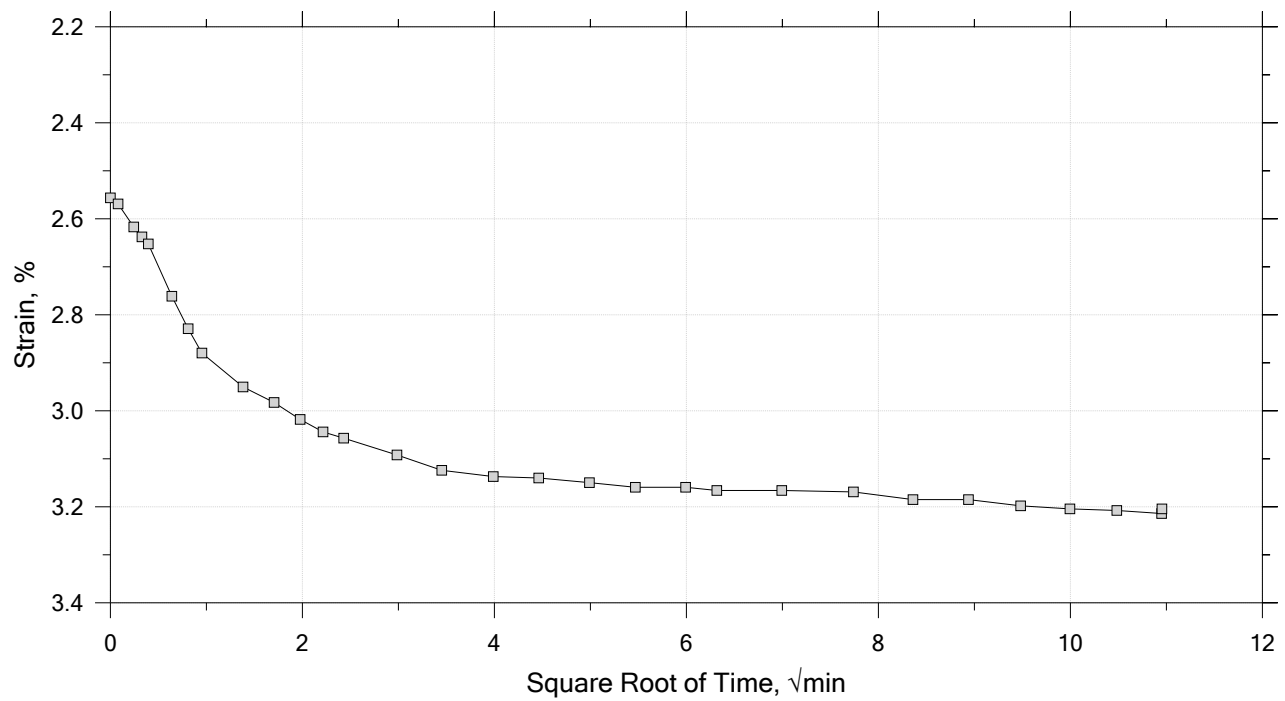
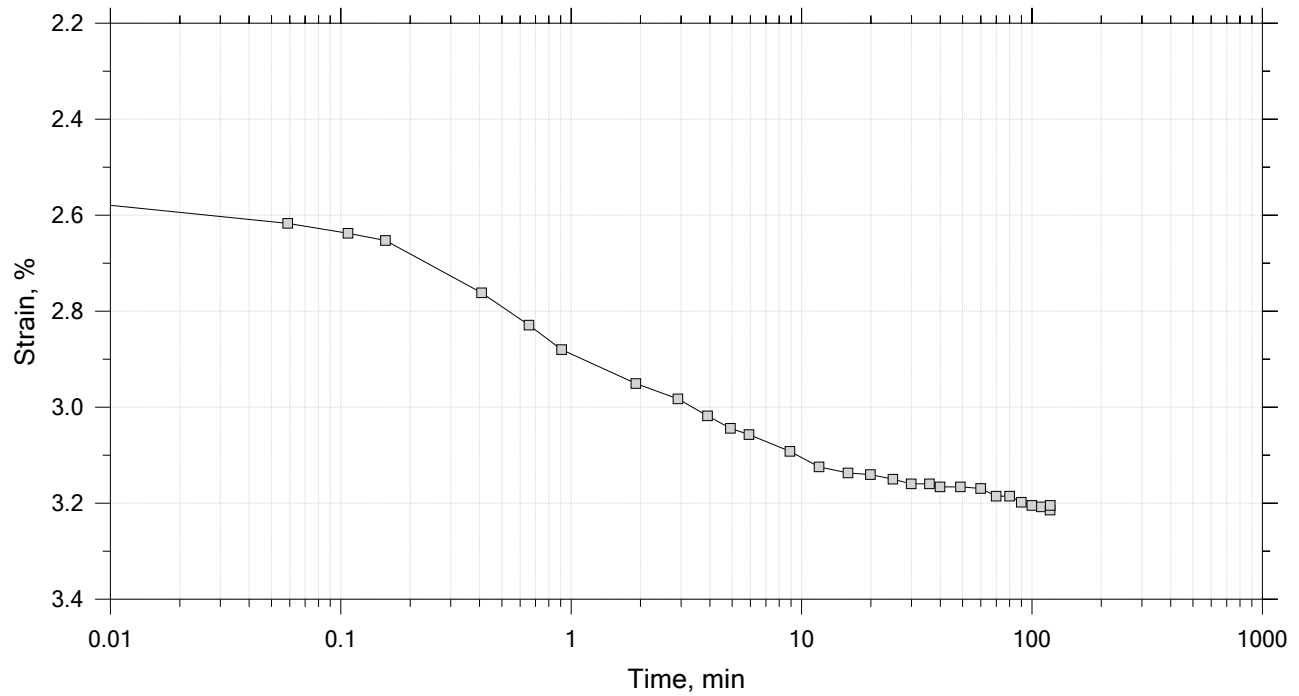
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



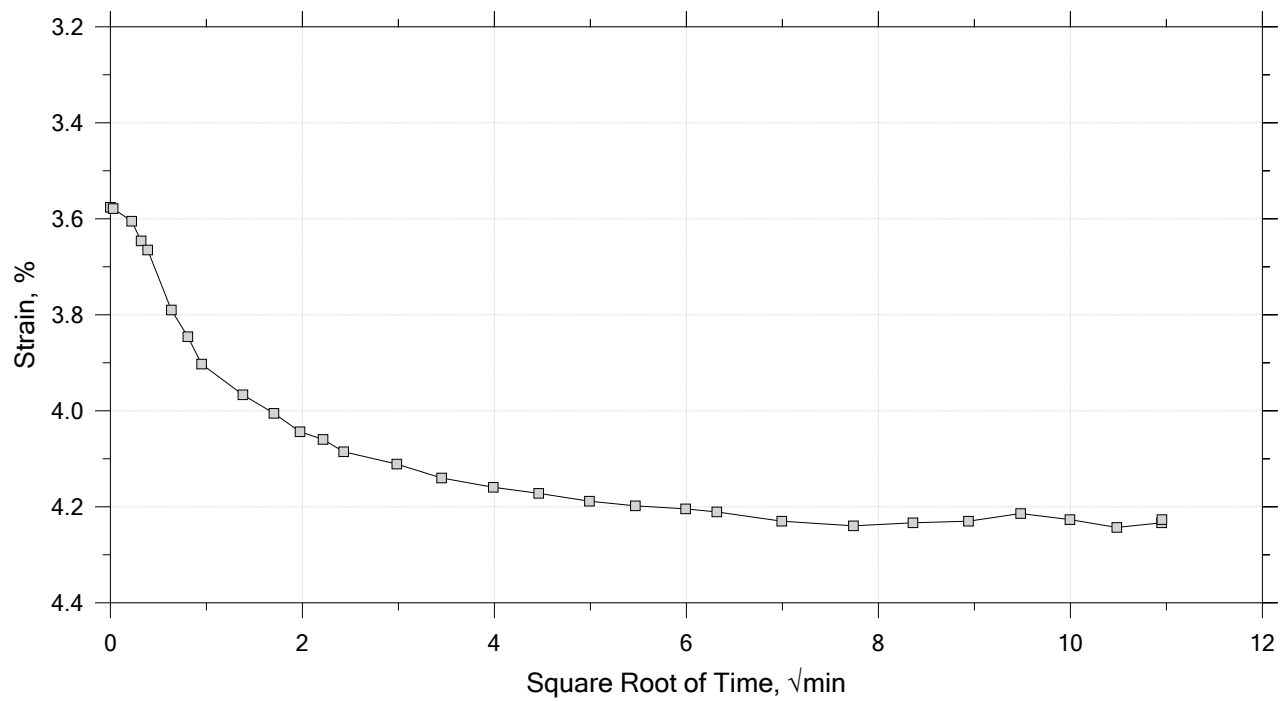
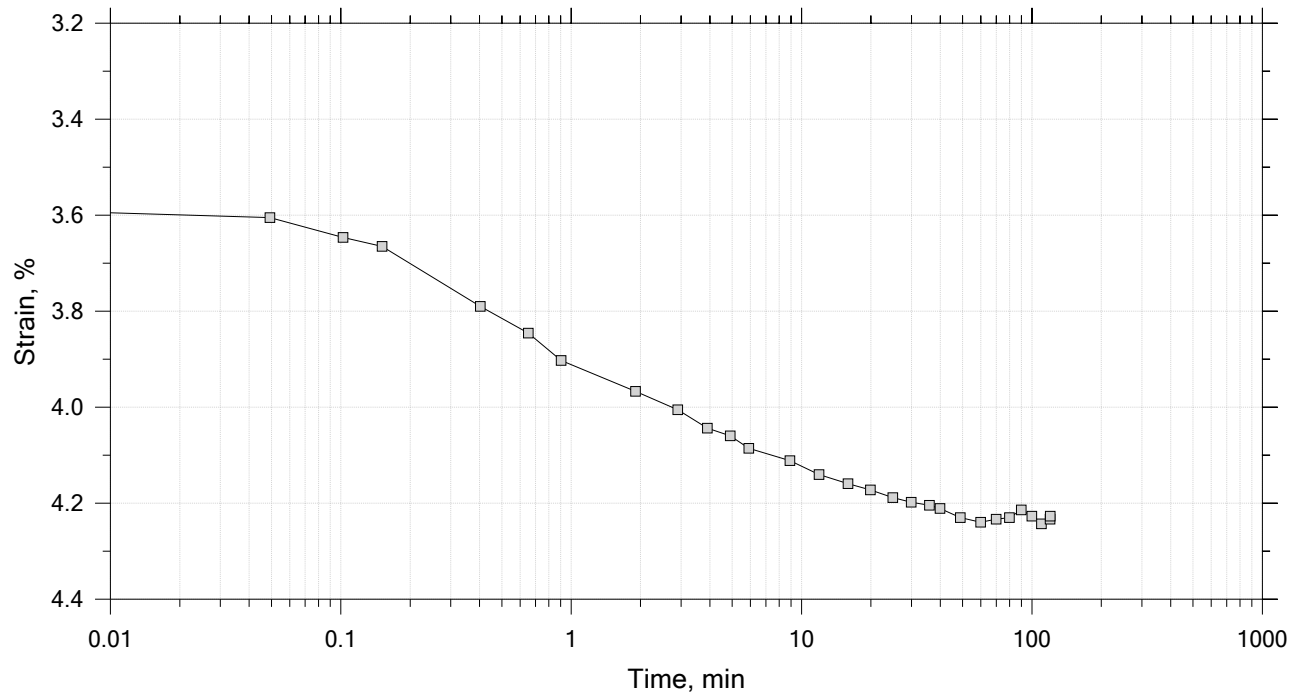
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



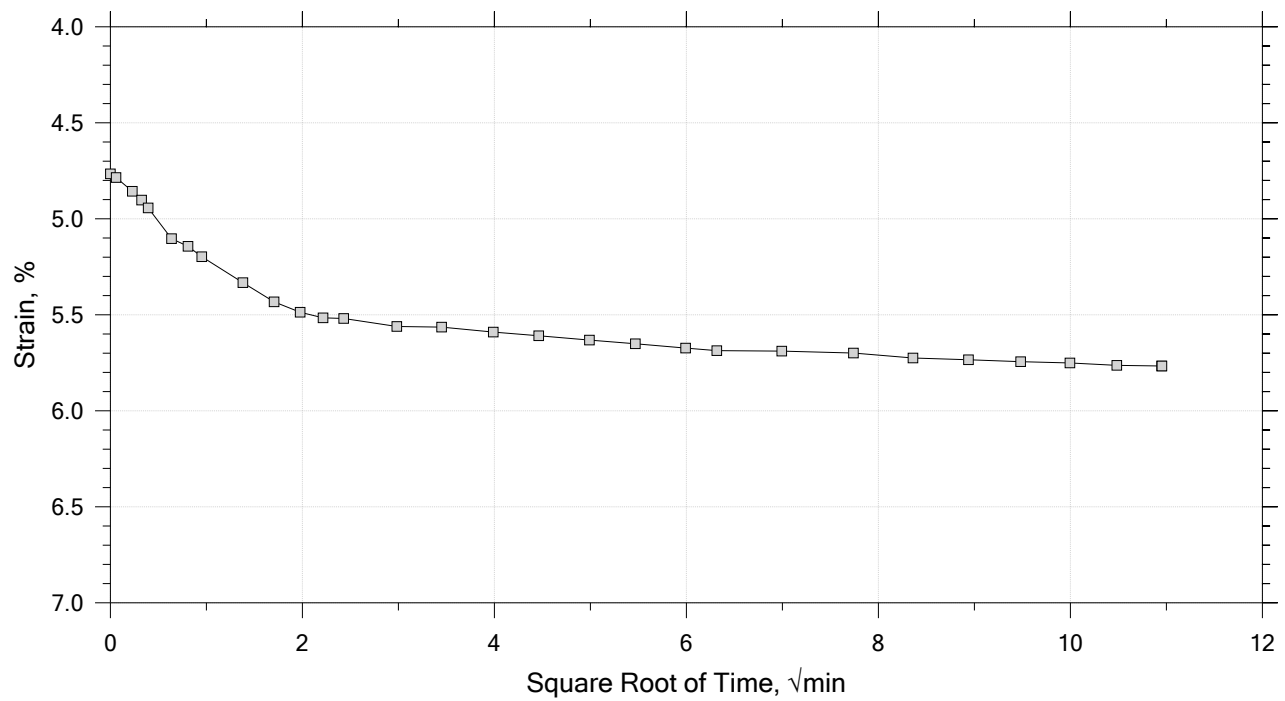
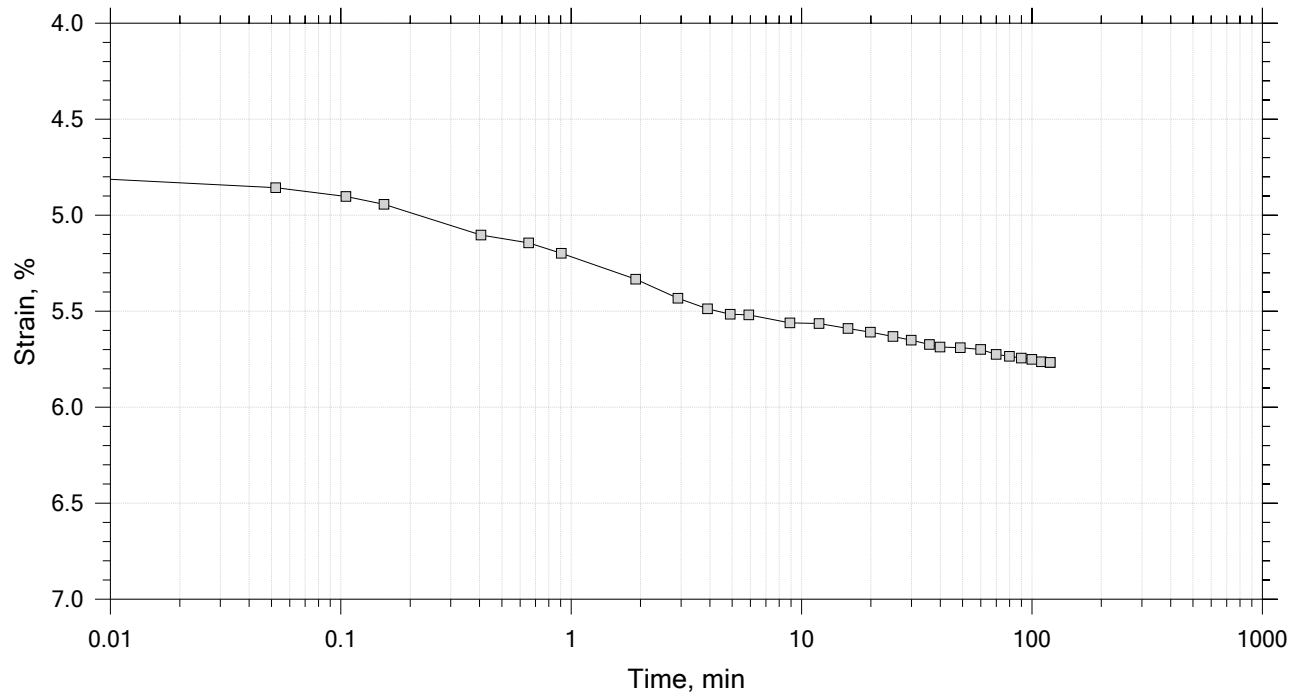
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



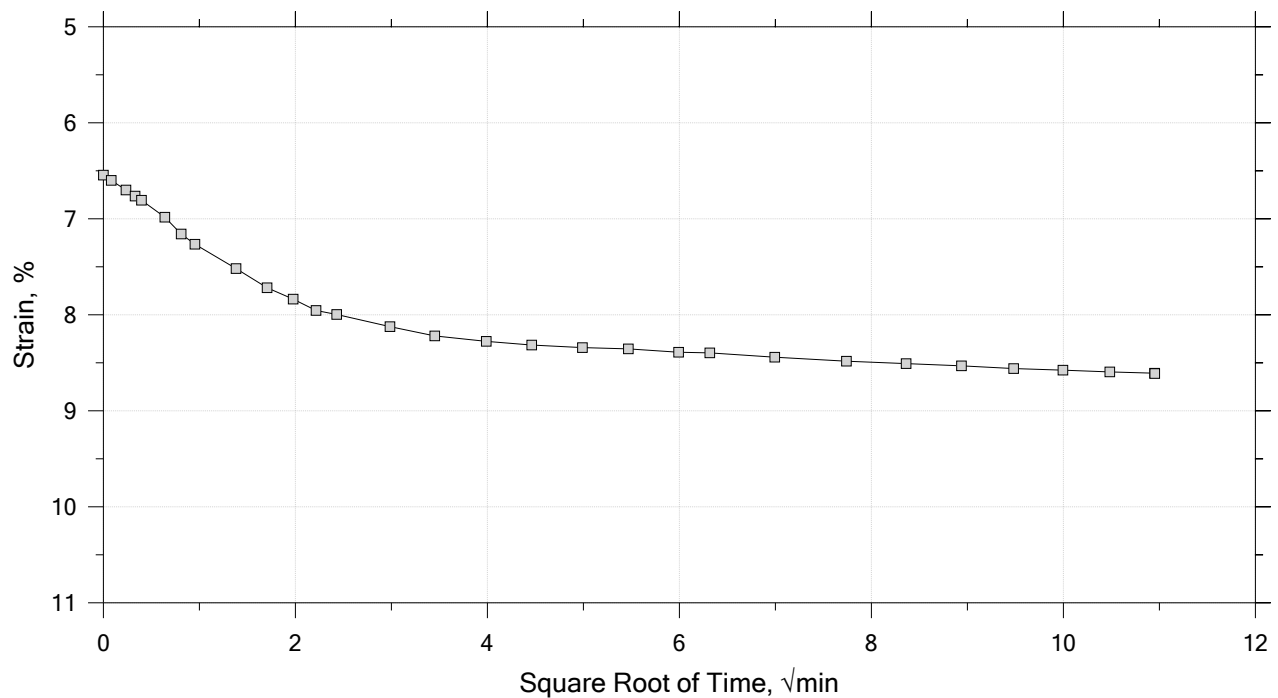
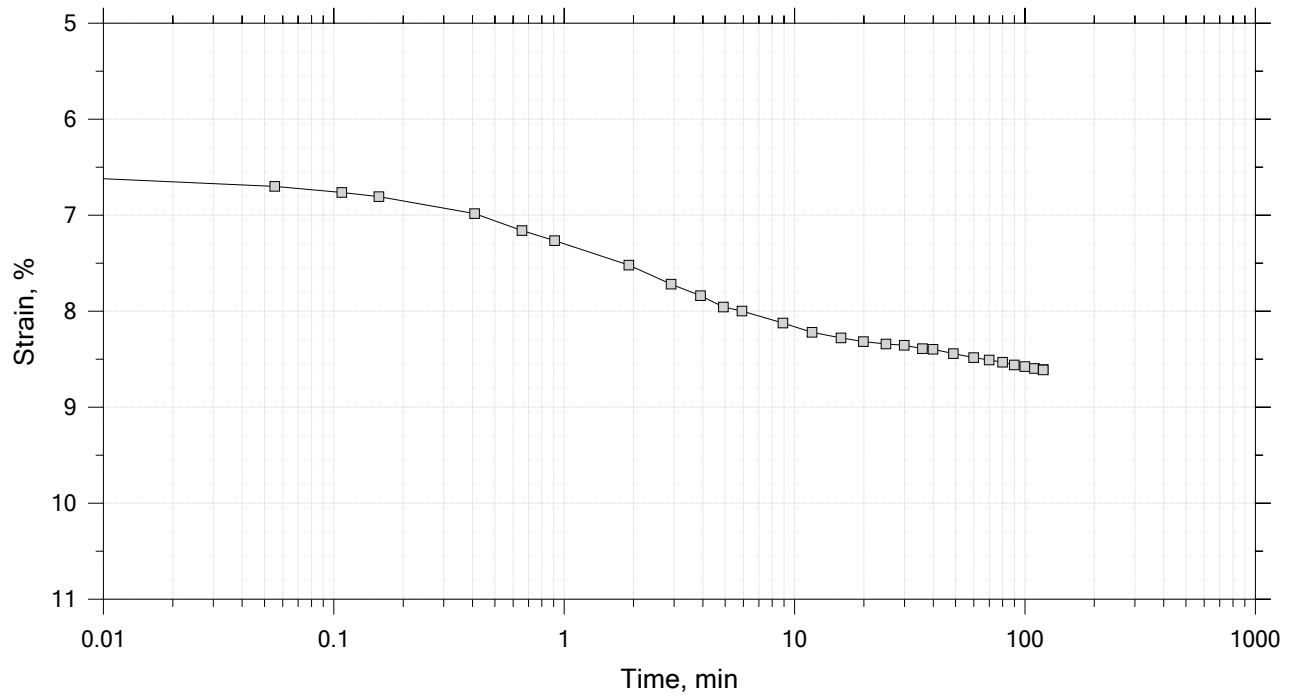
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



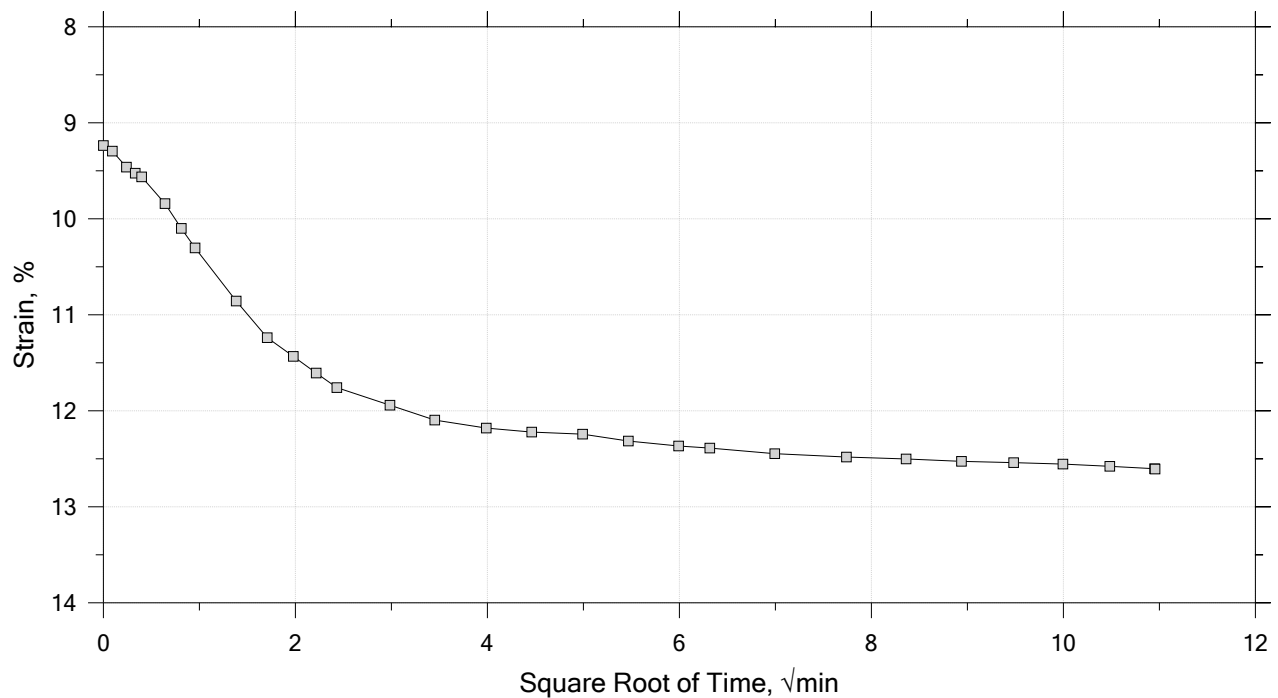
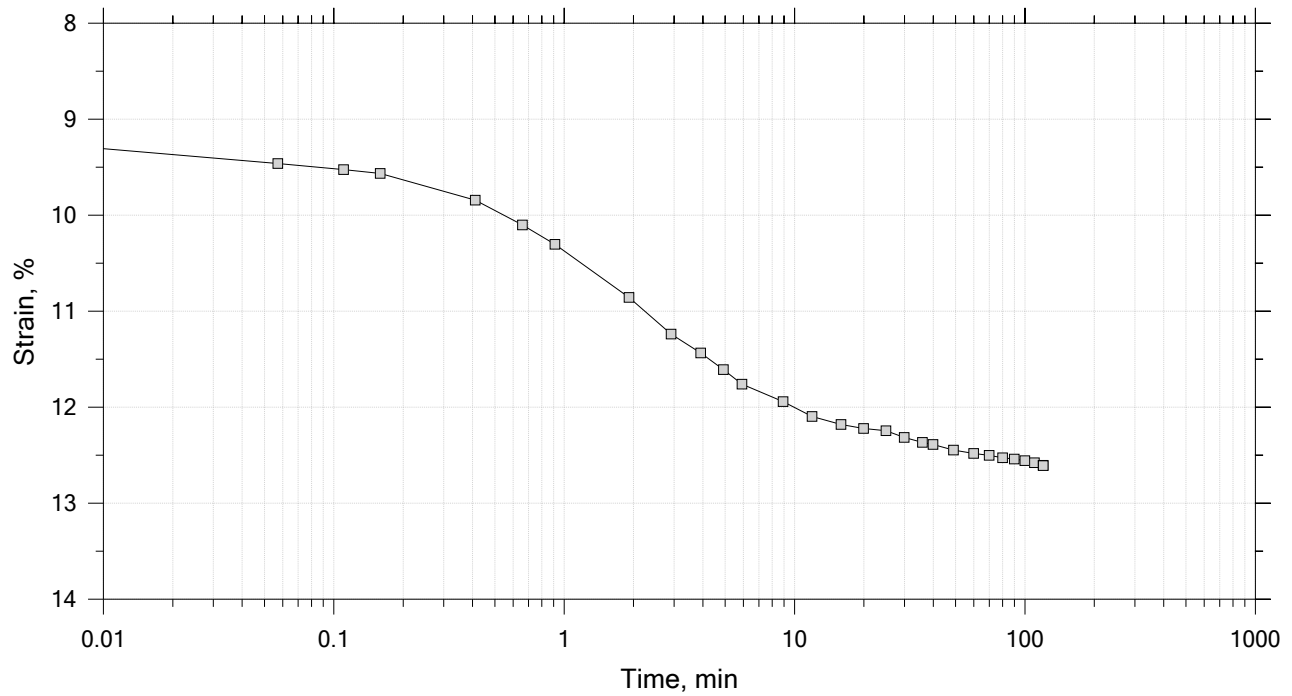
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



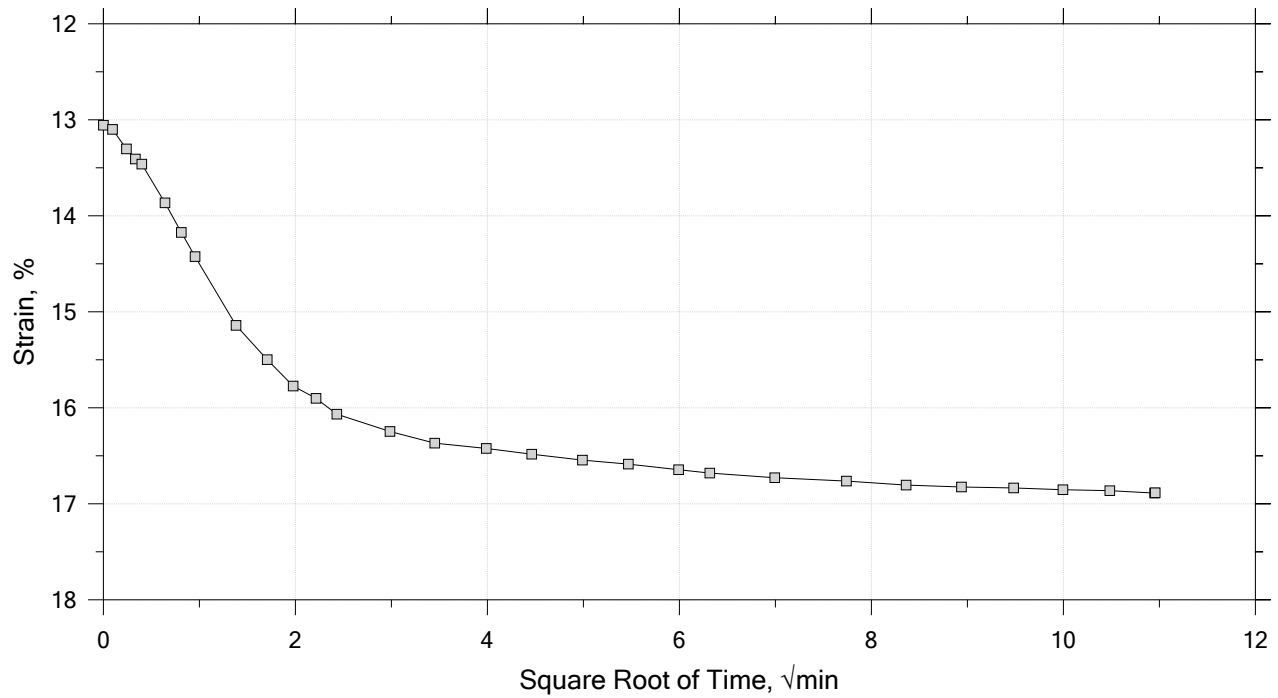
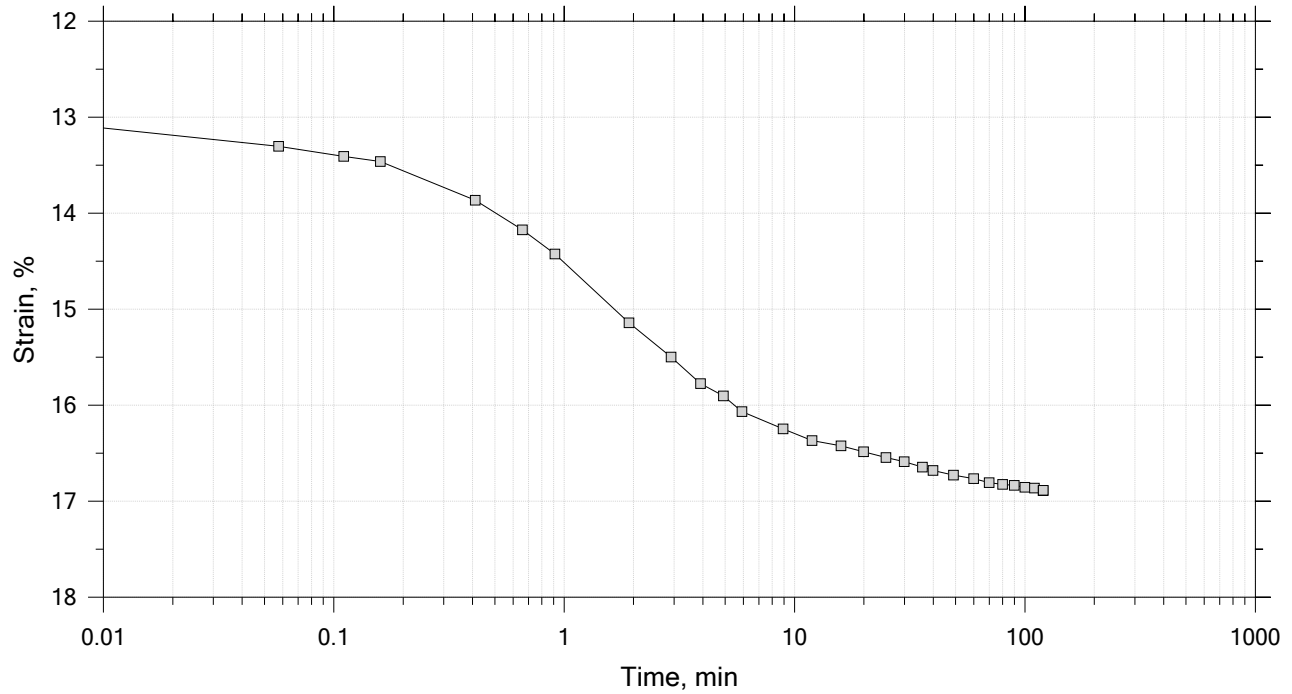
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



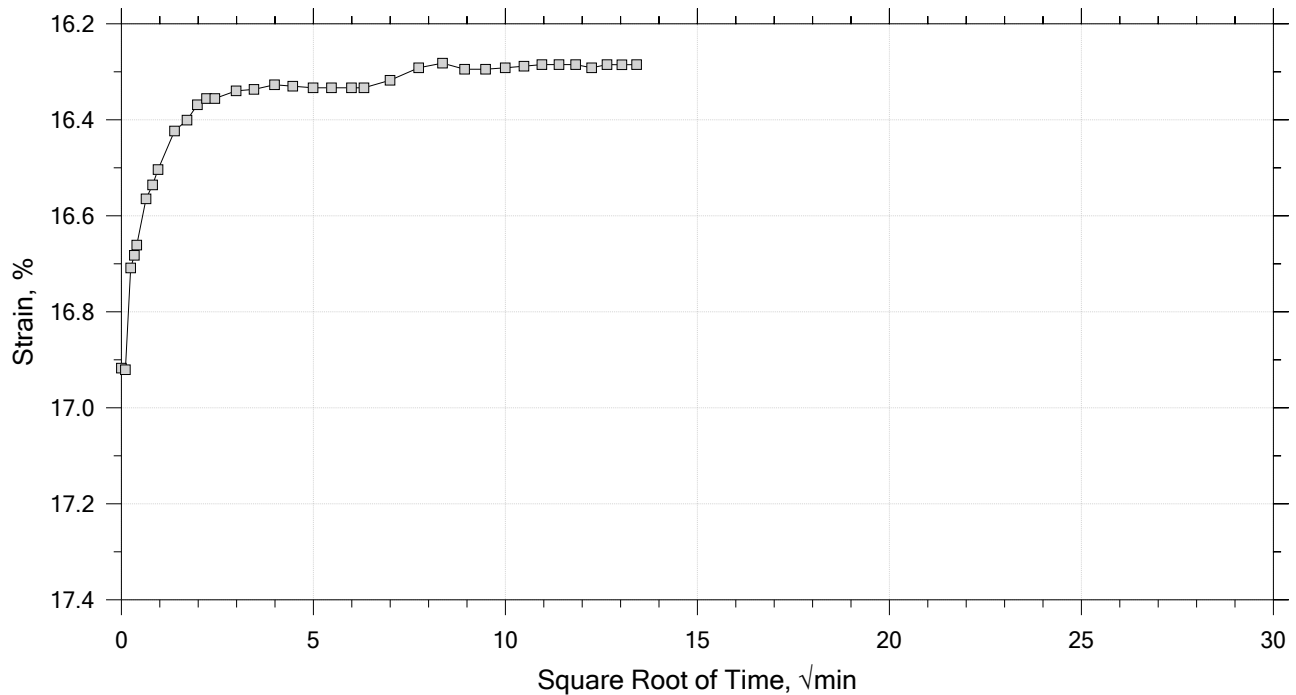
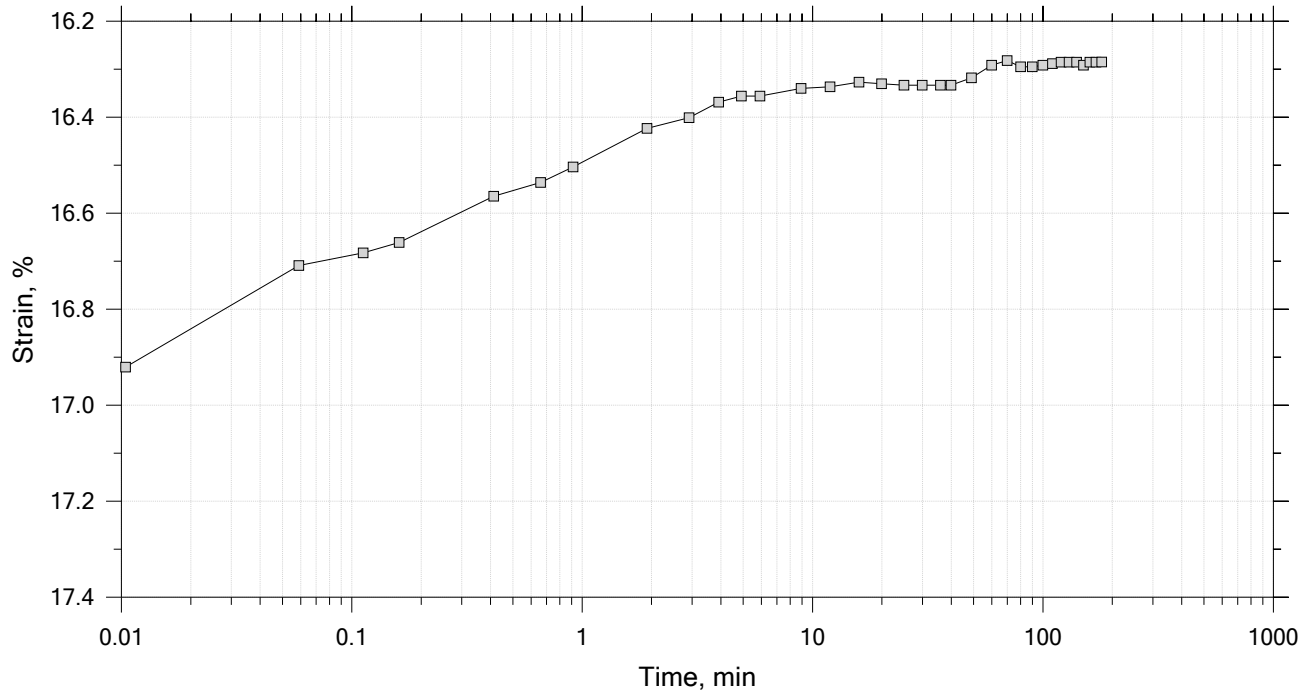
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



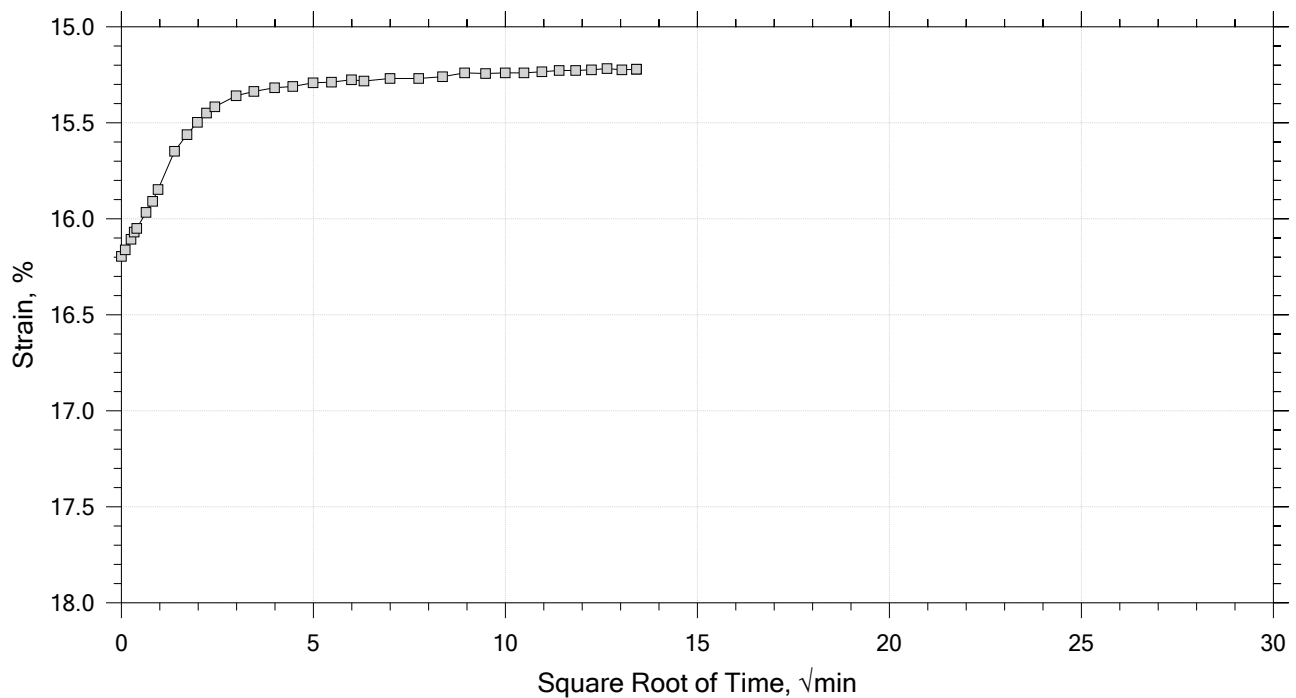
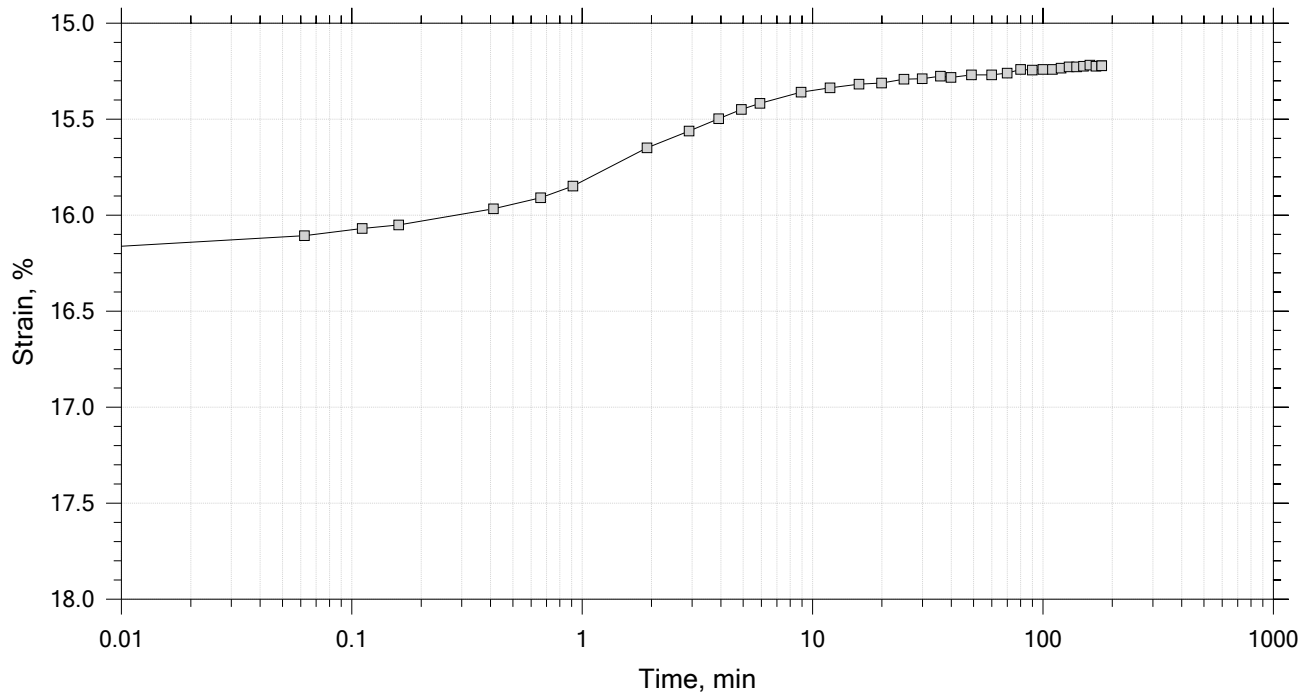
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



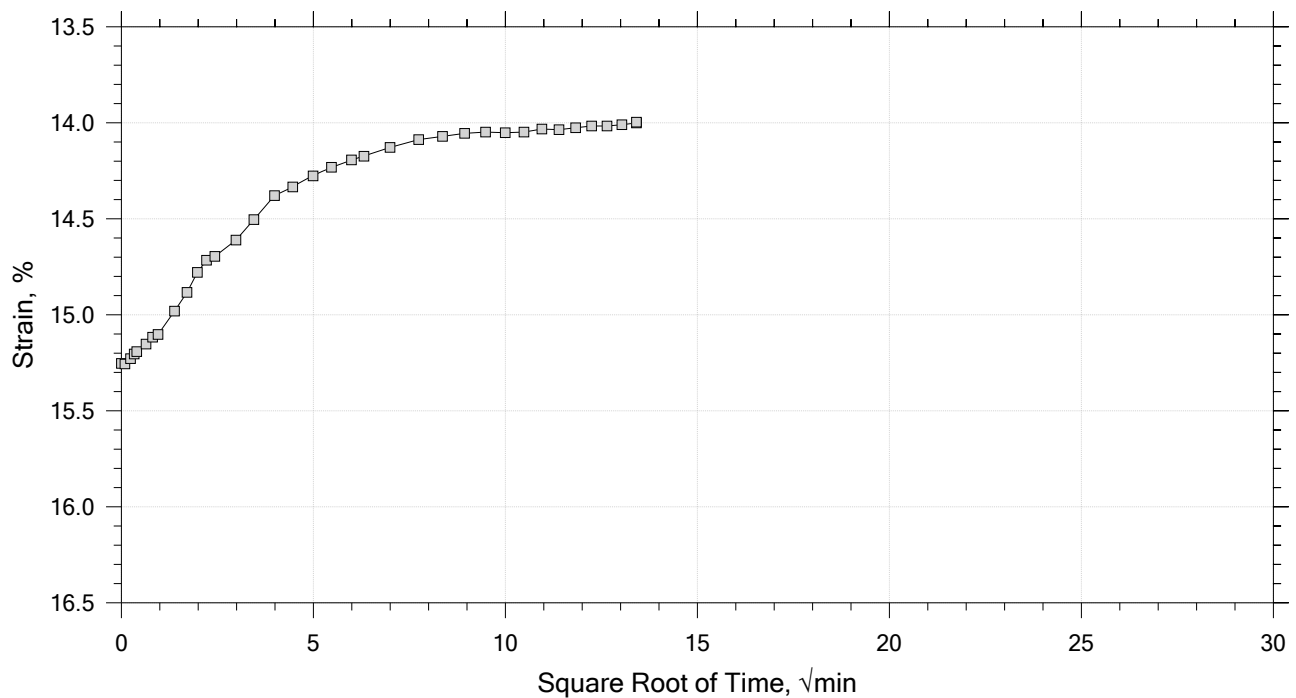
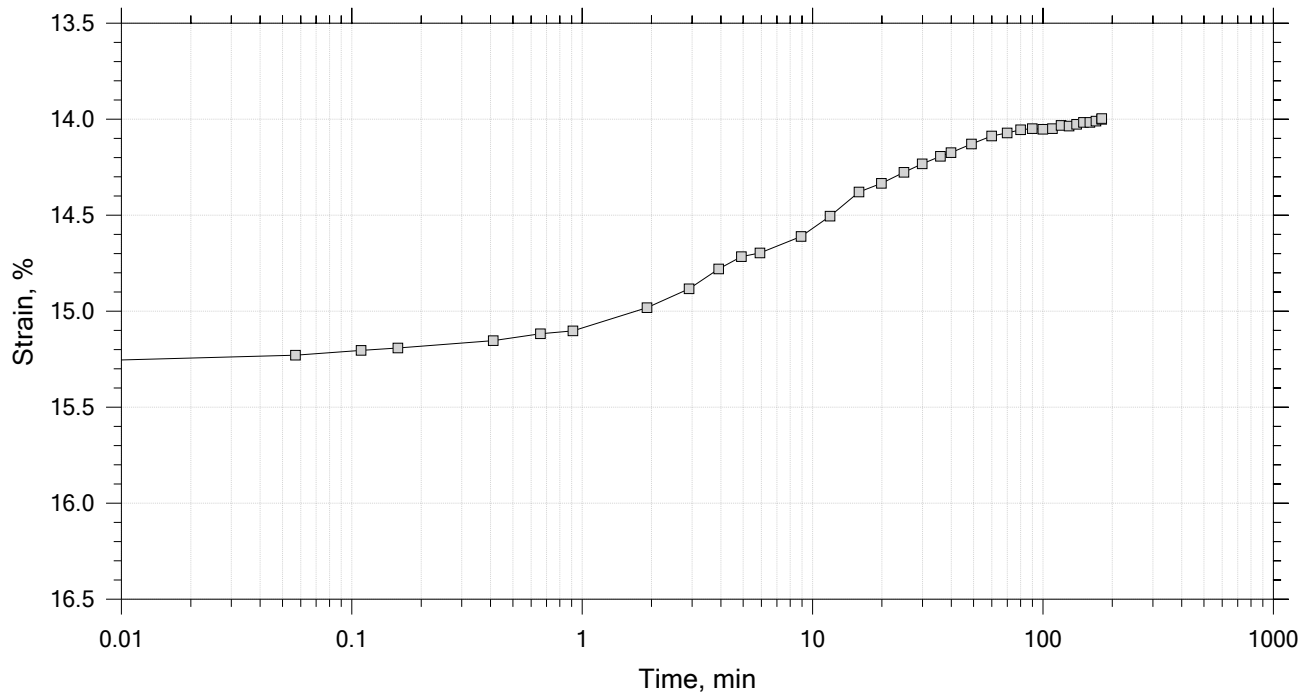
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	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



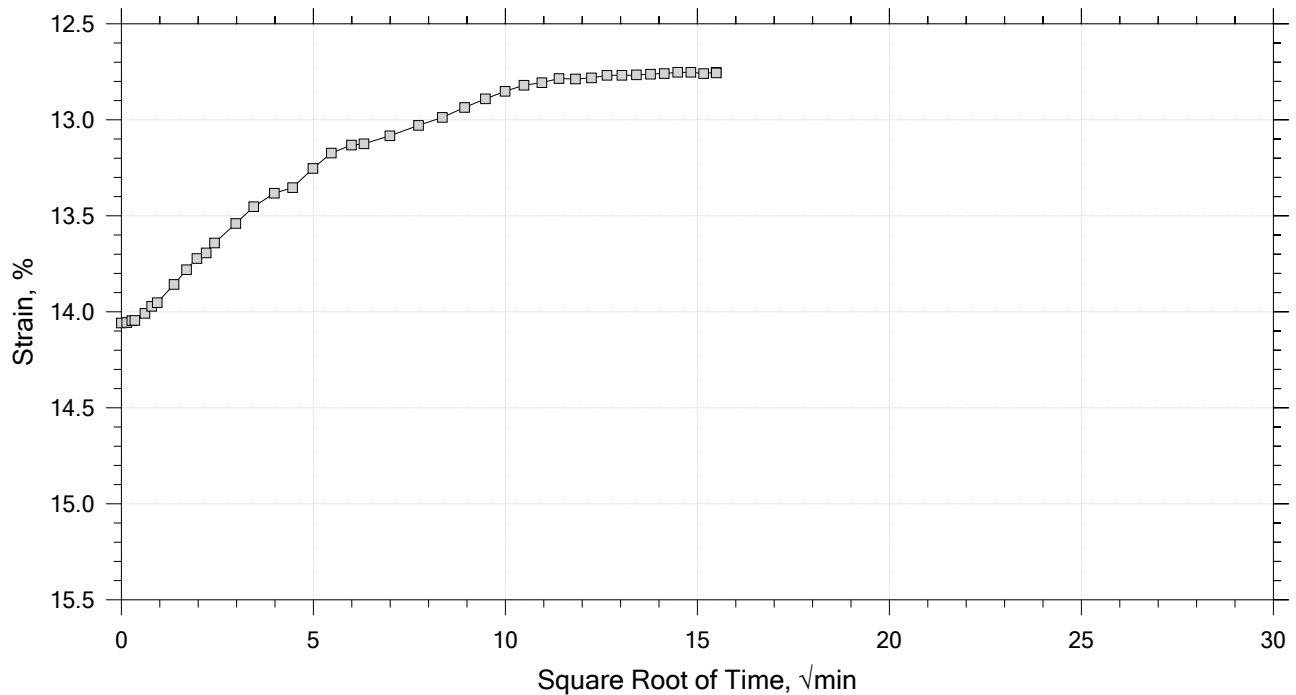
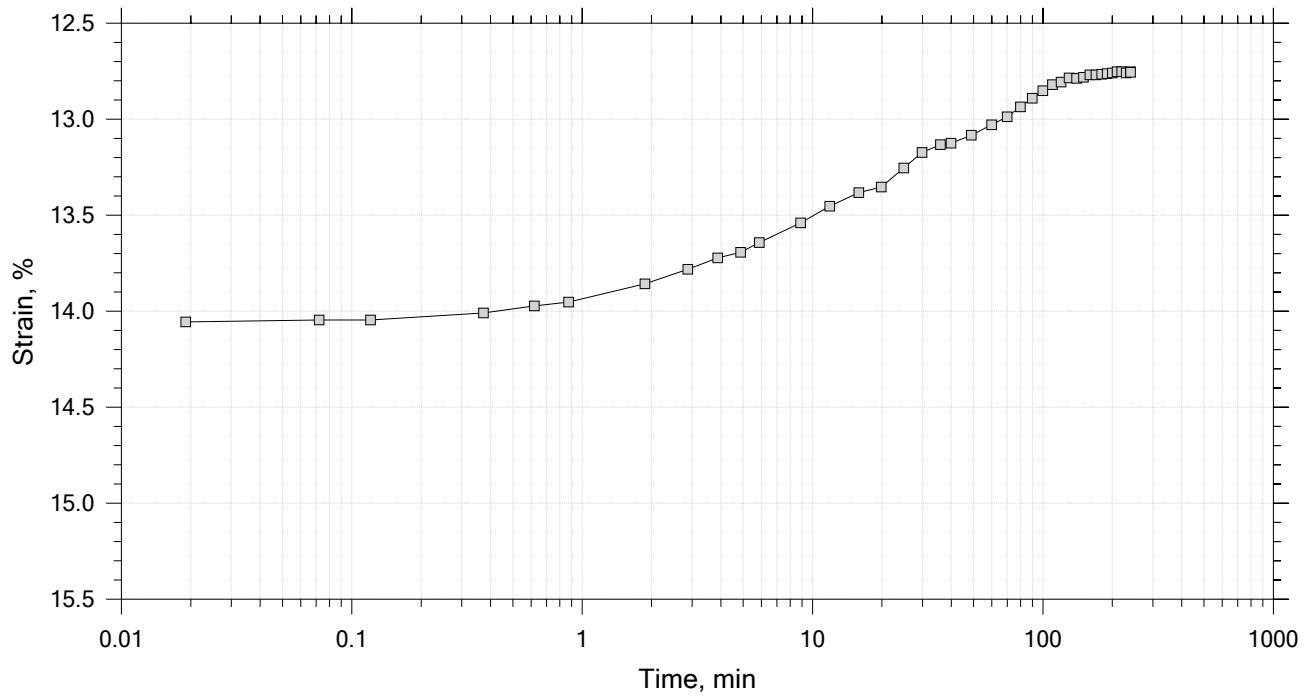
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



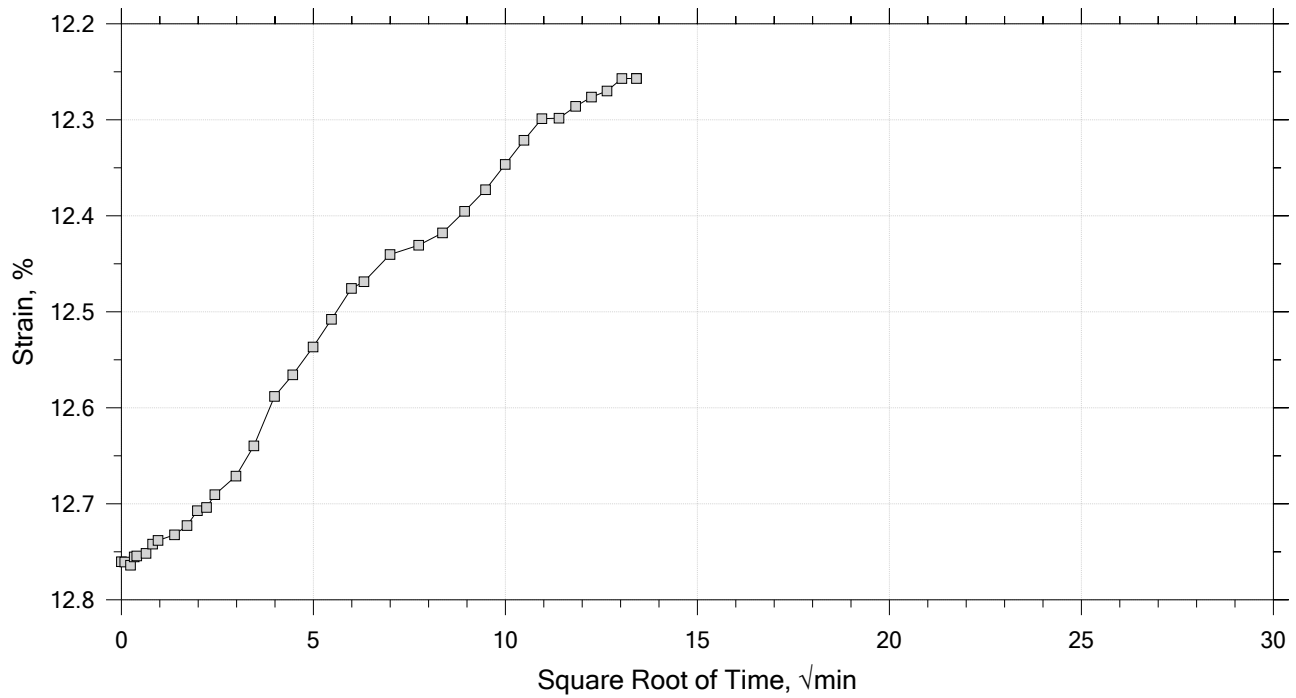
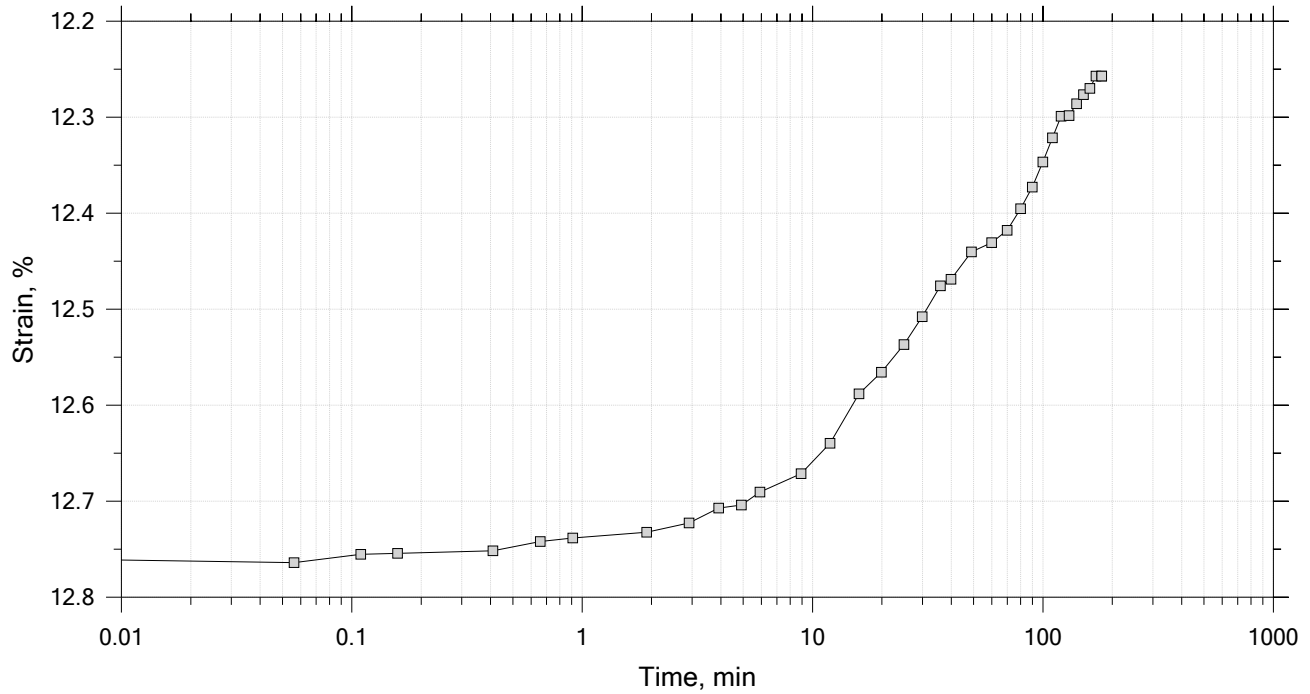
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.74	Liquid Limit: 31
Initial Height: 1.00 in	Initial Void Ratio: 0.86	Plastic Limit: 20
Final Height: 0.90 in	Final Void Ratio: 0.674	Plasticity Index: 11

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A1587	RING		D1276
Mass Container, gm	8.33	108.5	108.5	8.55
Mass Container + Wet Soil, gm	174.86	262.02	256.3	154.56
Mass Container + Dry Soil, gm	136.15	227.15	227.15	125.76
Mass Dry Soil, gm	127.82	118.65	118.65	117.21
Water Content, %	30.28	29.39	24.57	24.57
Void Ratio	---	0.86	0.67	---
Degree of Saturation, %	---	93.75	100.00	---
Dry Unit Weight, pcf	---	92.08	102.31	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.071 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIIB-B, Swell Pressure = 0.071 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

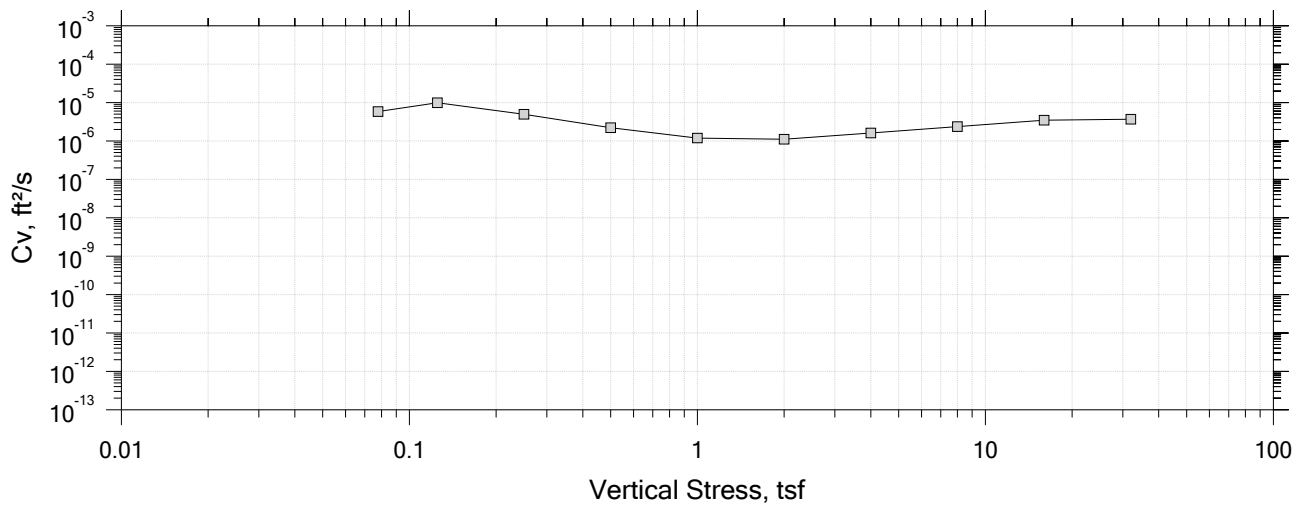
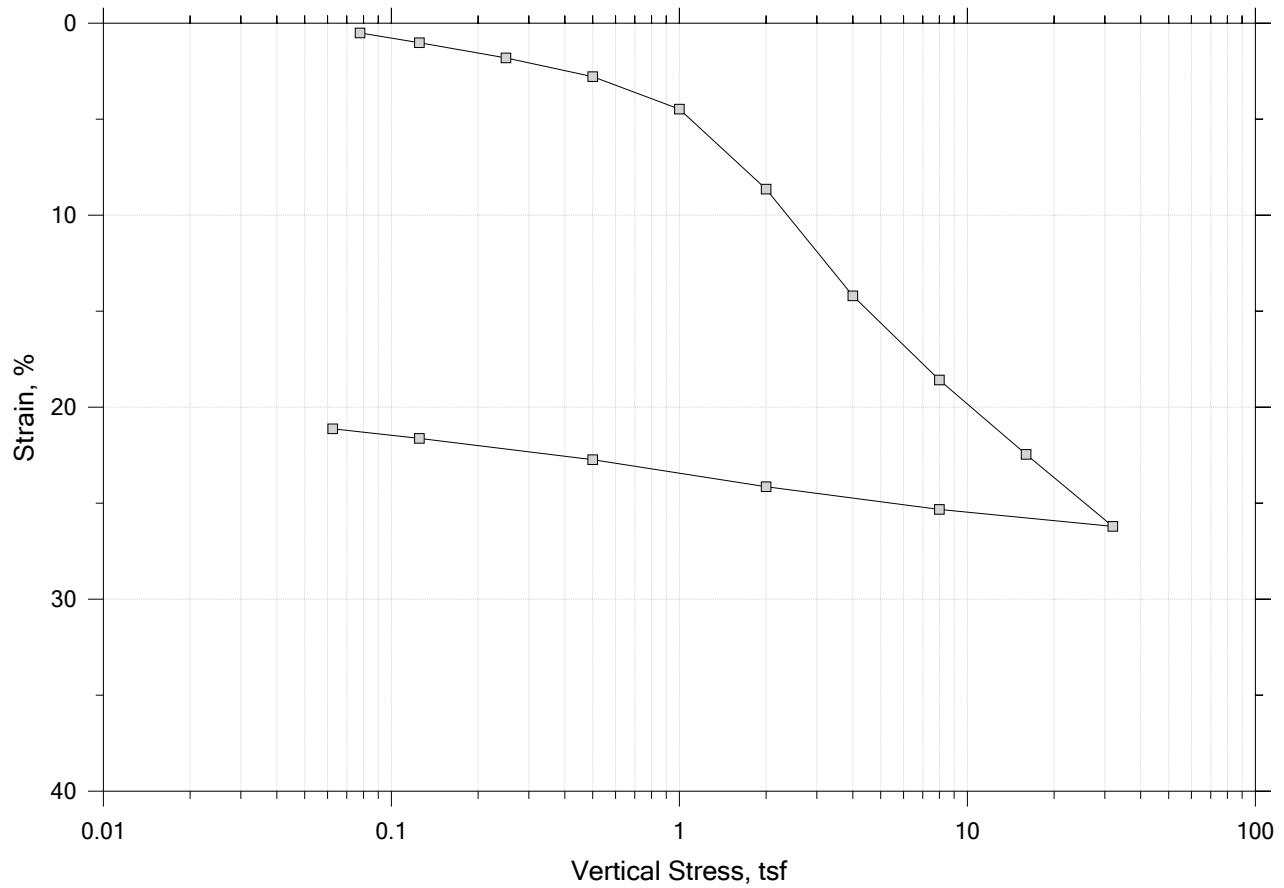
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-405 HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-6	Sample Type: intact	Elevation: ---
	Description: Moist, olive gray clay		
	Remarks: System LTIIB-B, Swell Pressure = 0.071 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

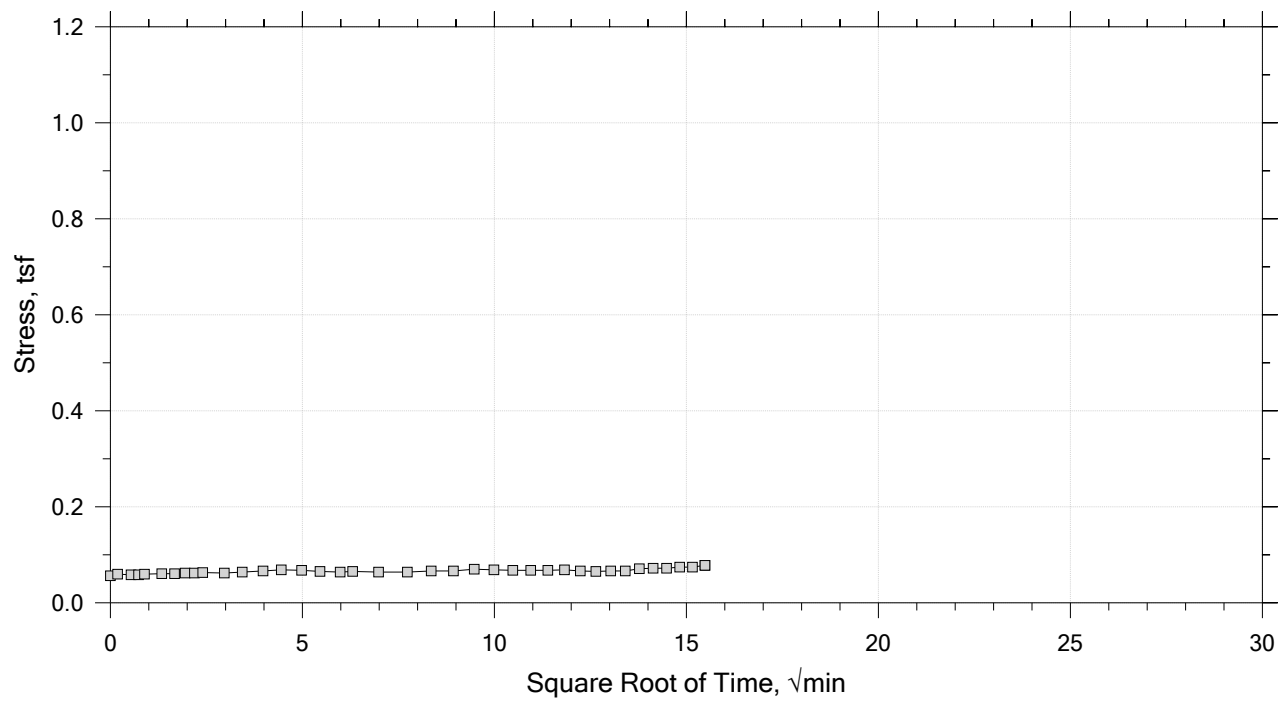
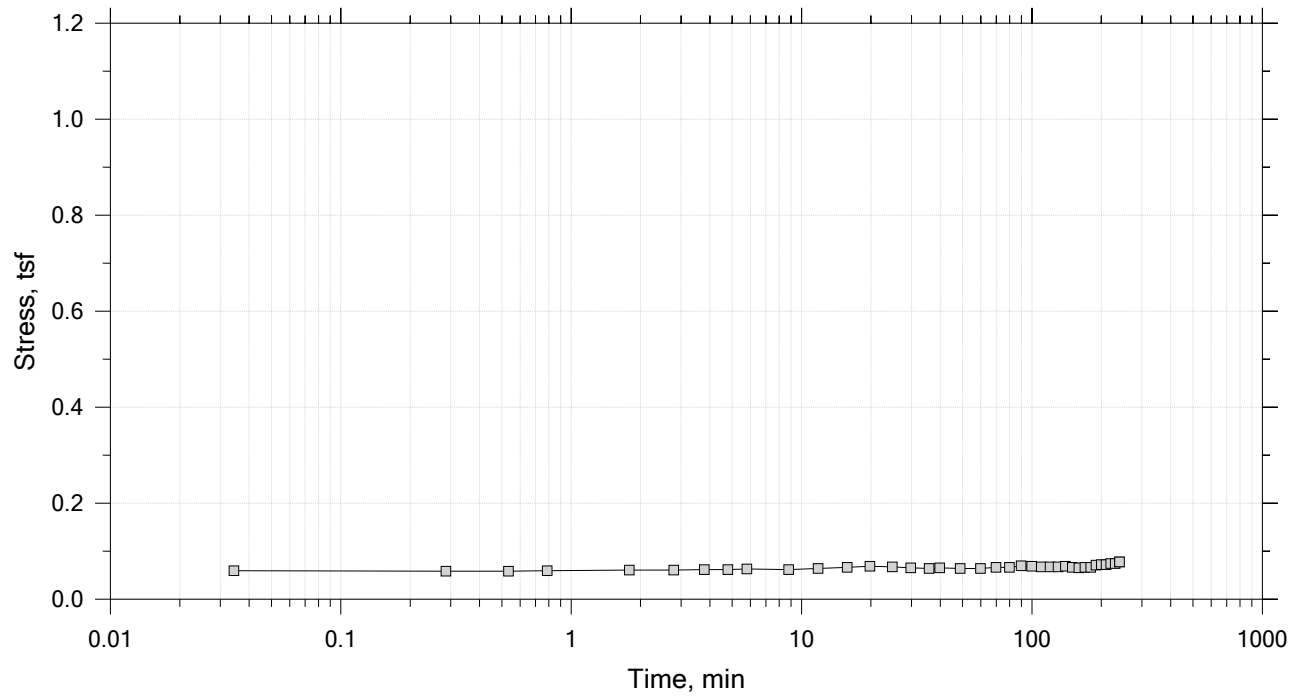
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0778 tsf



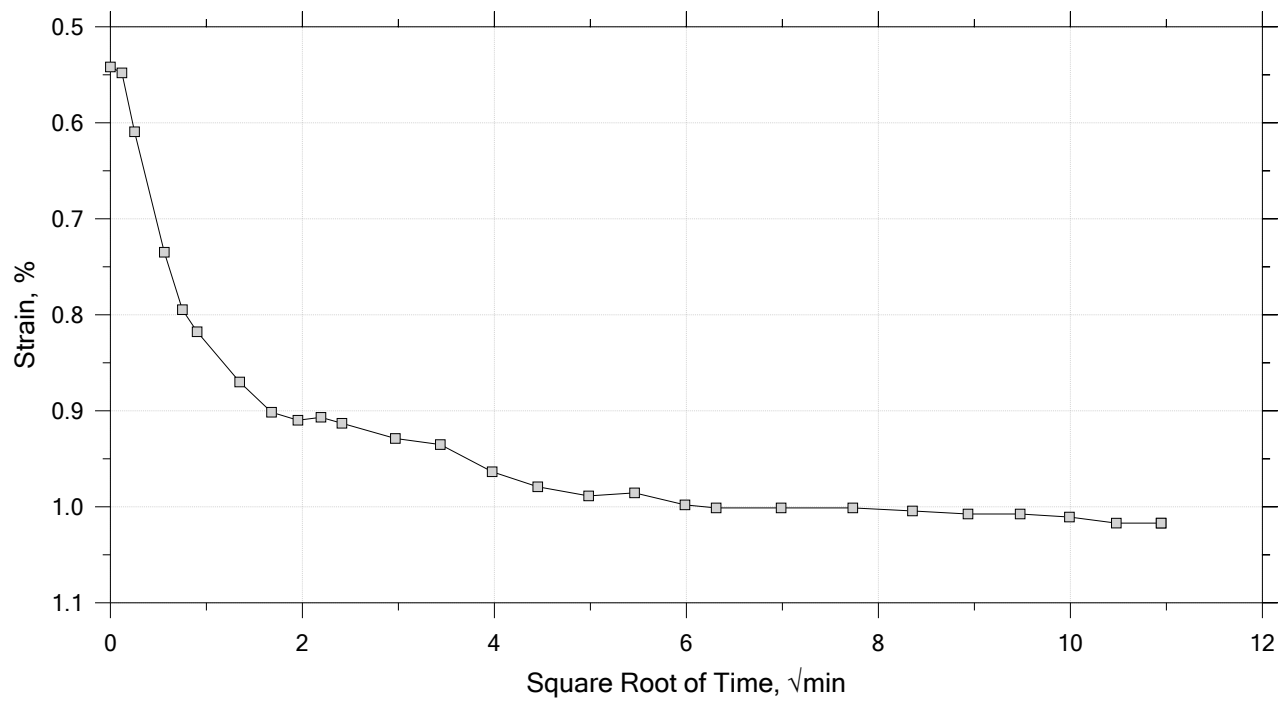
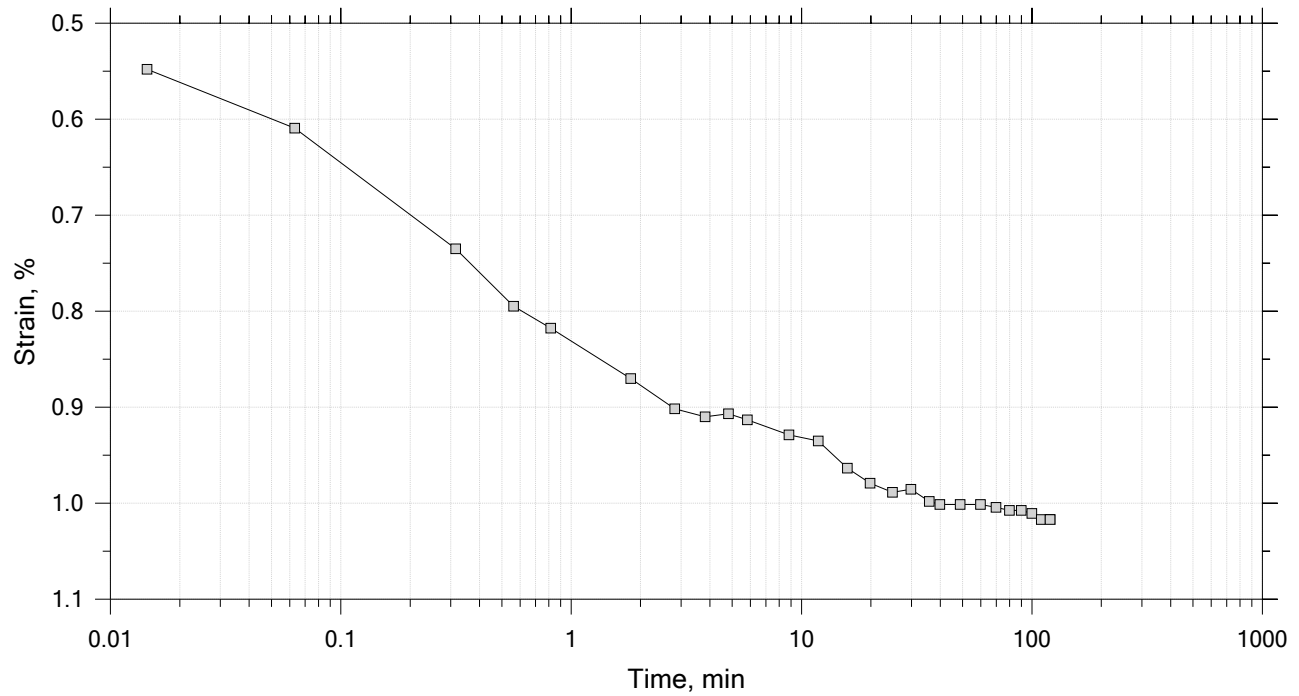
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



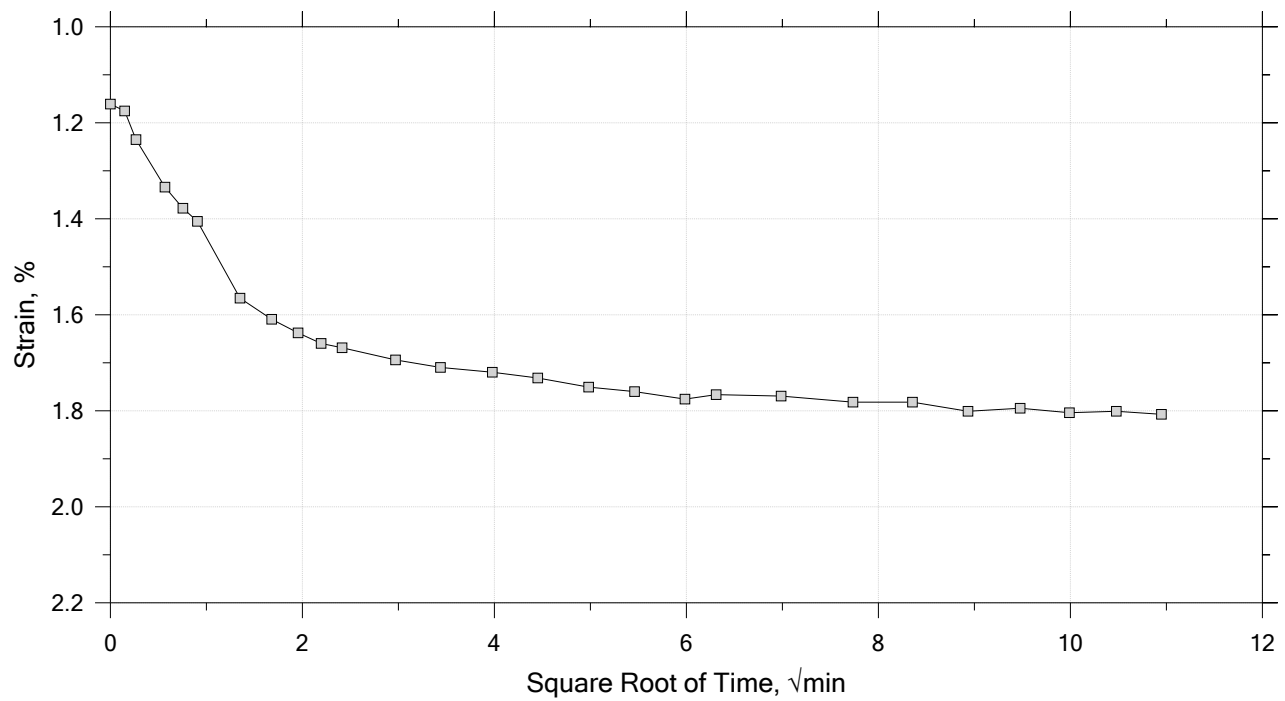
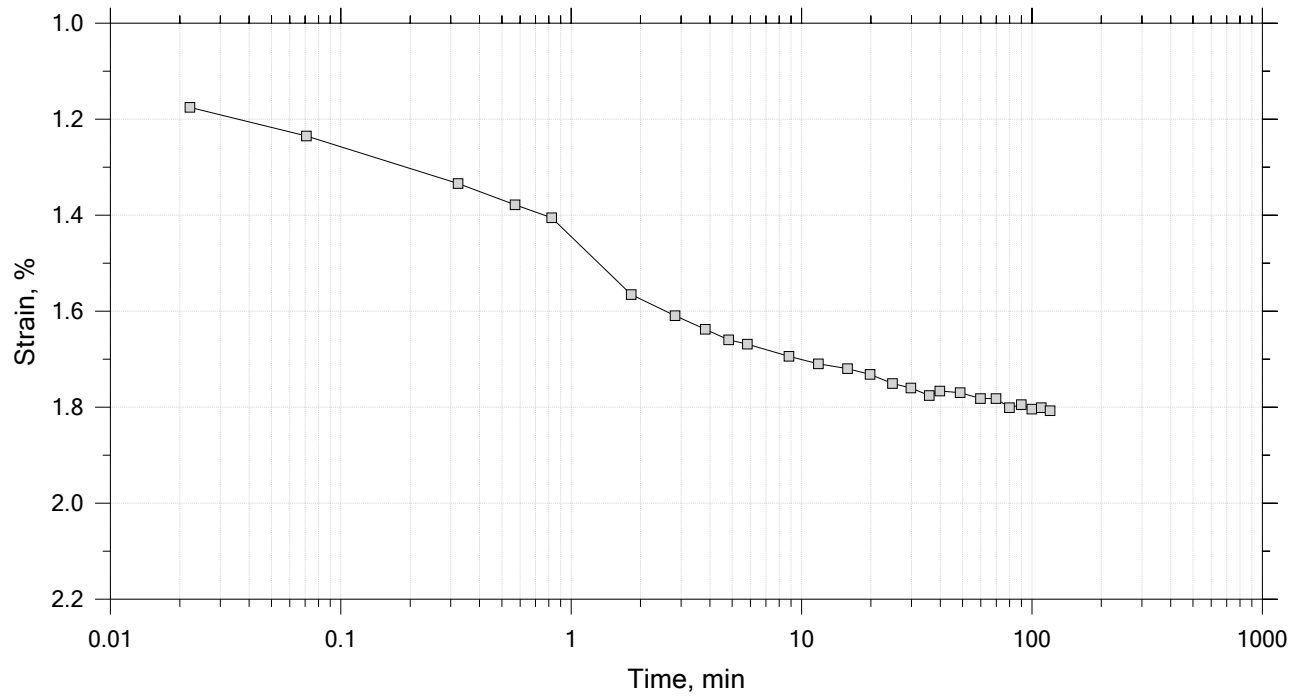
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



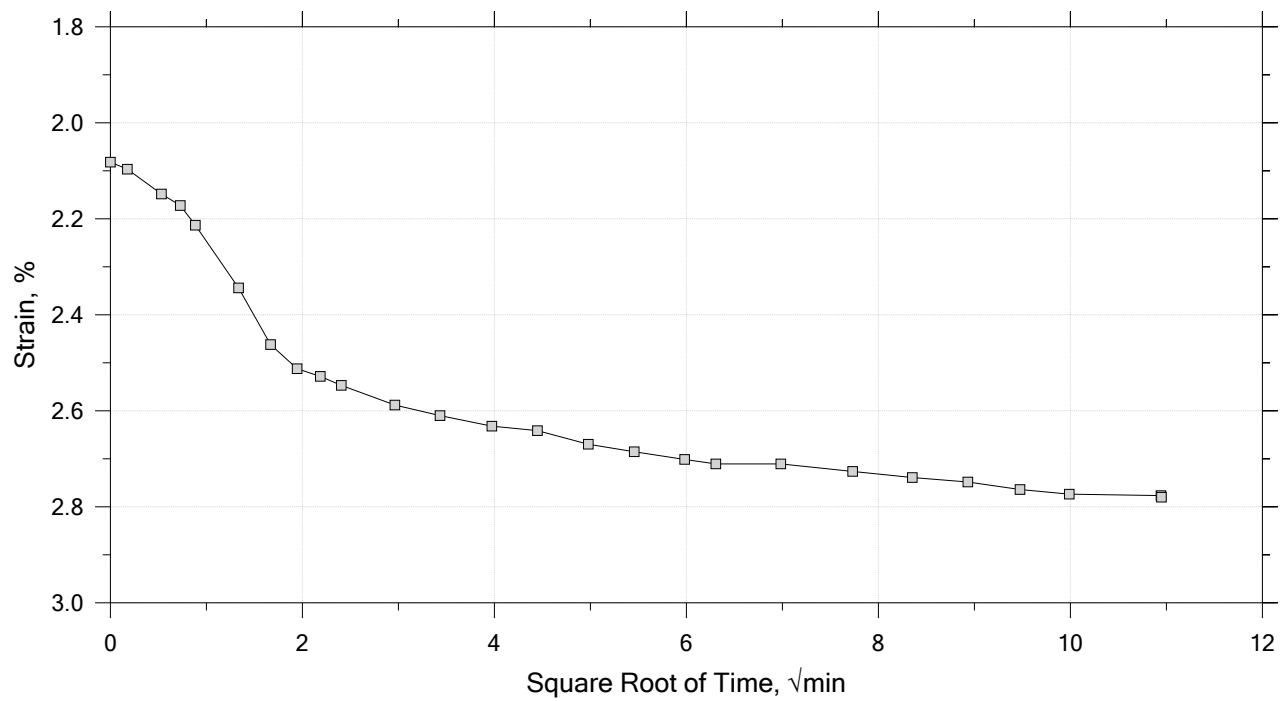
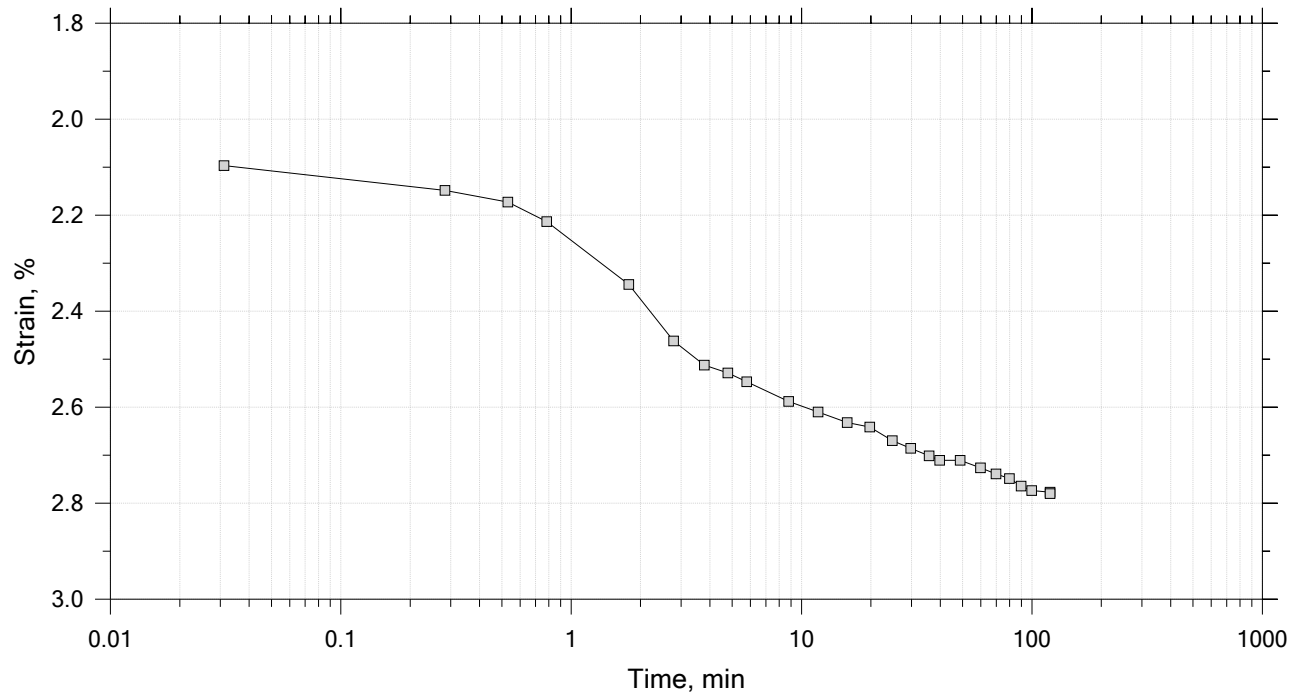
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



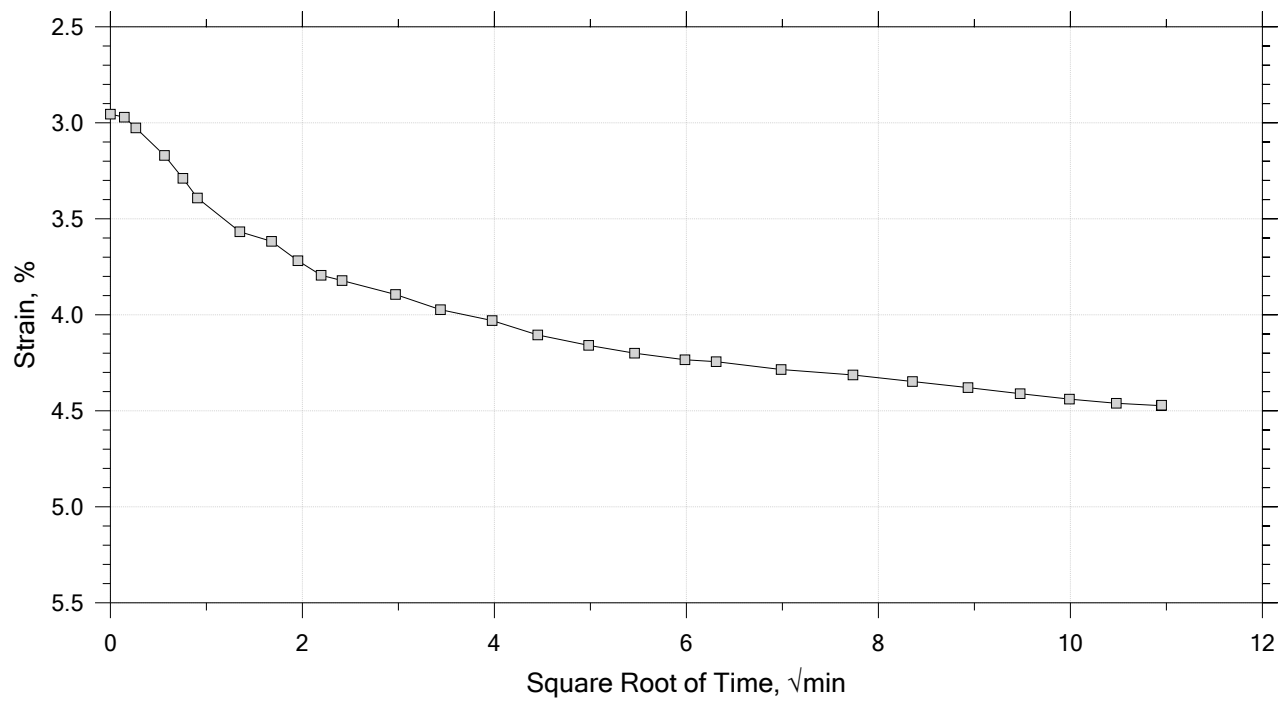
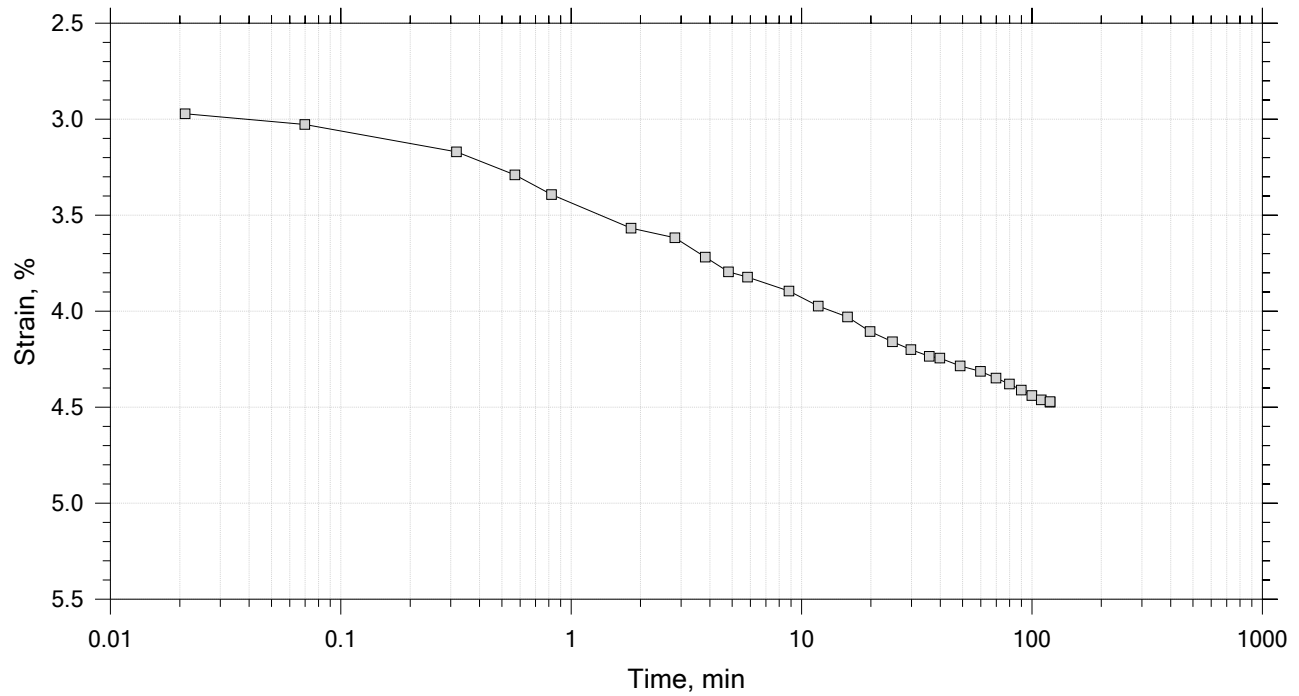
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	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



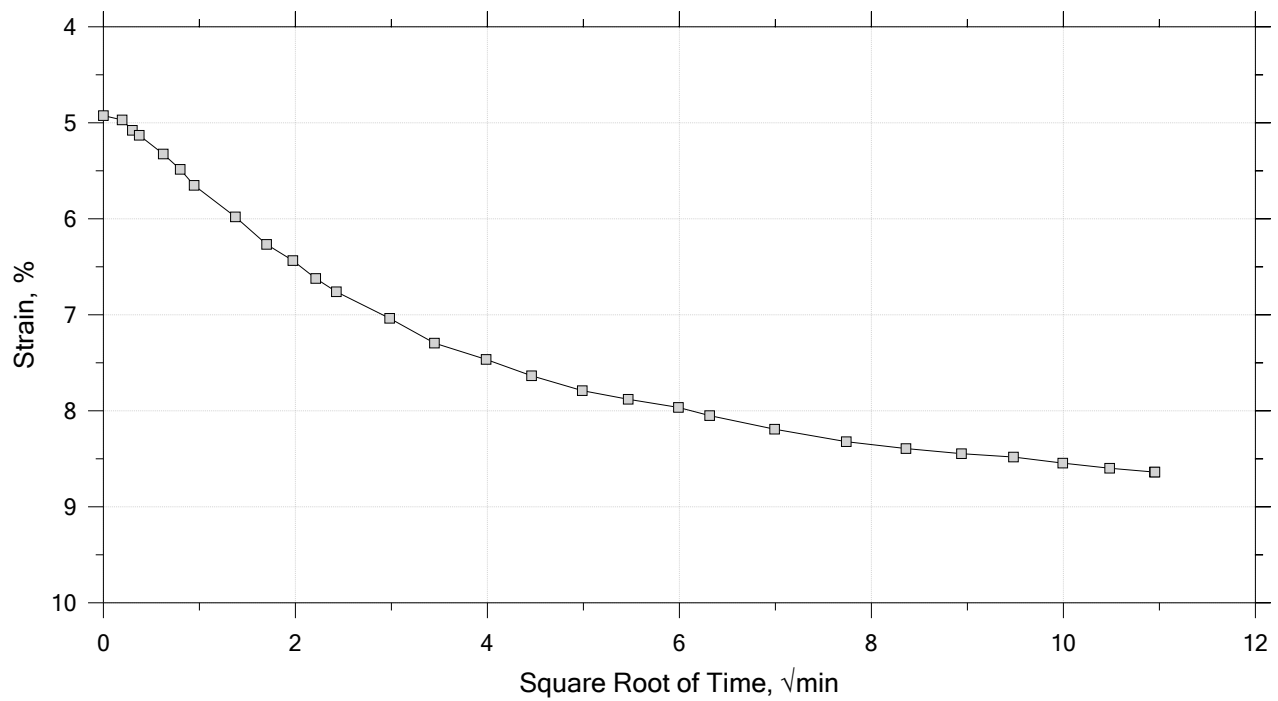
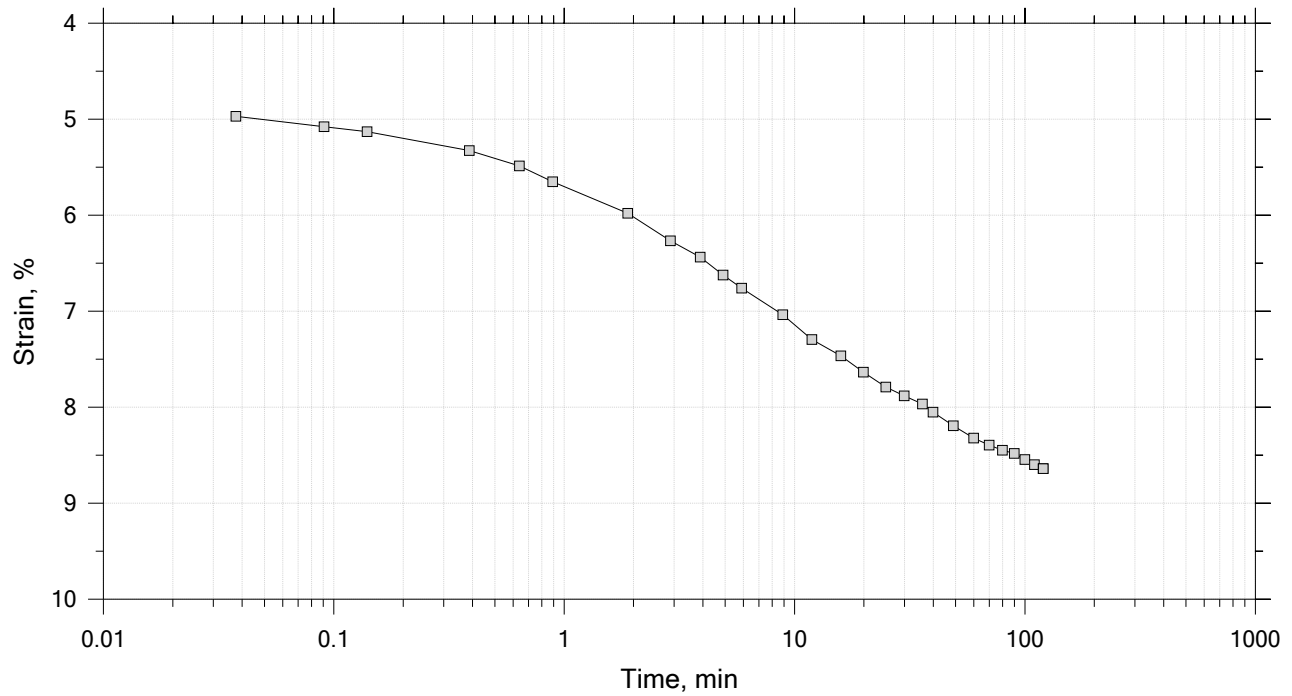
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	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



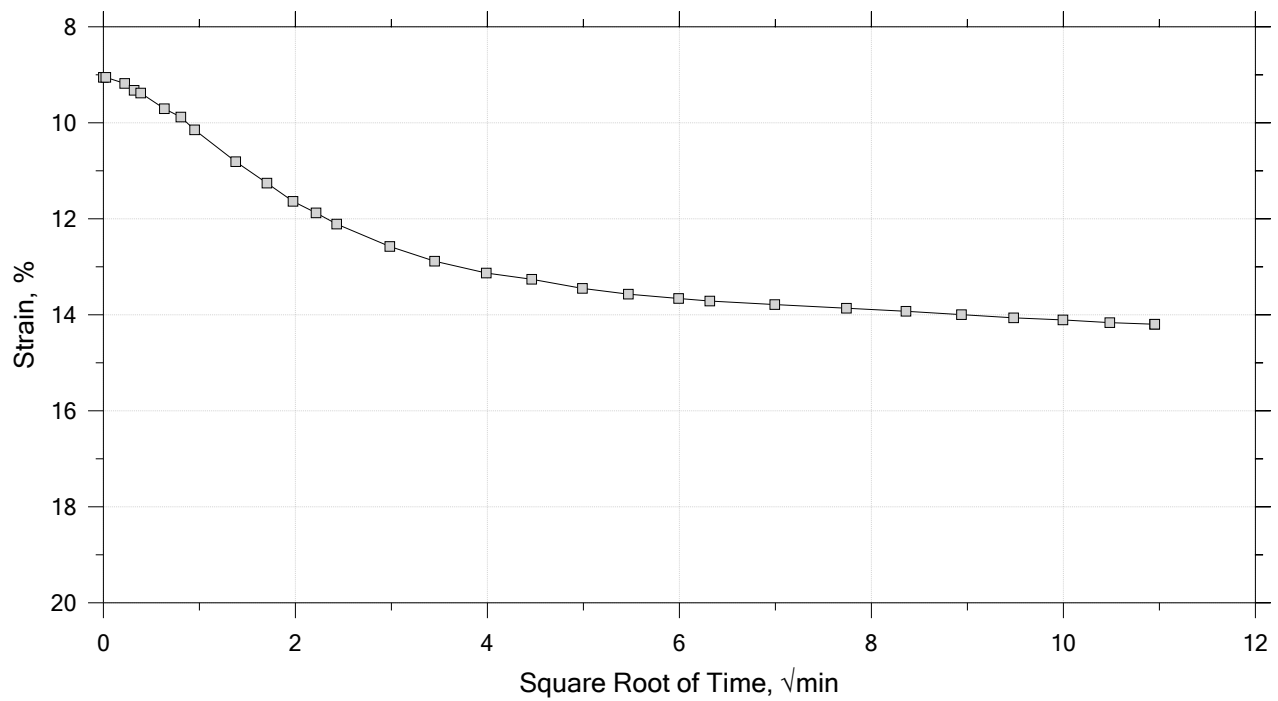
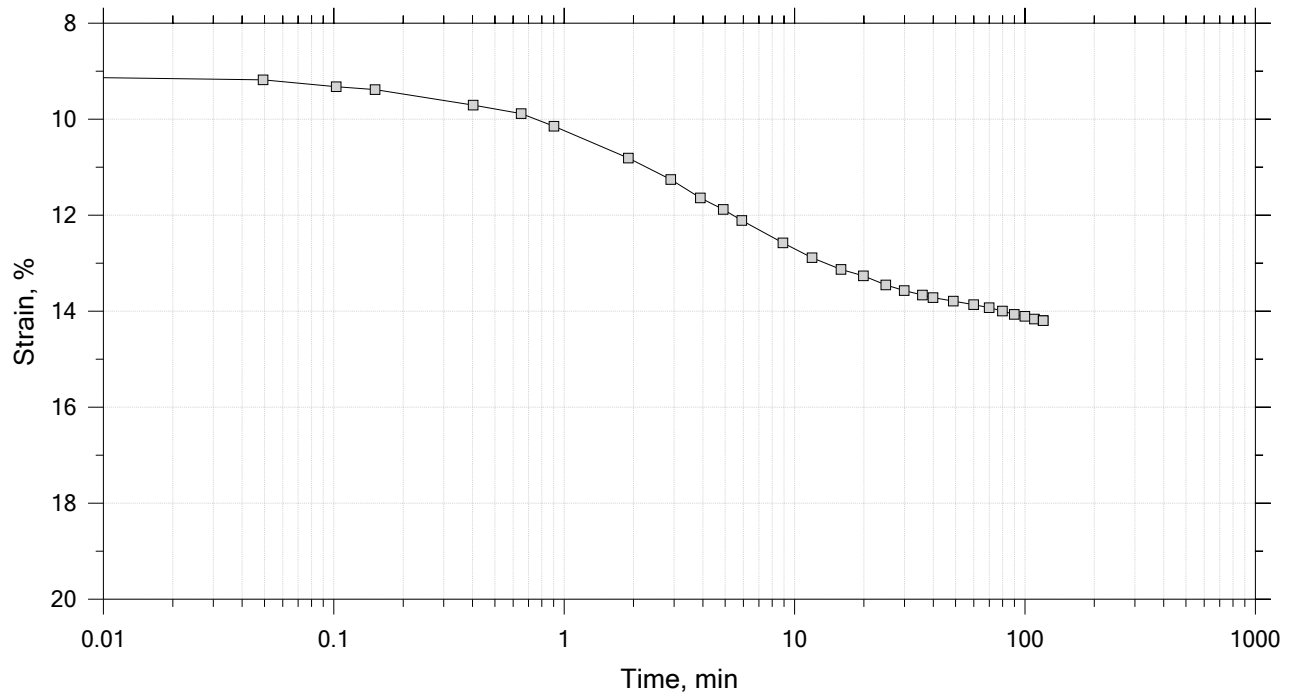
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	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



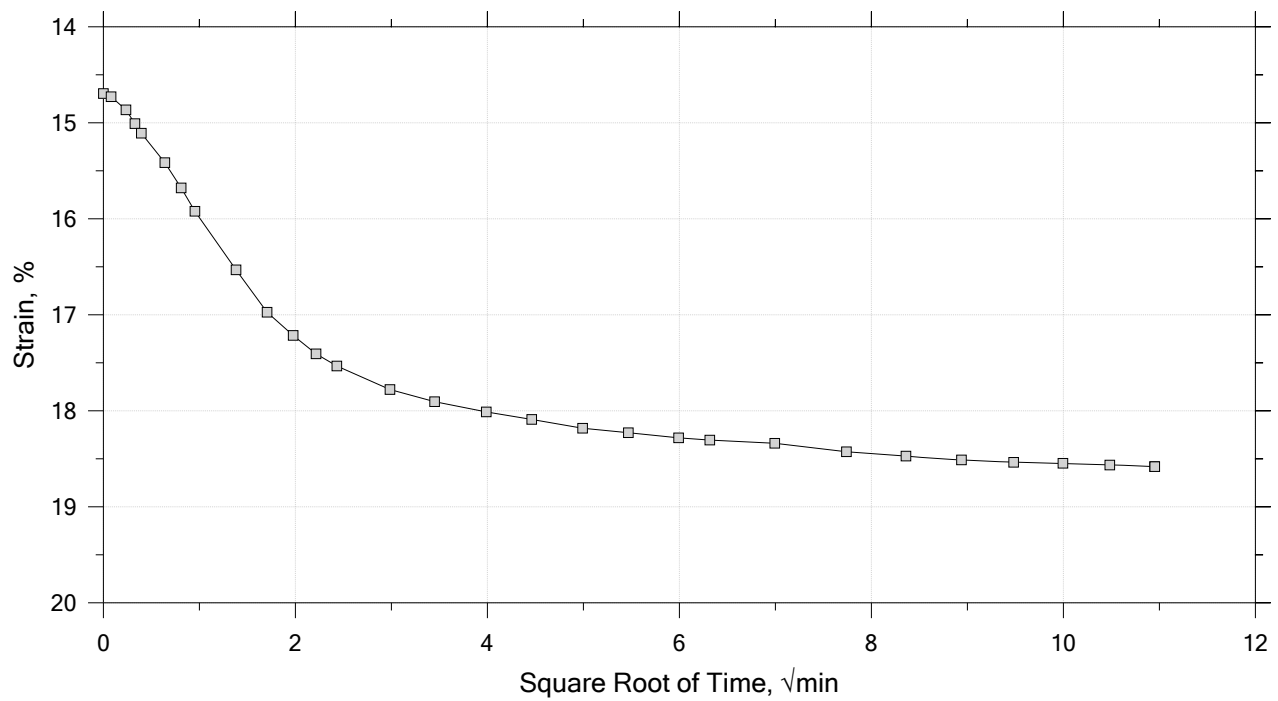
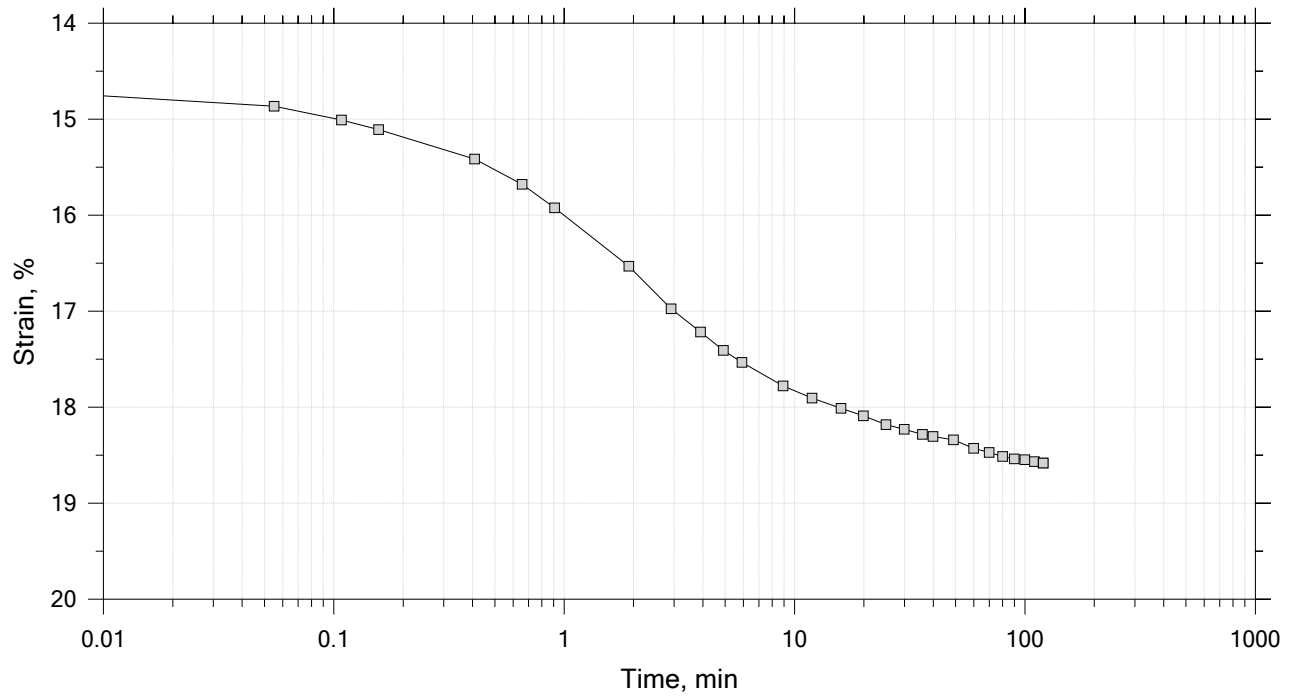
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



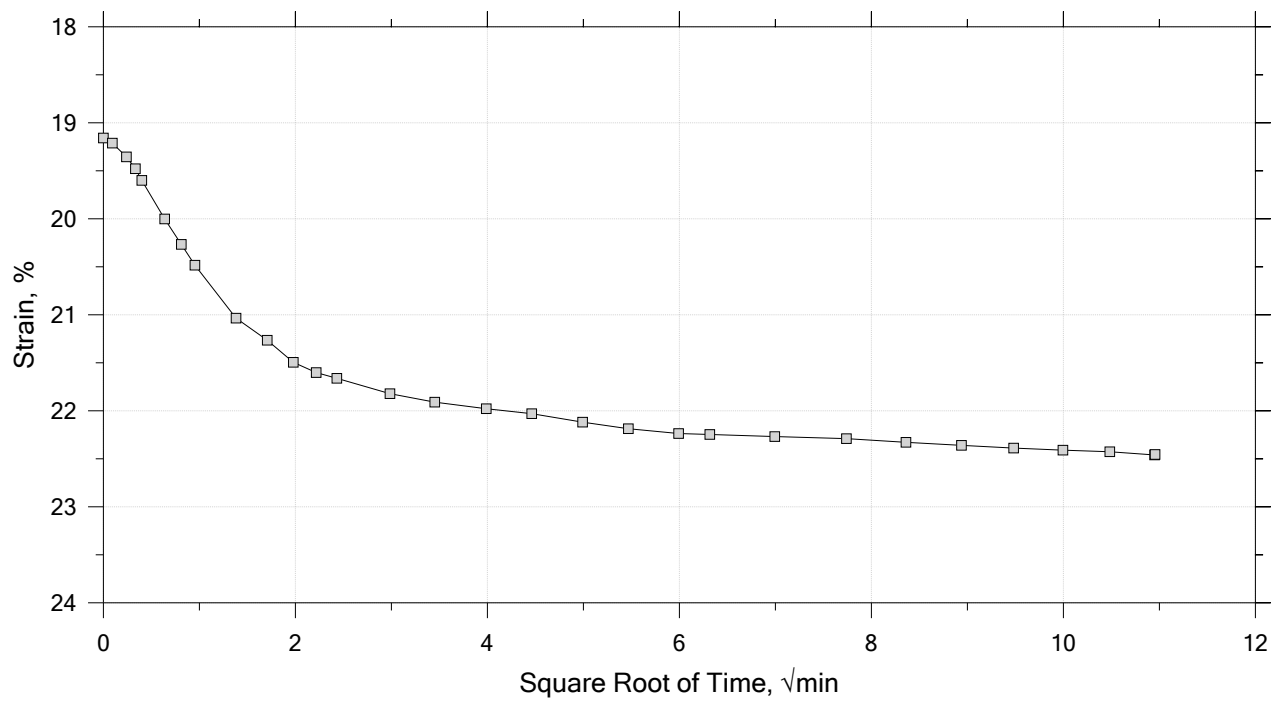
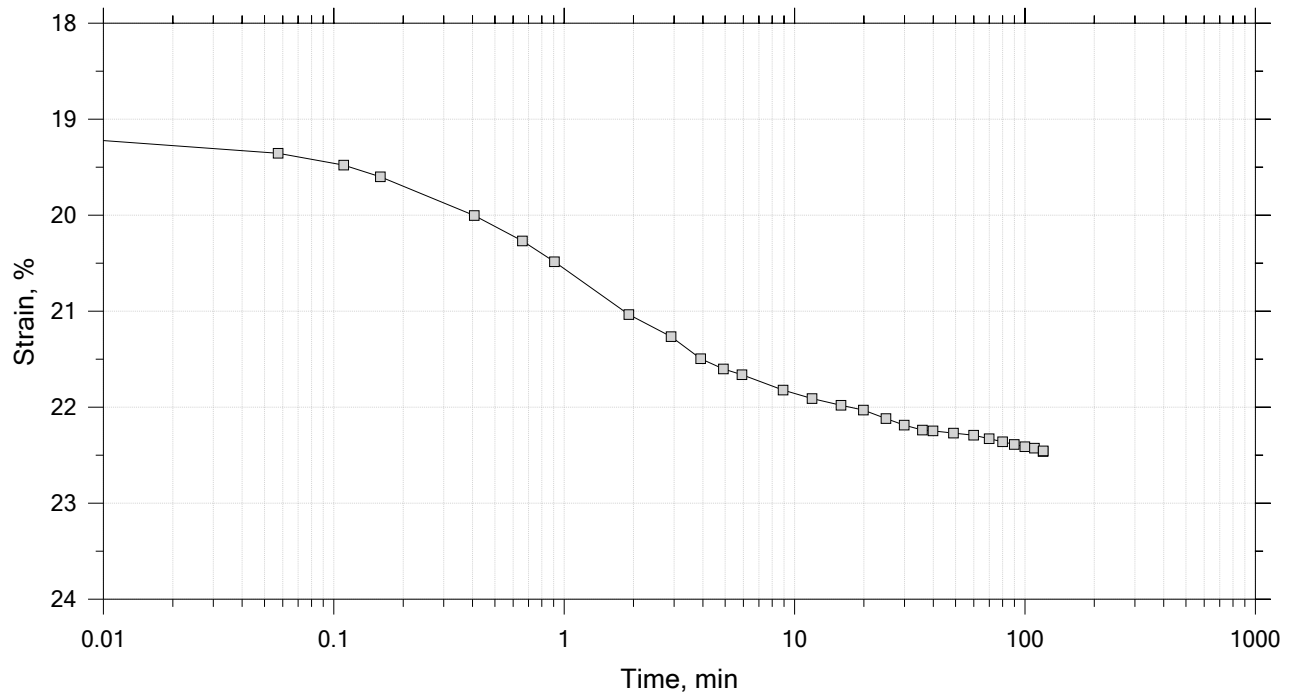
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



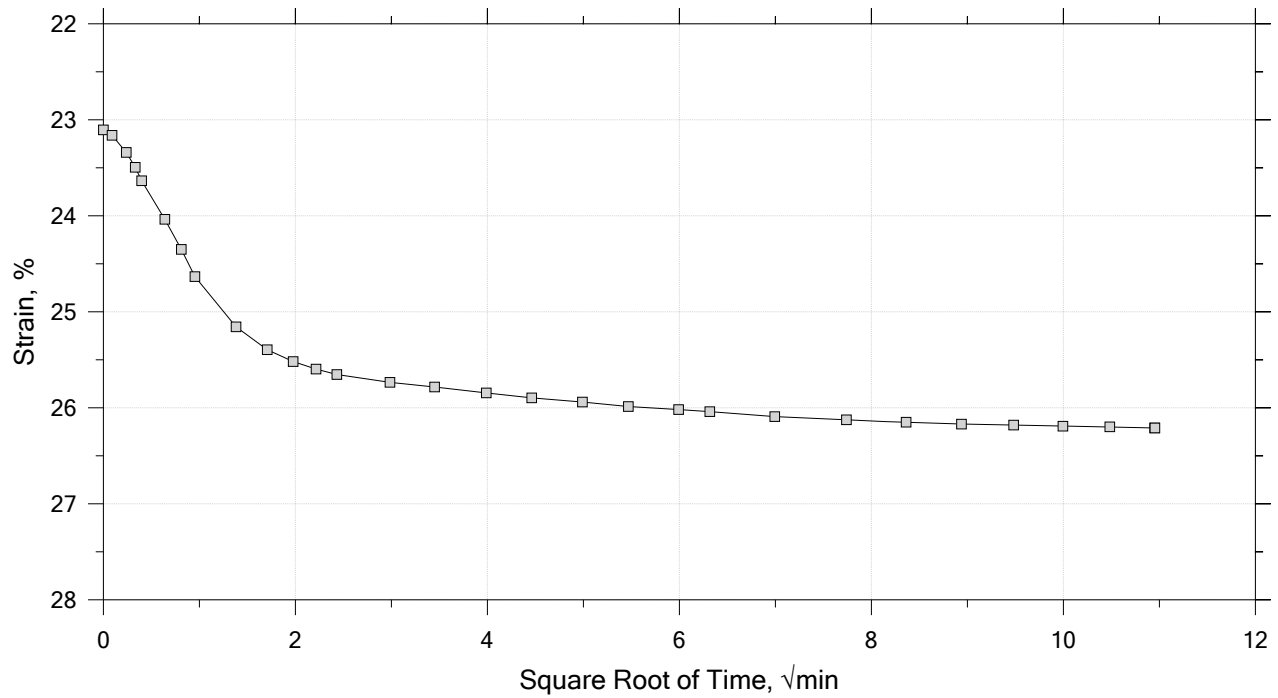
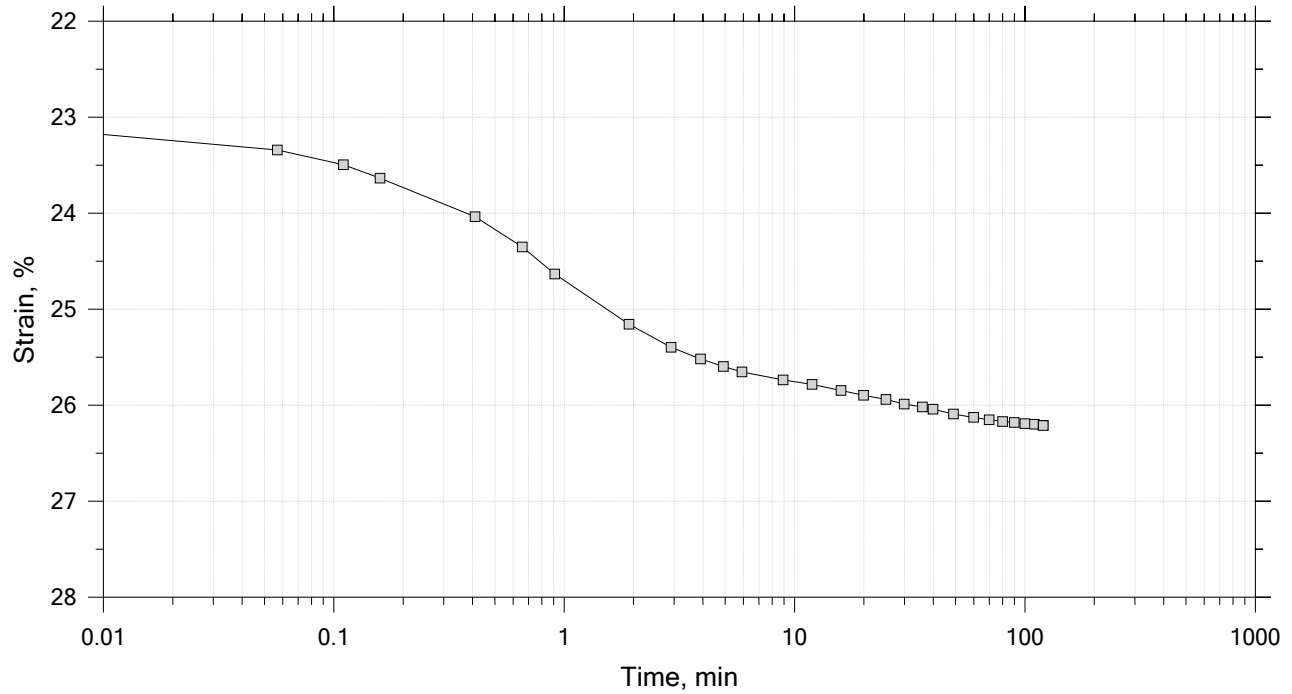
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



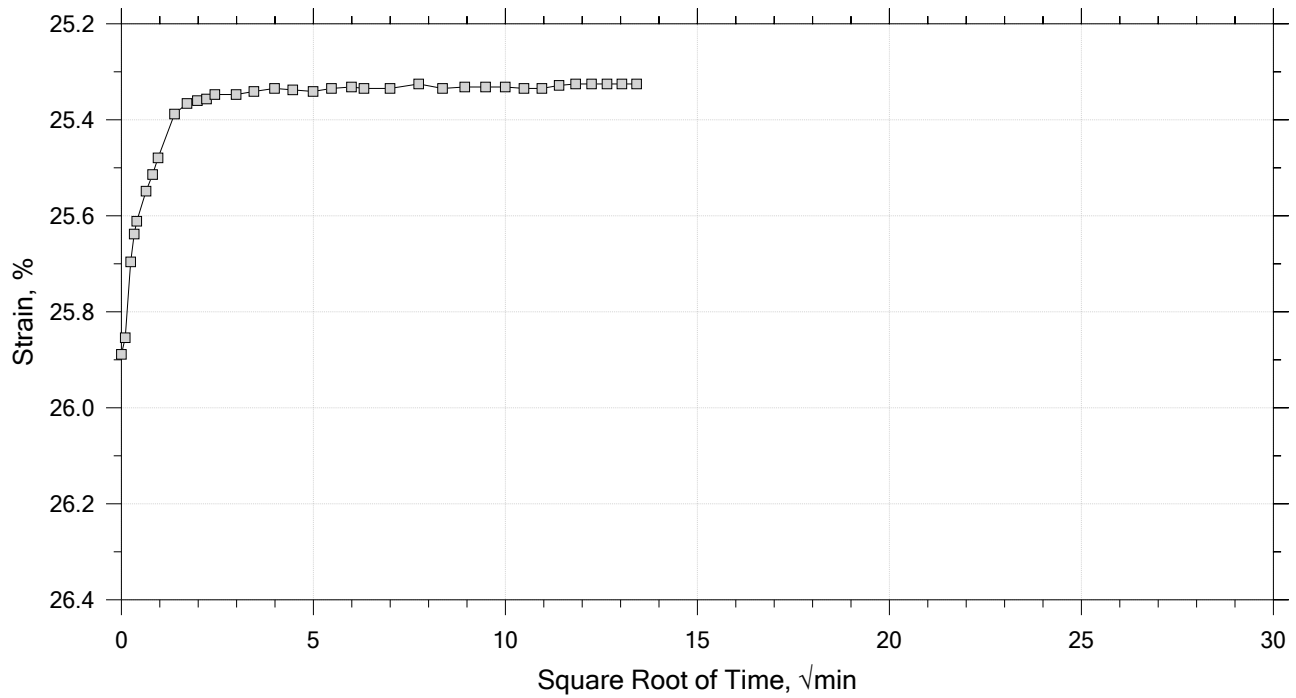
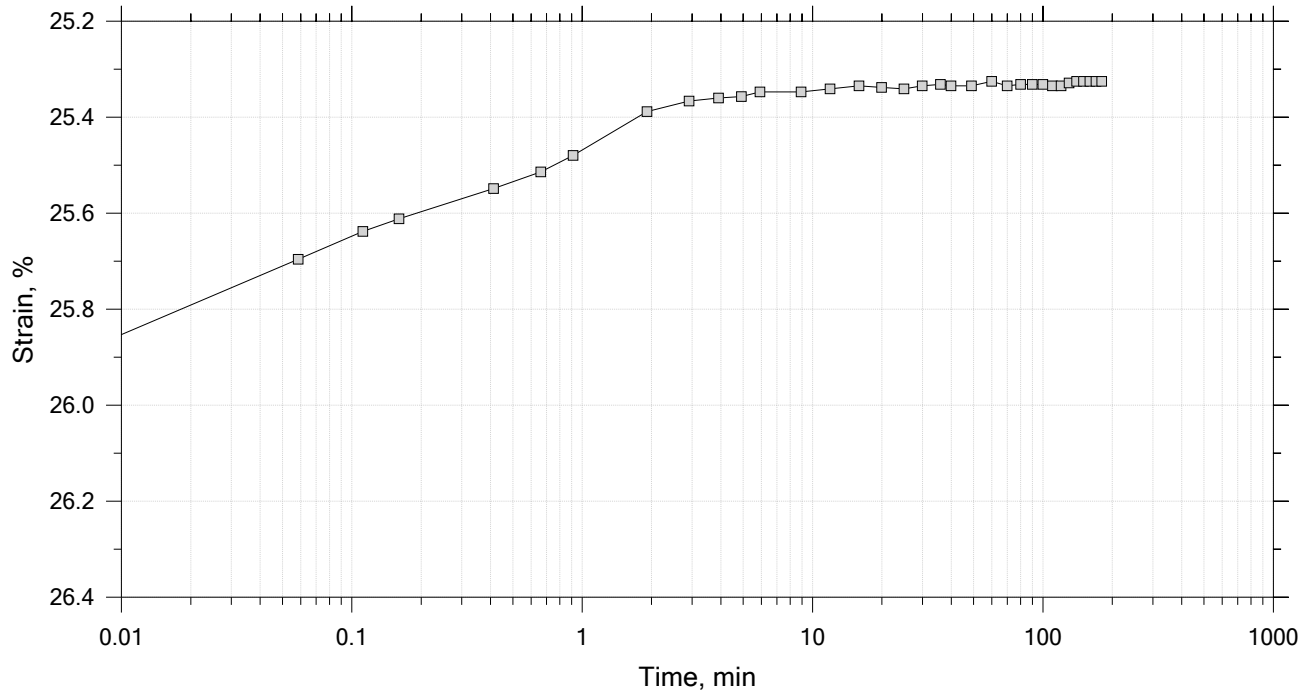
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



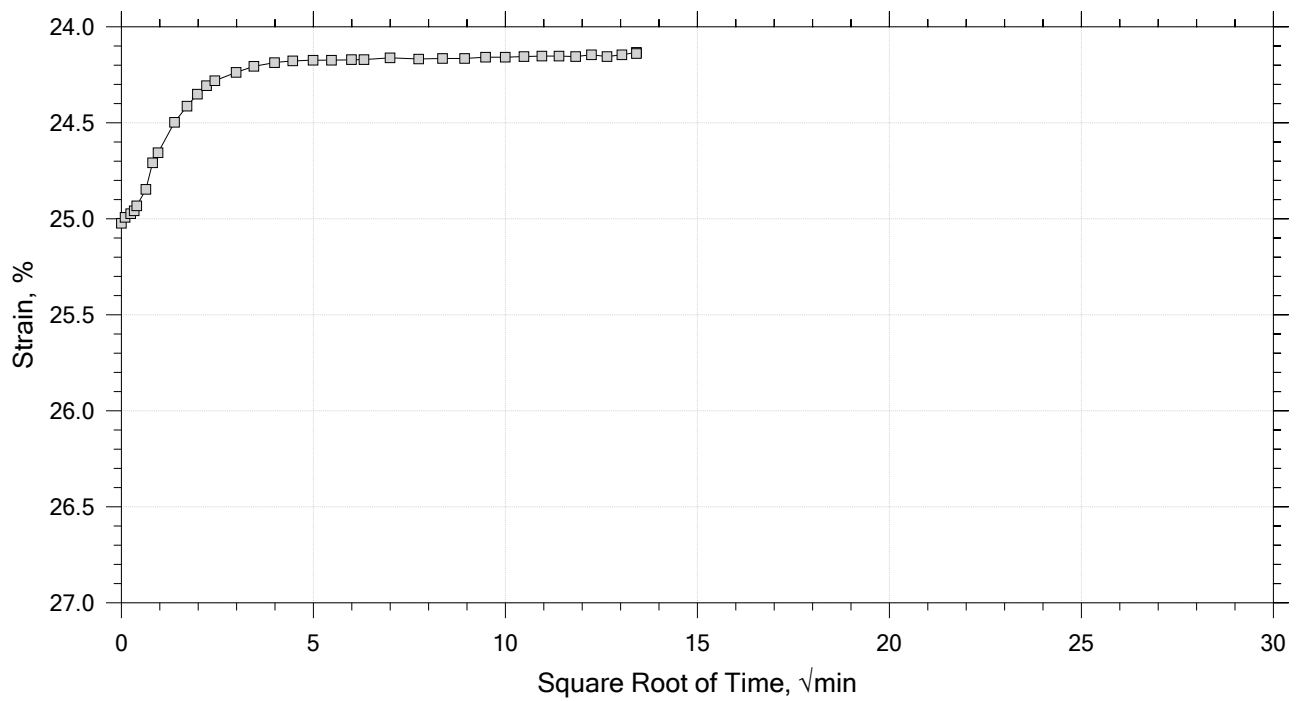
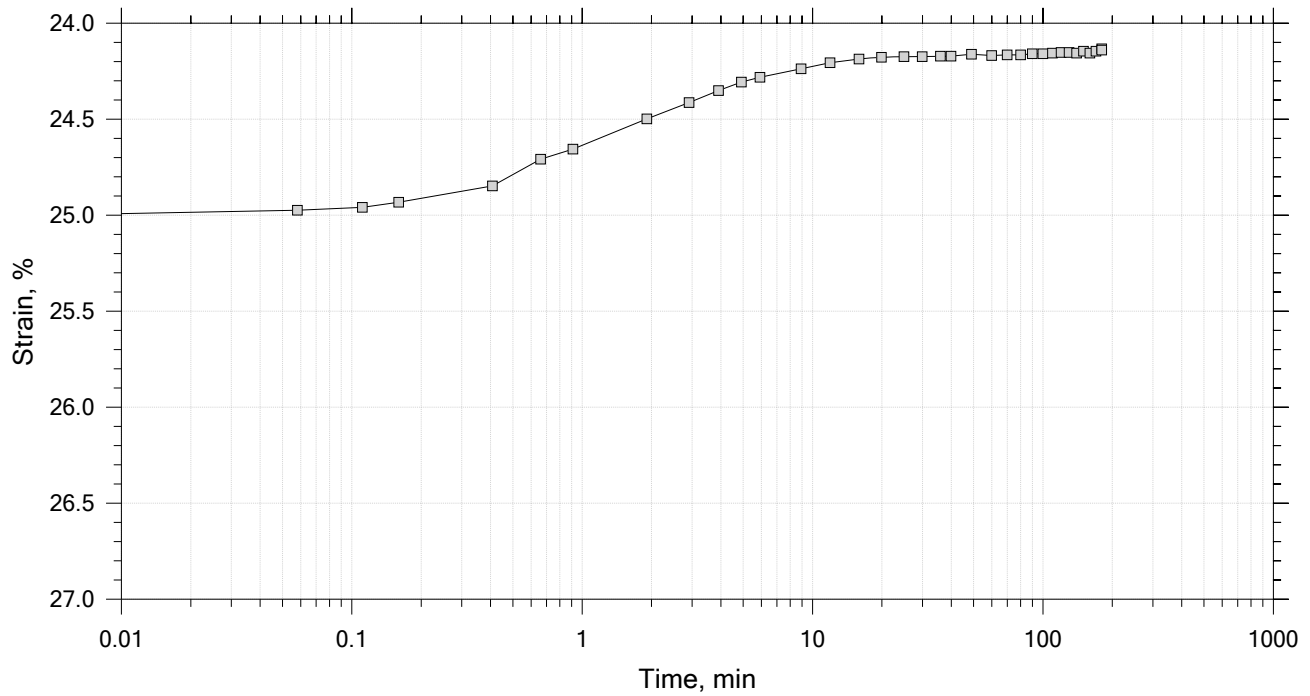
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



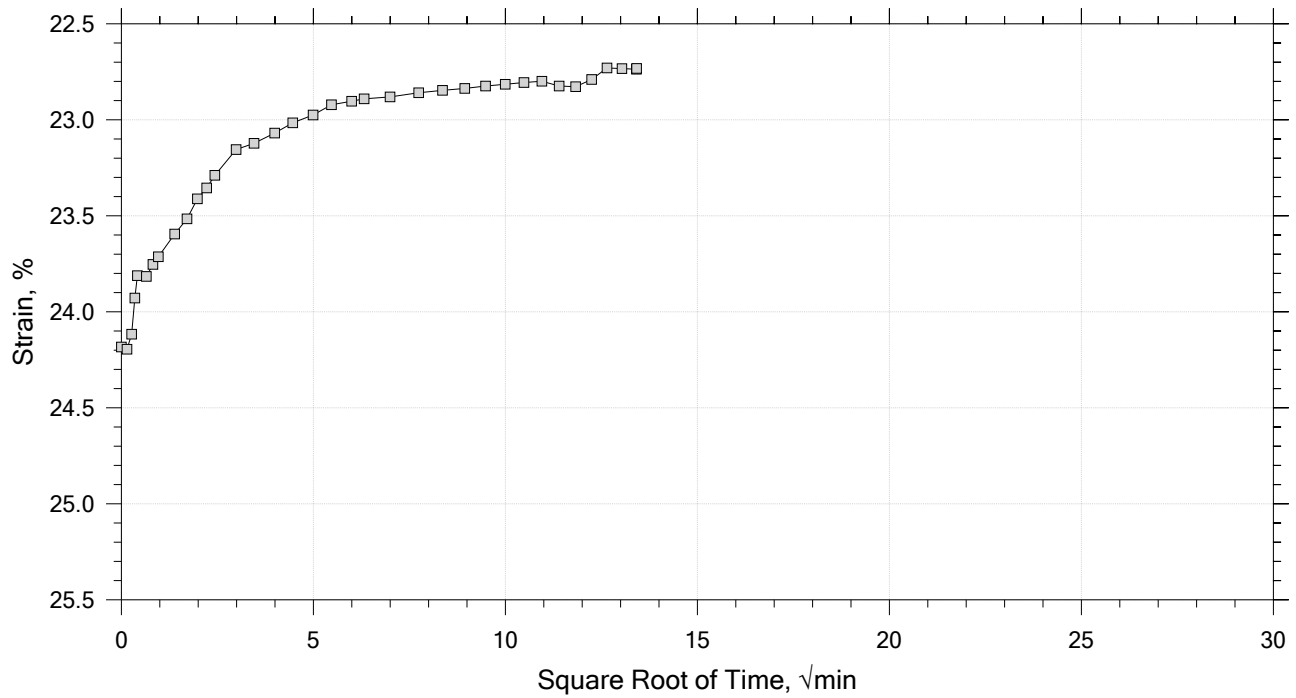
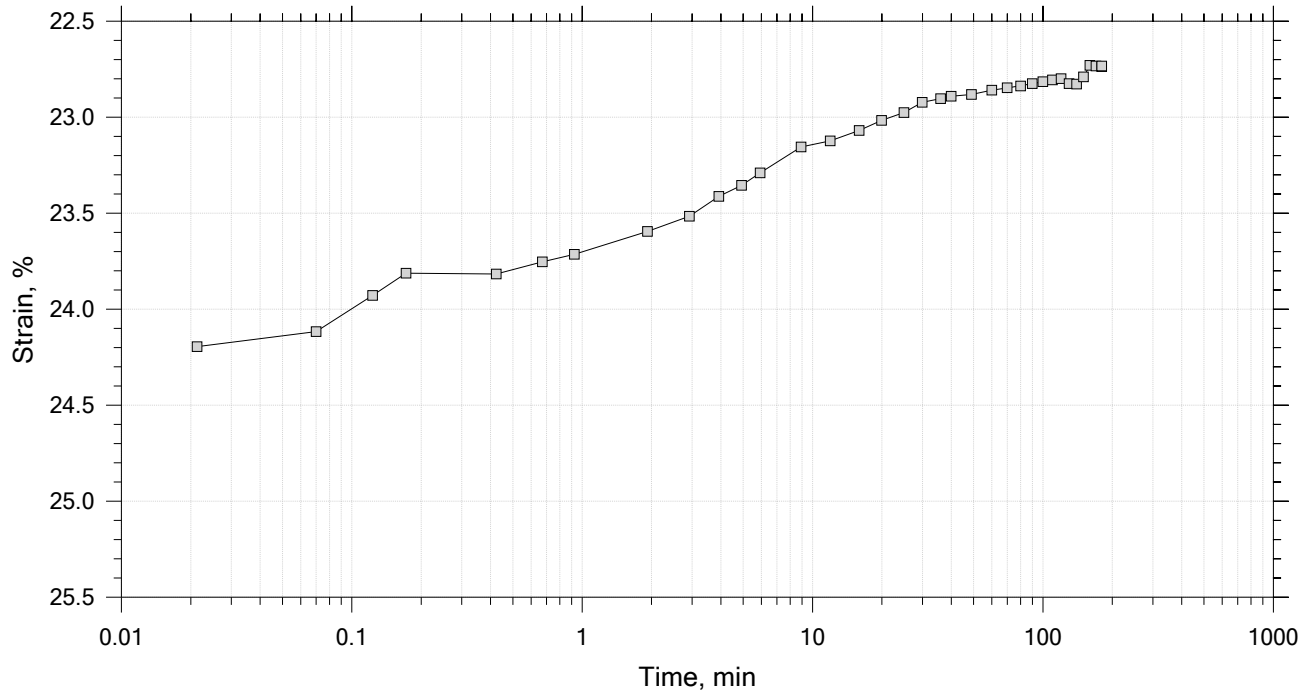
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	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



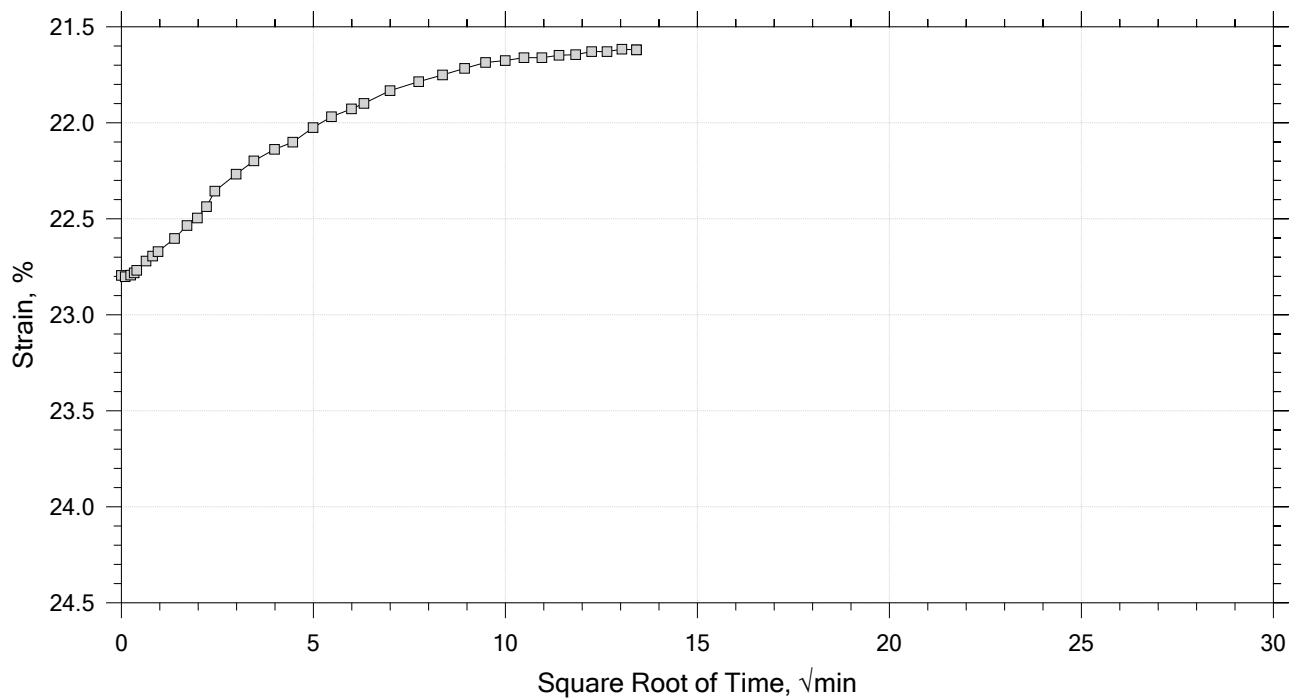
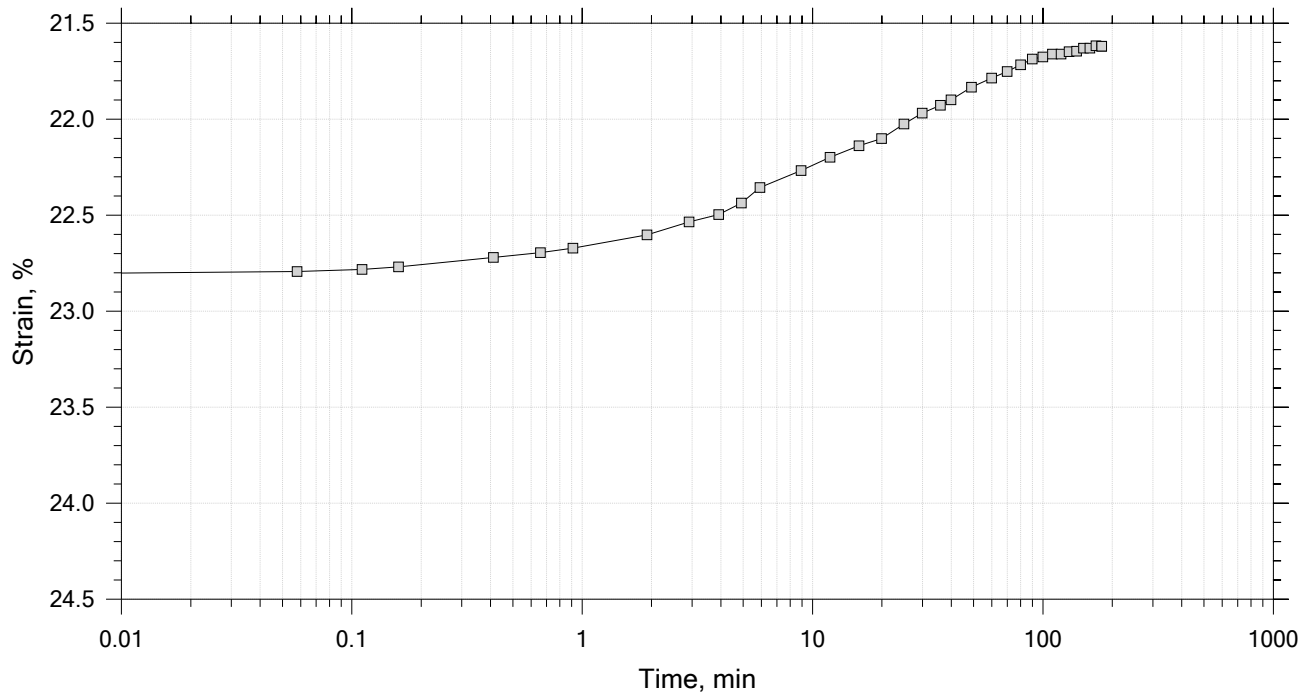
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



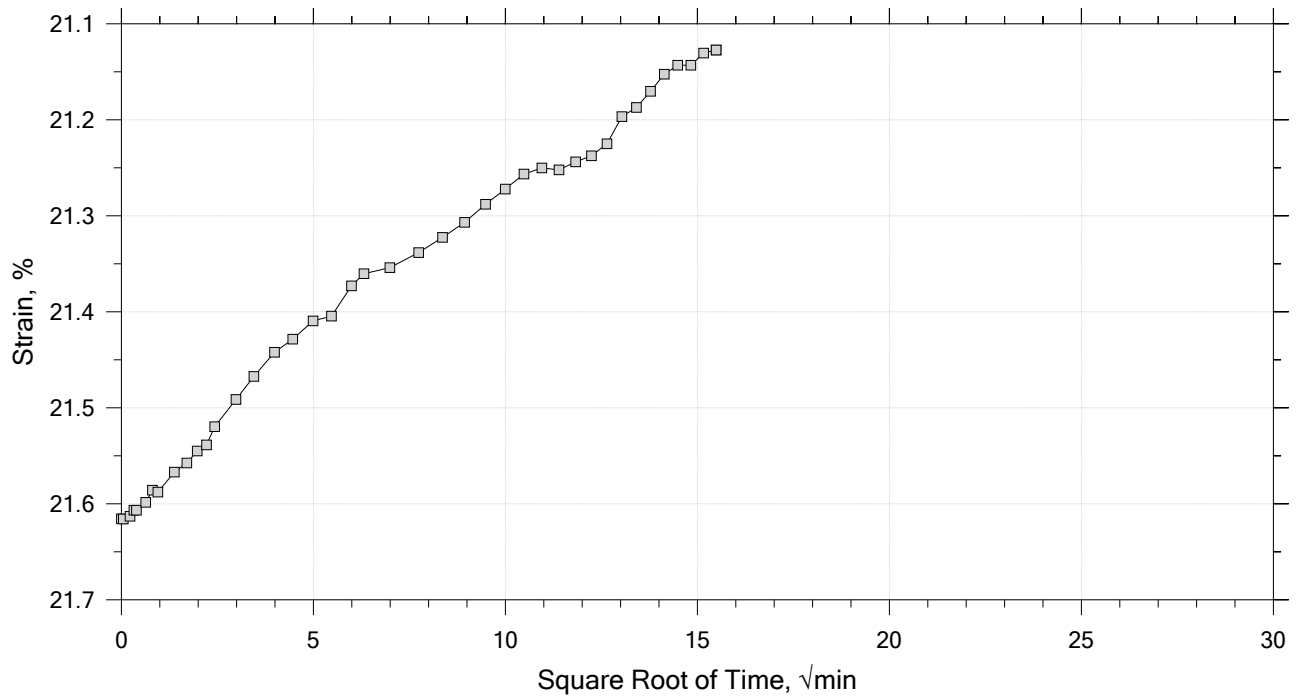
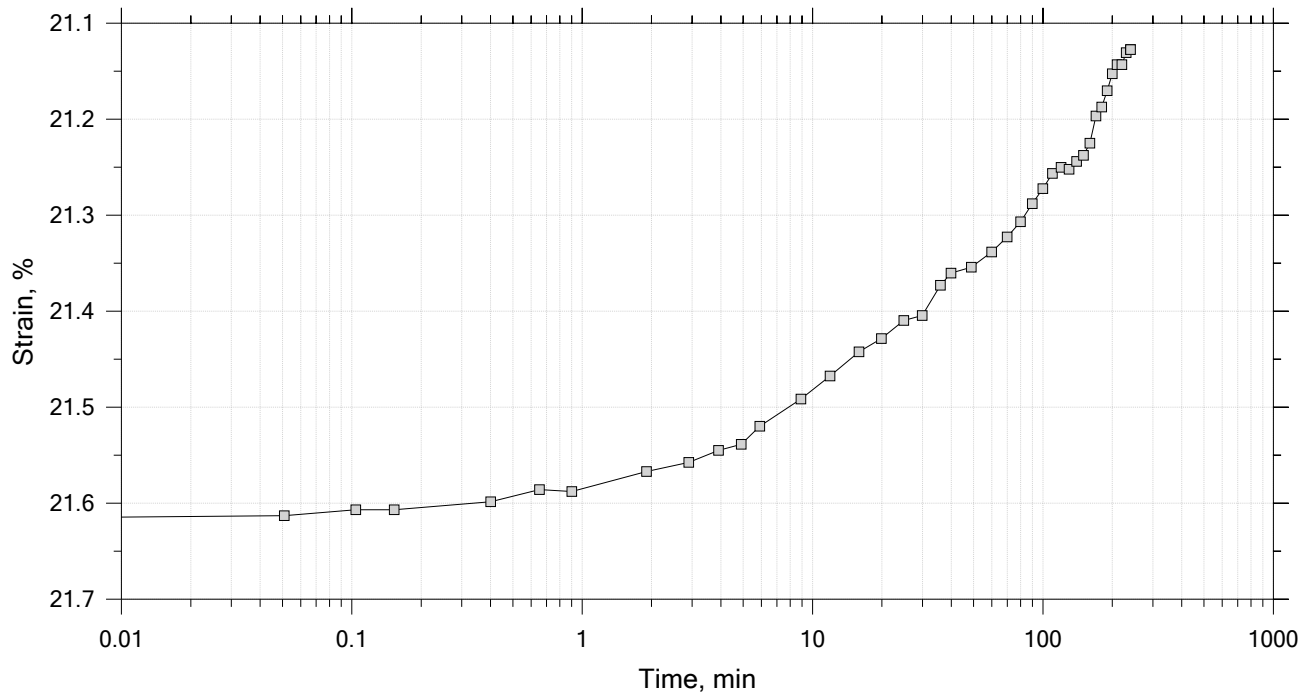
	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.77	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 1.11	Plastic Limit: 18
Final Height: 0.81 in	Final Void Ratio: 0.709	Plasticity Index: 17

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A2534	RING		D1387
Mass Container, gm	8.27	106.43	106.43	8.22
Mass Container + Wet Soil, gm	102.88	254.01	239.11	139.15
Mass Container + Dry Soil, gm	77.48	212.08	212.08	112.48
Mass Dry Soil, gm	69.21	105.65	105.65	104.26
Water Content, %	36.70	39.68	25.58	25.58
Void Ratio	---	1.11	0.71	---
Degree of Saturation, %	---	99.09	100.00	---
Dry Unit Weight, pcf	---	81.996	101.23	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

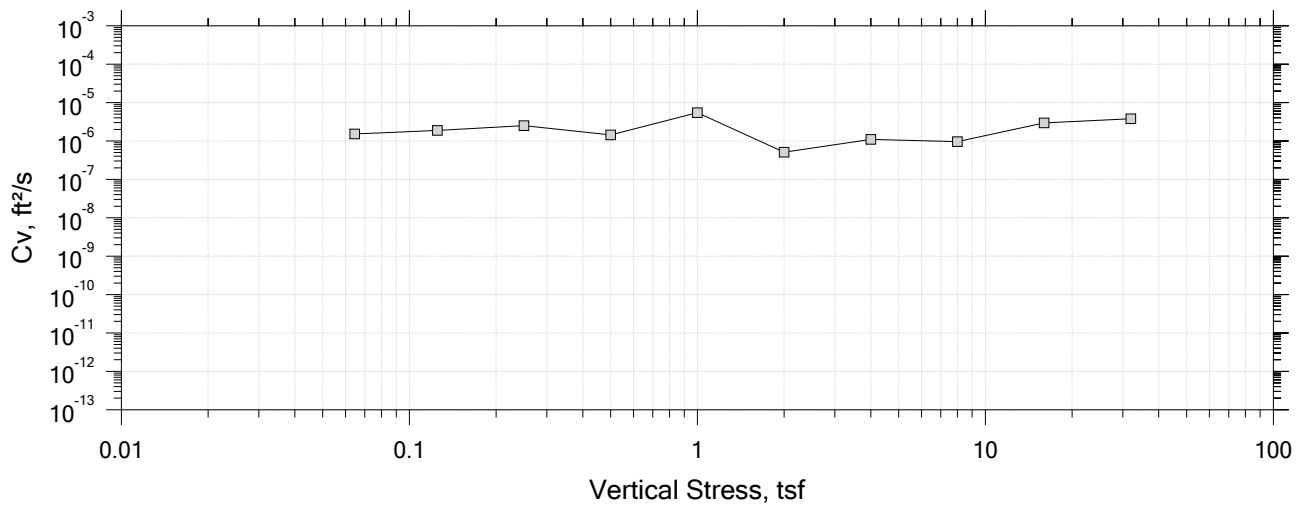
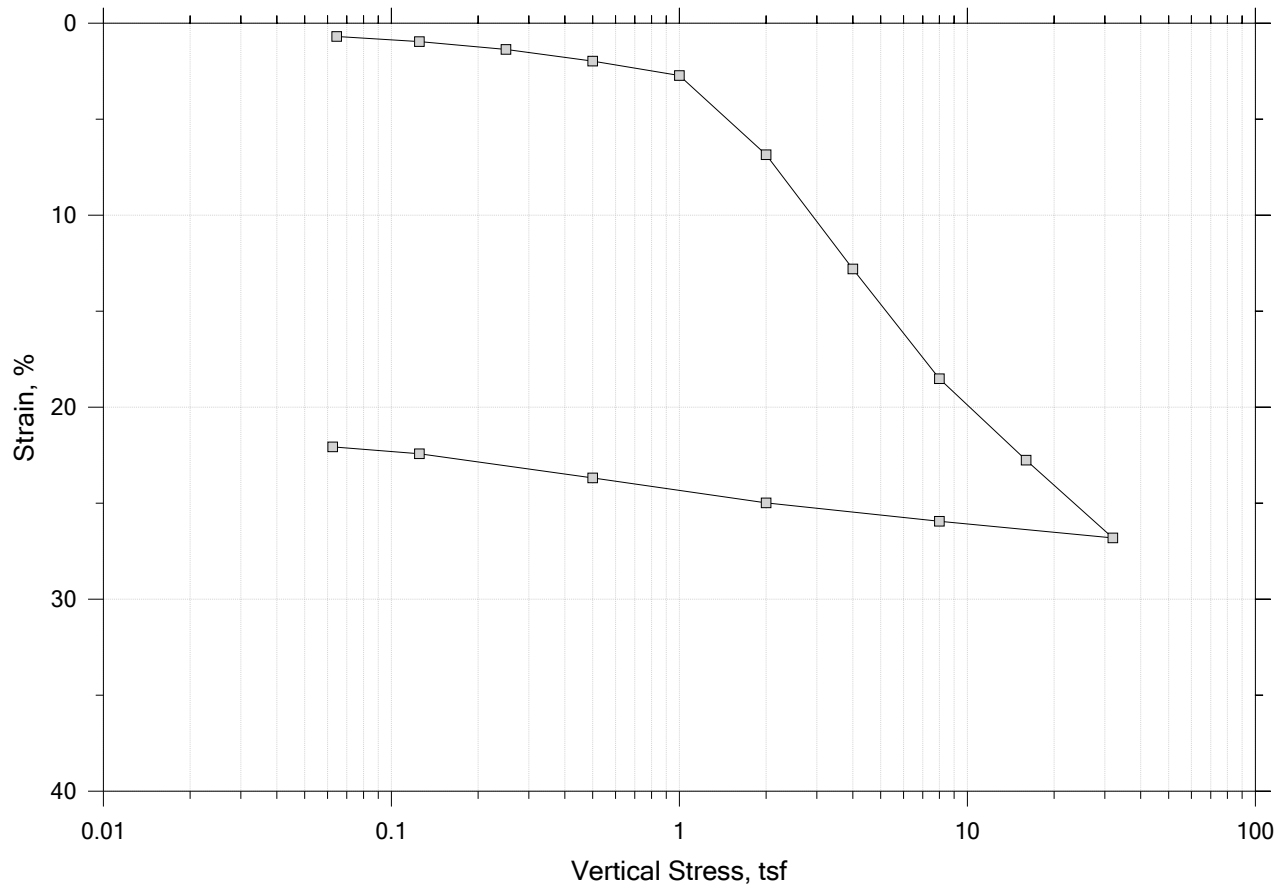
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-105	Tested By: trm	Checked By: mcm
	Sample No.: 2U	Test Date: 9/29/2018	Depth: 14-16 ft
	Test No.: IP-7	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System LTIII-A, Swell Pressure = 0.0778 tst		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

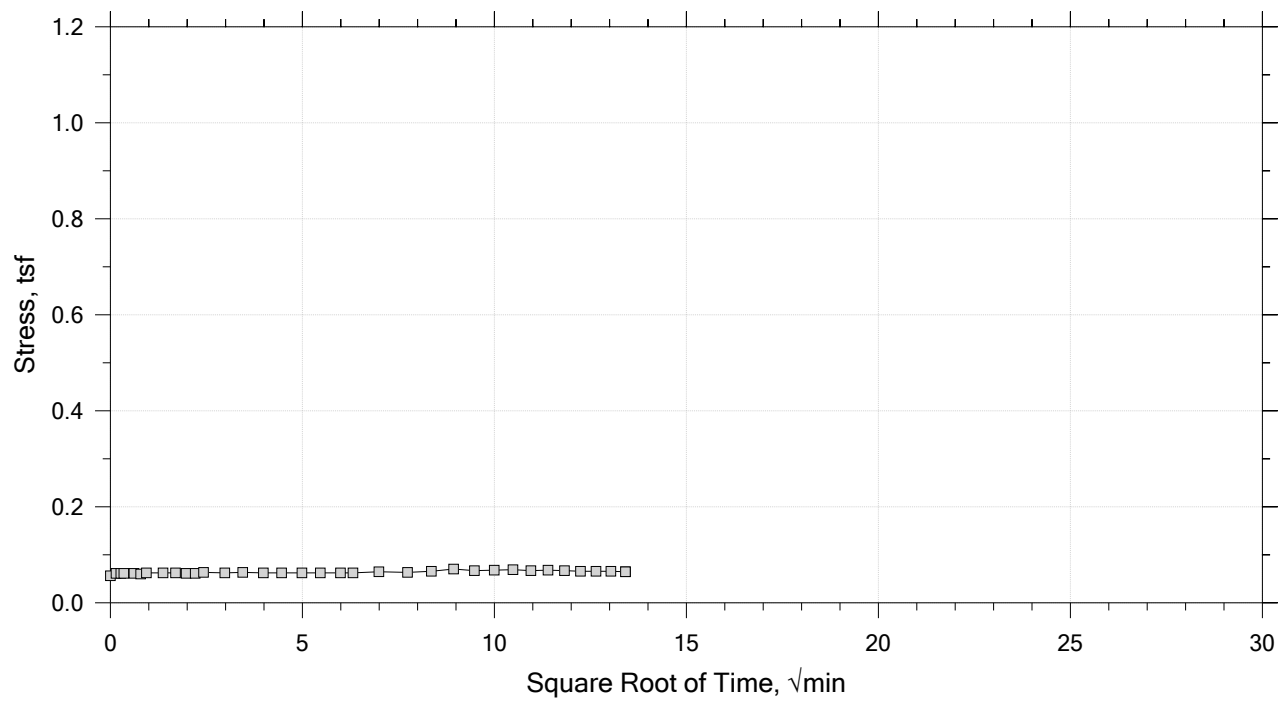
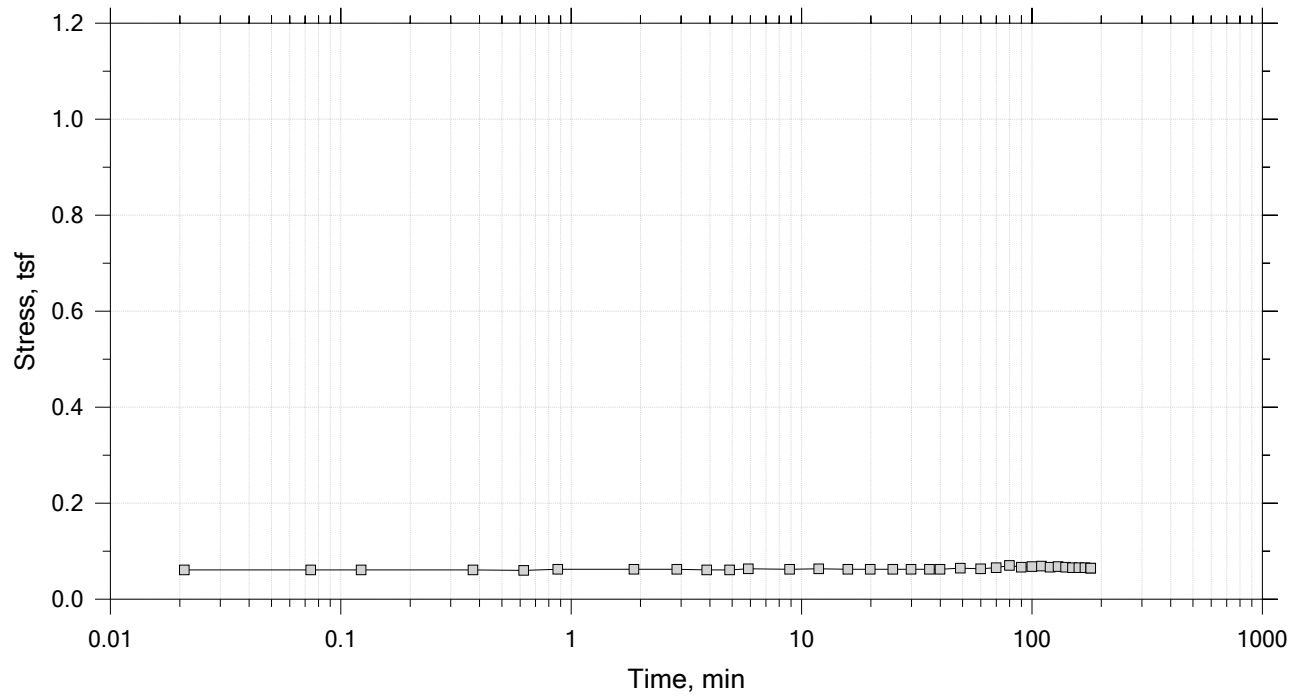
Summary Report




	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0645 tsf



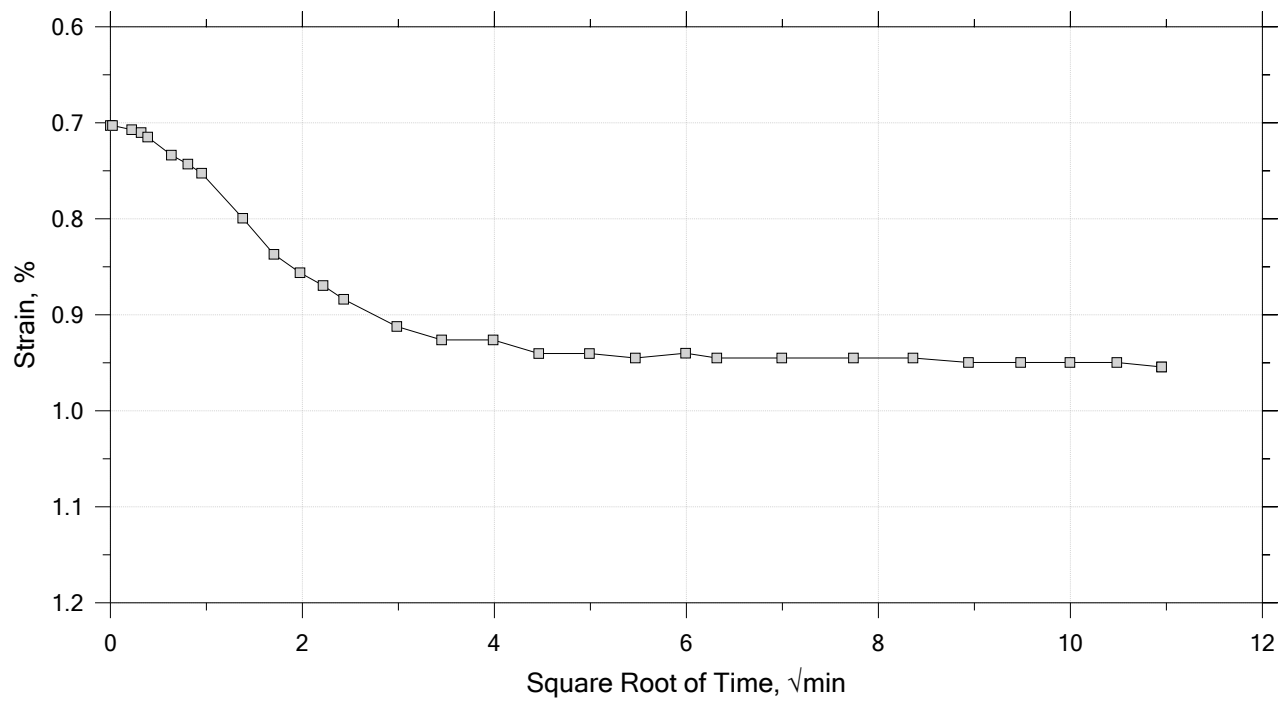
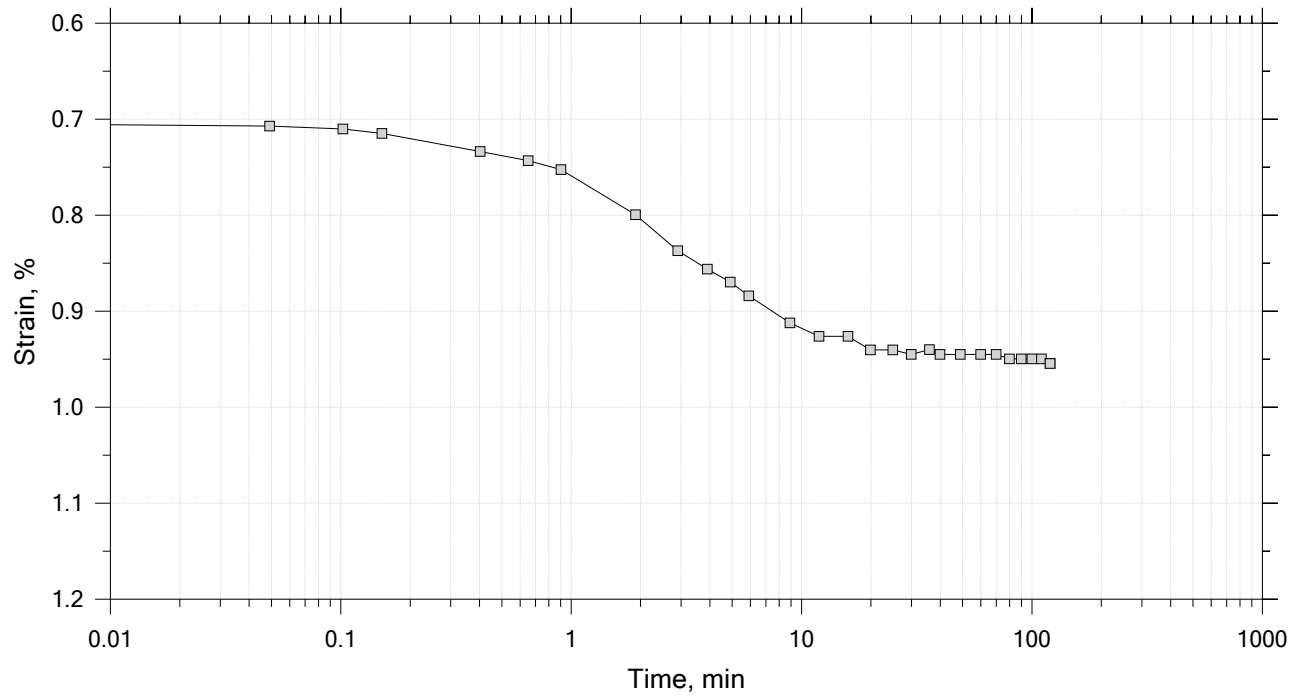
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



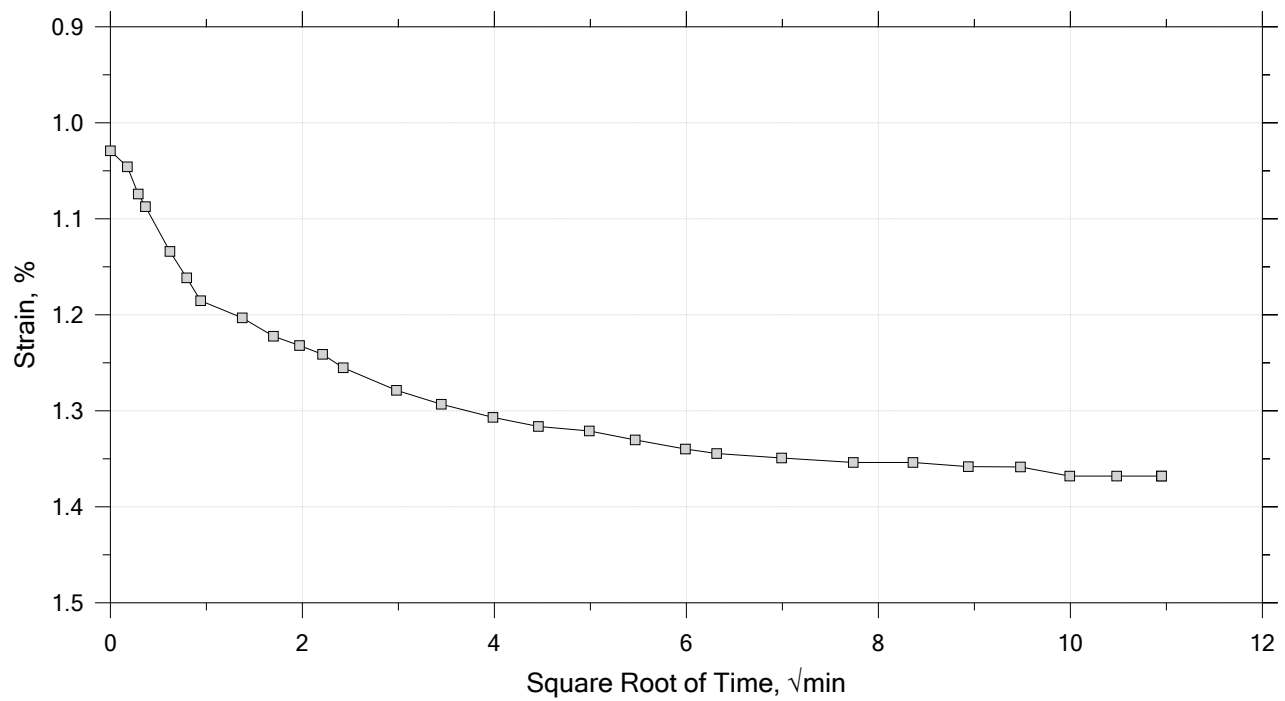
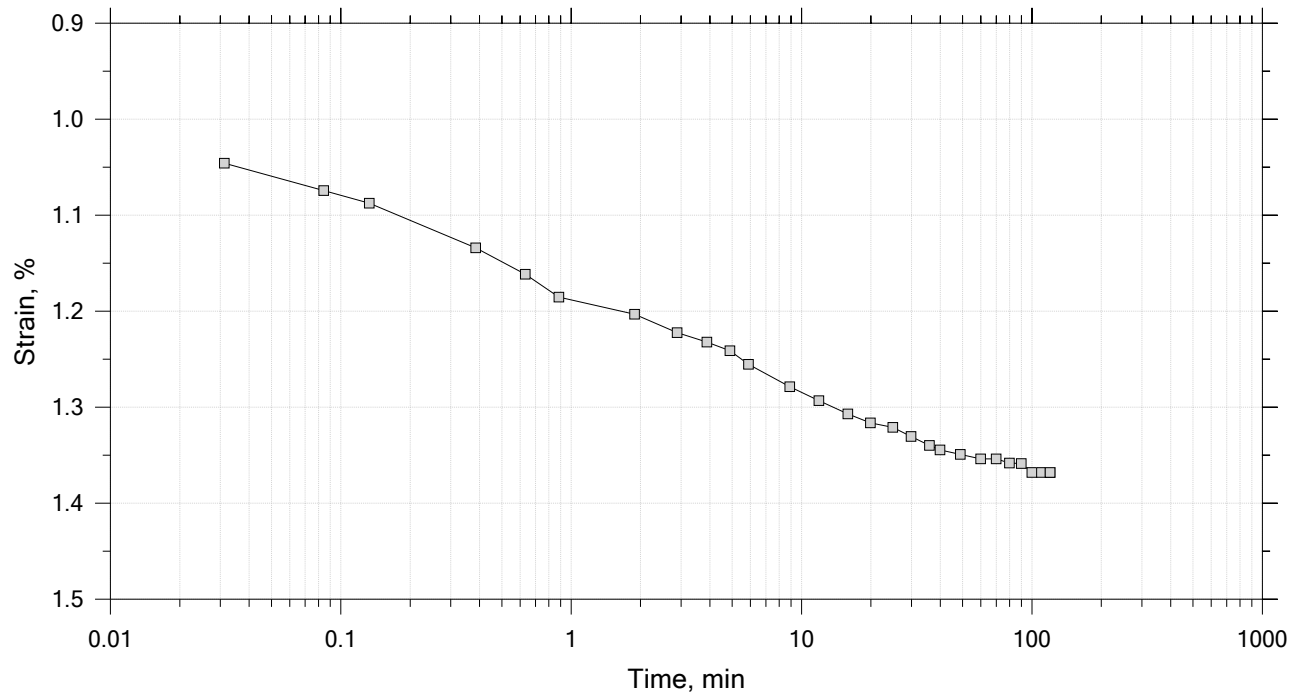
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



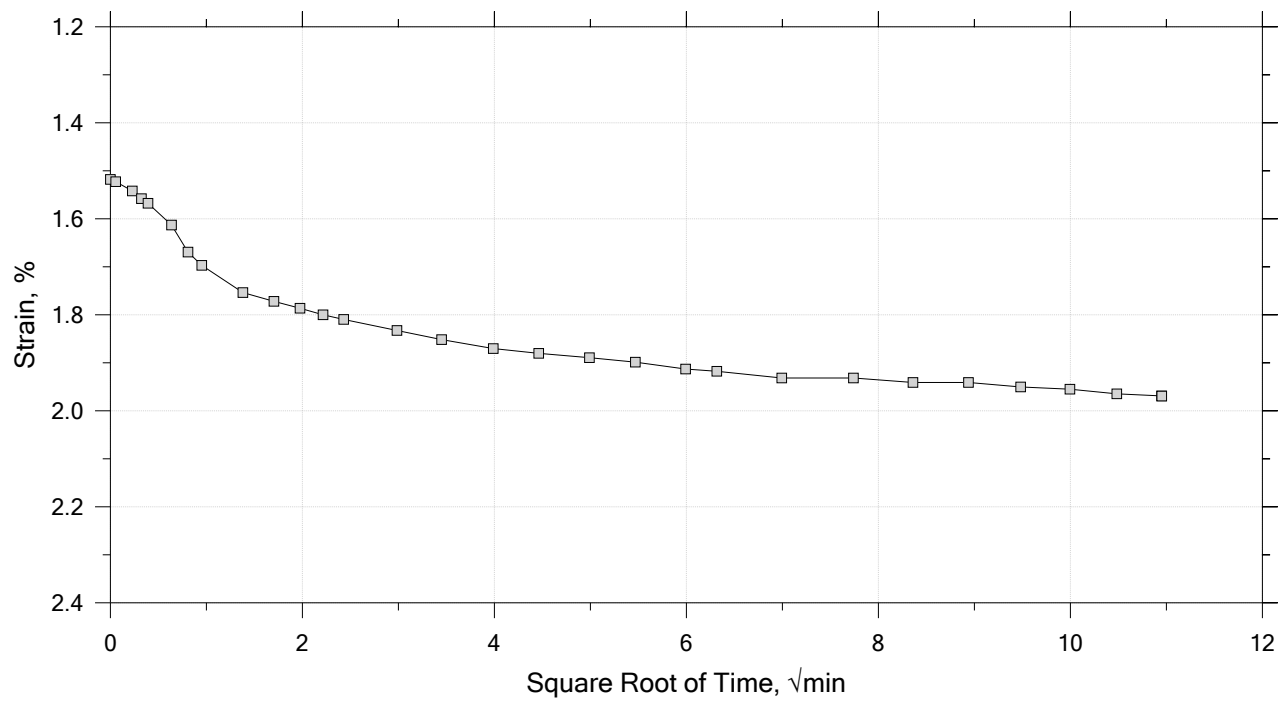
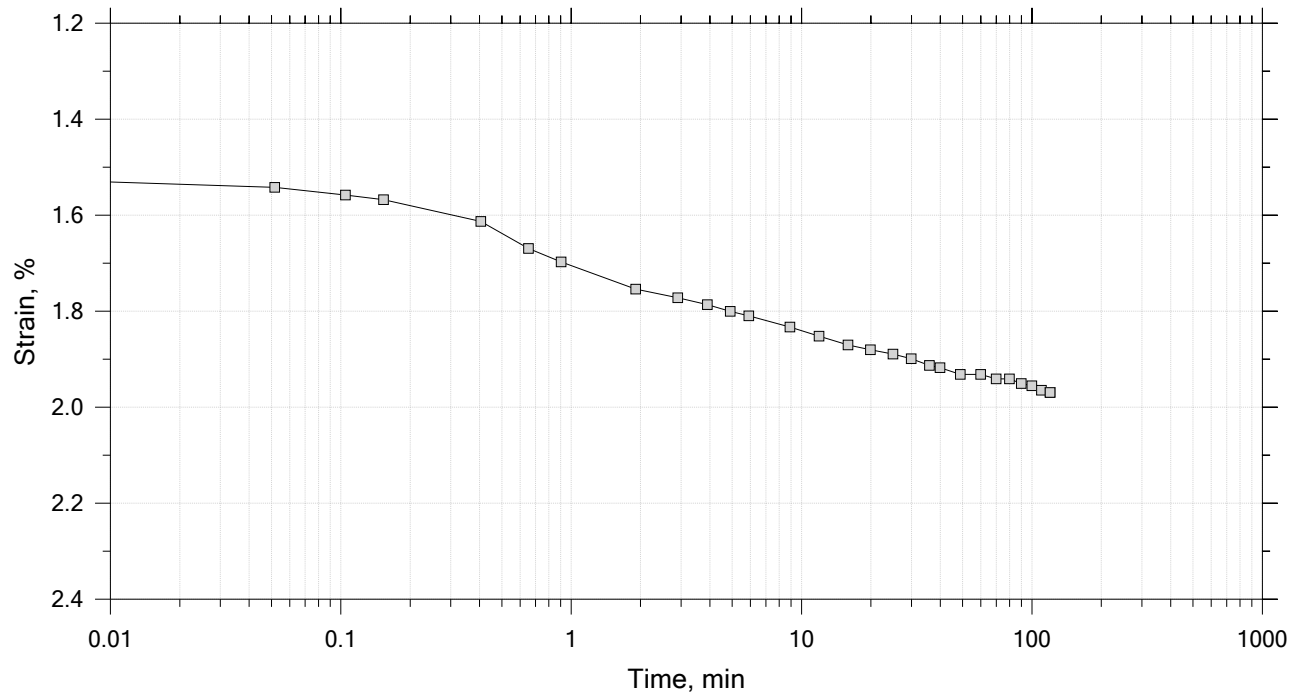
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



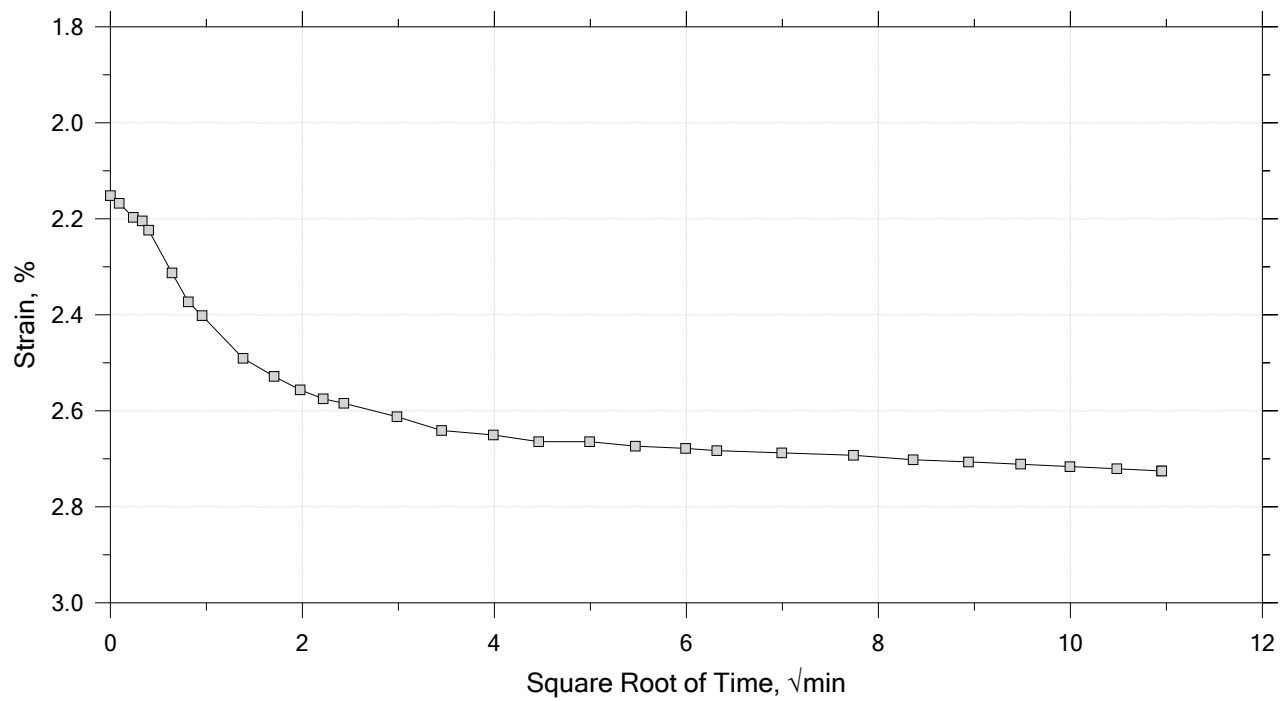
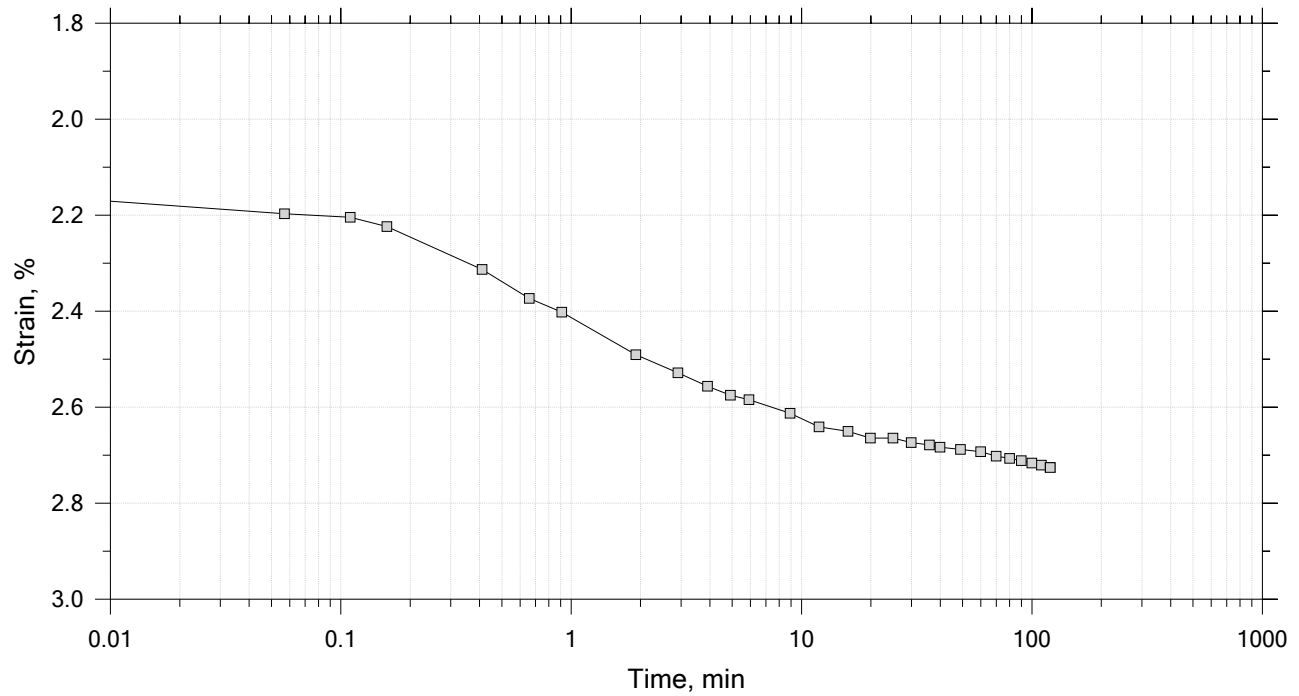
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



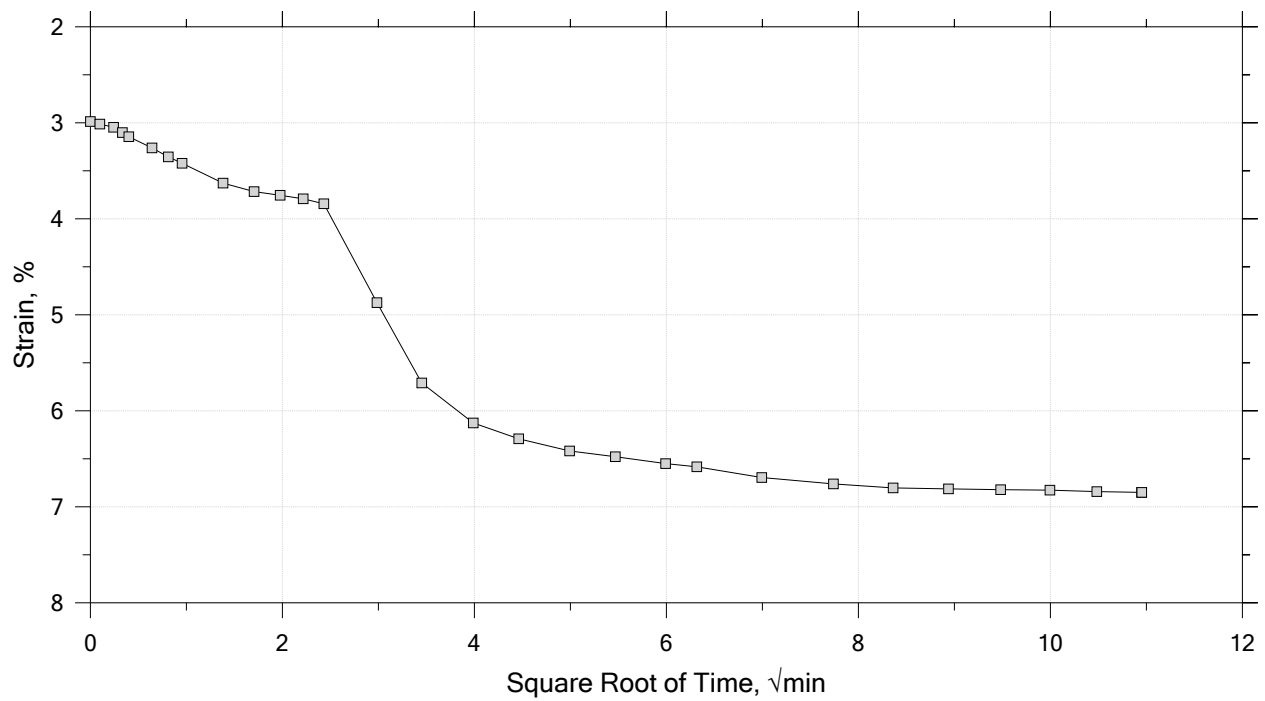
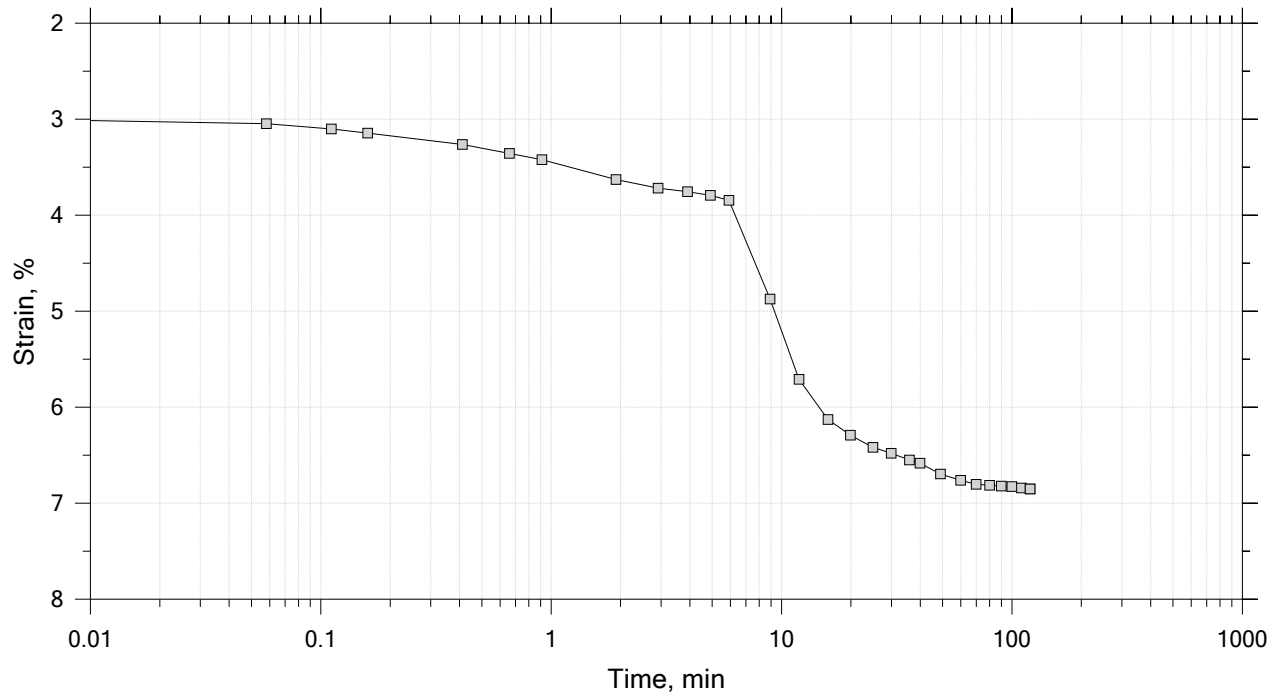
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



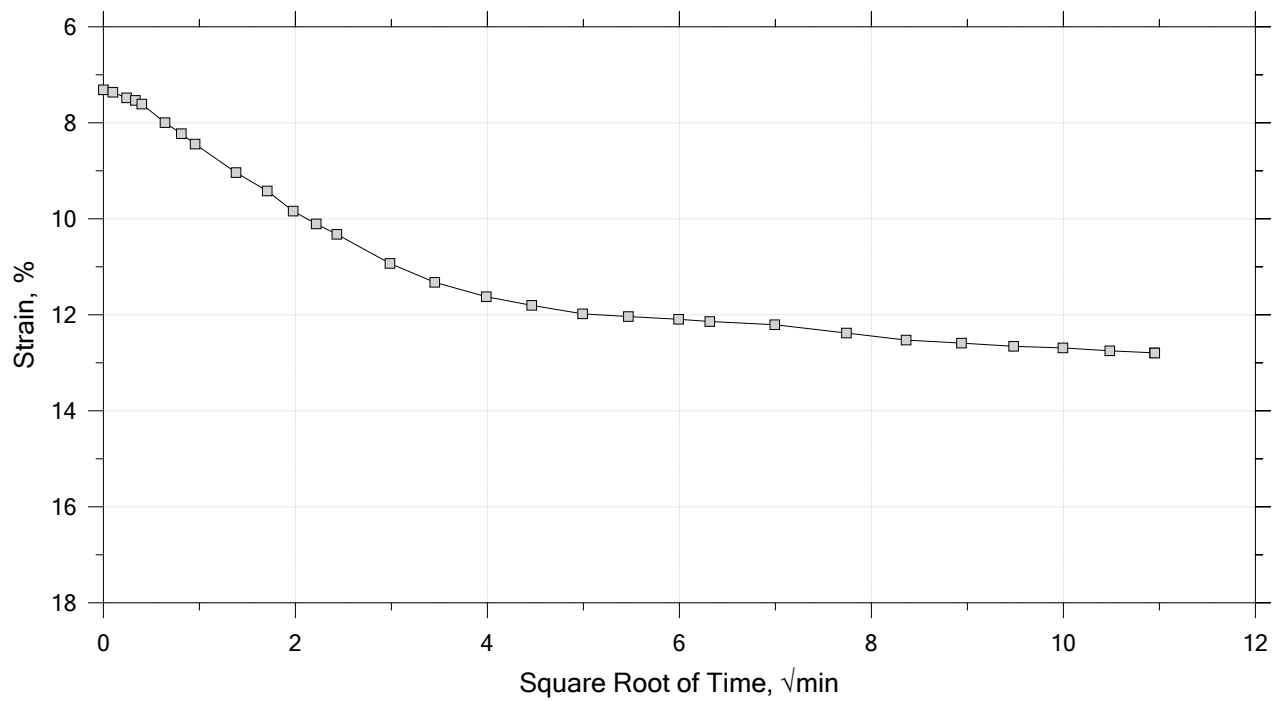
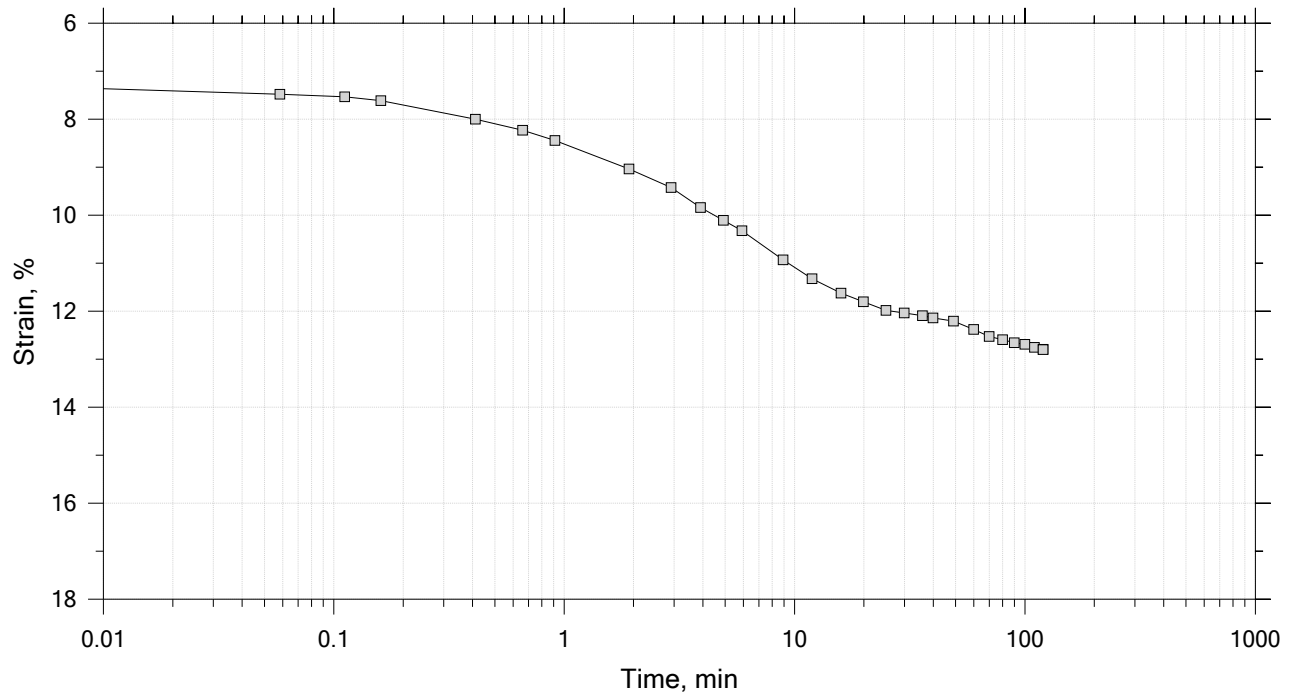
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



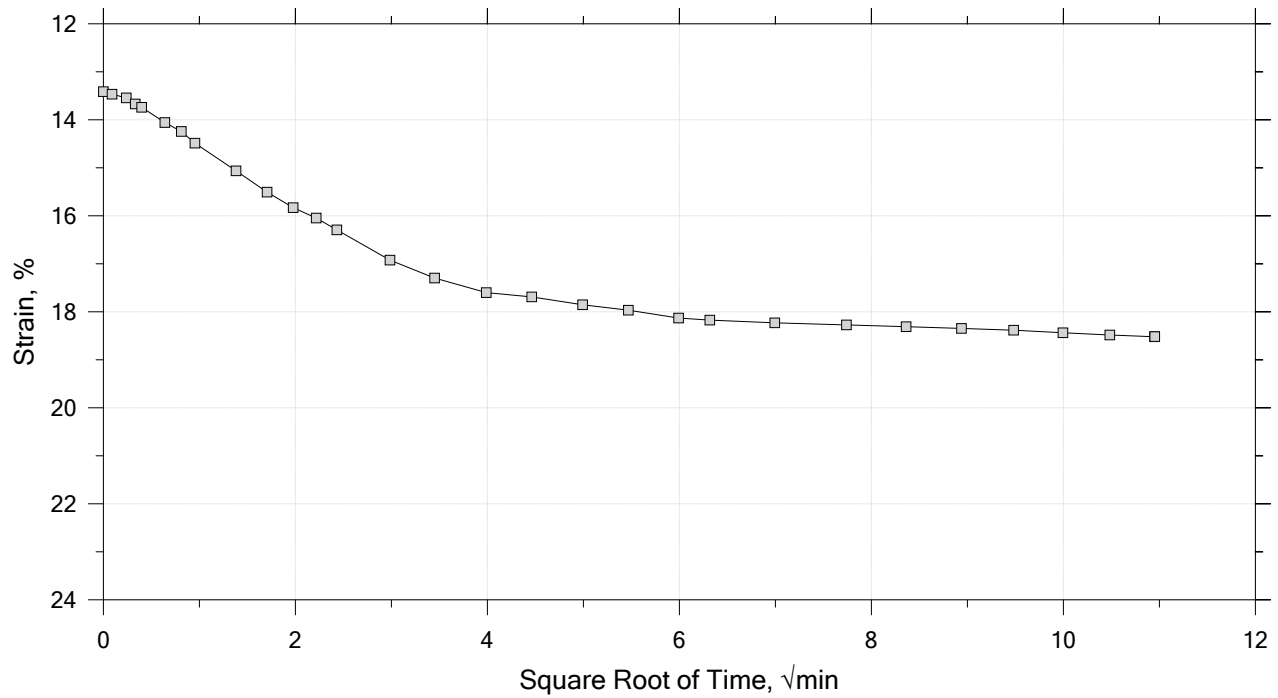
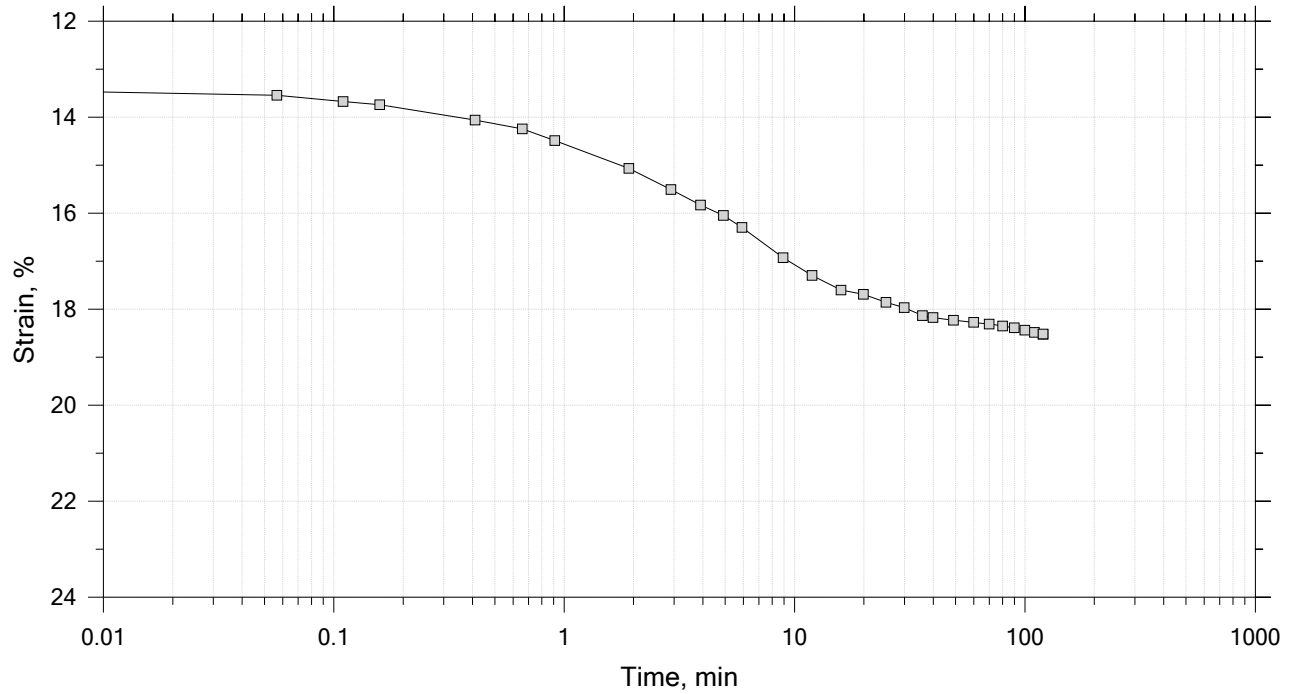
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



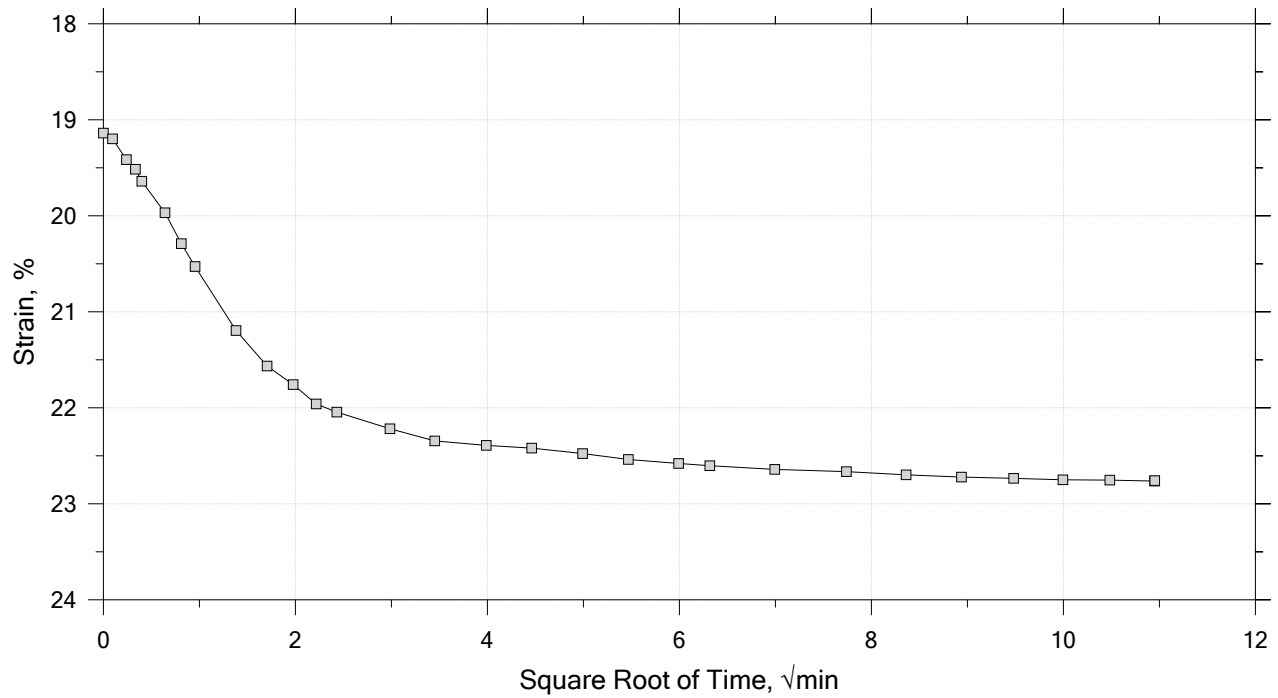
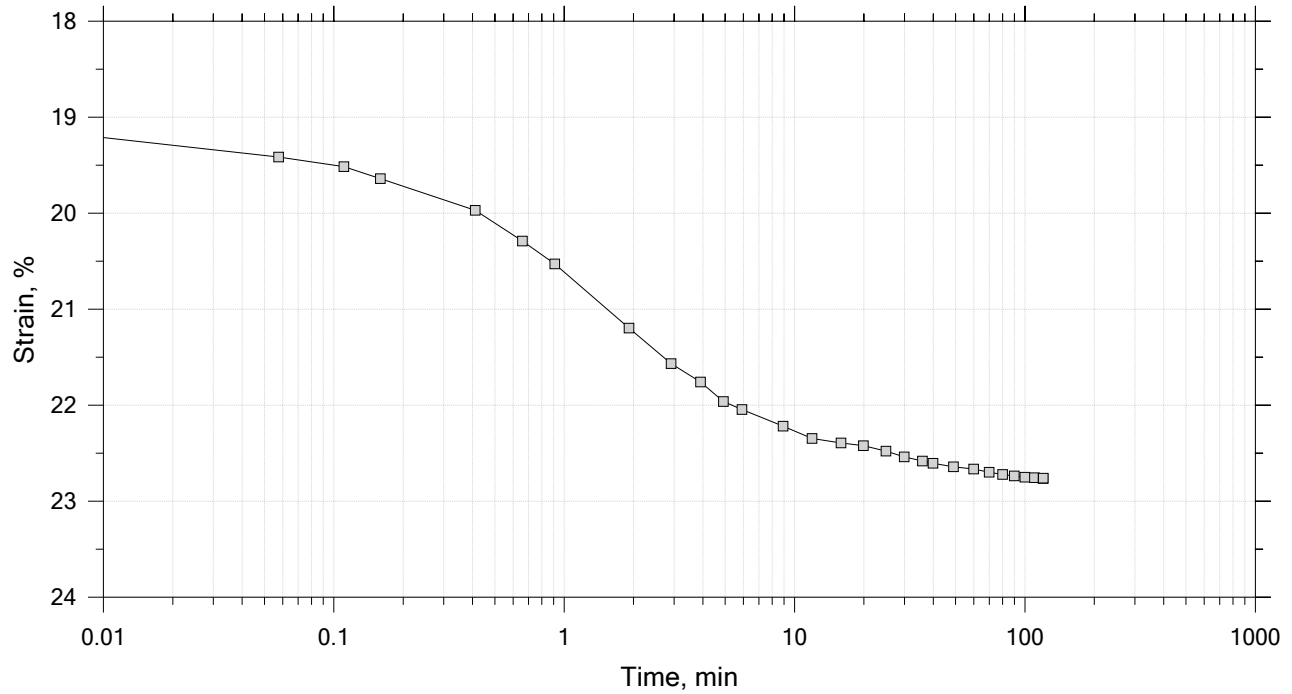
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



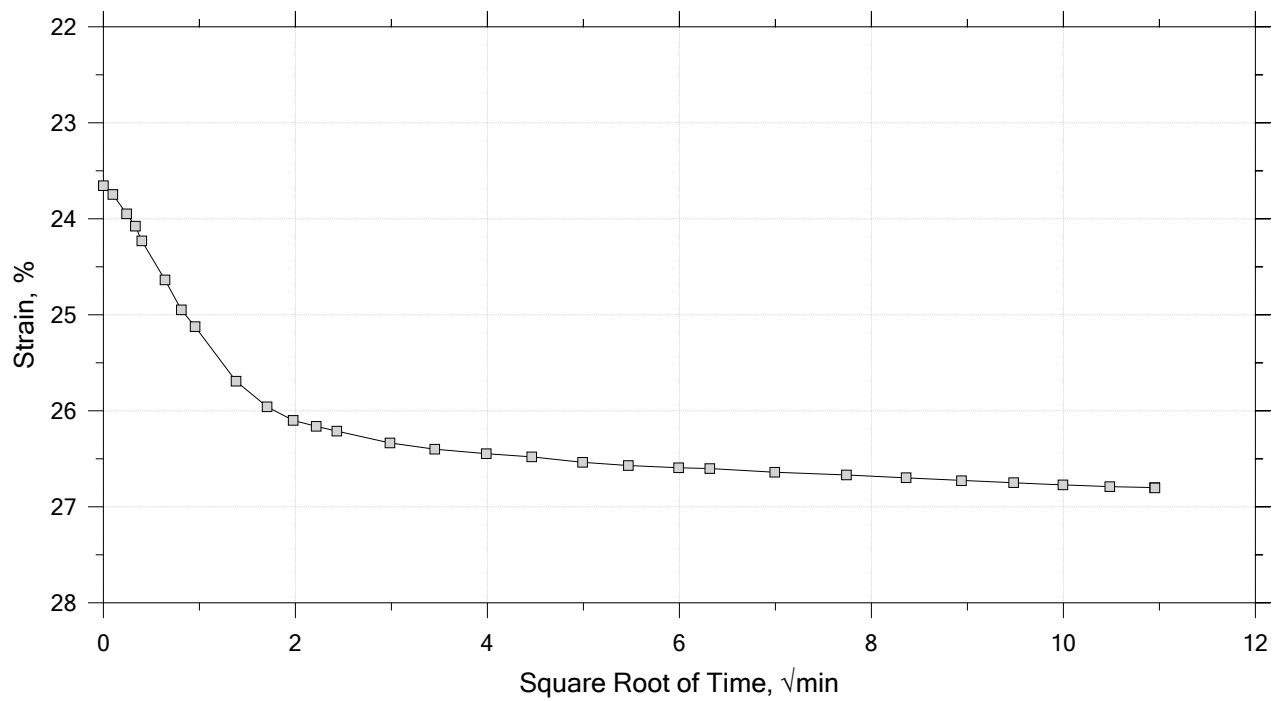
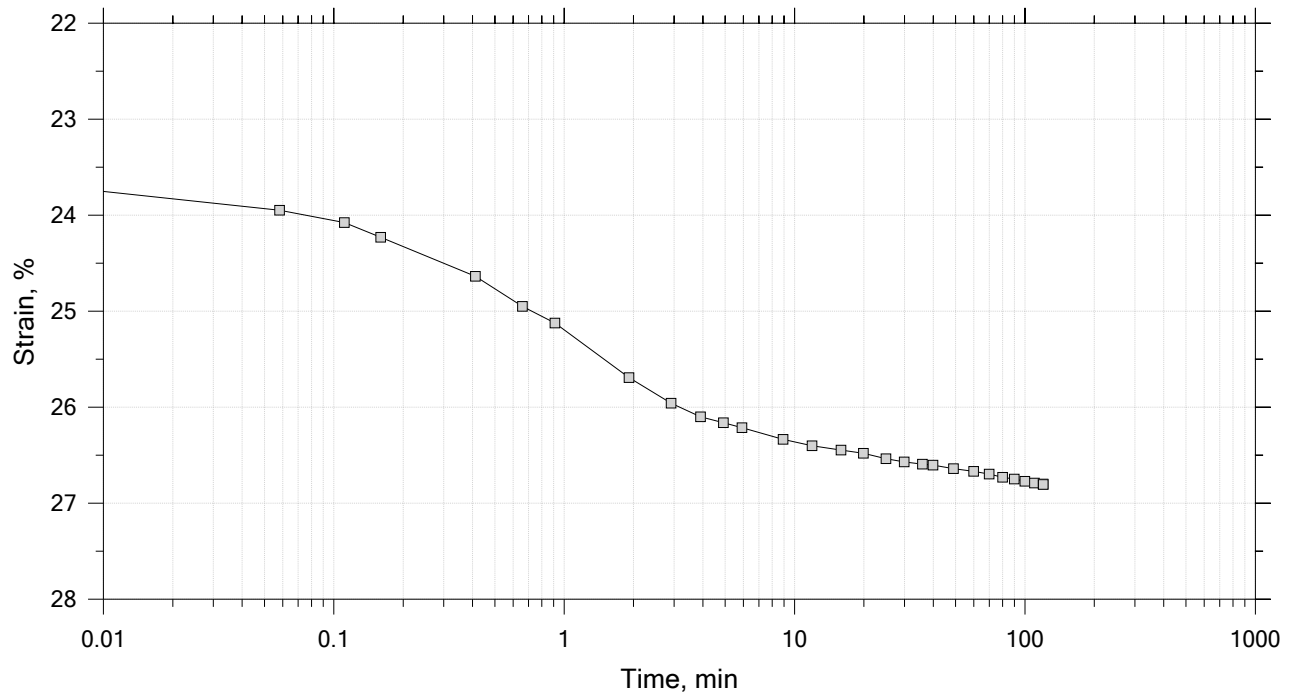
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



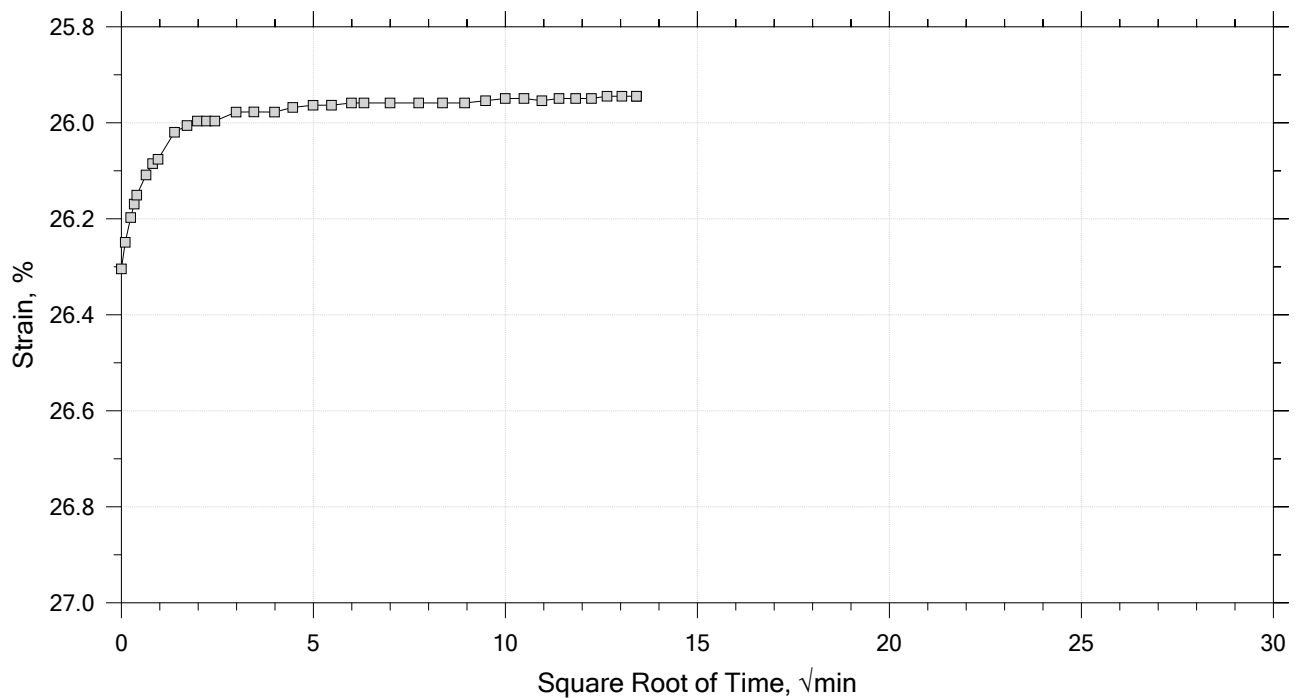
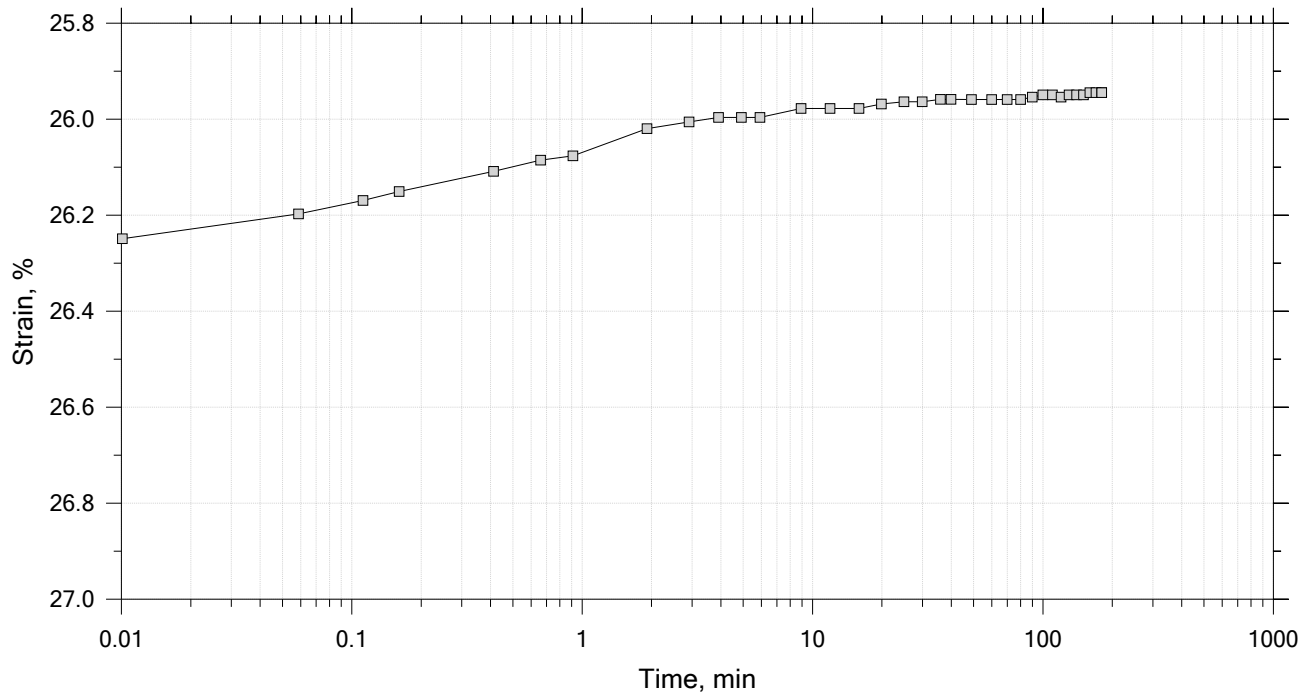
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



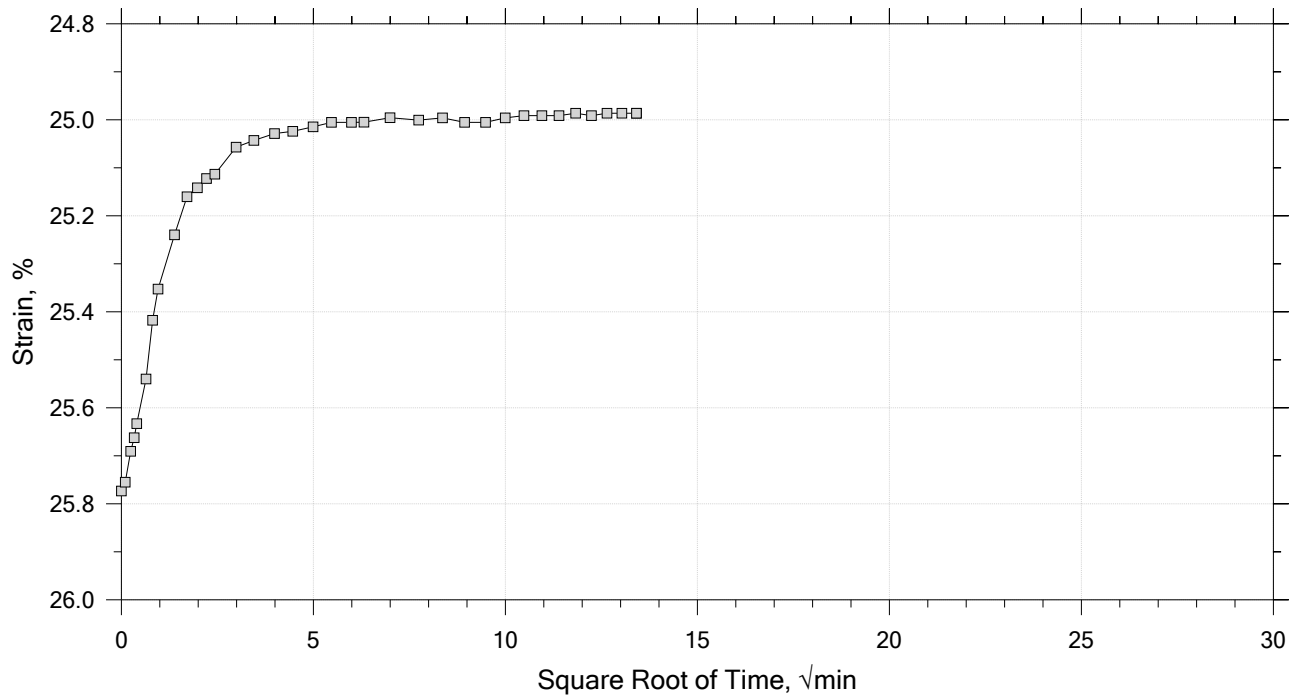
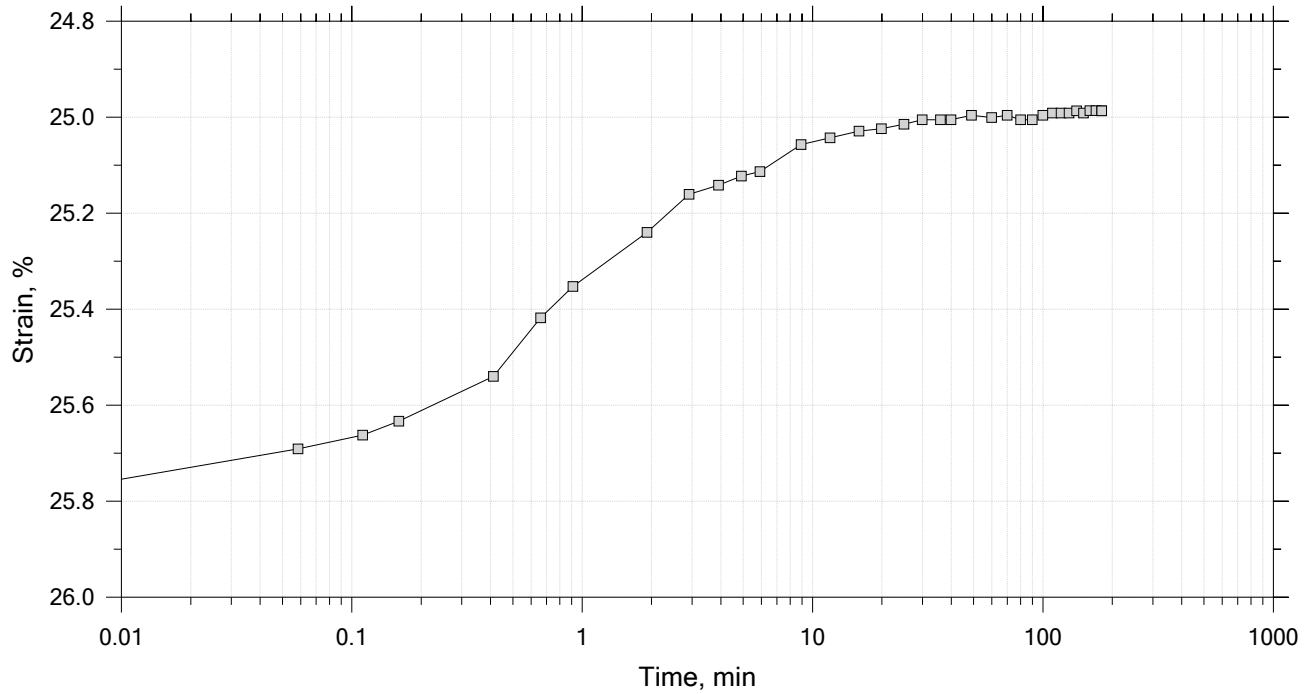
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



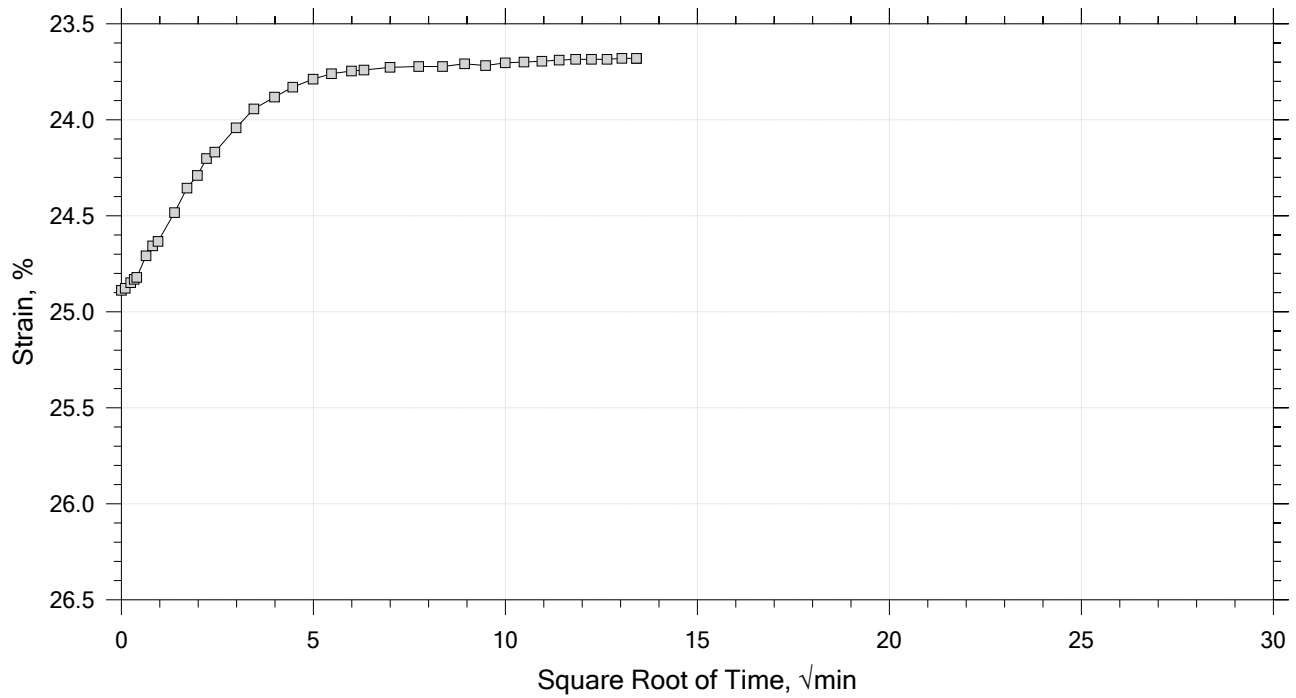
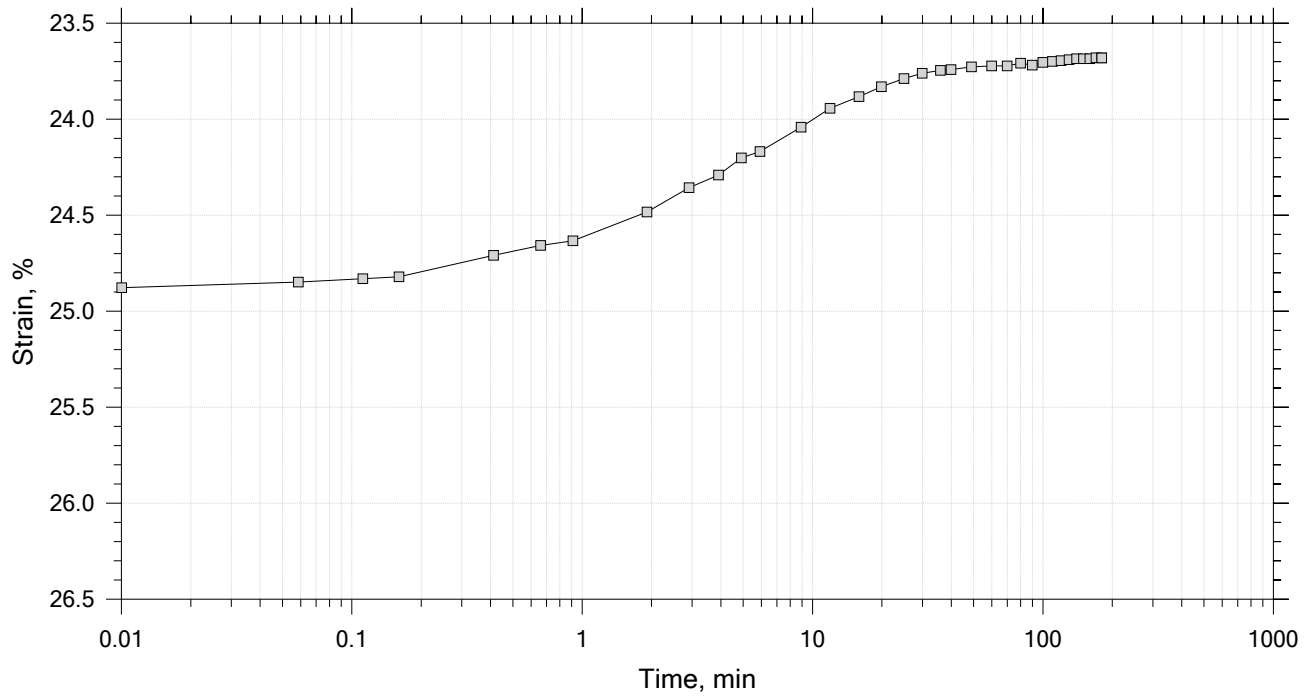
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	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



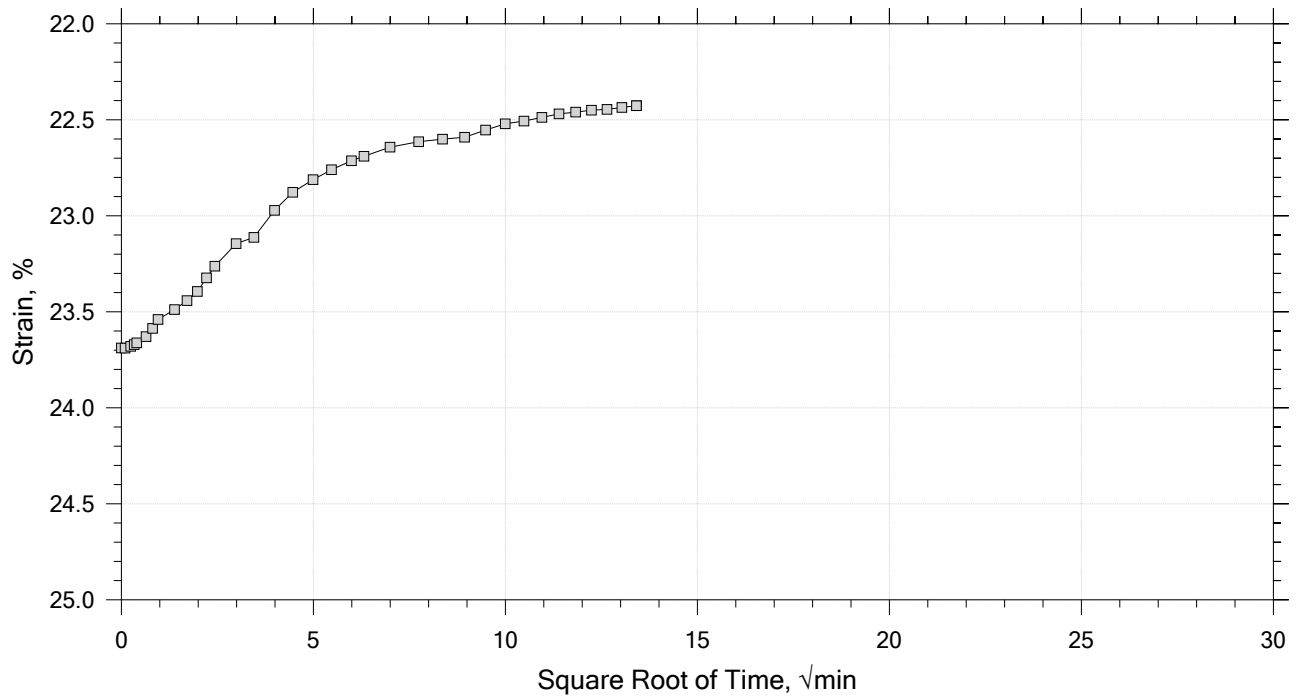
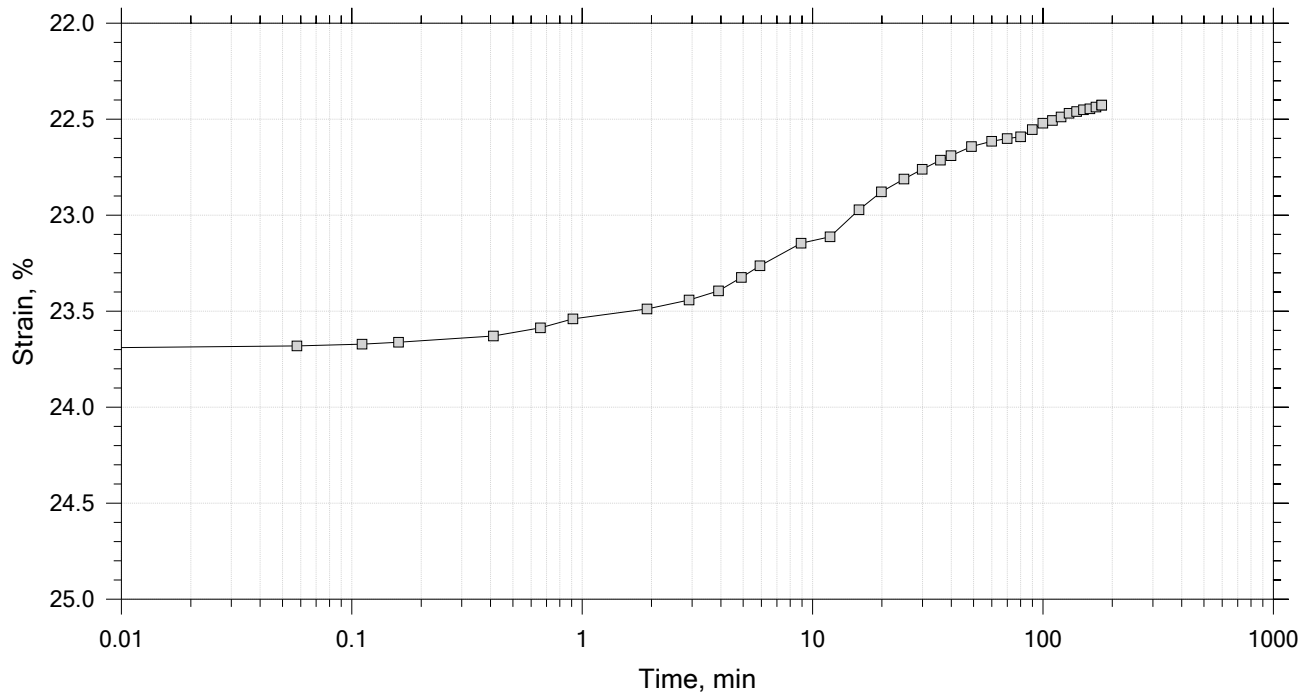
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



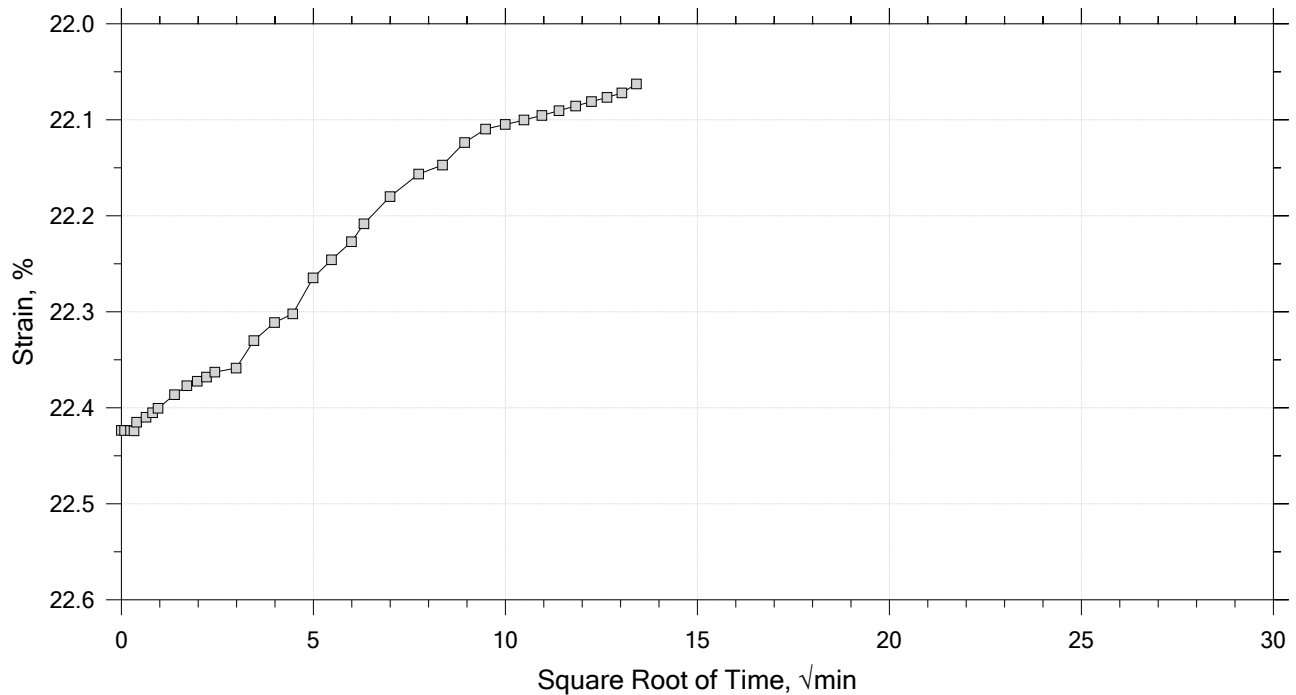
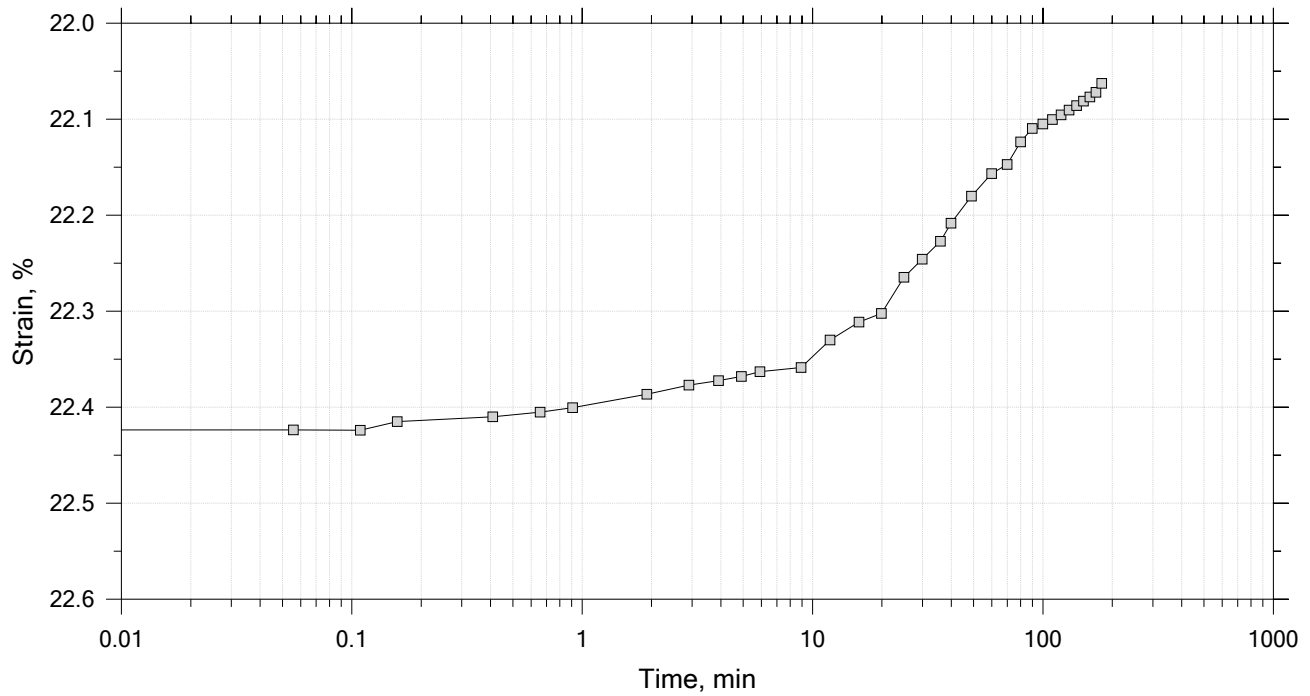
	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.78	Liquid Limit: 30
Initial Height: 1.00 in	Initial Void Ratio: 1.11	Plastic Limit: 17
Final Height: 0.78 in	Final Void Ratio: 0.645	Plasticity Index: 13

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	C961	RING		A2554
Mass Container, gm	8.84	112.61	112.61	8.19
Mass Container + Wet Soil, gm	238.66	260.73	243.39	136.17
Mass Container + Dry Soil, gm	173.72	218.79	218.79	112.1
Mass Dry Soil, gm	164.88	106.18	106.18	103.91
Water Content, %	39.39	39.49	23.16	23.16
Void Ratio	---	1.11	0.64	---
Degree of Saturation, %	---	99.16	100.00	---
Dry Unit Weight, pcf	---	82.407	105.65	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

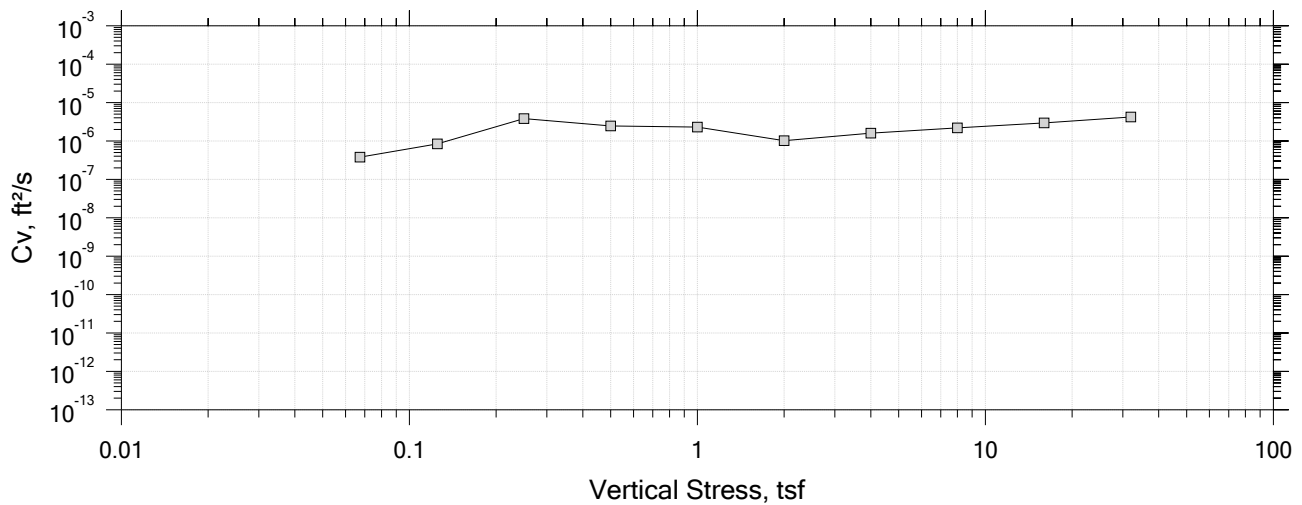
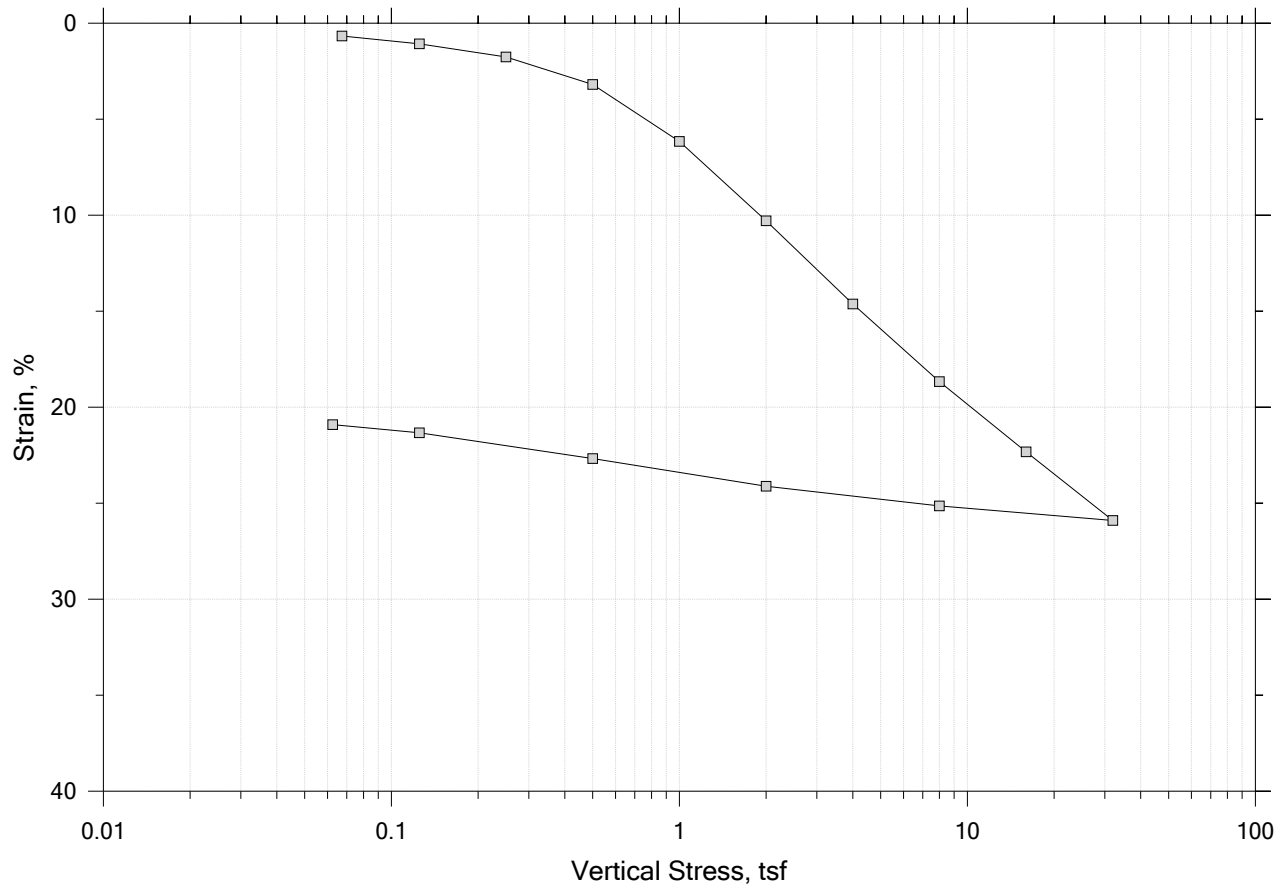
Square Root of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer, ME	Project No.: GTX-308853
	Boring No.: HB-BE-107A	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 9/29/2018	Depth: 10-12 ft
	Test No.: IP-4	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System W, Swell Pressure = 0.0645 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

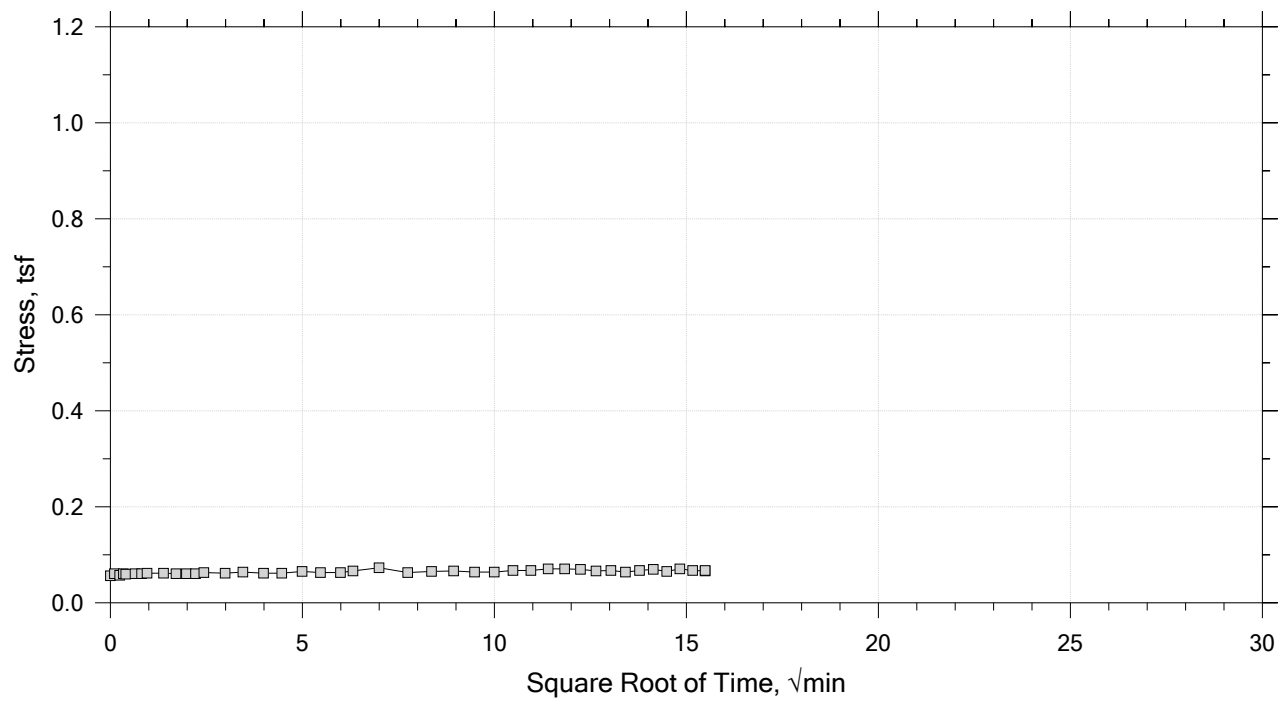
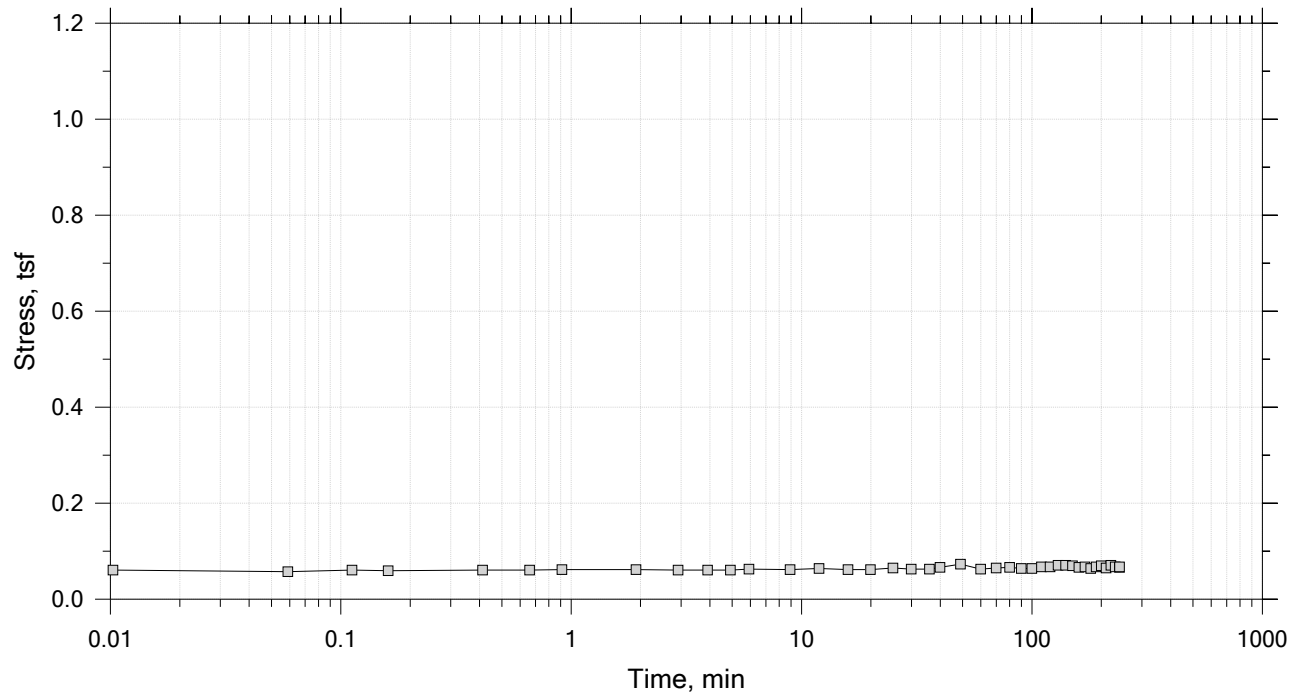
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0673 tsf



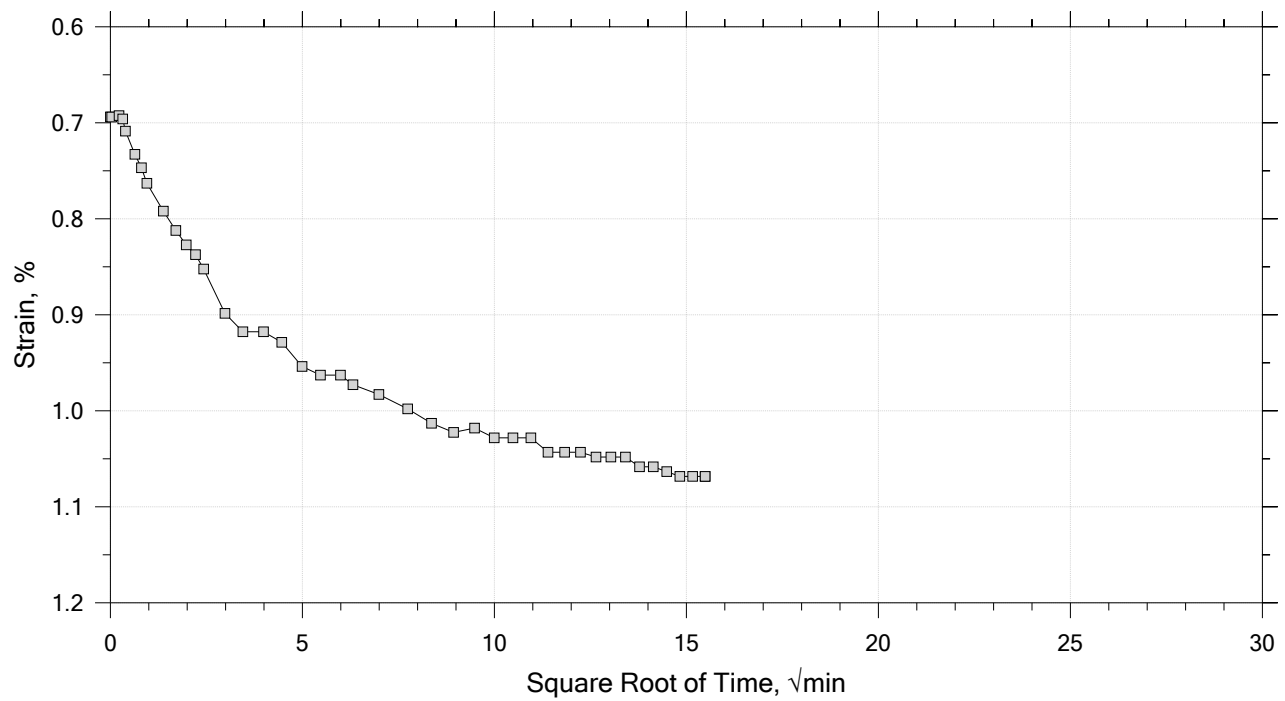
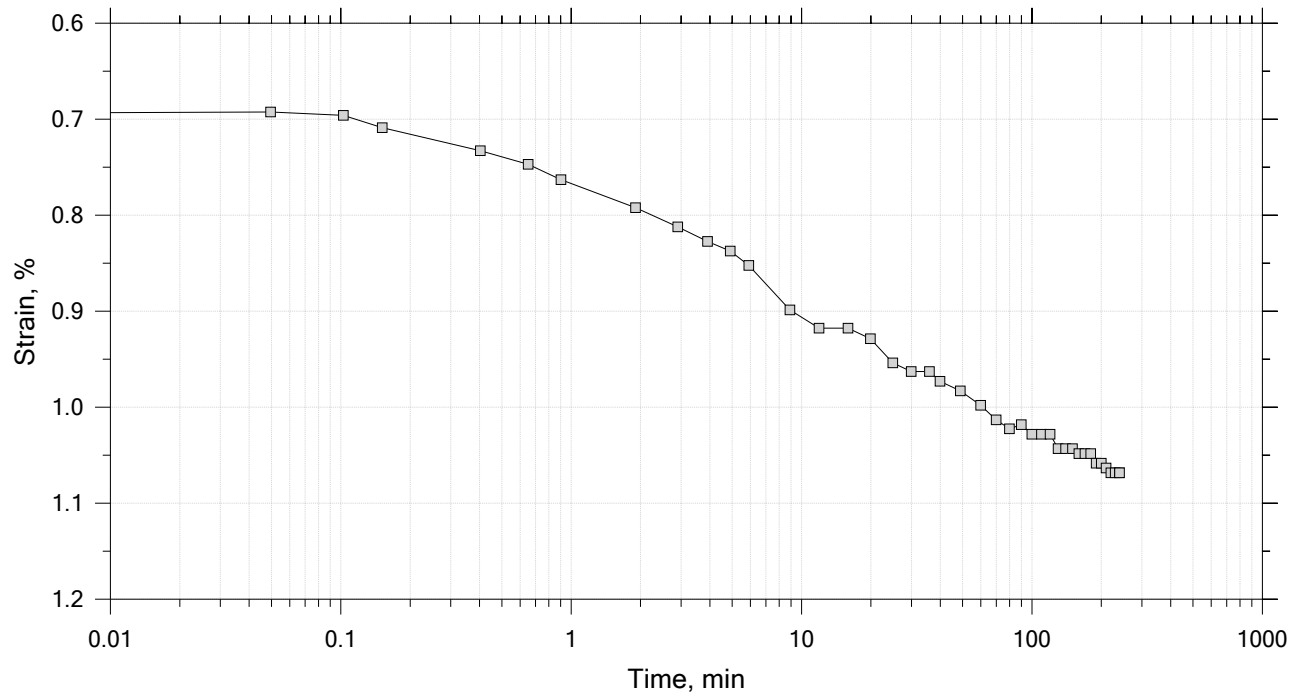
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



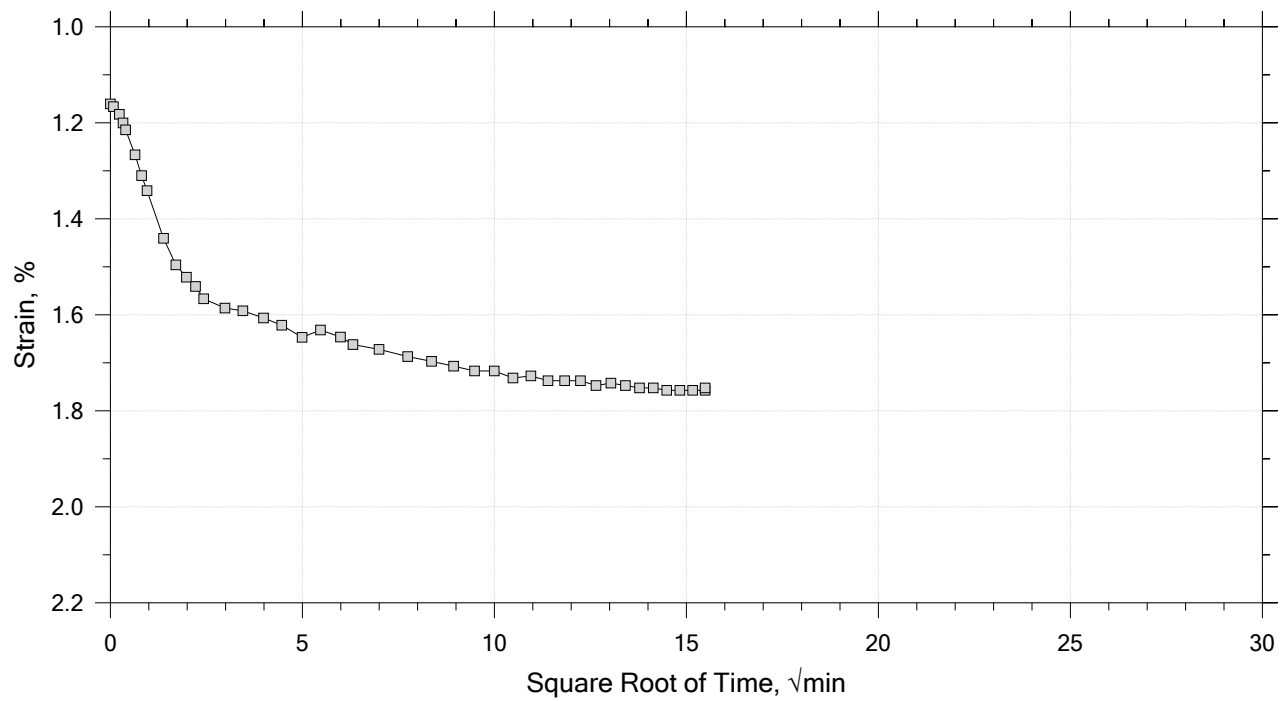
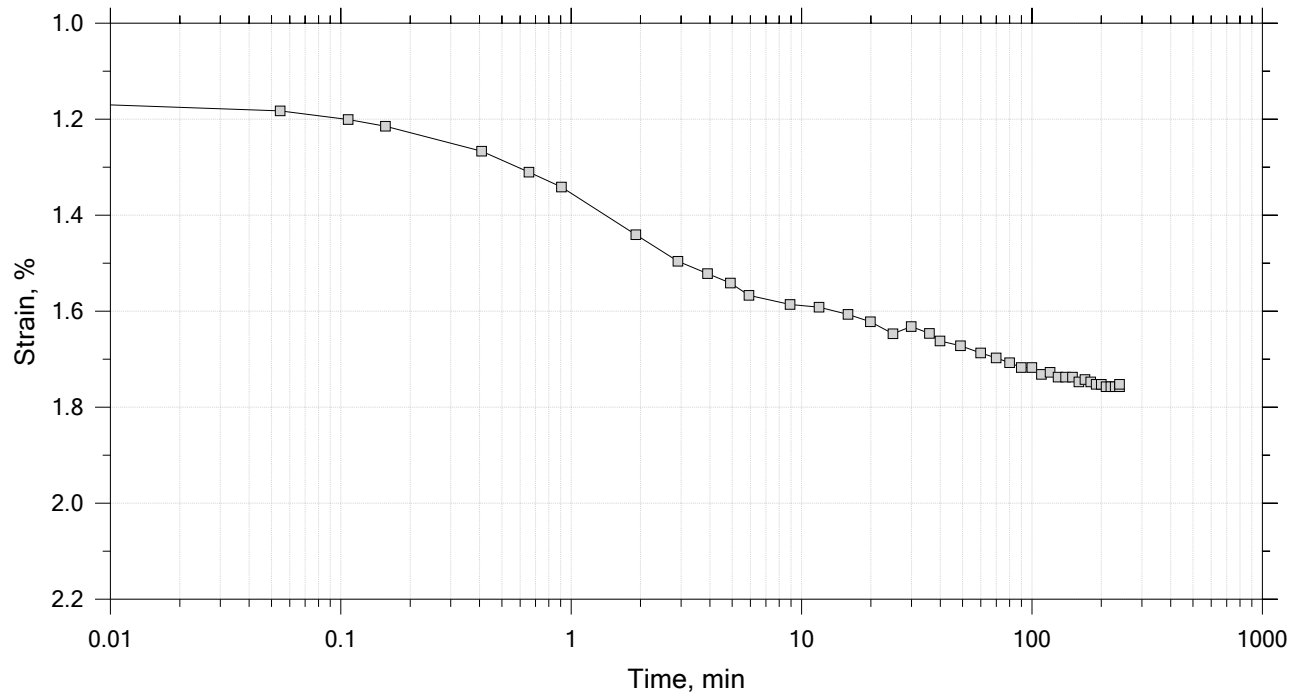
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



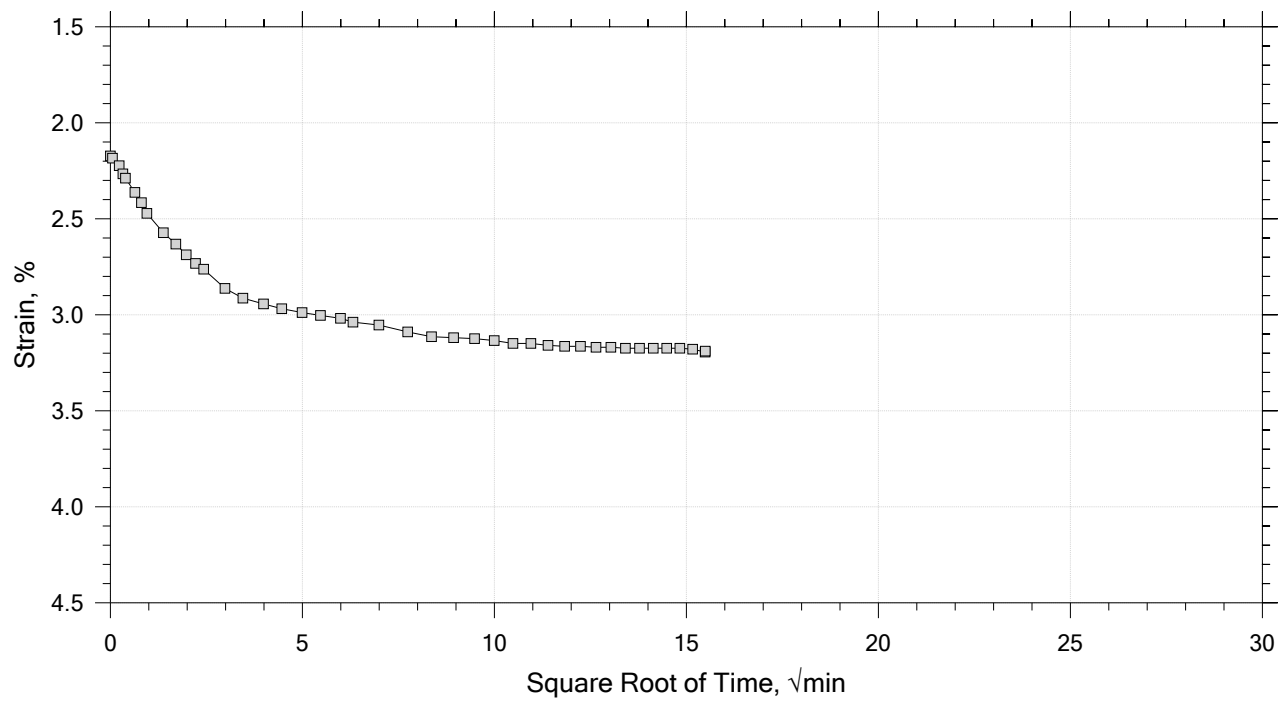
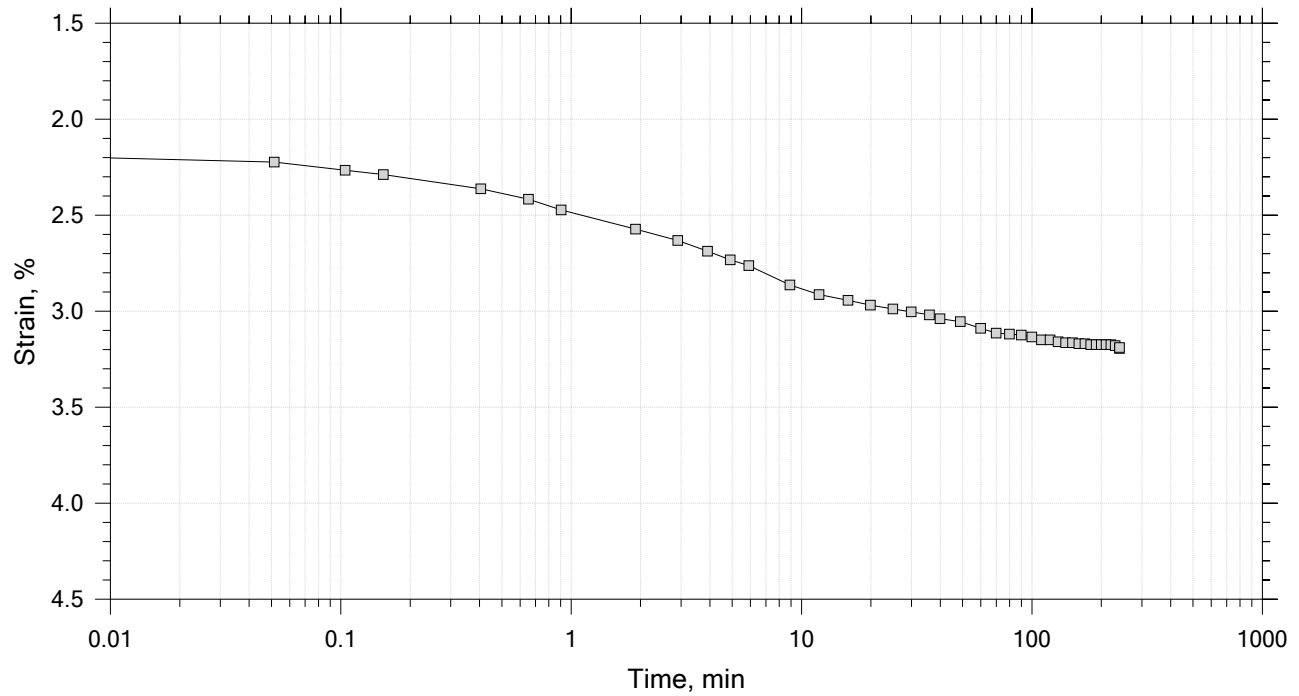
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



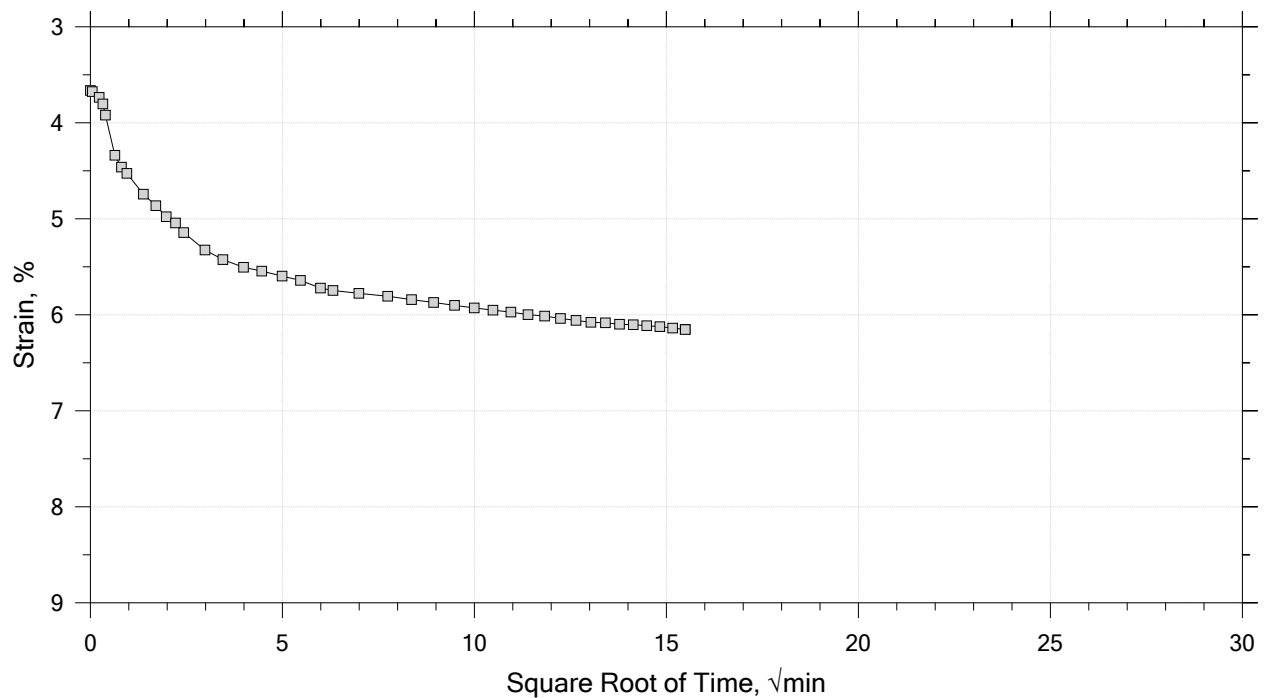
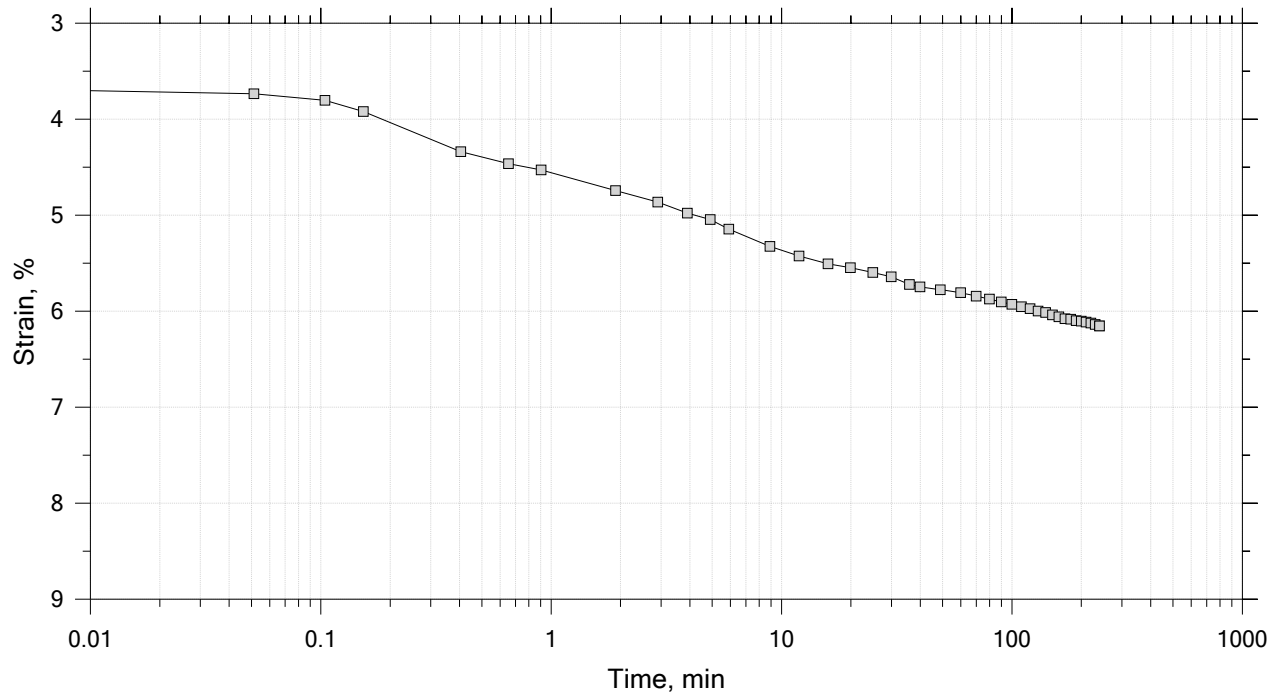
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



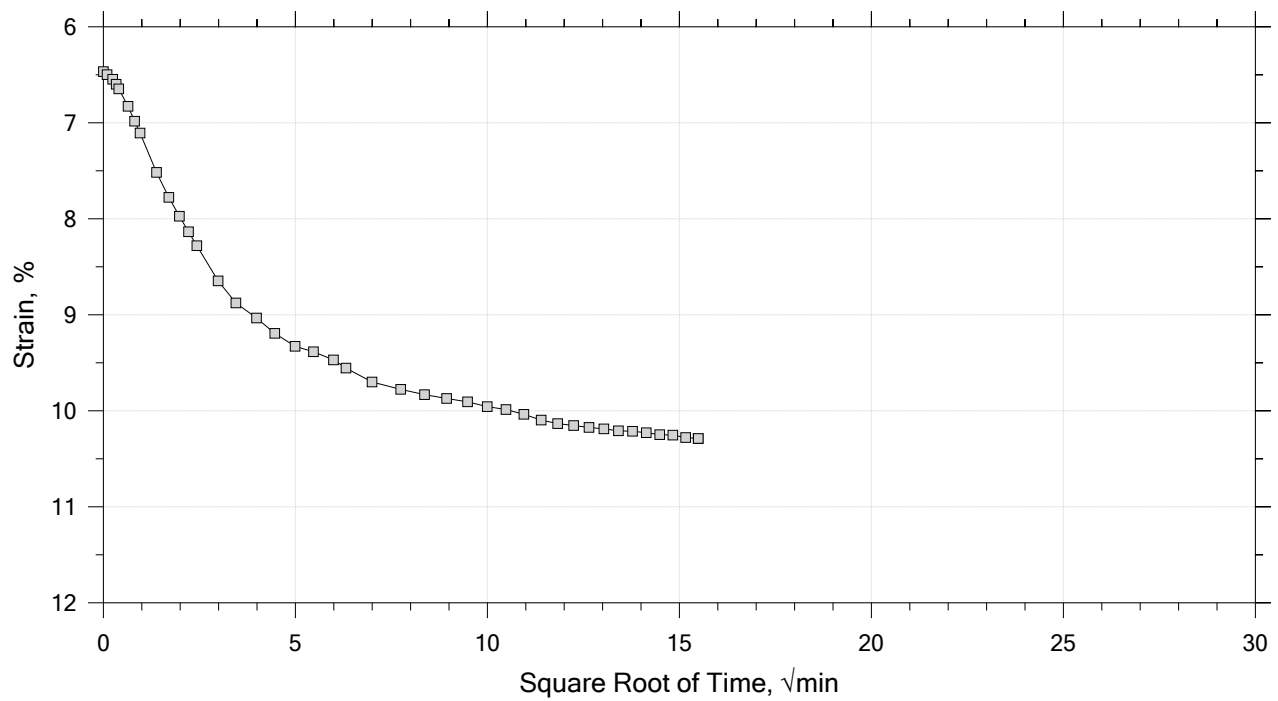
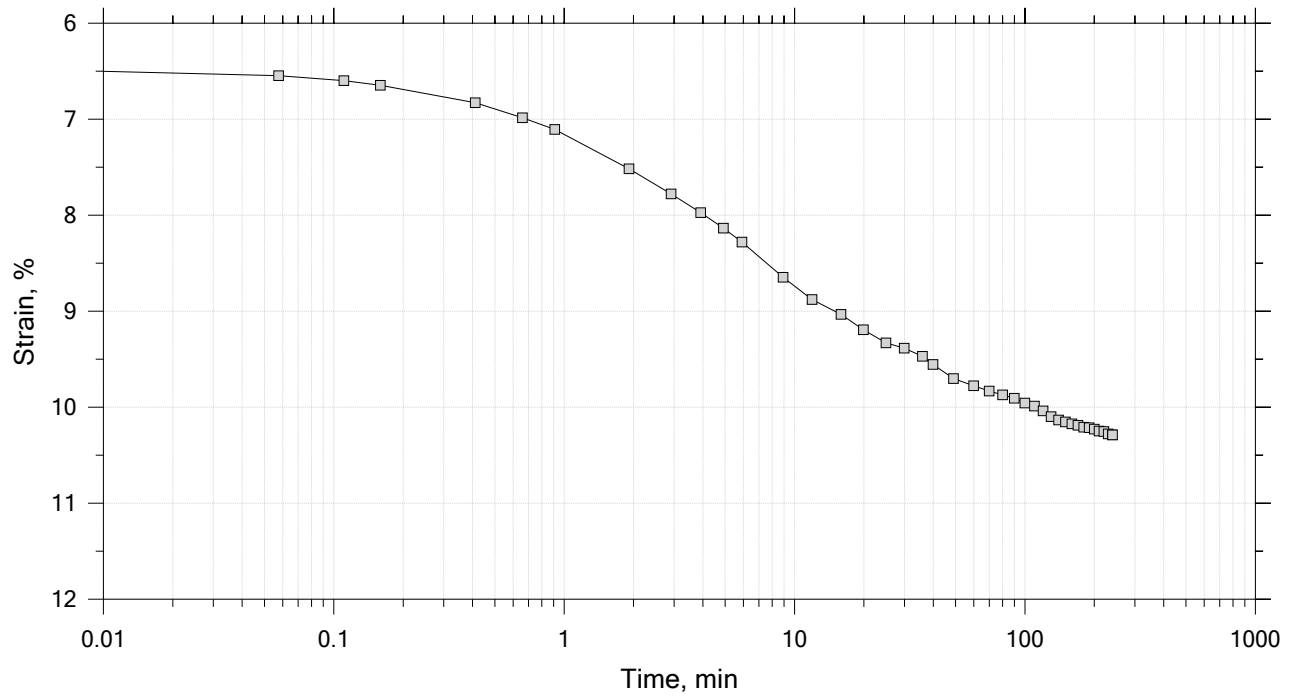
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



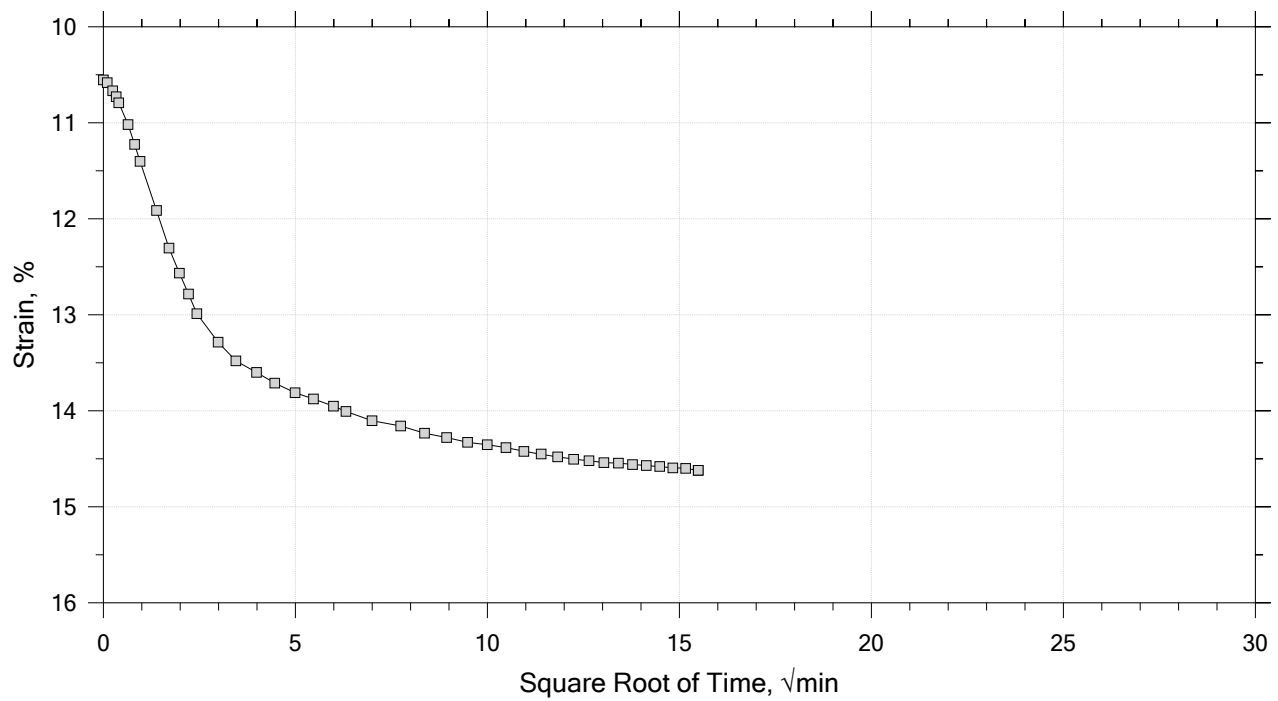
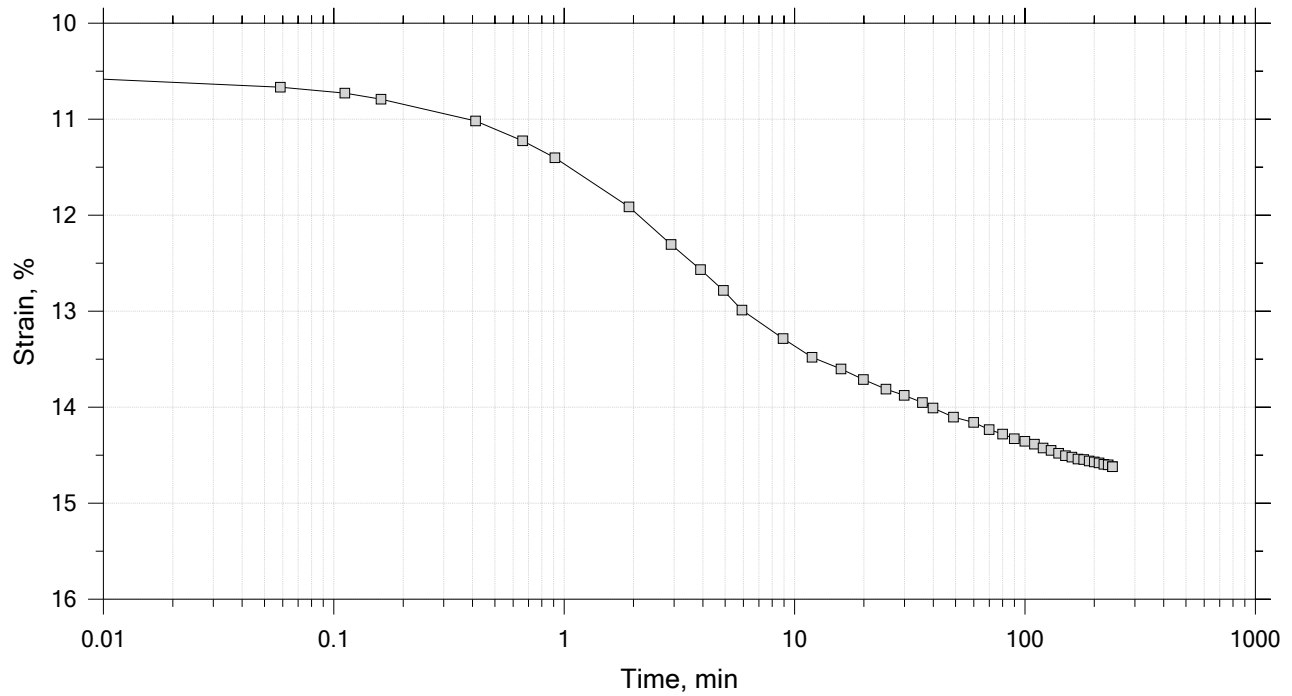
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



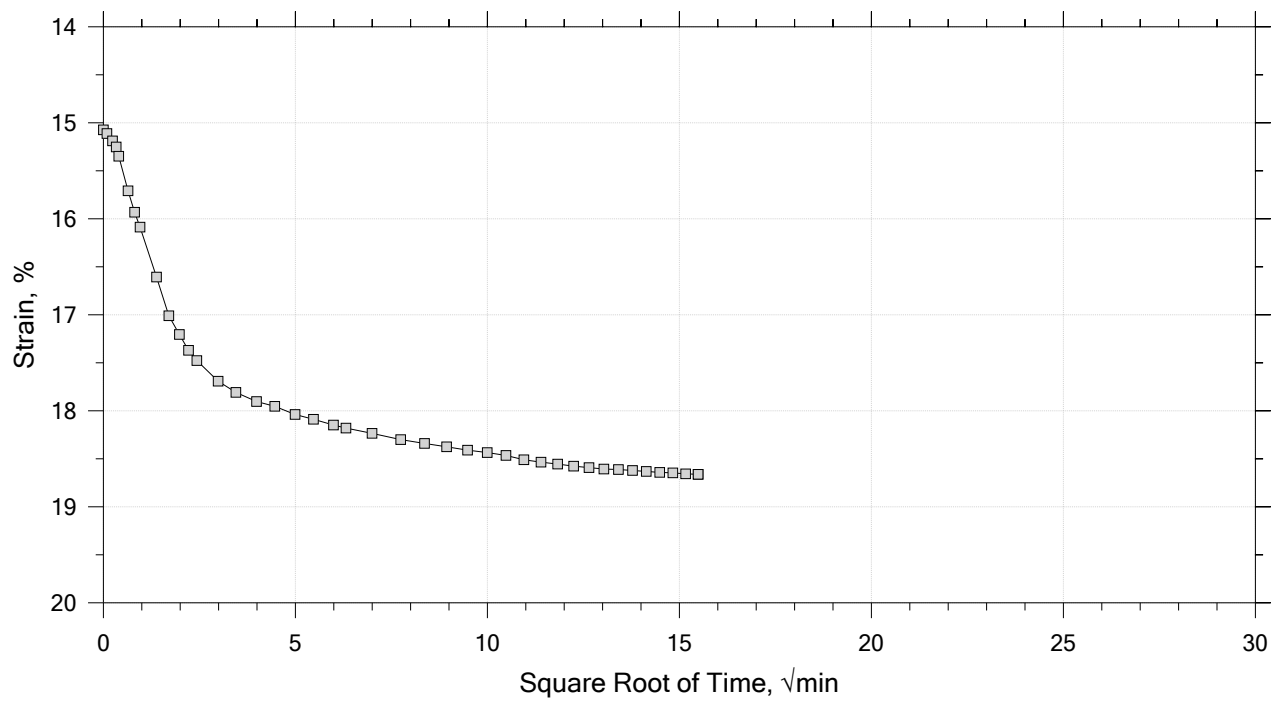
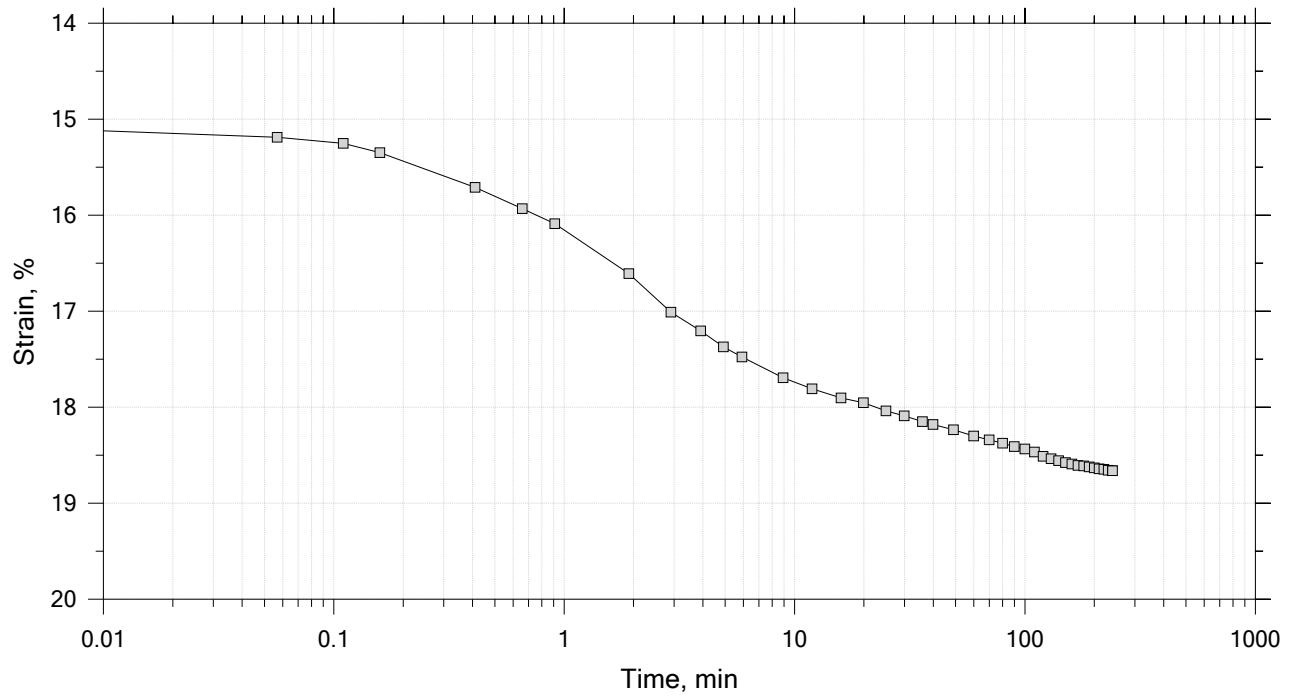
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



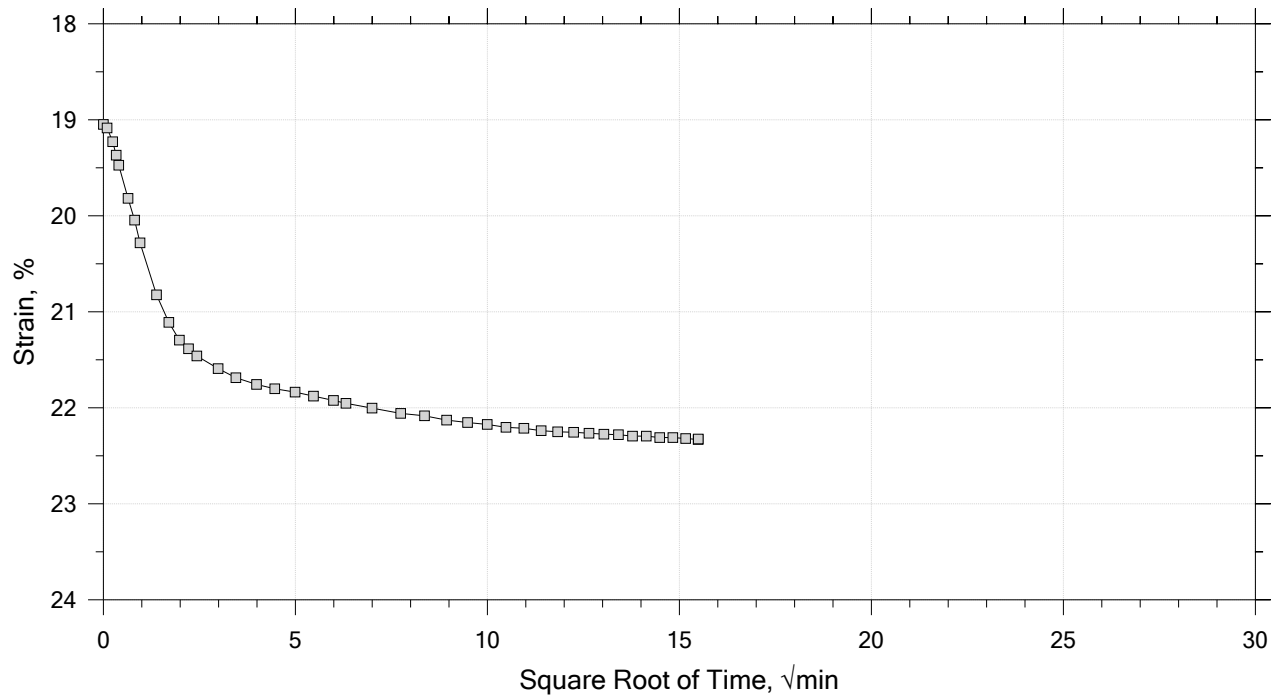
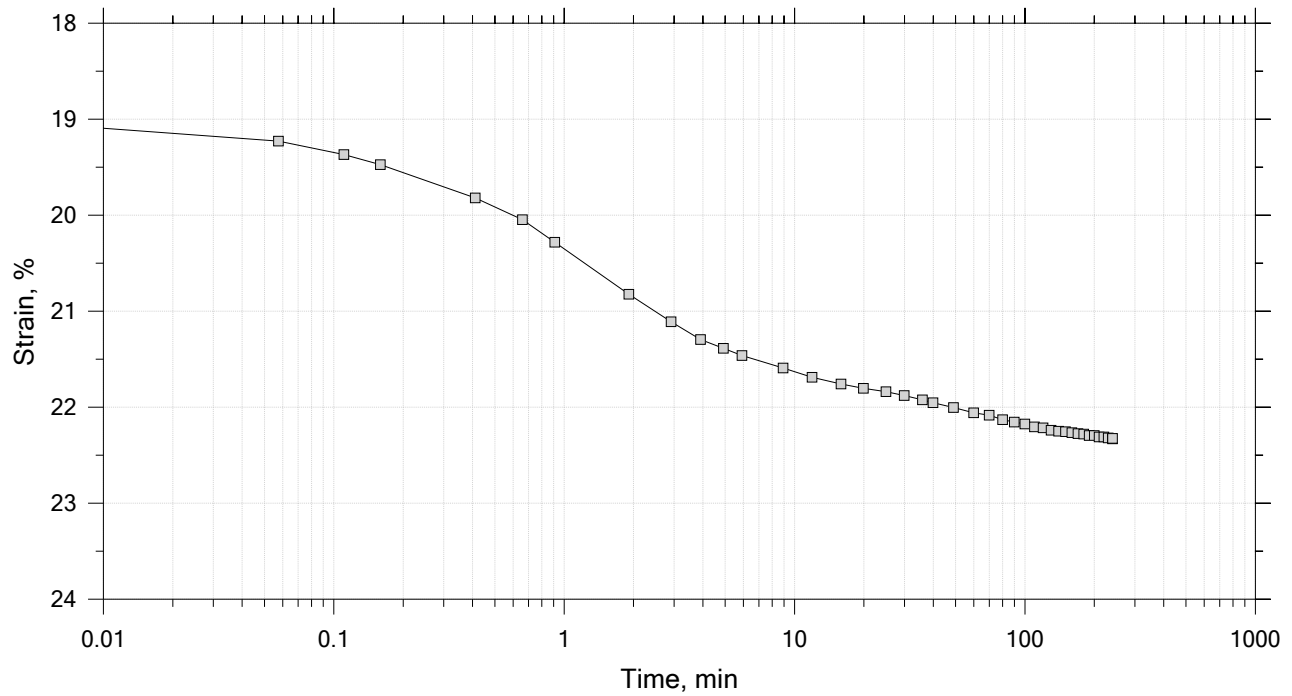
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



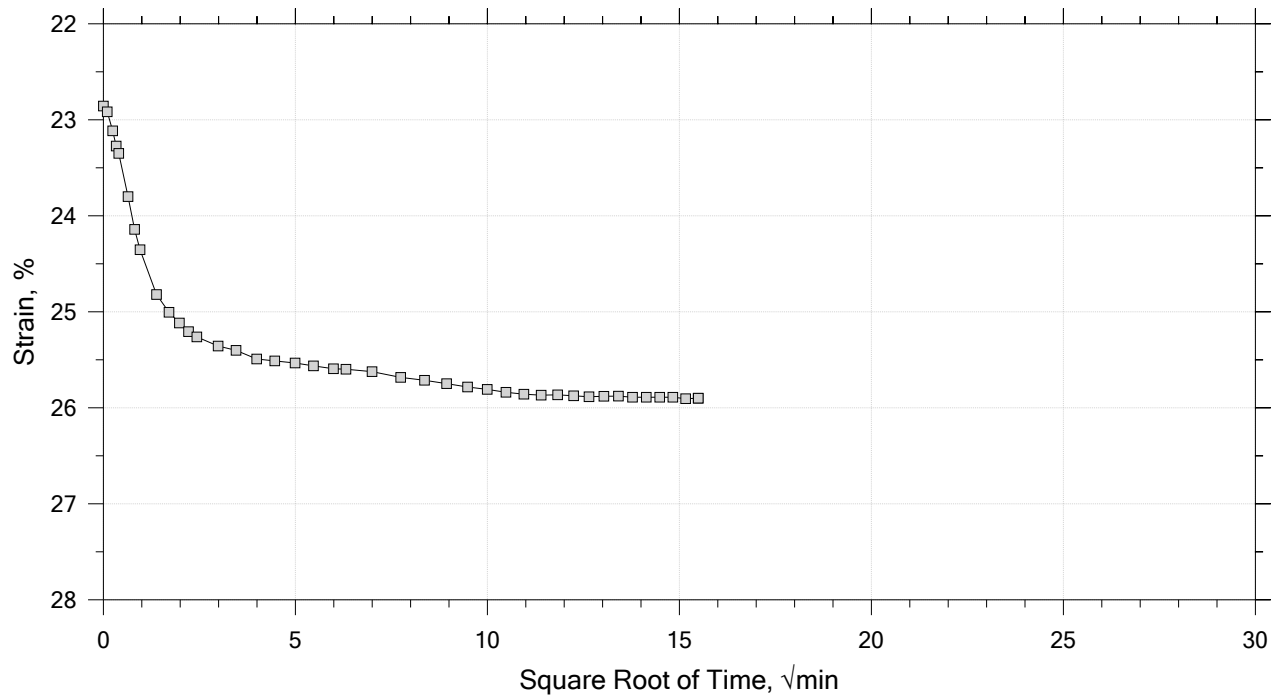
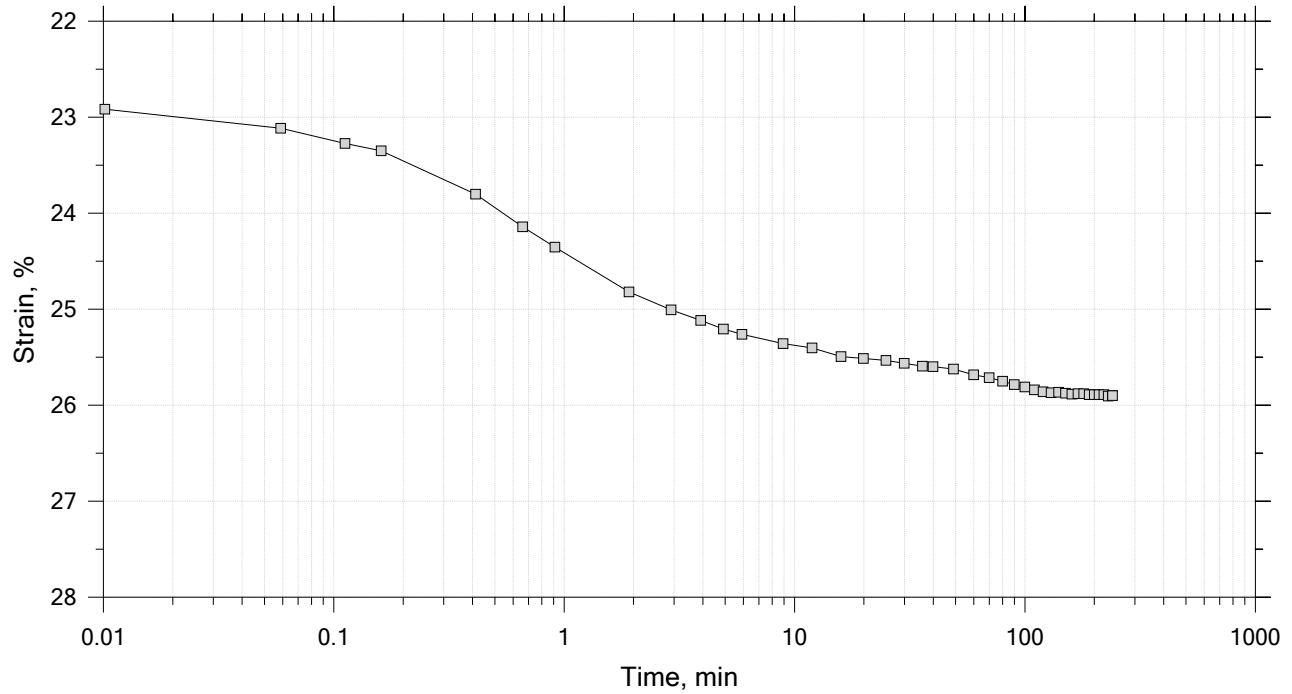
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



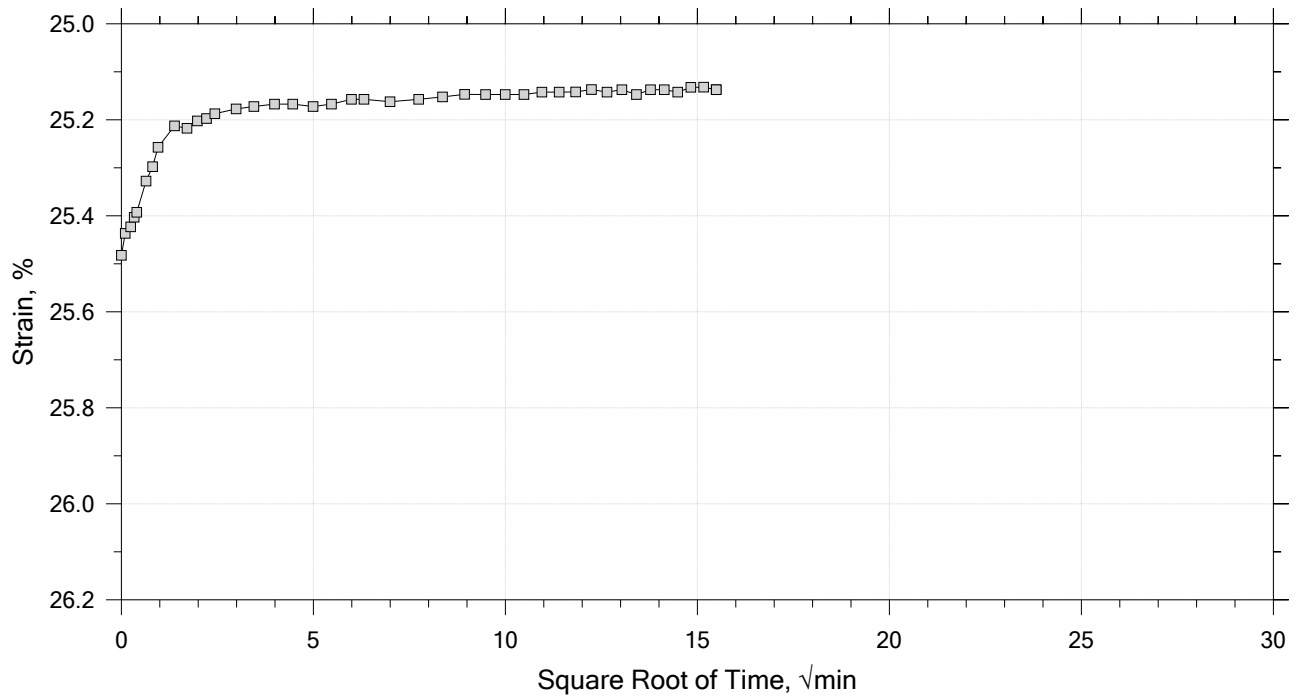
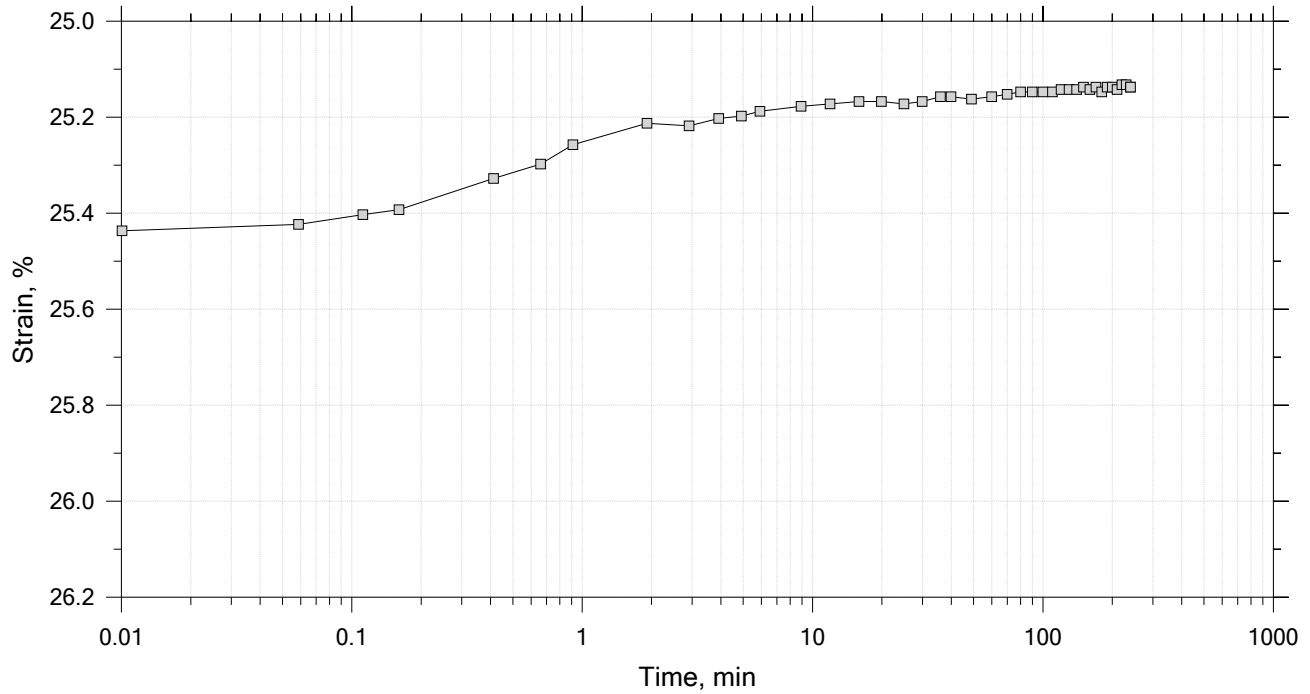
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



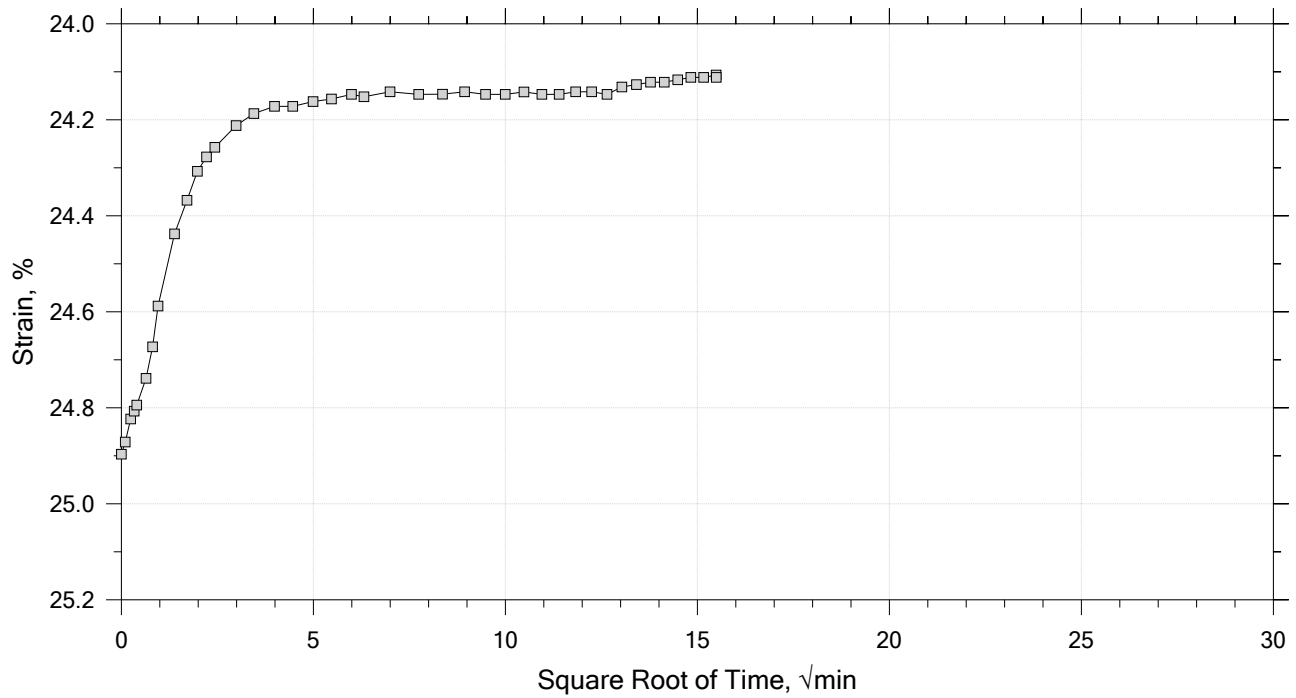
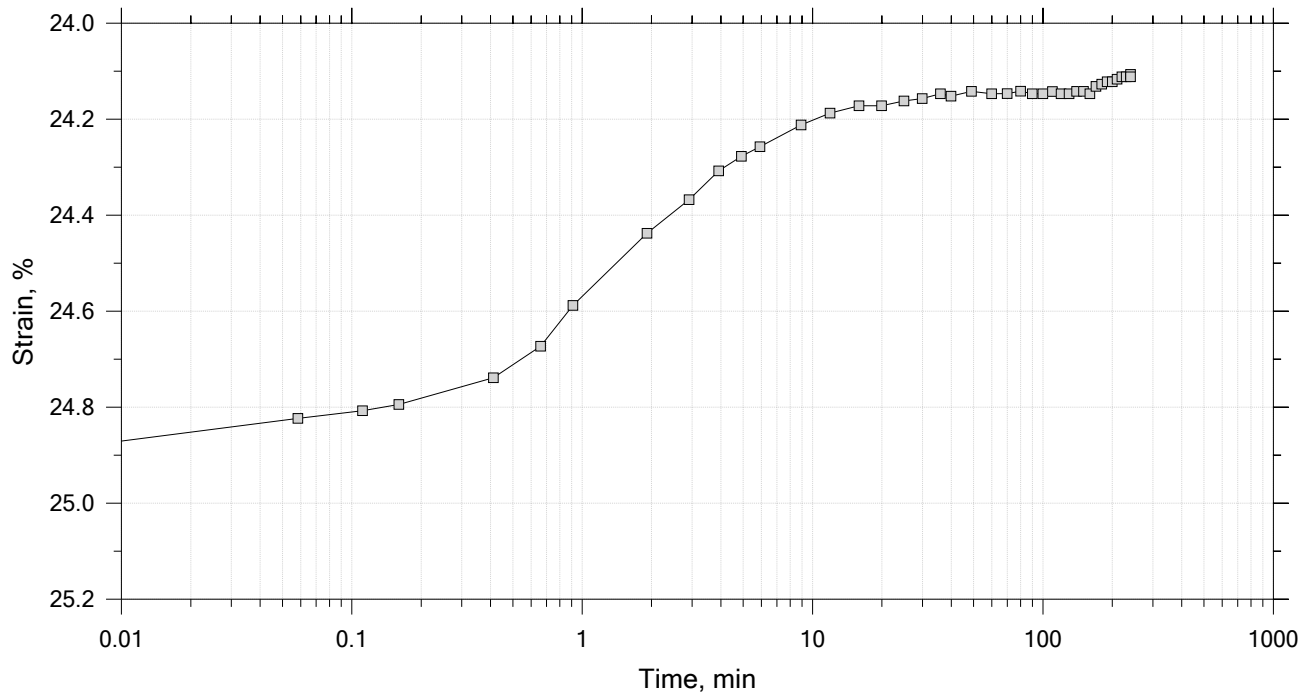
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



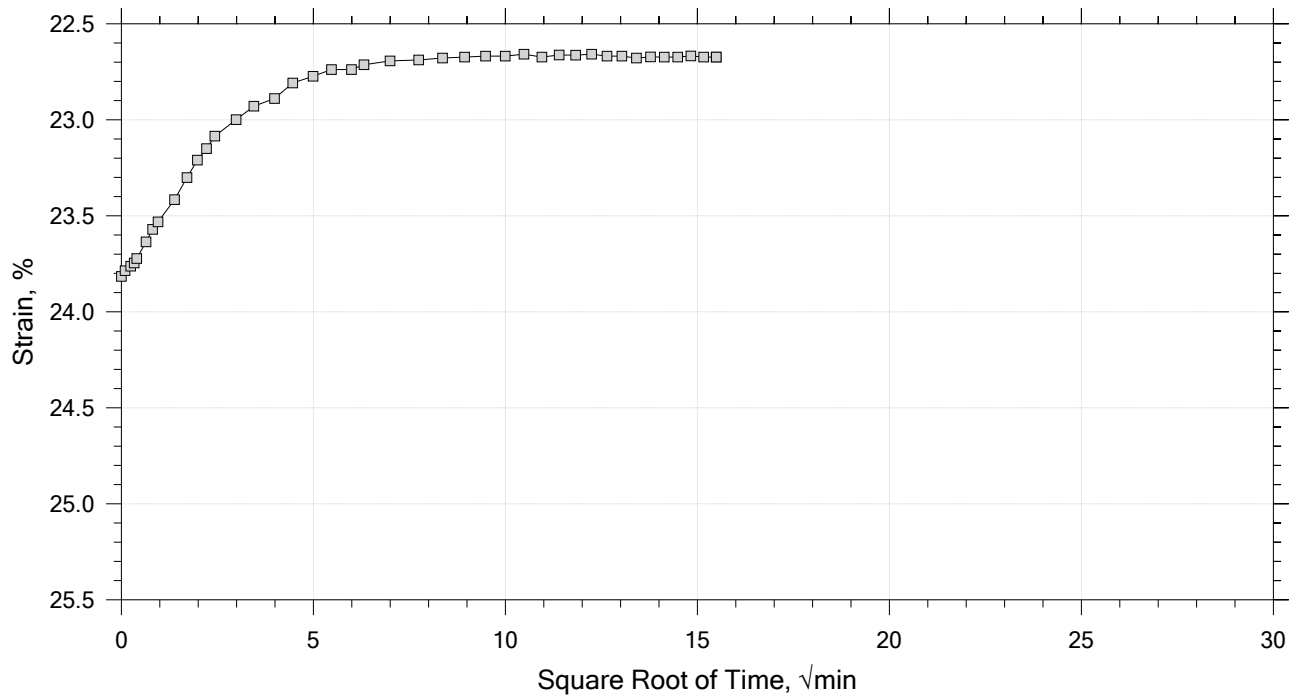
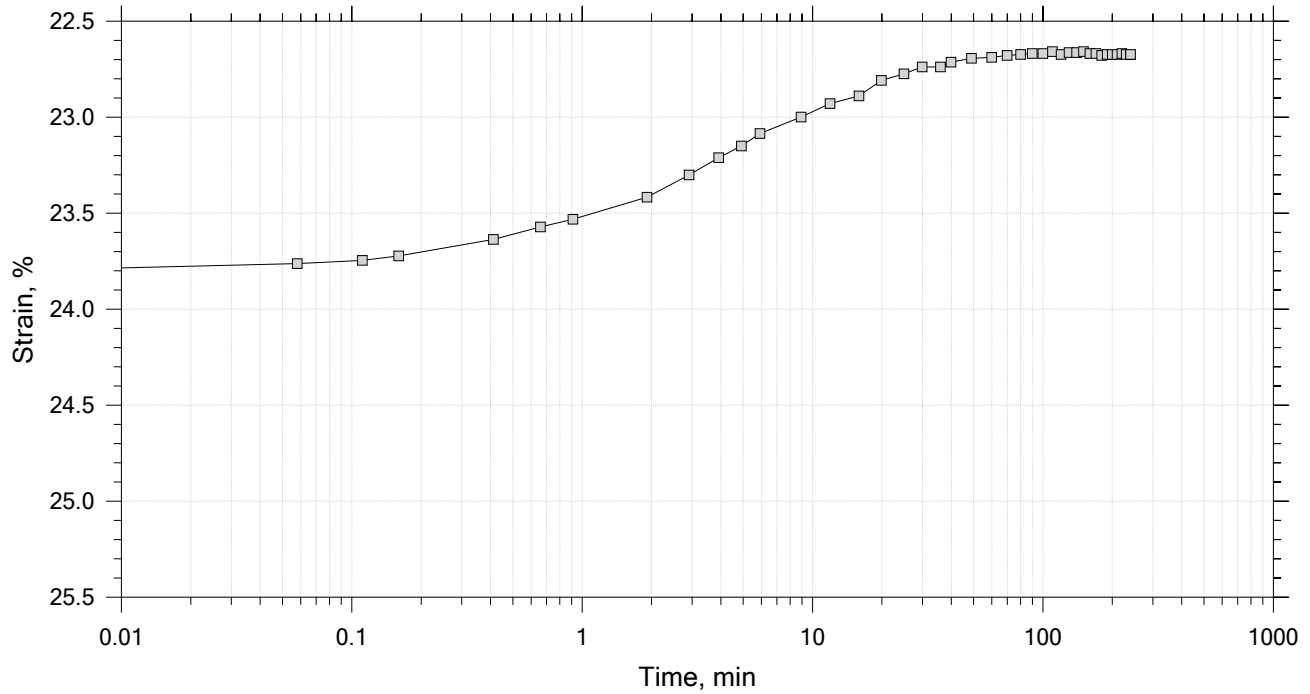
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



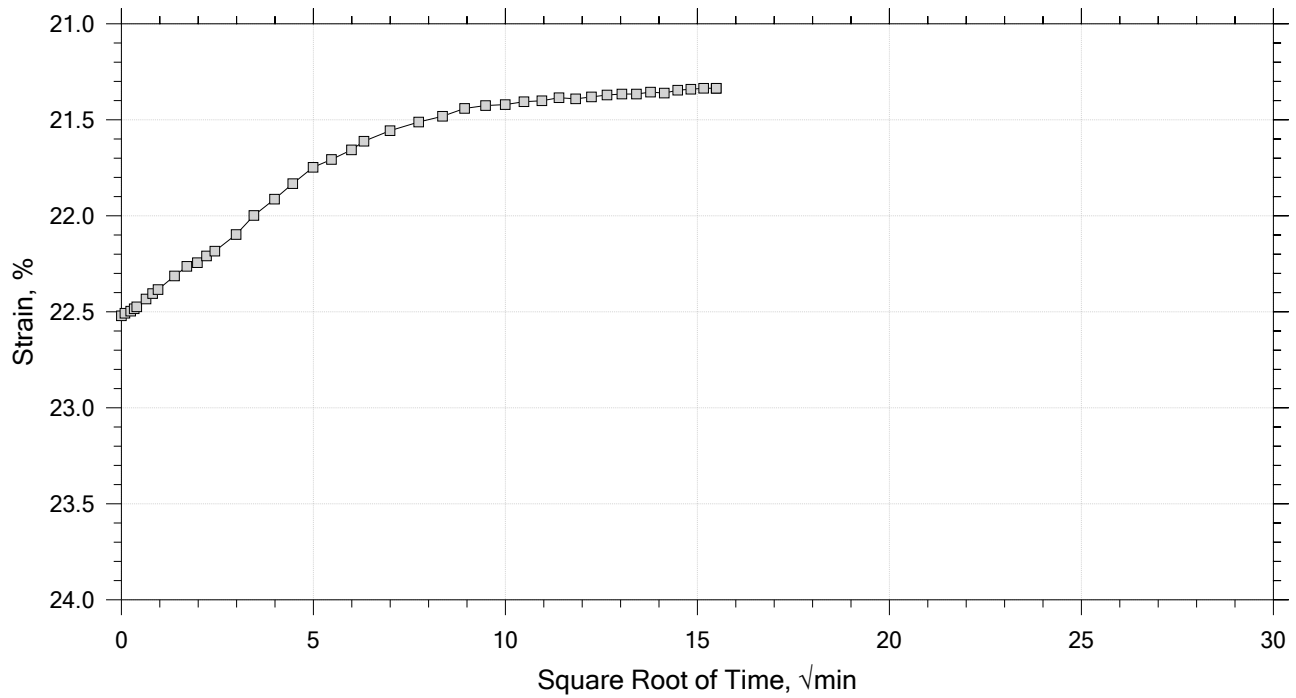
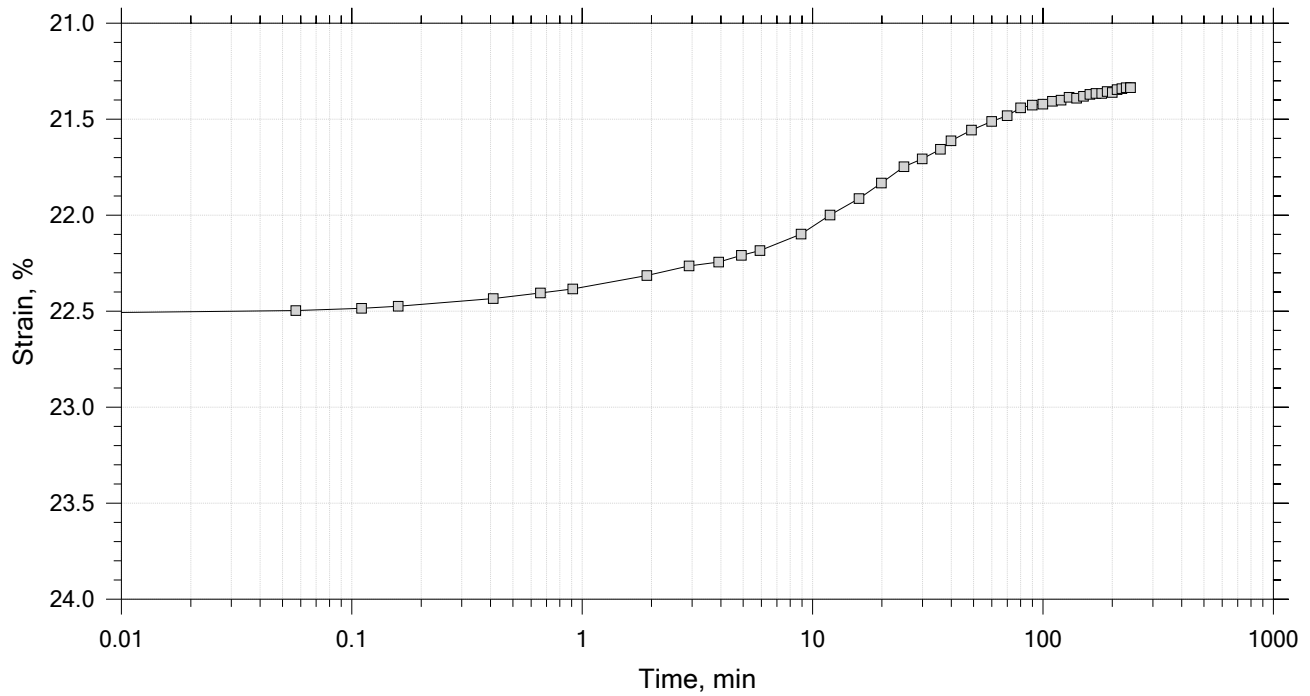
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	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



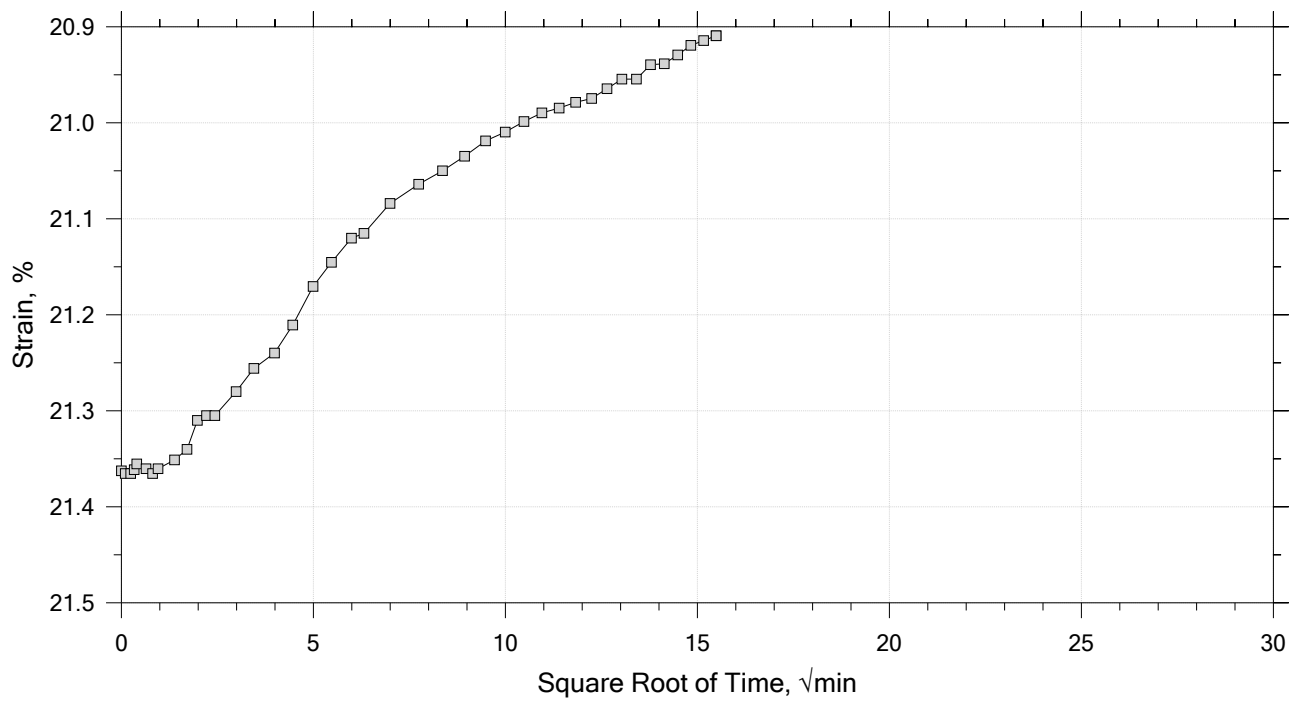
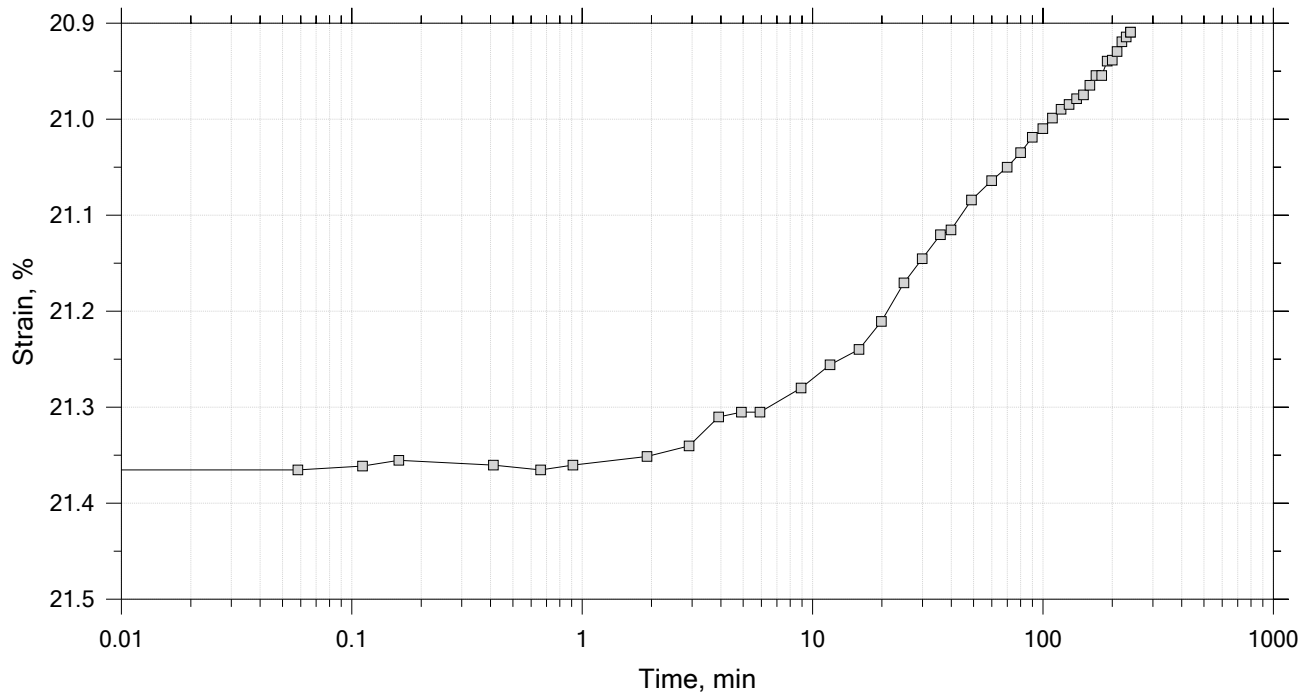
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 35
Initial Height: 1.00 in	Initial Void Ratio: 0.89	Plastic Limit: 19
Final Height: 0.79 in	Final Void Ratio: 0.495	Plasticity Index: 16

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	B2461	RING		D1558
Mass Container, gm	9.58	111.23	111.23	8.45
Mass Container + Wet Soil, gm	192.53	264.1	249.44	146.31
Mass Container + Dry Soil, gm	144.42	228.39	228.39	125.31
Mass Dry Soil, gm	134.84	117.16	117.16	116.86
Water Content, %	35.68	30.48	17.97	17.97
Void Ratio	---	0.89	0.49	---
Degree of Saturation, %	---	94.30	100.00	---
Dry Unit Weight, pcf	---	90.923	114.96	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

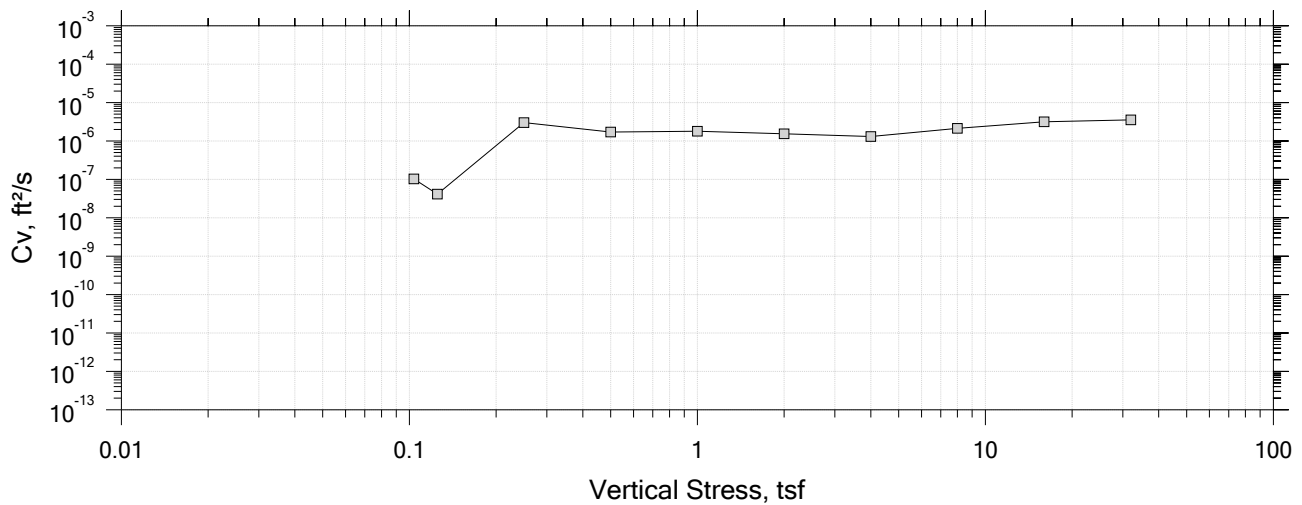
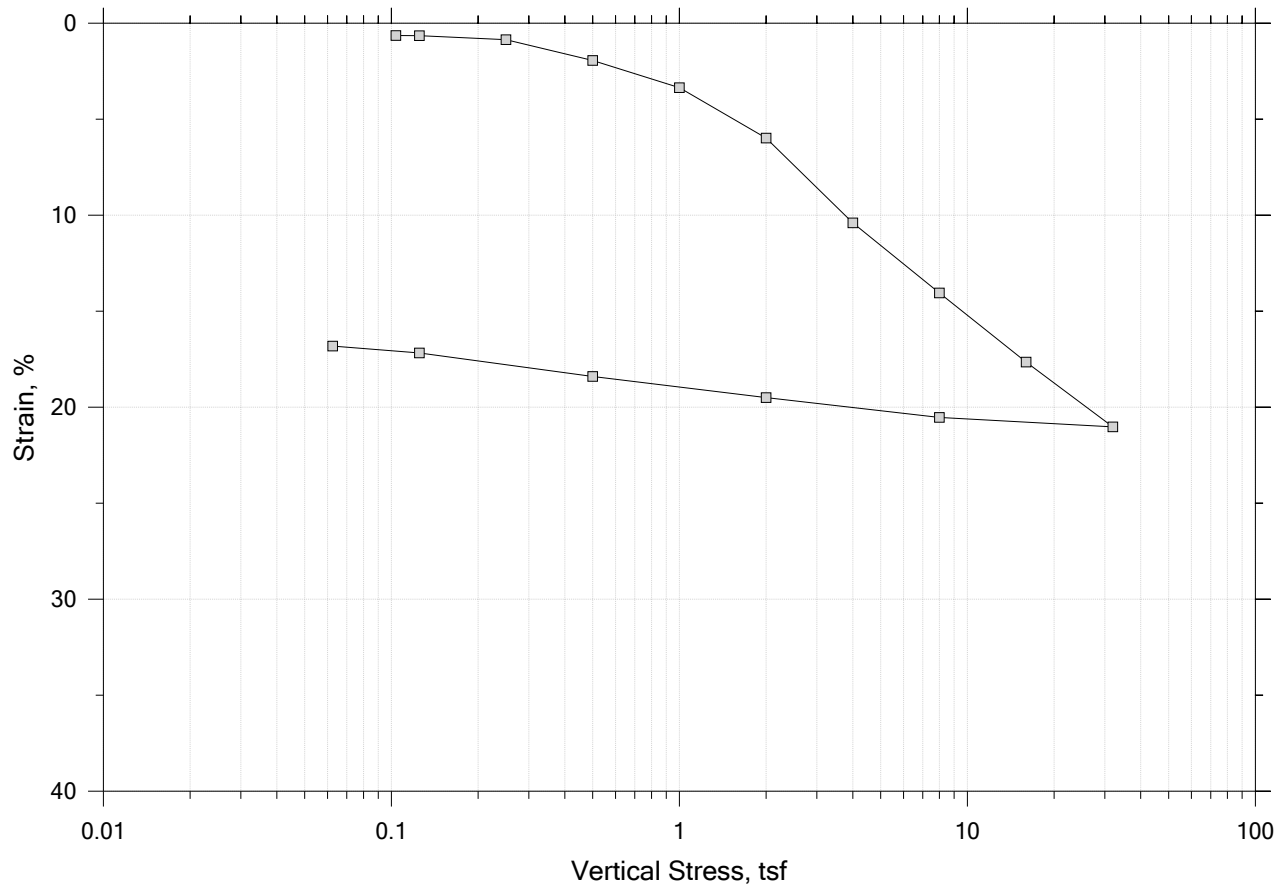
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-108	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/25/19	Depth: 12-14 ft
	Test No.: IP-20	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System Q, Swell Pressure = 0.0673 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

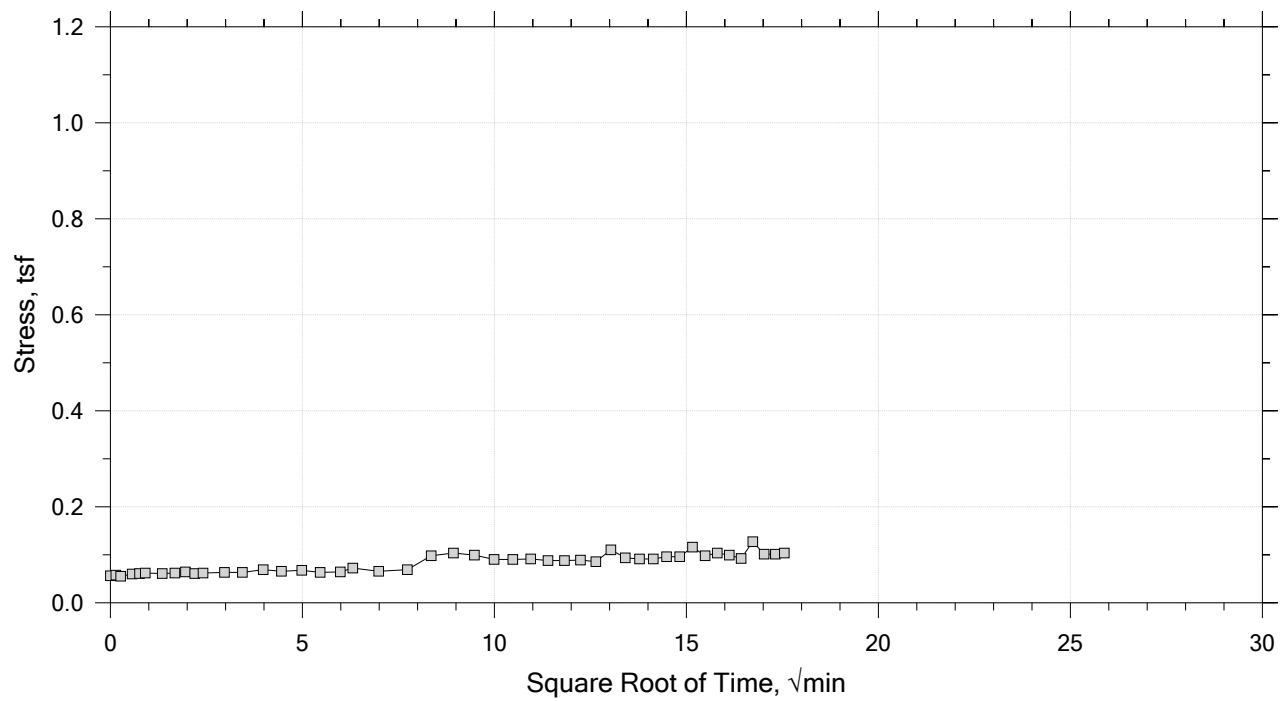
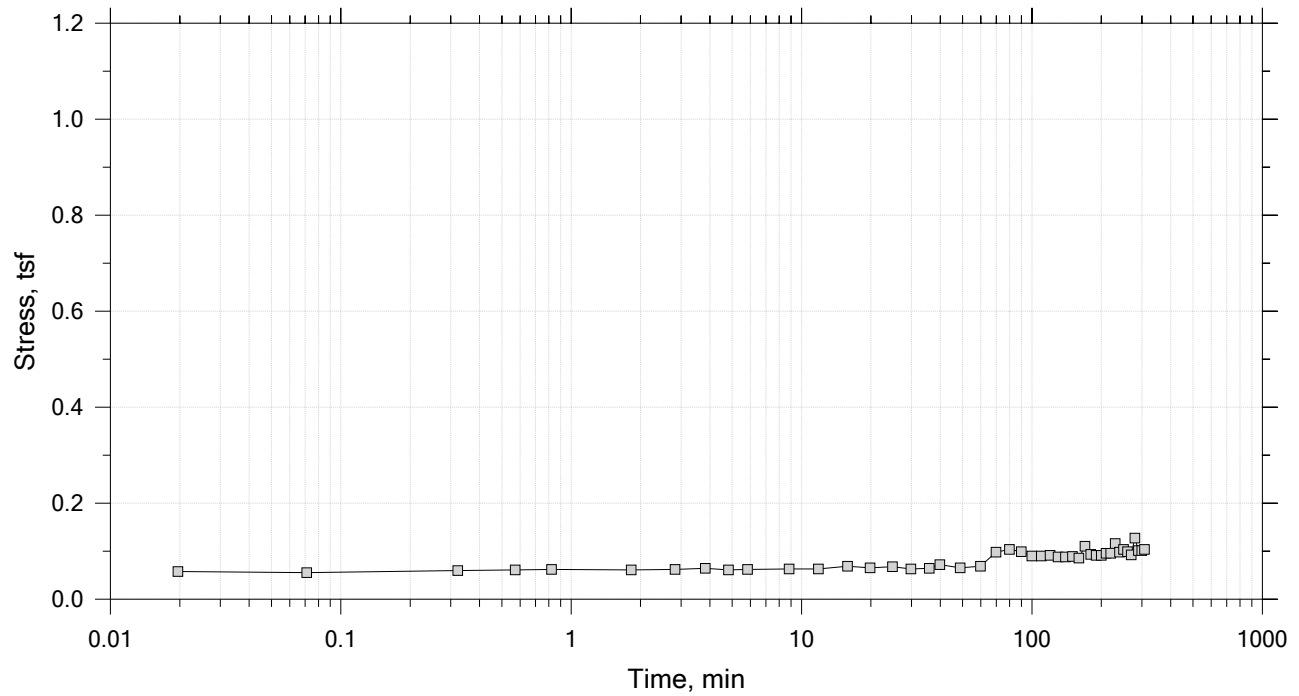
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.104 tsf



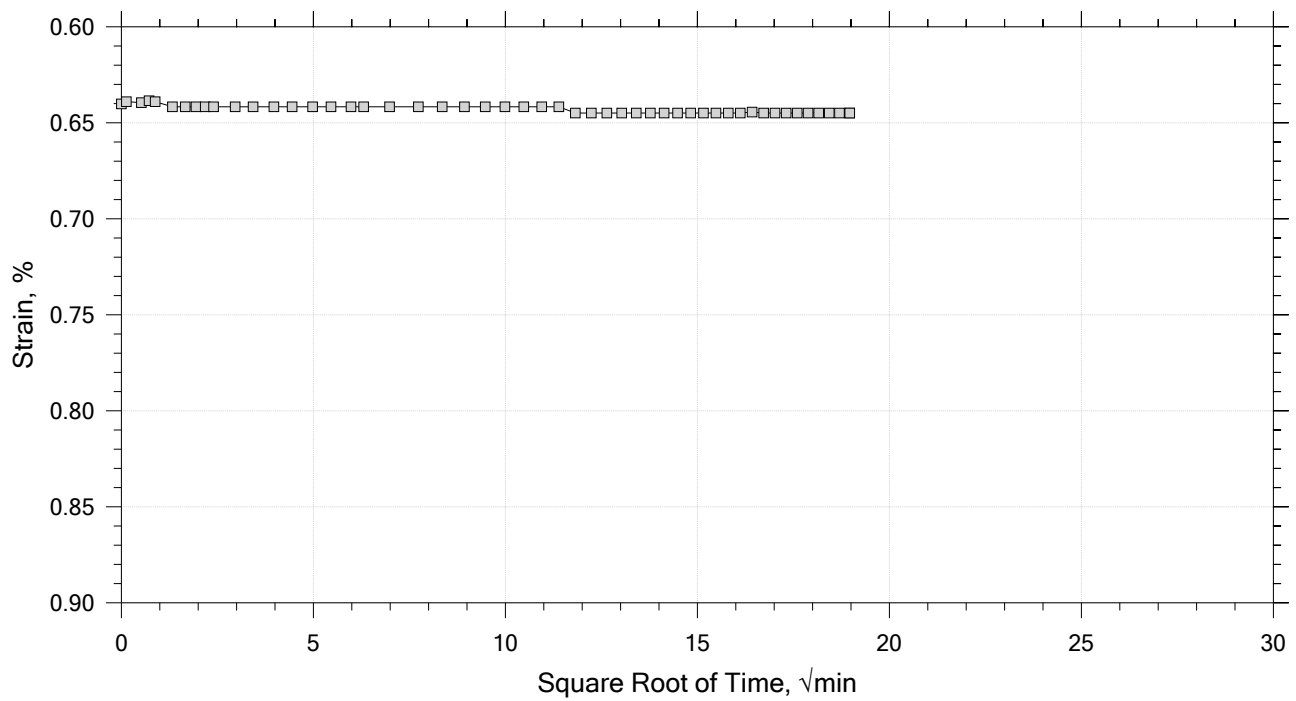
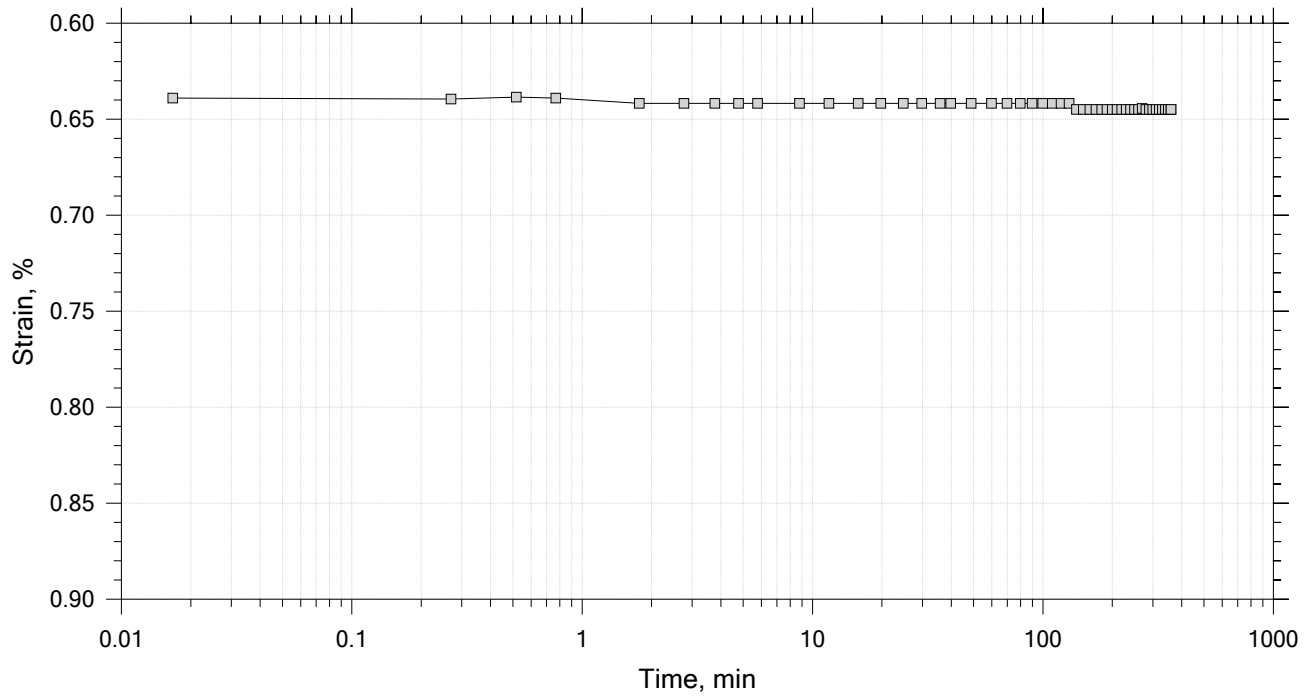
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



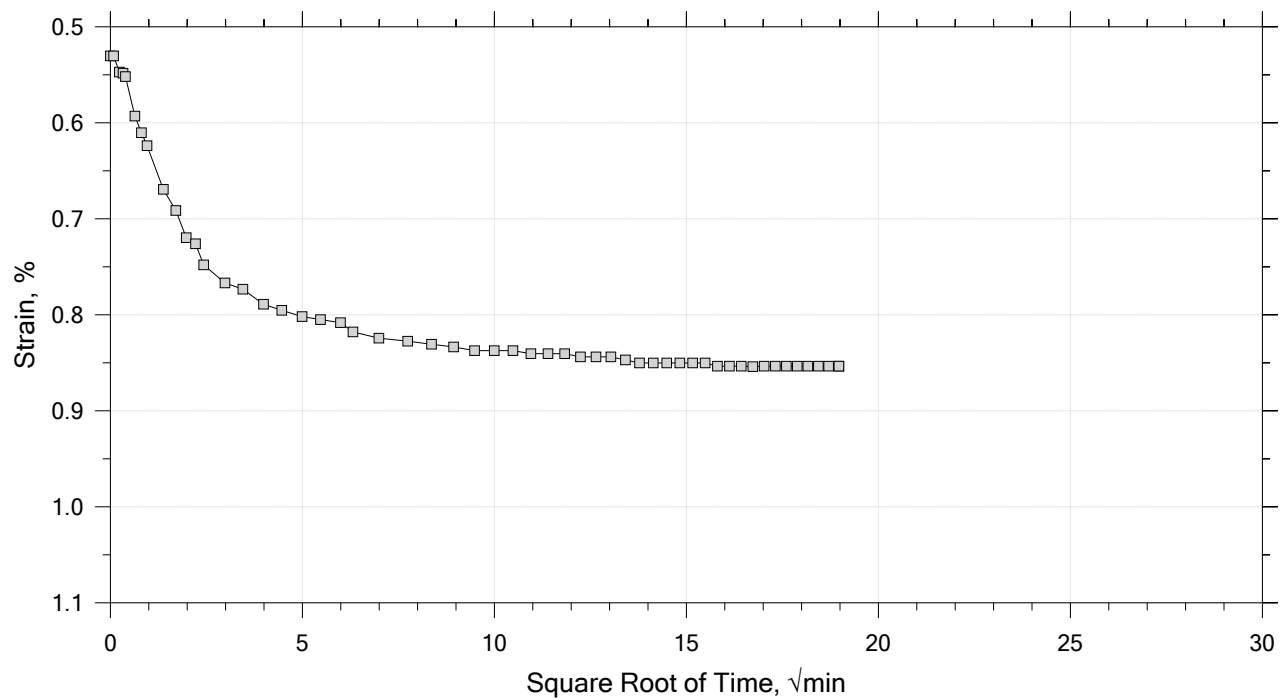
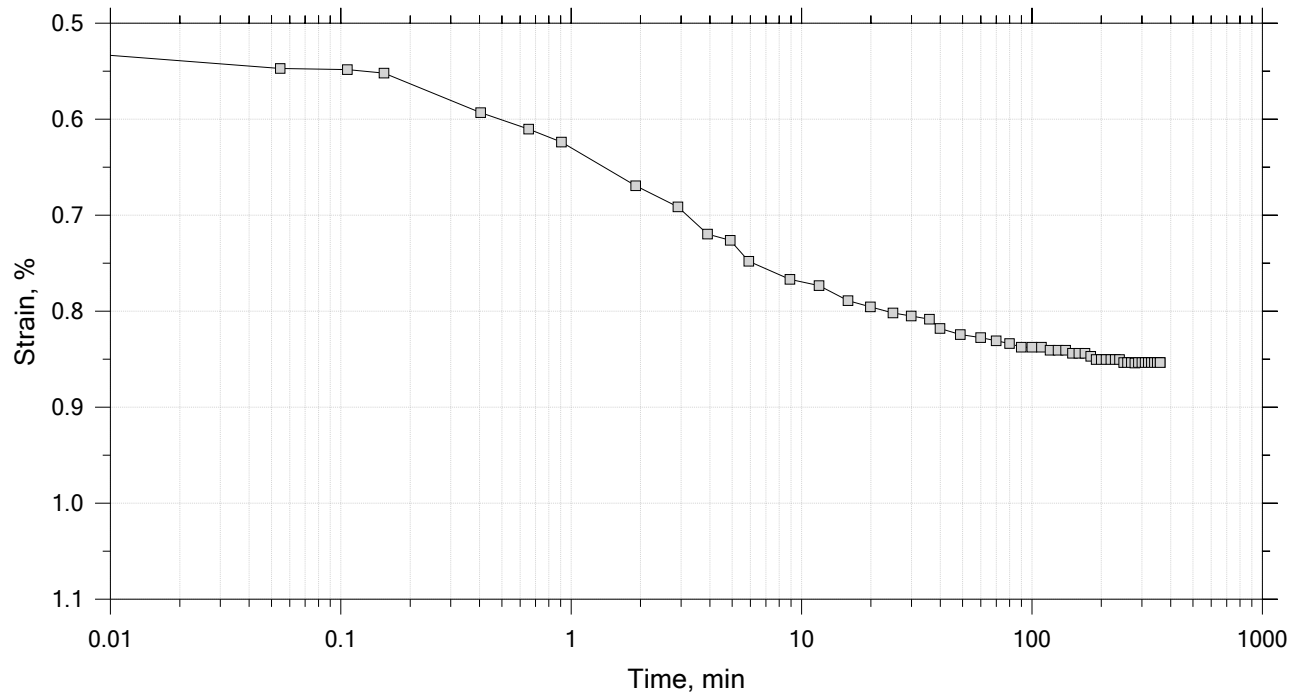
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

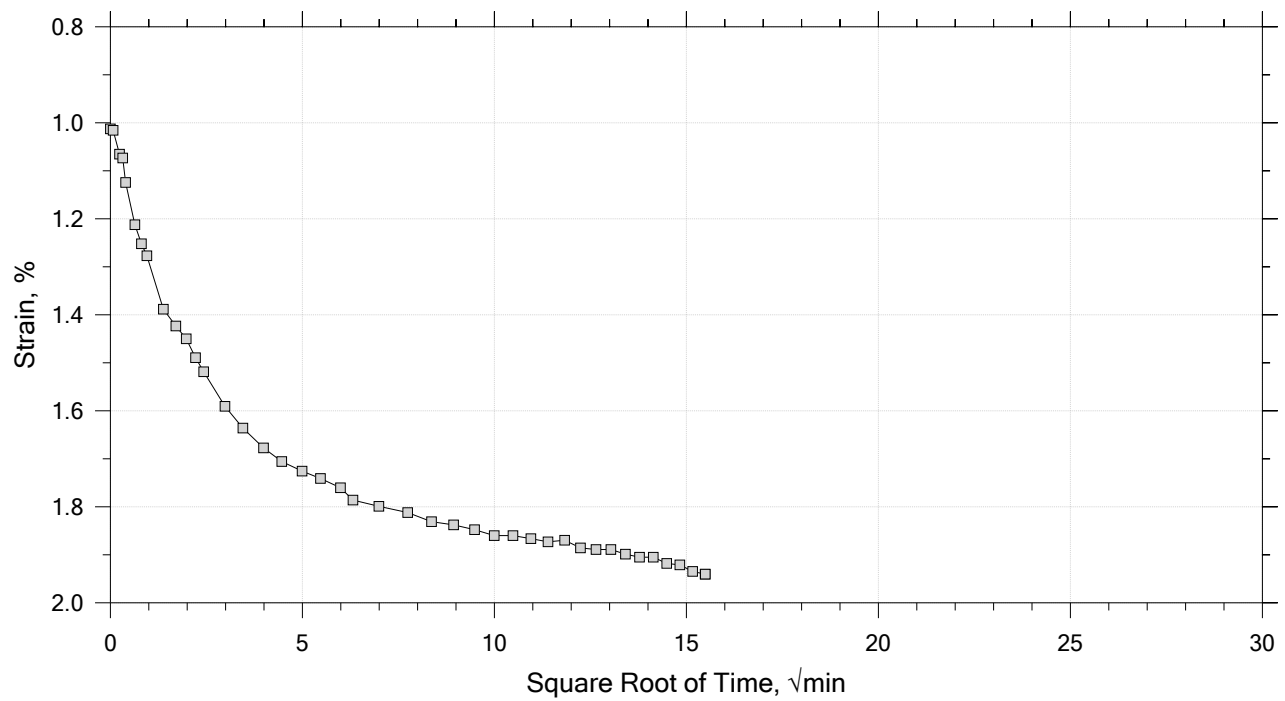
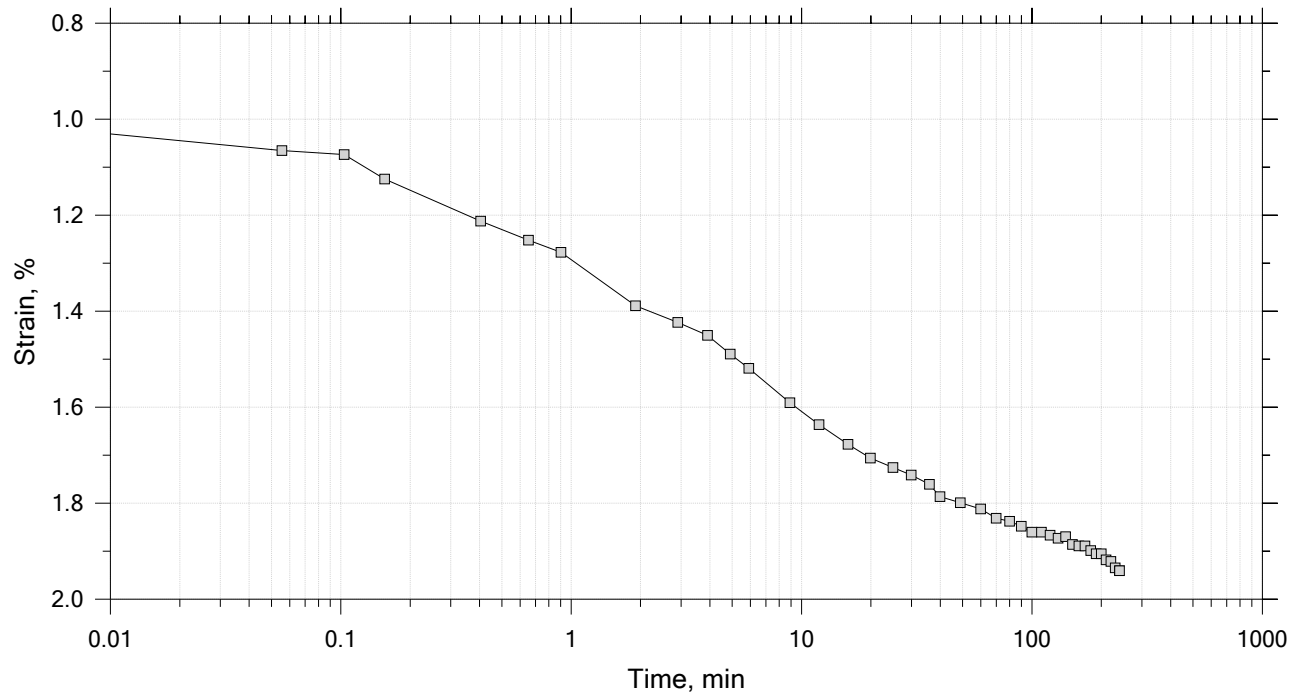
Stress: 0.25 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15
Constant Load Step
Stress: 0.5 tsf



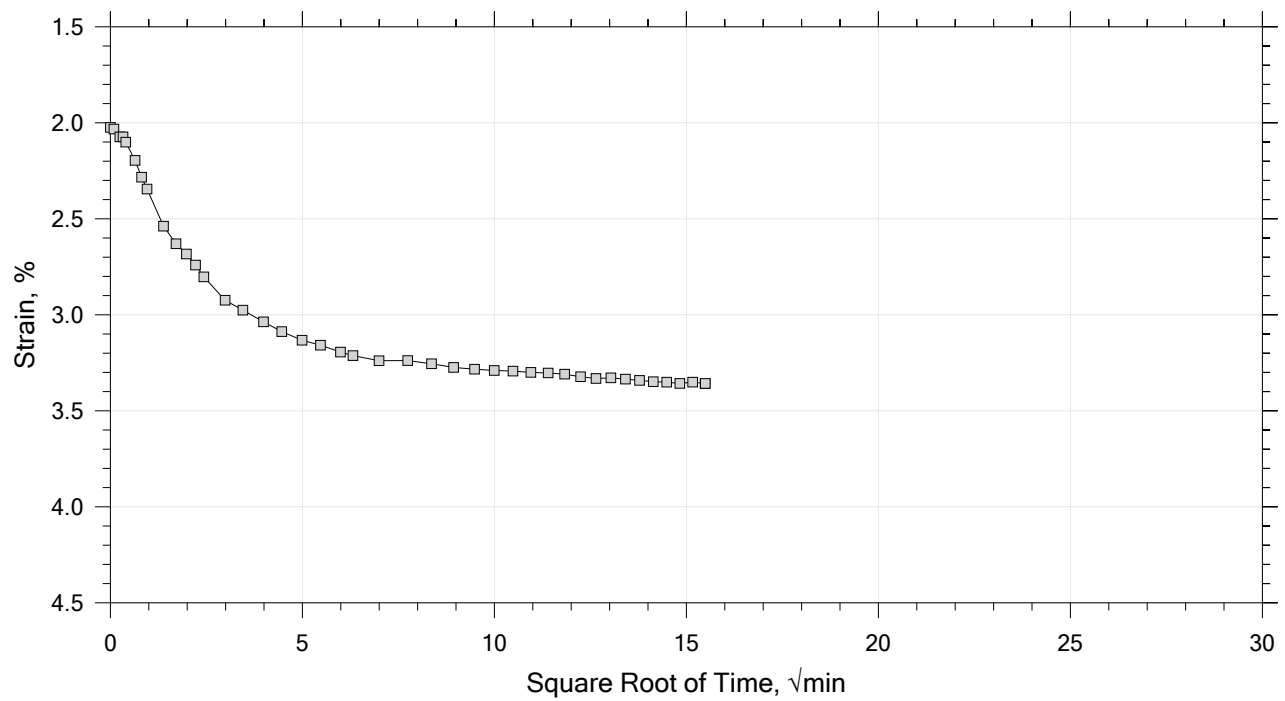
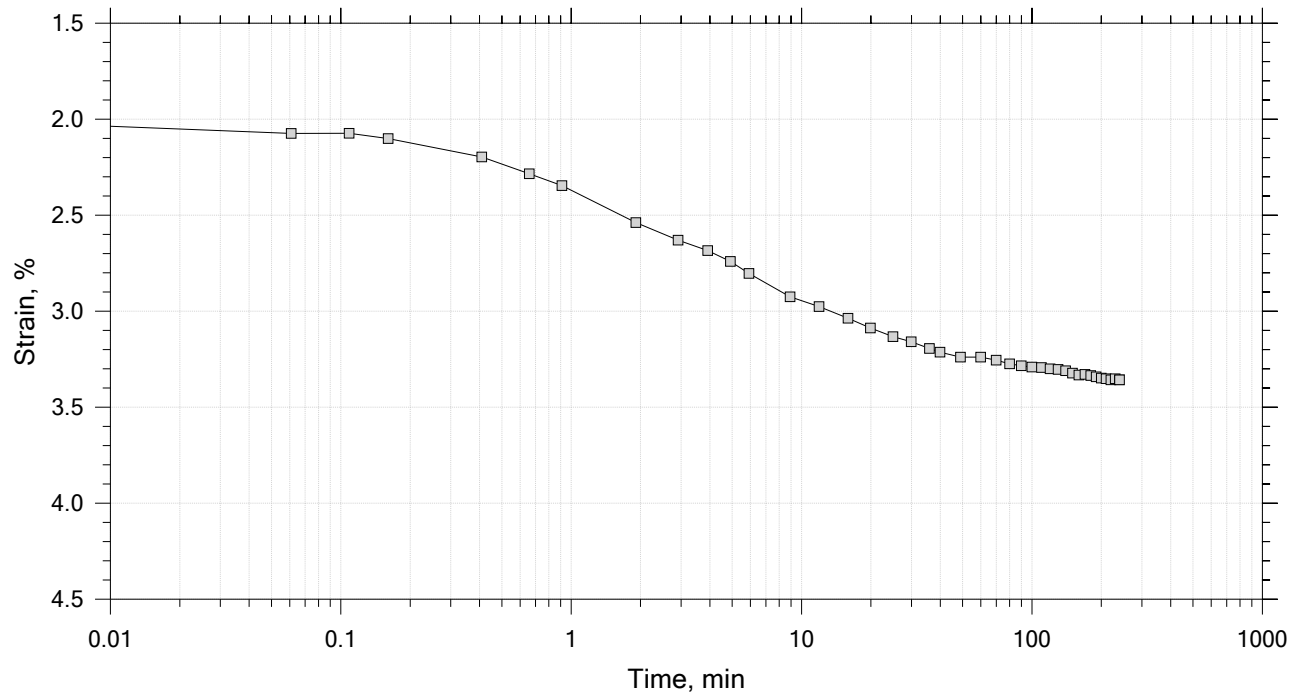
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



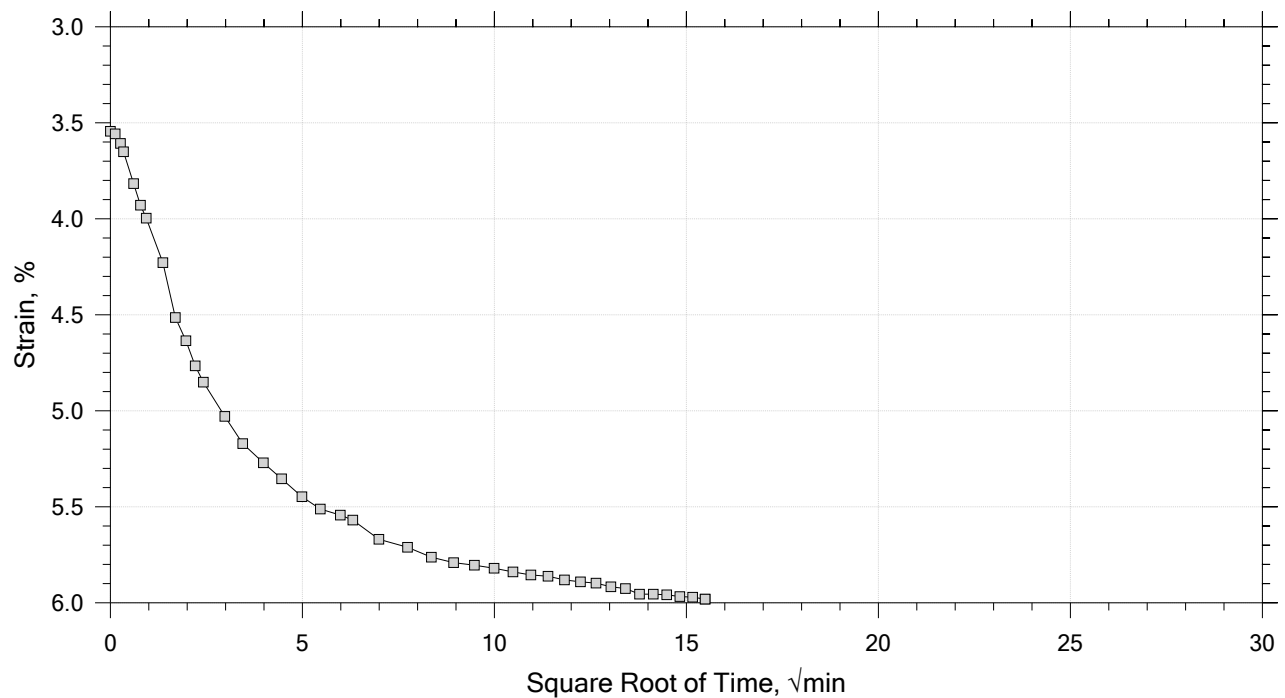
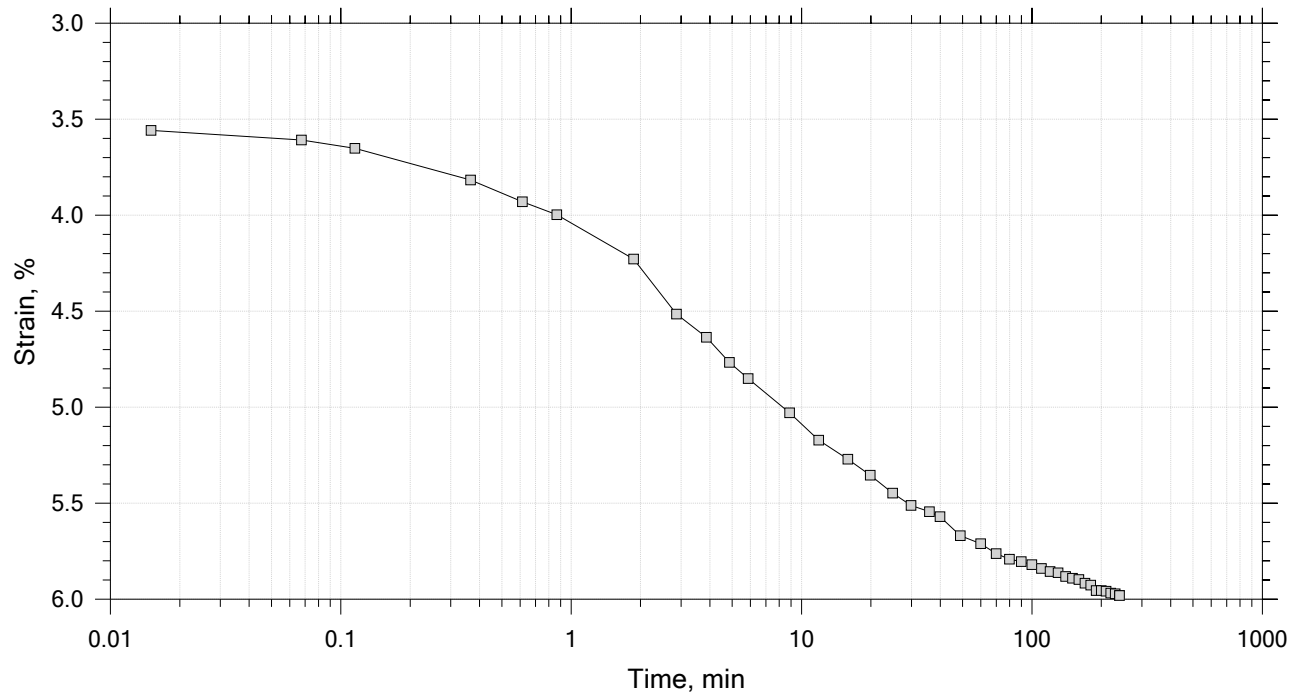
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



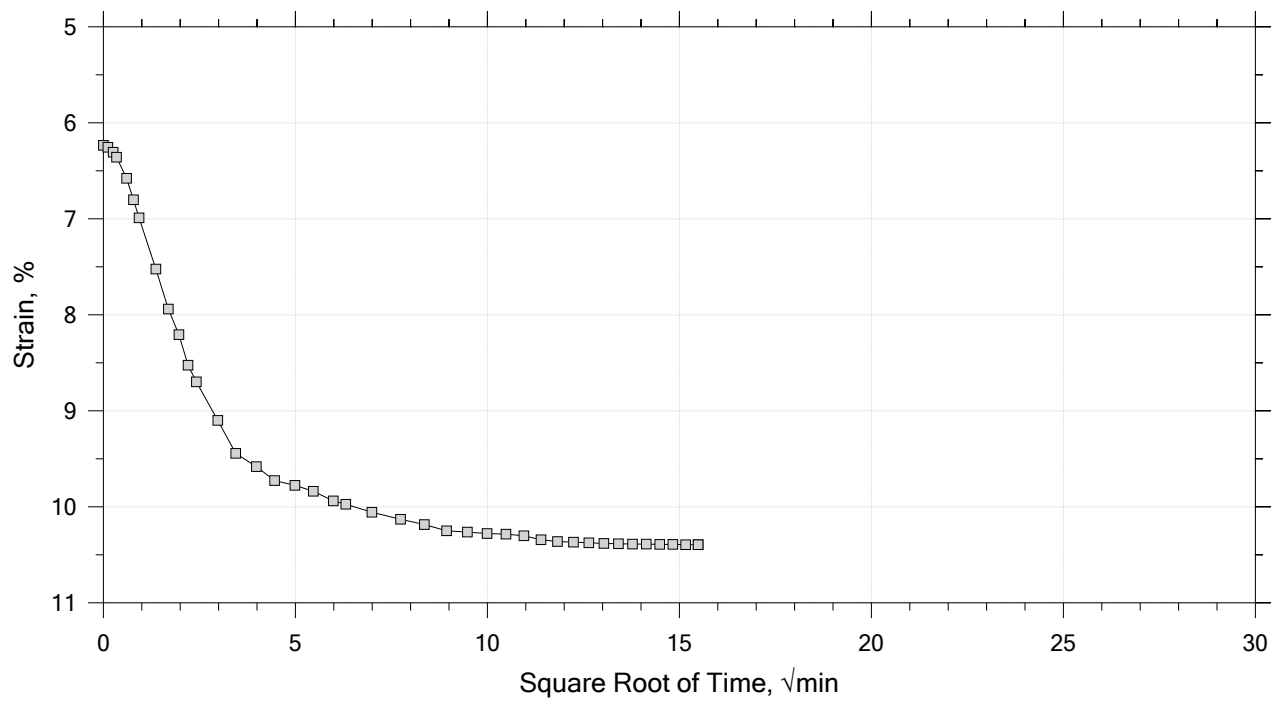
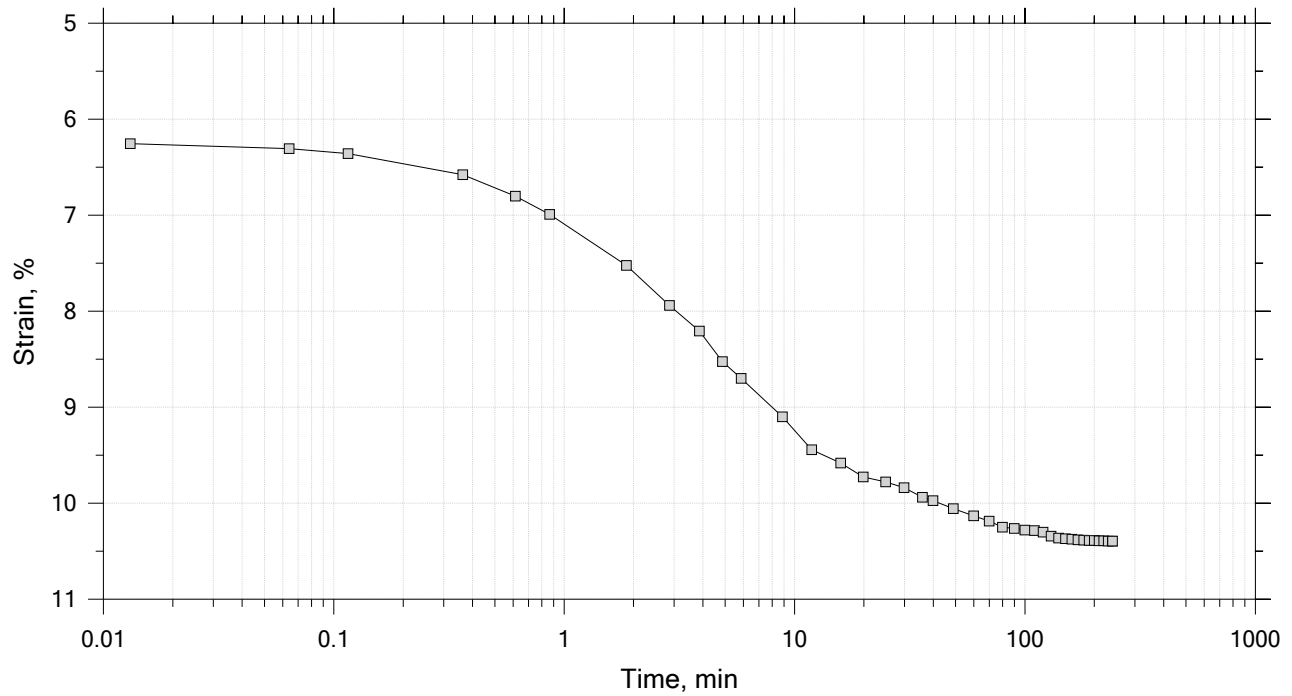
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



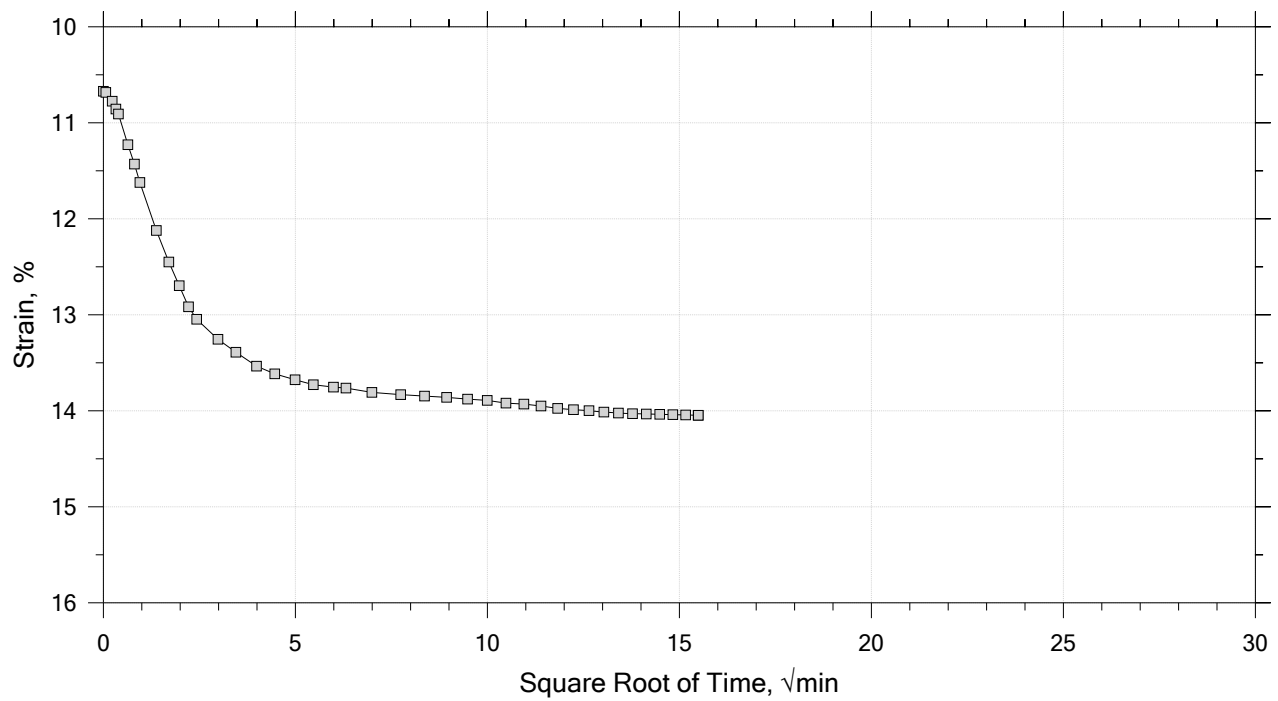
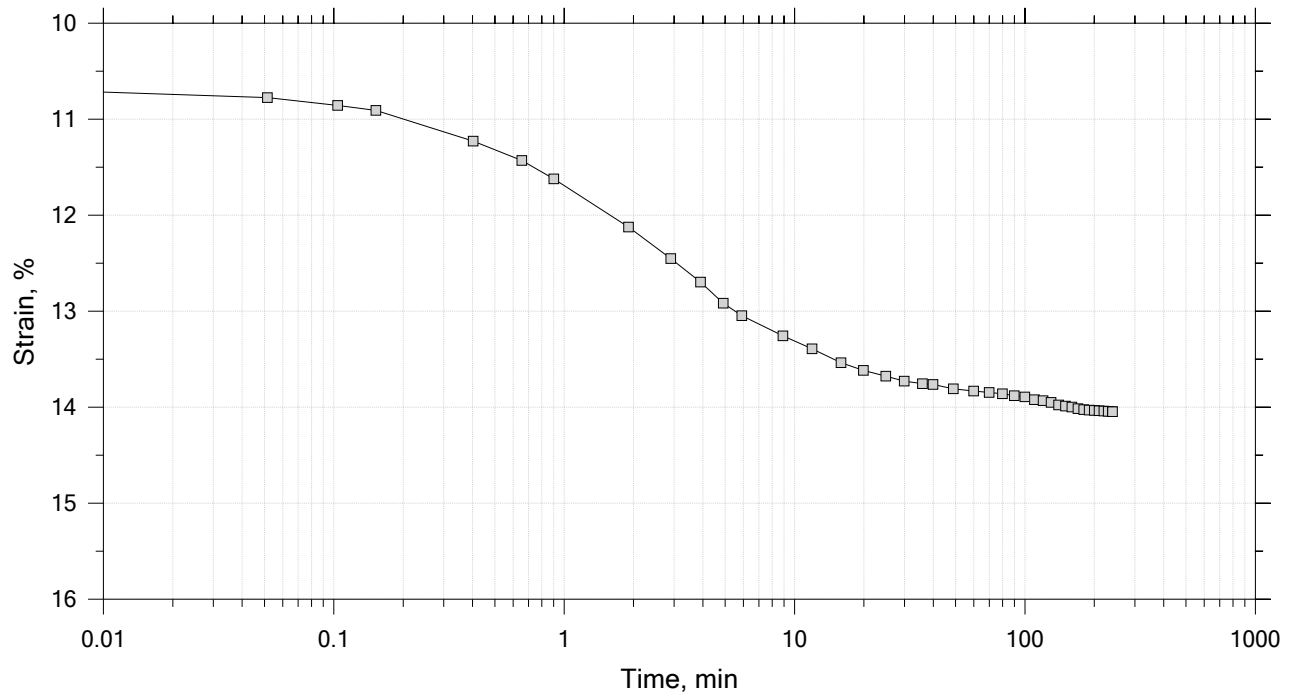
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



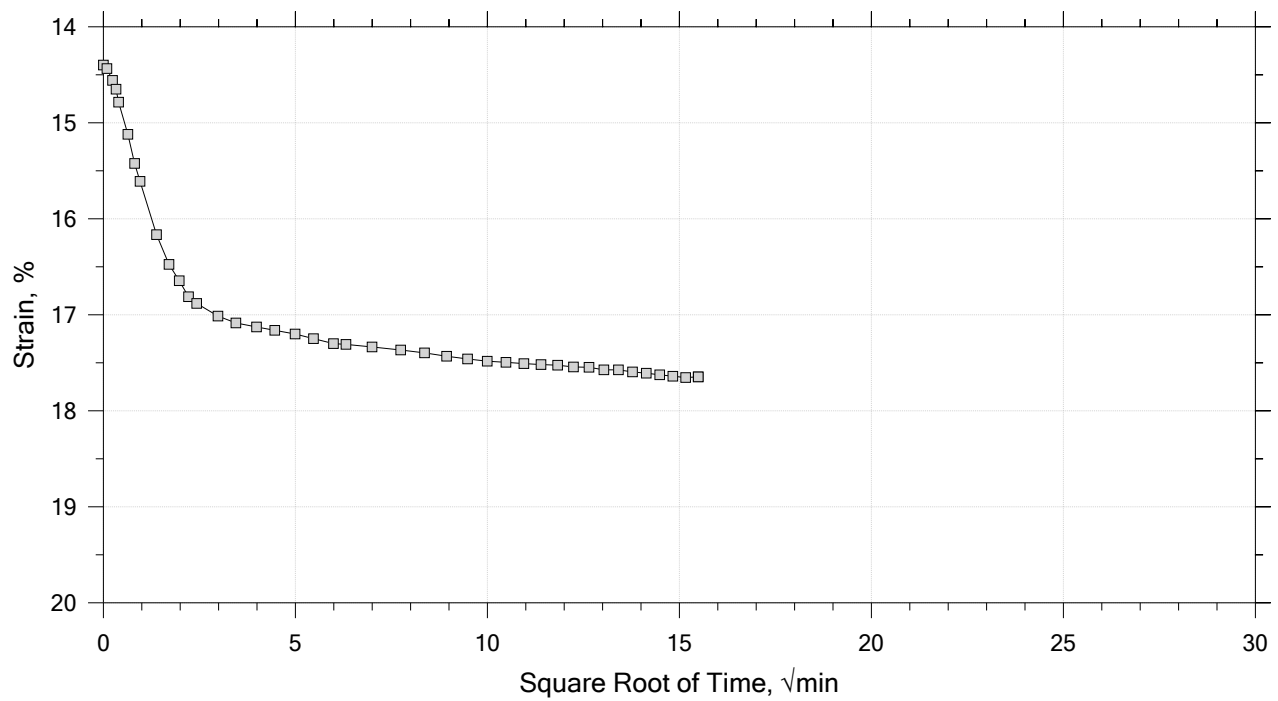
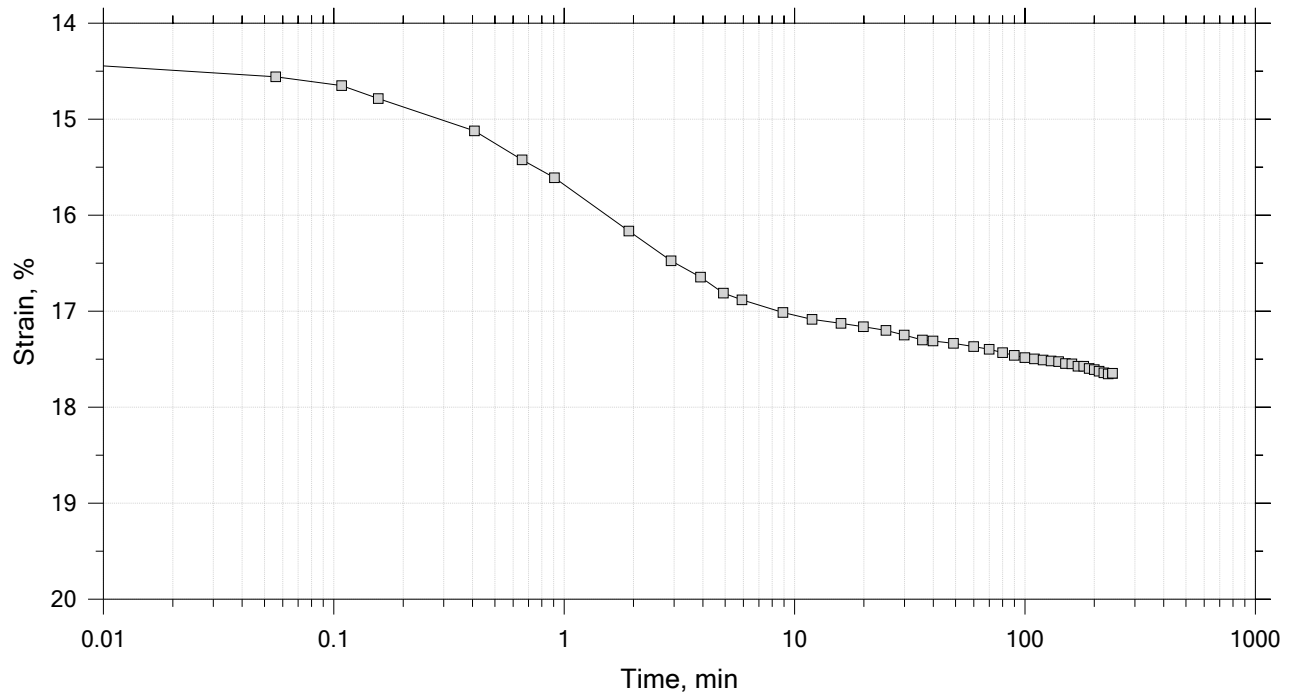
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



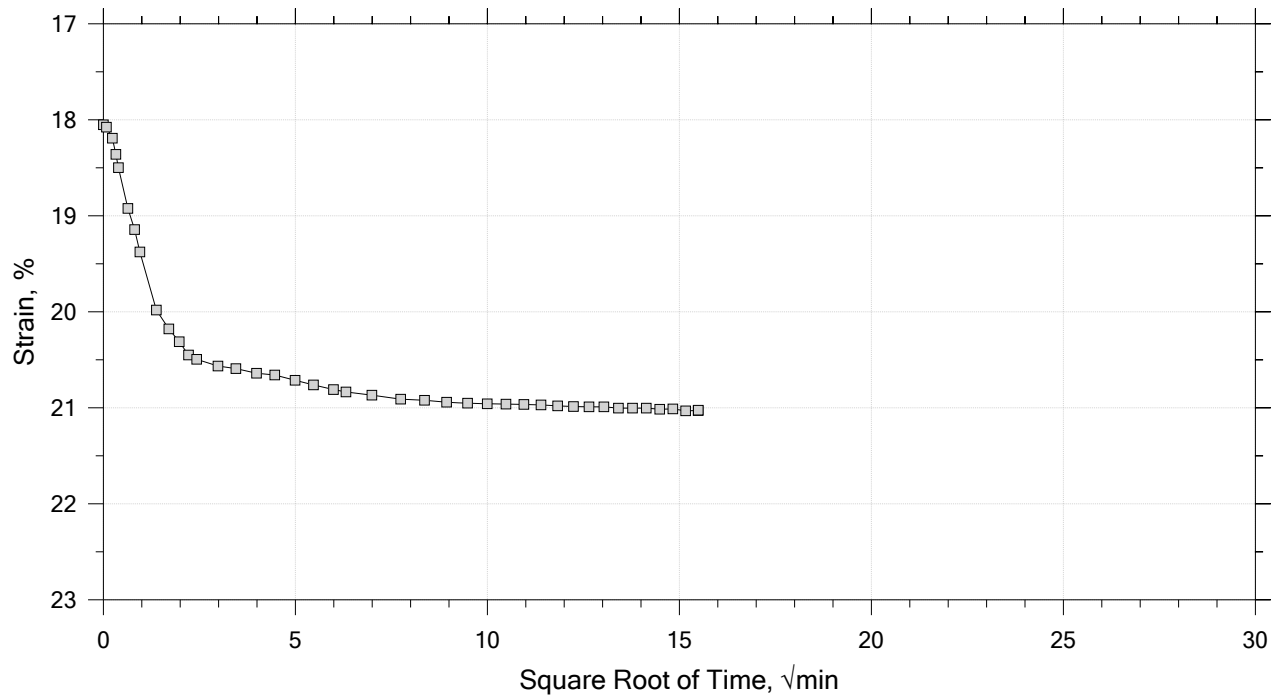
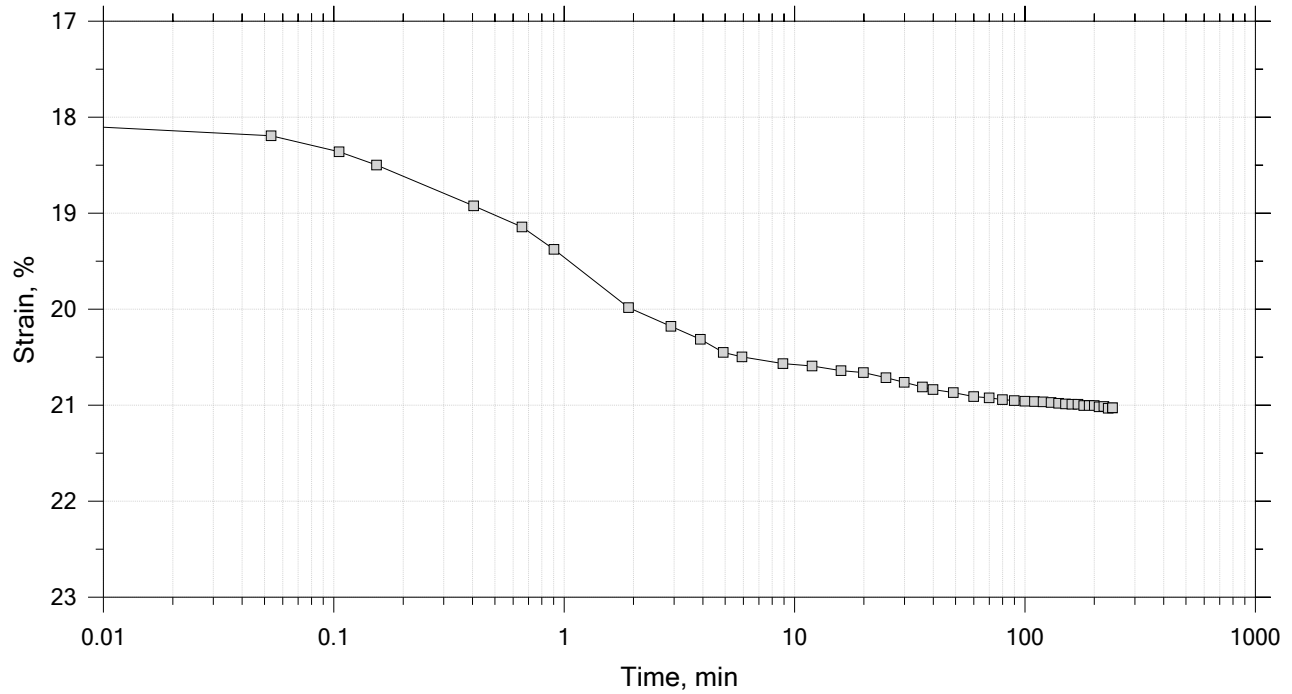
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



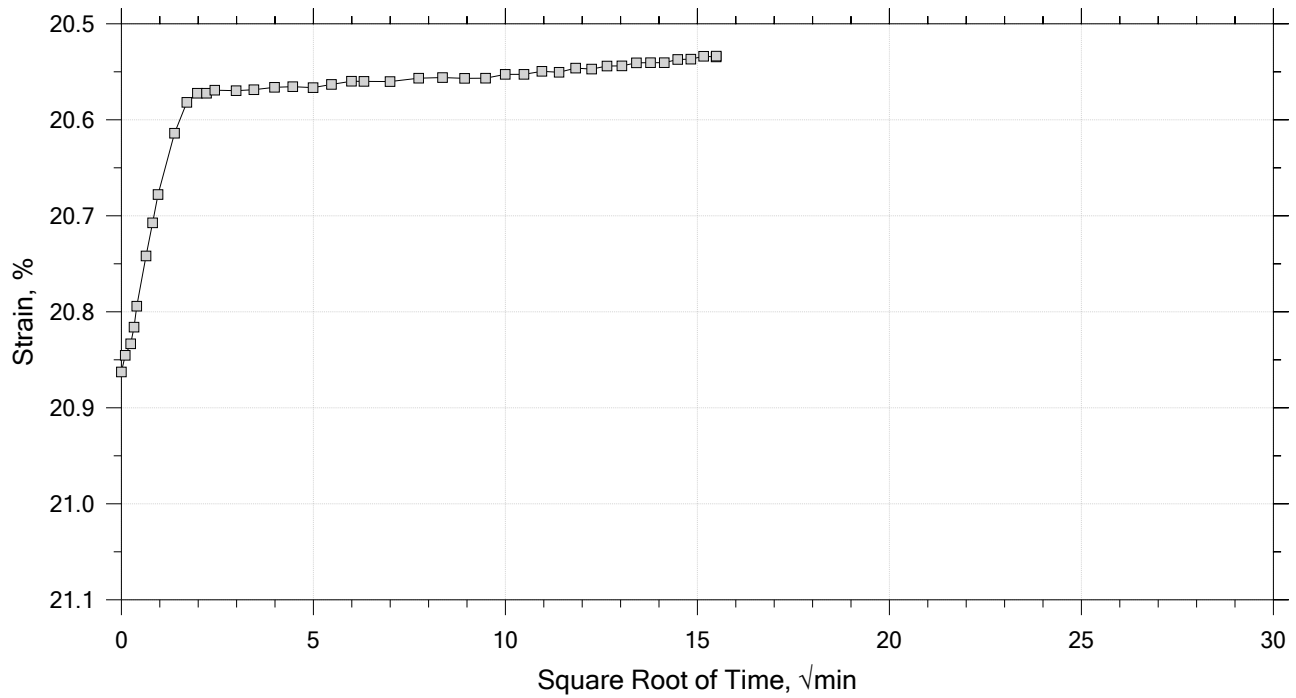
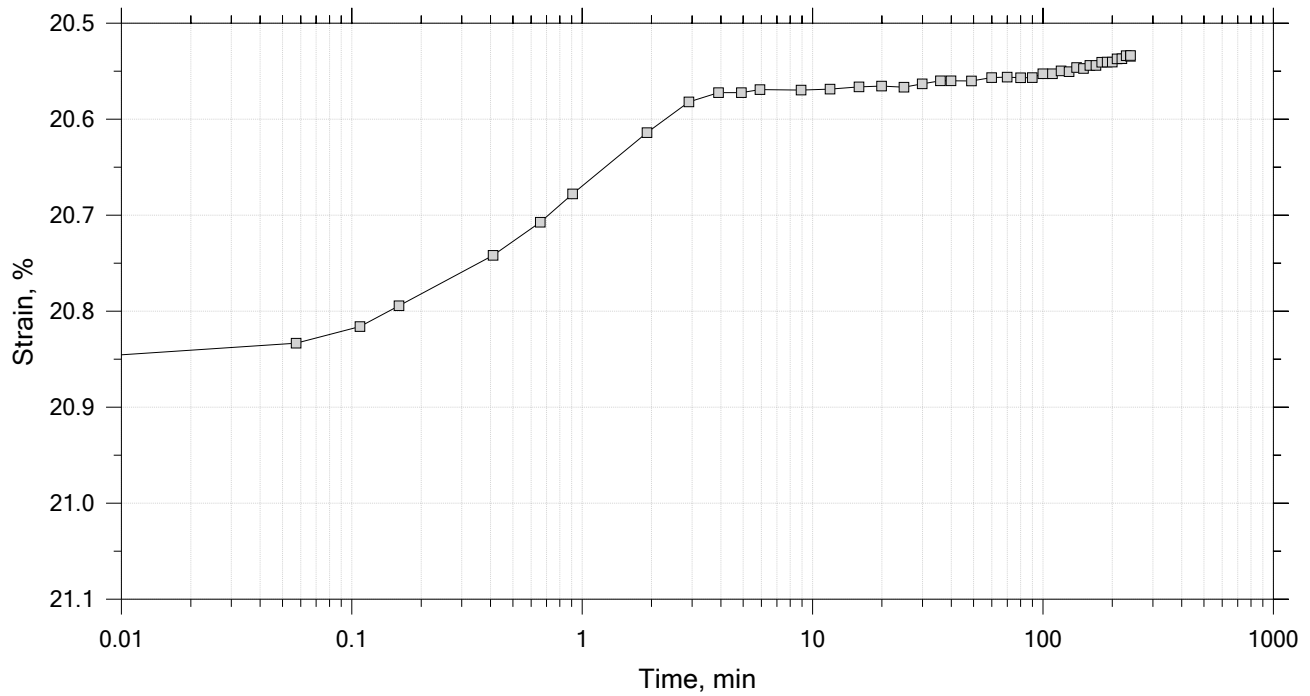
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



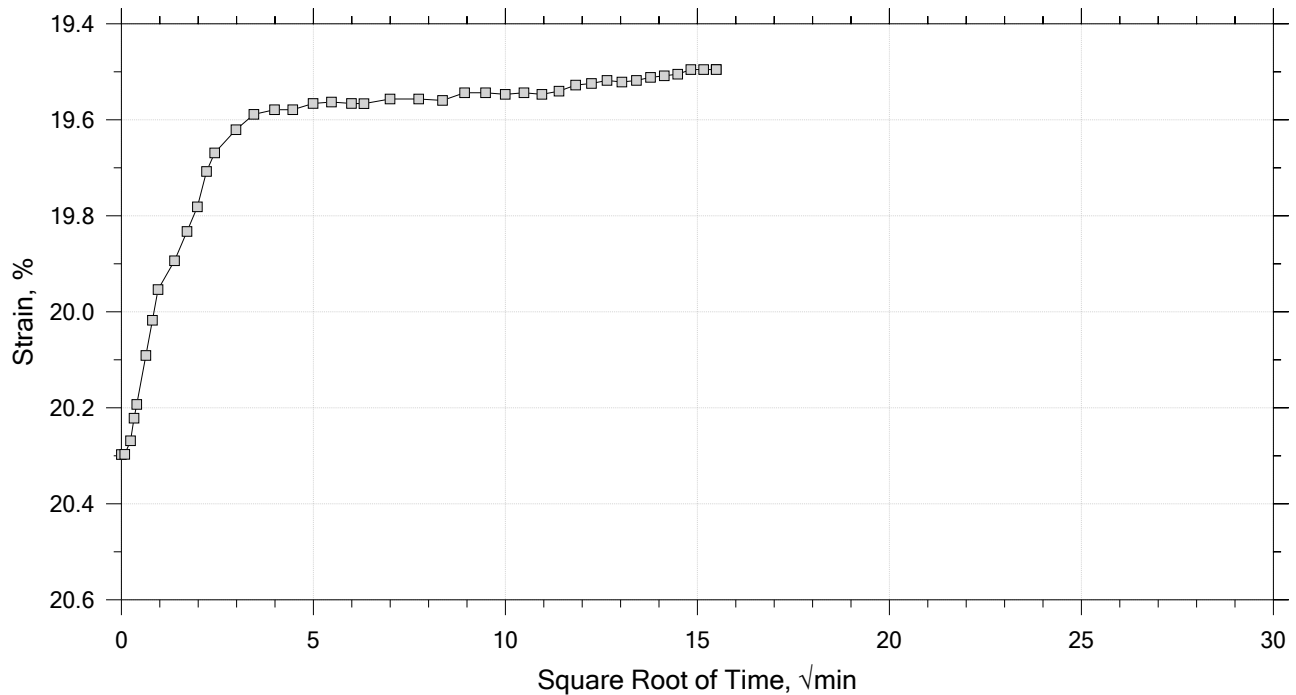
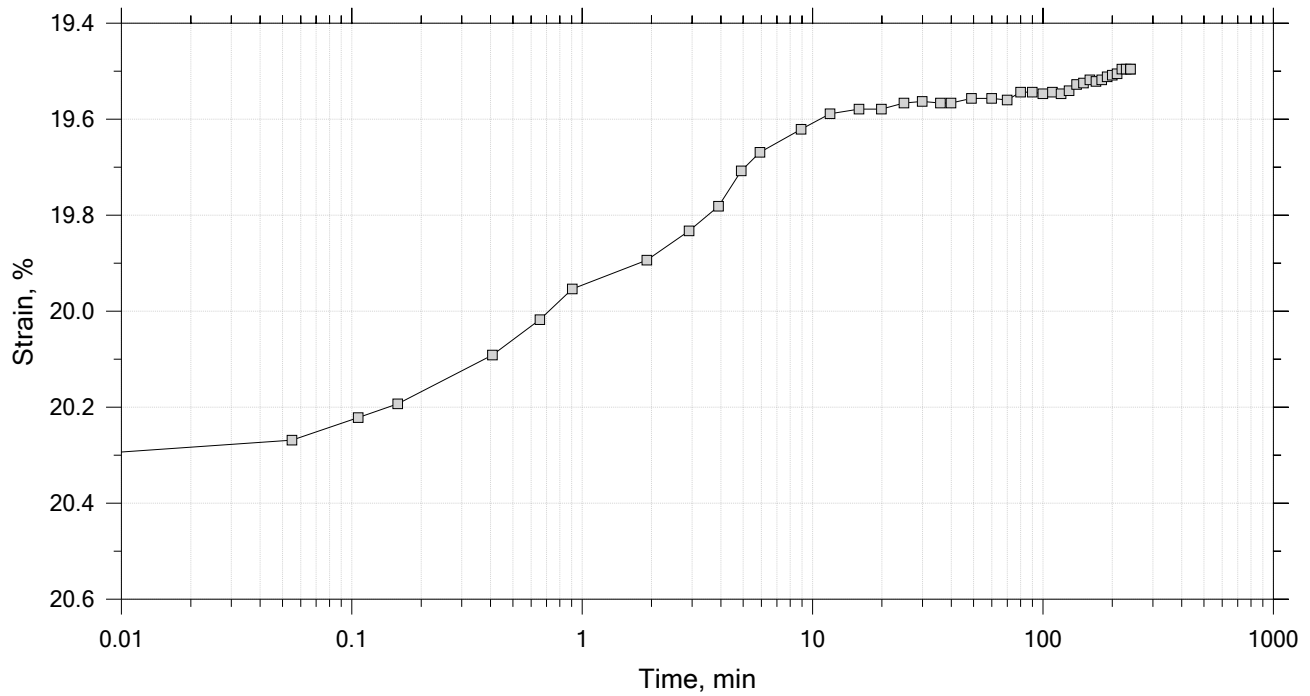
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



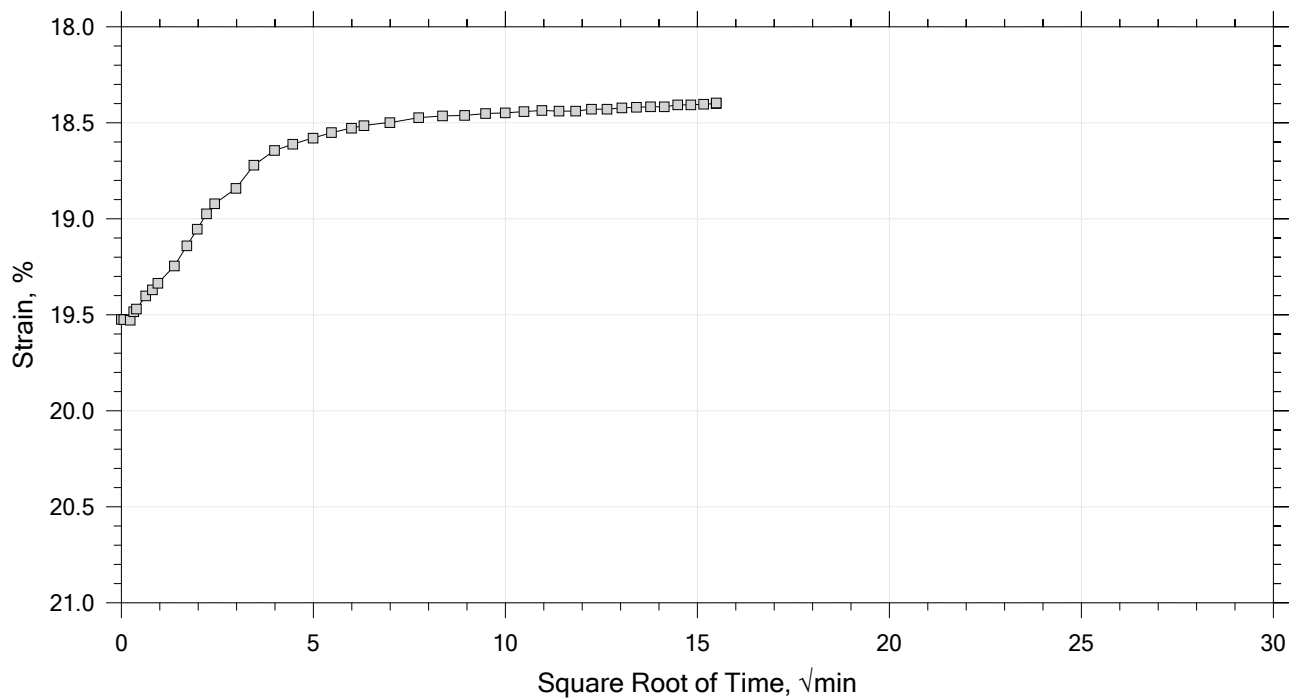
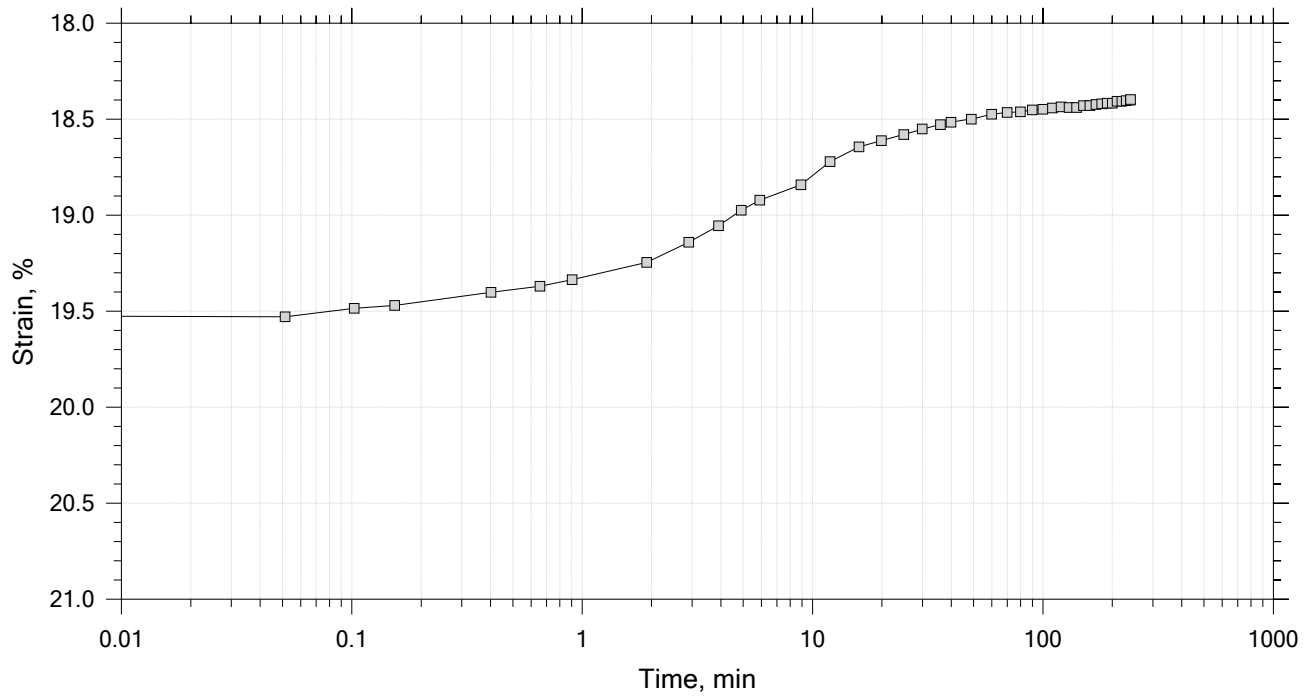
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



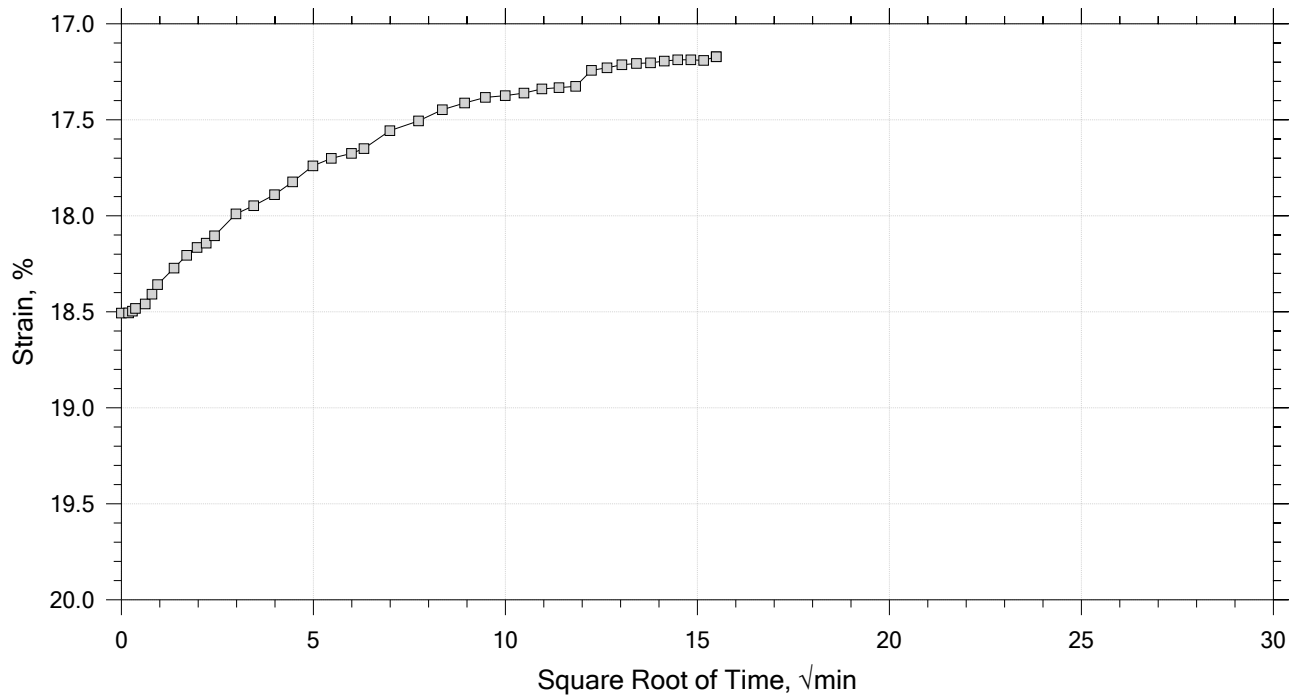
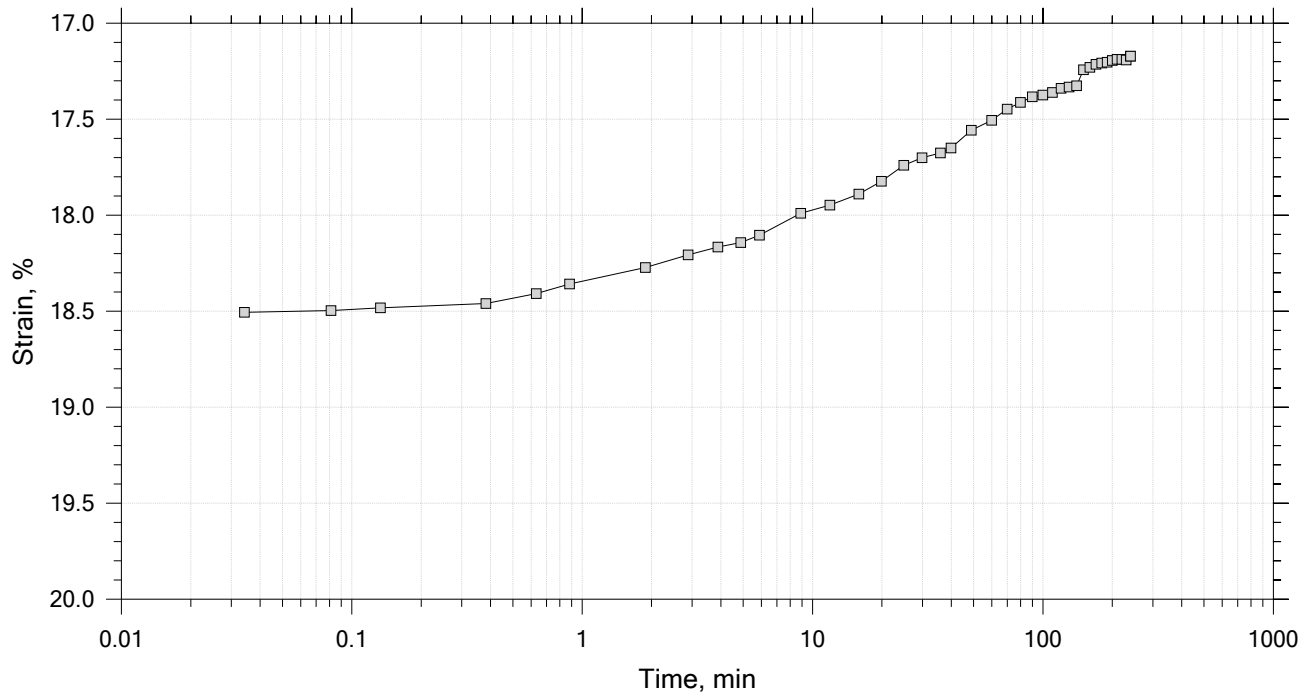
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



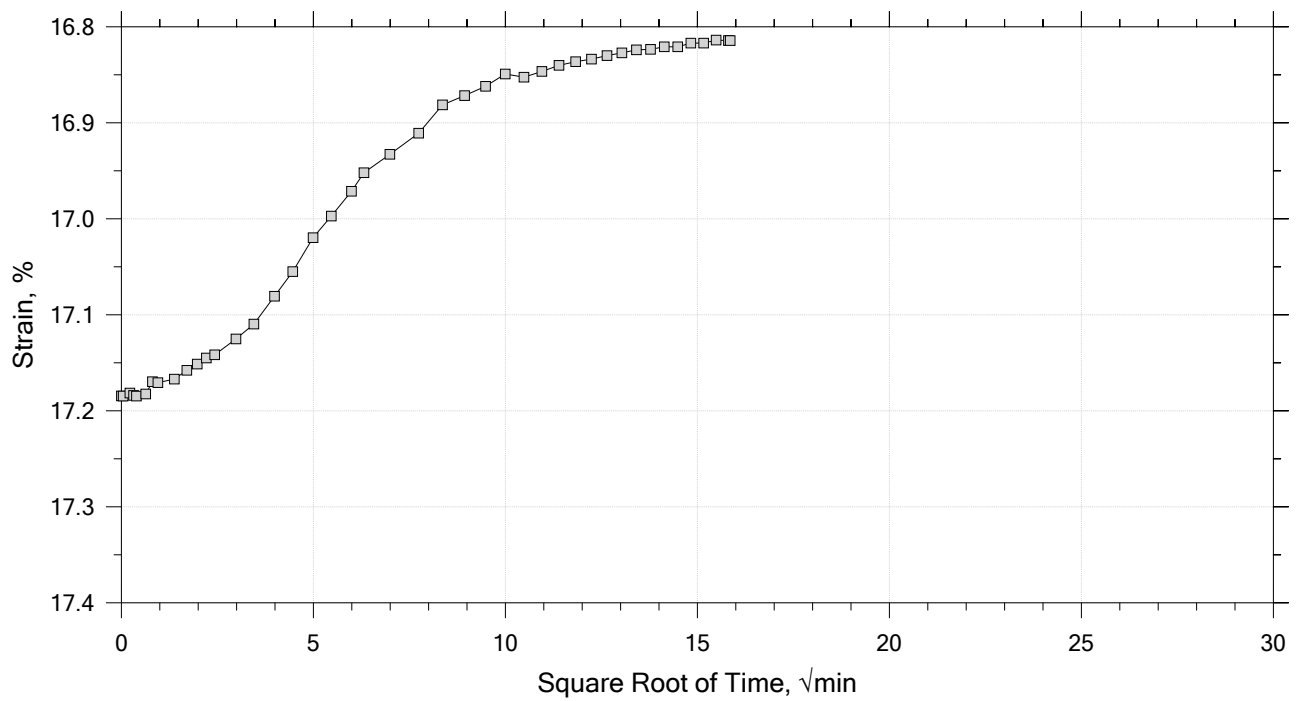
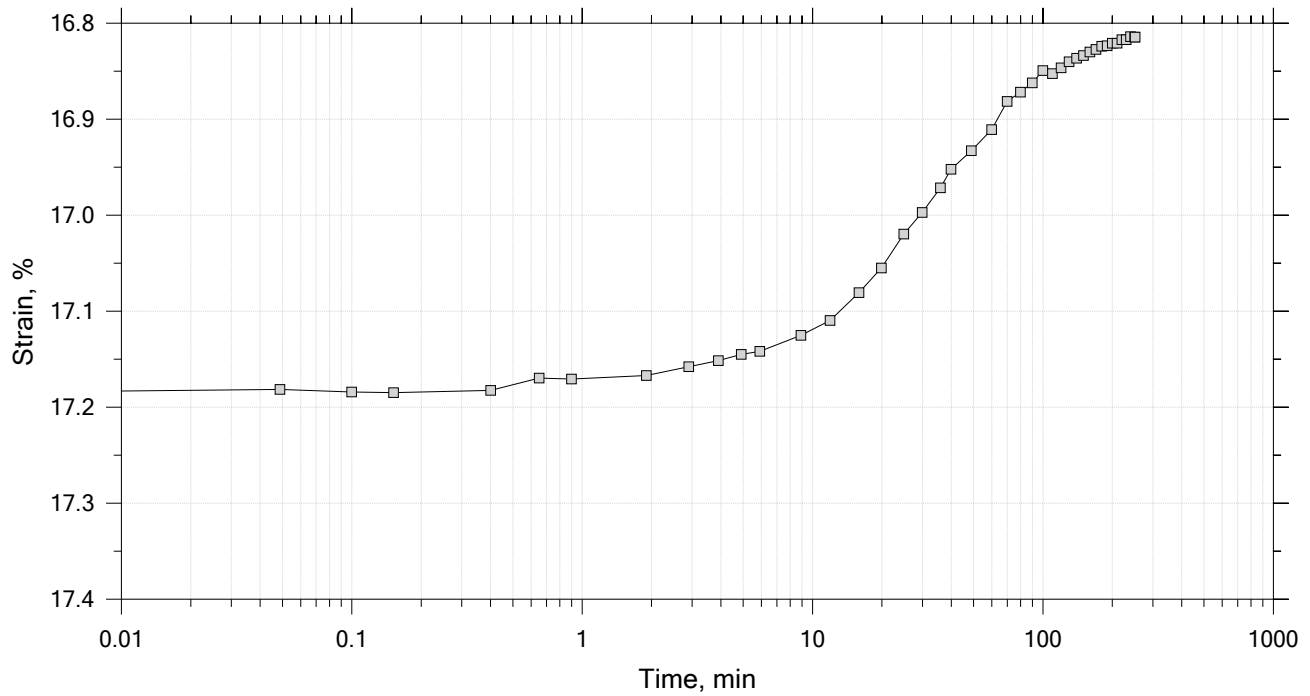
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	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.77	Liquid Limit: 34
Initial Height: 1.00 in	Initial Void Ratio: 1.02	Plastic Limit: 19
Final Height: 0.83 in	Final Void Ratio: 0.677	Plasticity Index: 15

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-2690	RING		D-665
Mass Container, gm	8.34	109.44	109.44	9.21
Mass Container + Wet Soil, gm	143.09	260.5	246.98	145.82
Mass Container + Dry Soil, gm	110.09	219.98	219.98	119
Mass Dry Soil, gm	101.75	110.54	110.54	109.79
Water Content, %	32.43	36.66	24.43	24.43
Void Ratio	---	1.02	0.68	---
Degree of Saturation, %	---	99.99	100.00	---
Dry Unit Weight, pcf	---	85.786	103.13	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIII-B, Swell Pressure = 0.104 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIIB-B, Swell Pressure = 0.104 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

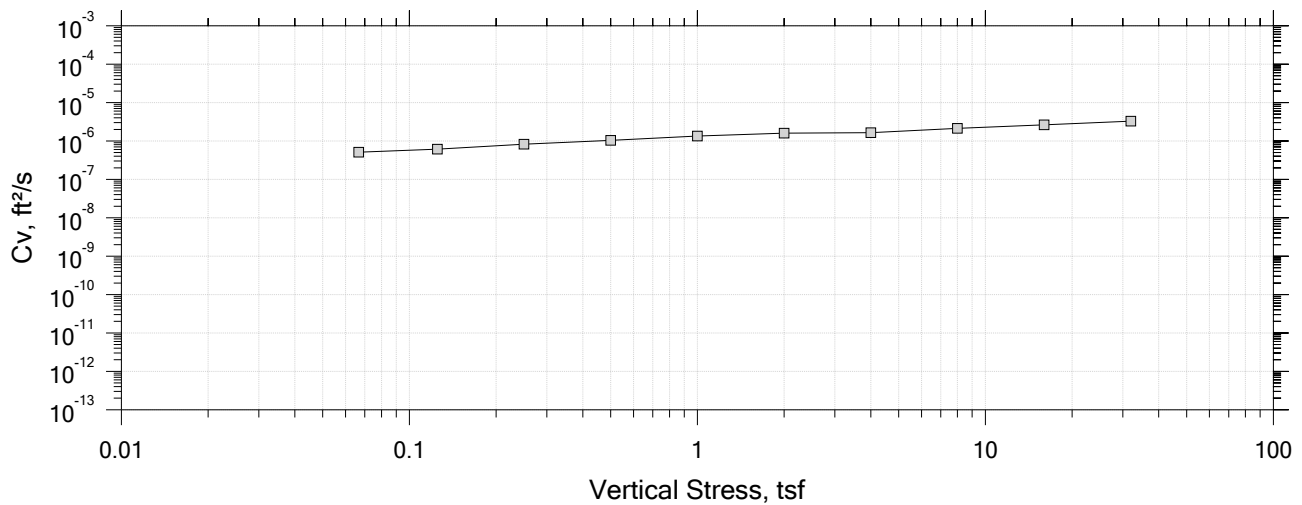
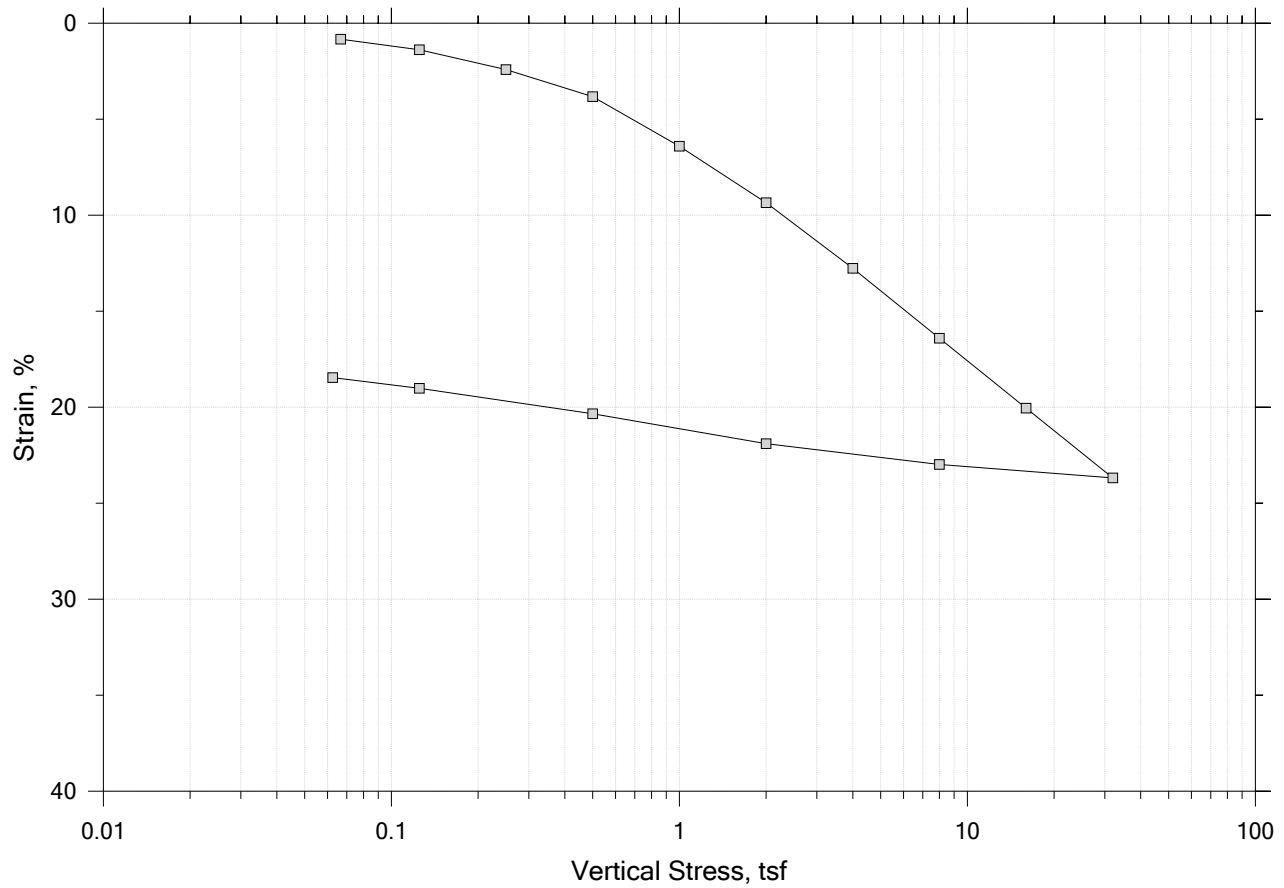
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-111	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/15/19	Depth: 10-12 ft
	Test No.: IP-12	Sample Type: Tube	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System LTIIB-B, Swell Pressure = 0.104 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

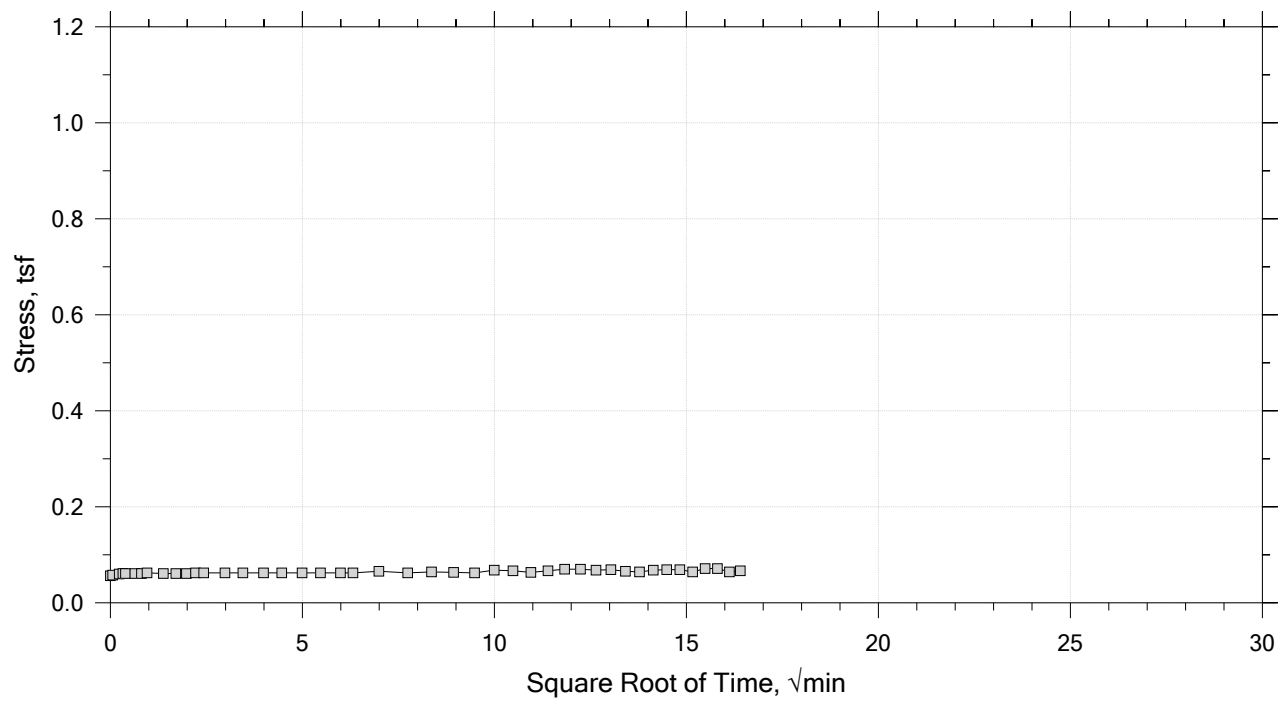
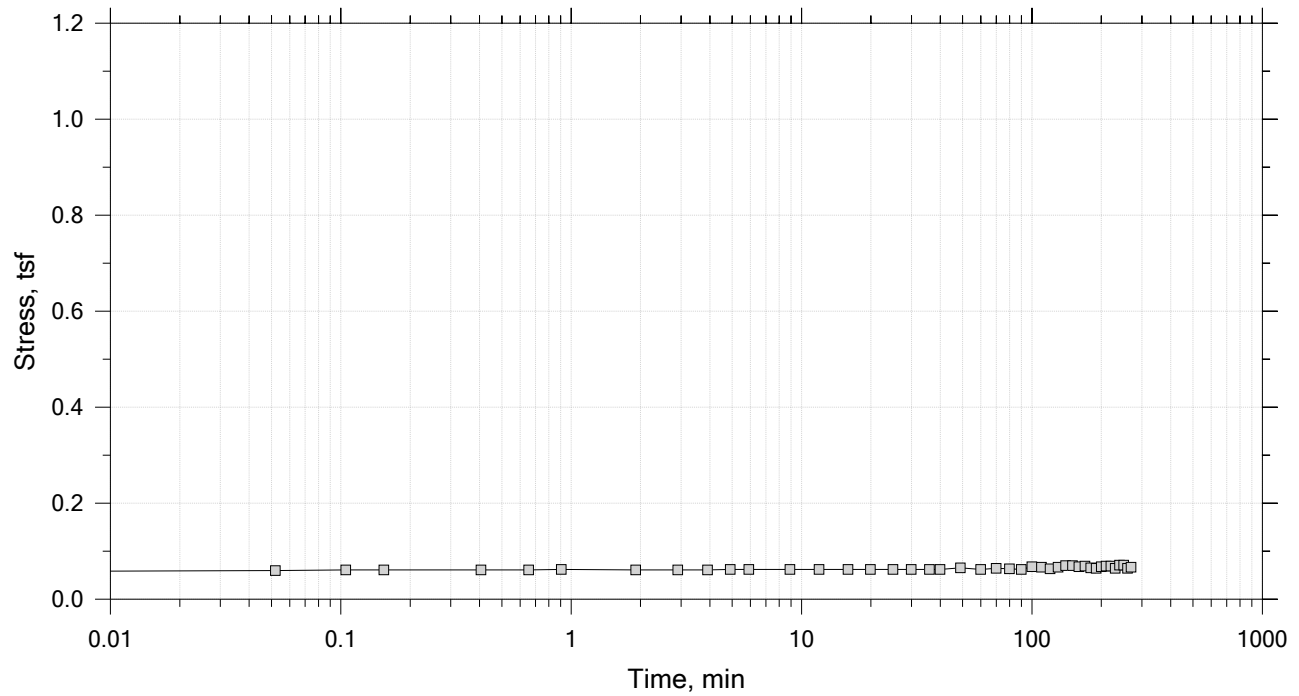
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

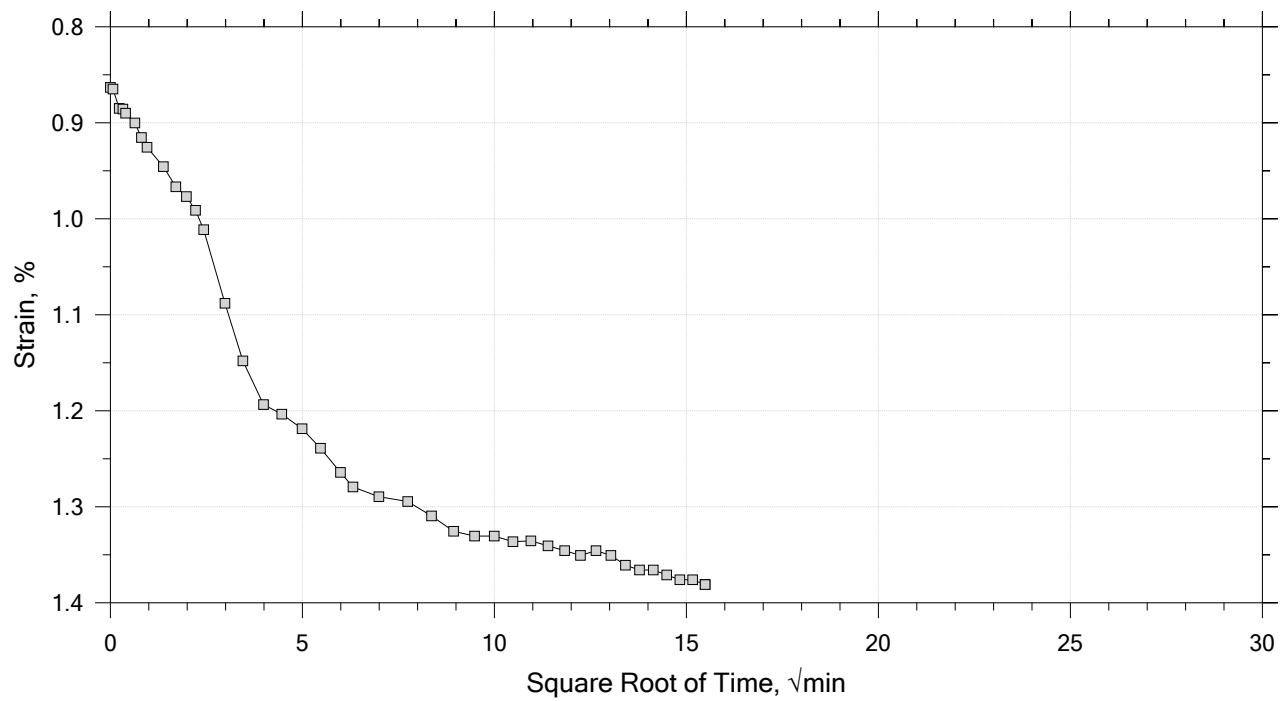
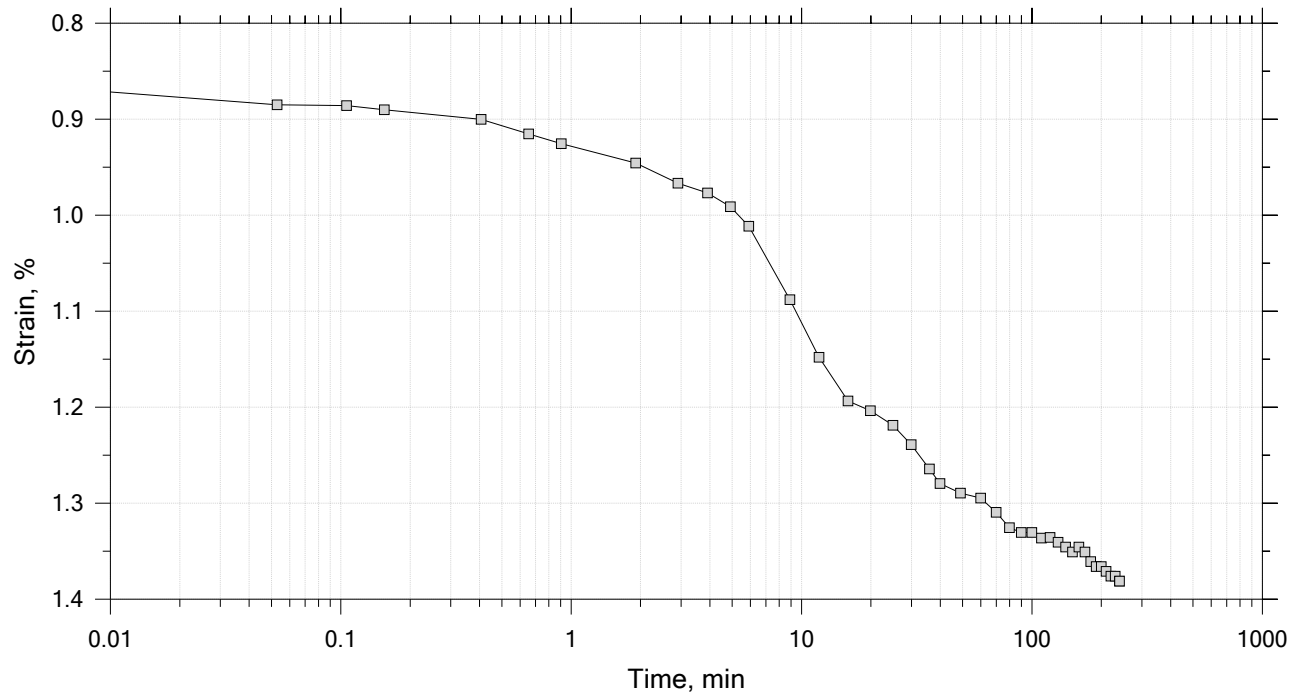
Time Curve 1 of 15
Constant Volume Step
Stress: 0.0666 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15
Constant Load Step
Stress: 0.125 tsf



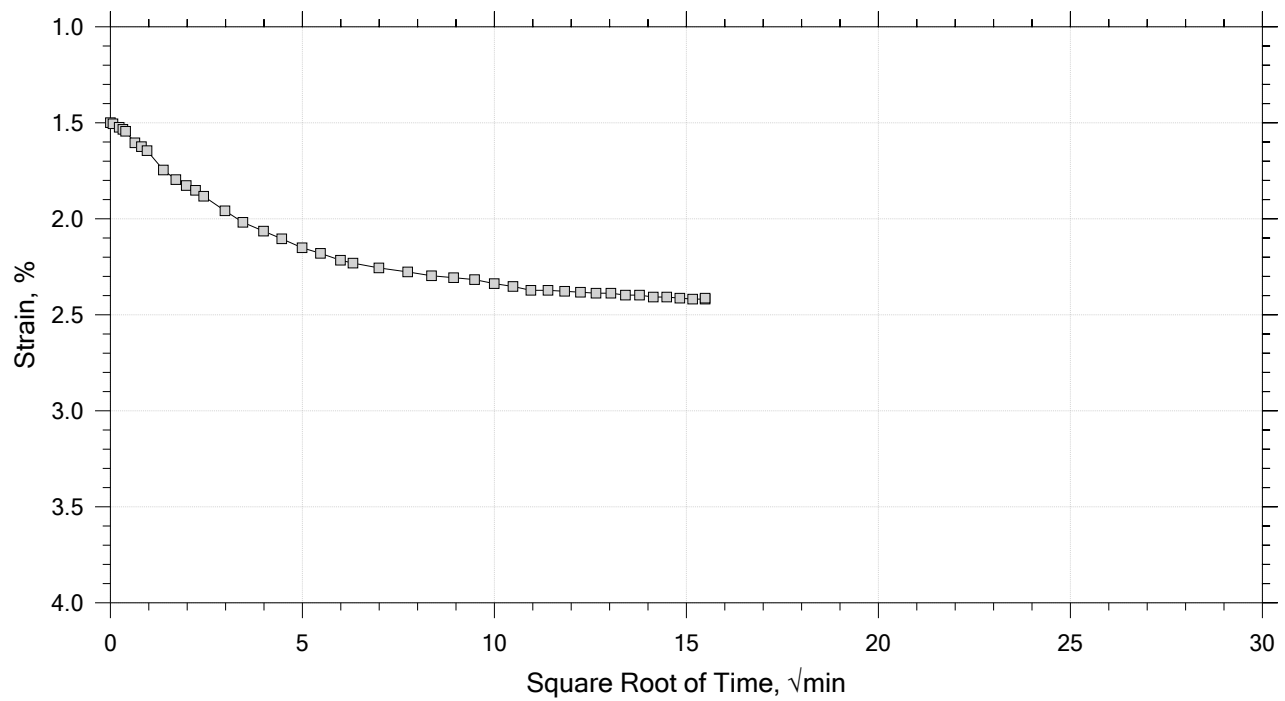
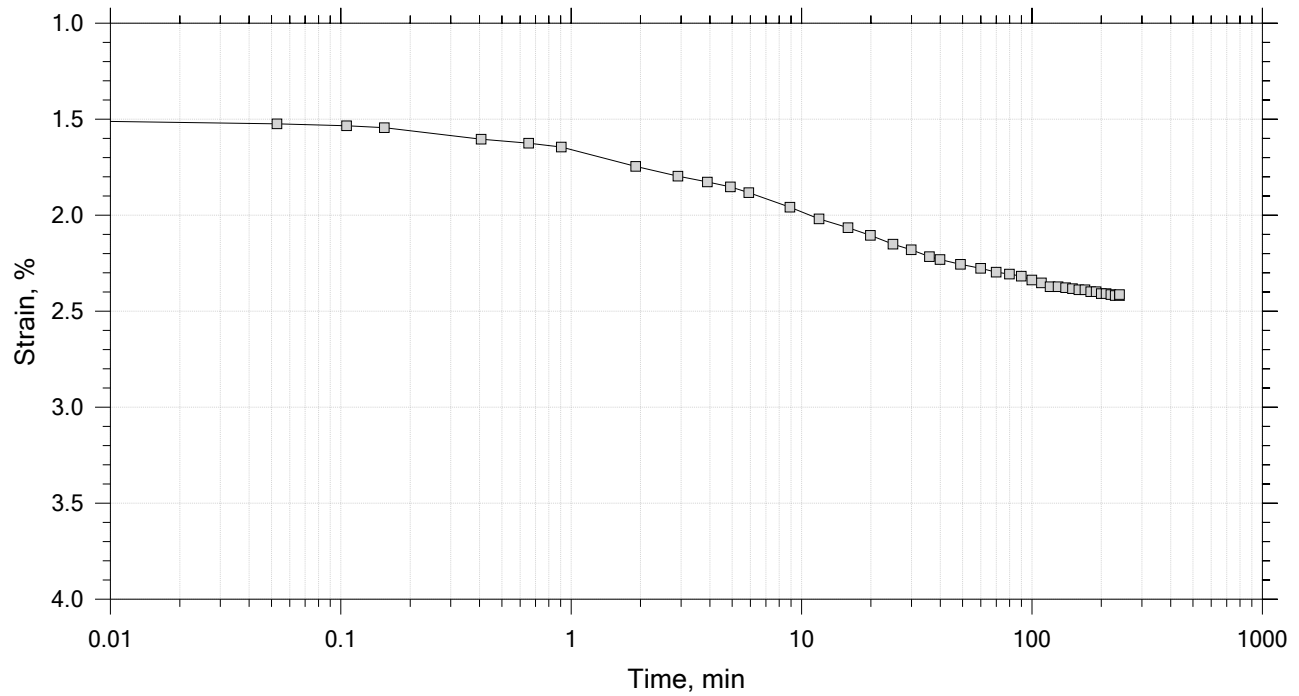
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



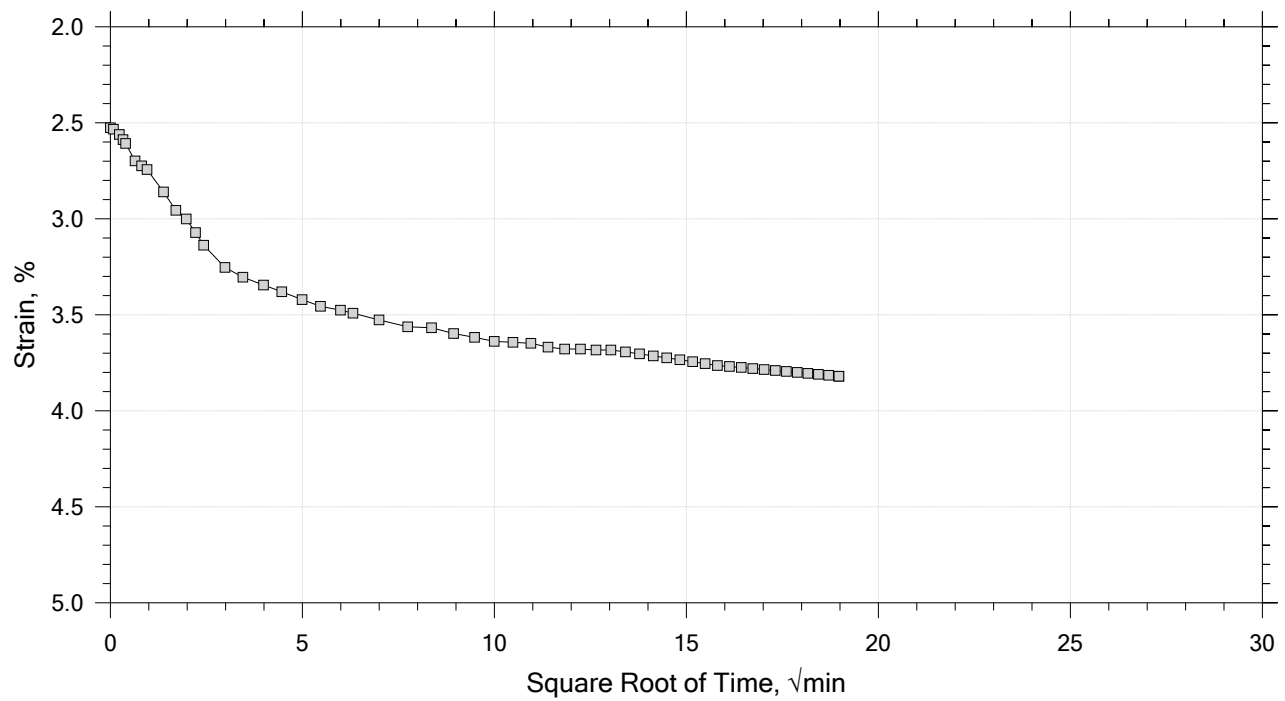
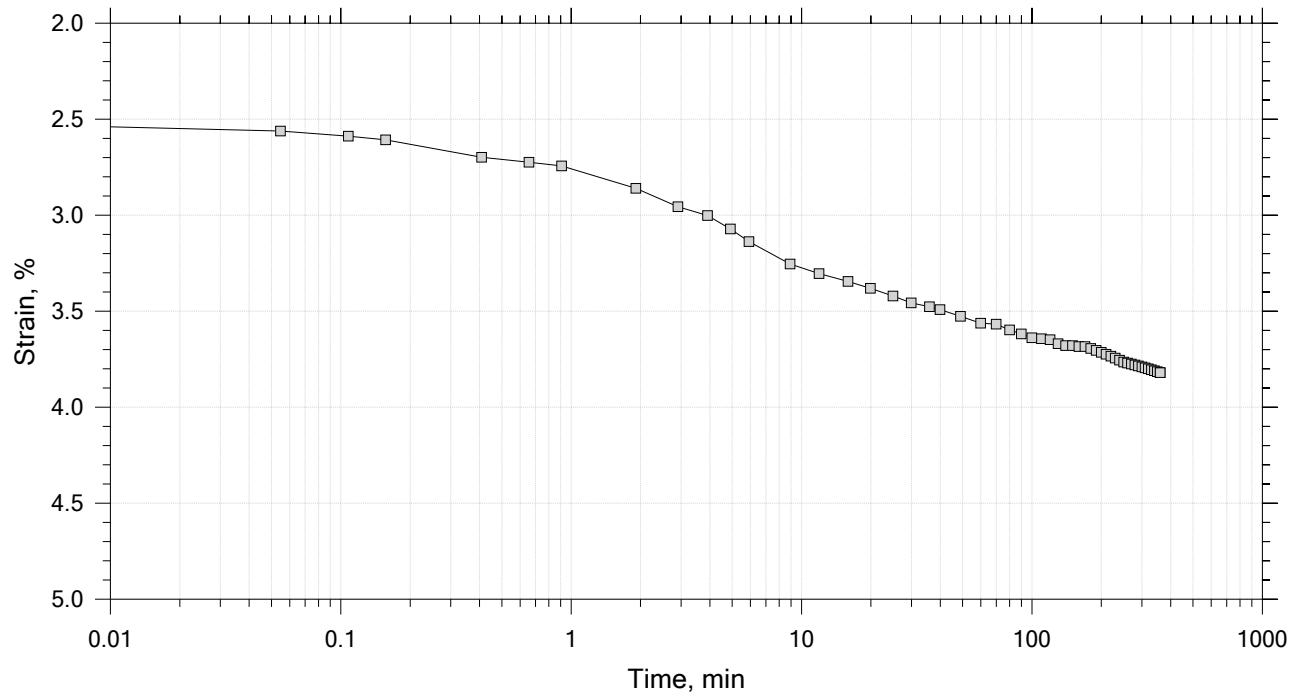
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



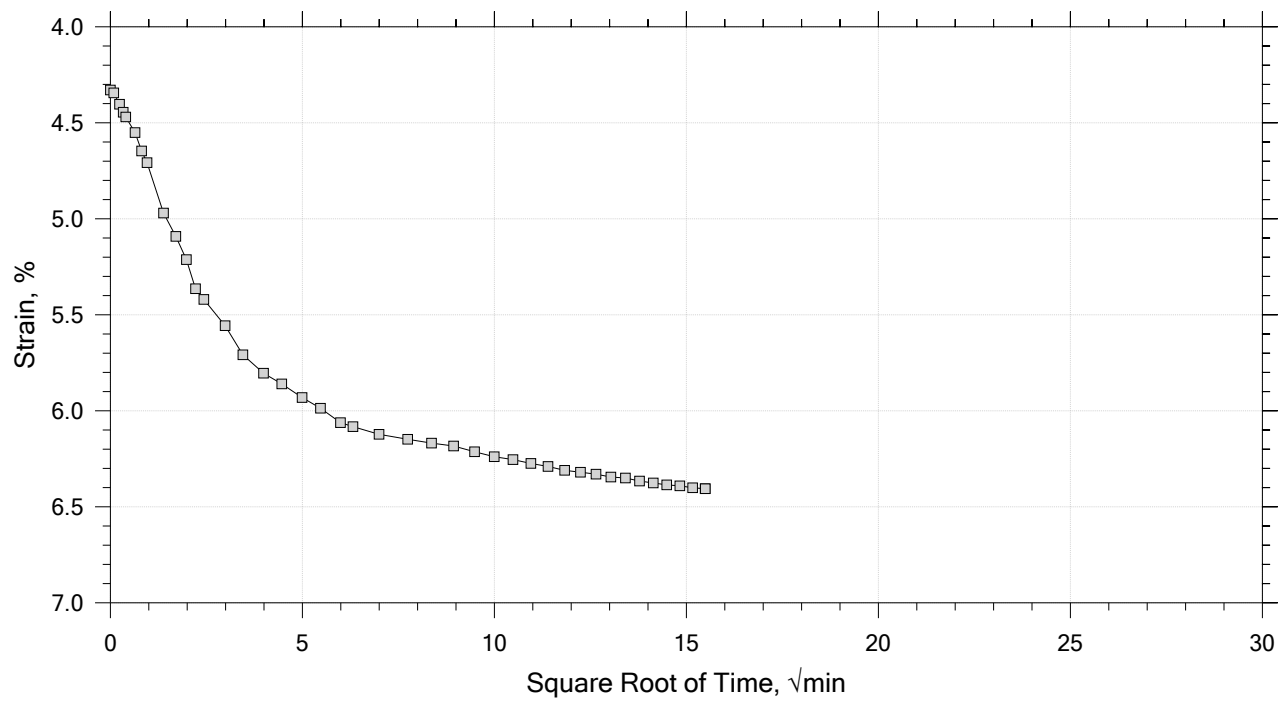
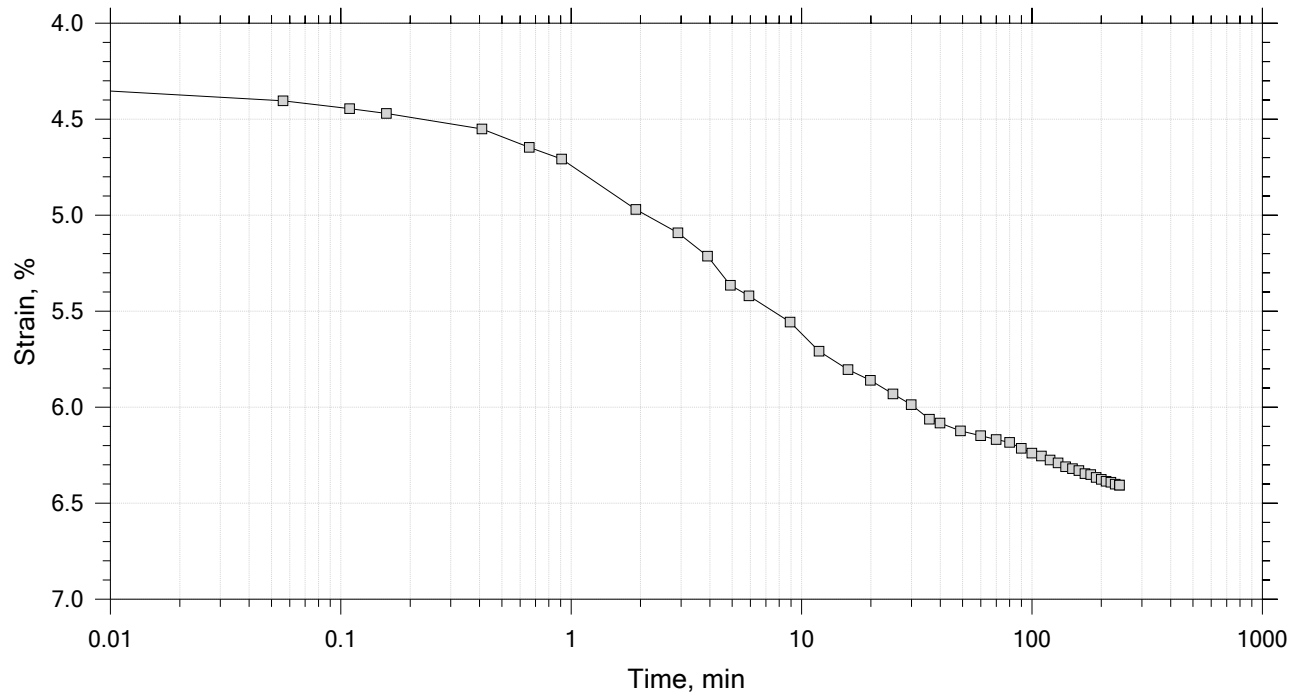
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



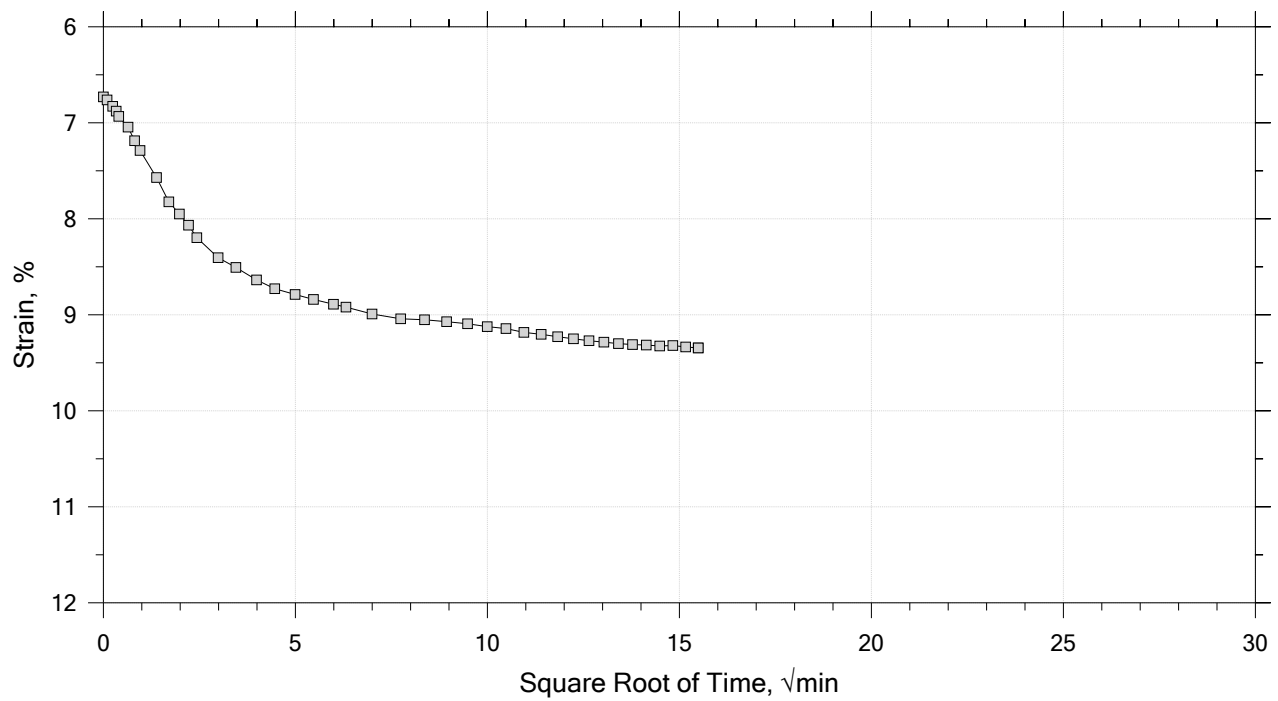
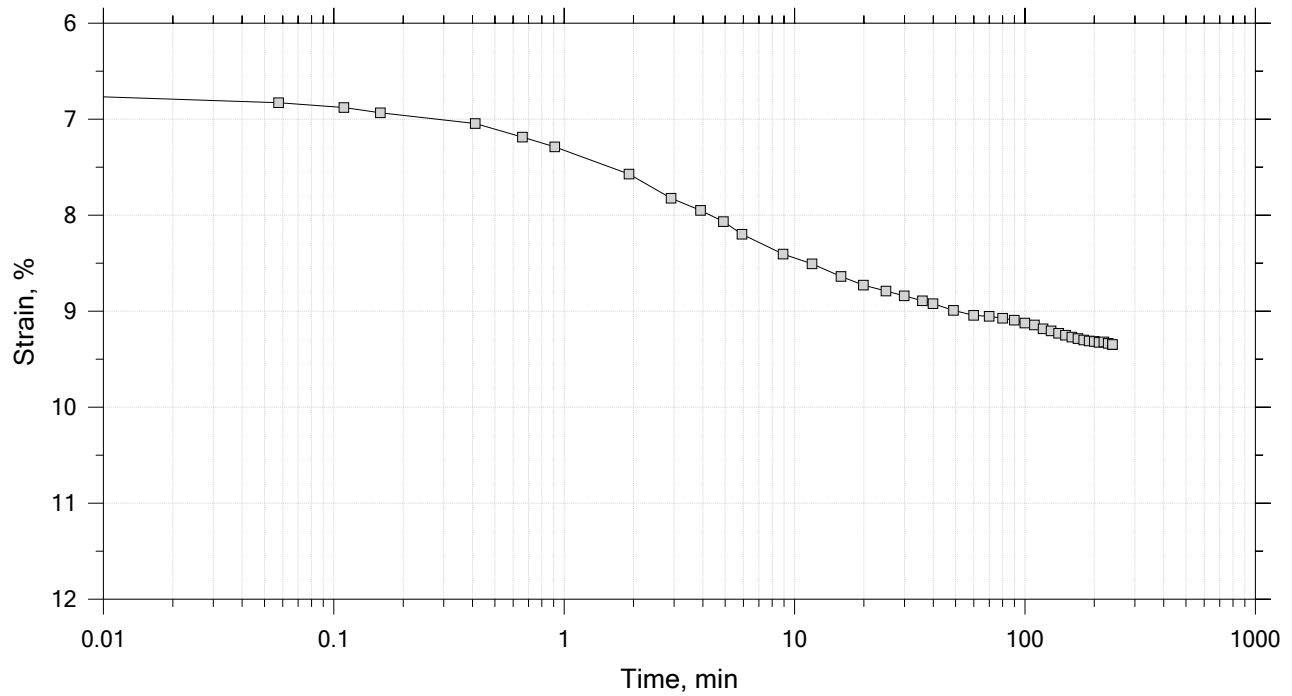
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



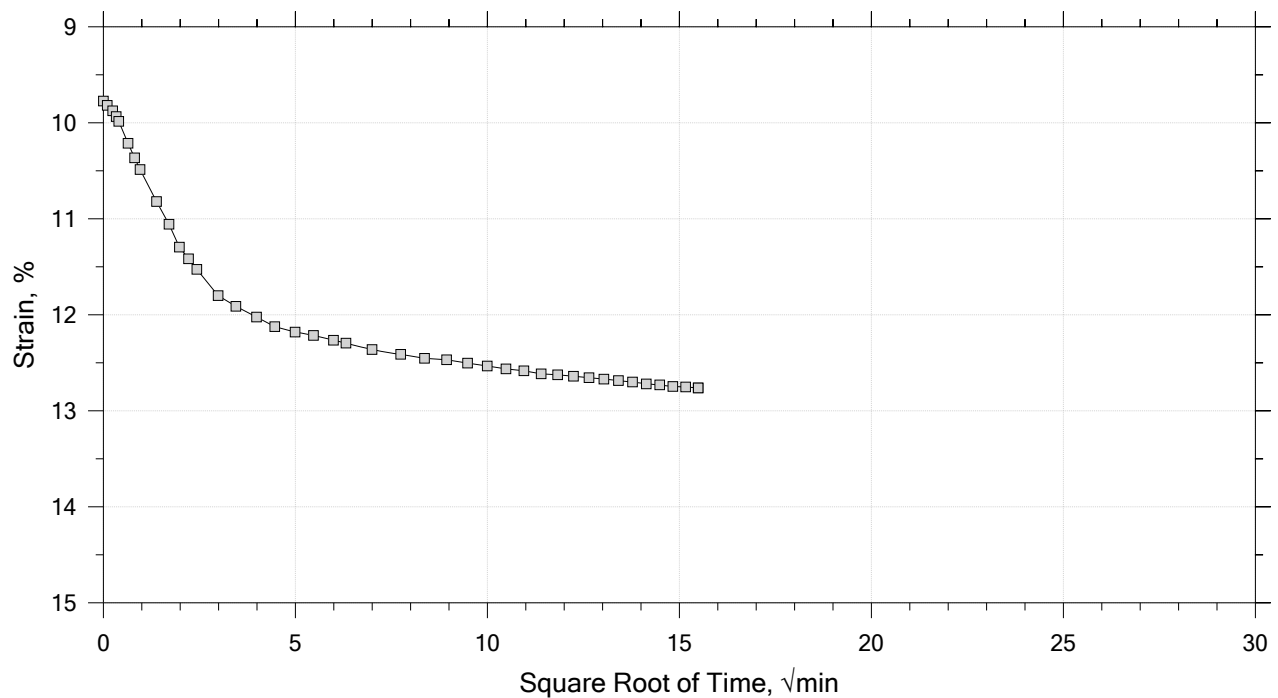
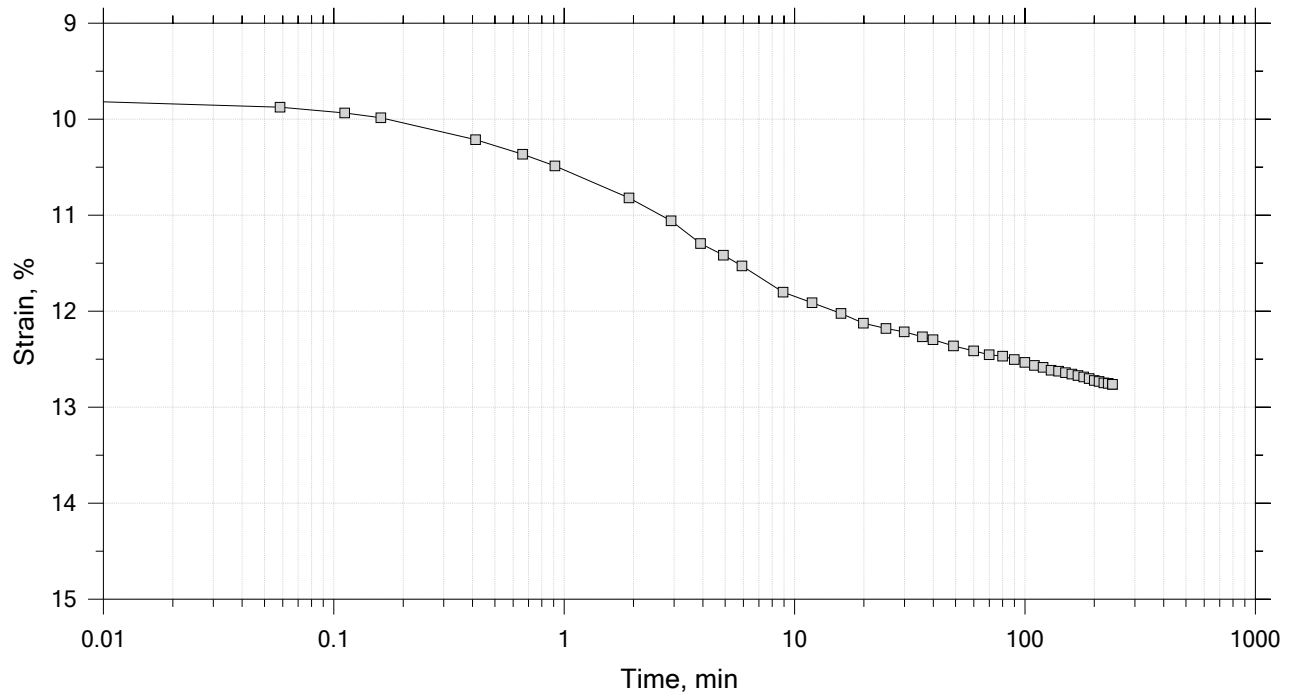
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	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



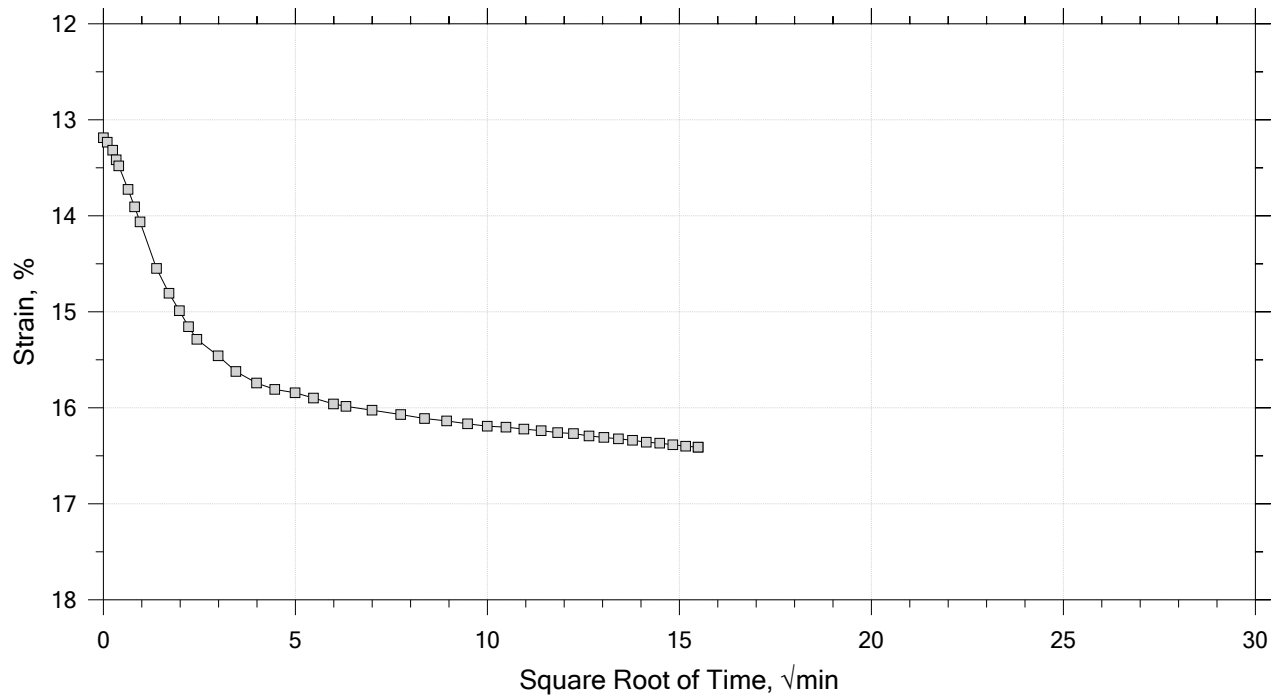
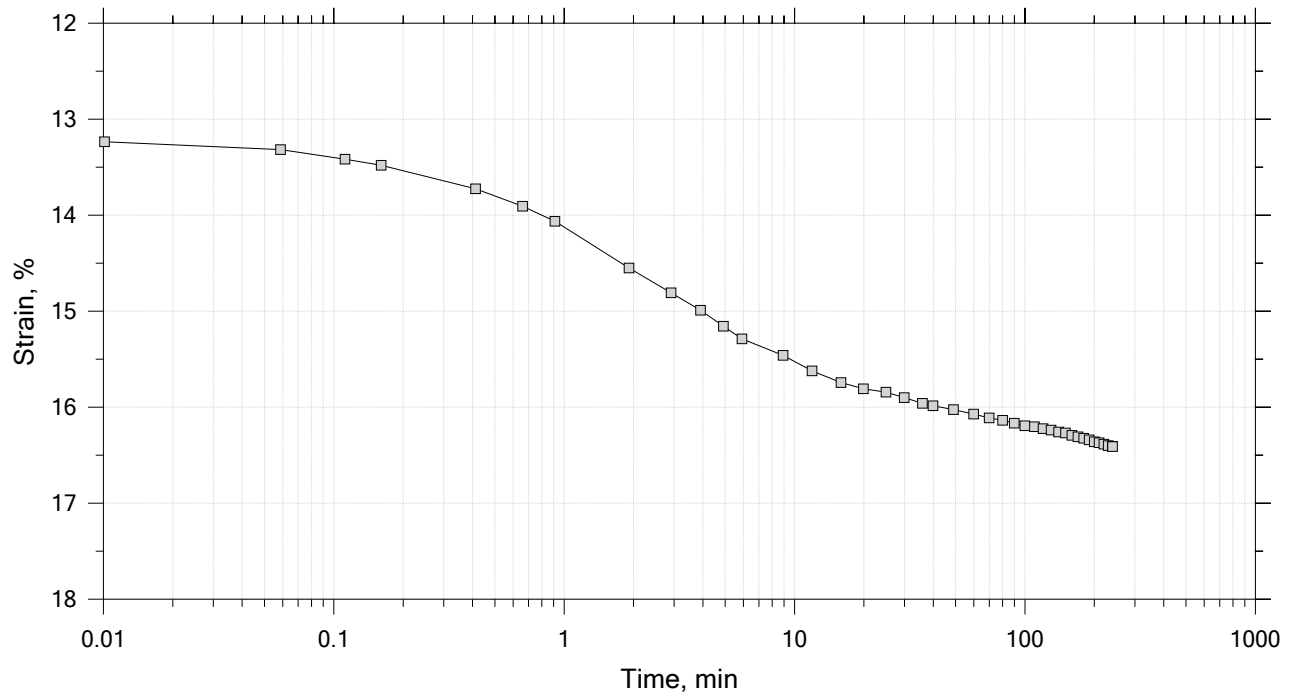
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



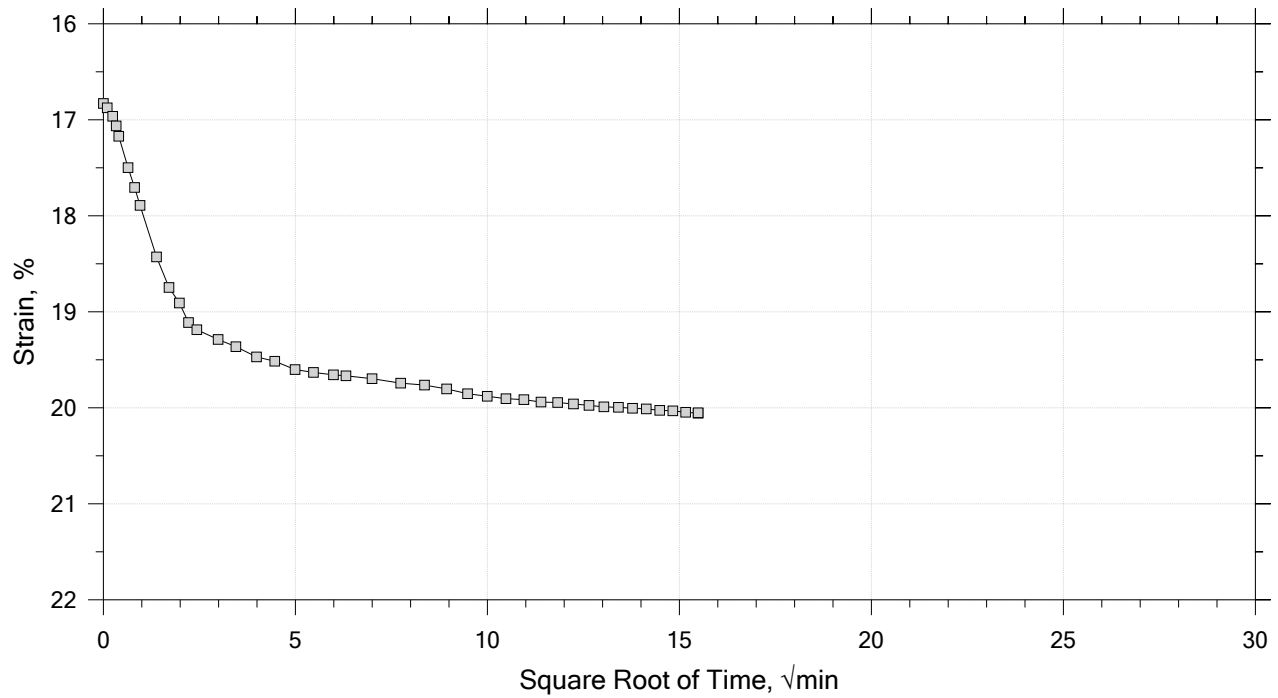
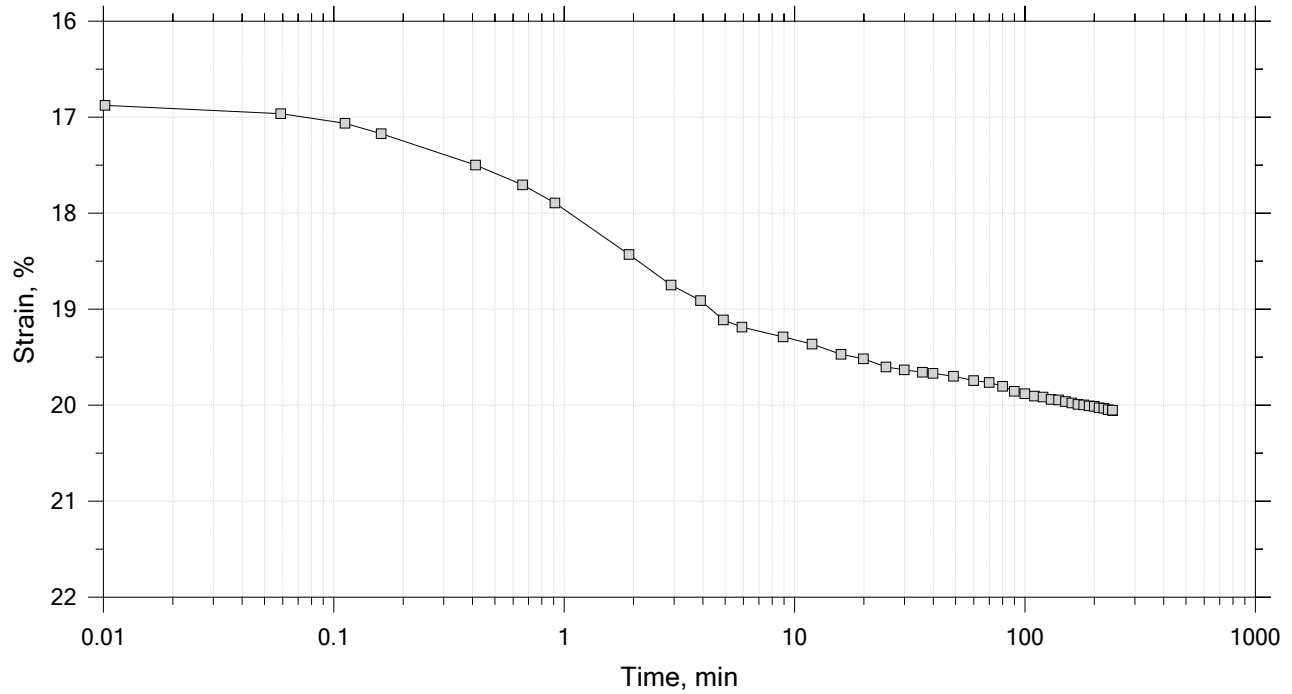
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



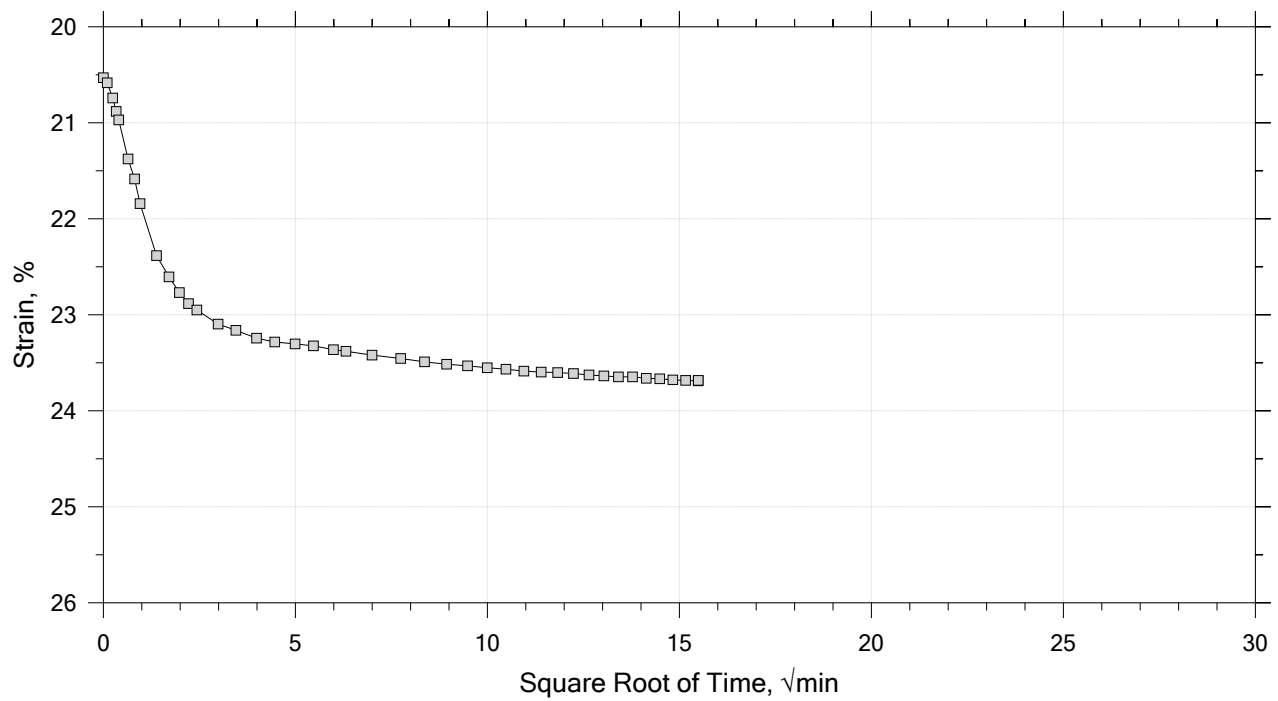
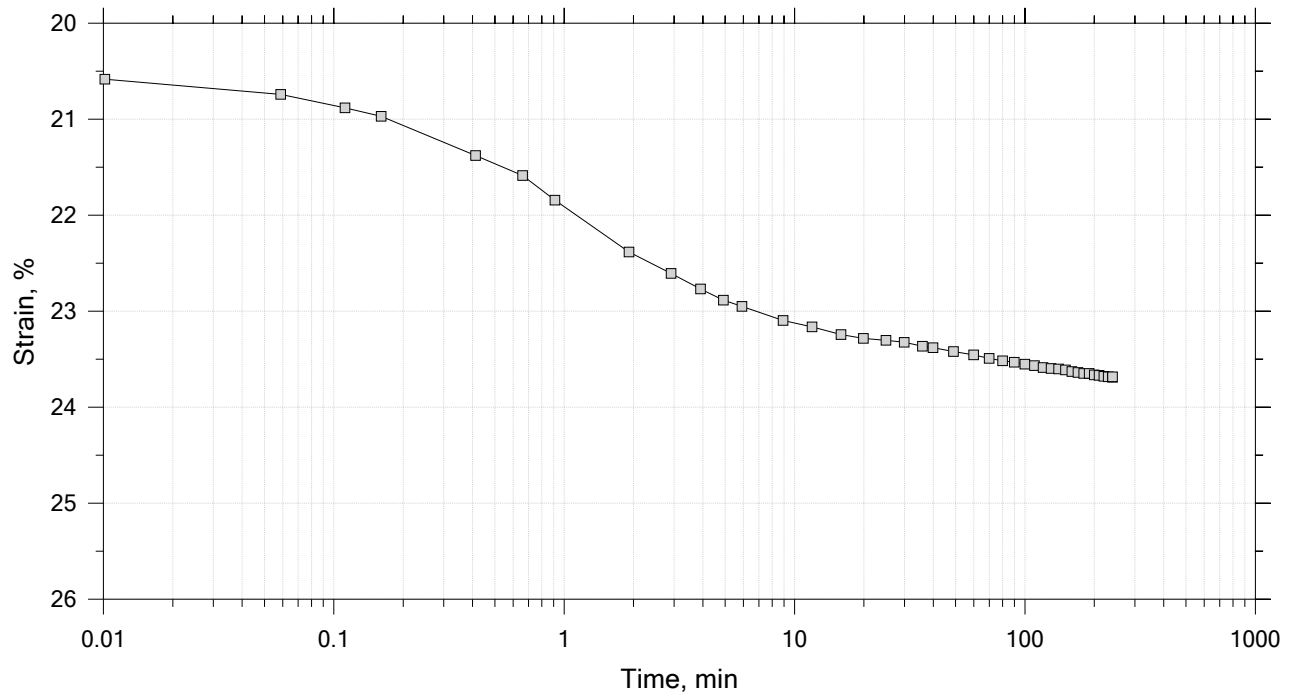
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



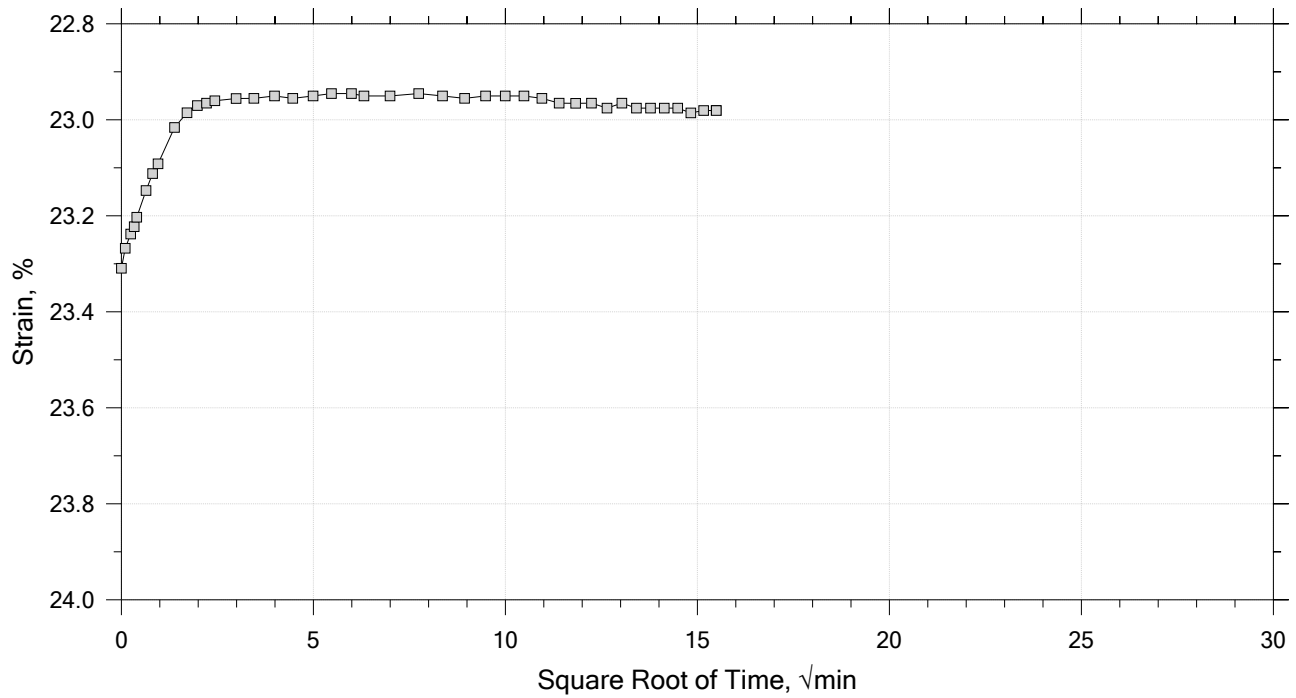
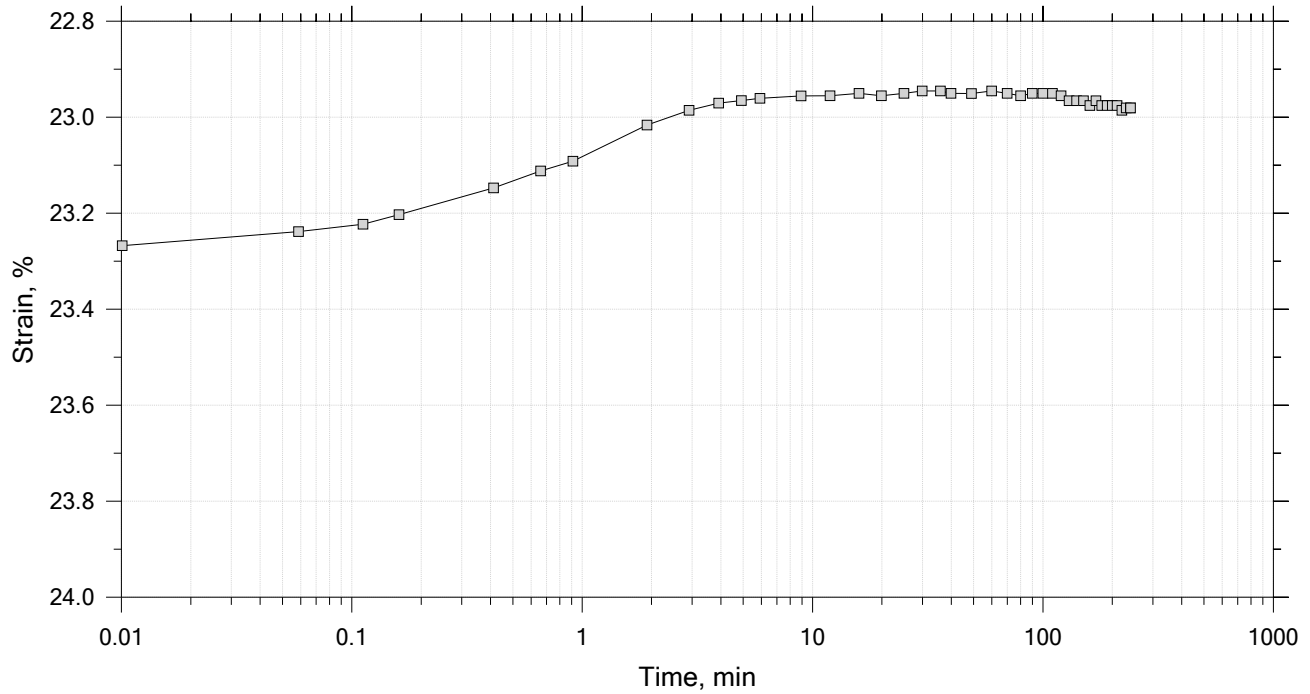
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	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



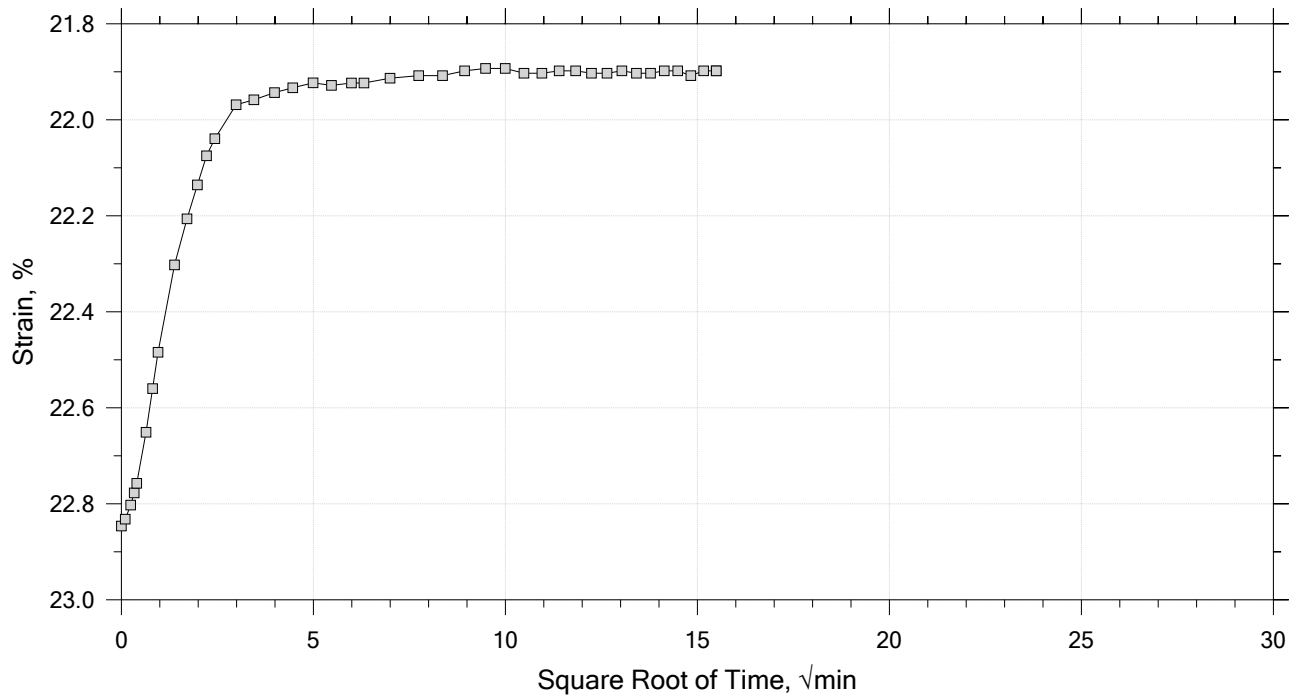
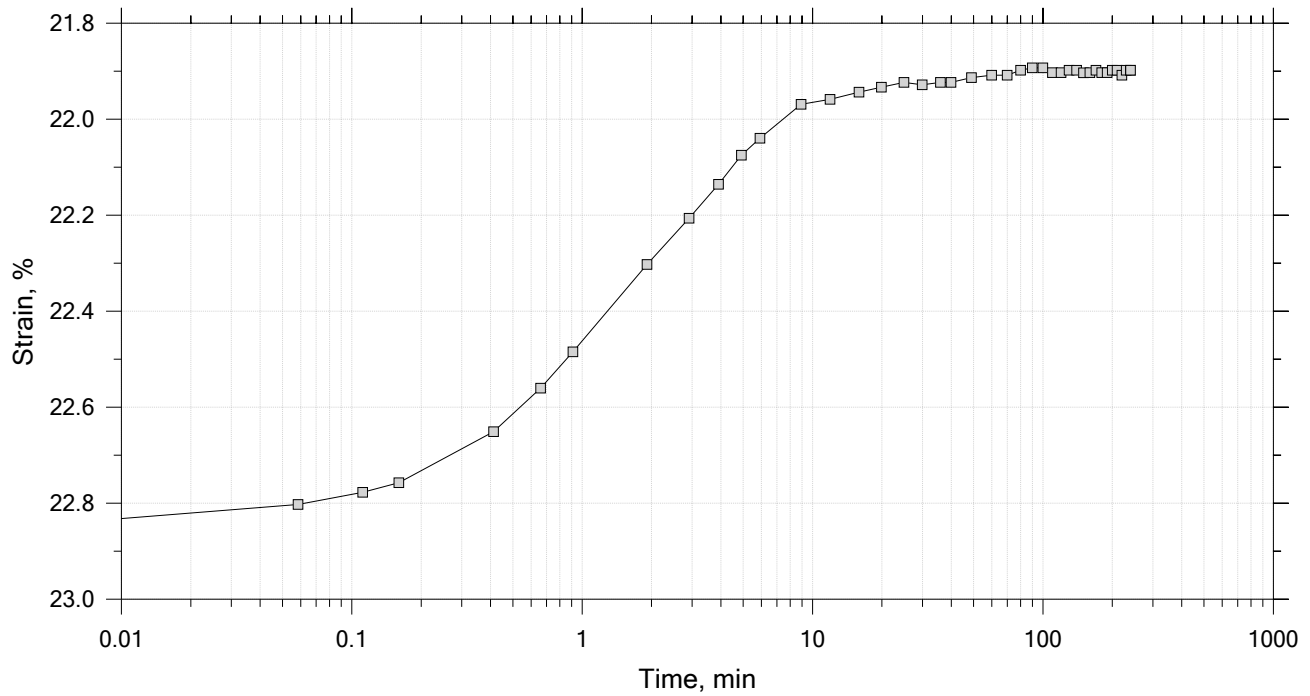
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



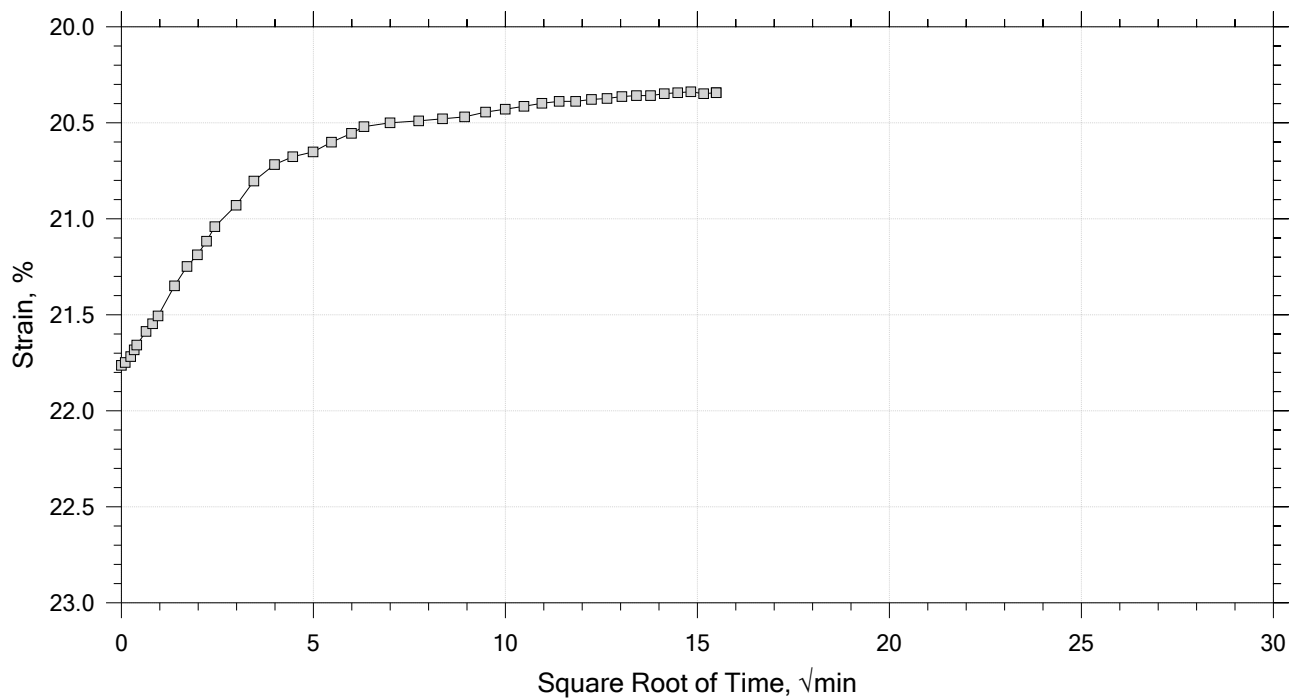
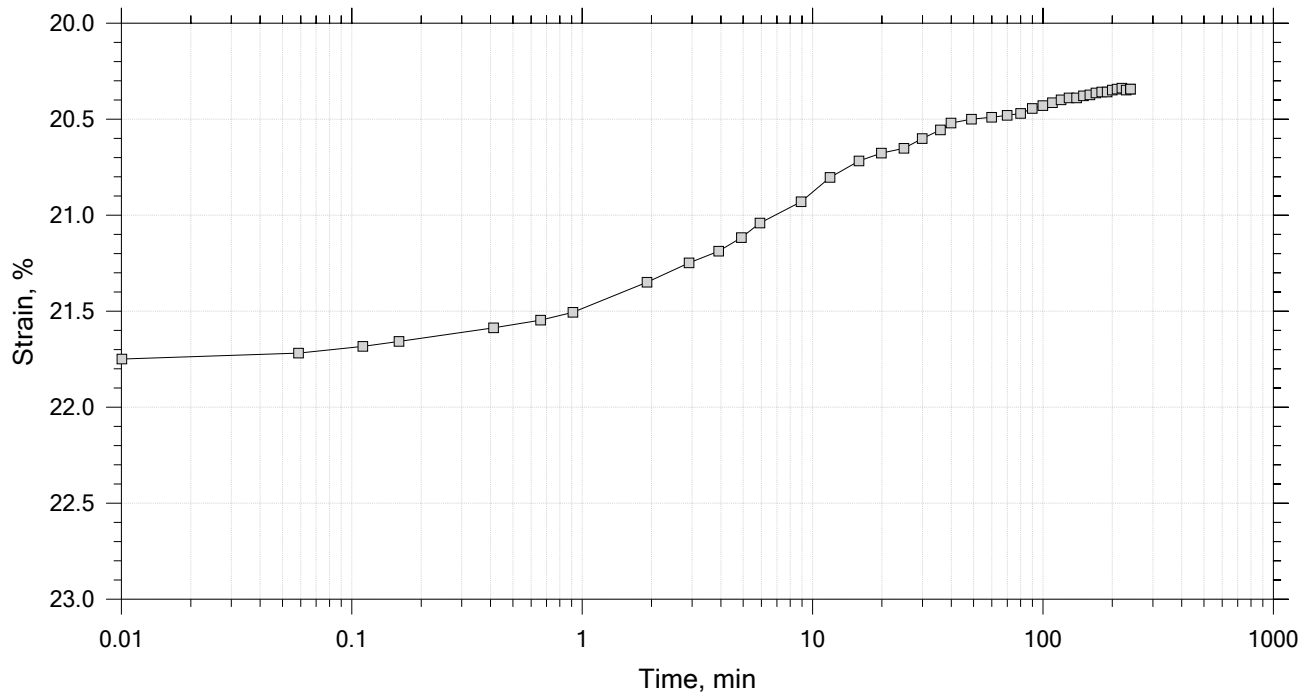
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



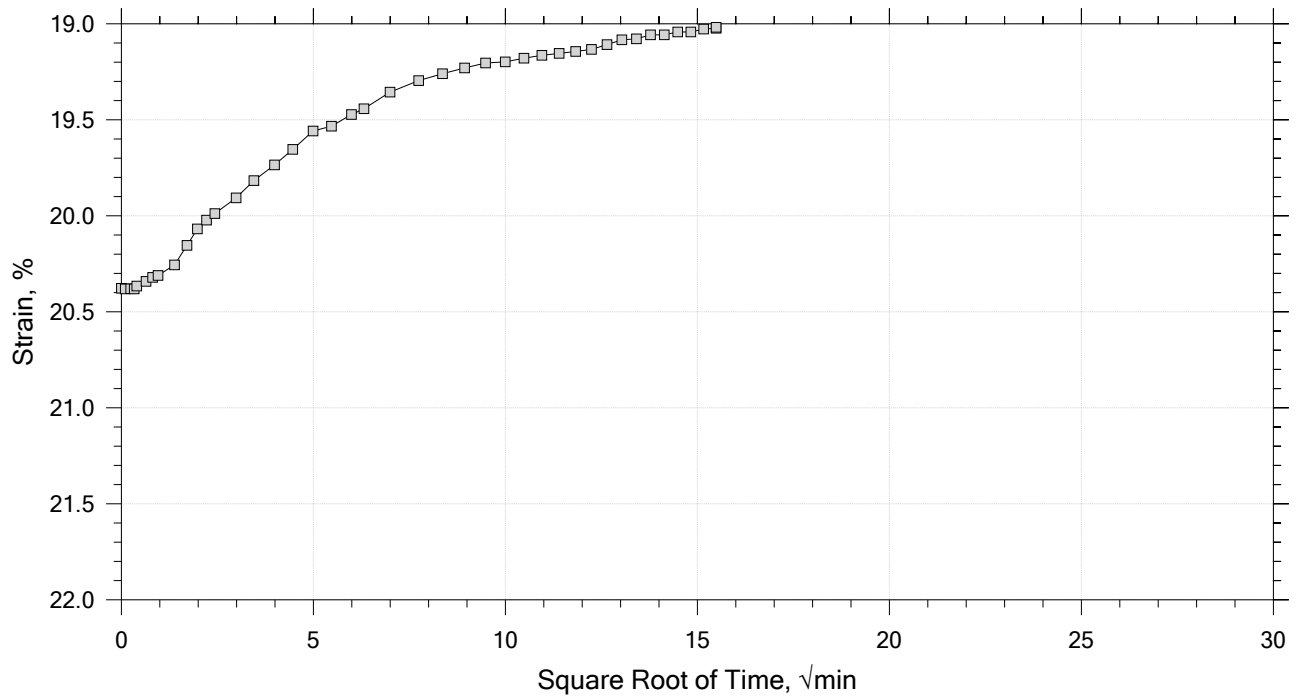
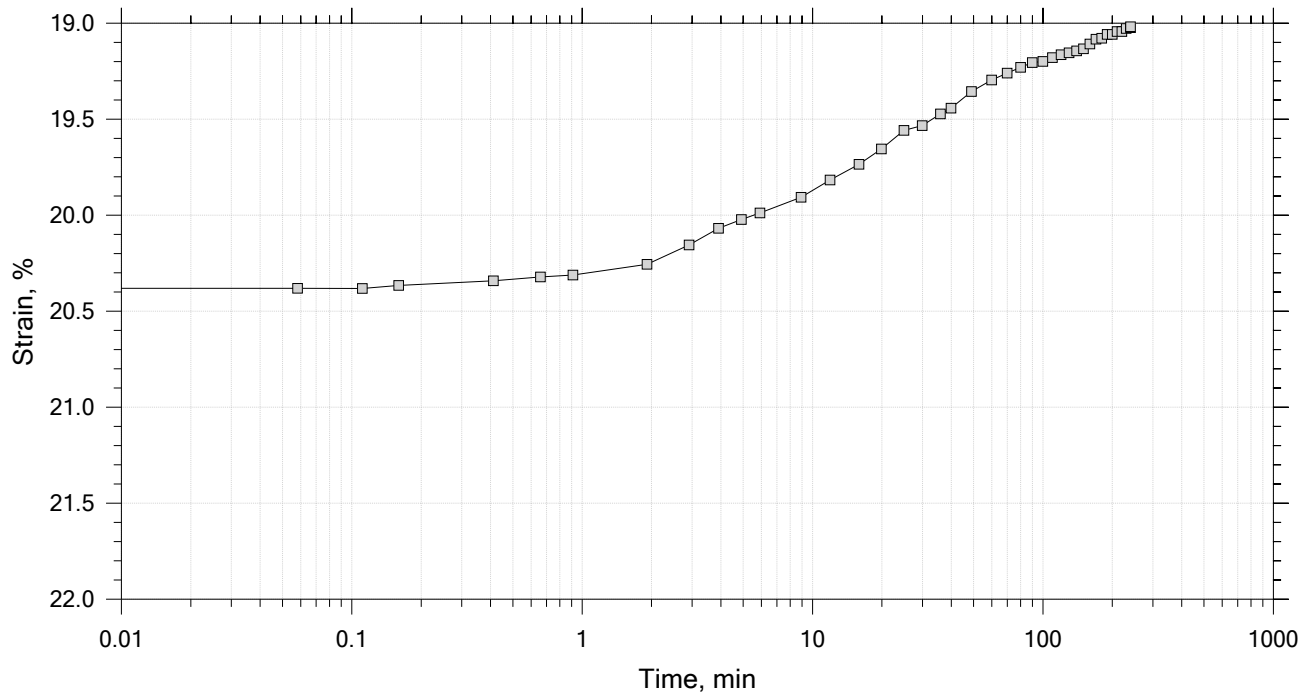
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



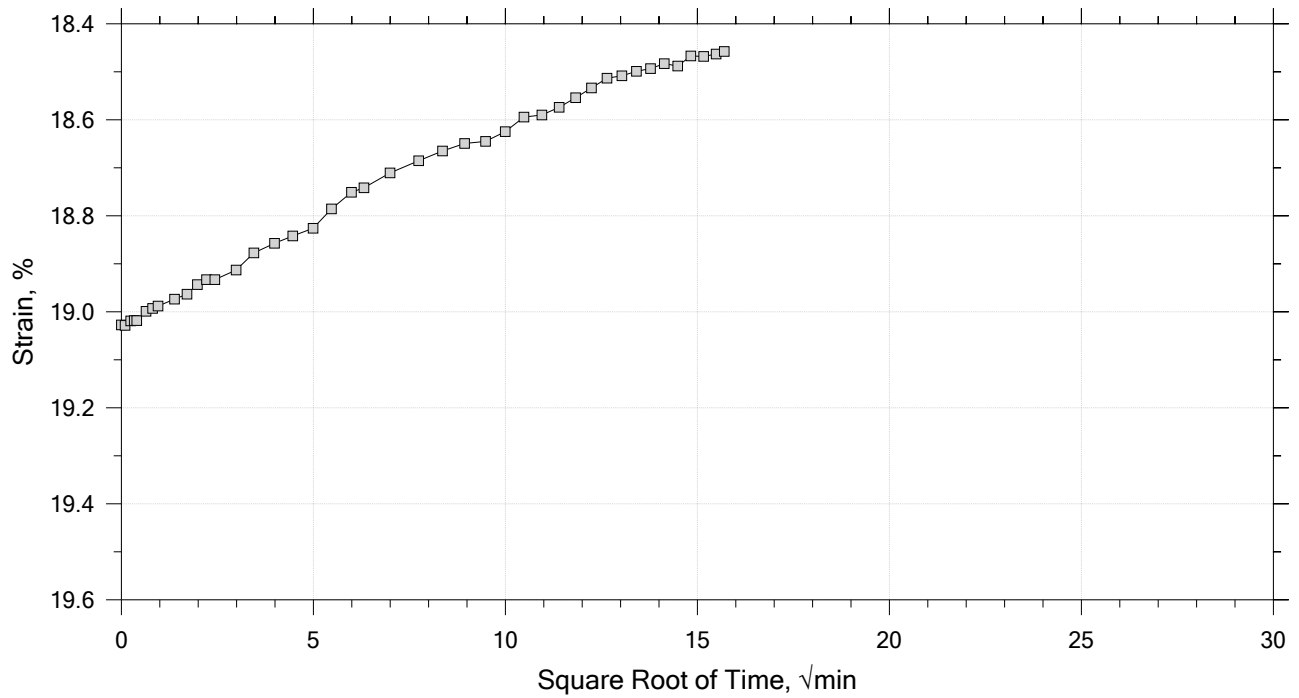
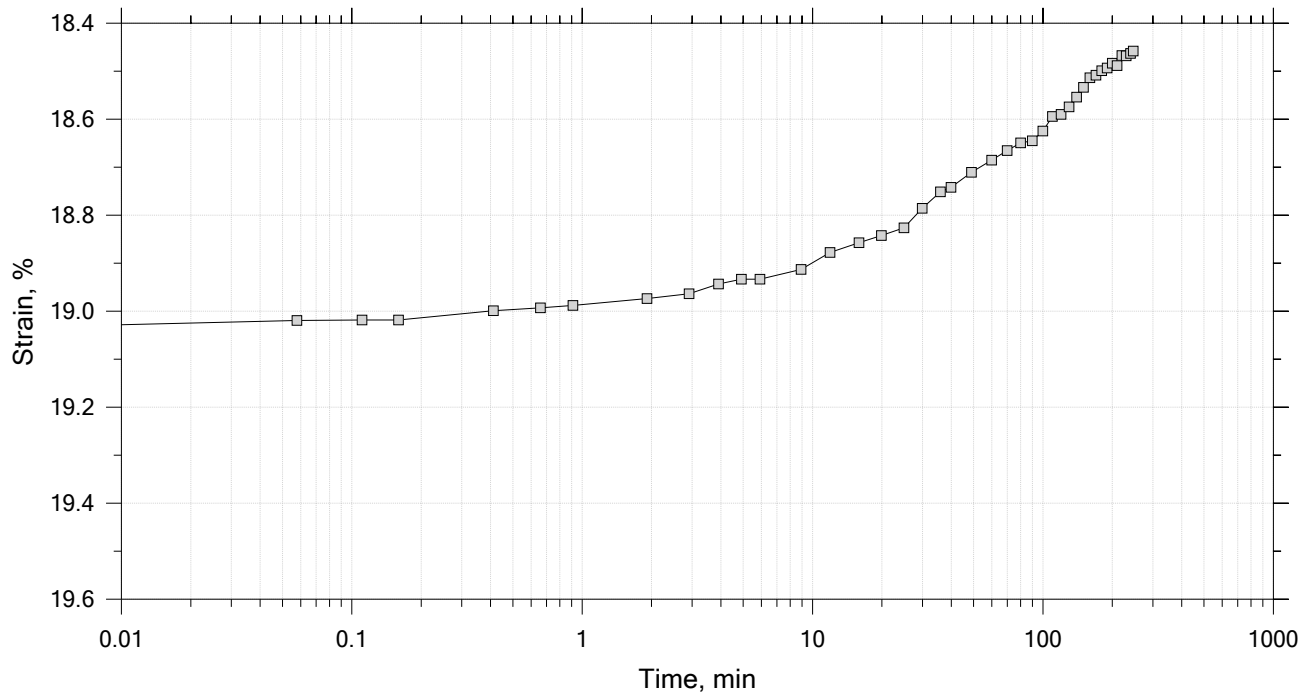
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.72	Liquid Limit: 37
Initial Height: 1.00 in	Initial Void Ratio: 0.953	Plastic Limit: 18
Final Height: 0.82 in	Final Void Ratio: 0.592	Plasticity Index: 19

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D716	RING		D-1488
Mass Container, gm	8.32	111.18	111.18	8.37
Mass Container + Wet Soil, gm	61.98	261.39	247.58	144.54
Mass Container + Dry Soil, gm	47.73	223.18	223.18	120.18
Mass Dry Soil, gm	39.41	112	112	111.81
Water Content, %	36.16	34.12	21.79	21.79
Void Ratio	---	0.95	0.59	---
Degree of Saturation, %	---	97.36	100.00	---
Dry Unit Weight, pcf	---	86.92	106.6	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		
Displacement at End of Increment			

One-Dimensional Consolidation by ASTM D2435 - Method B

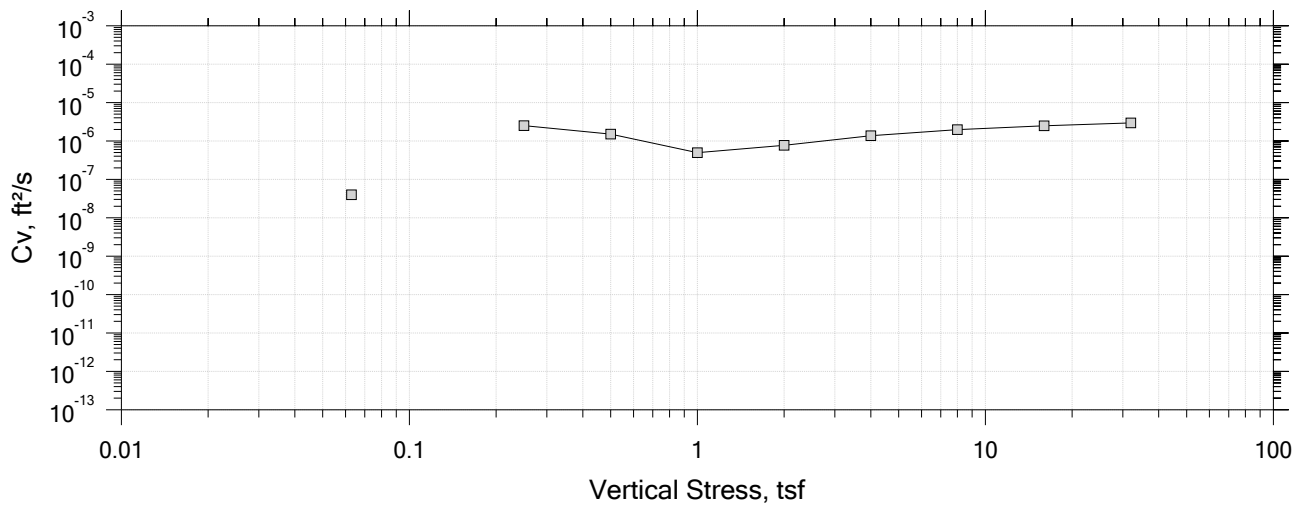
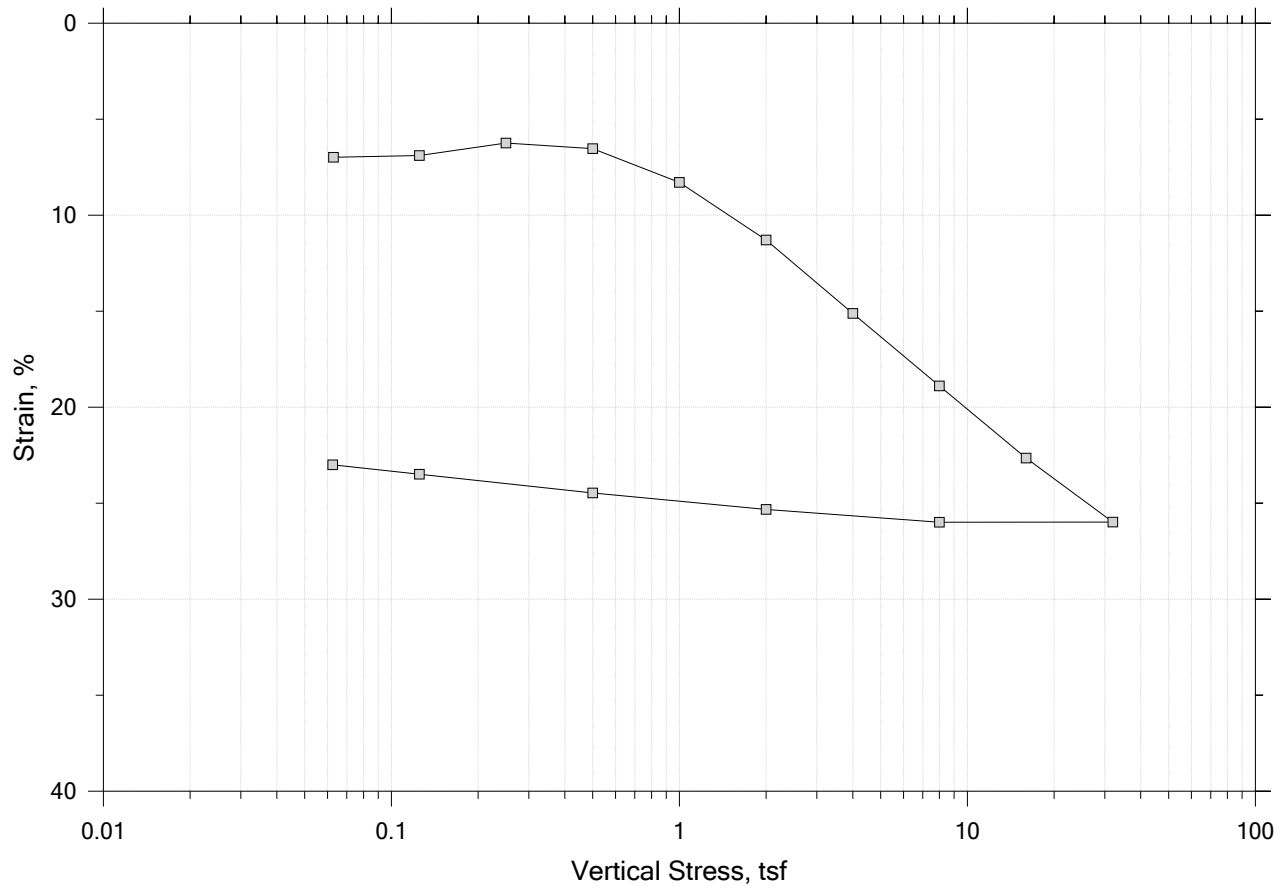
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-135	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/18/19	Depth: 5-7 ft
	Test No.: IP-19	Sample Type: intact	Elevation: ---
	Description: Moist, dark gray clay		
	Remarks: System O, Swell Pressure = 0.0666 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

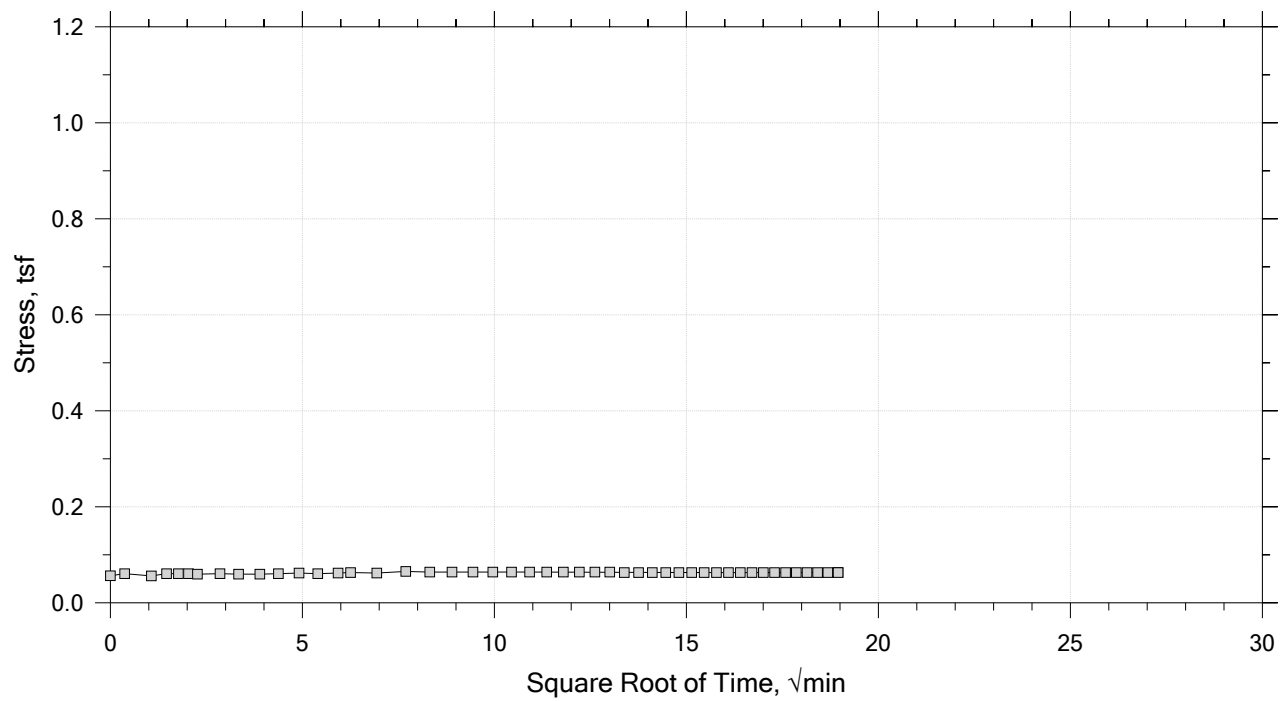
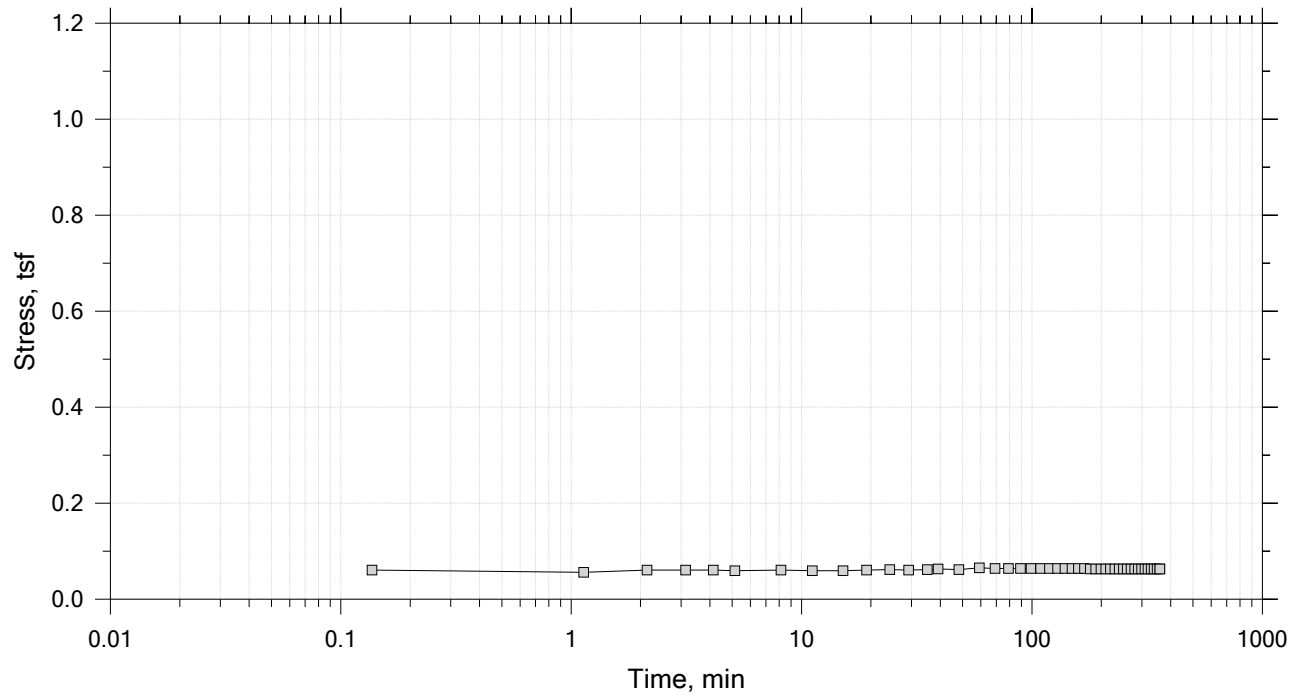
Summary Report




	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0629 tsf



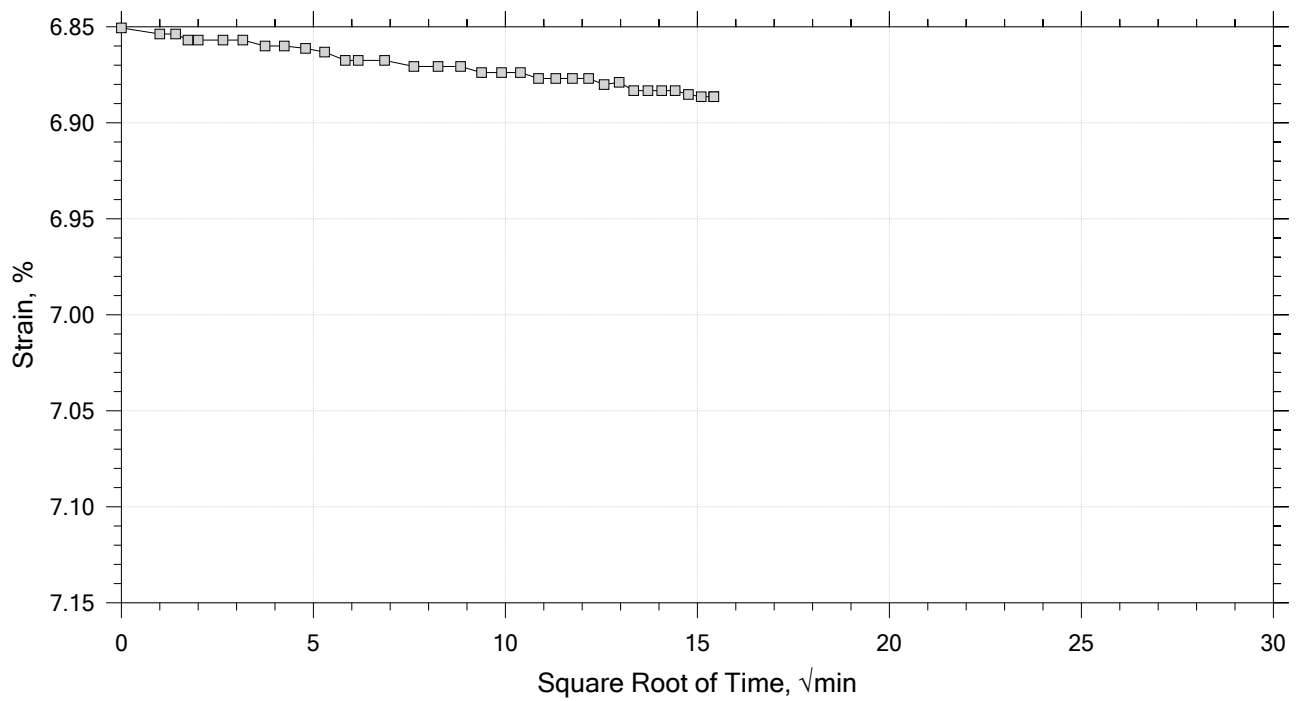
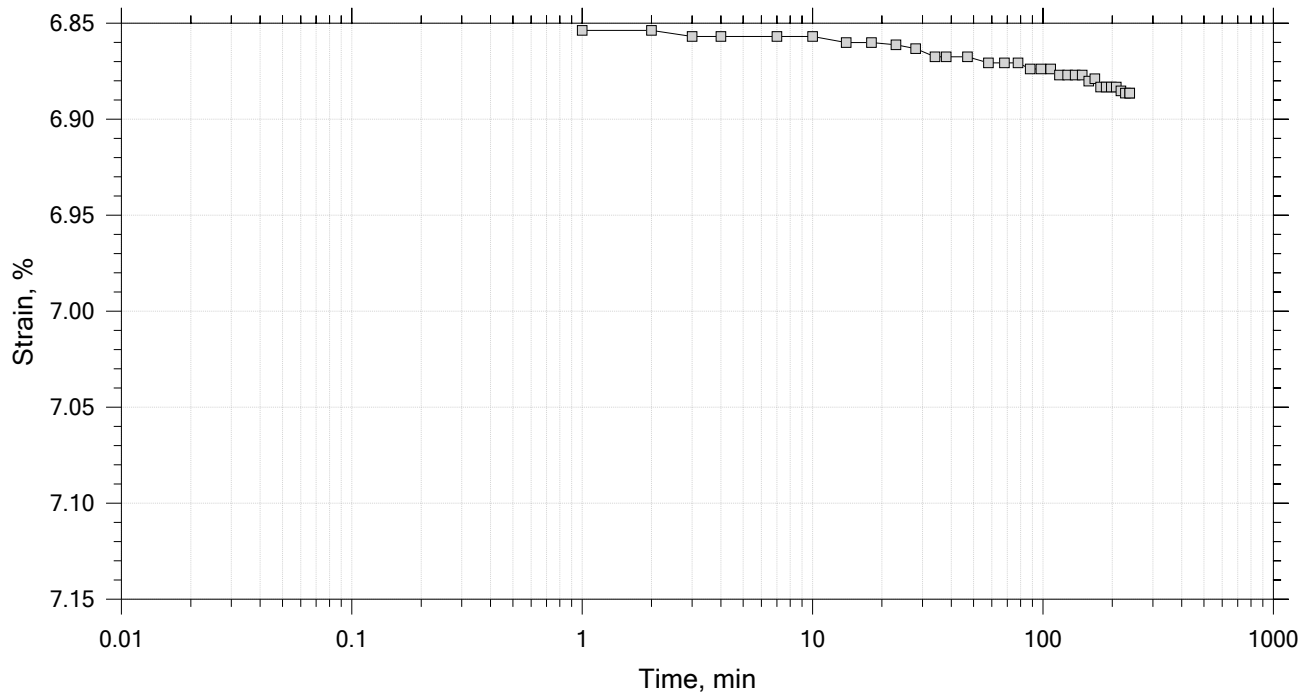
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



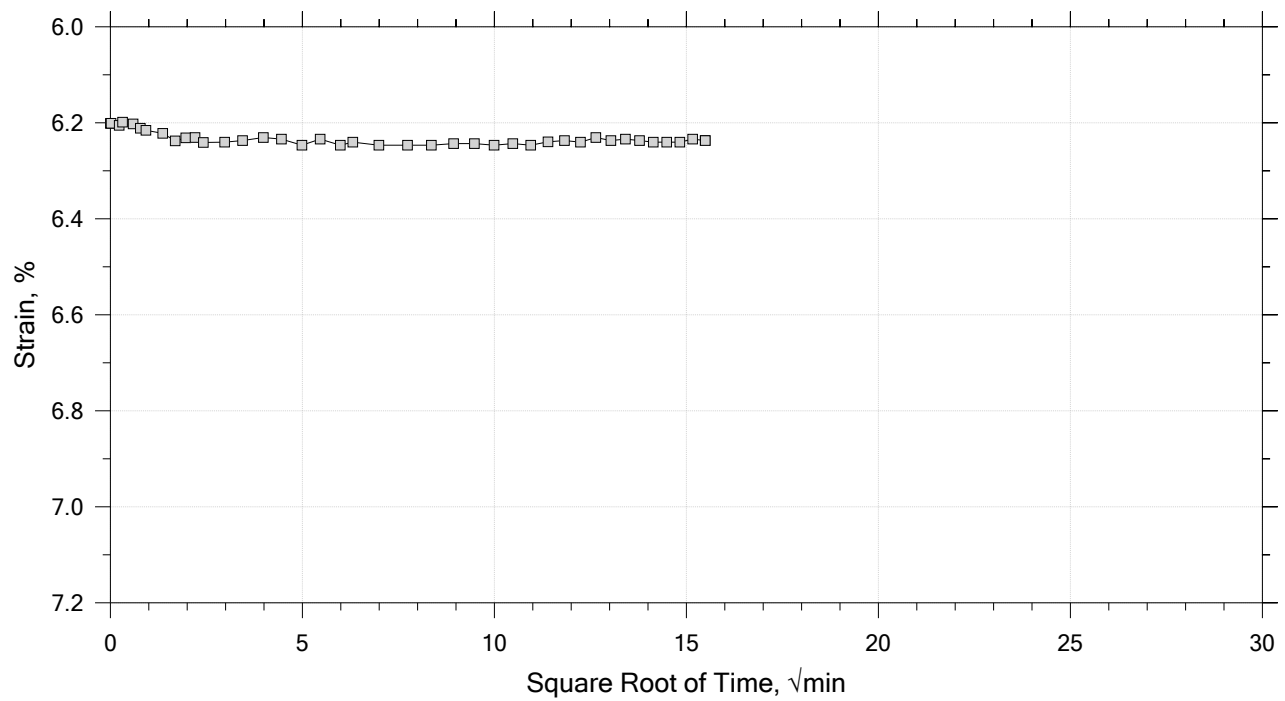
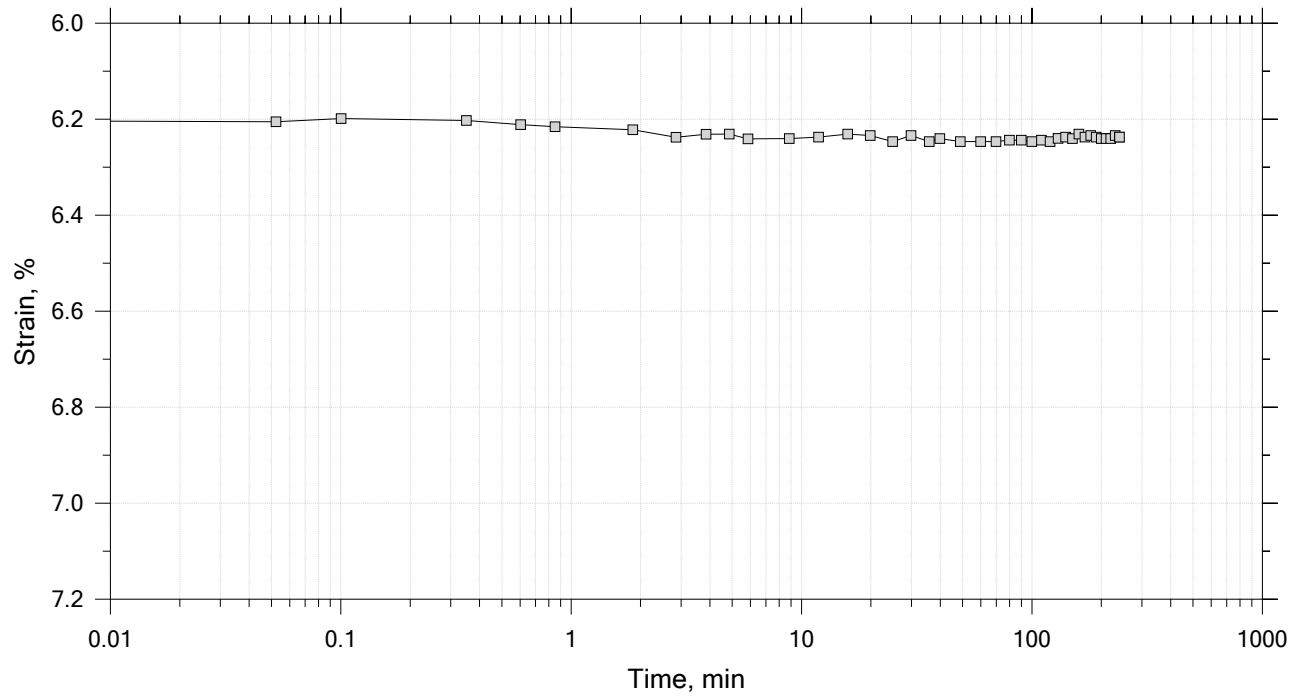
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



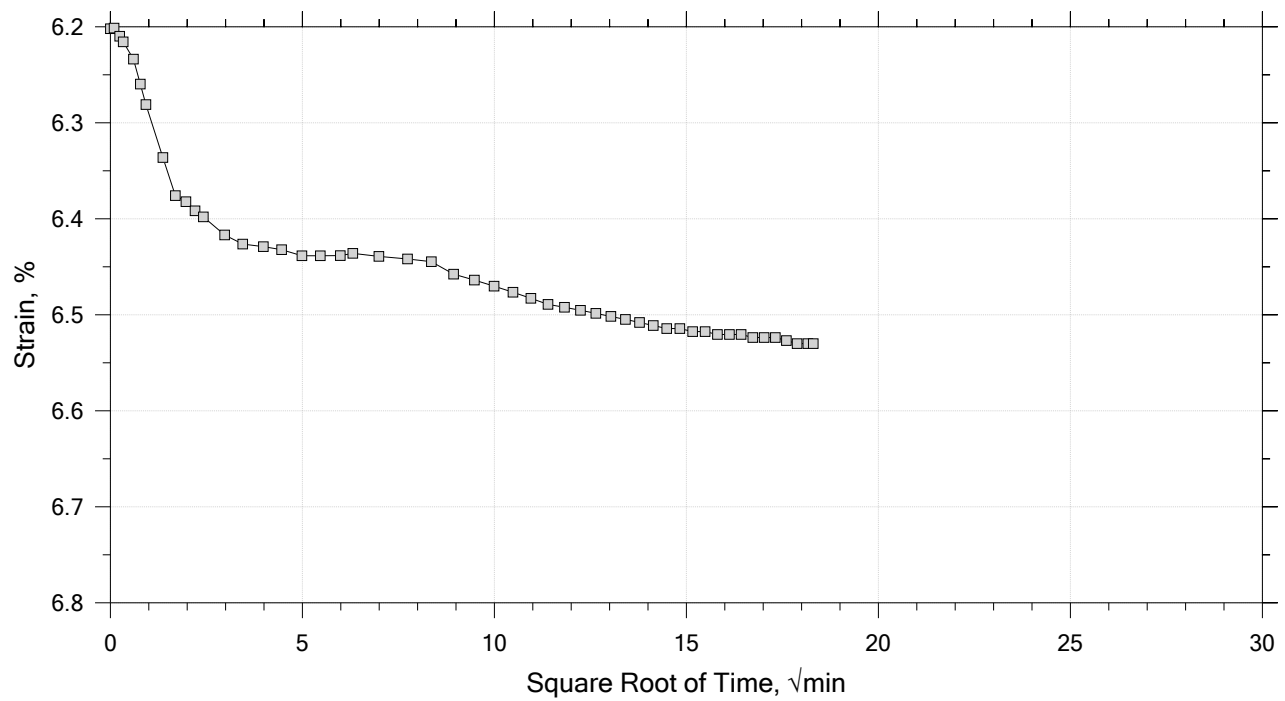
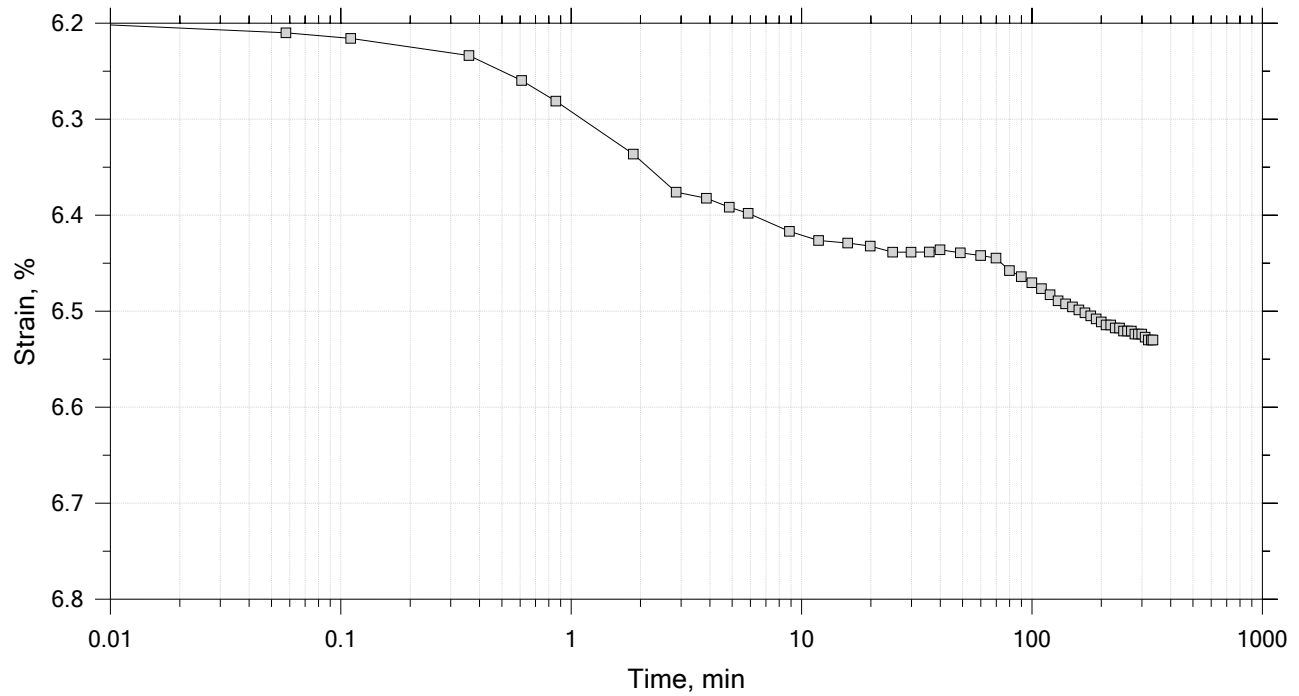
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



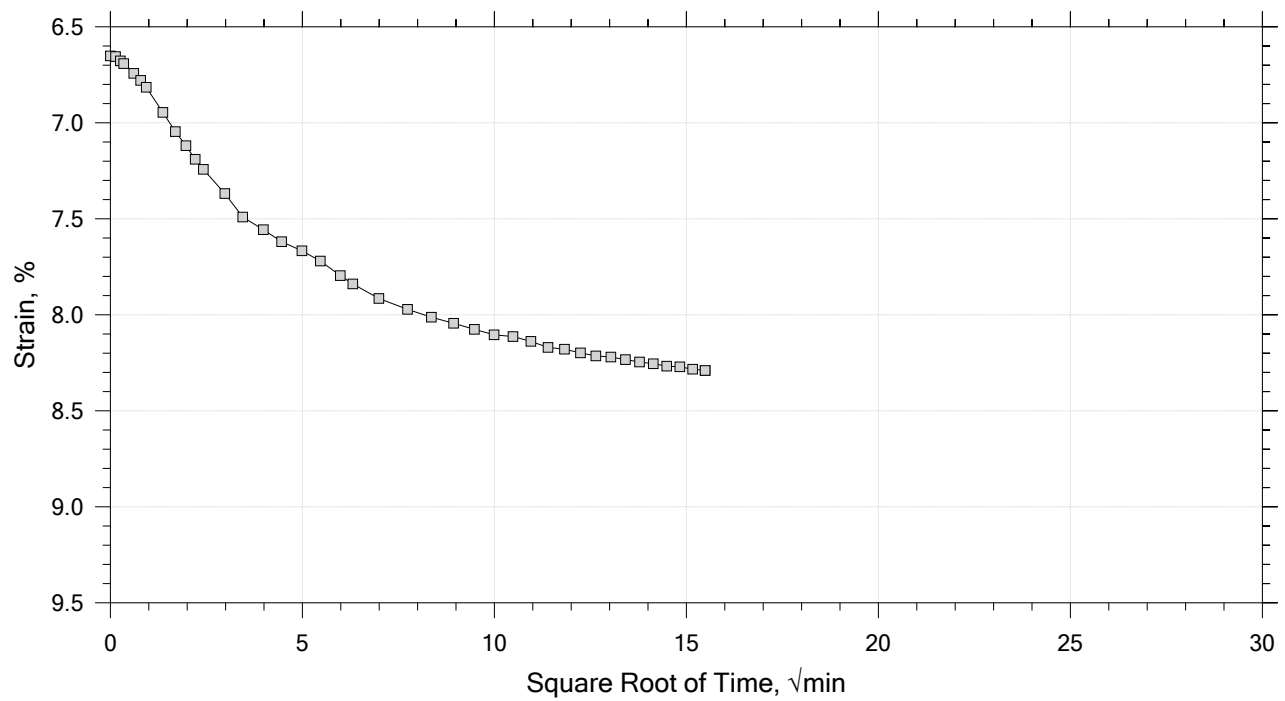
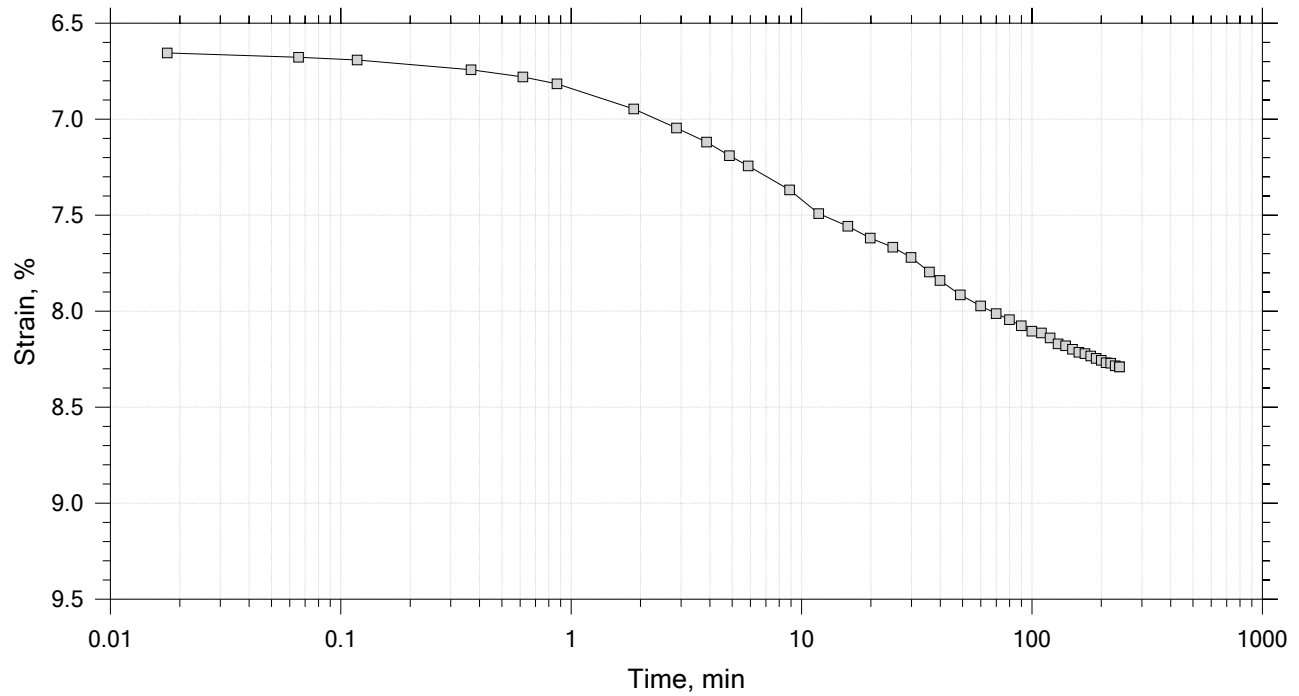
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



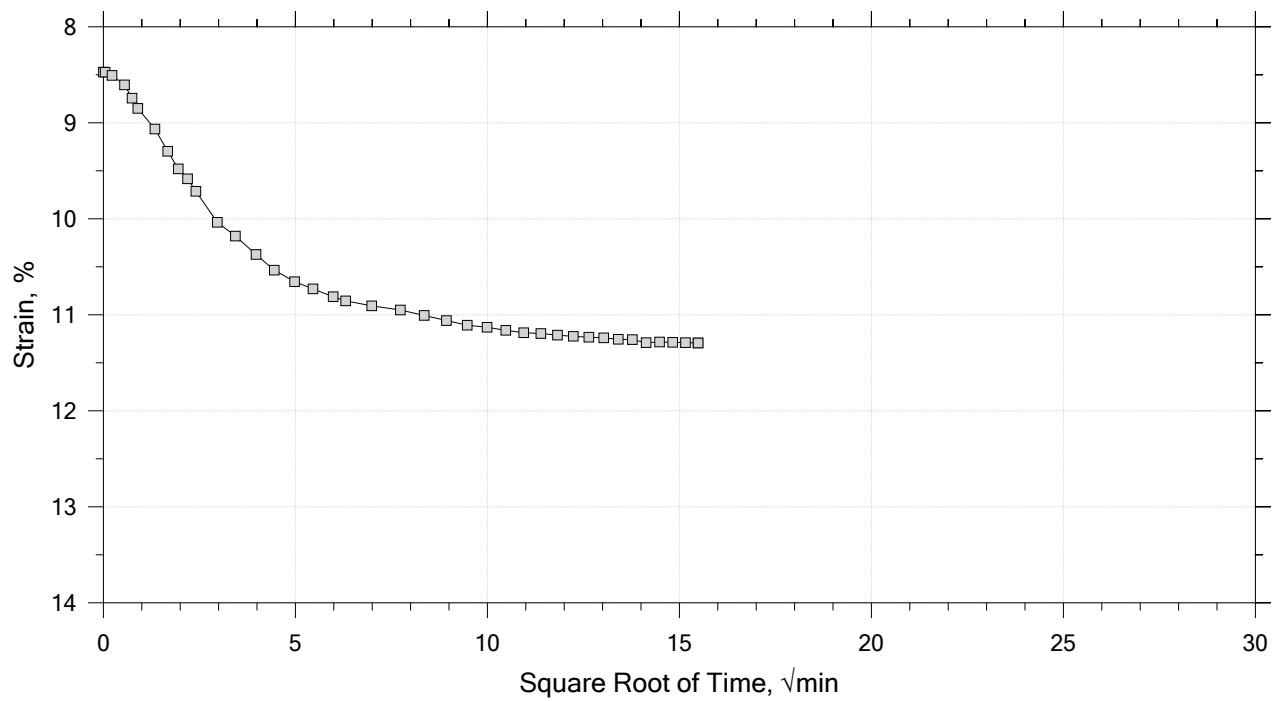
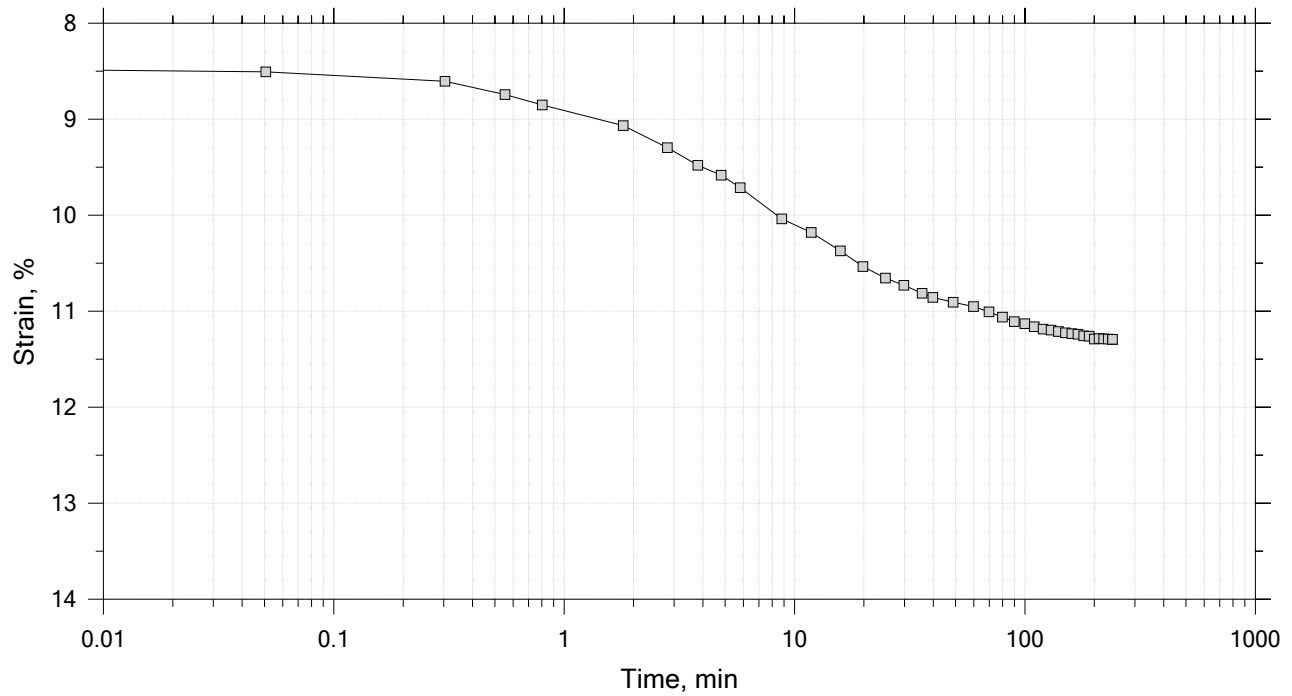
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



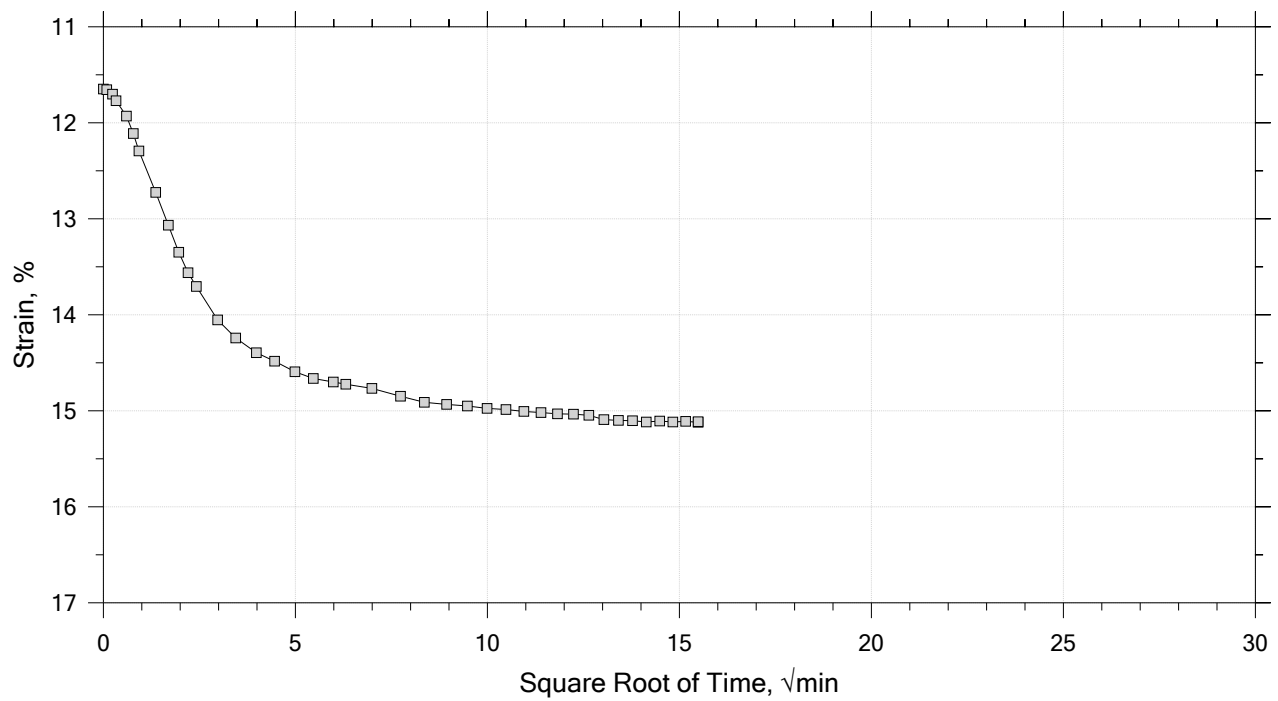
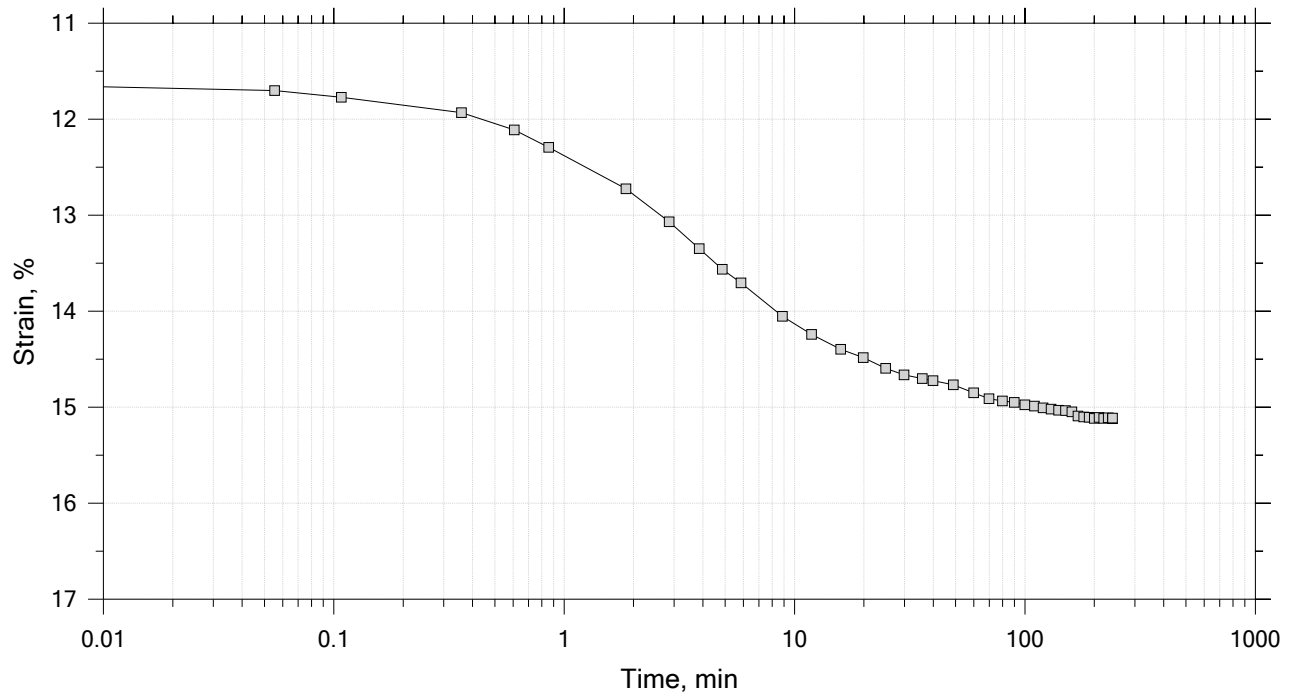
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



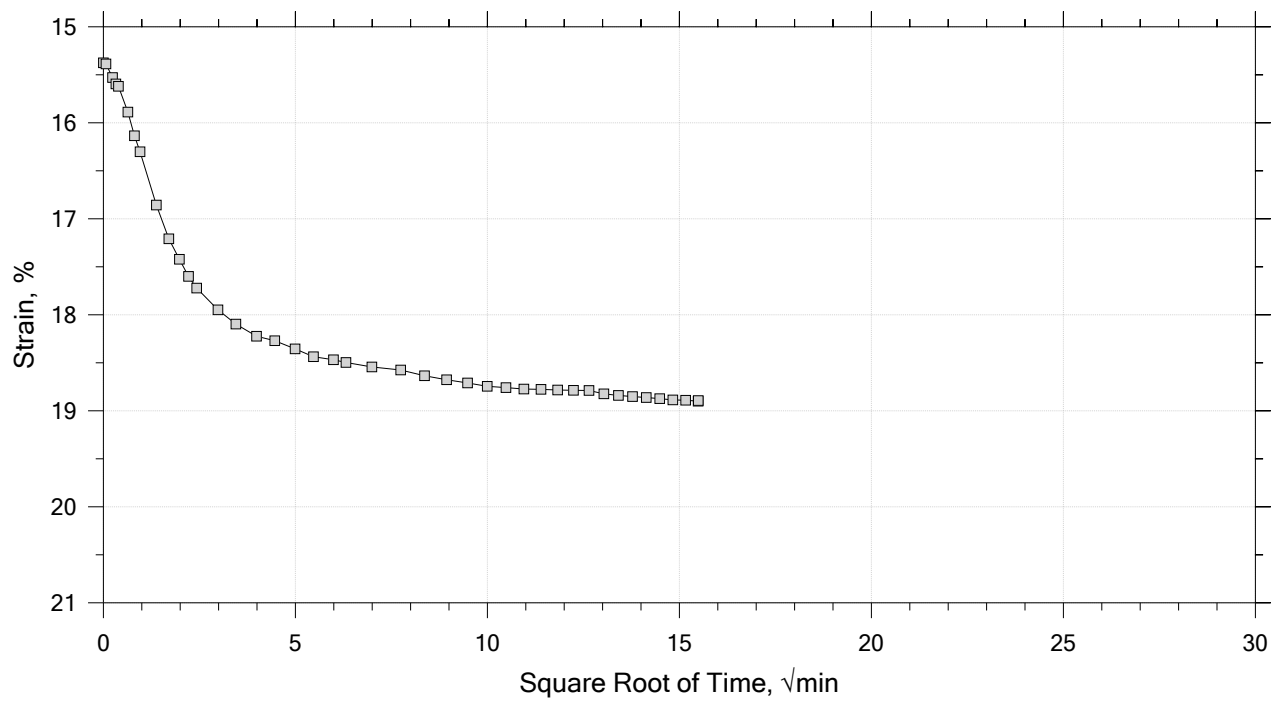
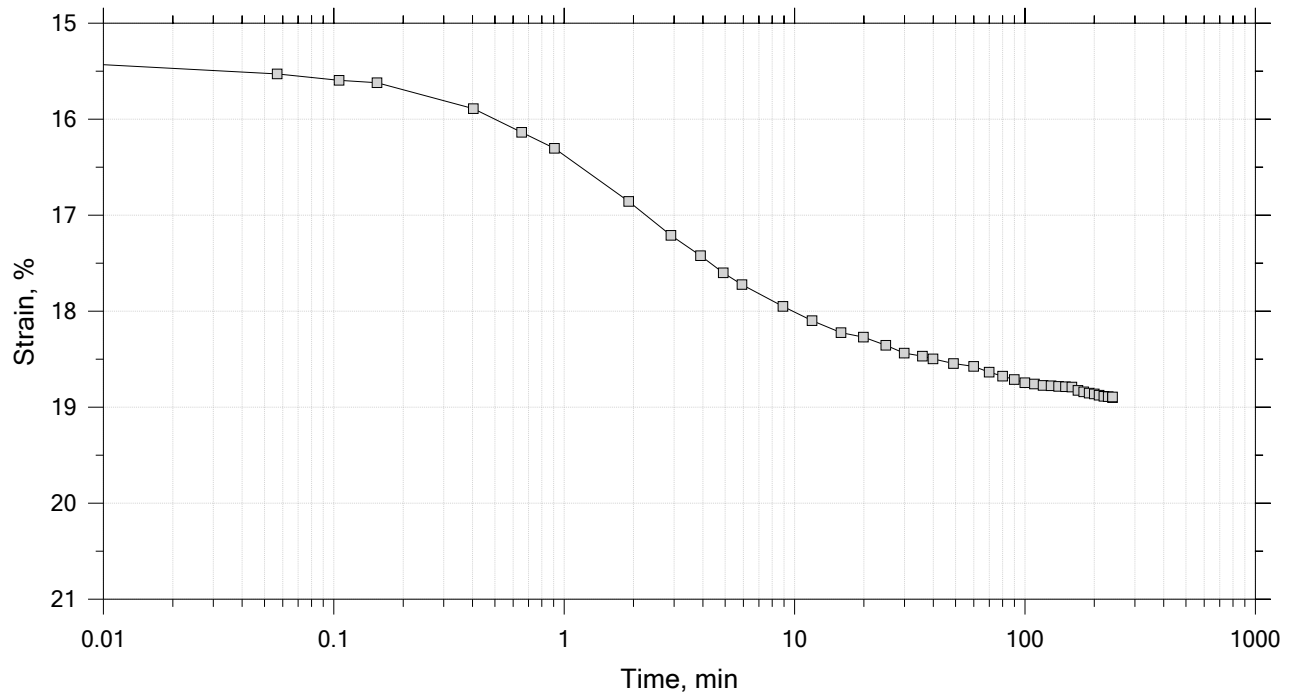
	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



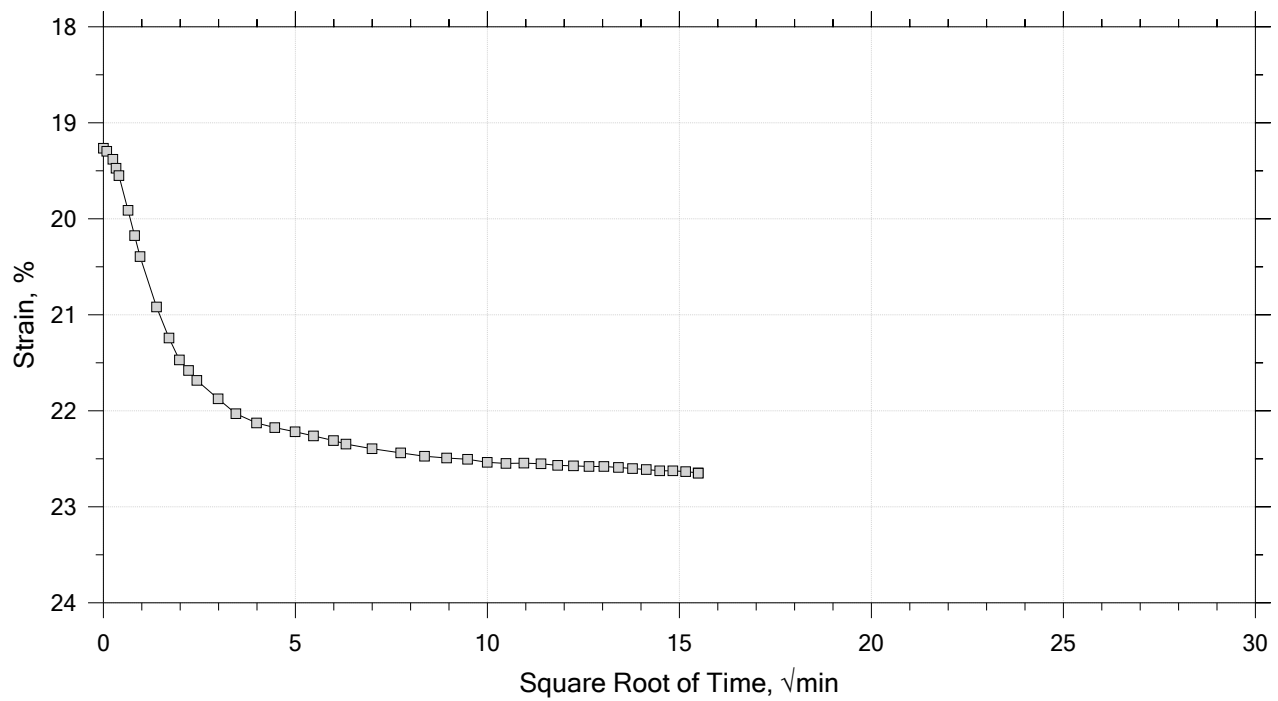
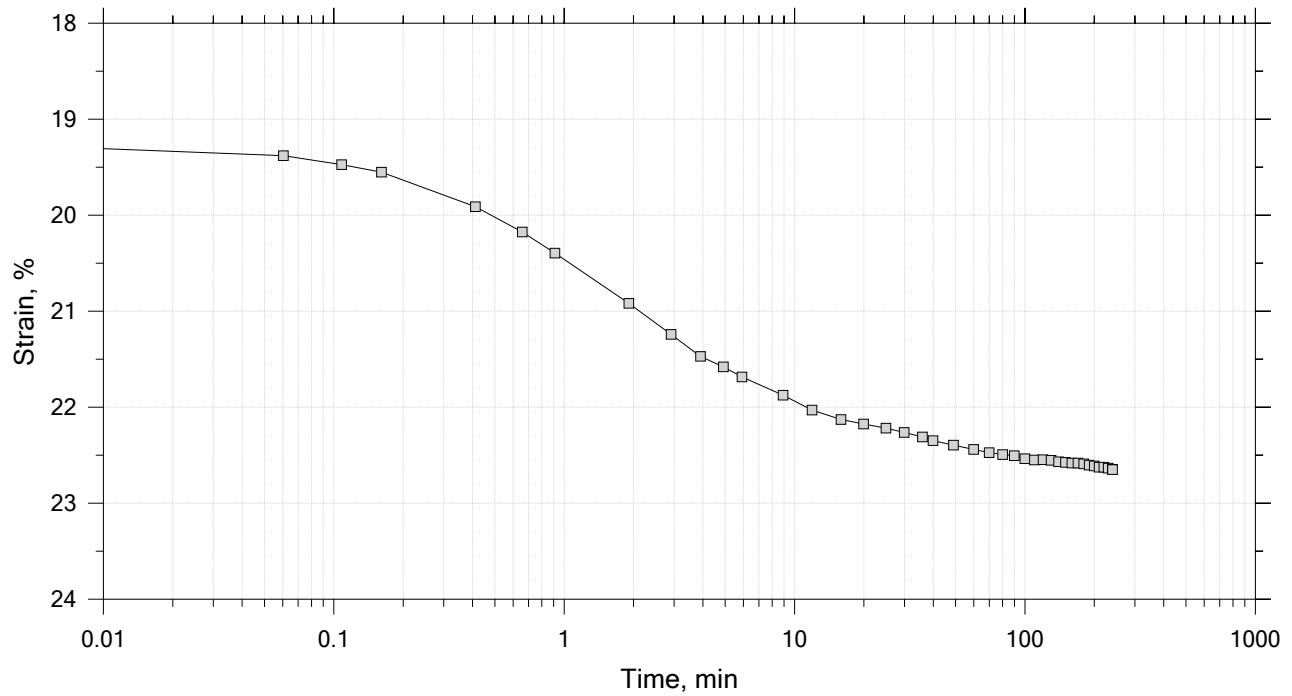
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



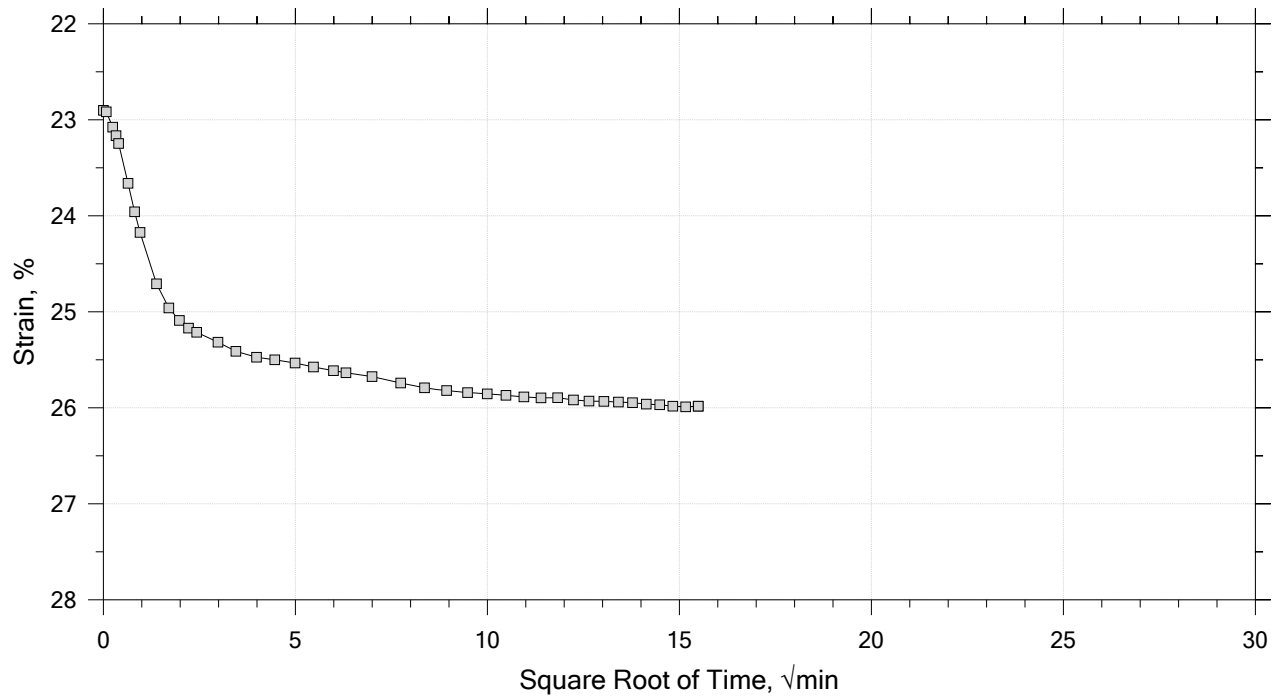
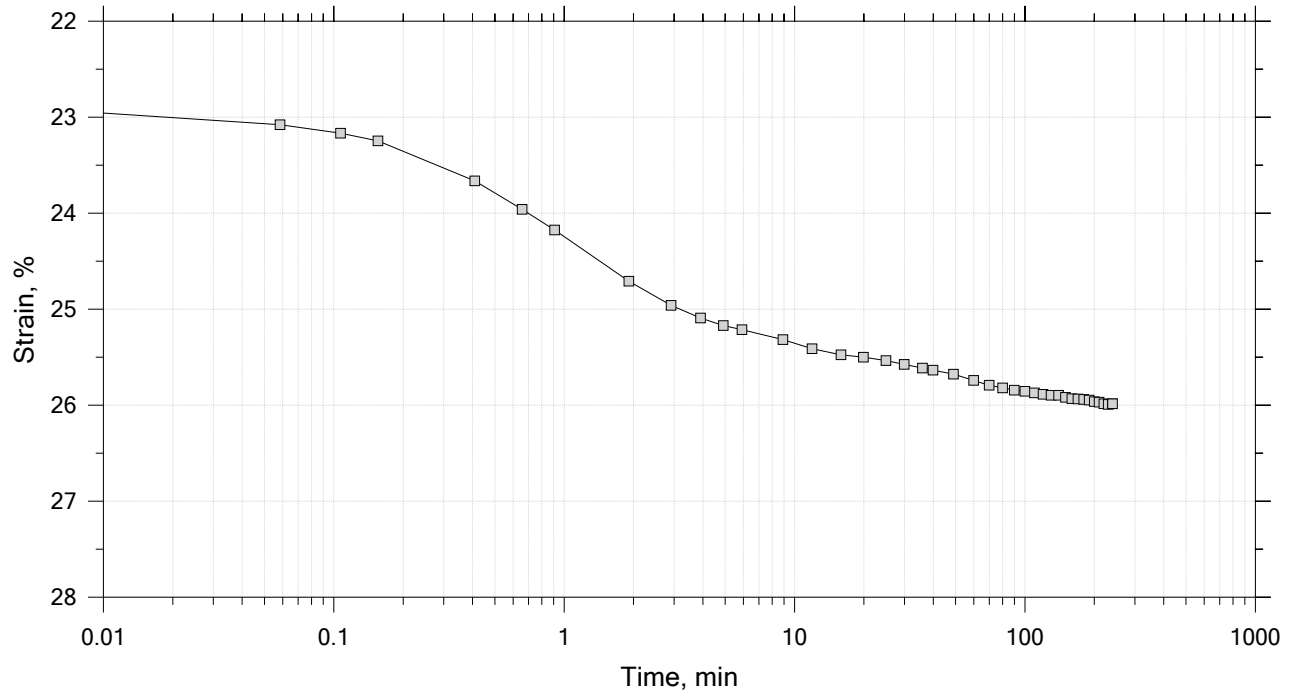
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



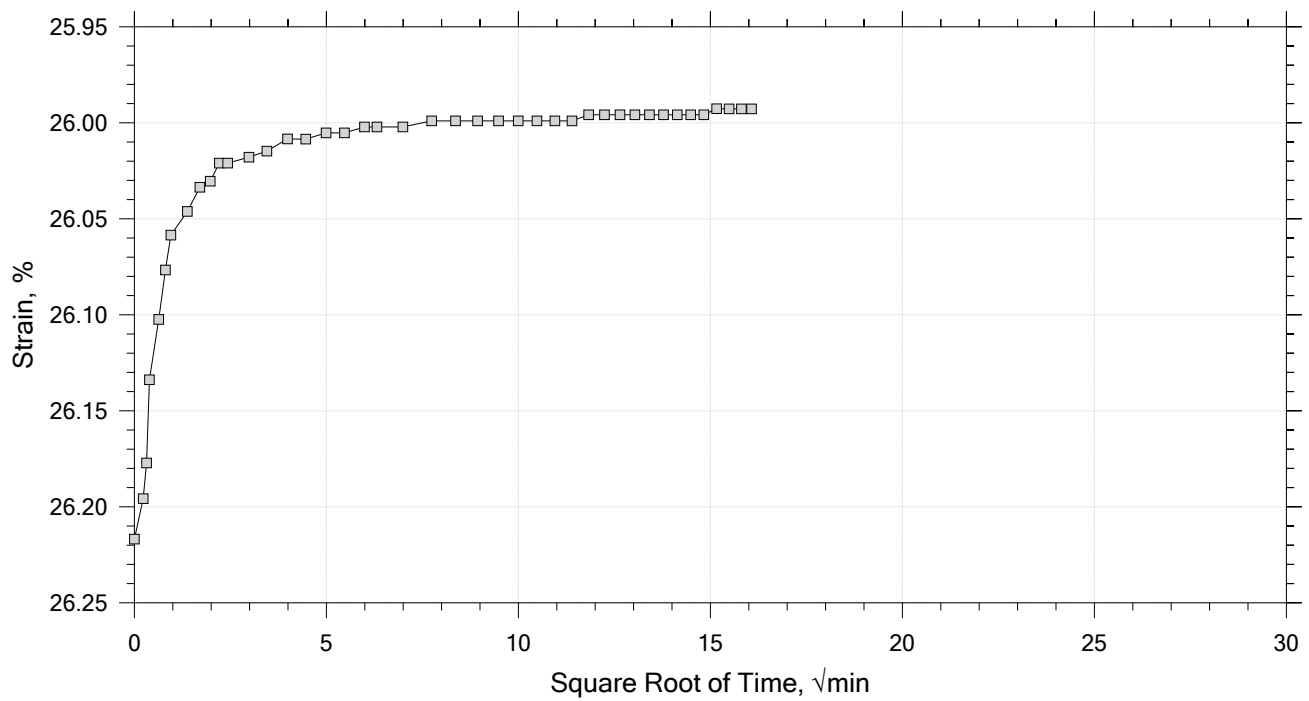
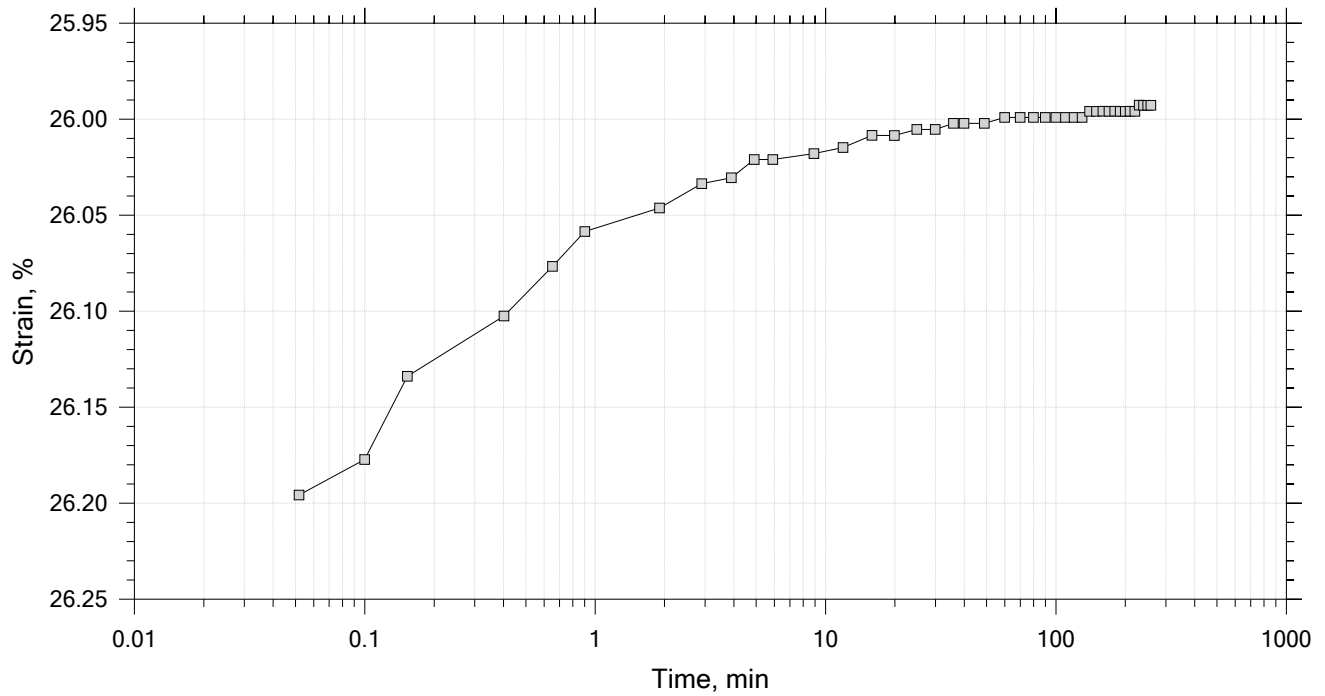
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



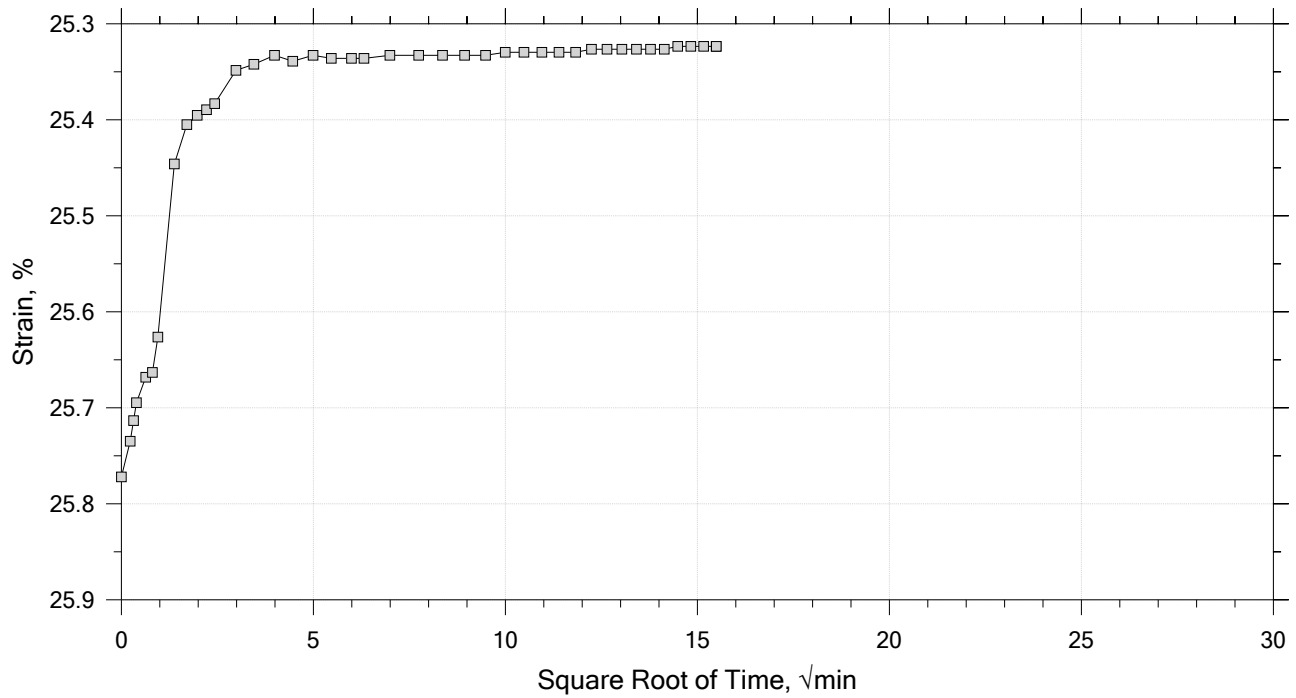
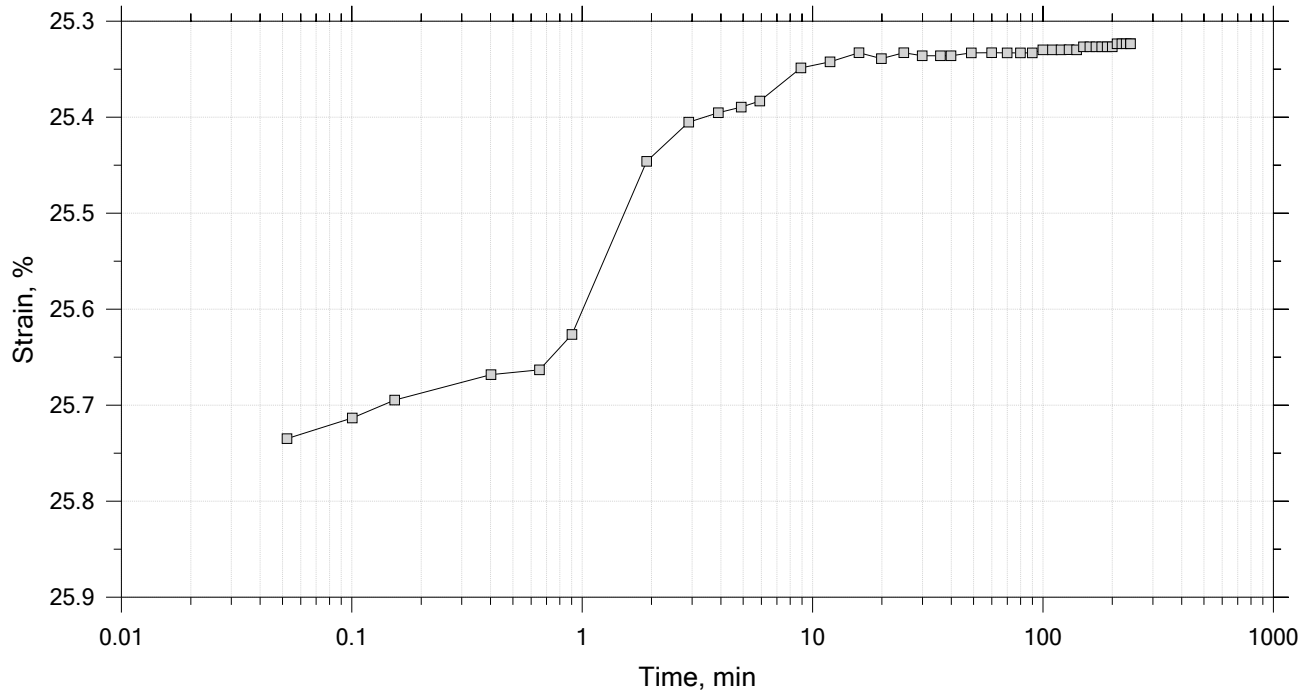
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



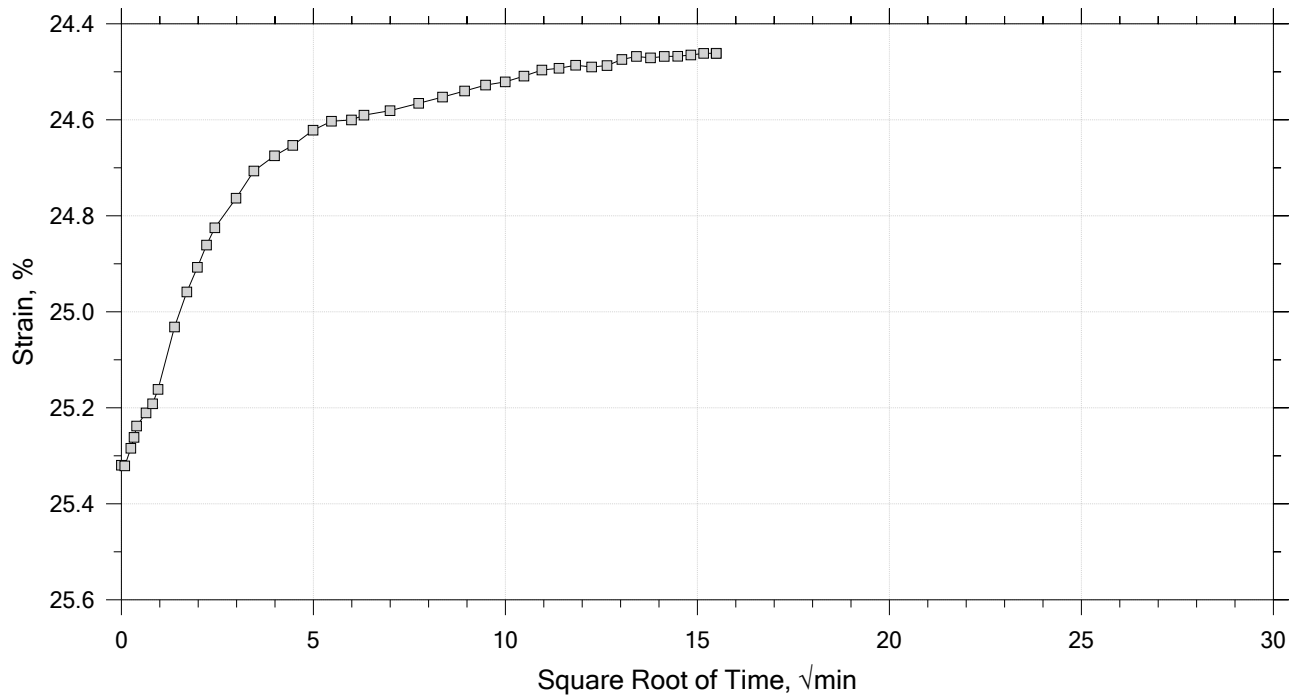
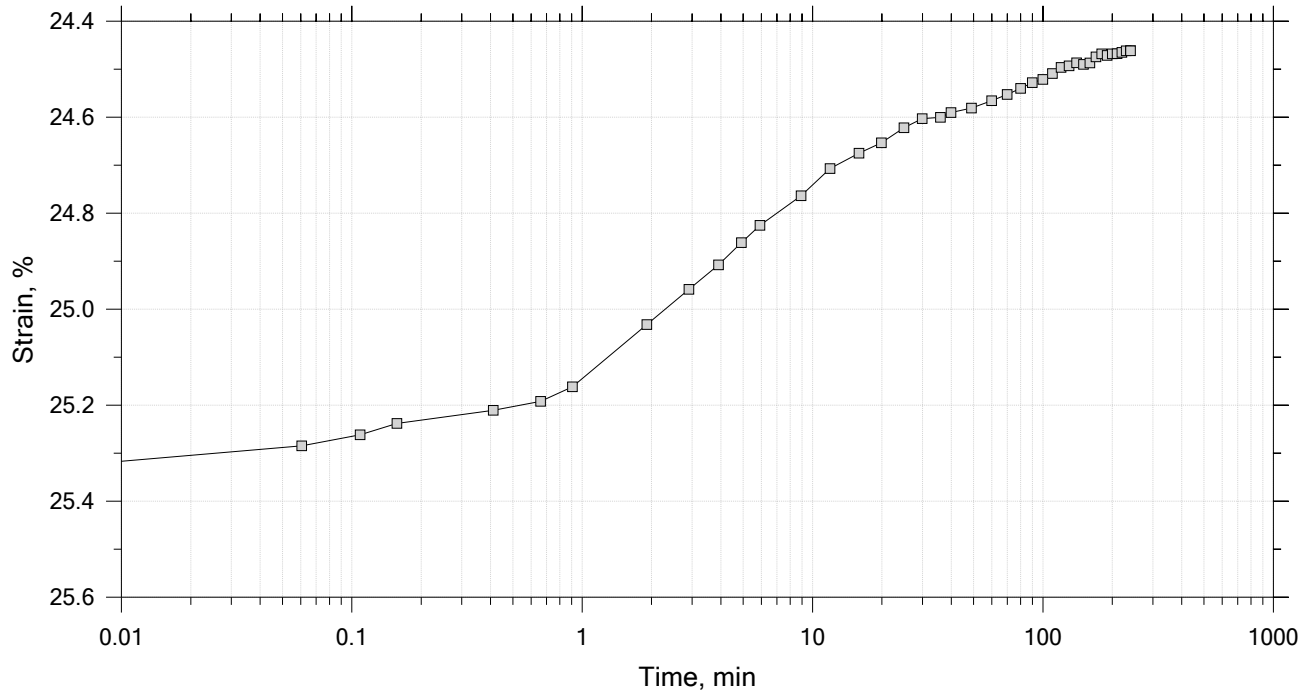
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	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



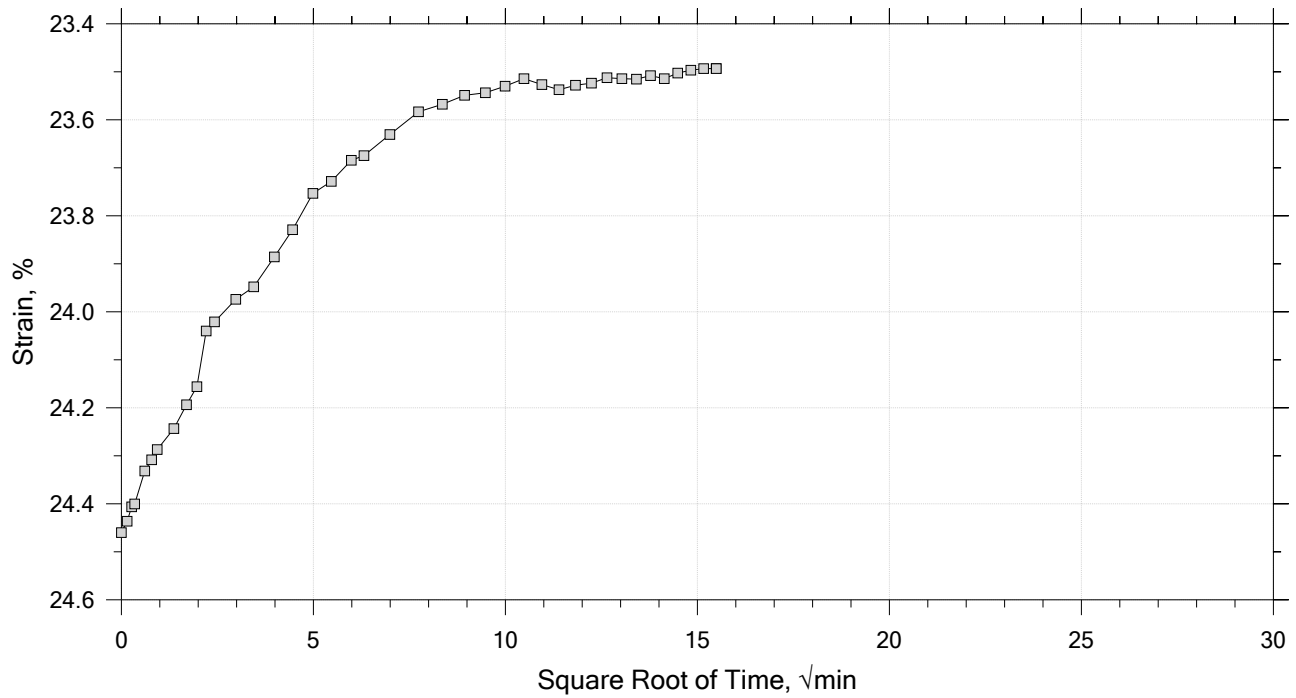
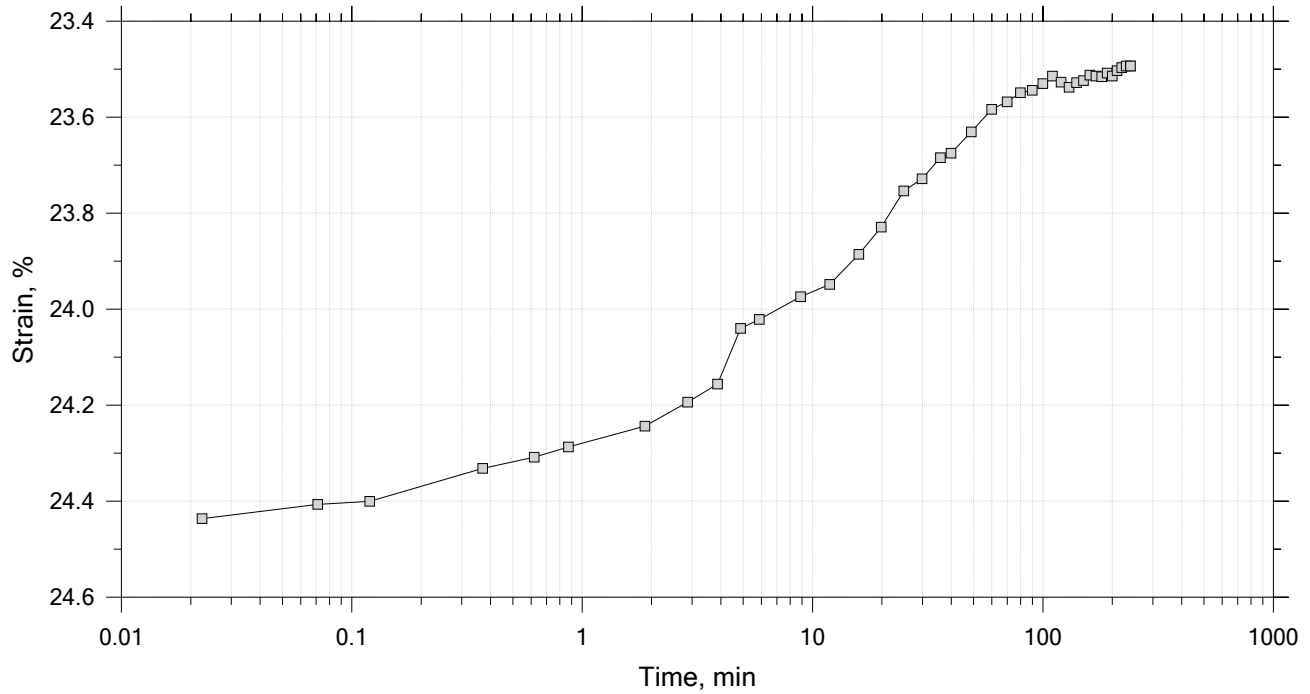
	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



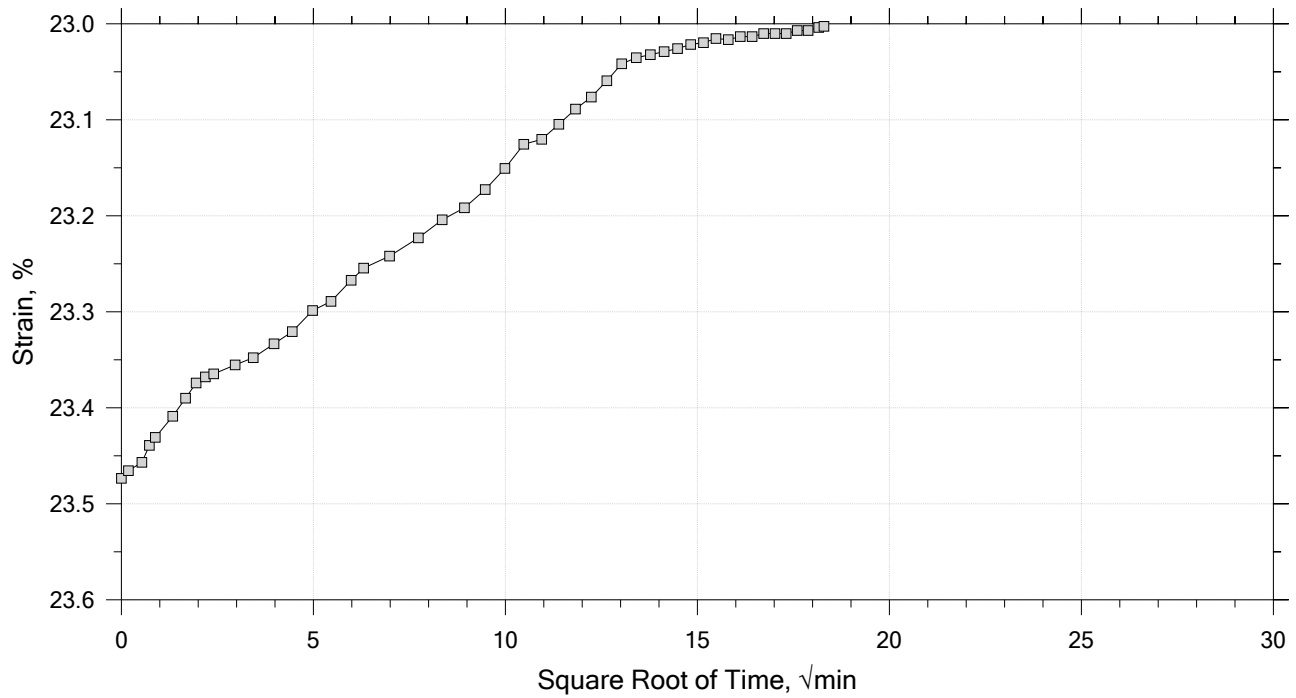
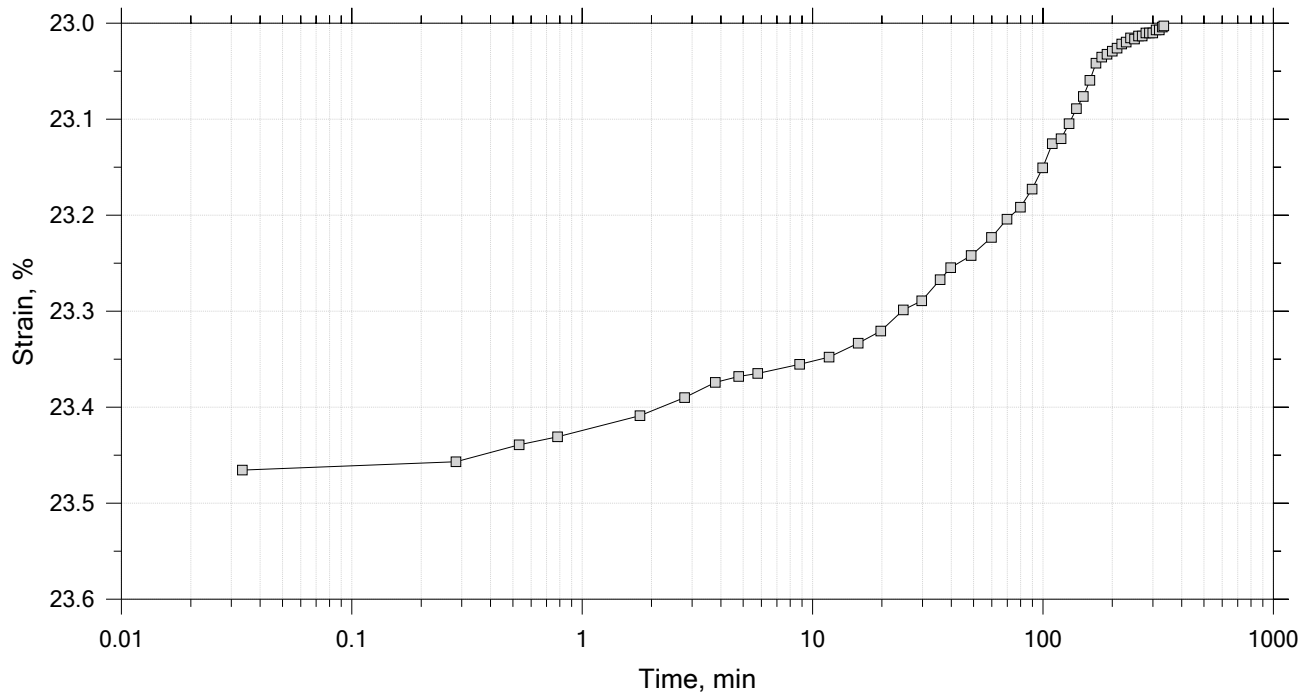
	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 38
Initial Height: 1.00 in	Initial Void Ratio: 1.06	Plastic Limit: 19
Final Height: 0.77 in	Final Void Ratio: 0.586	Plasticity Index: 19

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	A3068	RING		B-2371
Mass Container, gm	9.09	111.07	111.07	9.61
Mass Container + Wet Soil, gm	151.25	258.28	241.24	139.49
Mass Container + Dry Soil, gm	110.9	218.36	218.36	116.66
Mass Dry Soil, gm	101.81	107.29	107.29	107.05
Water Content, %	39.63	37.21	21.33	21.33
Void Ratio	---	1.06	0.59	---
Degree of Saturation, %	---	96.46	100.00	---
Dry Unit Weight, pcf	---	83.265	108.14	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTIII-A, Swell PPressure = 0.0629 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTI-A, Swell PPressure = 0.0629 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

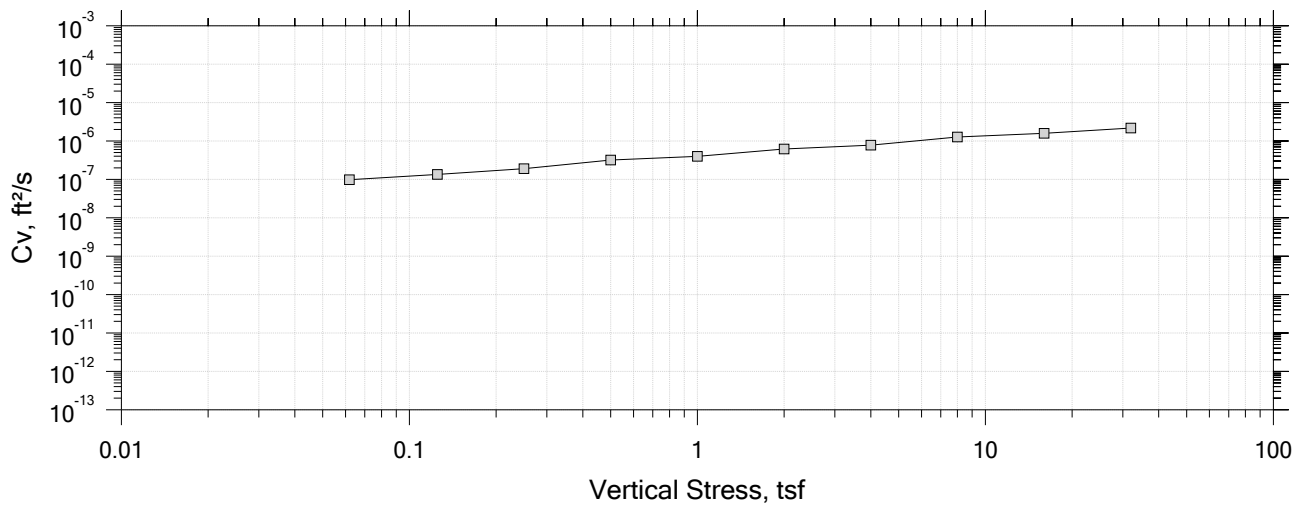
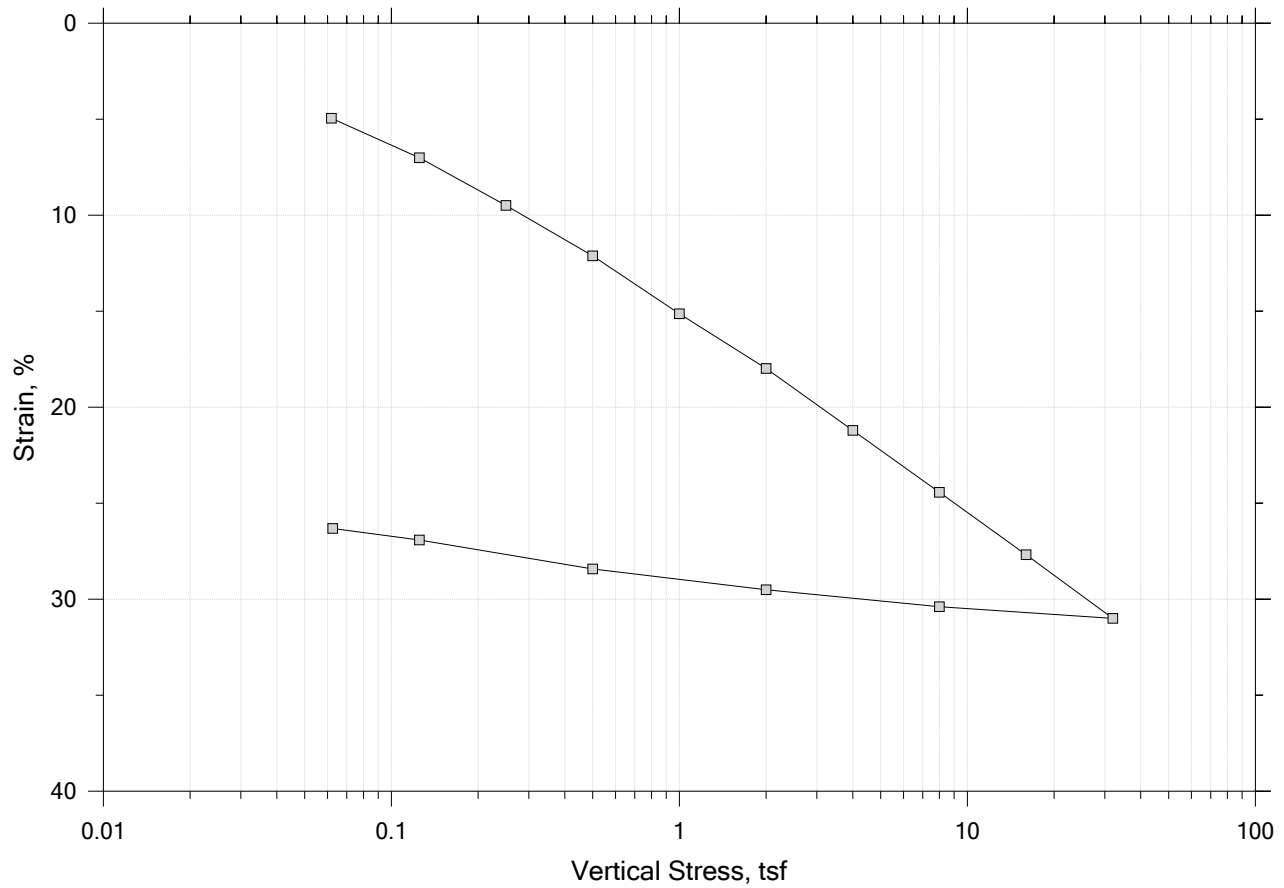
Square Root of Time Coefficients


[illegible]

	Project: RT-9/I-395 Connector	Location: Brewer and Eddington, Me	Project No.: GTX-308853
	Boring No.: HB-BE-136	Tested By: trm	Checked By: mcm
	Sample No.: 1U	Test Date: 7/12/19	Depth: 10-12 ft
	Test No.: IP-9	Sample Type: intact	Elevation: ---
	Description: Moist, gray clay		
	Remarks: System LTI-A, Swell PPressure = 0.0629 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

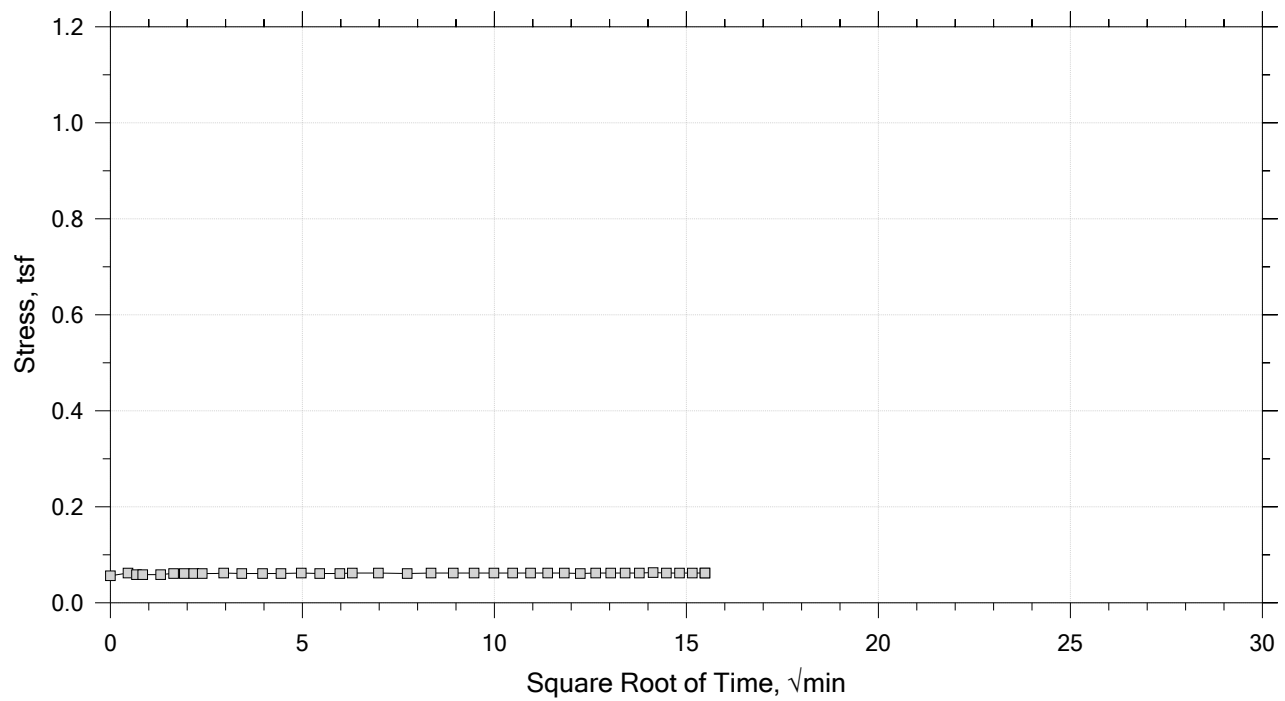
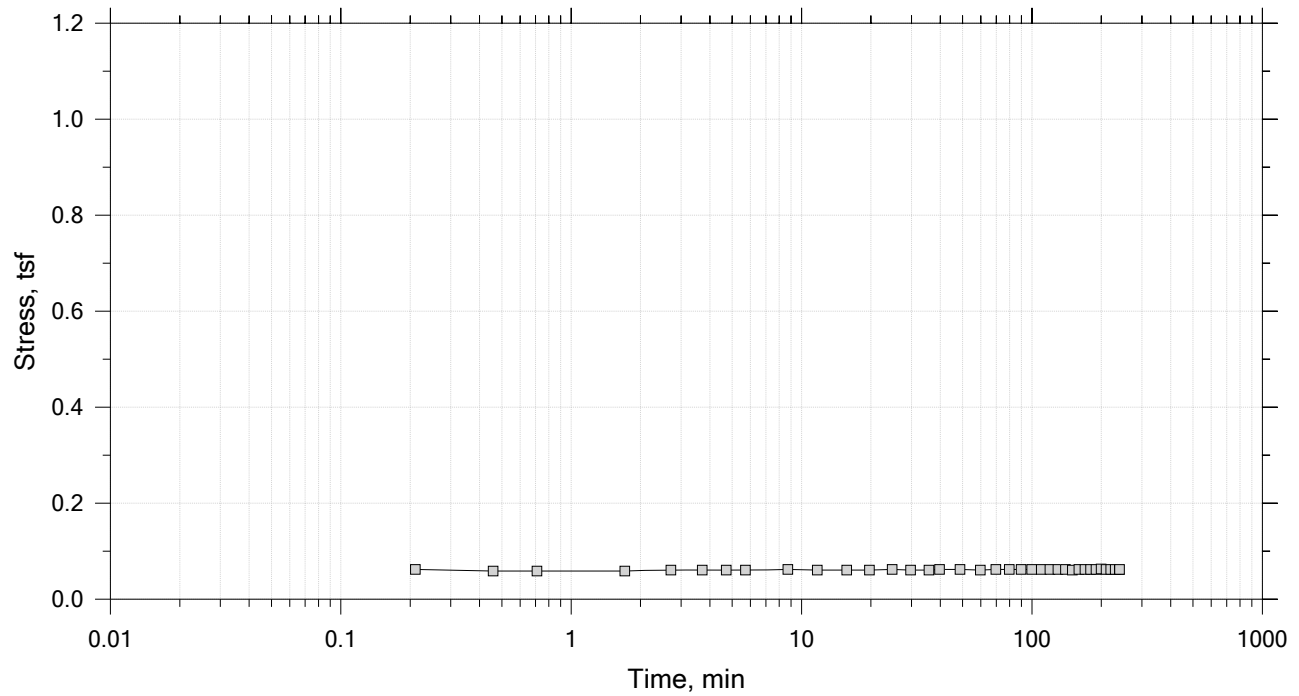
Summary Report




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0619 tsf



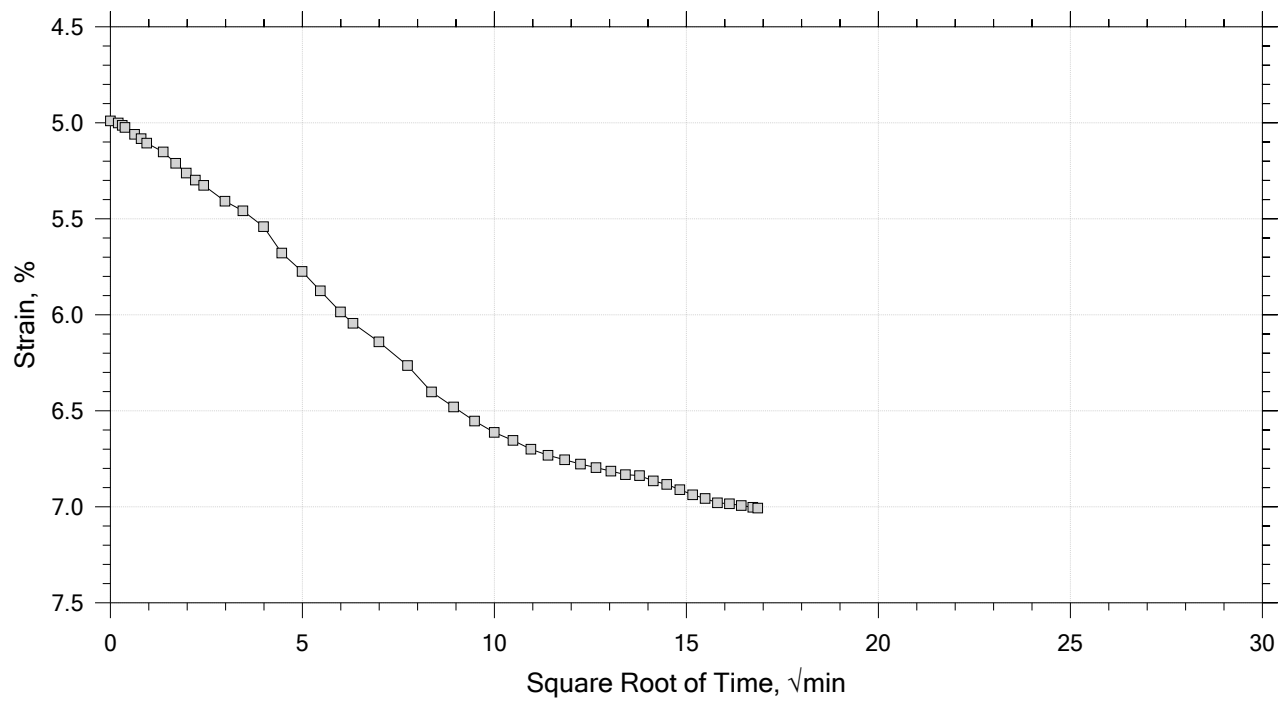
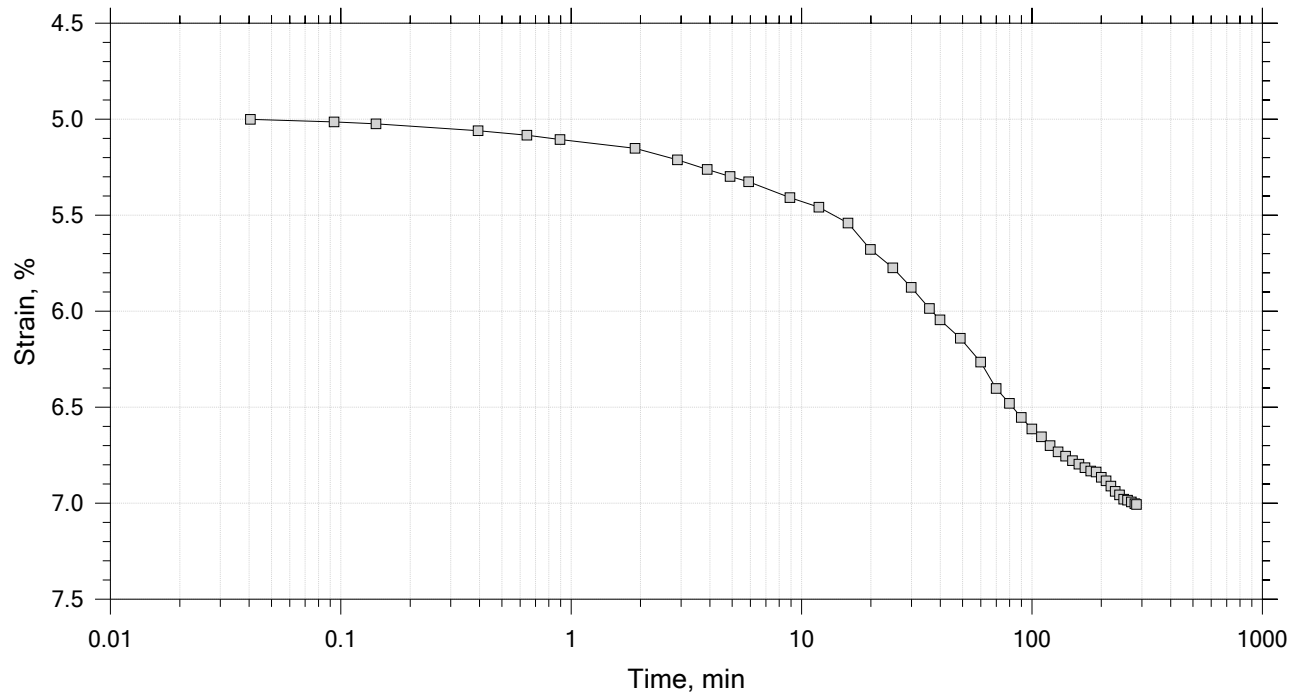
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	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



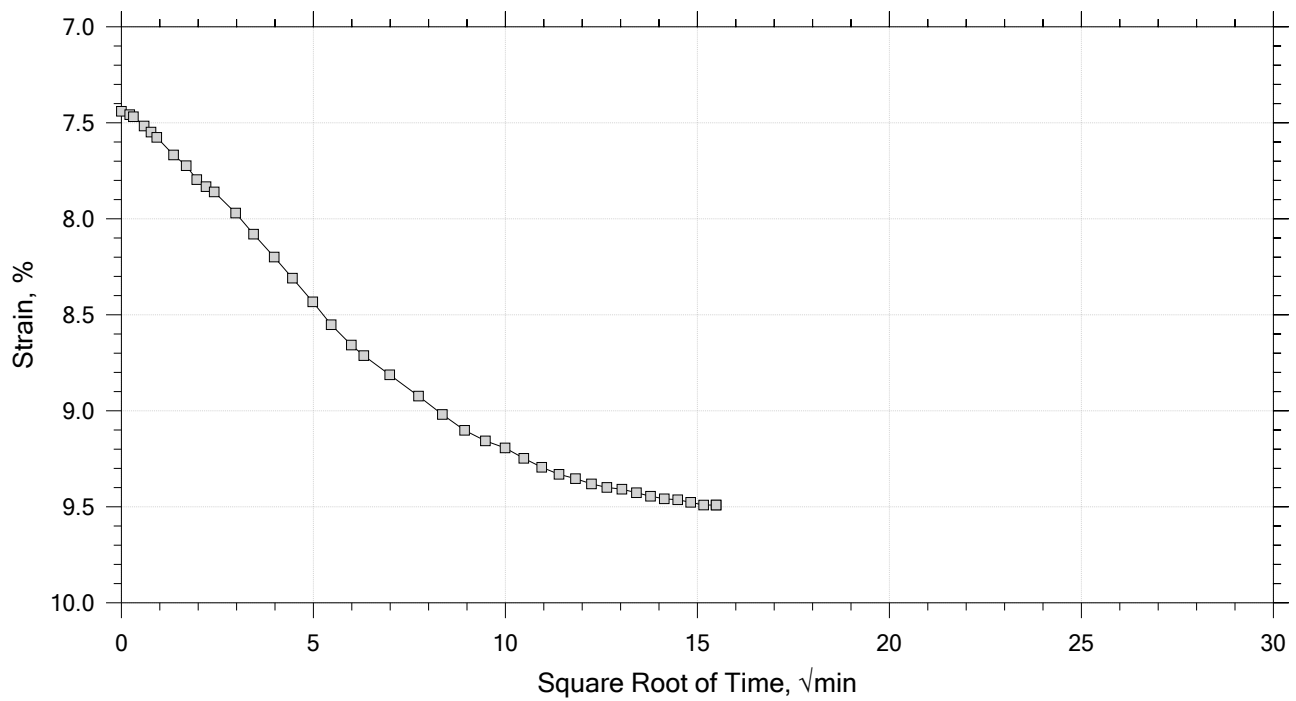
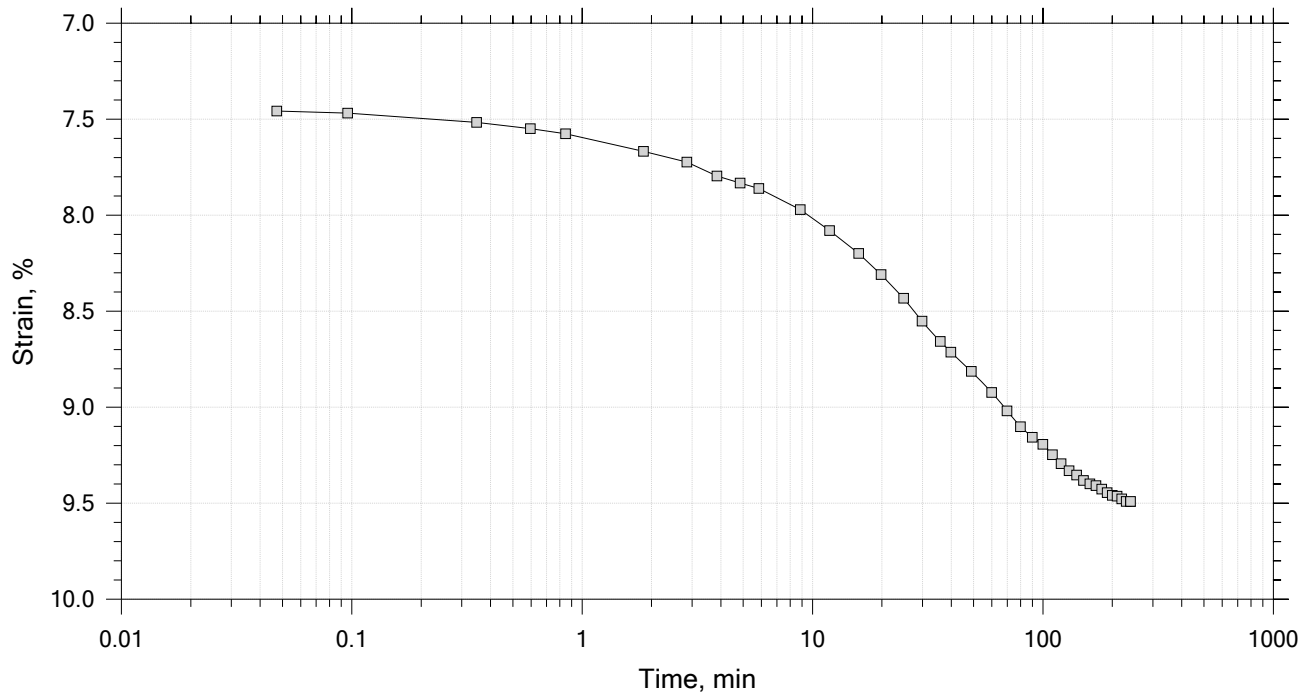
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



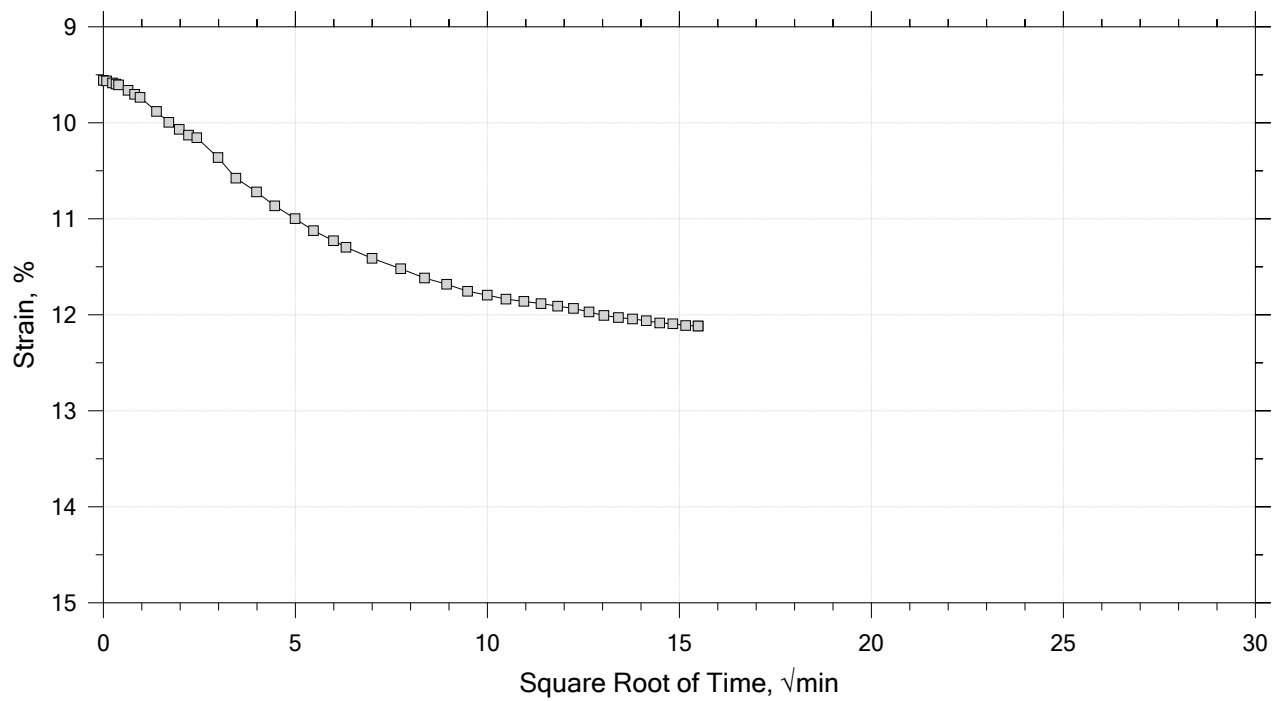
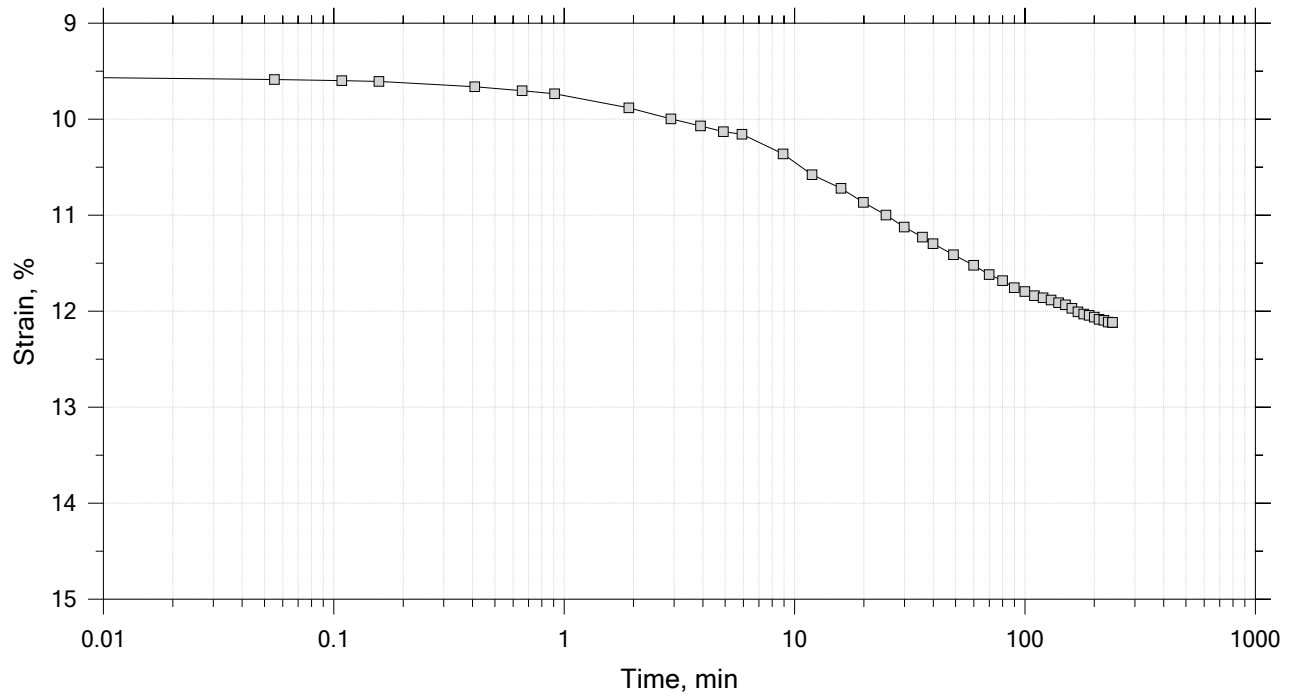
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



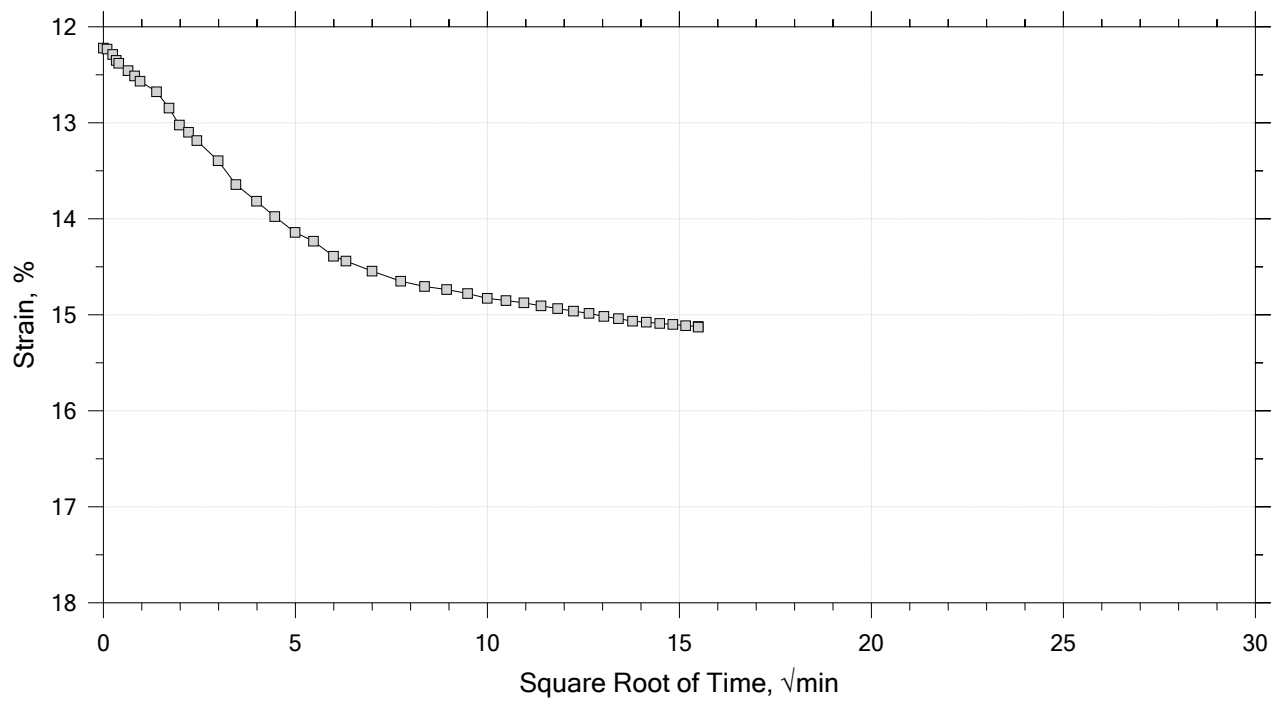
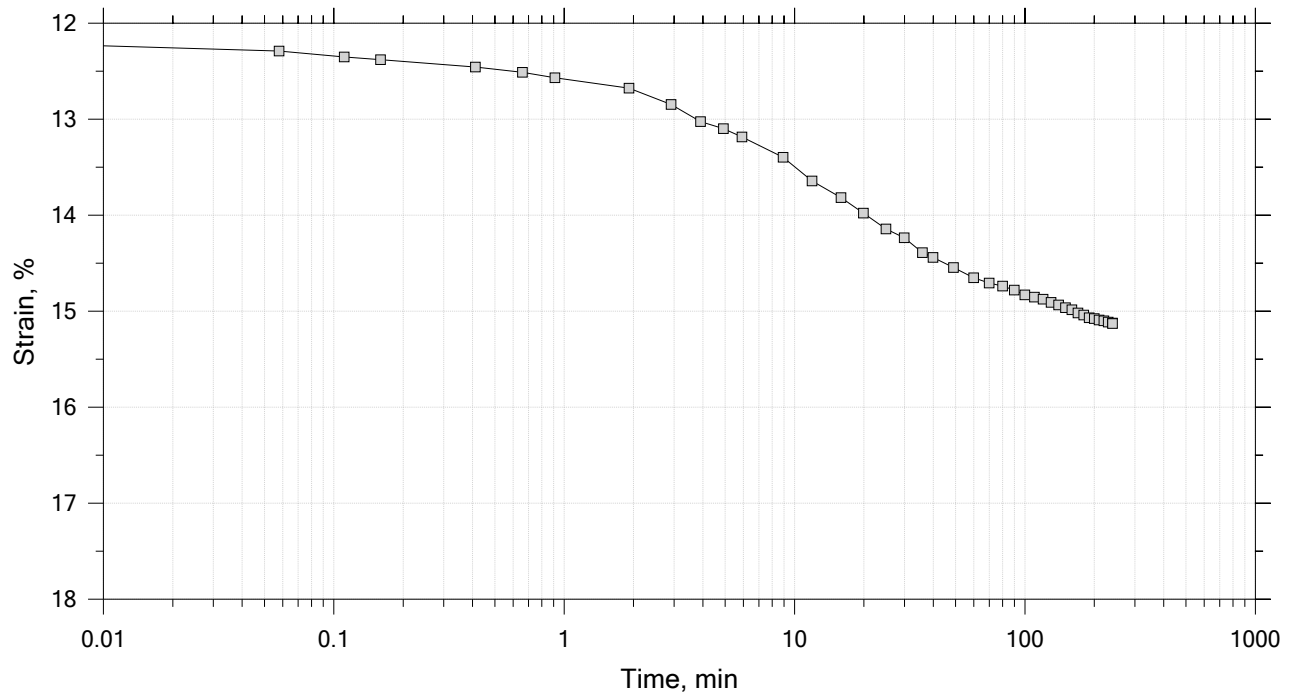
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	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



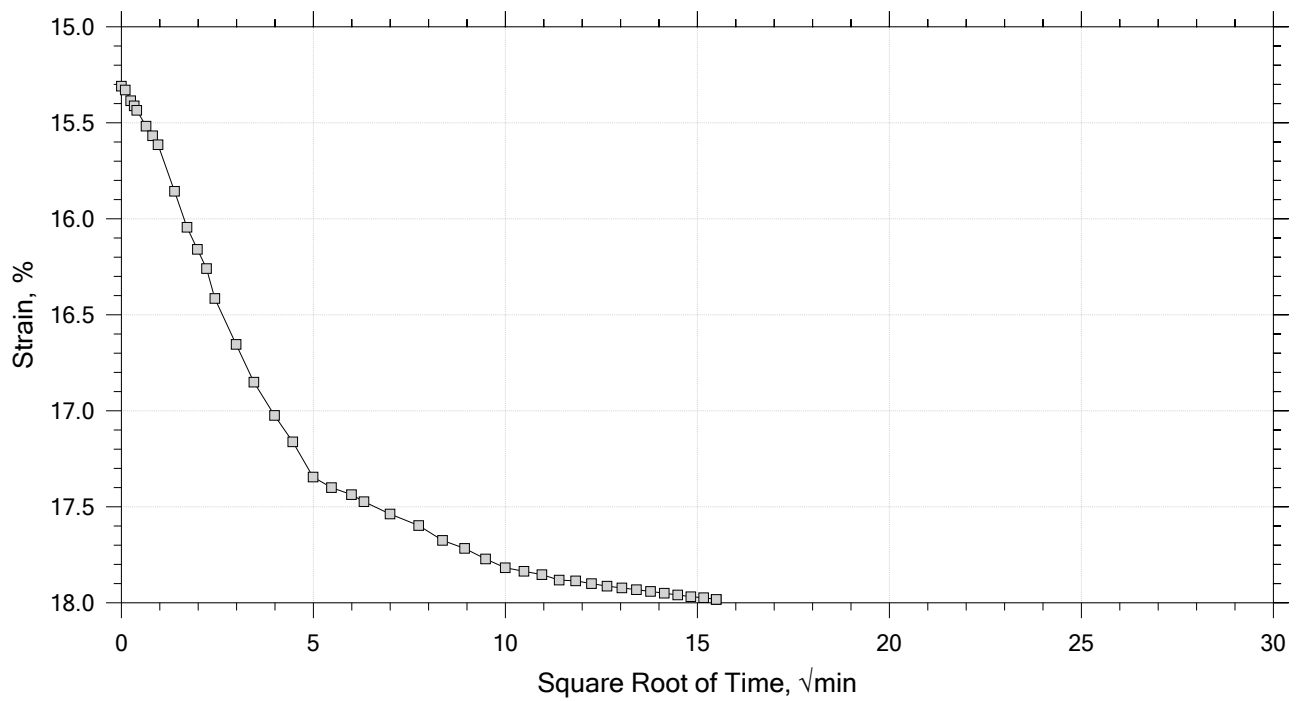
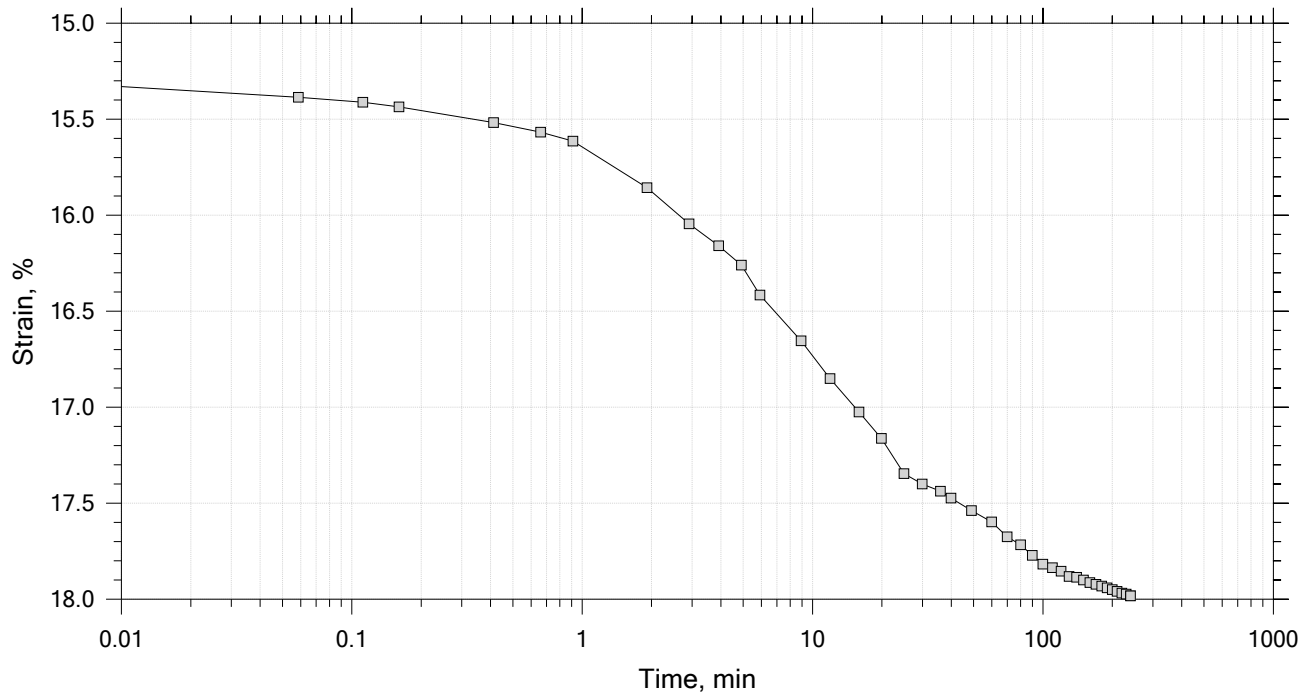
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



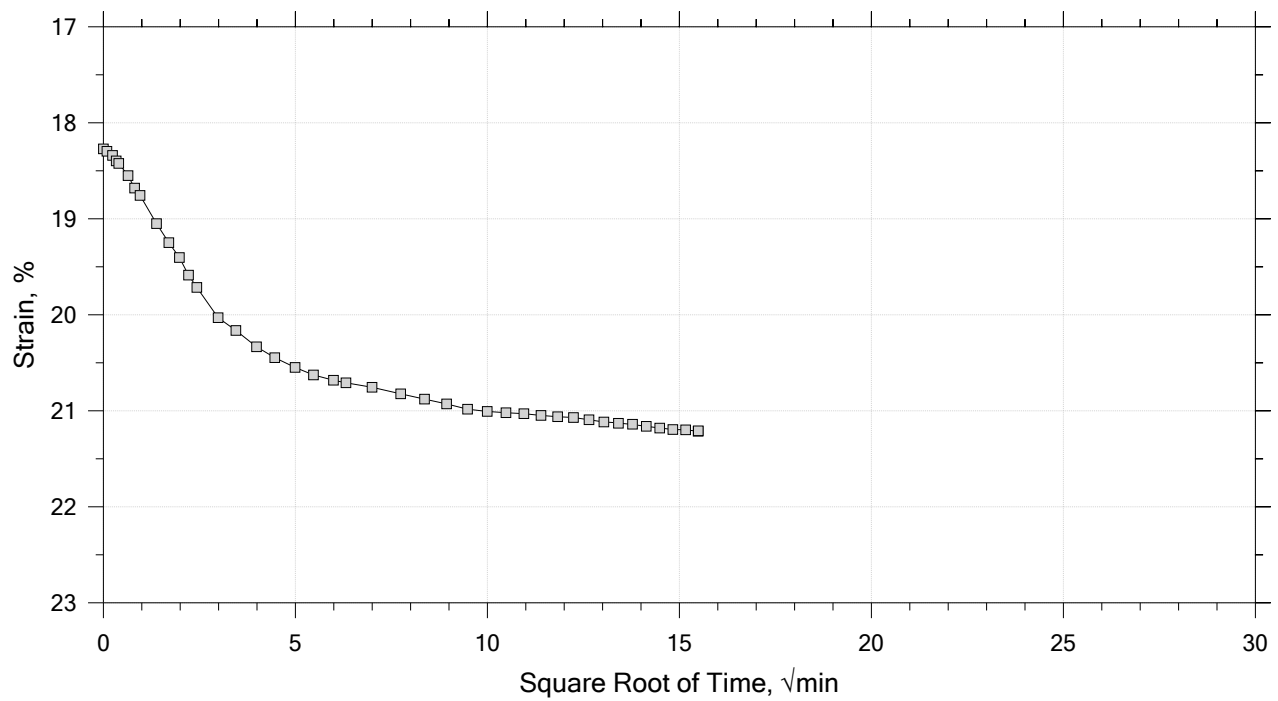
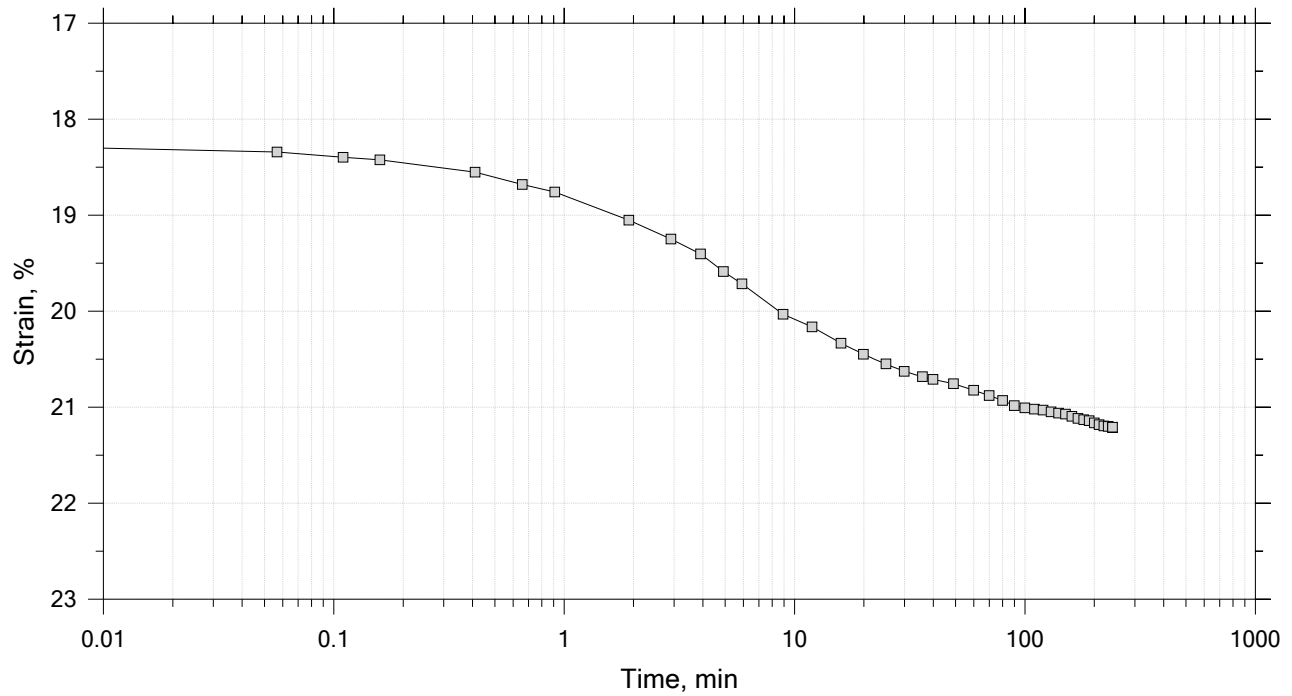
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



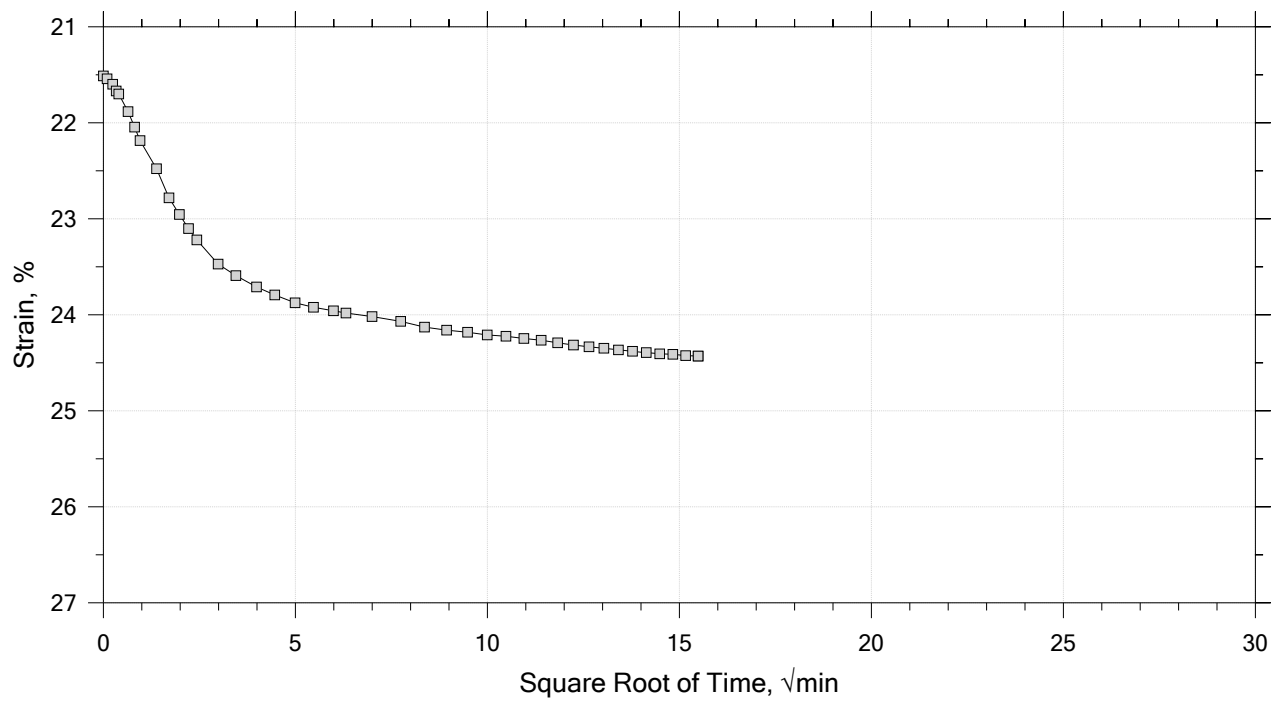
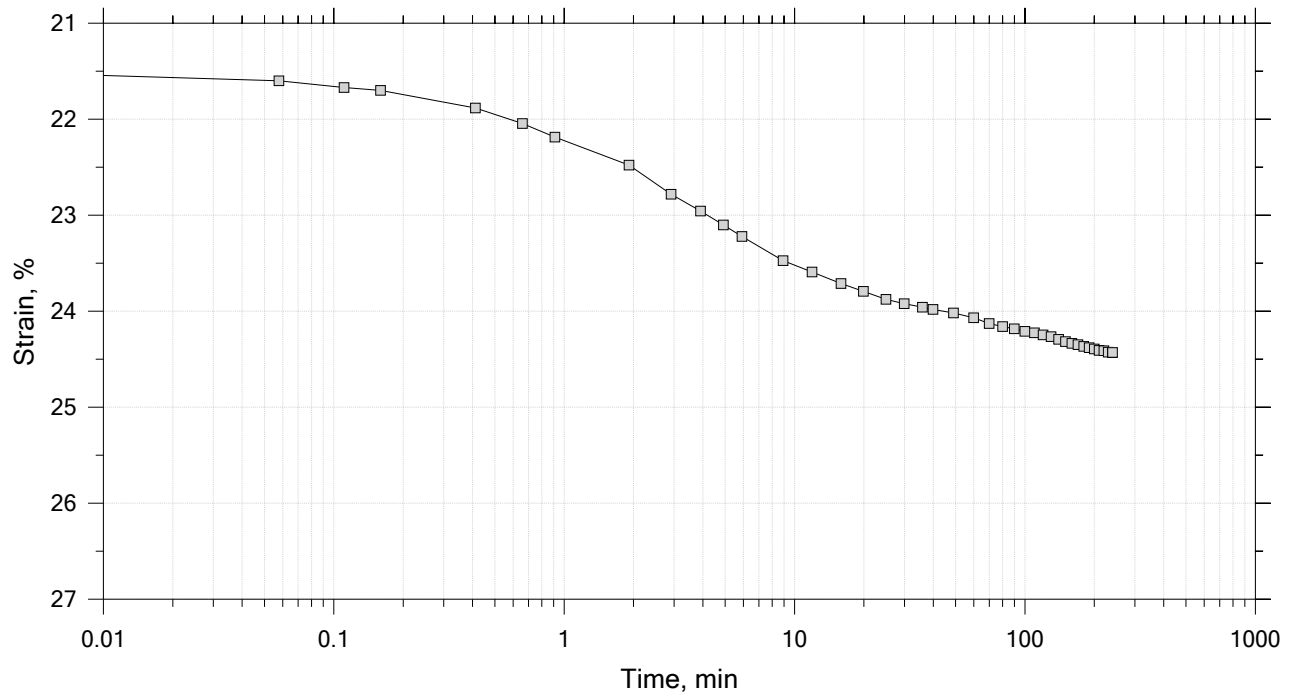
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	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



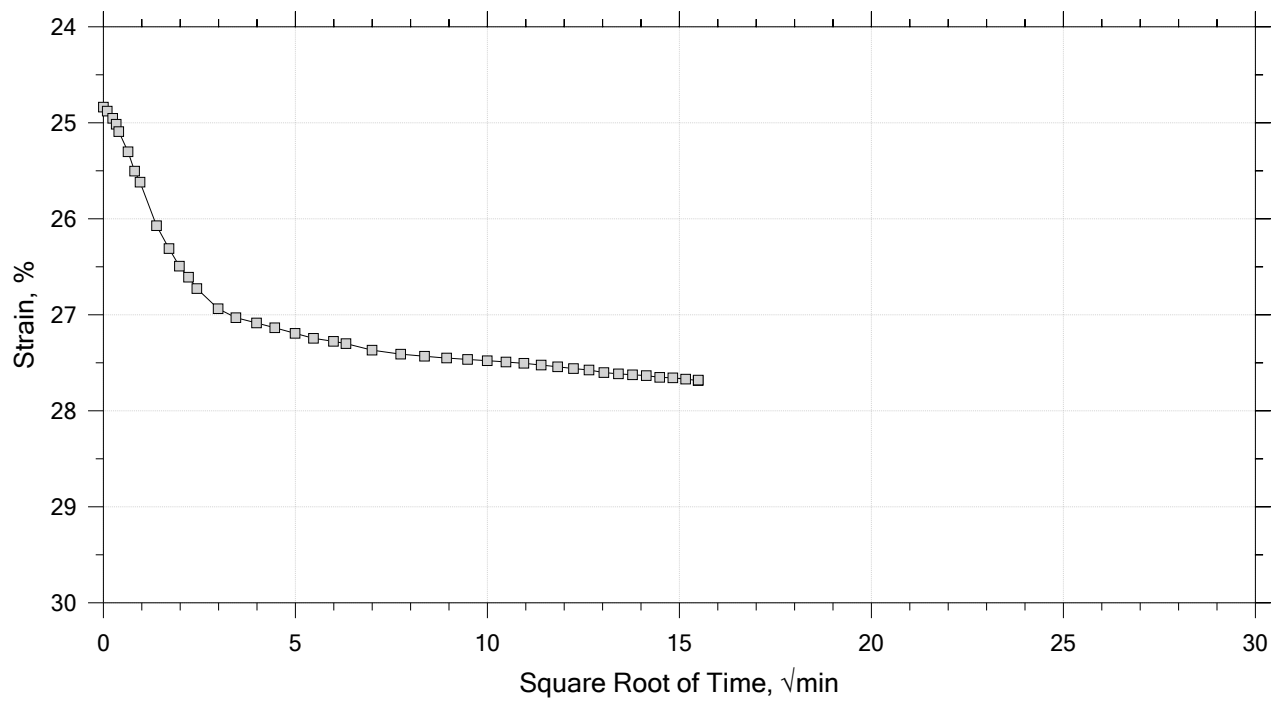
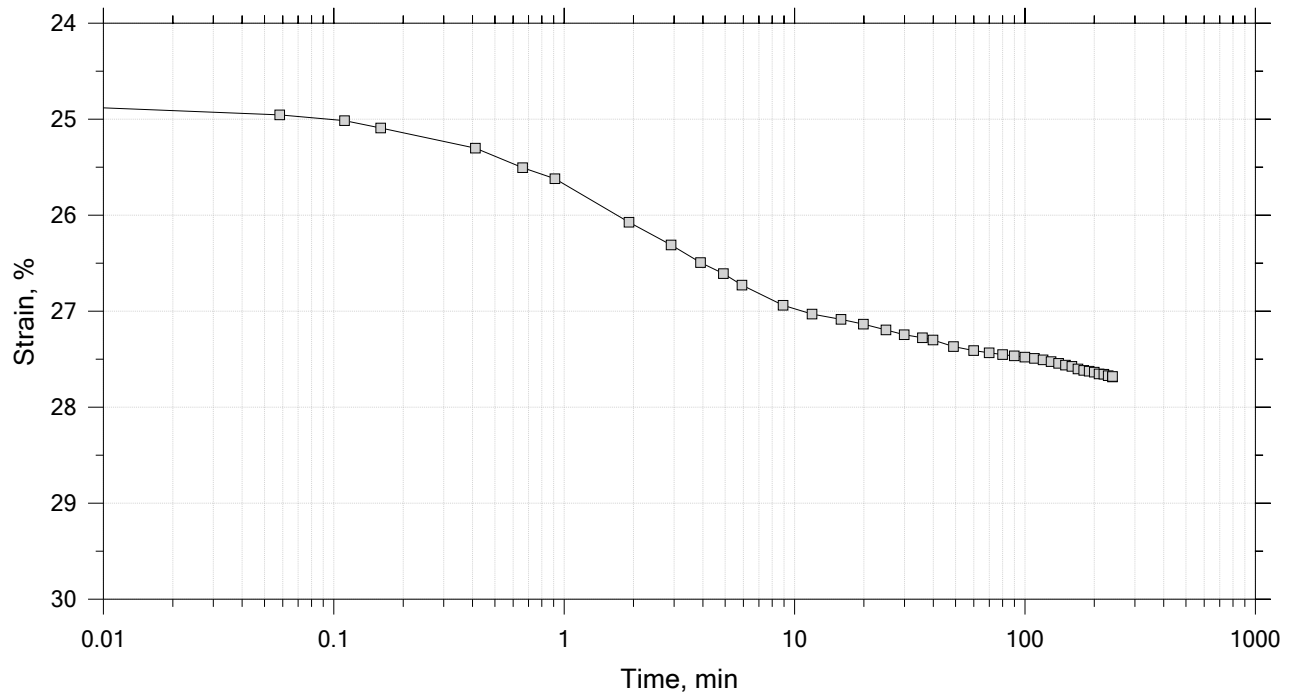
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	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



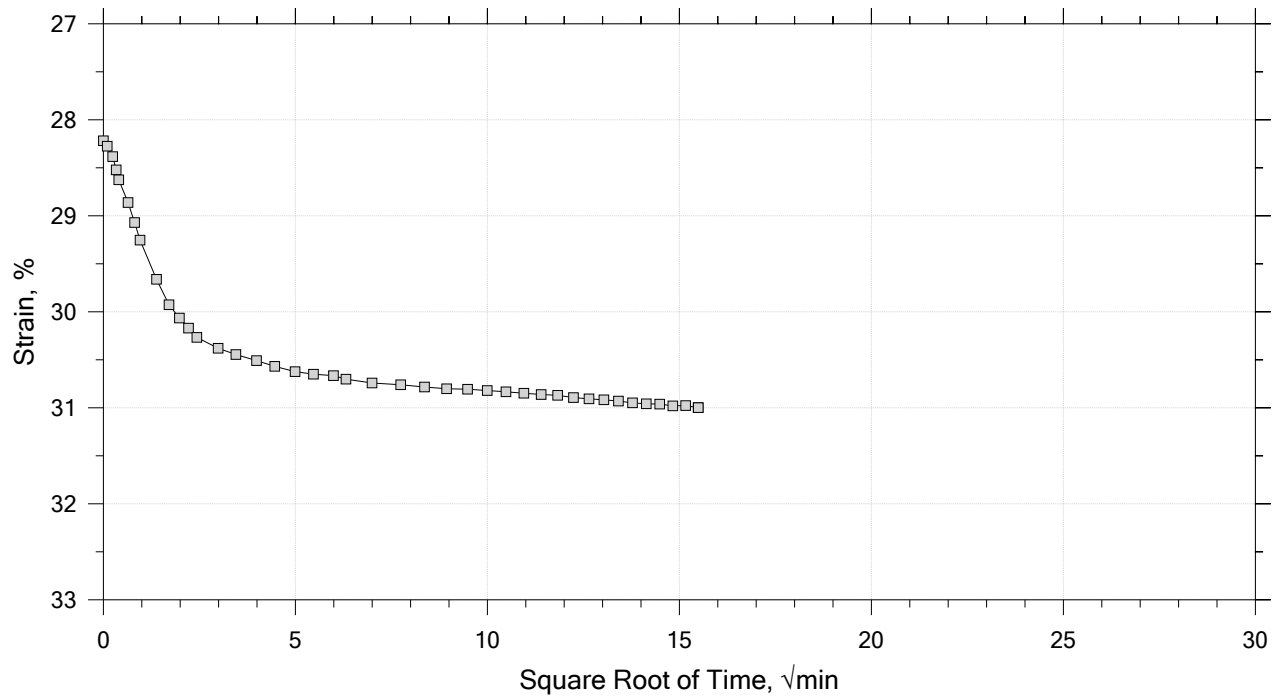
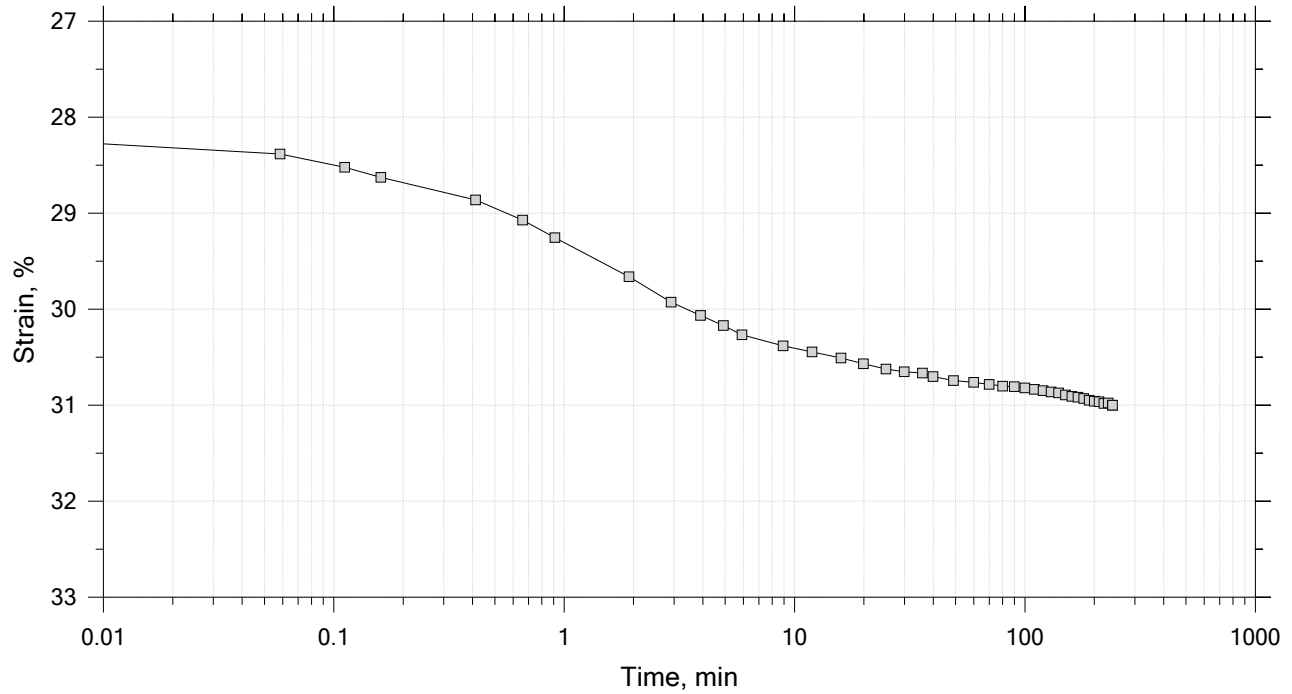
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	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



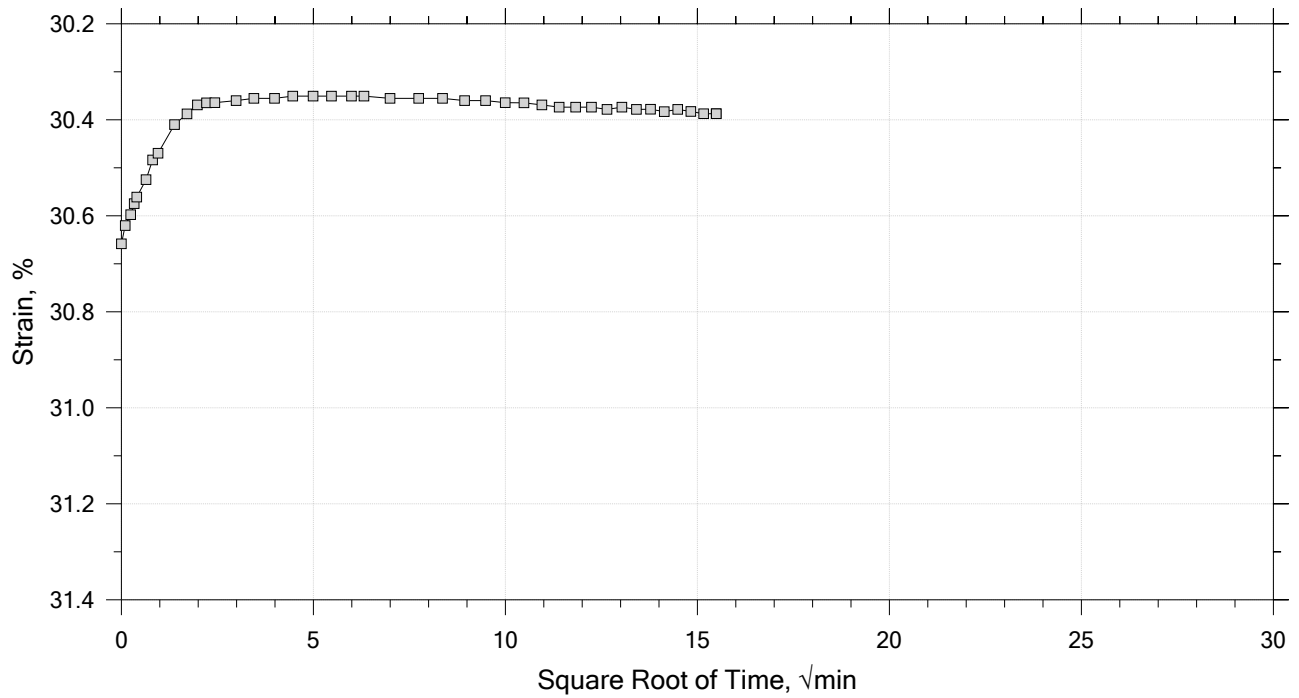
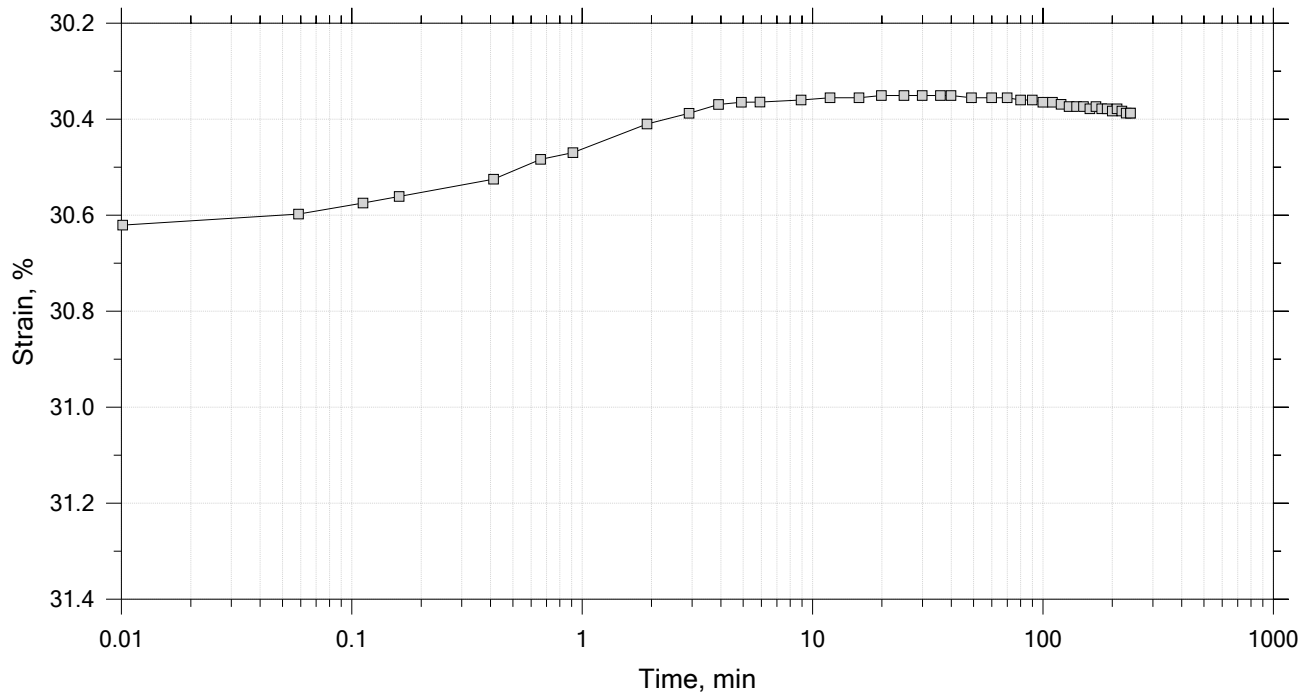
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



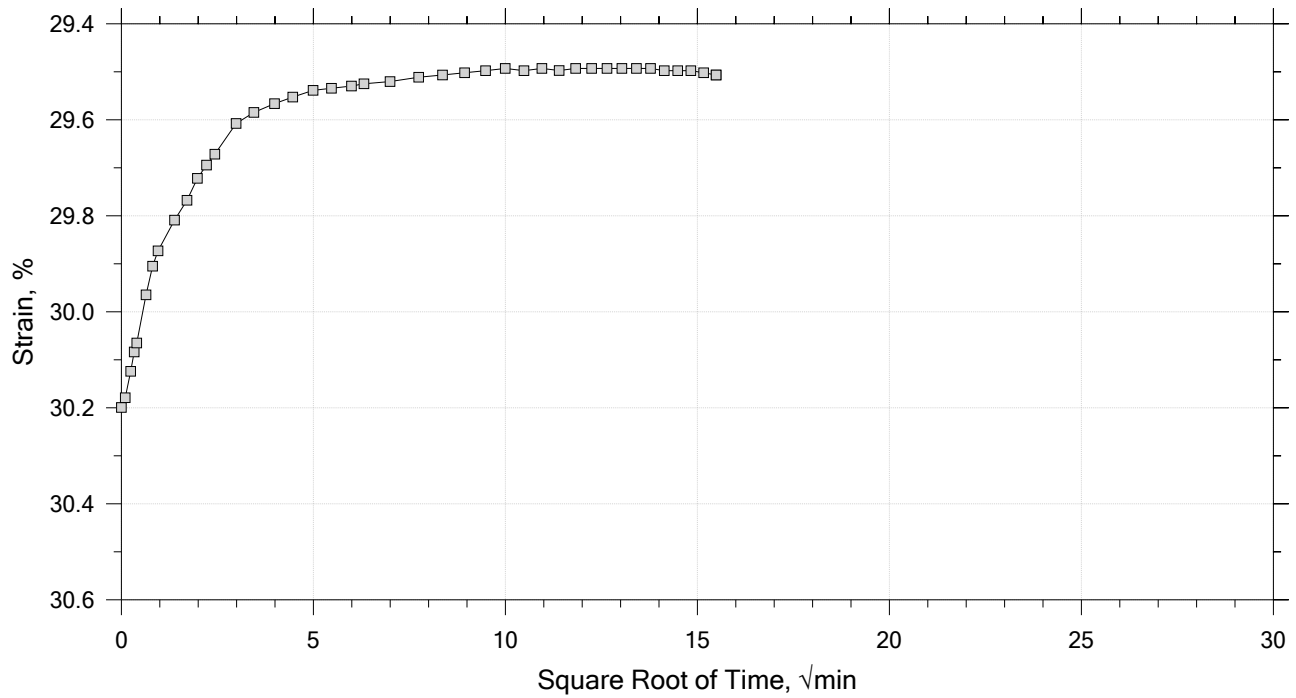
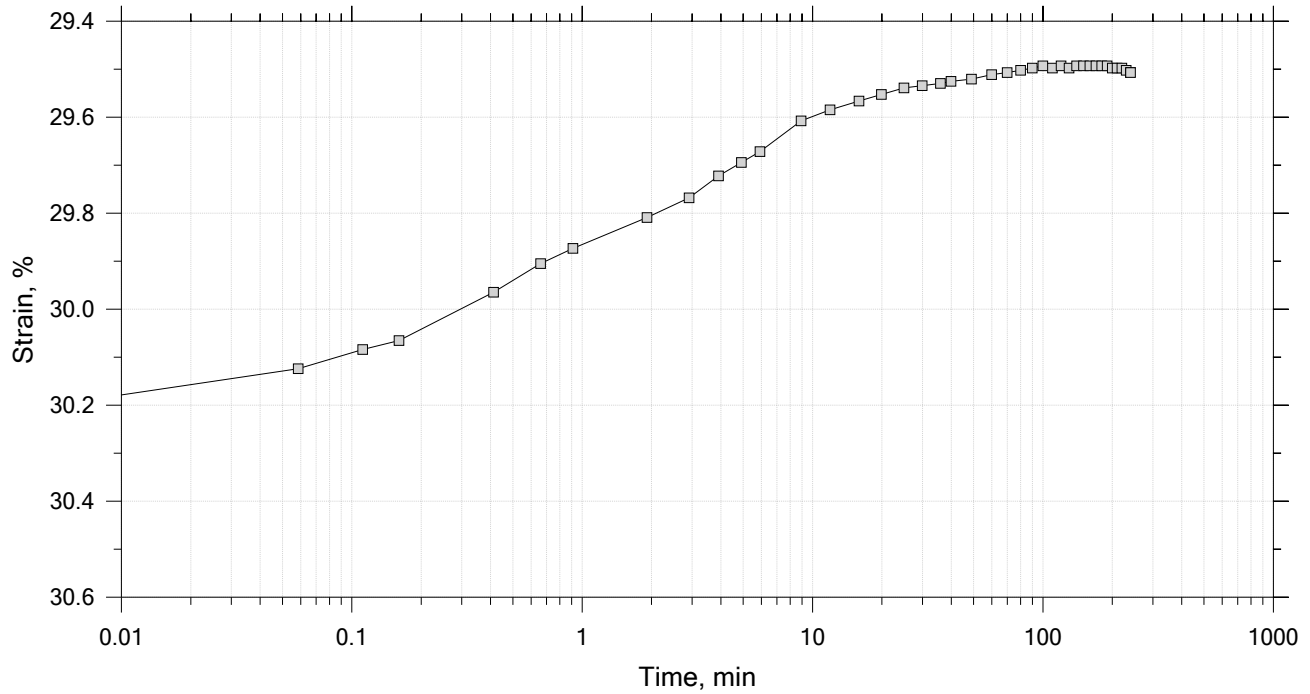
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 12 of 15

Constant Load Step

Stress: 2 tsf



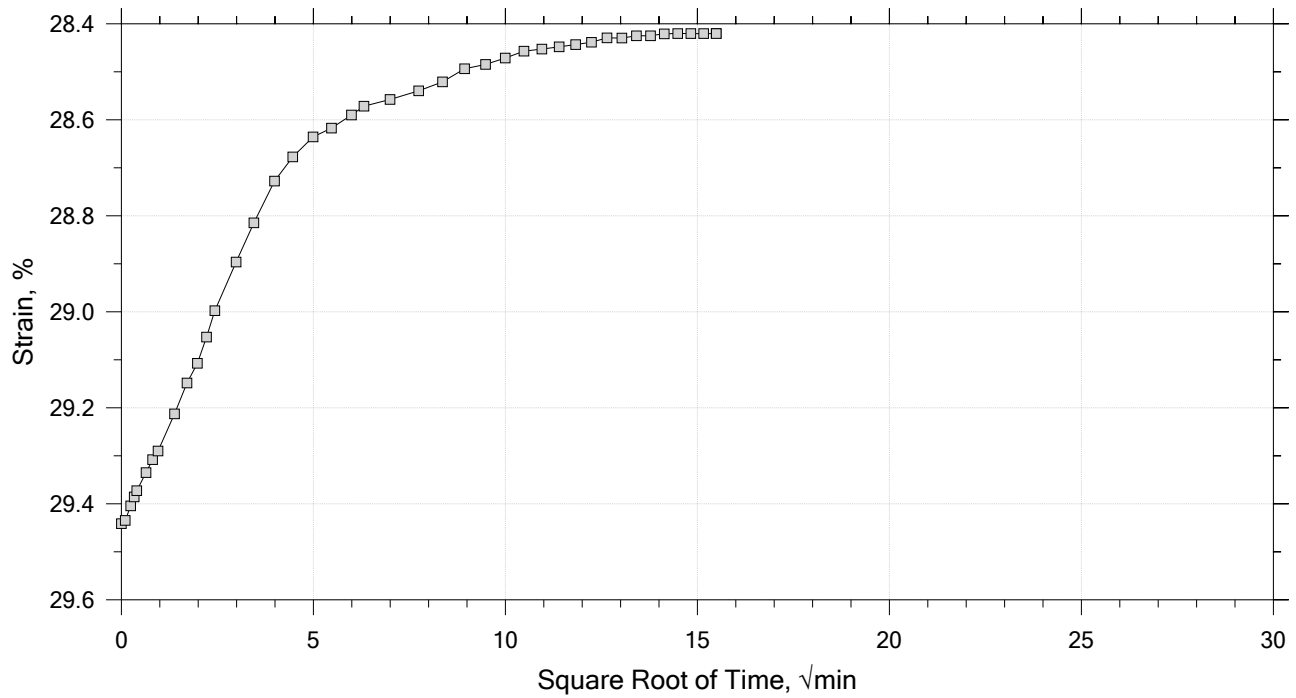
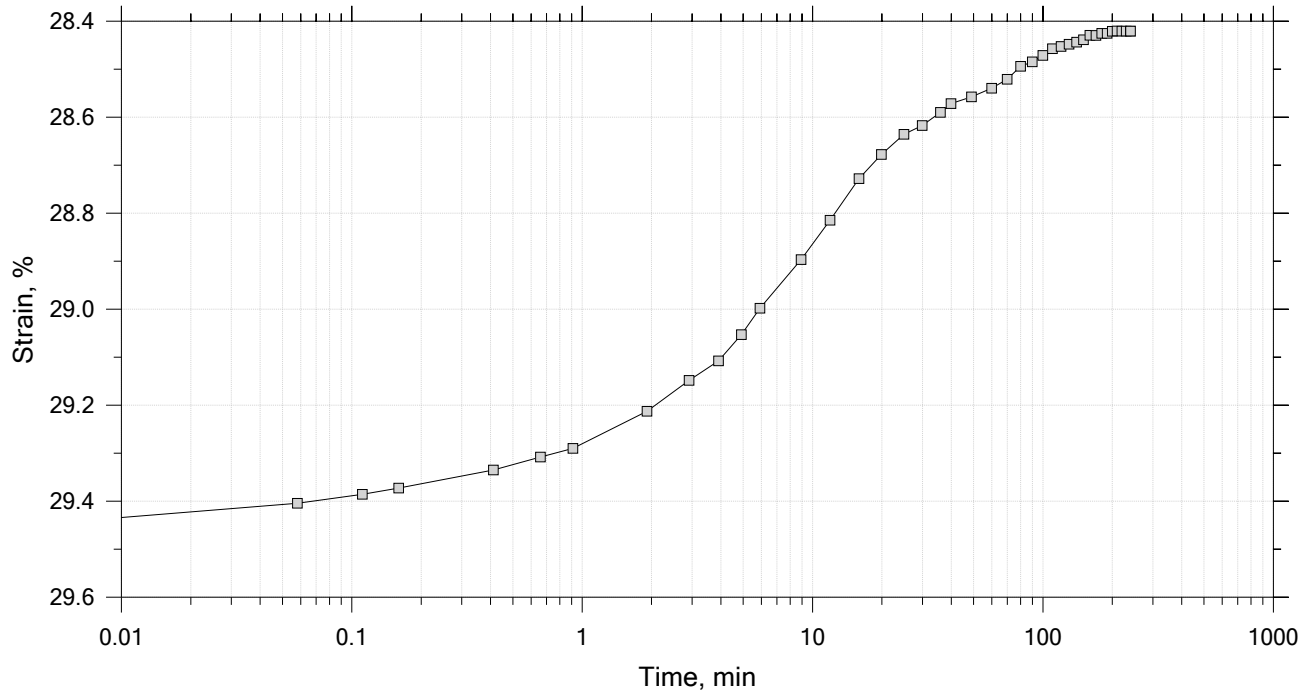
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 13 of 15

Constant Load Step

Stress: 0.5 tsf



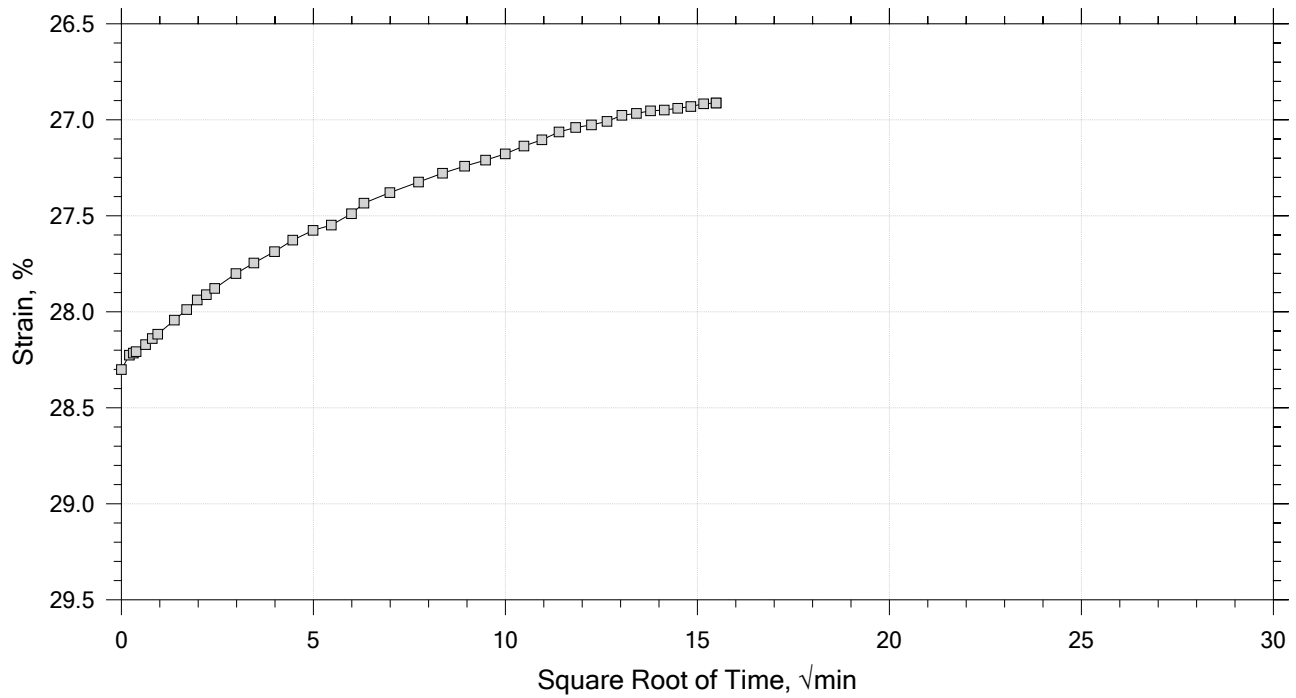
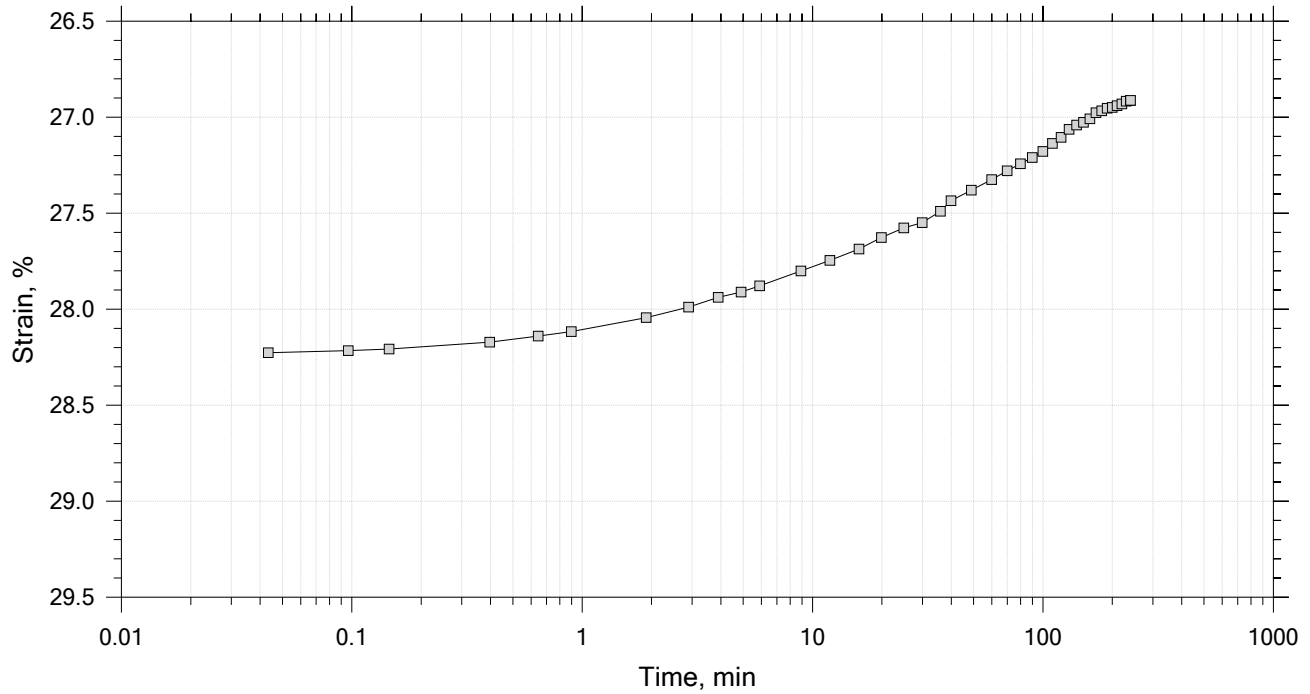
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 14 of 15

Constant Load Step

Stress: 0.125 tsf



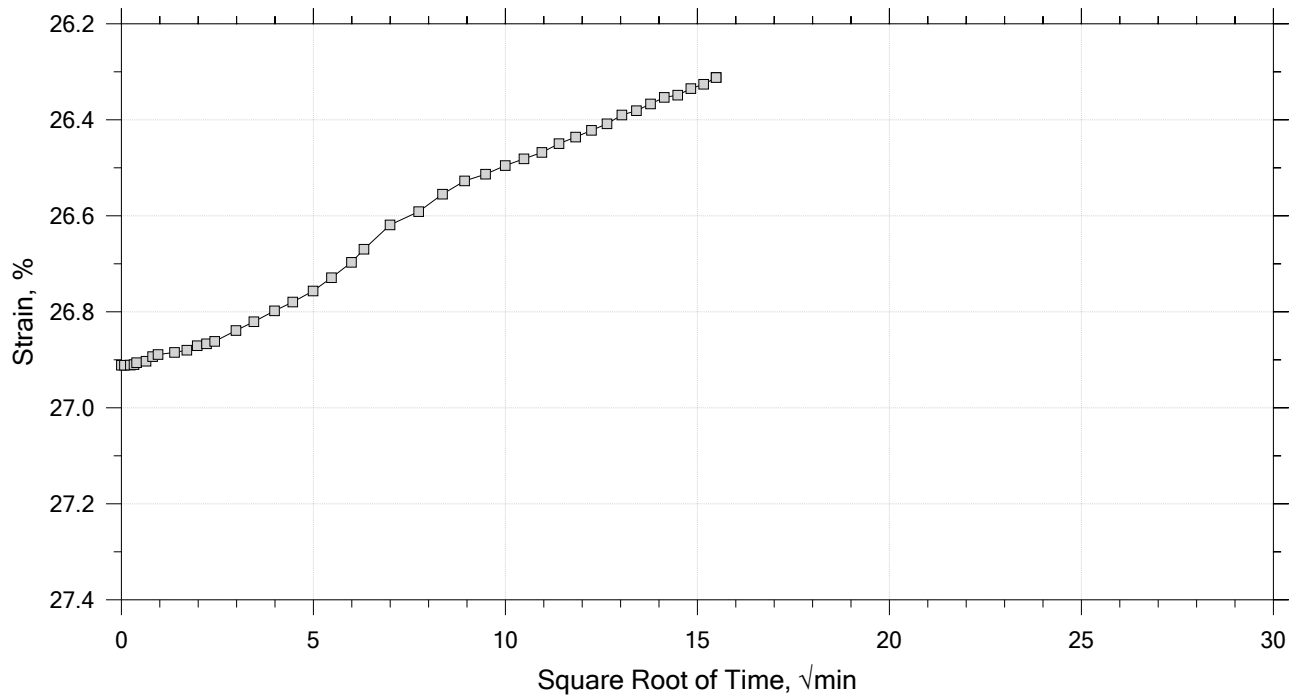
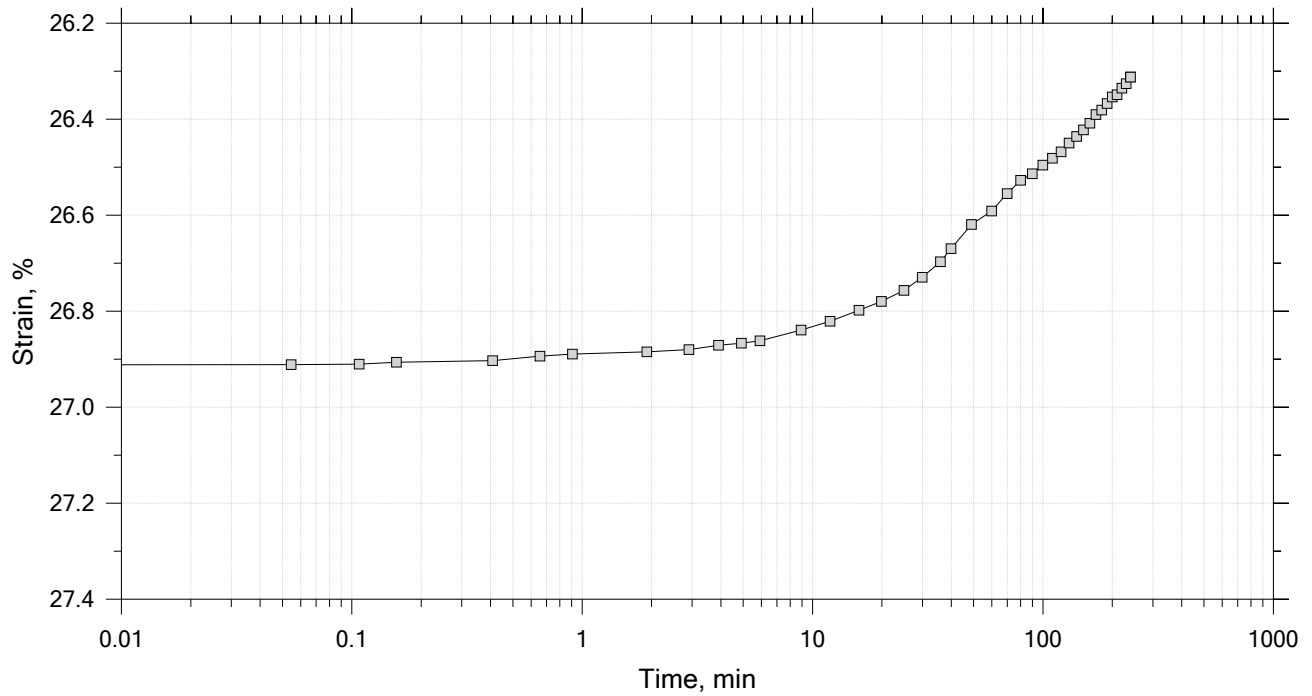
	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 15 of 15

Constant Load Step

Stress: 0.0625 tsf




	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Specimen Diameter: 2.50 in	Estimated Specific Gravity: 2.75	Liquid Limit: 32
Initial Height: 1.00 in	Initial Void Ratio: 1.15	Plastic Limit: 19
Final Height: 0.74 in	Final Void Ratio: 0.581	Plasticity Index: 13

	Before Test Trimmings	Before Test Specimen	After Test Specimen	After Test Trimmings
Container ID	D-1043	RING		B-2267
Mass Container, gm	8.28	109.61	109.61	8.37
Mass Container + Wet Soil, gm	150.08	255.17	234.63	133.16
Mass Container + Dry Soil, gm	108.47	212.84	212.84	111.41
Mass Dry Soil, gm	100.19	103.23	103.23	103.04
Water Content, %	41.53	41.01	21.11	21.11
Void Ratio	---	1.15	0.58	---
Degree of Saturation, %	---	98.54	100.00	---
Dry Unit Weight, pcf	---	80.115	108.72	---


Note: Specific Gravity and Void Ratios are calculated assuming the degree of saturation equals 100% at the end of the test. Therefore, values may not represent actual values for the specimen.

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		

One-Dimensional Consolidation by ASTM D2435 - Method B

Log of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

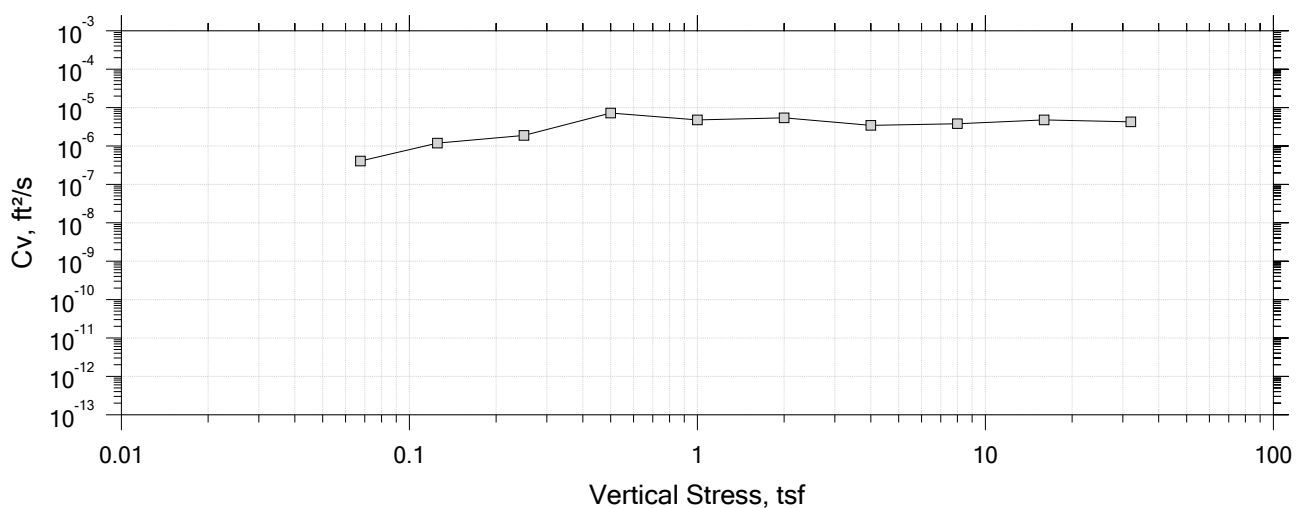
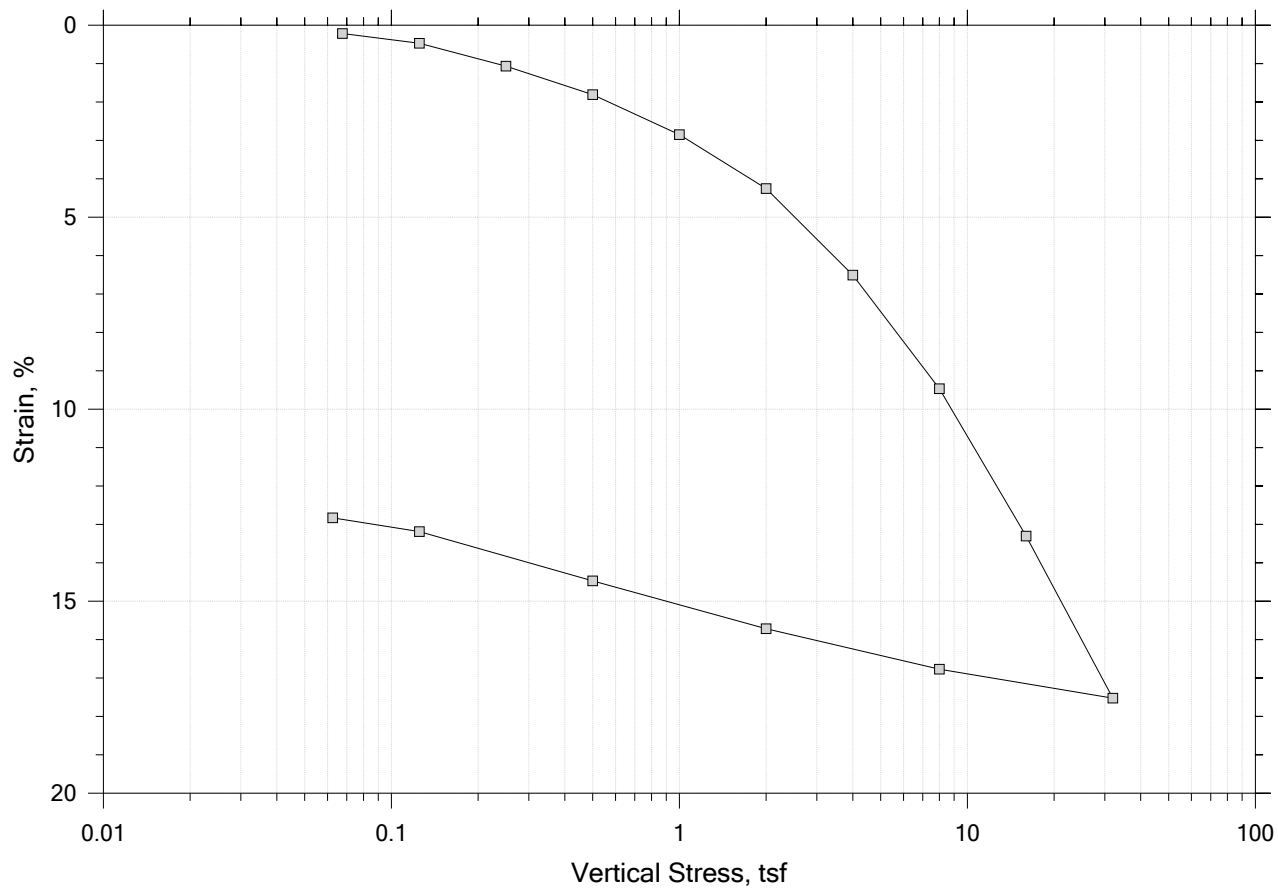
Square Root of Time Coefficients


[illegible]

	Project: Rt 9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-137	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/22/19	Depth: 20-22 ft
	Test No.: IP-18	Sample Type: intact	Elevation: ---
	Description: Wet, dark gray clay		
	Remarks: System S, Swell Pressure = 0.0619 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

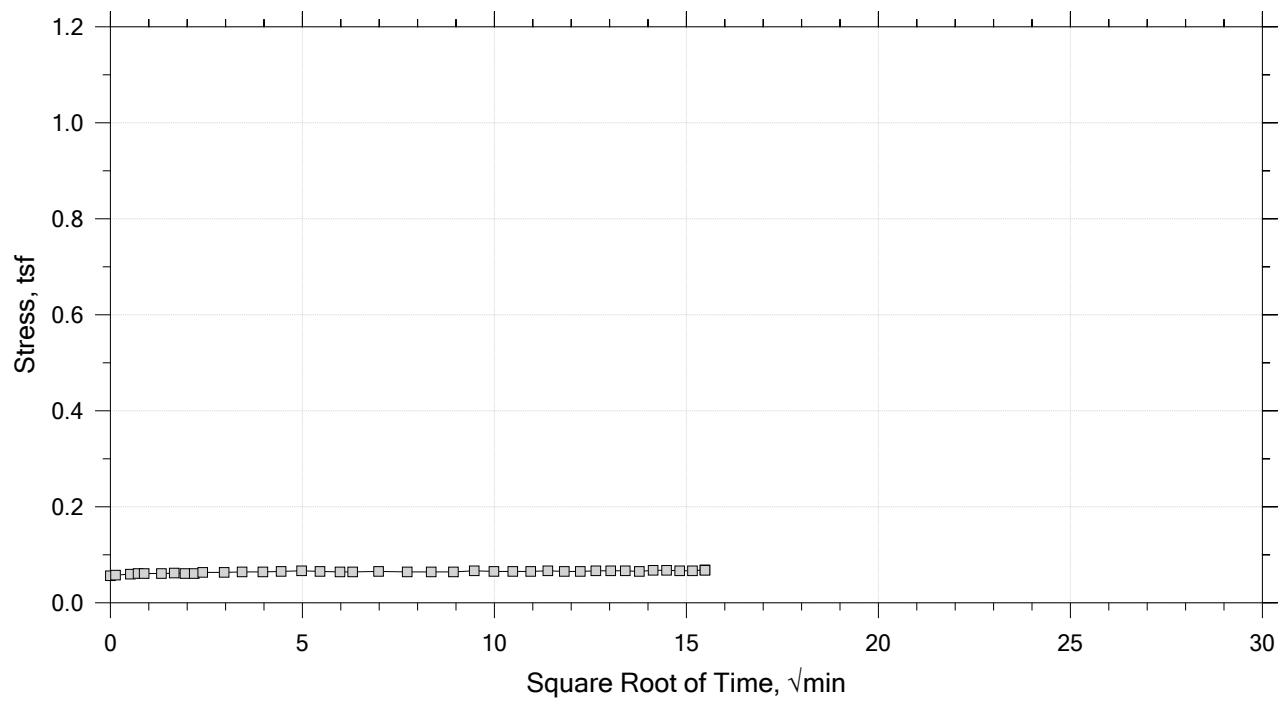
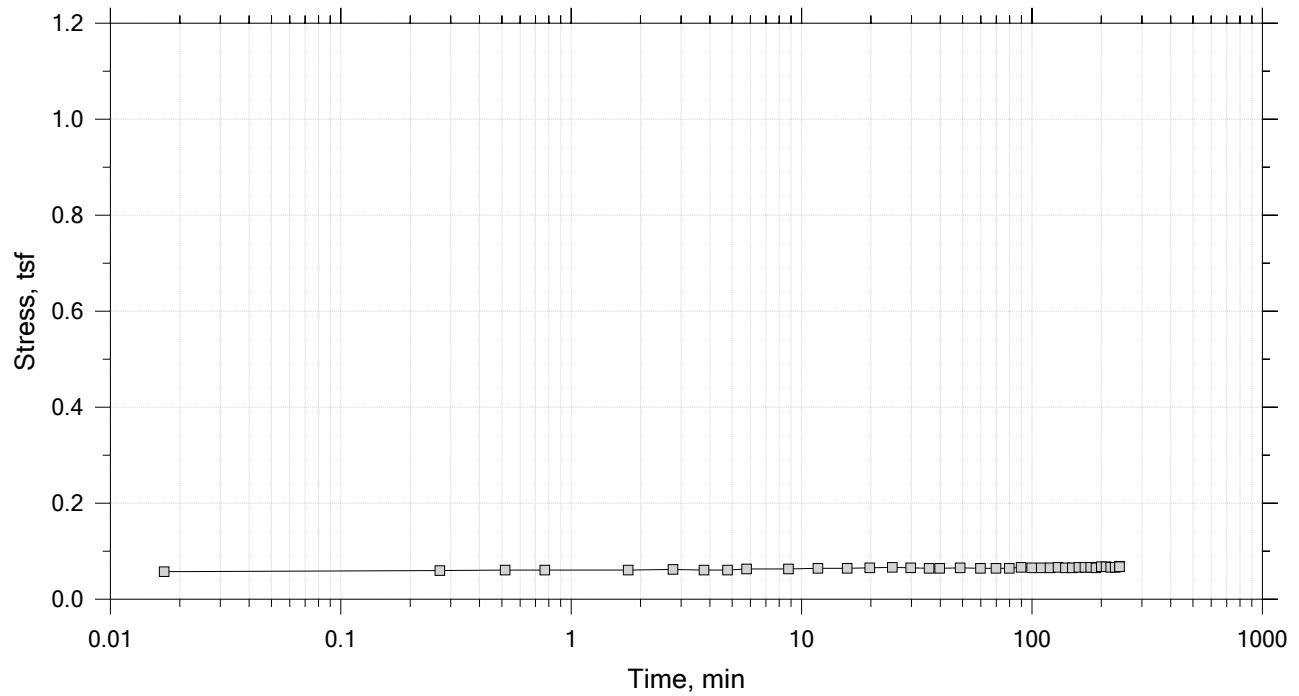
Summary Report




	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		
	Displacement at End of Increment		

One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 1 of 15
Constant Volume Step
Stress: 0.0676 tsf



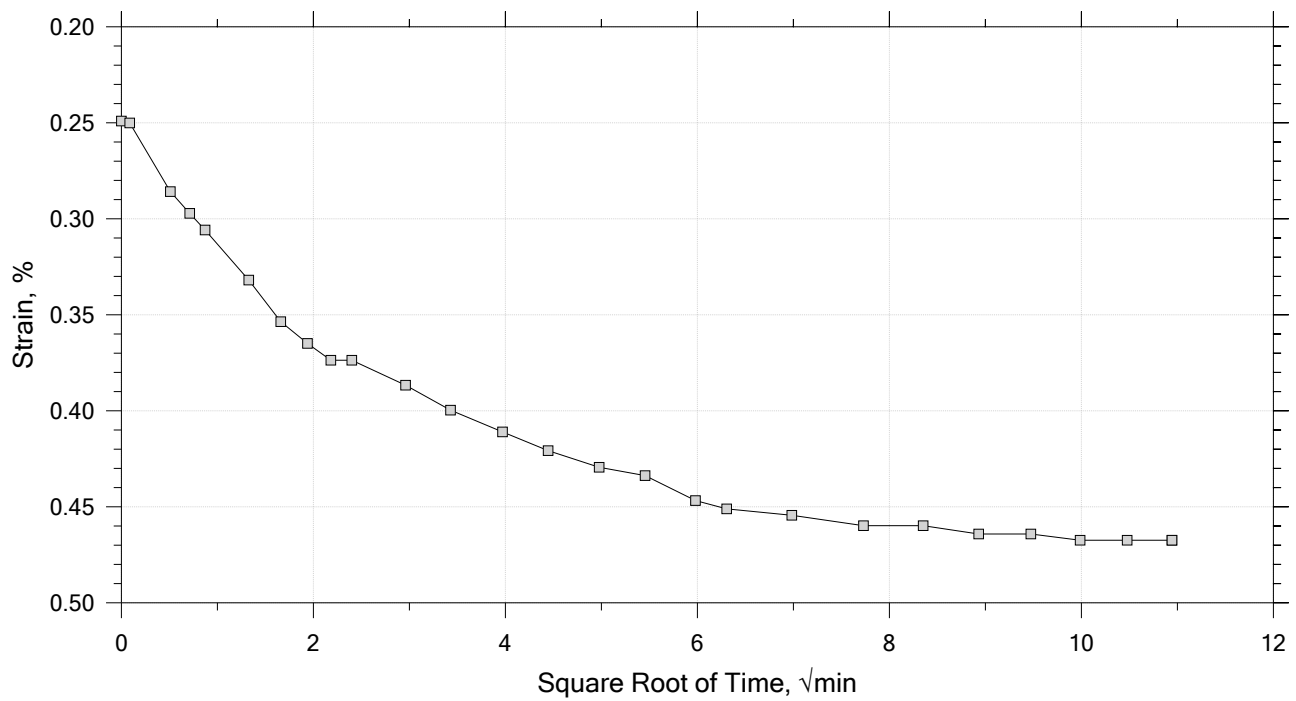
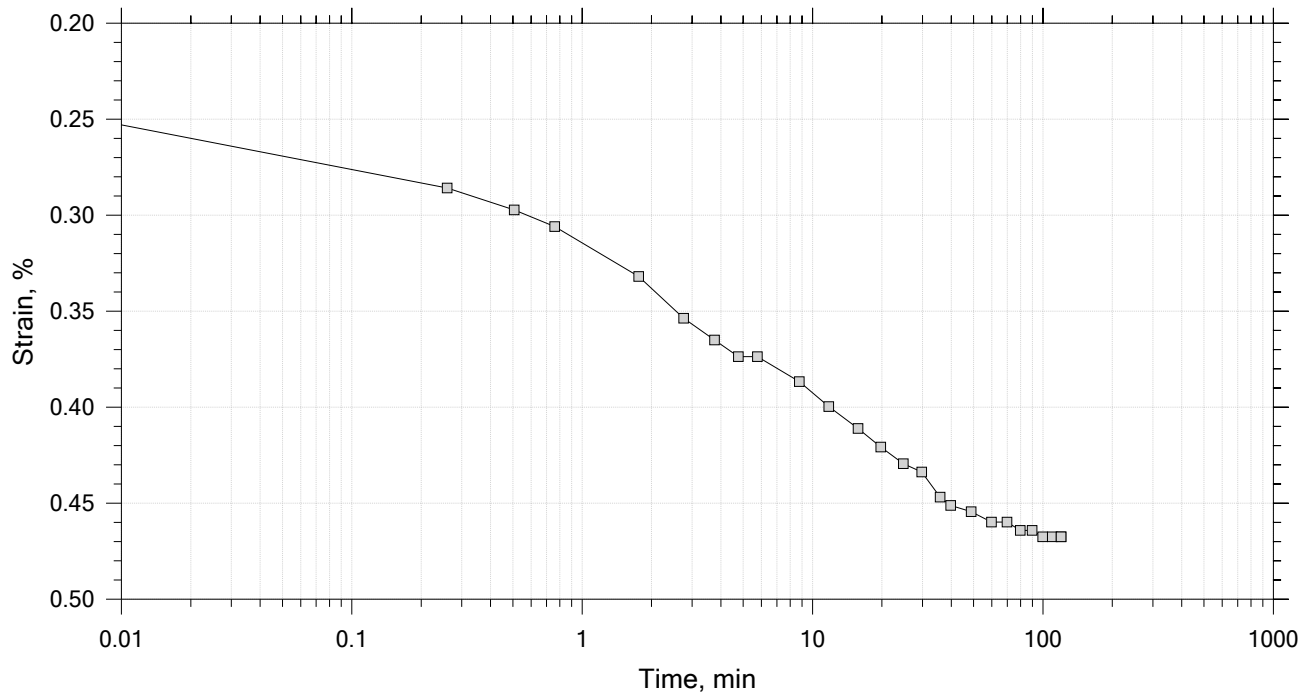
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 2 of 15

Constant Load Step

Stress: 0.125 tsf



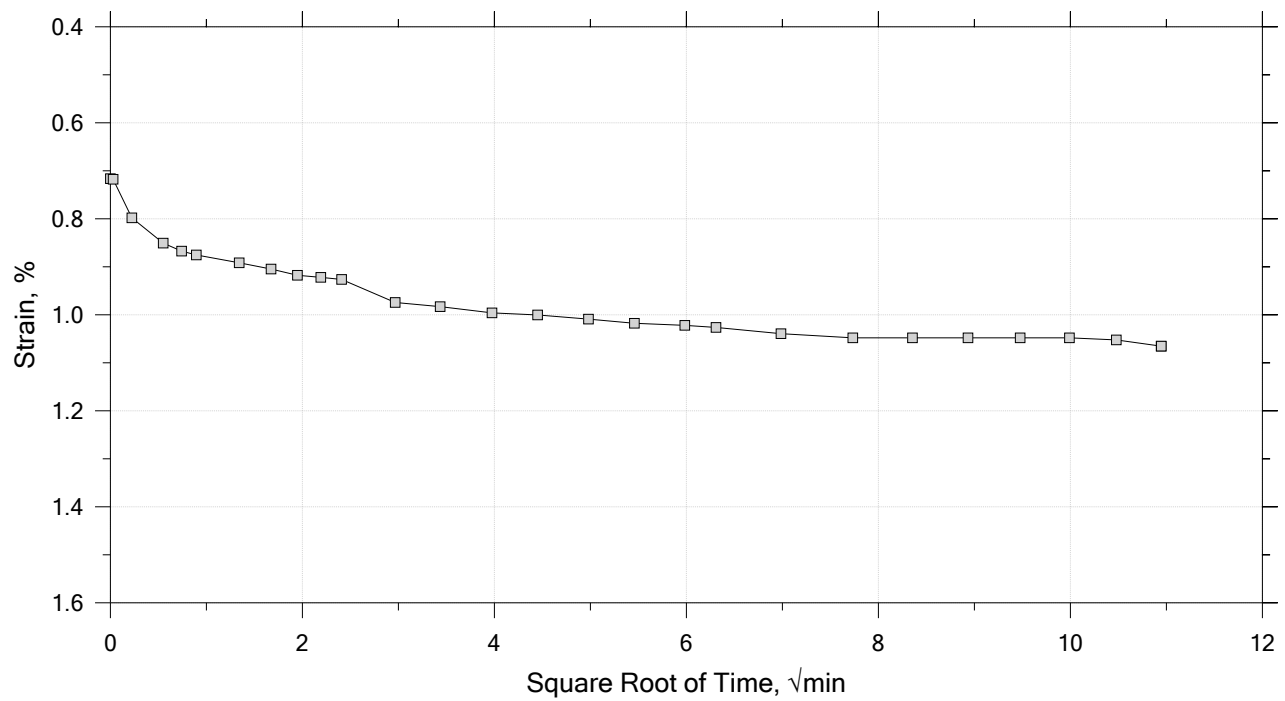
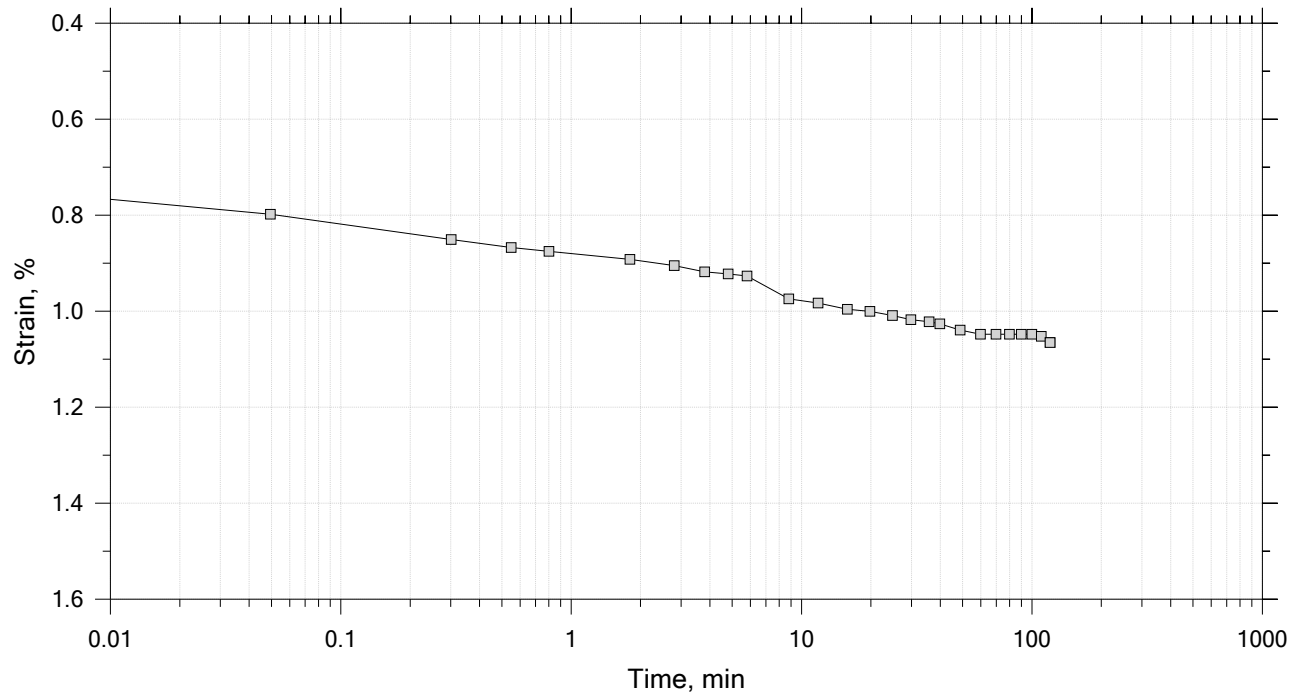
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 3 of 15

Constant Load Step

Stress: 0.25 tsf



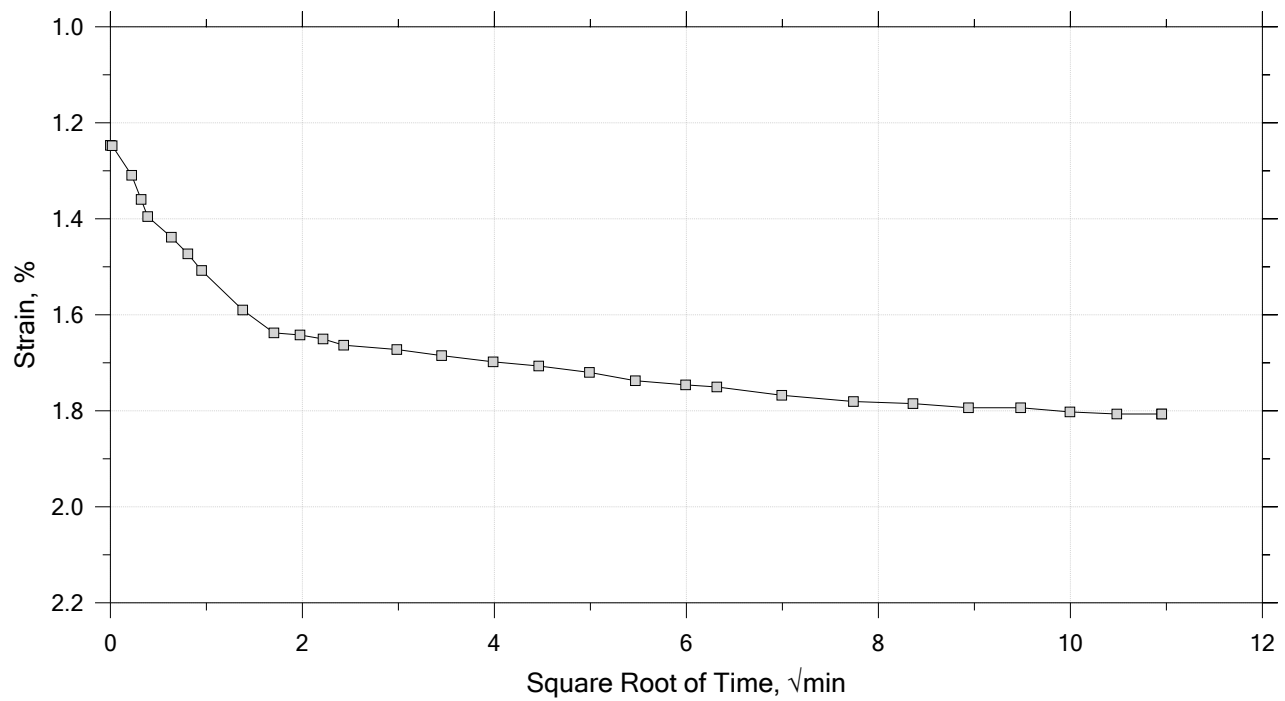
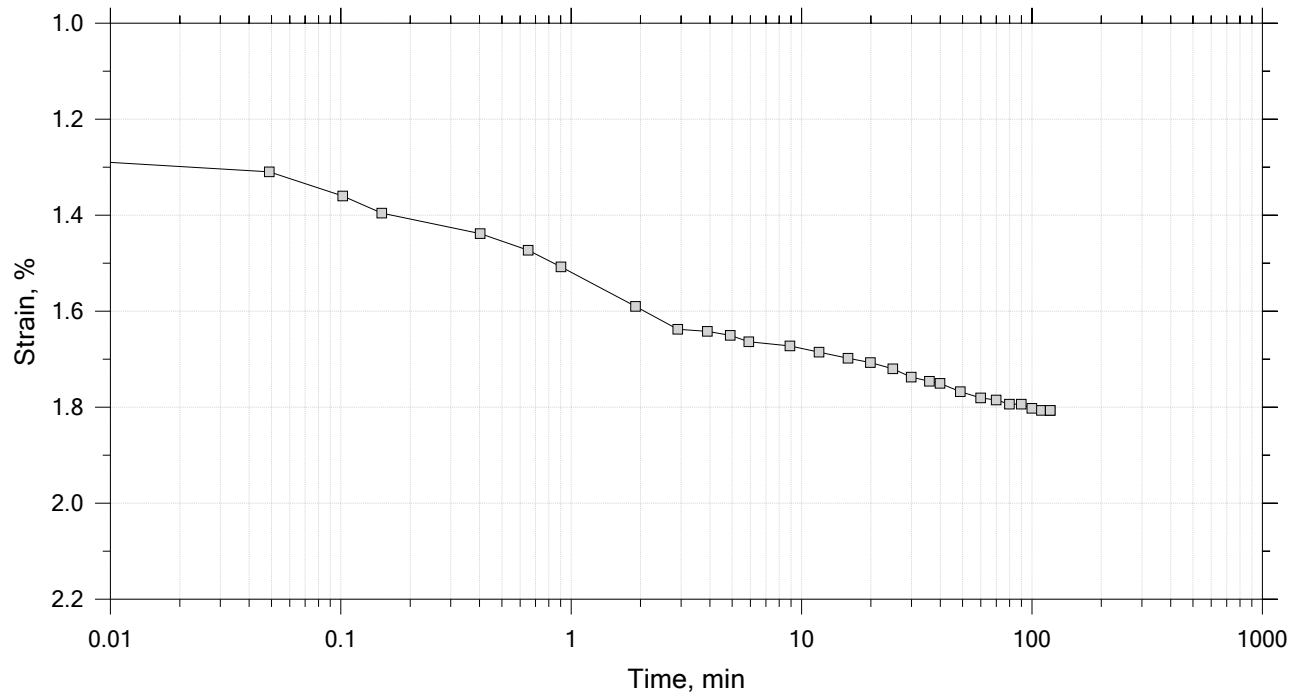
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 4 of 15

Constant Load Step

Stress: 0.5 tsf



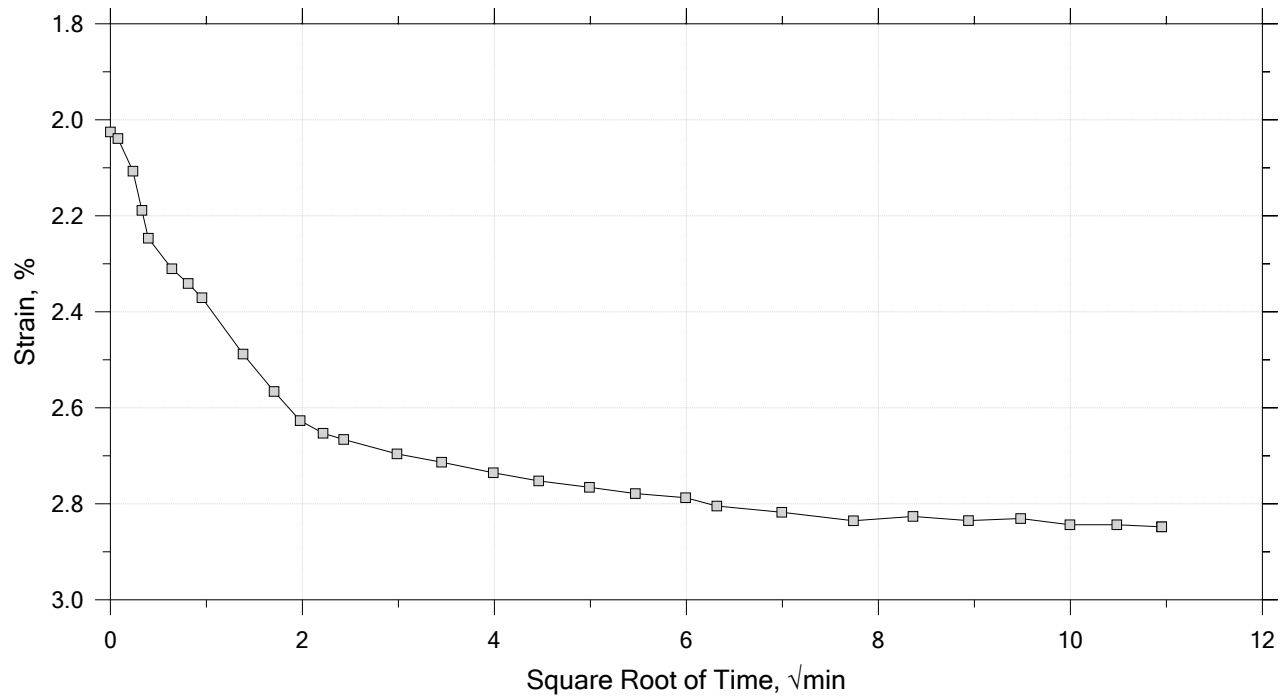
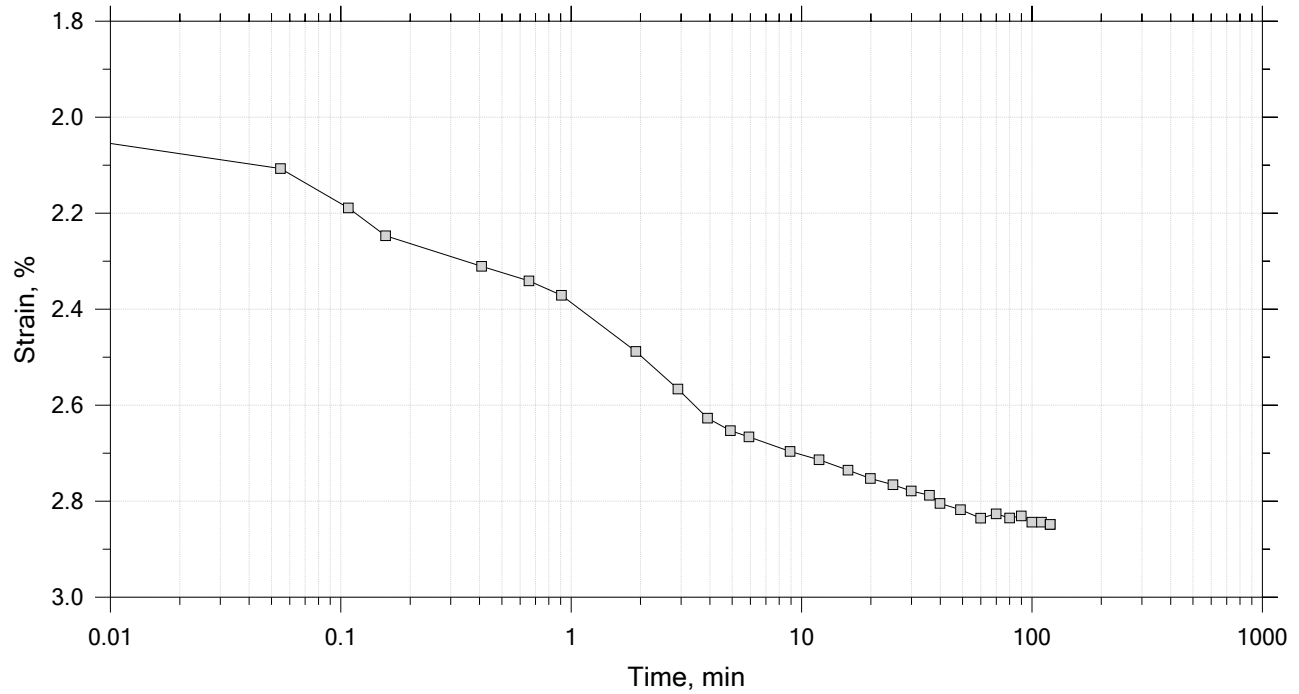
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 5 of 15

Constant Load Step

Stress: 1 tsf



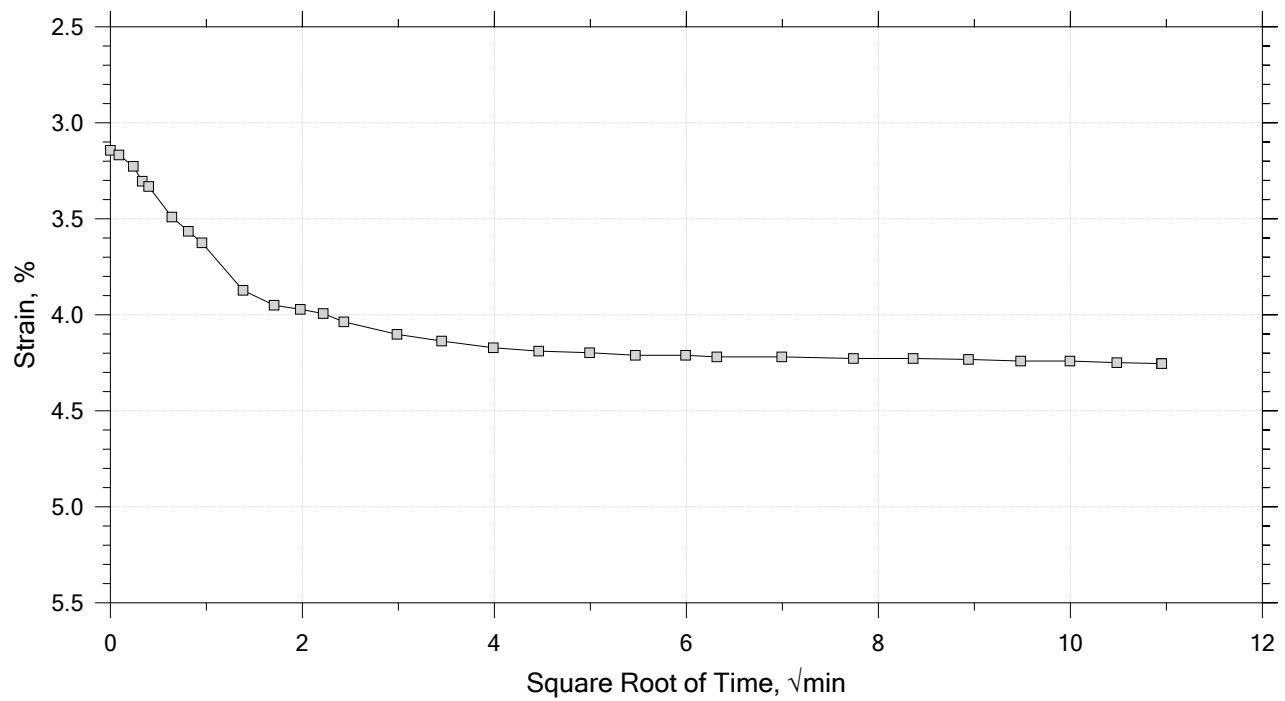
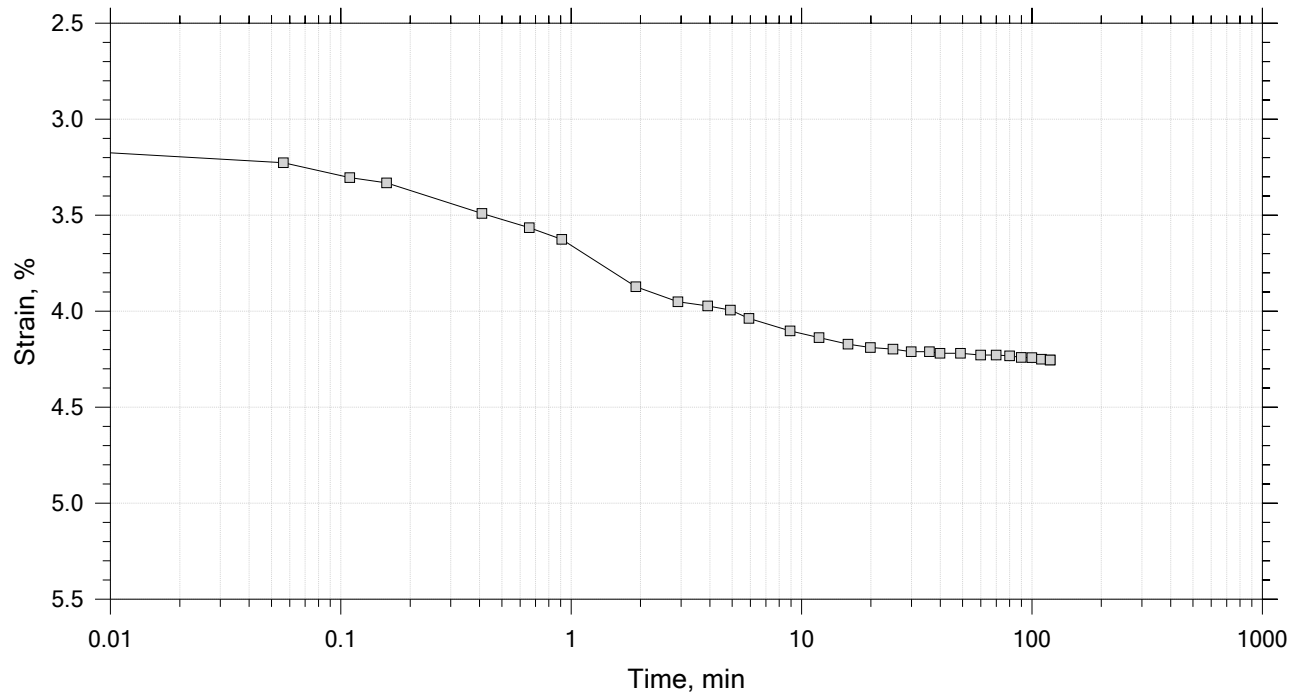
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 6 of 15

Constant Load Step

Stress: 2 tsf



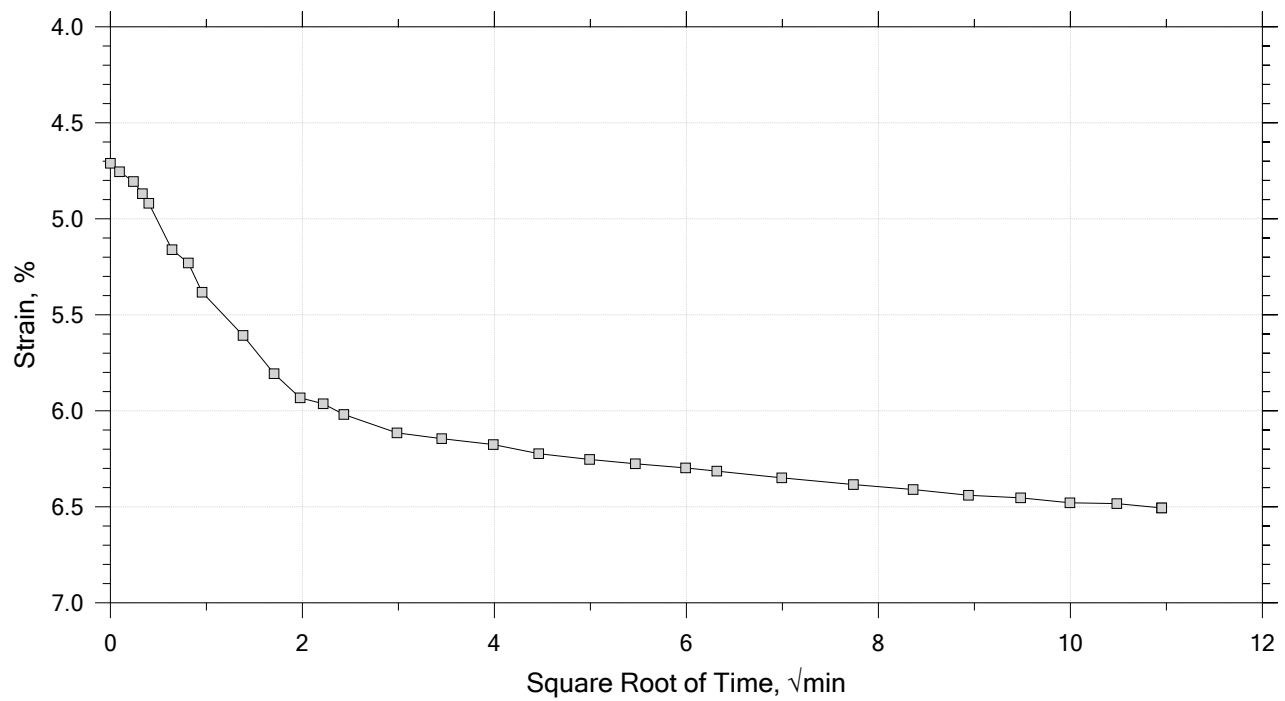
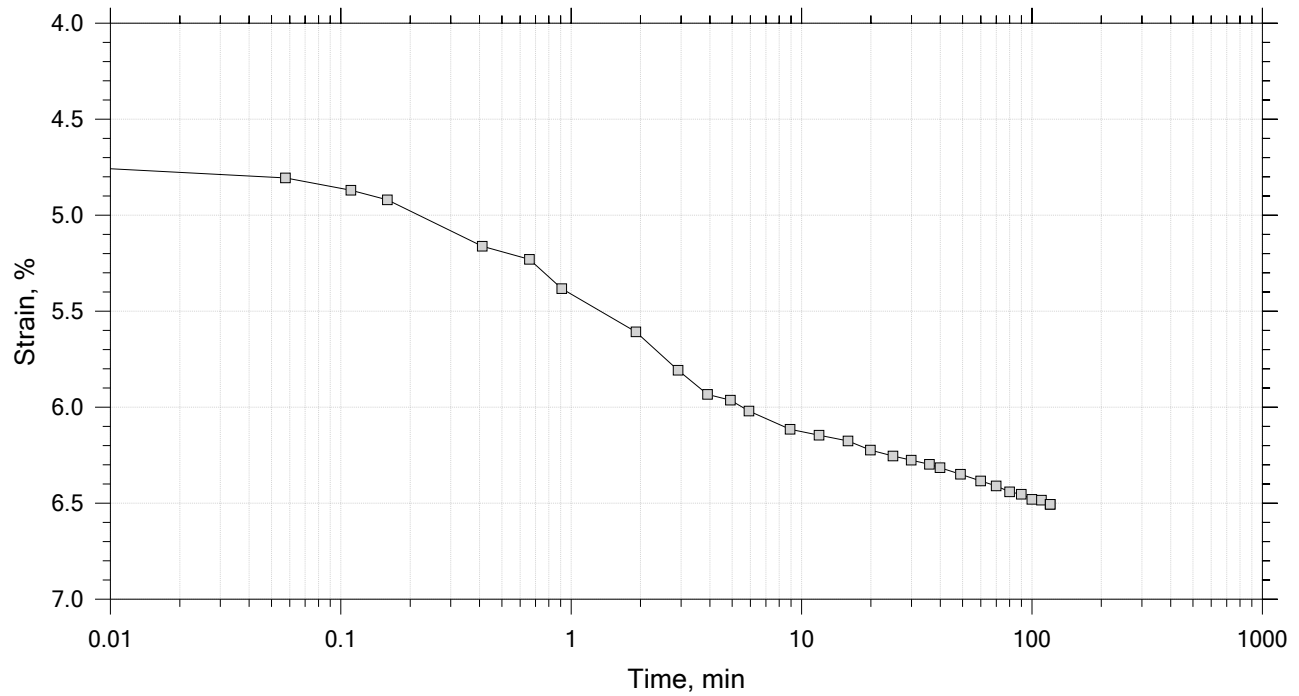
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 7 of 15

Constant Load Step

Stress: 4 tsf



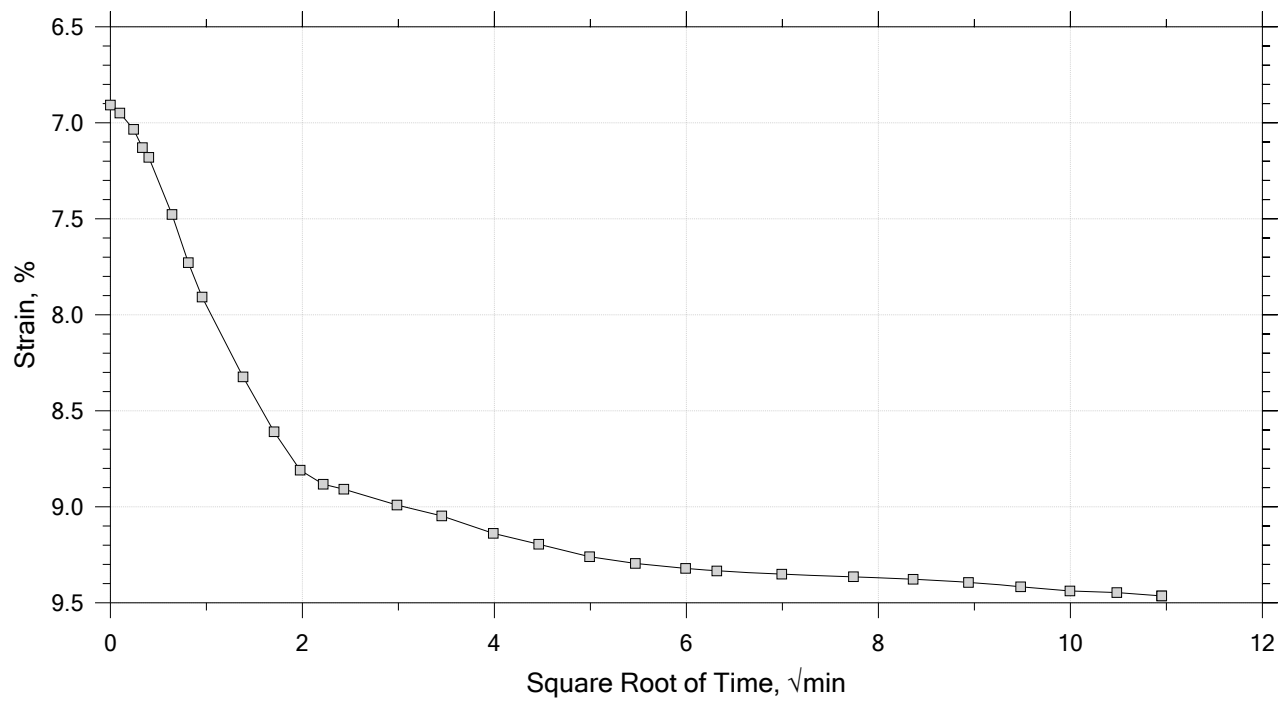
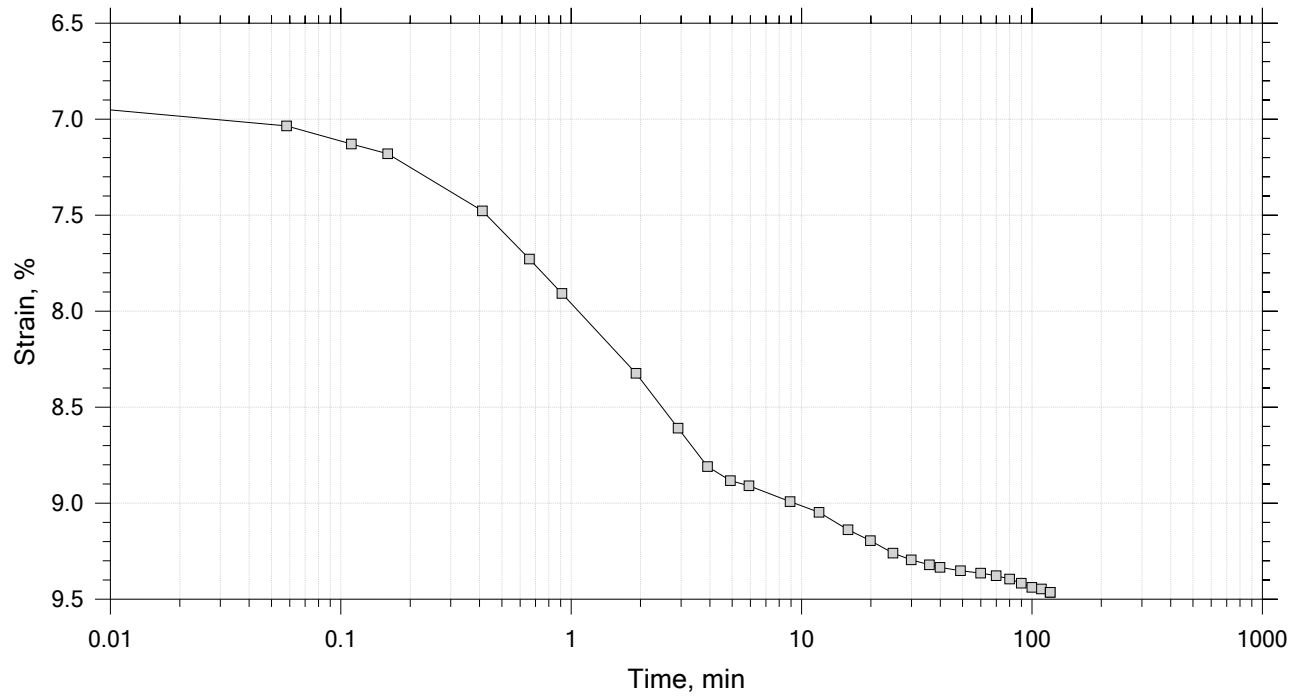
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 8 of 15

Constant Load Step

Stress: 8 tsf



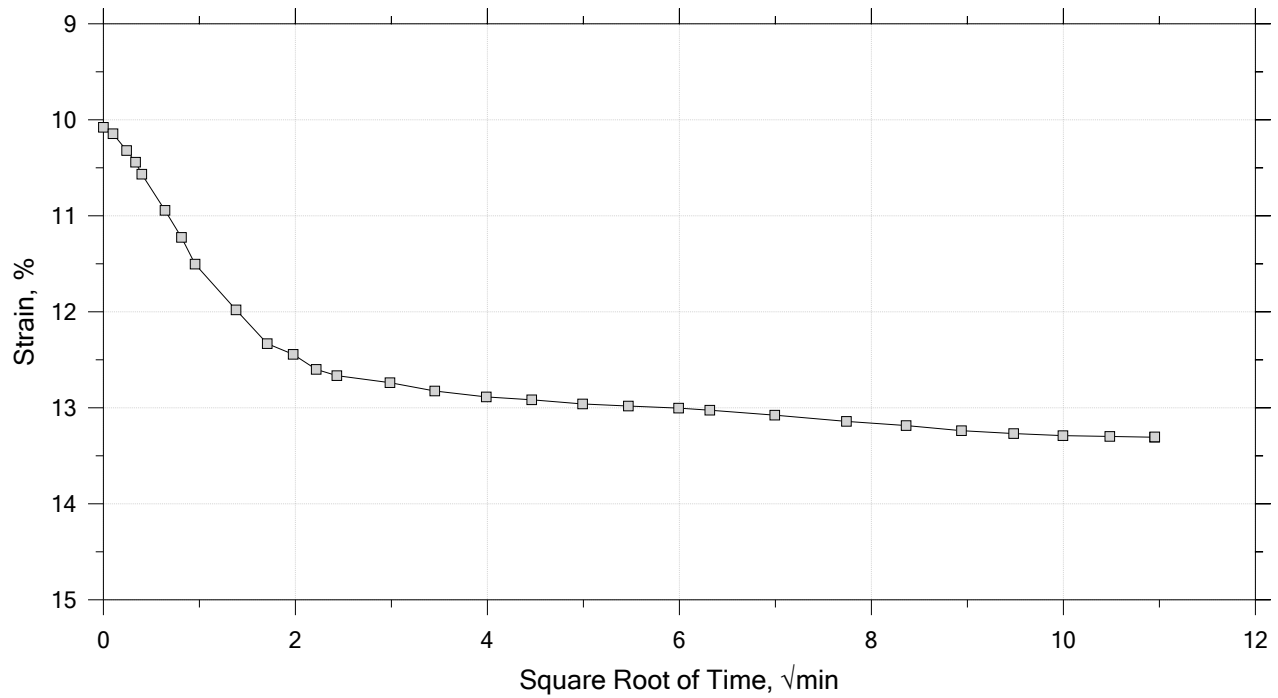
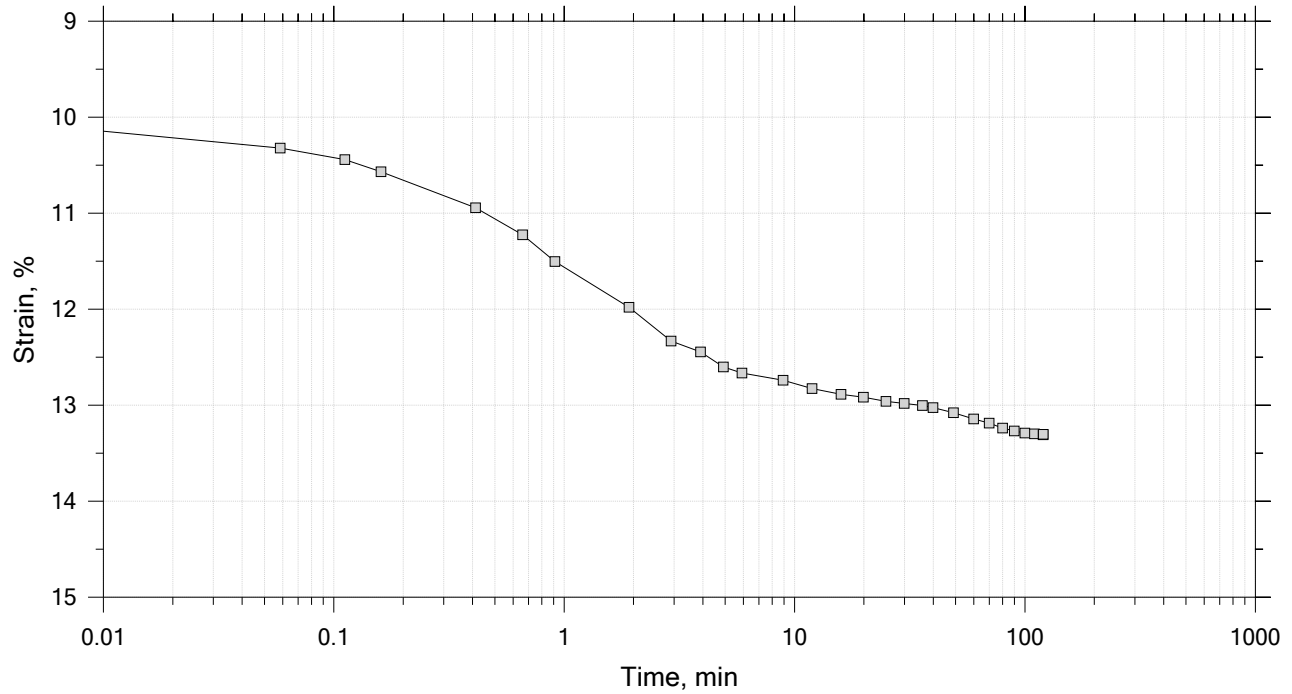
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 9 of 15

Constant Load Step

Stress: 16 tsf



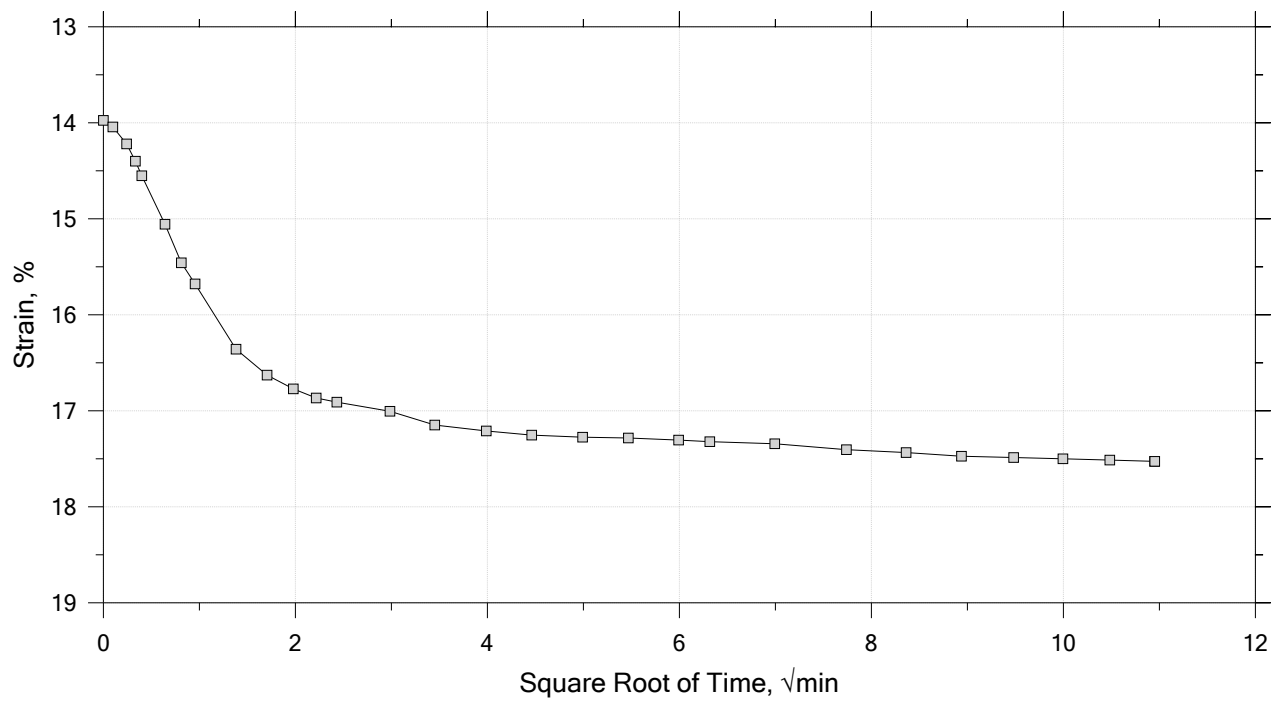
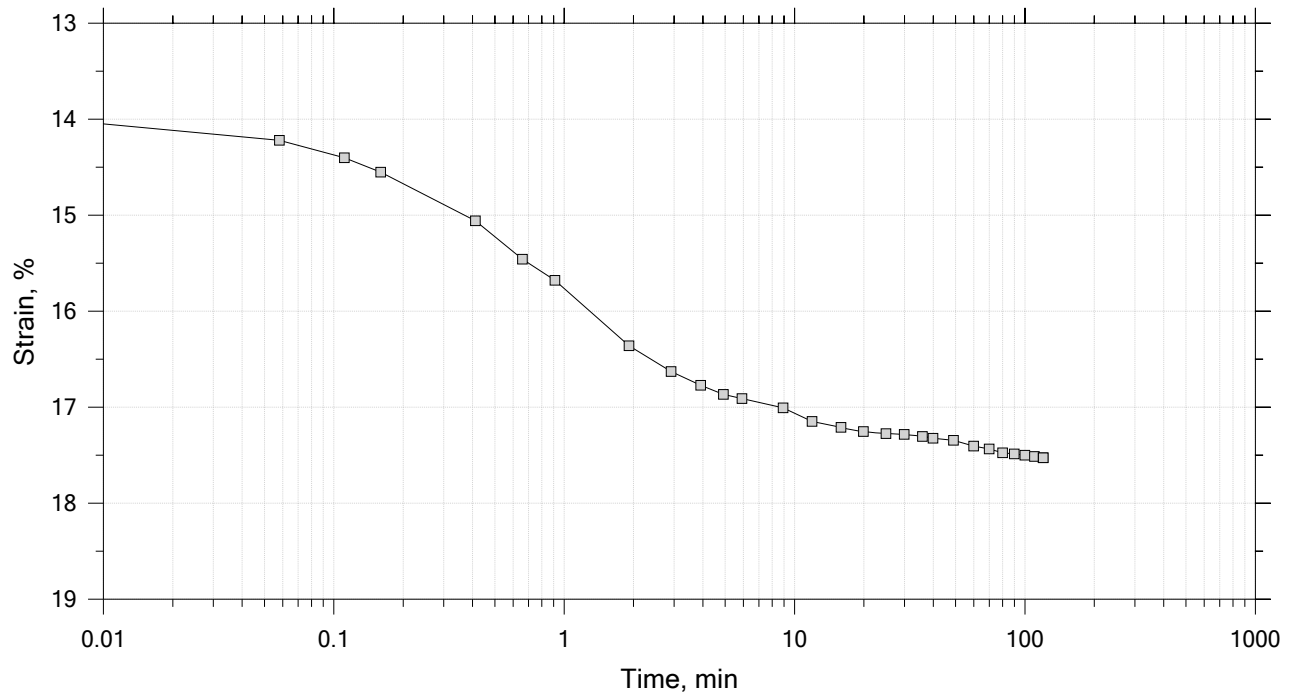
	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 10 of 15

Constant Load Step

Stress: 32 tsf



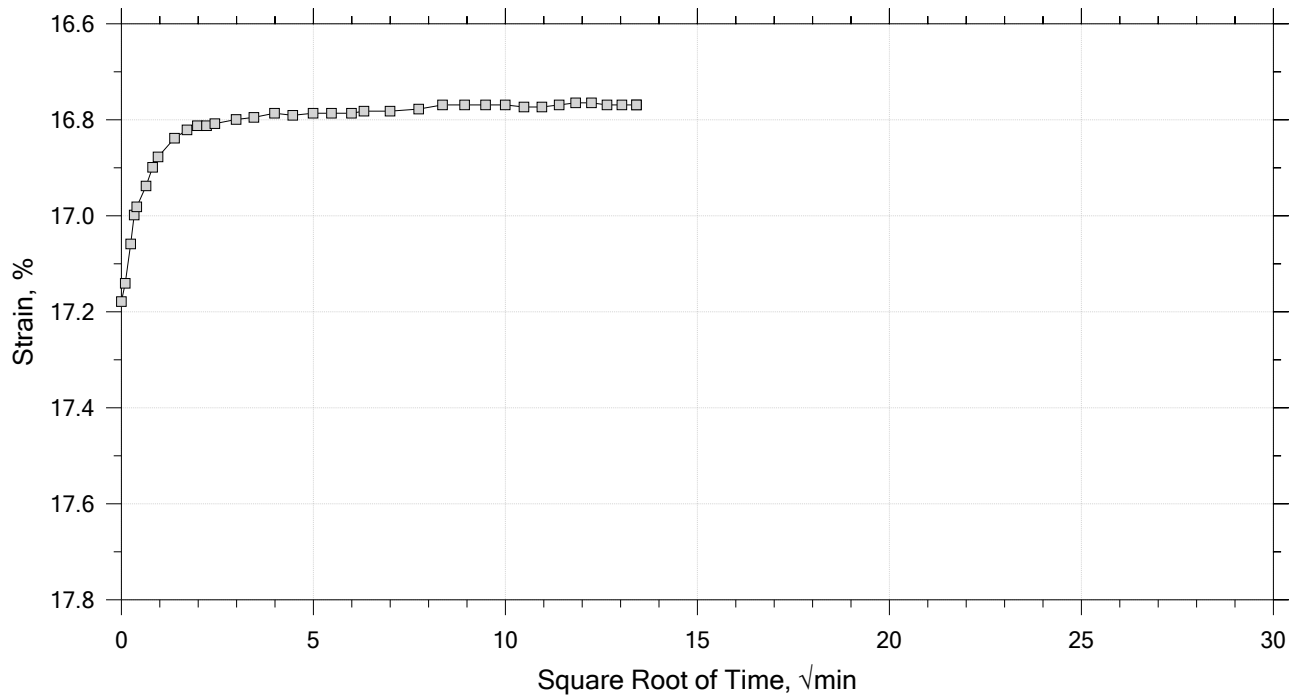
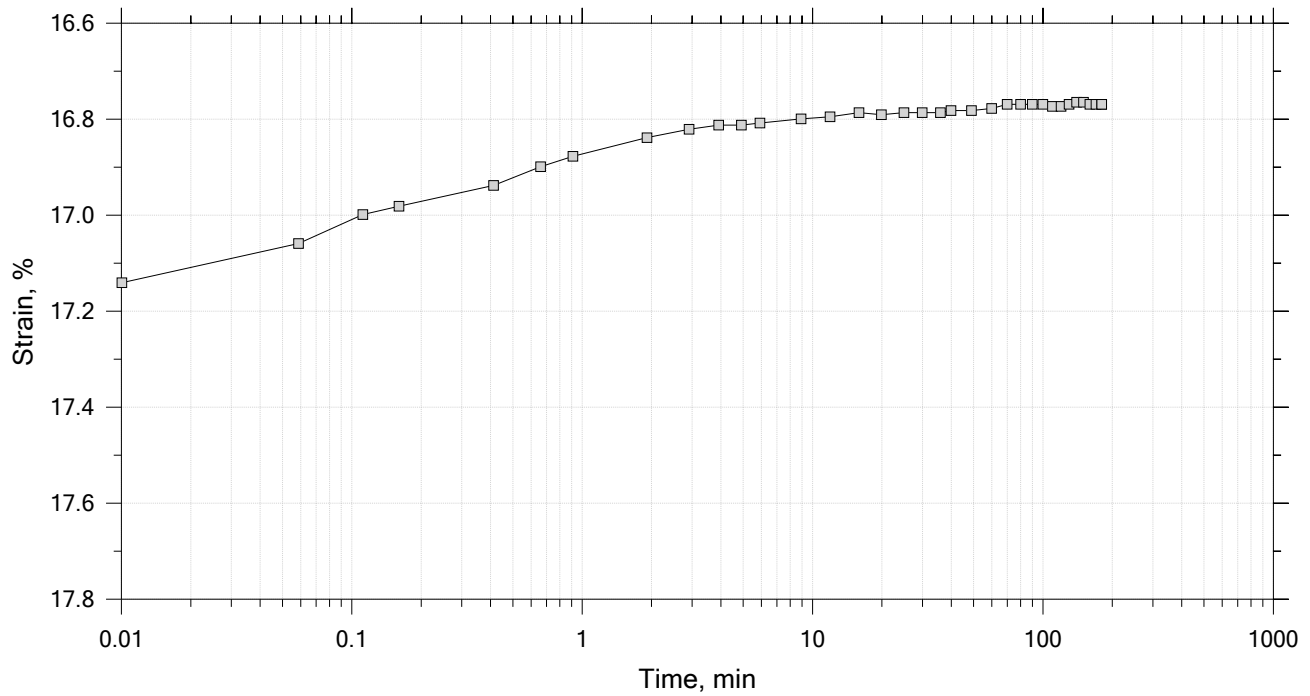
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	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		


One-Dimensional Consolidation by ASTM D2435 - Method B

Time Curve 11 of 15

Constant Load Step

Stress: 8 tsf



	Project: Rt-9/I-395 Connector	Location: Brewer and Eddington, ME	Project No.: GTX-308853
	Boring No.: HB-BE-138	Tested By: md	Checked By: mcm
	Sample No.: 1U	Test Date: 07/17/19	Depth: 8-10 ft
	Test No.: IP-13	Sample Type: intact	Elevation: ---
	Description: Moist, dark grayish olive clay		
	Remarks: System Y, Swell Pressure = 0.0676 tsf		